# 2615 Q ST

COUNTY OF SACRAMENTO, CA

## HYDROLOGIC AND HYDRAULIC STUDY

VERTICAL DATUM: NAVD88



Original Submittal: May 26, 2023 Revised:

Prepared For:

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### I. INTRODUCTION

This report presents the hydrologic, hydraulic and stormwater quality analyses for 2615 Q St (Project) by Thomas Peterson Trust, located in the Rio Linda, Sacramento County, California (See Appendix A – Vicinity Map). This report estimates storm flow hydrology as outlined in the *County of Sacramento Improvement Standards – Section 9* and the *City and County of Sacramento Drainage Manual – Volume 2 Hydrology Standards* (Reference 1, hereinafter referred to as the *Manual*. Additionally, this report presents Storm Water Quality measures prescribed in the *Stormwater Quality Design Manual for the Sacramento Region* (Reference 2, hereinafter referred to as the *SQDM*).

This report analyzes the Project's proposed hydrology, overland release routing, and stormwater quality characteristics including Low Impact Development (LID) requirements and hydromodification.

### II. PROJECT LOCATION AND DESCRIPTION

The Project encompasses 14.40± acres of mostly undeveloped land in Rio Linda, unincorporated Sacramento County. The site is located on the northwest corner of Q Street and 28<sup>th</sup> Street. An existing boat and RV storage borders the Project along the western boundary. Two existing single-family residences border the Project along the northern boundary. The eastern boundary of the Project is the centerline of 28<sup>th</sup> St. An existing single-family residences do the project adjacent to the 28<sup>th</sup> St along the Project boundary. The southern boundary of the Project is the centerline of Q St. An existing self-storage is located immediately south of Q St, adjacent to the project.

See Appendix A – Vicinity Map

The development is proposed as parking for boats and RV storage. The Project will construct paved parking and a bioretention basin for water quality. The Project's proposed improvements generally consist of the following:

- Paved driveway, drive aisles, and parking stalls (8.0± acres)
- Concrete curbs
- Chain-link fencing and gate
- Roadside ditch & culvert
- Bioretention Basin (31,600 SF)
- Storm drain inlet & pipe outfall
- Landscaping (36,911 SF)

### III. HYDROLOGY

### **METHODS**

The methodology contained in this study is in compliance with the procedures presented in the *Manual*. A hydrologic analysis was completed to estimate storm runoff from the proposed Project. The flow patterns for the undeveloped and developed conditions were determined from existing topography and the proposed grading plans. Both the Existing Shed Map and Developed Shed Map can be seen in Appendix D. An Offsite Shed Map has also been provided to identify the offsite sheds that are routed through the Project.

Existing and developed condition peak flow runoff for the Project site were estimated using the Sacramento Method, as prescribed by the *Manual*. The *Manual's* rainfall zone chart, Figure 2-11 show that the Project lies within Zone 2. This method was exercised using Sacramento County's Hydrologic Calculator (Sac-Calc).

### EXISTING SITE CONDITIONS AND HYDROLOGY

Previously stated, the Project site encompasses  $14.40\pm$  acres of undeveloped land and adjacent existing roadway improvements on Q Street and  $28^{th}$  Street (See Appendix C – Survey Map & Appendix D – Existing Shed Map). Native grasses on gently sloped terrain comprise the majority of the site's existing ground cover. Existing Conditions FEMA designation and Soil Group Classification are identified, as follows:

- FEMA National Flood Hazard Layer FIRMette Map, included in Appendix B, shows *The Project* is located within Non-Shaded Flood Zone "X" (Areas of Minimal Flood Hazard). There are no proposed building sites within a FEMA-designated Flood Zone or Special Flood Hazard Area (see Appendix B – FEMA Map).
- Onsite soils belong to the Hydrologic Soil Group "C" (Appendix C Soil Survey Map). An infiltration rate of 0.10 inches per hour for Open Space, Grassland, Ag (per Table 5-2 of the *Manual*) has been assumed for the site under existing conditions.

Existing drainage patterns are shown on the Existing Shed Map and Offsite Shed Map (Appendix D). Based on existing topography, three offsite sheds are routed through the Project Site which primarily sheet drains from south to north, to two points at the northern boundary. Runoff from the Project ultimately converges and discharges to a tributary of Dry Creek, near the north end of 26<sup>th</sup> Street, northwest from the Project Site. The existing sheds are more specifically described as follows:

SHED X1-1	Approximately one third of the existing undeveloped Site at 5.00± acres. Sheet
	drains from south toward the northern boundary. Runoff from Shed X1-1
	contributes to generated flow at Design Point #1.

SHED O1-1	Approximately 5.96± acres of offsite, existing paved boat and RV storage west of the Project. Sheet drains from southwest to northeast toward the western boundary of the Project. Runoff from Shed O1-1 converges with Shed X1-1 and contributes to generated flow at Design Point #1.
SHED X1-2	The majority of the undeveloped project site at approximately 9.40 acres. Also includes the northern half and western half of the existing Q Street and 28 <sup>th</sup> Street, respectively. Ruoff from 28 <sup>th</sup> Street is collected in a roadside ditch that widens out to a larger swale through the property to the northern boundary. Shed X1-2 contributes to generated flow at Design Point #2.
SHED O1-2	Approximately 31.31± acre offsite shed south of the Project Site. Mostly commercial/industrial uses (i.e. self-storage, boat & RV storage). Drains from south to north toward the southeast corner of the Project where it discharges to the upstream end of the roadside ditch along 28 <sup>th</sup> Street, converging with Shed X1-2. Shed O1-2 contributes to generated flow at Design Point #2.
SHED O2-2	Approximately 2.82± acre offsite shed east of the Project Site. Mostly undeveloped open space with a single-family residence at the northeast corner of Q Street and 28 <sup>th</sup> Street. Drains west along Q Street and north along 28 <sup>th</sup> Street where it discharges to the tail end of the roadside ditch on the west side of 28 <sup>th</sup> Street, converging with Shed X1-2. Shed O2-2 contributes to generated flow at Design Point #2.
SHED O3-2	Approximately 0.52± acre offsite shed northeast of the Project Site. Entirely undeveloped open space draining west where it converges with Shed X1-2. Shed O3-2 contributes to generated flow at Design Point #2.

Runoff estimates for the Project's existing conditions, Sacramento Method 10-yr and 100-yr storm events are provided in the Sac-Calc Sacramento Method Results (Existing Condition) in Appendix E. Table 1 below summarizes the Sac-Calc results for the existing condition runoff estimates.

TABLE 1EXISTING PEAK FLOW CHARACTERISTICS(Sacramento Method – 10 & 100-YEAR)							
Location         Q10 (cfs)         Q100 (cfs)							
Design Point #1	17.5	32					
[SHED X1-1 & O1-1]	17.5	32					
Design Point #2	77.4	137.9					
[SHEDS X1-2, O1-2, O2-2, O3-2]	//.4	137.9					

### **DEVELOPED SITE CONDITIONS & HYDROLOGY**

Under Developed Site Conditions, the Project will maintain the existing drainage patterns (See Appendix D – Developed Shed Map). The offsite sheds will continue to be routed through the site and the Project will continue to drain from south to north to the same two Design Points along the northern boundary. Beyond the project, drainage will continue on the same course to the Dry Creek Tributary as in existing condition.

The proposed site layout and finish grading were used to define the Developed Drainage Sheds. The Project site was divided into four onsite sheds (A-1, A-2, B-2, & C-2). As a result of the proposed improvements, a portion of the area draining to Design Point #1 in existing condition is shifted and contributes to flows at Design Point #2. This is due to the paved surface that is directed to the bioretention basin for treatment, which discharges to Shed C-2. The Developed Sheds are more specifically described as follows:

SHED A-1	Reduction of Shed X1-1 to 1.84± acres for the paving in the developed condition. Runoff from Shed A-1 contributes to generated flow at Design Point #1.
SHED A-2	0.29± acres including a portion of existing Q Street improvements. Drainage is collected in a ditch and routed to Shed C-2. Runoff from Shed A-2 contributes to generated flow at Design Point #2.
SHED B-2	The largest onsite shed (9.08± acres) in the developed condition. Consists of the entire new paved surface and the bioretention basin. Runoff from the paved surface sheet flows south to north and is conveyed to the bioretention basin by concrete curbs as the northern end of the paved surface. An underdrain in the bioretention basin conveys flow to the drain inlet and outfall pipe out to Shed C-2. Runoff from Shed B-2 contributes to generated flow at Design Point #2.
SHED C-2	3.18± acres including a portion of Q Street and 28 <sup>th</sup> Street existing road improvements. Runoff is collected in roadside ditch and conveyed to the existing onsite swale adjacent to the proposed bioretention basin. Runoff from Shed C-2 contributes to generated flow at Design Point #2.
SHEDS 01-1, 01-2, 02-2, & 03-2	Same as existing offsite sheds. Shed O1-1 converges with Shed A-1 and contributes to generated flow at Design Point #1. Sheds O1-2, O2-2, & O3-2 converge with Shed C-2 and contribute to generated flow at Design Point #2.

Sac-Calc was also used to estimate runoff for the Developed Condition, Sacramento 10-yr and 100-yr storm events for the purposes of comparing to flows in the Existing Condition. The flow estimates are provided in the Sac-Calc Sacramento Method Results (Developed Condition) Appendix E. Table 2 below also summarizes the developed condition runoff estimates.

TABLE 2							
PROPOSED PEAK FLOW CHARACTERISTICS							
(Sacramento Method – 10 & 100-YEAR)							
Location         Q10 (cfs)         Q100 (cfs)							
Design Point #1	14.3	25.7					
[SHEDS A-1 & O1-1]	14.5	23.7					
Design Point #2	91.4	158.8					
[SHEDS A-2, B-2, C-2, O1-2, O2-2, O3-2]	91.4	130.0					

With respect to the proposed peak flow runoff rates, shown in Table 2, above, please note the following:

- 1. The above listed peak flow rates represent estimated unmitigated runoff based on proposed developed conditions.
- 2. Mitigation of peak flow rates (detention) is not proposed at this time.
  - a. Onsite flows are quite low relative to the offsite flows directed through the site (approximately 16% of total in existing condition, approximately 25% of total in developed condition).
  - b. Drainage leaving the Project discharges directly to a tributary of Dry Creek approximately 1,500 ft away. From that point, Dry Creek is approximately 2,000 ft away.
  - c. Some attenuation is provided with the proposed bioretention basin and the design to meet hydromodification requirement (See Section V below).

### IV. OVERLAND RELEASE ROUTES

As outlined in Sections 9-1 and 9-15 of the *Sacramento County Improvement Standards*, the 100yr water surface elevation must allow for 1.2-ft of freeboard to the existing building pad elevations. For underground storm drain not designed to convey the 100-year runoff or in the event the storm drain system is inoperable, the streets are to safely route stormwater. To confirm that these requirements are met throughout, a ponding exhibit was prepared to model critical points where localized ponding might occur during the 100-yr storm event. The overland release routes have been analyzed and the results are compiled in Appendix F – Overland Release Exhibit.

### V. PROPOSED LID, STORMWATER QUALITY, & HYDROMODIFICATION

### LID

The Project addresses Low Impact Development (LID) measures required by the County of Sacramento for commercial projects. Per the *SQDM* the goal of LID measures is "to mimic a site's predevelopment balance of runoff and infiltration by using design techniques that infiltrate, store, evaporate, and detain runoff close to its source."

To mitigate increased stormwater runoff from developed areas, the Project proposes the following LID measures:

- Common Drainage Plan Open Space (Landscaped Areas including Trees and Vegetation)
- Interceptor Trees (Proposed)
- Bio-Retention Basin

In order to determine the effectiveness of the proposed LID measures, the County's spreadsheet for Commercial Sites: Low Impact Development (LID) Credits was utilized (See Appendix G). The Project's proposed pervious and impervious surfaces were calculated for Shed B-1 as it is the only shed with an increase in impervious area. Credits for the landscaped area and interceptor trees shown on the Project's Landscape Plan have not been applied to this shed as the landscaping is within Sheds A-2 & C-2 (Appendix D – Developed Shed Map). Drainage Shed B-1 and the corresponding bioretention basin are shown on the Stormwater Management Plan in Appendix G.

### Stormwater Quality Treatment

The LID Credits spreadsheet provided in Appendix G shows that the Project's proposed LID features meet the required Water Quality Treatment for the site's design runoff.

### **Hydromodification**

The Project's proposed bioretention basin provides hydromodification under developed conditions by regulating the outflow through the outlet structure and accounting for infiltration. A perforated underdrain (4-inch dia.) is proposed beneath the basin's compost and bioretention soil mix layers that connects to the riser outlet structure. Flows larger than the underdrain capacity pond within the basin until reaching the top of the riser structure. The outlet structure is proposed as a 24-inch flat-top drain inlet.

The Sacramento Area Hydrology Model (SAHM) software was utilized to confirm the basin's adequacy in meeting hydromodification requirements per the *SQDM*. The SAHM-generated report for the Project is provided in Appendix G.

### VI. CONCLUSIONS

Based on the following, the 2615 Q St Project design meets or exceeds drainage performance standards as required by the County of Sacramento.

- 1. The methods used to calculate storm runoff for this project are in compliance with the *Manual*.
- 2. The proposed Project lies outside of a FEMA-designated Flood Zone or Special Flood Hazard Area.
- 3. Generated storm runoff from impervious surfaces shall be routed through bioretention basins providing permanent BMP facilities, LID measures, and Hydromodification, prior to discharging offsite.
- 4. Storm flows generated from offsite sheds will be conveyed through the project site as they do in the existing condition.
- 5. Developed Conditions The need to mitigate the increase in onsite Peak Runoff flows (10-yr & 100-yr) is negated by the following:
  - a. The onsite peak flows are minimal in comparison to the combined flows leaving the site in both the existing and developed conditions.
  - b. Flows discharge directly to a tributary of Dry Creek, with Dry Creek and the tributary both near the Project site.
  - c. Some attenuation is provided by the bioretention basin (not quantified except as done so by the SAHM for hydromodification).

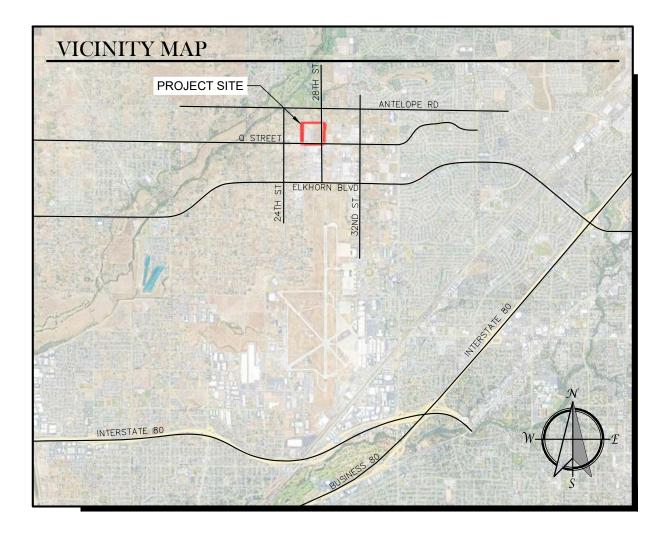
### VII. REFERENCES

- 1. County of Sacramento. *County of Sacramento Improvement Standards Section 9.* April 1, 2018
- 2. Sacramento Region. *Stormwater Quality Design Manual for the Sacramento Region*. July 2018.
- 3. City/County of Sacramento. *Drainage Manual Volume 2 Hydrology Standards*. December 1996.

### APPENDIX A

### VICINITY MAP

Vicinity Map



### APPENDIX B

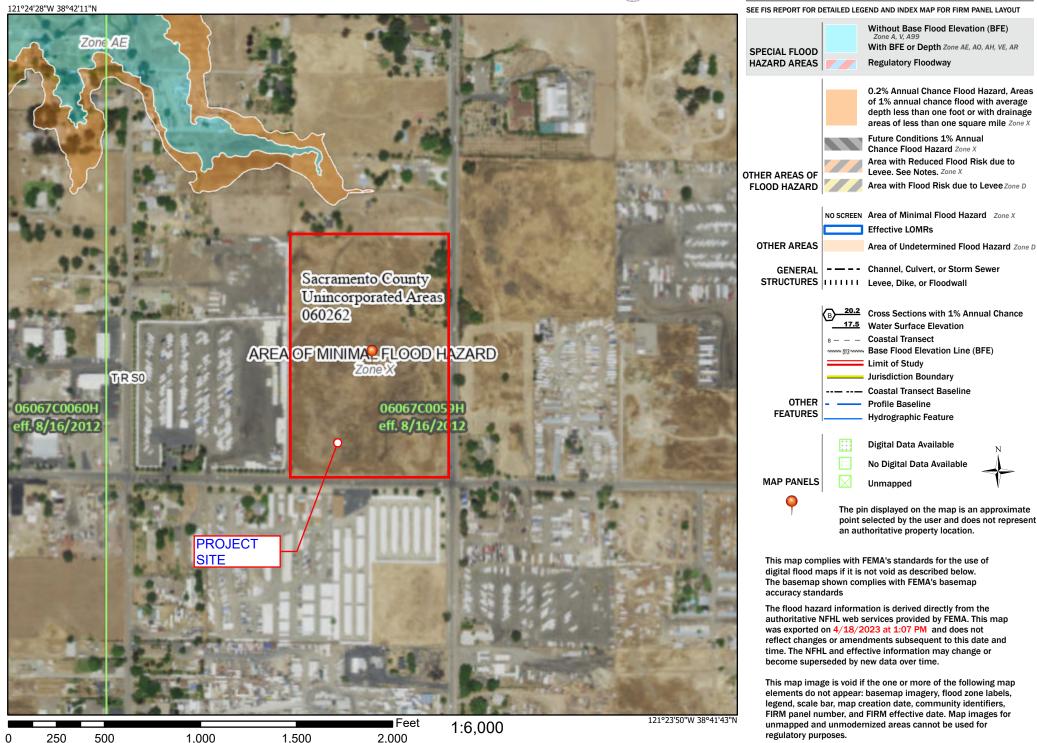
### FEMA MAP

FEMA Map

# National Flood Hazard Layer FIRMette



### Legend



Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

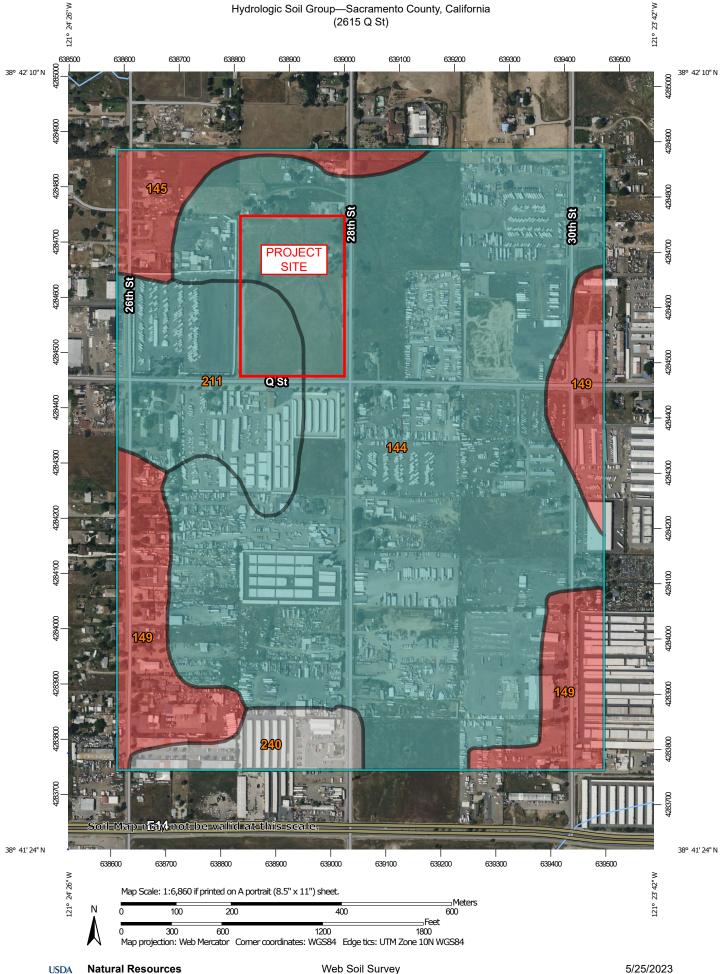
### APPENDIX C

### EXISTING CONDITIONS

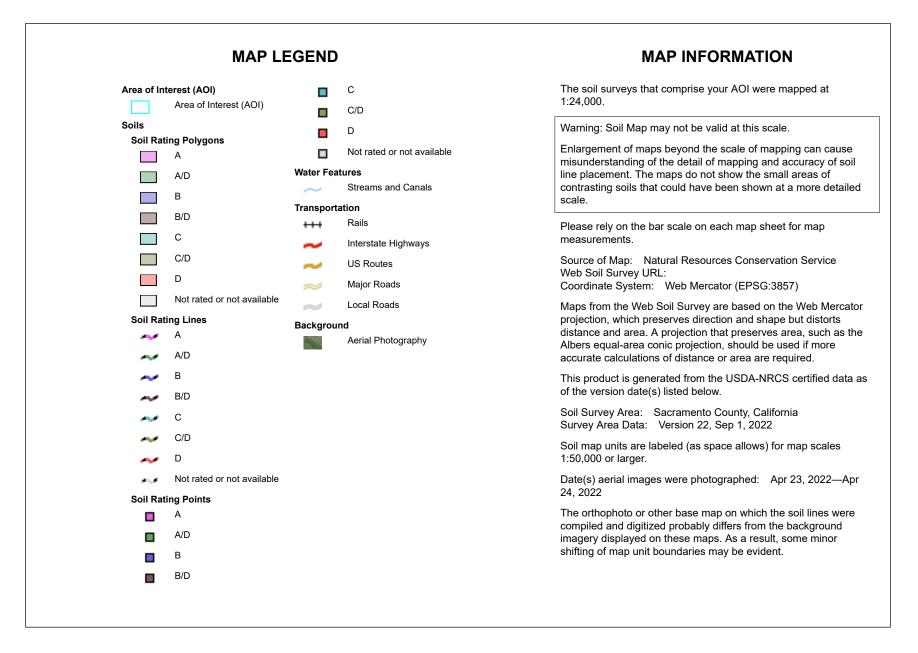
**EXISTING CONDITIONS – TOPO MAP** 

SOIL SURVEY MAP





Web Soil Survey National Cooperative Soil Survey



## Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI		
144	Fiddyment fine sandy loam, 0 to 1 percent slopes	С	164.9	66.8%		
145	Fiddyment fine sandy loam, 1 to 8 percent slopes	D	10.0	4.1%		
149	Fiddyment-Urban land complex, 1 to 8 percent slopes	D	34.5	14.0%		
211	San Joaquin fine sandy loam, 0 to 3 percent slopes	С	28.8	11.7%		
240	Xerarents-Urban land- San Joaquin complex, 0 to 5 percent slopes		8.4	3.4%		
Totals for Area of Inter	rest	•	246.8	100.0%		

### Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

### **Rating Options**

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

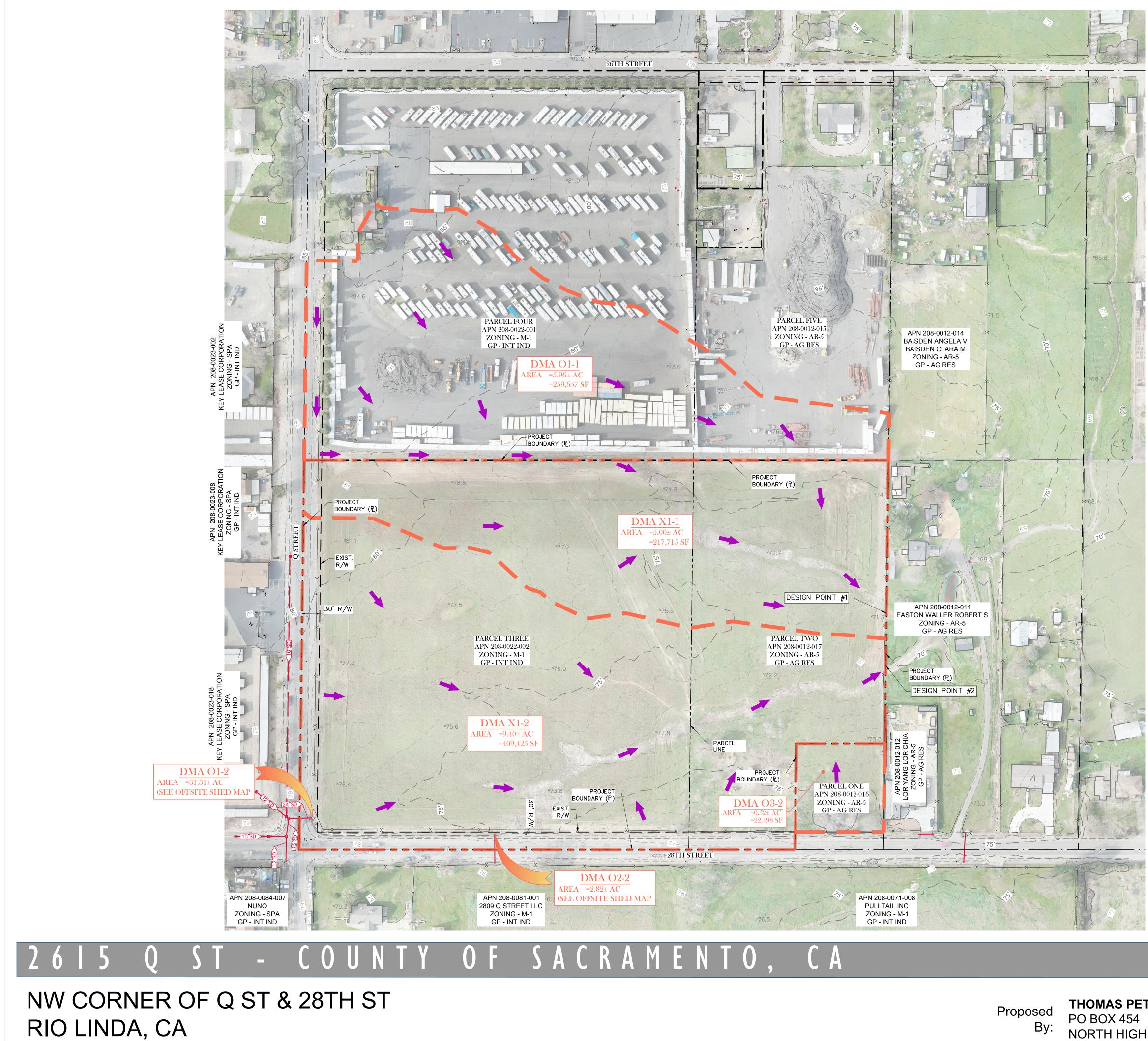
### APPENDIX D

### SHED MAPS

EXISTING SHED MAP

**OFFSITE SHED MAP** 

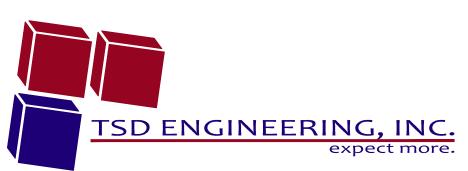
DEVELOPED SHED MAP

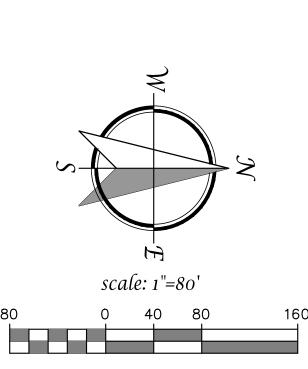


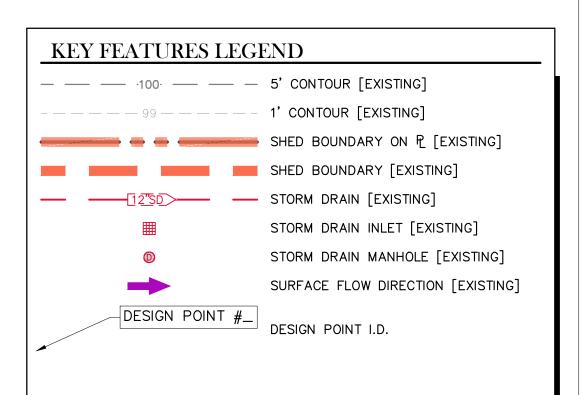
THOMAS PETERSON TRUST NORTH HIGHLANDS, CA 95660

MAY 24, 2023 INITIAL SUBMITTAL

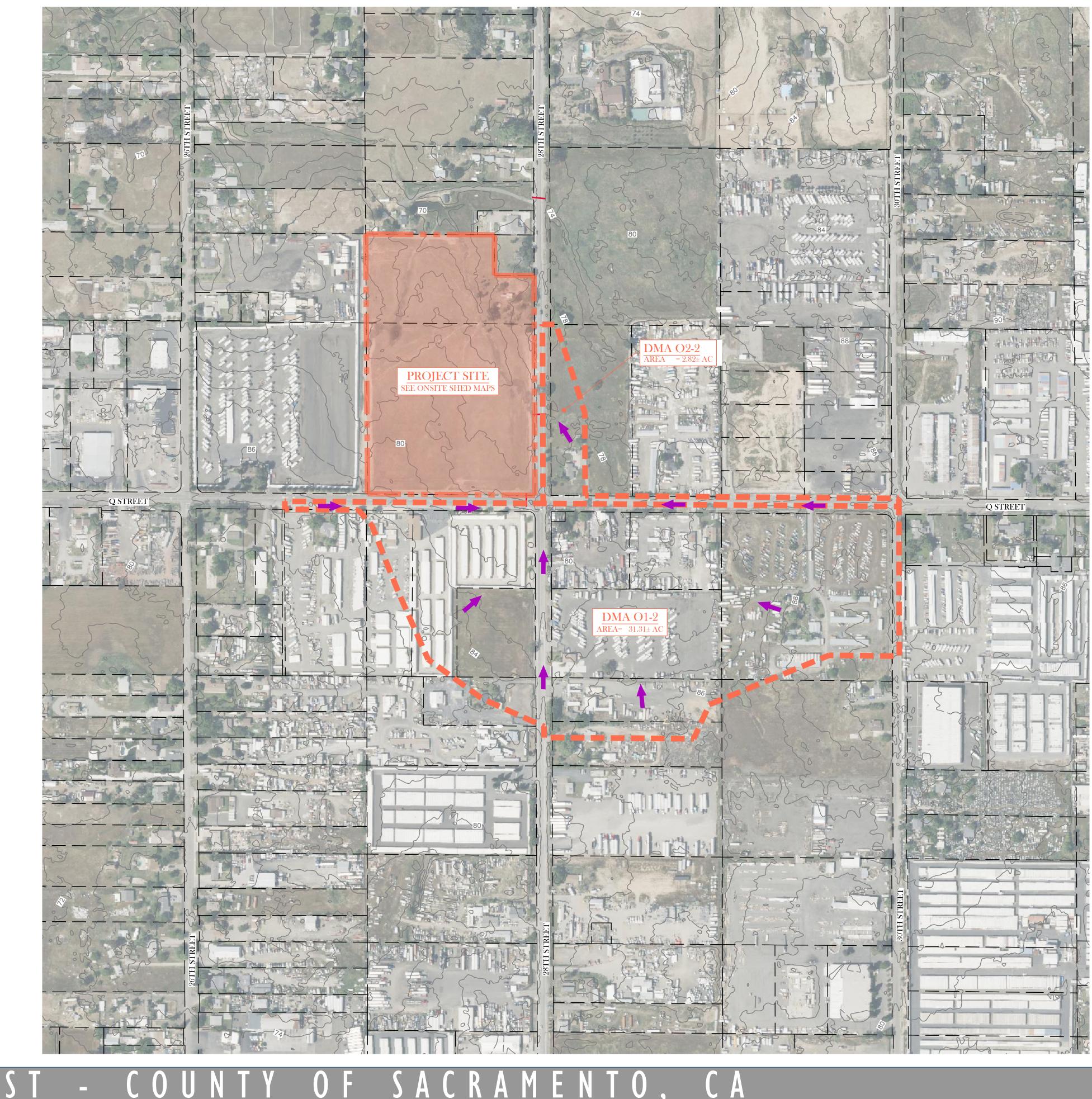
785 Orchard Drive, Suite #110 Folsom, CA 95630 Phone: (916) 608-0707 Fax: (916) 608-0701







EXISTING SHED MAP



# NW CORNER OF Q ST & 28TH ST RIO LINDA, CA

# OF SACRAMENTO, CA

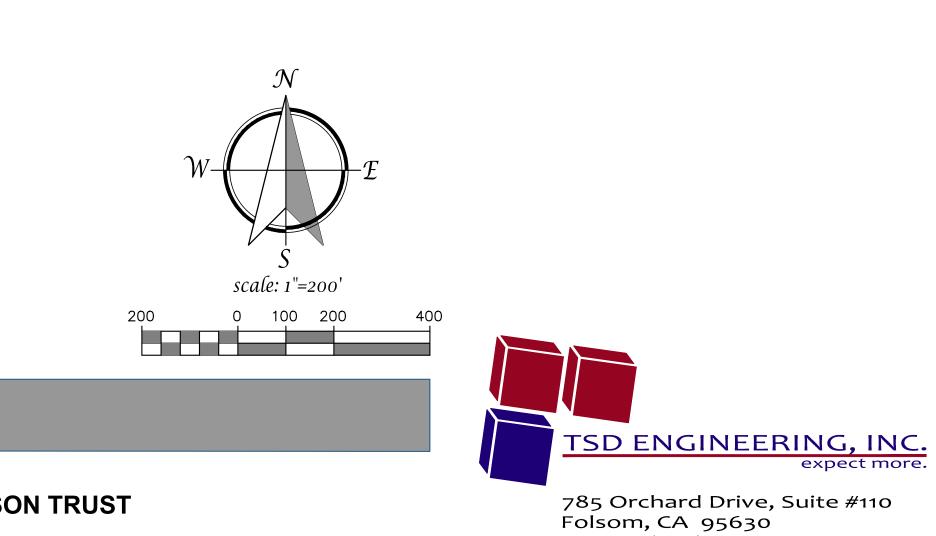
Proposed By:

THOMAS PETERSON TRUST PO BOX 454 NORTH HIGHLANDS, CA 95660

# OFFSITE SHED MAP

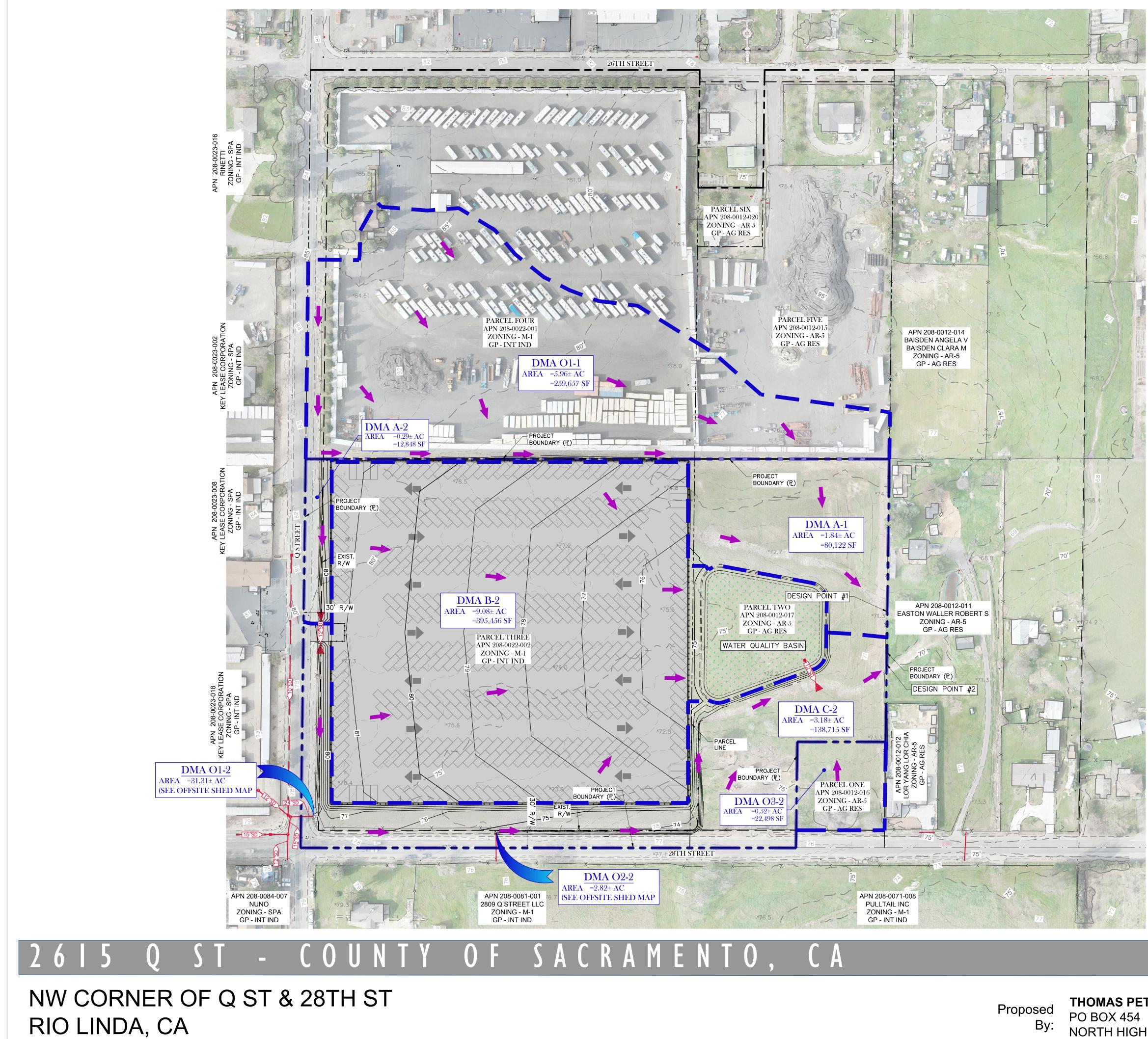
## KEY FEATURES LEGEND

- - ·100· - 2' CONTOUR [EXISTING] PROJECT BOUNDARY
- SHED BOUNDARY [EXISTING]
  - SURFACE FLOW DIRECTION [EXISTING]



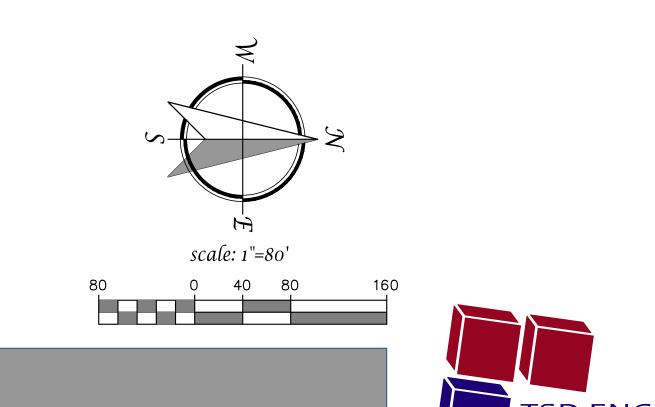
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# **DEVELOPED SHED MAP**

## **KEY FEATURES LEGEND** — — — ·100· — — — 5' CONTOUR [EXISTING] 1' CONTOUR [EXISTING] 5' CONTOUR [PROPOSED] 1' CONTOUR [PROPOSED] SHED BOUNDARY ON P. [EXISTING] SHED BOUNDARY [EXISTING] — STORM DRAIN [EXISTING] STORM DRAIN [PROPOSED] STORM DRAIN INLET [EXISTING] STORM DRAIN MANHOLE [EXISTING] STORM DRAIN INLET [PROPOSED] SURFACE FLOW DIRECTION [EXISTING] -DESIGN POINT #\_ DESIGN POINT I.D.



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### APPENDIX E

### HYDROLOGY

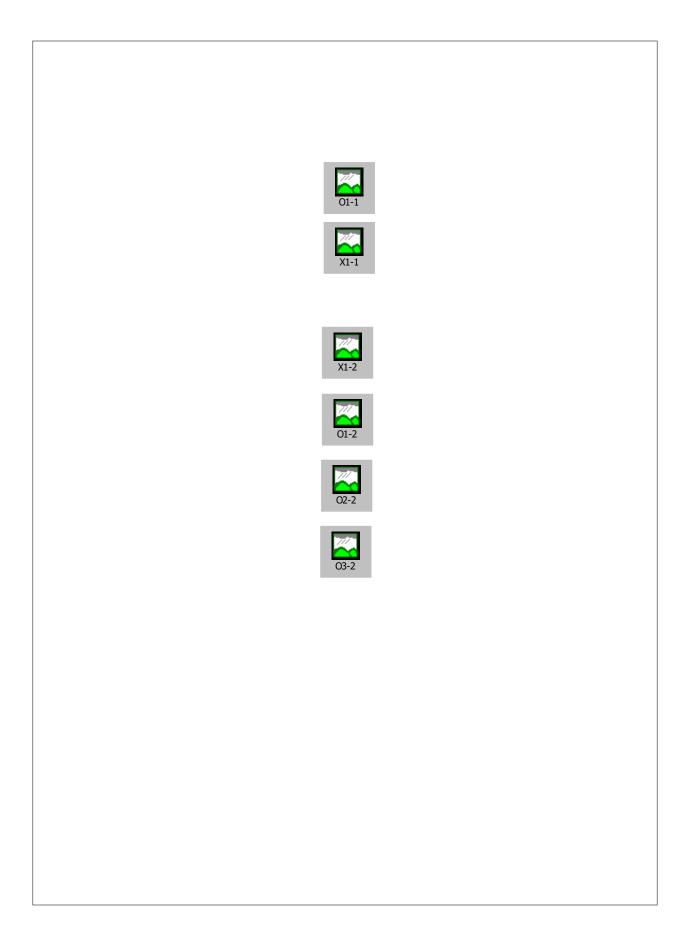
<u>SacCalc – Sacramento Method</u>

Shed Layout – Existing Condition

Peak Flow Results – Existing Condition

Shed Layout – Developed Condition

Peak Flow Results – Developed Condition

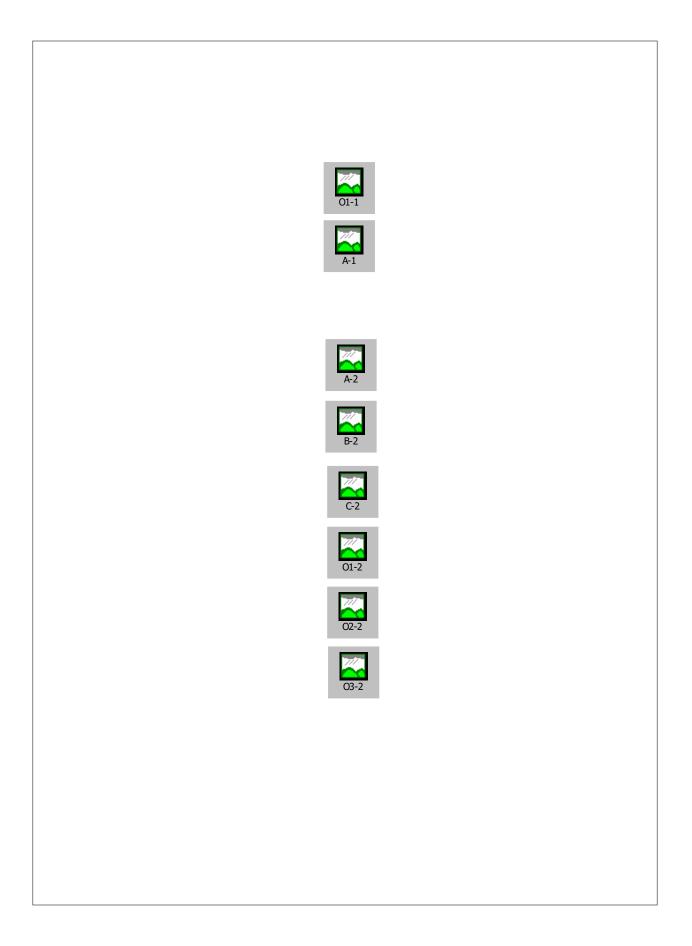


### View HEC-1 output

	(Project: 2615 Q St (Existing Condition)) (100-year, 1-day rainfall)							
Peak flowTime of peakBasin areaPeak 								
01-1	21.	12:02	.01					
X1-1	11.	12:09	.01					
X1-2	20.	12:10	.01					
03-2	1.2	12:08	.00					
01-2	111.	12:02	.05					
O2-2	5.7	12:12	.00					

# Sacramento method results

(10-year, 1-day rainfall)							
ID	Peak flow (cfs)	Time of peak (hours)	Basin area (sq. mi)	Peak stage (feet)	Peak storage (ac-ft)	Diversion volume (ac-ft)	
01-1	12.	12:02	.01				
X1-1	5.5	12:12	.01				
X1-2	9.9	12:14	.01				
O3-2	.6	12:11	.00				
01-2	64.	12:02	.05				
O2-2	2.9	12:16	.00				



### View HEC-1 output

	(Project: 2615 Q St (Developed Condition)) (100-year, 1-day rainfall)							
ID	Peak flow (cfs)	Time of peak (hours)	Basin area (sq. mi)	Peak stage (feet)	Peak storage (ac-ft)	Diversion volume (ac-ft)		
01-1	21.	12:02	.01					
A-1	4.7	12:06	.00					
A-2	.7	12:07	.00					
03-2	1.2	12:08	.00					
01-2	111.	12:02	.05					
O2-2	5.7	12:12	.00					
B-2	32.	12:02	.01					
C-2	8.2	12:06	.00					

# Sacramento method results

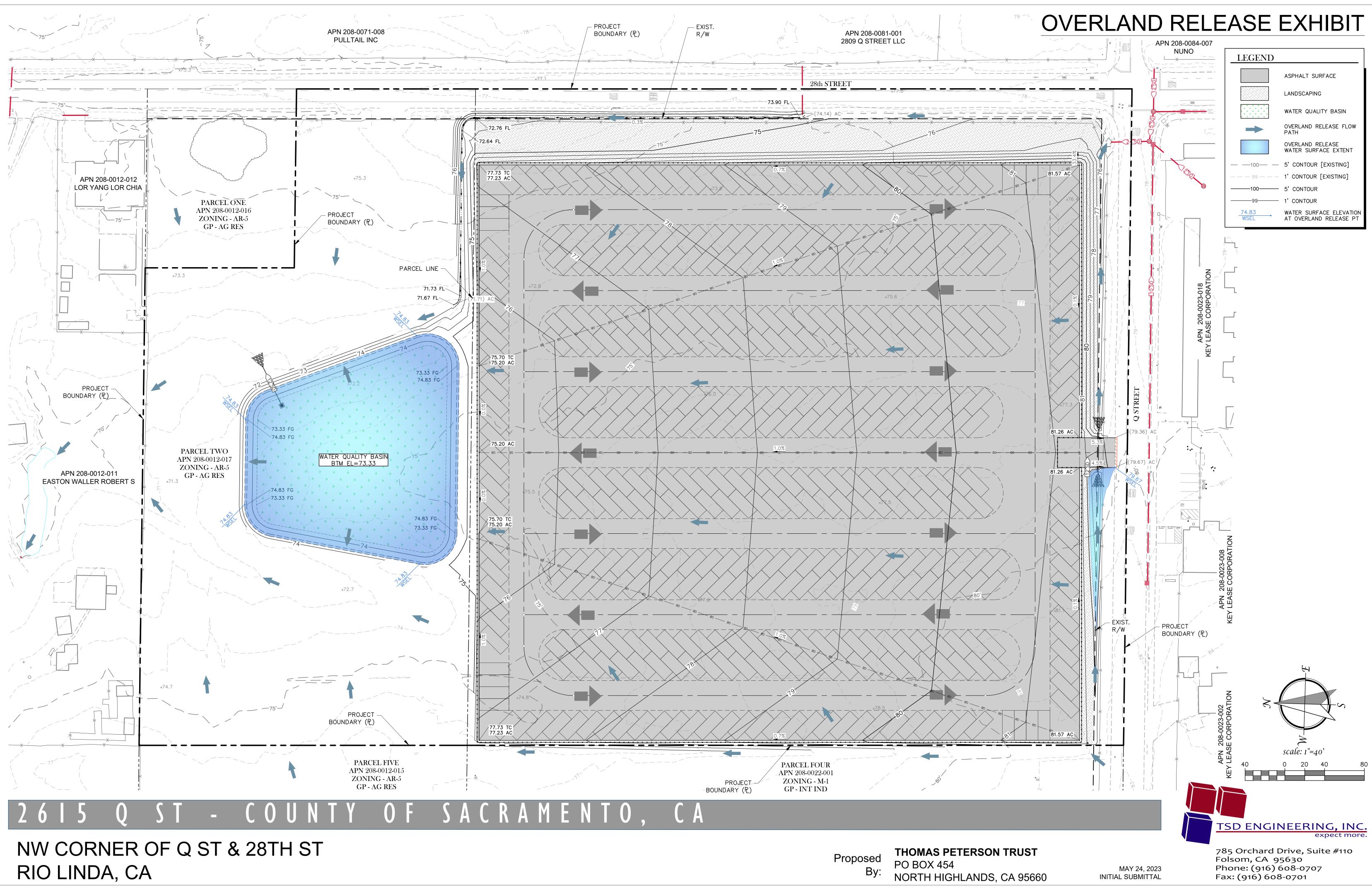
### (10-year, 1-day rainfall)

ID	Peak flow (cfs)	Time of peak (hours)	Basin area (sq. mi)	Peak stage (feet)	Peak storage (ac-ft)	Diversion volume (ac-ft)
O1-1	12.	12:02	.01			
A-1	2.3	12:09	.00			
A-2	.4	12:10	.00			
03-2	.6	12:11	.00			
01-2	64.	12:02	.05			
02-2	2.9	12:16	.00			
В-2	19.	12:02	.01			
C-2	4.5	12:07	.00			

### APPENDIX F

### OVERLAND RELEASE

**Overland Release Exhibit** 



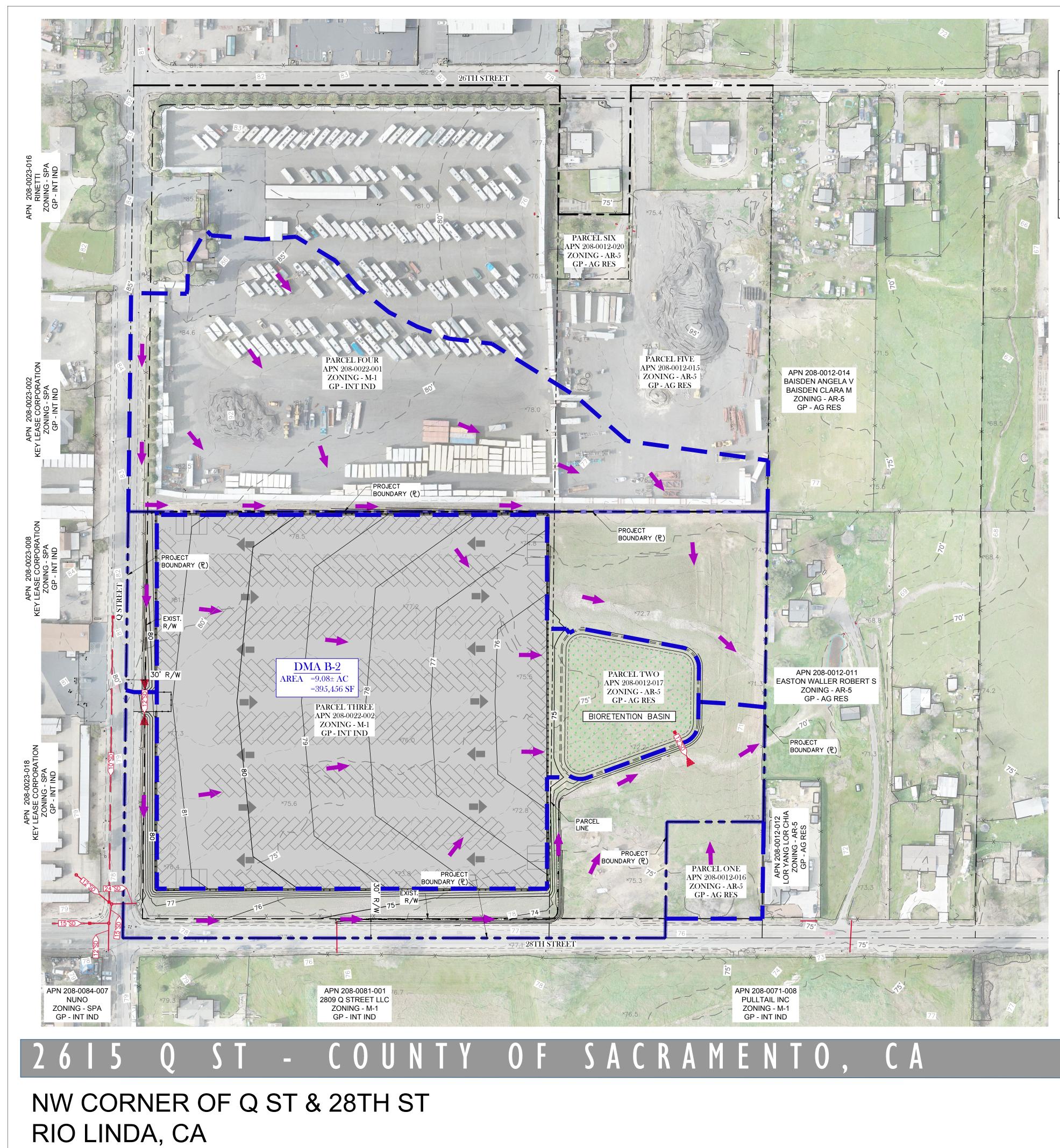
### APPENDIX G

### LID, STORMWATER QUALITY TREATMENT, & HYDROMODIFICATION

Stormwater Management Plan (Exhibit)

Commercial Sites LID Credits and Treatment BMP Sizing Worksheet

SAHM Project Report



	DMA SUMMARY										
DMA I.D.	TOTAL AREA (SF)	TOTAL AREA (AC)	% IMPERVIOUS	AREA IMPERVIOUS (SF)	AREA PERVIOUS (SF)	AREA PERVIOUS (AC)	INTERCEPTOR TREES (EVERGREEN)	INTERCEPTOR TREES (DECIDUOUS)	BIO-RETENTION FEATURE AREA (SF)		
B-2	395,456	9.08	89	350,045	45,411	1.04	0	0	31,600		

Proposed By:

## THOMAS PETERSON TRUST PO BOX 454 NORTH HIGHLANDS, CA 95660

# STORMWATER MANAGEMENT PLAN

KEY FEATURES LEG	END
·100·	5' CONTOUR [EXISTING]
99	1' CONTOUR [EXISTING]
100	5' CONTOUR [PROPOSED]
99	1' CONTOUR [PROPOSED]
	SHED BOUNDARY ON P [EXISTING]
	SHED BOUNDARY [EXISTING]
<u> </u>	STORM DRAIN [EXISTING]
<u>12"SD</u> >	STORM DRAIN [PROPOSED]
<b></b>	STORM DRAIN INLET [EXISTING]
0	STORM DRAIN MANHOLE [EXISTING]
	STORM DRAIN INLET [PROPOSED]
	SURFACE FLOW DIRECTION [EXISTING]





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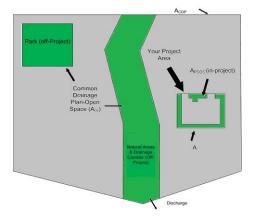
scale: 1"=80'

40 80

Name of Drainage Shed: DMA B-2			Fill in Blue Highlighted boxes	3
Location of project: Sacramento				
Step 1 - Open Space and Pervious Area Cro	edits			
Is your project within the drainage area of a common drainage p	lan that includes open space	? If not, skip to 1 b.		
1 a. Common Drainage Plan Area		15 acre	es A <sub>CDP</sub>	
Common Drainage Plan Open Space (Off-project)		0 acre	as A <sub>os</sub>	see area example
a. Natural storage reservoirs and drainage corridors		0 acre	es	below
b. Buffer zones for natural water bodies		0 acre		DEIOW
c. Natural areas including existing trees, other vegetat	ion, and soil	0 acre	es	
d. Common landscape area/park		0 acre	9S	
e. Regional Flood Control/Drainage basins		0 асте	9S	
1 b. Project Drainage Shed Area (Total)		9.08 acre	as A	
Project-Specific Open Space (In-project, communa	al**)	1.04 acre		
a. Natural storage reservoirs and drainage corridors		0.00 acre	es	
b. Buffer zones for natural water bodies		0.00 acre	es	see area example
c. Natural areas including existing trees, other vegetat	ion, and soil	0.31 acre	9S	below
d. Landscape area/park		0.00 acre	es	
e. Flood Control/Drainage basins		0.73 acre	es	
** Doesn't include impervious areas within individual lo	ts and surrounding indivi	vidual units. That is accounted for below	using Form D-1a in Step 2.	
Area with Runoff Reduction Potential	A - A <sub>PSOS</sub> =	8.04 acre	es A <sub>T</sub>	
Assumed Initial Impervious Fraction	$A_T / A =$	0.89	I	
Open Space & Pervious Area LID Credit (Step 1)				
	<sub>os</sub> /A <sub>CDP</sub> +A <sub>PSOS</sub> /A)x100 =	11 pts		



Ai - Area with Runoff Reduction Potential



### Step 2 - Runoff Reduction Credits Impervious Area Managed Efficiency Factor Effective Area Managed (A<sub>C</sub>) Runoff Reduction Treatments Porous Pavement: 0.000 **Option 1: Porous Pavement** 0 acres acres (see Fact Sheet, excludes porous pavement used in Option 2) **Option 2: Disconnected Pavement** 0.00 use Form D-2a for credits acres (see Fact Sheet, excludes porous pavement used in Option 1) Landscaping used to Disconnect Pavement (see Fact Sheet) 0.0000 0.00 acres acres 0.00 Disconnected Roof Drains 0 acres acres (see Fact Sheet and/or Table D-2b for summary of requirements) 0.00 Ecoroof 0 acres acres (see Fact Sheet) Interceptor Trees use Form D-2b for credits 0.00 acres (see Fact Sheet) 0.00 Total Effective Area Managed by Runoff Reduction Measures $\mathsf{A}_\mathsf{C}$ acres 0 Runoff Reduction Credit (Step 2) (A<sub>C</sub> / A<sub>T</sub> )\*100 = pts

### Table D-2a

### Table D-2b

Porous Pavement Type	Efficiency Multiplier	Maximum roof size	Minimum tı distanc
Cobblestone Block Pavement	0.40	≤ 3,500 sq ft	21 f
Pervious Concrete/Asphalt	0.60	≤ 5,000 sq ft	24 ft
Deroug Croud Boyomont	0.75	≤ 7,500 sq ft	28 ft
Reinforced Grass Pavement	1.00	≤ 10,000 sq ft	32 ft

### Form D-2a: Disconnected Pavement Worksheet See Fact Sheet for more information regarding Disconnected Pavement credit guidelines Effective Area Managed (A<sub>C</sub>) **Pavement Draining to Porous Pavement** 2. Enter area draining onto Porous Pavement 0.00 Box K1 acres 3. Enter area of Receiving Porous Pavement 0.00 acres Box K2 (excludes area entered in Step 2 under Porous Pavement) 4. Ratio of Areas (Box K1 / Box K2) 0.00 Box K3 5. Select multiplier using ratio from Box K3 and enter into Box K4 Ratio (Box D) Ratio is ≤ 0.5 Multiplier 1.00 Ratio is > 0.5 and < 1.0 Box K4 0.83 Ratio is > 1.0 and < 1.5 0.71 Ratio is > 1.5 and < 2.0 $\,$ 0.55 6. Enter Efficiency of Porous Pavement (see table below) Box K5 Efficiency Porous Pavement Type Multiplier Cobblestone Block Pavement 0.40 Pervious Concrete 0.60 Asphalt Pavement Modular Block Pavement 0.75 Porous Gravel Pavement Reinforced Grass Pavement 1.00 0.00 7. Multiply Box K2 by Box K5 and enter into Box K6 acres Box K6 8. Multiply Boxes K1,K4, and K5 and enter the result in Box K7 0.00 Box K7 acres 0.00 acres 9. Add Box K6 to Box K7 and multiply by 60%, and enter the Result in Box K8 This is the amount of area credit to enter into the "Disconnected Pavement" Box of Form D-2

Form D-2b: Interceptor Tree Worksheet				
See Fact Sheet for more information regarding Interceptor Tree credit guidelines				
New Evergreen Trees				
1. Enter number of new evergreen trees that qualify as Interceptor Trees in Box L1.	0	trees	Box L1	
2. Multiply Box L1 by 200 and enter result in Box L2	0	sq. ft.	Box L2	
New Deciduous Trees				
3. Enter number of new deciduous trees that qualify as Interceptor Trees in Box L3.	0	trees	Box L3	
4. Multiply Box L3 by 100 and enter result in Box L4	0	sq. ft.	Box L4	
Existing Tree Canopy				
5. Enter square footage of existing tree canopy that qualifies as Existing Tree canopy in Box L5.	0	sq. ft.	Box L5	
6. Multiply Box L5 by 0.5 and enter the result in Box L6	0	sq. ft.	Box L6	
Total Interceptor Tree EAM Credits				
Add Boxes L2, L4, and L6 and enter it into Box L7	0	sq. ft.	Box L7	
Divide Box L7 by 43,560 and multiply by 20% to get effective area managed and enter result in Box L8 This is the amount of area credit to enter into the "Interceptor Trees" Box of Form D-2	0.00	acres	Box L8	

Commercial

Step 3 - Runoff Management Credits					
Capture and Use Credits Impervious Area Managed by Rain barrels, Cist	terns, and automatically-emptied systems				
(see Fact Sheet)	- enter gallons, for simple rain barrels	0.00	acres		
Automated-Control Capture and Use System (see Fact Sheet, then enter impervious area managed by	y the system)	0.00	acres		
Bioretention/Infiltration Credits					
Impervious Area Managed by Bioretention BMF					
(see Fact Sheet)	Subdrain Elevation         24         inches           Ponding Depth, inches         12         inches	10.82	acres		
		10.02	20103		
Impervious Area Managed by Infiltration BMPs					
(see Fact Sheet)	Drawdown Time, hrs drawdown_hrs, Soil Infiltration Rate, in/hr soil_inf_rate	_inf			
Sizing Option	1: Capture Volume, acre-ft 0.00 capture_vol_int	f 0.00	acres		
Sizing Option	2: Infiltration BMP surface area, sq ft0 soil_surface_ar	rea 0.00	acres		
Popio o	se transh2	0.00 ft			
Dasiii U	or trench? approximate BMP depth	0.00			
Impervious Area Managed by Amended Soil or	Muleh Pode				
(see Fact Sheet)	Mulched Infiltration Area, sq ft mulch_area	0.00	acres		
Total Effective Area Managed by Capture-and-Use	e/Bioretention/Infiltration BMPs	10.82	A <sub>LIDc</sub>		
			2.00		
Runoff Management Credit (Step 3)		$A_{LIDC}/A_{T}^{*}200 = 269.1$	pts		
	LID compliant, check for treatment				
Total LID Credits (Step 1+2+3)					
Total LID Credits (Step 1+2+3)	nent? If yes, proceed to using SacHM.		A <sub>AT</sub>		
Total LID Credits (Step 1+2+3) Does project require hydromodification managen Adjusted Area for Flow-Based, Non-LID Treatmer	nent? If yes, proceed to using SacHM. nt A <sub>T</sub> - A	sizing in Step 4 280.6	A <sub>AT</sub>		
Total LID Credits (Step 1+2+3) Does project require hydromodification managen	nent? If yes, proceed to using SacHM. nt A <sub>T</sub> - A	sizing in Step 4 280.6	_		
Total LID Credits (Step 1+2+3) Does project require hydromodification managen Adjusted Area for Flow-Based, Non-LID Treatmer	nent? If yes, proceed to using SacHM. nt A <sub>T</sub> - A ased, Non-LID Treatment	sizing in Step 4 280.6	A <sub>AT</sub>		
Total LID Credits (Step 1+2+3) Does project require hydromodification managen Adjusted Area for Flow-Based, Non-LID Treatmer Adjusted Impervious Fraction of A for Volume-Ba	nent? If yes, proceed to using SacHM. nt A <sub>T</sub> - A ased, Non-LID Treatment	sizing in Step 4 280.6	A <sub>AT</sub>		
Total LID Credits (Step 1+2+3) Does project require hydromodification managen Adjusted Area for Flow-Based, Non-LID Treatmer Adjusted Impervious Fraction of A for Volume-Ba	nent? If yes, proceed to using SacHM. nt A <sub>T</sub> - A nsed, Non-LID Treatment eeded	sizing in Step 4 280.6	A <sub>AT</sub>		
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Total LID Credits (Step 1+2+3) Does project require hydromodification manager Adjusted Area for Flow-Based, Non-LID Treatmer Adjusted Impervious Fraction of A for Volume-Ba STOP: No additional treatment net a Treatment - Flow-Based (Rational Method te treatment flow (cfs):	nent? If yes, proceed to using SacHM. nt A <sub>T</sub> - A ased, Non-LID Treatment eeded d) Flow = Runoff Coefficient x Rainfall Intensity x Area	sizing in Step 4 280.6 $A_{c} - A_{LIDC} = -2.78$ $A_{AT} / A = -0.31$ Table D-2c	A <sub>AT</sub> I <sub>A</sub>		
Total LID Credits (Step 1+2+3) Does project require hydromodification managen Adjusted Area for Flow-Based, Non-LID Treatmer Adjusted Impervious Fraction of A for Volume-Ba STOP: No additional treatment ne	nent? If yes, proceed to using SacHM. nt A <sub>T</sub> - A ised, Non-LID Treatment eeded	sizing in Step 4 280.6 $A_c - A_{LDC} = -2.78$ $A_{AT} / A = -0.31$ Table D-2c Rainfal	I Intensity		
Total LID Credits (Step 1+2+3) Does project require hydromodification manager Adjusted Area for Flow-Based, Non-LID Treatmer Adjusted Impervious Fraction of A for Volume-Ba STOP: No additional treatment net a Treatment - Flow-Based (Rational Method te treatment flow (cfs):	nent? If yes, proceed to using SacHM. nt A <sub>T</sub> - A ased, Non-LID Treatment eeded d) Flow = Runoff Coefficient x Rainfall Intensity x Area	sizing in Step 4 280.6 $A_{c} - A_{LDC} = -2.78$ $A_{AT} / A = -0.31$ Table D-2c Rainfal Roseville	A <sub>AT</sub> I <sub>A</sub>		
Total LID Credits (Step 1+2+3) Does project require hydromodification manager Adjusted Area for Flow-Based, Non-LID Treatmer Adjusted Impervious Fraction of A for Volume-Ba STOP: No additional treatment no ta Treatment - Flow-Based (Rational Method te treatment flow (cfs):	nent? If yes, proceed to using SacHM. nt A <sub>T</sub> - A ased, Non-LID Treatment eeded J Flow = Runoff Coefficient x Rainfall Intensity x Area 0.18 i -2.78 A <sub>AT</sub>	sizing in Step 4 280.6 $A_{c} - A_{LIDC} = -2.78$ $A_{AT} / A = -0.31$ Table D-2c Rainfal Roseville Sacramento	A <sub>AT</sub>   I <sub>A</sub>   I <u>Intensity</u>   = 0.20 in/hr		
Total LID Credits (Step 1+2+3) Does project require hydromodification manager Adjusted Area for Flow-Based, Non-LID Treatmer Adjusted Impervious Fraction of A for Volume-Ba STOP: No additional treatment no ta Treatment - Flow-Based (Rational Methor te treatment flow (cfs):	nent? If yes, proceed to using SacHM. nt A <sub>T</sub> - A ased, Non-LID Treatment eecded d) Flow = Runoff Coefficient x Rainfall Intensity x Area 0.18]i	sizing in Step 4 280.6 $A_{c} - A_{LDC} = -2.78$ $A_{AT} / A = -0.31$ Table D-2c Rainfal Roseville Sacramento	☐ A <sub>AT</sub> ☐ I <sub>A</sub> ☐ I I Intensity i = 0.20 in/hr 0.18 in/hr		
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Total LID Credits (Step 1+2+3) Does project require hydromodification manager Adjusted Area for Flow-Based, Non-LID Treatmer Adjusted Impervious Fraction of A for Volume-Ba STOP: No additional treatment no a Treatment - Flow-Based (Rational Method te treatment flow (cfs): o value for i in Table D-2c (Rainfall Intensity) A <sub>AT</sub> from Step 3 = 0.95	nent? If yes, proceed to using SacHM. nt A <sub>T</sub> - A ased, Non-LID Treatment eeded d) Flow = Runoff Coefficient x Rainfall Intensity x Area 0.18 i -2.78 A <sub>AT</sub> 0.95 C	sizing in Step 4 280.6 $A_{c} - A_{LDC} = -2.78$ $A_{AT} / A = -0.31$ Table D-2c Rainfal Roseville Sacramento	☐ A <sub>AT</sub> ☐ I <sub>A</sub> II Intensity i = 0.20 in/hr 0.18 in/hr		
Total LID Credits (Step 1+2+3) Does project require hydromodification manager Adjusted Area for Flow-Based, Non-LID Treatmer Adjusted Impervious Fraction of A for Volume-Ba STOP: No additional treatment no a Treatment - Flow-Based (Rational Method te treatment flow (cfs): o value for i in Table D-2c (Rainfall Intensity) A <sub>AT</sub> from Step 3 = 0.95	nent? If yes, proceed to using SacHM. nt A <sub>T</sub> - A ased, Non-LID Treatment eeded d) Flow = Runoff Coefficient x Rainfall Intensity x Area 0.18 i -2.78 A <sub>AT</sub> 0.95 C	sizing in Step 4 280.6 $A_{c} - A_{LDC} = -2.78$ $A_{AT} / A = -0.31$ Table D-2c Rainfal Roseville Sacramento	☐ A <sub>AT</sub> ☐ I <sub>A</sub> II Intensity i = 0.20 in/hr 0.18 in/hr		
Total LID Credits (Step 1+2+3)         Does project require hydromodification manager         Adjusted Area for Flow-Based, Non-LID Treatmer         Adjusted Impervious Fraction of A for Volume-Based         STOP: No additional treatment no         STOP: No additional treatment no         La Treatment - Flow-Based (Rational Methor         te treatment flow (cfs):         ovalue for i in Table D-2c (Rainfall Intensity)         A <sub>AT</sub> from Step 3         Elow = 0.95 * i * A <sub>AT</sub>	nent? If yes, proceed to using SacHM. nt A <sub>T</sub> - A ased, Non-LID Treatment eeded d) Flow = Runoff Coefficient x Rainfall Intensity x Area 0.18 i -2.78 A <sub>AT</sub> 0.95 C	sizing in Step 4 280.6 $A_{c} - A_{LDC} = -2.78$ $A_{AT} / A = -0.31$ Table D-2c Rainfal Roseville Sacramento	☐ A <sub>AT</sub> ☐ I <sub>A</sub> ☐ I I Intensity i = 0.20 in/hr 0.18 in/hr		
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Total LID Credits (Step 1+2+3)         Does project require hydromodification managem         Adjusted Area for Flow-Based, Non-LID Treatmer         Adjusted Impervious Fraction of A for Volume-Ba         STOP: No additional treatment ne         STOP: No additional treatment ne         Iteratment - Flow-Based (Rational Method         Iteratment flow (cfs):         o value for i in Table D-2c (Rainfall Intensity)         A <sub>AT</sub> from Step 3         Elow = 0.95 * i * A <sub>AT</sub> Iteratment - Volume-Based (ASCE-WEF)	nent? If yes, proceed to using SacHM. nt A <sub>T</sub> - A ased, Non-LID Treatment eeded d) Flow = Runoff Coefficient x Rainfall Intensity x Area 0.18 i -2.78 A <sub>AT</sub> 0.95 C -0.48 cfs	sizing in Step 4 280.6 $A_{c} - A_{LDC} = -2.78$ $A_{AT} / A = -0.31$ Table D-2c Rainfal Roseville Sacramento	☐ A <sub>AT</sub> ☐ I <sub>A</sub> I = 0.20 in/hr = 0.18 in/hr = 0.20 in/hr		
Total LID Credits (Step 1+2+3)         Does project require hydromodification manager         Adjusted Area for Flow-Based, Non-LID Treatmer         Adjusted Impervious Fraction of A for Volume-Based         STOP: No additional treatment net         STOP: No additional treatment net         Adjusted Impervious Fraction of A for Volume-Based         STOP: No additional treatment net         Adjusted Impervious Fraction of A for Volume-Based         STOP: No additional treatment net         Additional treatment net         Impervious Fraction of A for Volume-Based         Additional treatment net         Impervious Fraction of A for Volume-Based         Additional treatment net         Impervious Fractional Method         Advance Fractional Method         Impervious Fractional Method         Impervious Fractional Method         Advance Fractional Method         Impervious Fractional Method         Advance Fractional M	nent? If yes, proceed to using SacHM.         nt       A <sub>T</sub> - A         ised, Non-LID Treatment         eeded         d)         Flow = Runoff Coefficient x Rainfall Intensity x Area         0.18 i         -2.78 A <sub>AT</sub> 0.95 c         -0.48 cfs         WQV = Area x Maximized Detention Volume (P <sub>0</sub> )         9.08       A	sizing in Step 4 280.6 $A_{c} - A_{LDC} = -2.78$ $A_{AT} / A = -0.31$ Table D-2c Rainfal Roseville Sacramento Folsom	☐ A <sub>AT</sub> ☐ I <sub>A</sub> I Intensity i = 0.20 in/hr = 0.18 in/hr i = 0.20 in/hr		
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Total LID Credits (Step 1+2+3)         Does project require hydromodification manager         Adjusted Area for Flow-Based, Non-LID Treatmer         Adjusted Impervious Fraction of A for Volume-Based         STOP: No additional treatment net         Adjusted Impervious Fraction of A for Volume-Based         STOP: No additional treatment net         Adjusted Impervious Fraction of A for Volume-Based         STOP: No additional treatment net         Image: Adjusted Impervious Fractional Method         Image: Adjusted Impervious Colspan="2">Image: Adjusted Adjusted Impervious Colspan="2"         Image: Adjuster Colspan="2" <td <="" colspan="2" td=""><td>nent? If yes, proceed to using SacHM.         nt       <math>A_T - A</math>         ased, Non-LID Treatment         eeded         d)         Flow = Runoff Coefficient x Rainfall Intensity x Area         0.18         i         -2.78         <math>A_{AT}</math>         0.95         -0.48         cfs</td><td>sizing in Step 4 280.6 <math>A_{c} - A_{LDC} = -2.78</math> <math>A_{AT} / A = -0.31</math> Table D-2c Rainfal Roseville Sacramento Folsom</td><td>☐ A<sub>AT</sub> ☐ I<sub>A</sub> I Intensity i = 0.20 in/hr = 0.18 in/hr i = 0.20 in/hr</td></td>	<td>nent? If yes, proceed to using SacHM.         nt       <math>A_T - A</math>         ased, Non-LID Treatment         eeded         d)         Flow = Runoff Coefficient x Rainfall Intensity x Area         0.18         i         -2.78         <math>A_{AT}</math>         0.95         -0.48         cfs</td> <td>sizing in Step 4 280.6 <math>A_{c} - A_{LDC} = -2.78</math> <math>A_{AT} / A = -0.31</math> Table D-2c Rainfal Roseville Sacramento Folsom</td> <td>☐ A<sub>AT</sub> ☐ I<sub>A</sub> I Intensity i = 0.20 in/hr = 0.18 in/hr i = 0.20 in/hr</td>		nent? If yes, proceed to using SacHM.         nt $A_T - A$ ased, Non-LID Treatment         eeded         d)         Flow = Runoff Coefficient x Rainfall Intensity x Area         0.18         i         -2.78 $A_{AT}$ 0.95         -0.48         cfs	sizing in Step 4 280.6 $A_{c} - A_{LDC} = -2.78$ $A_{AT} / A = -0.31$ Table D-2c Rainfal Roseville Sacramento Folsom	☐ A <sub>AT</sub> ☐ I <sub>A</sub> I Intensity i = 0.20 in/hr = 0.18 in/hr i = 0.20 in/hr
Total LID Credits (Step 1+2+3) Does project require hydromodification managem Adjusted Area for Flow-Based, Non-LID Treatmer Adjusted Impervious Fraction of A for Volume-Ba STOP: No additional treatment ne a Treatment - Flow-Based (Rational Method te treatment flow (cfs): to value for i in Table D-2c (Rainfall Intensity) $A_{AT}$ from Step 3 = 0.95 Flow = 0.95 * i * $A_{AT}$ b Treatment - Volume-Based (ASCE-WEF) te water quality volume (Acre-Feet): A from Step 1 P <sub>0</sub> : Maximized Detention Volume from figures E-1 to E- ndix E of this manual using I <sub>A</sub> from Step 2.	nent? If yes, proceed to using SacHM.         nt       A <sub>T</sub> - A         ised, Non-LID Treatment         eeded         d)         Flow = Runoff Coefficient x Rainfall Intensity x Area         0.18 i         -2.78 A <sub>AT</sub> 0.95 c         -0.48 cfs         WQV = Area x Maximized Detention Volume (P <sub>0</sub> )         9.08       A	sizing in Step 4 280.6 $A_{c} - A_{LDC} = -2.78$ $A_{AT} / A = -0.31$ Table D-2c Rainfal Roseville Sacramento Folsom	☐ A <sub>AT</sub> ☐ I <sub>A</sub> I = 0.20 in/hr = 0.18 in/hr = 0.20 in/hr		



# **General Model Information**

Project Name:	518-001_SAHM
Site Name:	PETERSON STORAGE
Site Address:	2615 Q ST
City:	RIO LINDA
Report Date:	5/25/2023
Gage:	RANCHO C
Data Start:	1961/10/01
Data End:	2004/09/30
Timestep:	Hourly
Precip Scale:	0.944
Version Date:	2019/12/01

## POC Thresholds

Low Flow Threshold for POC1:	25 Percent of the 2 Year
High Flow Threshold for POC1:	10 Year

# Landuse Basin Data Pre-Project Land Use

#### DMA B-2

Bypass:	No
GroundWater:	No
Pervious Land Use C,Grass,Flat(0-1%)	acre 9.08
Pervious Total	9.08
Impervious Land Use	acre
Impervious Total	0
Basin Total	9.08
Element Flows To	

Element Flows To: Surface Ir

Interflow

Groundwater

## Mitigated Land Use

#### DMA B-2

Bypass:	No
GroundWater:	No
Pervious Land Use C,Grass,Flat(0-1%)	acre 8.04
Pervious Total	8.04
Impervious Land Use Imperv,Flat(0-1%)	acre 1.04
Impervious Total	1.04
Basin Total	9.08

Element Flows To: Surface Interflow Groundwater BIORETENTION BASENORETENTION BASIN Routing Elements Pre-Project Routing

## Mitigated Routing

#### **BIORETENTION BASIN**

Depth: 1.5 ft. Volume at riser head: 0.7503 acre-feet. Infiltration On	
Infiltration rate: 0.1	
Infiltration safety factor: 0.333	
Wetted surface area On Total Volume Infiltrated (ac-ft.): 134.1	16
Total Volume Through Riser (ac-ft.): 45.54	
Total Volume Through Facility (ac-ft.): 179.6	
Percent Infiltrated: 74.65	
Total Precip Applied to Facility: 43.64	
Total Evap From Facility:8.877Side slope 1:3 To 1	
Side slope 2: 3 To 1	
Side slope 3: 3 To 1	
Side slope 4: 3 To 1	
Discharge Structure	
Riser Height: 1 ft.	
Riser Diameter: 27.1 in. Element Flows To:	
Outlet 1 Outlet 2	

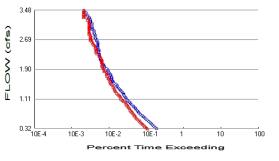
#### Pond Hydraulic Table

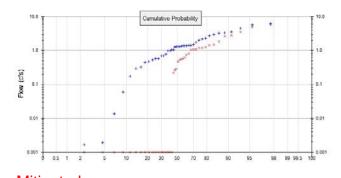
<b>Stage(feet)</b> 73.330 73.347	<b>Area(ac.)</b> 0.725 0.726	Volume(ac-ft.) 0.000 0.012	0.000 0.000	0.000 0.024
73.363 73.380	0.727 0.727	0.024 0.036	0.000 0.000	0.024 0.024
73.397	0.728	0.048	0.000	0.024
73.413	0.729	0.060	0.000	0.024
73.430	0.730	0.072	0.000	0.024
73.447	0.731	0.085	0.000	0.024
73.463 73.480	0.732 0.732	0.097 0.109	0.000 0.000	0.024 0.024
73.497	0.733	0.121	0.000	0.024
73.513	0.734	0.133	0.000	0.024
73.530	0.735	0.146	0.000	0.024
73.547	0.736	0.158	0.000	0.024
73.563 73.580	0.737 0.737	0.170 0.182	0.000 0.000	0.024 0.024
73.597	0.738	0.195	0.000	0.024
73.613	0.739	0.207	0.000	0.024
73.630	0.740	0.219	0.000	0.024
73.647	0.741	0.232	0.000	0.024
73.663 73.680	0.741 0.742	0.244 0.256	0.000 0.000	0.024 0.024
73.697	0.742	0.269	0.000	0.024
73.713	0.744	0.281	0.000	0.025
73.730	0.745	0.294	0.000	0.025
73.747	0.746	0.306	0.000	0.025

73.763 $73.780$ $73.797$ $73.813$ $73.830$ $73.847$ $73.863$ $73.847$ $73.863$ $73.913$ $73.930$ $73.947$ $73.930$ $73.947$ $73.930$ $73.947$ $73.930$ $73.947$ $73.930$ $73.947$ $73.930$ $73.947$ $73.930$ $73.947$ $73.930$ $73.947$ $73.930$ $73.947$ $73.930$ $73.947$ $74.033$ $74.047$ $74.063$ $74.080$ $74.147$ $74.130$ $74.147$ $74.213$ $74.230$ $74.247$ $74.263$ $74.280$ $74.297$ $74.313$ $74.363$ $74.380$ $74.347$ $74.363$ $74.347$ $74.430$ $74.443$ $74.463$ $74.547$ $74.563$ $74.580$ $74.580$ $74.580$ $74.613$ $74.633$	0.746 0.747 0.748 0.749 0.750 0.751 0.752 0.753 0.754 0.755 0.756 0.756 0.757 0.758 0.759 0.760 0.761 0.761 0.762 0.763 0.763 0.764 0.765 0.766 0.767 0.768 0.769 0.770 0.772 0.772 0.773 0.772 0.773 0.774 0.775 0.778 0.777 0.778 0.777 0.778 0.777 0.778 0.777 0.778 0.779 0.780 0.781 0.783 0.783 0.783 0.784 0.785 0.783 0.783 0.783 0.784 0.785 0.783 0.783 0.783 0.783 0.784 0.785 0.787 0.788 0.789 0.790 0.790	0.319 0.331 0.343 0.356 0.368 0.381 0.393 0.406 0.419 0.431 0.444 0.456 0.469 0.482 0.494 0.507 0.519 0.532 0.545 0.558 0.570 0.583 0.596 0.609 0.621 0.634 0.660 0.673 0.685 0.698 0.711 0.724 0.737 0.750 0.763 0.776 0.789 0.802 0.815 0.828 0.841 0.854 0.841 0.854 0.880 0.906 0.919 0.932 0.945 0.972 0.985	0.000 0	0.025 0.026 0.02
74.580 74.597 74.613 74.630 74.647 74.663 74.680 74.697	0.788 0.789 0.790 0.791 0.792 0.793 0.794	0.945 0.958 0.972 0.985 0.998 1.011 1.024 1.038	2.971 3.268 3.573 3.885 4.203 4.528 4.858 5.193	0.026 0.026 0.026 0.026 0.026 0.026 0.026 0.026
74.613 74.630 74.647 74.663 74.680	0.789 0.790 0.791 0.792 0.793	0.972 0.985 0.998 1.011 1.024	3.573 3.885 4.203 4.528 4.858	0.020 0.020 0.020 0.020 0.020

74.730 74.747	0.795 0.796	1.064 1.077	5.876 6.222	0.026 0.026
74.763	0.797	1.091	6.571	0.020
74.780	0.798	1.104	6.921	0.026
74.797	0.799	1.117	7.273	0.026
74.813	0.800	1.131	7.625	0.026
74.830	0.800	1.144	7.977	0.026
74.847	0.801	1.157	8.329	0.026

# Analysis Results POC 1





+ Pre-Project



Pre-Project Landuse Totals for POC #1Total Pervious Area:9.08Total Impervious Area:0

Mitigated Landuse Totals for POC #1 Total Pervious Area: 8.04 Total Impervious Area: 1.04

Flow Frequency Method: Log

Log Pearson Type III 17B

Flow Frequency Return Periods for Pre-Project. POC #1Return PeriodFlow(cfs)2 year1.29555 year2.39665110 year3.48085625 year5.806757

Flow Frequency Return Periods for Mitigated. POC #1Return PeriodFlow(cfs)2 year0.2787565 year1.29693910 year2.74794525 year4.994666

#### **Annual Peaks**

Annual Peaks for Pre-Project and Mitigated. POC #1

Year	Pre-Project	Mitigated
1962	1.313	0.618
1963	0.697	0.000
1964	0.059	0.000
1965	1.260	0.550
1966	0.014	0.000
1967	1.397	1.085
1968	0.288	0.000
1969	1.321	1.058
1970	1.041	0.466
1971	1.435	0.565
1972	0.002	0.000
1973	3.249	1.207
1974	0.990	0.000
1975	1.339	0.000

#### **Ranked Annual Peaks**

Ranked Annual Peaks for Pre-Project and Mitigated. POC #1 **Pre-Project** Mitigated Rank 6.1518 5.7237 1 2 3 4.8796 5.7523 4.5978 3.5071 4 5 6 7 3.5701 2.8284 3.3738 2.6514 1.8463 3.2494 2.9437 1.5165 8 2.6939 1.4089 9 1.2721 2.3306 10 2.1625 1.2071 11 1.9827 1.1845 1.0848 12 1.7282 13 1.5188 1.0583 14 1.4350 1.0393 15 1.4044 0.8140 16 1.3965 0.7487 0.6180 17 1.3752 18 1.3733 0.5654 0.5499 19 1.3392 20 1.3214 0.5206 21 1.3131 0.4655 22 0.2788 1.2955 23 0.2638 1.2601 24 1.0412 0.2187 25 1.0385 0.0000

26 27	0.9896 0.9817	0.0000 0.0000
28	0.7884	0.0000
29	0.7024	0.0000
30	0.6972	0.0000
31	0.5809	0.0000
32	0.5666	0.0000
33	0.5192	0.0000
34	0.4706	0.0000
35	0.4499	0.0000
36	0.3234	0.0000
37	0.2876	0.0000
38	0.1761	0.0000
39	0.0588	0.0000
40	0.0137	0.0000
41	0.0019	0.0000
42	0.0017	0.0000
43	0.0016	0.0000

### **Duration Flows**

The Facility PASSED

<b>Flow(cfs)</b> 0.3239 0.3558	<b>Predev</b> 714 650	<b>Mit</b> 403 373	<b>Percentage</b> 56 57	<b>Pass/Fail</b> Pass Pass
0.3877	596	340	57	Pass
0.4195	554	313	56	Pass
0.4514 0.4833	511 475	294 274	57 57	Pass Pass
0.5152	439	259	58	Pass
0.5471	397	239	60	Pass
0.5790 0.6109	366 338	215 202	58 59	Pass Pass
0.6428	311	195	62	Pass
0.6747	290	185	63	Pass
0.7065	261 240	174 164	66 68	Pass
0.7384 0.7703	240 227	156	68	Pass Pass
0.8022	214	145	67	Pass
0.8341	203	138	67	Pass
0.8660 0.8979	192 177	132 126	68 71	Pass Pass
0.9298	166	122	73	Pass
0.9616	154	114	74	Pass
0.9935 1.0254	143 135	108 104	75 77	Pass Pass
1.0573	121	93	76	Pass
1.0892	117	86	73	Pass
1.1211 1.1530	109 106	81 77	74 72	Pass Pass
1.1849	98	75	76	Pass
1.2168	95	70	73	Pass
1.2486 1.2805	93 87	65 60	69 68	Pass Pass
1.3124	82	57	69	Pass
1.3443	75	55	73	Pass
1.3762 1.4081	71	53 52	74 78	Pass
1.4400	66 62	52 49	78 79	Pass Pass
1.4719	62	48	77	Pass
1.5038	58	44	75	Pass
1.5356 1.5675	50 49	42 41	84 83	Pass Pass
1.5994	47	40	85	Pass
1.6313	45	39	86	Pass
1.6632 1.6951	44 44	36 36	81 81	Pass Pass
1.7270	43	34	79	Pass
1.7589	42	33	78	Pass
1.7908 1.8226	38 37	33 32	86 86	Pass Pass
1.8545	36	28	77	Pass
1.8864	35	28	80	Pass
1.9183 1.9502	32 28	27 26	84 92	Pass Pass
1.9821	7X	Zn	y/	Pass

Water Quality

# Model Default Modifications

Total of 0 changes have been made.

#### **PERLND Changes**

No PERLND changes have been made.

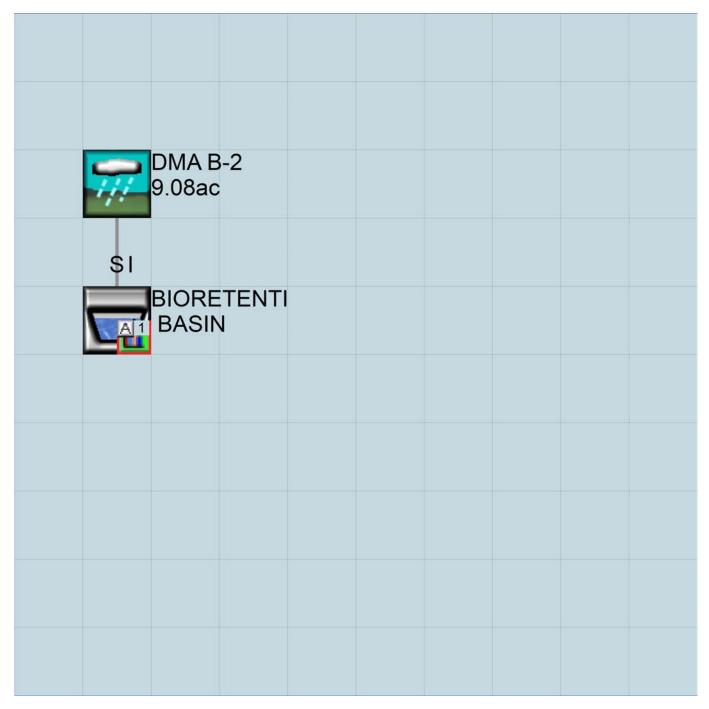
#### **IMPLND Changes**

No IMPLND changes have been made.

# Appendix Pre-Project Schematic

DMA B-2 9.08ac		

## Mitigated Schematic



#### Pre-Project UCI File

RUN

GLOBAL WWHM4 model simulation END 3 0 2004 09 30 START 1961 10 01 RUN INTERP OUTPUT LEVEL RESUME 0 RUN 1 UNIT SYSTEM 1 END GLOBAL FILES <File> <Un#> <-----File Name---->\*\*\* \* \* \* <-ID-> WDM 26 518-001\_SAHM.wdm MESSU 25 Pre518-001\_SAHM.MES 27 Pre518-001\_SAHM.L61 28 Pre518-001\_SAHM.L62 POC518-001\_SAHM1.dat 30 END FILES OPN SEOUENCE INGRP INDELT 00:60 33 PERLND 501 COPY DISPLY 1 END INGRP END OPN SEQUENCE DISPLY DISPLY-INF01 # - #<-----Title----->\*\*\*TRAN PIVL DIG1 FIL1 PYR DIG2 FIL2 YRND 1 DMA B-2 1 2 30 MAX 9 END DISPLY-INF01 END DISPLY COPY TIMESERIES # - # NPT NMN \*\*\* 1 1 )1 1 1 501 1 END TIMESERIES END COPY GENER OPCODE # # OPCD \*\*\* END OPCODE PARM K \*\*\* # # END PARM END GENER PERLND GEN-INFO <PLS ><-----Name---->NBLKS Unit-systems Printer \*\*\* User t-series Engl Metr \*\*\* # - # in out \* \* \* C,Grass,Flat(0-1%) 1 1 1 1 27 0 33 END GEN-INFO \*\*\* Section PWATER\*\*\* ACTIVITY 

 # # ATMP SNOW PWAT SED
 PST
 PWG PQAL MSTL PEST NITR PHOS TRAC \*\*\*

 33
 0
 0
 1
 0
 0
 0
 0
 0

 END ACTIVITY PRINT-INFO END PRINT-INFO

PWAT-PARM1 <PLS > PWATER variable monthly parameter value flags \*\*\* 

 # - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE INFC HWT \*\*\*

 33
 0
 0
 1
 0
 0
 1
 0

 END PWAT-PARM1 PWAT-PARM2 
 <PLS >
 PWATER input info: Part 2
 \*\*\*

 # - # \*\*\*FOREST
 LZSN
 INFILT
 LSUR
 SLSUR
 KVARY
 AGWRC

 33
 0
 4.5
 0.045
 400
 0.01
 3
 0.92
 END PWAT-PARM2 PWAT-PARM3 BASETP AGWETP 0 0 0.05 END PWAT-PARM3 PWAT-PARM4 <PLS > PWATER input info: Part 4 \* \* \* 
 # - #
 CEPSC
 UZSN
 NSUR
 INTFW
 IRC
 LZETP \*\*\*

 33
 0
 0.3
 0.25
 0.7
 0.5
 0
 END PWAT-PARM4 MON-LZETPARM <PLS > PWATER input info: Part 3 \* \* \* 
 # # JAN
 FEB
 MAR
 APR
 MAY
 JUN
 JUL
 AUG
 SEP
 OCT
 NOV
 DEC
 \*\*\*

 33
 0.4
 0.4
 0.4
 0.4
 0.5
 0.55
 0.55
 0.55
 0.45
 0.4
 END MON-LZETPARM MON-INTERCEP \* \* \* <PLS > PWATER input info: Part 3 # - # JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC \*\*\* 33 0.12 0.12 0.12 0.11 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.11 0.12 END MON-INTERCEP PWAT-STATE1 <PLS > \*\*\* Initial conditions at start of simulation ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 \*\*\* GWVS 
 # # \*\*\* CEPS
 SURS
 UZS
 IFWS
 LZS
 AGWS

 33
 0
 0
 0.15
 0
 4
 0.05
 0 END PWAT-STATE1 END PERLND IMPLND GEN-INFO <PLS ><-----Name----> Unit-systems Printer \*\*\* User t-series Engl Metr \*\*\* # - # \* \* \* in out END GEN-INFO \*\*\* Section IWATER\*\*\* ACTIVITY # - # ATMP SNOW IWAT SLD IWG IQAL \*\*\* END ACTIVITY PRINT-INFO <ILS > \*\*\*\*\*\*\* Print-flags \*\*\*\*\*\*\* PIVL PYR # - # ATMP SNOW IWAT SLD IWG IQAL \*\*\*\*\*\*\*\* END PRINT-INFO IWAT-PARM1 <PLS > IWATER variable monthly parameter value flags \*\*\* # - # CSNO RTOP VRS VNN RTLI \*\*\* END IWAT-PARM1 IWAT-PARM2 <PLS > IWATER input info: Part 2 \* # - # \*\*\* LSUR SLSUR NSUR RETSC \* \* \* END IWAT-PARM2

IWAT-PARM3 IWATER input info: Part 3 \*\*\* <PLS > # - # \*\*\*PETMAX PETMIN END IWAT-PARM3 IWAT-STATE1 <PLS > \*\*\* Initial conditions at start of simulation # - # \*\*\* RETS SURS END IWAT-STATE1 END IMPLND SCHEMATIC <--Area--> <-Target-> MBLK \*\*\* <-factor-> <Name> # Tbl# \*\*\* <-Source-> <Name> # DMA B-2\*\*\* 9.08 COPY 501 12 9.08 COPY 501 13 PERLND 33 PERLND 33 \*\*\*\*\*\*Routing\*\*\*\*\* END SCHEMATIC NETWORK <-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> \*\*\* <Name> # <Name> # #<-factor->strg <Name> # # <Name> # # \*\*\*
COPY 501 OUTPUT MEAN 1 1 12.1 DISPLY 1 INPUT TIMSER 1 <-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> \*\*\* <Name> # <Name> # #<-factor->strg <Name> # # <Name> # # \*\*\* END NETWORK RCHRES GEN-INFO Name Nexits Unit Systems Printer \* \* \* RCHRES # - #<----> User T-series Engl Metr LKFG \* \* \* in out \* \* \* END GEN-INFO \*\*\* Section RCHRES\*\*\* ACTIVITY # - # HYFG ADFG CNFG HTFG SDFG GOFG OXFG NUFG PKFG PHFG \*\*\* END ACTIVITY PRINT-INFO # - # HYDR ADCA CONS HEAT SED GOL OXRX NUTR PLNK PHCB PIVL PYR \*\*\*\*\*\*\*\* END PRINT-INFO HYDR-PARM1 \* \* \* RCHRES Flags for each HYDR Section END HYDR-PARM1 HYDR-PARM2 # – # FTABNO LEN DELTH STCOR KS DB50 \* \* \* <----><----><----><----><----><----> \* \* \* END HYDR-PARM2 HYDR-INIT RCHRES Initial conditions for each HYDR section END HYDR-INIT END RCHRES

SPEC-ACTIONS END SPEC-ACTIONS FTABLES END FTABLES

EXT SOURCES <-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member-> \*\*\* <Name># <Name> # tem strg<-factor->strg<Name># #<Name> # #<Name> # #<Name> # #<Name> # #<Name> # #\*\*\*WDM2PRECENGL0.944PERLND1999EXTNLPRECWDM2PRECENGL0.944IMPLND1999EXTNLPRECWDM1EVAPENGL0.85PERLND1999EXTNLPETINPWDM1EVAPENGL0.85IMPLND1999EXTNLPETINP END EXT SOURCES EXT TARGETS <-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd \*\*\* <Name> # <Name> # #<-factor->strg <Name> # <Name> tem strg strg\*\*\* COPY 501 OUTPUT MEAN 1 1 12.1 WDM 501 FLOW ENGL REPL END EXT TARGETS MASS-LINK PERLND PWATER SURO 0.083333 COPY INPUT MEAN END MASS-LINK 12 MASS-LINK 13 PERLND PWATER IFWO 0.083333 COPY INPUT MEAN END MASS-LINK 13

END MASS-LINK

END RUN

#### Mitigated UCI File

RUN

GLOBAL WWHM4 model simulation END 2004 09 30 3 0 START 1961 10 01 RUN INTERP OUTPUT LEVEL RESUME 0 RUN 1 UNIT SYSTEM 1 END GLOBAL FILES <File> <Un#> <-----File Name---->\*\*\* \* \* \* <-ID-> WDM 26 518-001\_SAHM.wdm MESSU 25 Mit518-001\_SAHM.MES 27 Mit518-001\_SAHM.L61 28 Mit518-001\_SAHM.L62 POC518-001\_SAHM1.dat 30 END FILES OPN SEOUENCE INGRP INDELT 00:60 33 PERLND 1 IMPLND 1 RCHRES COPY 1 501 DISPLY 1 END INGRP END OPN SEQUENCE DISPLY DISPLY-INFO1 # -#<-----Title----->\*\*\*TRAN PIVL DIG1 FIL1PYR DIG2 FIL2 YRND1BIORETENTION BASINMAX12309 END DISPLY-INF01 END DISPLY COPY TIMESERIES # - # NPT NMN \*\*\* 1 1 501 1 1 1 END TIMESERIES END COPY GENER OPCODE # # OPCD \*\*\* END OPCODE PARM K \*\*\* # # END PARM END GENER PERLND GEN-INFO <PLS ><-----Name---->NBLKS Unit-systems Printer \*\*\* User t-series Engl Metr \*\*\* # - # in out 1 1 1 1 27 \* \* \* 33 C,Grass,Flat(0-1%) 0 END GEN-INFO \*\*\* Section PWATER\*\*\* ACTIVITY # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC \*\*\* 33 0 0 1 0 0 0 0 0 0 0 0 0 END ACTIVITY PRINT-INFO # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC \*\*\*\*\*\*\*\*

0 0 4 0 0 0 0 0 0 0 0 1 9 33 END PRINT-INFO PWAT-PARM1 <PLS > PWATER variable monthly parameter value flags \*\*\* 

 # # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE INFC HWT \*\*\*

 33
 0
 0
 1
 0
 0
 1
 0

 END PWAT-PARM1 PWAT-PARM2 <PLS > PWATER input info: Part 2 \*\*\*
# - # \*\*\*FOREST LZSN INFILT LSUR SLSUR KVARY AGWRC
33 0 4.5 0.045 400 0.01 3 0.92 <PLS > 33 END PWAT-PARM2 PWAT-PARM3 <PLS > PWATER input info: Part 3 \*\*\* 
 # # \*\*\*PETMAX
 PETMIN
 INFEXP

 33
 40
 35
 2
 INFILD DEEPFR BASETP 2 0 0 AGWETP 0.05 2 END PWAT-PARM3 PWAT-PARM4 
 <PLS >
 PWATER input info: Part 4
 \*\*\*

 # - #
 CEPSC
 UZSN
 NSUR
 INTFW
 IRC
 LZETP \*\*\*

 33
 0
 0.3
 0.25
 0.7
 0.5
 0
 0 END PWAT-PARM4 MON-LZETPARM <PLS > PWATER input info: Part 3 33 END MON-LZETPARM MON-INTERCEP <PLS > PWATER input info: Part 3 \*\*\* 

 # - # JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC \*\*\*

 33
 0.12
 0.12
 0.12
 0.11
 0.1
 0.1
 0.1
 0.1
 0.11
 0.12

 END MON-INTERCEP PWAT-STATE1 <PLS > \*\*\* Initial conditions at start of simulation ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 \*\*\* 
 # # \*\*\* CEPS
 SURS
 UZS
 IFWS
 LZS
 AGWS

 33
 0
 0
 0.15
 0
 4
 0.05
 GWVS 0 END PWAT-STATE1 END PERLND IMPLND GEN-INFO <PLS ><-----Name----> Unit-systems Printer \*\*\* User t-series Engl Metr \*\*\* # - # in out \*\*\* 1 Imperv,Flat(0-1%) 1 1 27 0 END GEN-INFO \*\*\* Section IWATER\*\*\* ACTIVITY \* \* \* END ACTIVITY PRINT-INFO <ILS > \*\*\*\*\*\*\* Print-flags \*\*\*\*\*\*\* PIVL PYR # - # ATMP SNOW IWAT SLD IWG IQAL \*\*\*\*\*\*\*\* 1 0 0 4 0 0 1 9 END PRINT-INFO IWAT-PARM1 <PLS > IWATER variable monthly parameter value flags \*\*\* # - # CSNO RTOP VRS VNN RTLI \*\*\* 1 0 0 0 0 0

END IWAT-PARM1 IWAT-PARM2 
 <PLND2</th>
 IWATER input info: Part 2
 \*

 # - # \*\*\* LSUR
 SLSUR
 NSUR
 RETSC

 1
 100
 0.01
 0.05
 0.1
 \* \* \* END IWAT-PARM2 IWAT-PARM3 IWATER input info: Part 3 \* \* \* <PLS > # - # \*\*\*PETMAX PETMIN 1 0 0 1 END IWAT-PARM3 IWAT-STATE1 <PLS > \*\*\* Initial conditions at start of simulation # - # \*\*\* RETS SURS 1 0 0 END IWAT-STATE1 END IMPLND SCHEMATIC <--Area--> <-Target-> MBLK \*\*\* <-factor-> <Name> # Tbl# \*\*\* <-Source-> <Name> # DMA B-2\*\*\* 8.04 RCHRES 1 2 8.04 RCHRES 1 3 1.04 RCHRES 1 5 PERLND 33 PERLND 33 IMPLND 1 \*\*\*\*\*Routing\*\*\*\*\* 8.04 COPY 1 12 1.04 COPY 1 15 8.04 COPY 1 13 1 COPY 501 17 PERLND 33 IMPLND 1 PERLND 33 RCHRES 1 END SCHEMATIC NETWORK <-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> \*\*\* <Name> # <Name> # # <Name> # # <Name> # # <Name> # # \*\*\*
COPY 501 OUTPUT MEAN 1 1 12.1 DISPLY 1 INPUT TIMSER 1 <-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> \*\*\* <Name> # <Name> # #<-factor->strg <Name> # # <Name> # # \*\*\* END NETWORK RCHRES GEN-INFO RCHRES Name Nexits Unit Systems Printer \* \* \* \* \* \* # - #<----> User T-series Engl Metr LKFG \* \* \* in out 1 BIORETENTION BA-005 2 1 1 1 28 0 1 END GEN-INFO \*\*\* Section RCHRES\*\*\* ACTIVITY END ACTIVITY PRINT-INFO \* \* \* \* \* \* \* \* \* 1 END PRINT-INFO

HYDR-PARM1

	VC A1 A2	FG possil	G for each ole exit	n *** ODGTFG *** possik * * *		FUNCT for possible e ***	
1 END HYDR-		0 4	5000	) 0 0	0 0 0	2 2 2	2 2
HYDR-PARM # - #	FTABNO		DELTH			DB50	* * *
<>< 1 END HYDR- HYDR-INIT	PARM2		<> 0.0	<>< 73.33	0.5		* * *
RCHRES # - # *	Initial *** VOL ** ac-ft	Initia for eac	al value ch possibl	HYDR section of COLIND le exit	Initia for eac	l value of OU h possible exit ><>	*** TDGT
1 END HYDR- END RCHRES	0					0.0 0.0 0.0	
SPEC-ACTION END SPEC-AC FTABLES FTABLE 91 5							
Depth (ft) 0.000000 0.016667 0.033333 0.050000 0.066667 0.083333 0.100000 0.116667 0.133333 0.150000 0.166667 0.183333 0.200000 0.216667 0.233333 0.250000	0.725455 0.726273 0.727091 0.727911 0.728730 0.729550 0.730371 0.731192 0.732013 0.732835 0.733657 0.734480 0.735304 0.736127 0.736952 0.737776	<pre>(acre-ft) 0.000000 0.012098 0.024209 0.036334 0.048473 0.060625 0.072791 0.084971 0.097164 0.109371 0.121592 0.133827 0.146075 0.158337 0.170612 0.182902 0.195205 0.207522 0.219852 0.232197 0.244555</pre>	(cfs) 0.000000 0.000000 0.000000 0.000000 0.000000	(cfs) 0.000000 0.024386 0.024414 0.024414 0.024441 0.024469 0.024524 0.024524 0.024552 0.024579 0.024607 0.024607 0.024607 0.0246490 0.024662 0.024662 0.0246490 0.024745 0.024745 0.024745 0.024745 0.024745 0.024828 0.024828 0.024884 0.024856 0.024884 0.024884 0.024884 0.024884 0.024995 0.024995 0.025023 0.025078 0.025078 0.025186 0.025186 0.025218 0.025302 0.025330 0.025330 0.025386 0.025442 0.025442 0.025470		Travel Time*** (Minutes)***	

0.700000 0.716667 0.733333 0.750000 0.766667 0.783333 0.800000 0.816667 0.833333 0.850000 0.866667 0.883333 0.900000 0.916667 0.933333 0.950000 0.966667 0.983333 1.000000 1.016667 1.033333 1.050000 1.066667 1.083333 1.150000 1.16667 1.133333 1.150000 1.216667 1.233333 1.250000 1.266667 1.283333 1.250000 1.266667 1.283333 1.350000 1.316667 1.33333 1.350000 1.316667 1.383333 1.350000 1.316667 1.383333 1.450000 1.416667 1.483333 1.450000 1.46667 1.483333 1.450000 1.46667 1.483333 1.450000 1.46667 1.483333 1.450000 1.46667 1.483333 1.50000 1.46667 1.483333 1.50000 1.46667 1.483333 1.50000 1.46667 1.483333 1.50000 1.46667 1.483333 1.450000 1.46667 1.48333 1.450000 1.46667 1.48333 1.450000 1.46667 1.48333 1.450000 1.46667 1.48333 1.450000 1.46667 1.483333 1.450000 1.46667 1.483333 1.450000 1.46667 1.483333 1.450000 1.46667 1.483333 1.450000 1.46667 1.483333 1.450000 1.46667 1.483333 1.450000 1.46667 1.483333 1.450000 1.46667 1.483333 1.450000 1.4667 1.483333 1.450000 1.4667 1.483333 1.450000 1.4667 1.483333 1.450000 1.4667 1.483333 1.450000 1.4667 1.483333 1.450000 1.4667 1.483333 1.450000 1.4667 1.483333 1.450000 1.4667 1.483333 1.450000 1.4667 1.483333 1.450000 1.4667 1.483333 1.450000 1.4667 1.483333 1.450000 1.4667 1.483333 1.450000 1.4667 1.483333 1.50000 1.4667 1.483333 1.450000 1.4667 1.483333 1.	0.800934	0.519937 0.532615 0.545306 0.558011 0.570730 0.583463 0.596210 0.608971 0.621747 0.634536 0.647339 0.660156 0.672987 0.685833 0.698692 0.711565 0.724453 0.737354 0.750270 0.763200 0.776144 0.789102 0.802074 0.815060 0.841074 0.854103 0.841074 0.854103 0.841074 0.854103 0.841074 0.906359 0.919458 0.932572 0.945700 0.958842 0.971998 0.92572 0.945700 0.958842 0.971998 0.924766 1.024766 1.037993 1.051235 1.064491 1.077761 1.091045 1.104344 1.17657 1.130985 1.144327	0.000000 0.051590 0.145865 0.267884 0.412312 0.576057 0.757015 0.953616 1.164619 1.388990 1.625839 1.874373 2.133870 2.403654 2.971547 3.268438 3.573165 3.885141 4.203780 4.528495 4.858696 5.193790 5.533178 5.876256 6.222418 6.571051 6.921538 7.273260 7.625595	0.025526 0.025526 0.025582 0.025611 0.025639 0.025639 0.025695 0.025723 0.025723 0.025723 0.025780 0.025808 0.025808 0.025803 0.025803 0.025921 0.025921 0.026035 0.026035 0.026035 0.026035 0.026120 0.026120 0.026120 0.026120 0.026234 0.026234 0.026234 0.026234 0.026234 0.026234 0.026234 0.026234 0.026234 0.026234 0.026234 0.0264291 0.026442 0.026445 0.026445 0.026445 0.026445 0.026520 0.026548 0.026577 0.026520 0.026548 0.026577 0.026634 0.026577 0.026634 0.026577 0.026634 0.026749 0.026749 0.026749 0.026749 0.026749 0.026749 0.026749 0.026836 0.026893				
WDM         2         H           WDM         2         H           WDM         1         H           WDM         1         H           WDM         2         H           WDM         2         H	<name> # t PREC E PREC E EVAP E EVAP E PREC E</name>		actor->strg 44 44 5 5 44		# # 1 999 1 999 1 999		<name> PREC PREC PETINP PETINP PREC</name>	
END EXT SOURCES								
RCHRES 1 H			Mult>Tran actor->strg 1 1 1	<name> WDM 10 WDM 10</name>		me> W 1 W 1	Isys Tgap tem strg ENGL ENGL ENGL	

RCHRES 1 HYDR COPY 1 OUTPUT COPY 501 OUTPUT END EXT TARGETS	MEAN	1 1 1 1 1 1	1 12.1 12.1	WDM 701	FLOW H	NGL REPI NGL REPI NGL REPI	
MASS-LINK <volume> &lt;-Grp&gt; <name> MASS-LINK PERLND PWATER END MASS-LINK</name></volume>	<name> 2 SURO</name>	# #<-fac	tor->	<target> <name> RCHRES</name></target>	<-Grp	- <-Member->** <name> # #** / IVOL</name>	
MASS-LINK PERLND PWATER END MASS-LINK	3 IFWO 3	0.08	3333	RCHRES	INFLO	IVOL	
MASS-LINK IMPLND IWATER END MASS-LINK	5 SURO 5	0.08	3333	RCHRES	INFLO	IVOL	
MASS-LINK PERLND PWATER END MASS-LINK		0.08	3333	COPY	INPUT	MEAN	
MASS-LINK PERLND PWATER END MASS-LINK	13 IFWO 13	0.08	3333	COPY	INPUT	MEAN	
MASS-LINK IMPLND IWATER END MASS-LINK	15 SURO 15	0.08	3333	COPY	INPUT	MEAN	
MASS-LINK RCHRES OFLOW END MASS-LINK	17 OVOL 17	1		COPY	INPUT	MEAN	

END MASS-LINK

END RUN

# Pre-Project HSPF Message File

Mitigated HSPF Message File

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