

# LEVEL 3 DRAINAGE STUDY FOR BEECH AVE & ROLOFF WAY TENTATIVE PARCEL MAP

**Prepared for:**

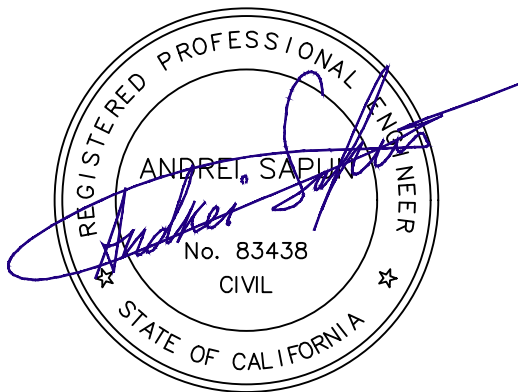
Toni Conners  
P.O. Box 725  
Orangevale, CA 95662

**APN:**

261-0250-032 and 033

**Planning Application Number:**

PLNP2022-00180



8/19/2024

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**Job Number:**

21068

**Preparation Date:**

September 2022

**Last Updated On:**

August 19, 2024

Vertical Datum: NAVD88  
Watershed: Arcade Creek

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## **1.0 INTRODUCTION**

### **1.1 PROJECT DESCRIPTION AND EXISTING CONDITIONS**

This drainage study is prepared to support Beech Avenue and Roloff Way Tentative Parcel Map (TPM). Vicinity map and TPM are provided in the Appendix 1. Project objective is to split existing parcel and an adjacent vacant parcel into four lots with remainder lot. Remainder lot will include existing house at 6018 Roloff Way.

Existing conditions include 0.37-acre parcel # 261-0250-033 with existing house (estimated impervious area is 40%) and 2.26-acre vacant parcel # 261-0250-032 (estimated impervious area is 2%). Project is not located in the FEMA floodplain hazard area. According to USDA-NRCS soil survey data, project soil consists of 25% Orangevale, 6% Kaseburg, 20% Urban Land, 40% Fiddymment, 6% Xerarents, and 3% unknown soil profiles. Orangevale is considered as Type B soil, however, Kaseburg, Urban Land and Fiddymment are type D hydrologic groups.

### **1.2 APPLICABLE STANDARDS**

The design computations shall be in conformance with Sections 9 of the Sacramento County Improvement Standards and Volume 2 of the Sacramento City/County Drainage Manual. LID measures shall comply with Stormwater Quality Design Manual for Sacramento Region.

### **1.3 PREVIOUS STUDIES**

No previous studies are available for the project area.

### **1.4 STUDY OBJECTIVES**

Per Sacramento County Department of Water Resources request dated August 4, 2022, the current study analyses the following list of items:

1. On-site and off-site drainage shed maps. Based on existing topography it seems like some of the western portion of Beech Avenue would drain towards the property. Please verify and include or not include in the off-site drainage shed map as necessary.
2. A discussion of existing conditions and proposed conditions on the project site.
3. Provide an evaluation of the impacts of the increase in 100-year runoff due to the change in land use and the proposed development. The report should summarize that there are no significant impacts, with mitigation as necessary.
4. A 100-year runoff analysis will be required to determine how the drainage will be affected by the proposed development of the project site. Use SacCalc for hydrologic calculations and provide the SacCalc model files with the study submittal.
5. An analysis and discussion of overland release flow and point(s) of discharge off the project site.
  - a. The site should release runoff at the project boundaries without impacting upstream or downstream properties.



- b. Minimum pad elevations will need to be above the overland release control elevation.
6. Provide a preliminary grading and drainage plan. Where are the lots proposed to drain? If the lots will drain to the roads and be picked up by the existing storm drain systems, the project will need to analyze the existing systems capacity (this can be done at a design-level Level 4 study prior to improvement plan submittal). If the lots will utilize the public storm drain pipe to the west, then the project will likely need to upsize the public storm drain line, which lies in the backyards of multiple properties to the west. This will likely be a complicated option and would require an analysis now (in this Level 3 study). A drainage facility map is attached for your reference.

## 2.0 EXISTING CONDITIONS

### 2.1 HISTORICAL LAND USE AND TOPOGRAPHY

The existing project land use is LDR (Low Density Residential). Total project area is 2.6± acres. Existing topography varies from 2% to 8% slopes. The current drainage generally runs from east to west as shown in the Existing Shed Map in Appendix 3. Existing shed map is determined using Sacramento County LIDAR information. The project site does have two watersheds that drain to the west and to the south. The first watershed drains to the west towards Illinois Avenue via the existing 15' drainage easement and the second watershed drains to the south towards Kevmich Way.

### 2.2 EXISTING CONDITIONS MODELING

Hydrologic modeling for existing conditions was made using Sacramento Hydrologic Calculator (SacCalc), a Microsoft Windows application developed for Sacramento County. SacCalcs model of existing watershed schematics is provided in Figures 1 below.

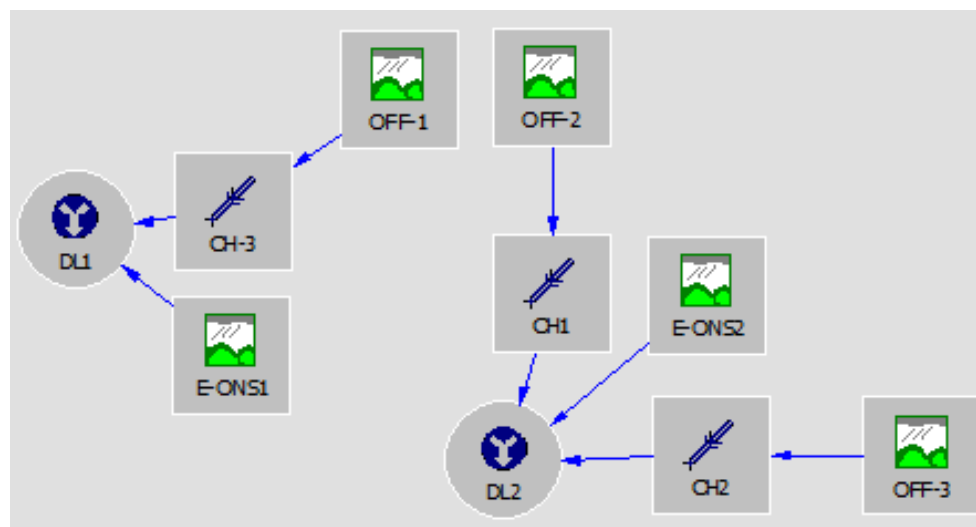


Figure 1 - SacCalcs Model Schematics for Existing Conditions

The existing project site has two discharge locations: to the west (DL-1) towards existing drain inlet and overland release easement, and to the south (DL-2) towards Kevmich Way. OFF-1 represents a 0.17-acre offsite watershed that drains from Rollof Way into the project site. E-ONS1 represents the existing 1.12-acre watershed which drains to the west towards Illinois Avenue. It consists of 0.19-acre of developed portion of parcel with APN 261-0250-033. OFF-2 represents a 2.44-acre offsite watershed area that drains to the site consisted of RD10 and RD4 zoning areas. E-ONS2 represents 1.54-acre project site area with the majority of the developed parcel with APN 261-0250-033 and vacant project site area. OFF-3 represents 0.25-acre offsite shed from developed RD-4 zone. CH1, CH2 and CH3 represent the distance that offsite watersheds have to flow through project site before reaching the discharge locations.

The soil type for all watersheds was selected as Type 'D' since it is the prevailing soil type as stated in USGS soil map for the area.

Lag transformation and infiltration loss rate are selected to be computed. All watersheds are selected as developed except for the vacant portions of the project site. Developed watershed portion consists of RD-4 and RD-10 zoning. 100-year peak for 24-hour flows were calculated using Zone 3 for the Sacramento Method (100-year event). Modeling results are summarized in Table below and Modeling Report is provided in Appendix 5.

**Table 1 - Sacramento Method Results for Existing Conditions**

Watershed, ID	Area, ac	Peak flow, (cfs)	Time of Peak (hours)
E-ONS1	1.12	3.9	12:03
E-ONS2	1.54	4.1	12:07
OFF-1	0.17	0.6	12:02
OFF-2	2.44	9.0	12:02
OFF-3	0.25	0.9	12:02
CH1	2.44	8.8	12:04
CH2	0.25	0.9	12:07
CH3	0.17	0.6	12:04
DL1	1.29	4.5	12:03
DL2	4.23	13.0	12:03

### **3.0 PROPOSED CONDITIONS**

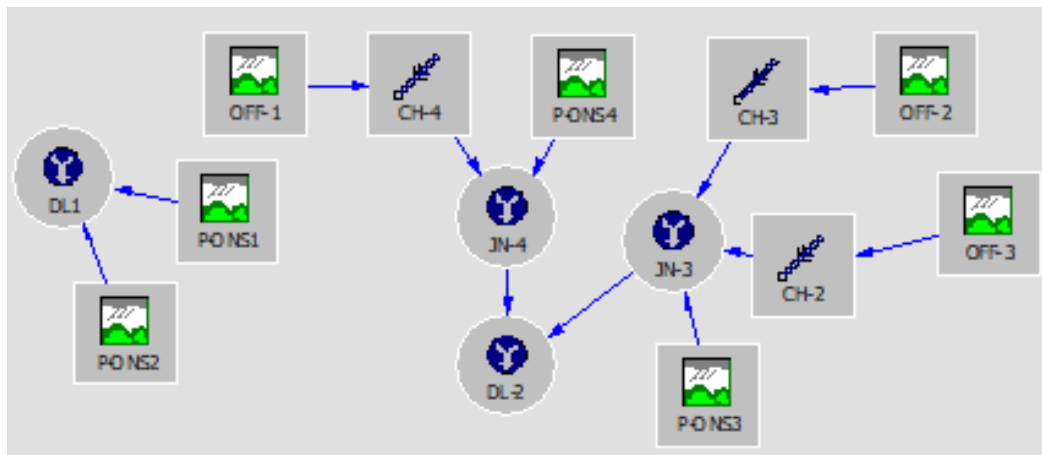
#### **3.1 PROPOSED LAND USE**

The vacant project site is proposed to split into 4 lots on a 2.63-acre project area. For the design purposes, all resultant lots are assumed to be developed to RD-4 zone density. However, no lots of improvements are proposed at this point and the existing topography will remain. No pads grading is proposed under the current application and current drainage patterns will remain as in existing conditions. The proposed shed map is provided in Appendix 3. Any lot grading and

improvement plans will be done separately by future owner or owners. Ultimately, when all parcels will be developed, it will be a minor shift in drainage: offsite watershed north of remainder parcel and remain parcel itself will drain south towards Kevmich Way due to the road, that will connect Roloff Way and Kevmich Way.

### 3.2 PROPOSED CONDITIONS MODELING

A public road is proposed to connect Roloff Way and Kevmich Way. Because of the road, some of the runoff which was draining towards the west in existing conditions, will drain south instead. Therefore, the drainage to the west will be reduced, but drainage to the south will be increased after development. Hydrologic modeling schematics for the proposed conditions are provided in Figures 2 below.



**Figure 2 - SacCalcs Model Schematics for Proposed Conditions**

OFF-1, 2 and 3 represent offsite watersheds which were discussed previously. CH-2 and 3 represent overland release routes for the offsite watersheds through the project site. CH-4 represents the proposed public road. P-ONS1, 2, 3 and 4 represent proposed parcels as shown in the TPM.

Land use for all project site watersheds is selected as RD-4. Even so the proposed Parcel 4 is larger than 1 acre and the Remainder Parcel area will be almost half an acre, an ultimate development of RD-4 is used which will allow future splits into RD-4 lots without any drainage impacts analysis for the impacts. Proposed drainage patterns are provided in the Overland Release Exhibit in the Attachment 4. Soil type for all watersheds as well as zoning assumptions were kept the same as in existing conditions. Lag transformation and infiltration loss rate are selected to be computed automatically. Modeling results are summarized in the Table below.

**Table 2 - Sacramento Method Results for Proposed Conditions**

Watershed, ID	Area, ac	Peak flow, (cfs)	Time of Peak (hours)
P-ONS1	0.36	1.3	12:02
P-ONS2	0.40	1.5	12:02
P-ONS3	1.24	4.2	12:03
P-ONS4	0.67	2.5	12:03
OFF-1	0.17	0.6	12:02
OFF-2	2.44	9.0	12:02
OFF-3	0.25	0.9	12:02
CH-2	2.69	0.9	12:04
CH-3	2.44	8.9	12:03
CH-4	0.17	0.6	12:03
JN-3	3.93	14	12:03
JN-4	0.84	3.1	12:02
DL1	0.76	2.8	12:02
DL2	4.77	17.0	12:03

### 3.3 ANALYSIS

Project area (2.67 acres) watershed is relatively small part of the existing watershed (28.5 acres) that drains to the Windshire Lane where drainage runoff from both project discharge locations, described in previous section, are merging together as shown in the Existing Watershed Map. Based on the SacCalc modeling results, runoff towards DL1 for 100-year event will be reduced in peak discharge from 4.5 cfs to 2.8 cfs due to smaller shed draining to the west after development as described in Section 3.1. However, due to the increase in its watershed area and due to proposed development, peak runoff towards Kevmich Way will be increased from 13 cfs to 17 cfs. This increase lasts only for 6 minutes (from 12:01 to 12:07). Sacramento County does allow an increase of water surface elevation (WSE) up to 0.1 ft. Existing and proposed conditions WSE were analyzed using Hydroflow Express software comparing existing and proposed project scenarios in form of open channel calculations for the Kevmich Way road. Design reports for Kevmich Way at the discharge location will be increased from 239.3 ft to 239.32 ft, see Figures 3 and 4 below. A 0.02 ft increase of WSE is an acceptable range by the County.

Project will have insignificant impact downstream at Windshire Lane due to its relative size and County's acceptable runoff increase. However, to address existing drainage concerns, the project was further analyzed for possible storage onsite to meet existing drainage volume after development. See next sections for onsite storage modeling.

# Channel Report

Hydroware Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Friday, Aug 16 2024

## 100-YR PEAK DISCHARGE THROUGH KEVMICH WAY AT EXISTING CONDITIONS

### User-defined

Invert Elev (ft) = 238.93  
Slope (%) = 2.00  
N-Value = 0.016

### Calculations

Compute by: Known Q  
Known Q (cfs) = 13.00

### Highlighted

Depth (ft) = 0.37  
Q (cfs) = 13.00  
Area (sqft) = 4.43  
Velocity (ft/s) = 2.94  
Wetted Perim (ft) = 36.52  
Crit Depth, Yc (ft) = 0.42  
Top Width (ft) = 36.40  
EGL (ft) = 0.50

### (Sta, El, n)-(Sta, El, n)...

(96.00, 241.28, 0.016)-(116.00, 239.28, 0.016)-(120.00, 239.23, 0.016)-(120.83, 238.93, 0.016)-(123.00, 239.08, 0.016)-(136.00, 239.34, 0.016)-(149.00, 239.08, 0.016)-(151.17, 238.93, 0.016)-(152.00, 239.23, 0.016)-(156.00, 239.28, 0.016)-(176.00, 241.28, 0.016)

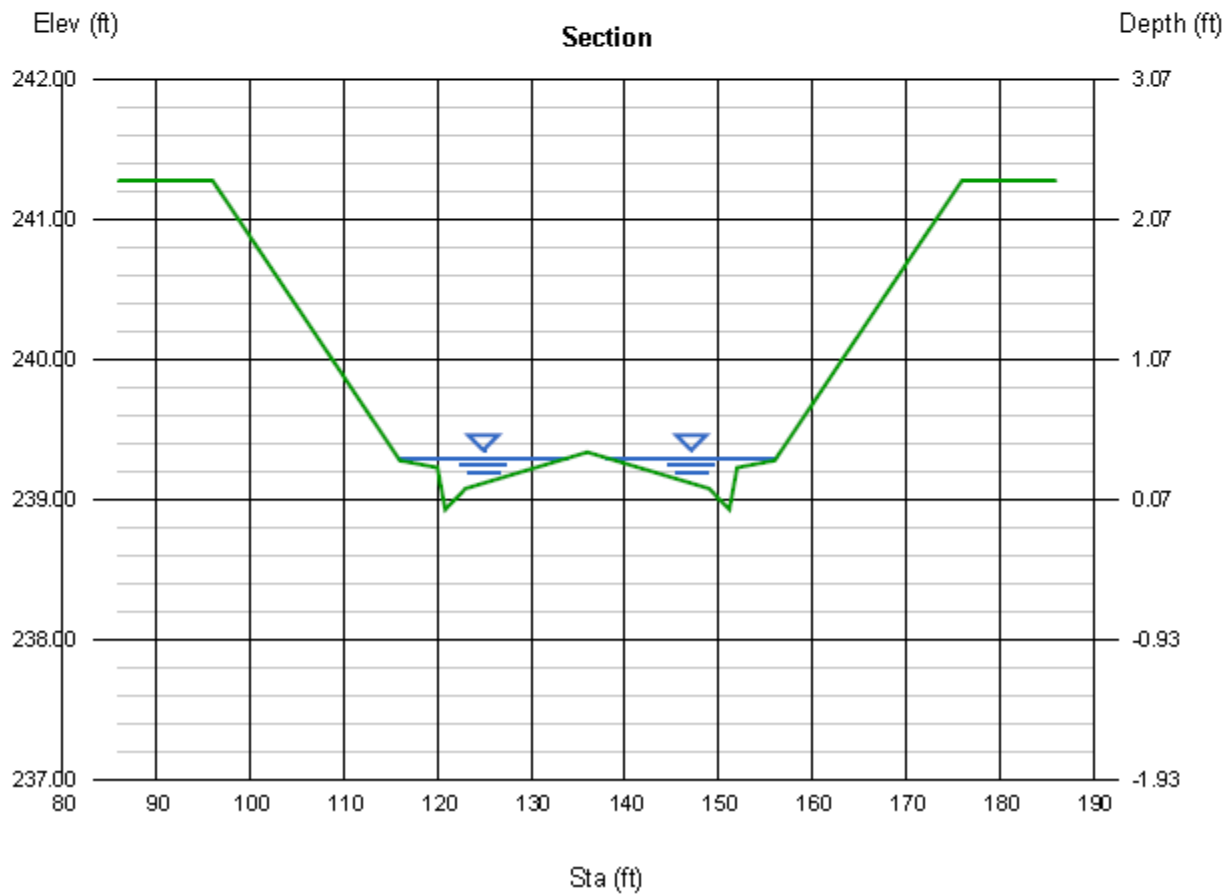


Figure 3 - 100-yr WSE for Existing Conditions

# Channel Report

Hydroware Express Extension for Autodesk AutoCAD Civil 3D by Autodesk, Inc.

Friday, Aug 16 2024

## 100-YR PEAK DISCHARGE THROUGH KEVMICH WAY AT PROPOSED CONDITIONS

### User-defined

Invert Elev (ft) = 238.93  
Slope (%) = 2.00  
N-Value = 0.016

### Highlighted

Depth (ft) = 0.39  
Q (cfs) = 17.00  
Area (sqft) = 5.18  
Velocity (ft/s) = 3.28  
Wetted Perim (ft) = 38.92  
Crit Depth, Yc (ft) = 0.44  
Top Width (ft) = 38.80  
EGL (ft) = 0.56

### Calculations

Compute by: Known Q  
Known Q (cfs) = 17.00

### (Sta, El, n)-(Sta, El, n)...

(96.00, 241.28)-(116.00, 239.28, 0.016)-(120.00, 239.23, 0.016)-(120.83, 238.93, 0.016)-(123.00, 239.08, 0.016)-(136.00, 239.34, 0.016)-(149.00, 239.08, 0.016)  
-(151.17, 238.93, 0.016)-(152.00, 239.23, 0.016)-(156.00, 239.28, 0.016)-(176.00, 241.28, 0.016)

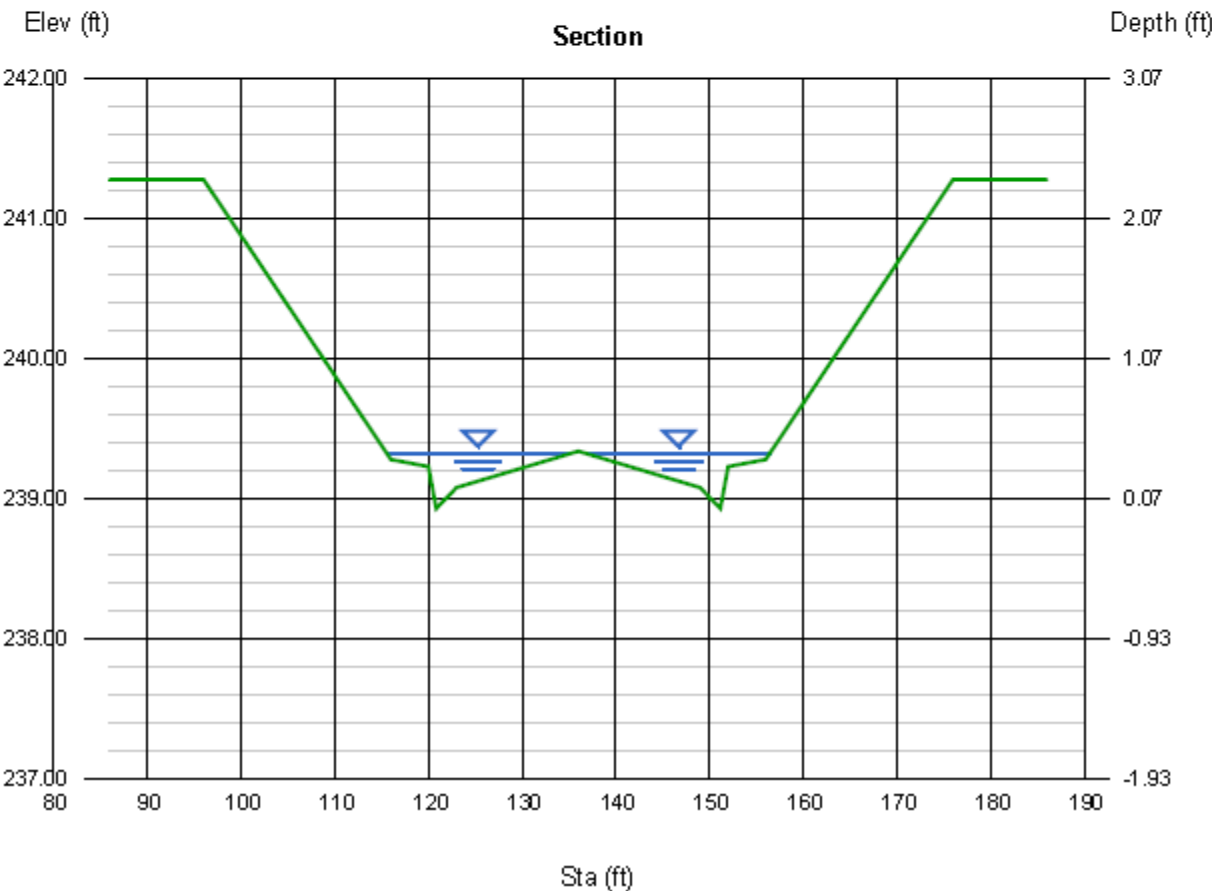


Figure 4 - 100-yr WSE for Proposed Conditions

### **3.4 100-YR RUNOFF STORAGE**

Project can provide onsite storage in public street as long as its ponding will be below 1 ft. Due to significant elevation difference between Roloff Way and Kevmich Way, maximum storage in the proposed street is estimated around 670 cf which will not be enough to meet pre-development levels. However, if additional storage will be applied at Parcel 4, then project development runoff can match and be even lower than at existing conditions at Kevmich Way. This storage can be achieved via private road at Parcel 4 in case it will be further subdivided to match RD-4 zoning or via onsite storage. Since further subdivision of Parcel 4 and its analysis goes beyond proposed Tentative Map, a basic rectangular storage was modeled to show effect of storage to the proposed runoff volume. A hydrograph information from SacCalc model which flows through Parcel 4 was routed via a rectangular storage of 30ft by 50ft with 3 to 1 side slope and 2.5ft depth (watersheds P-ONS3, OFF-2 and OFF-3). Pond discharge was modeled using box inlet (2ft by 2ft) as pond riser, 12" pipe that will connect private inlet to public drain system and 20 ft wide weir on top of the pond with elevation at 239.0 ft. Street storage was modeled based on the assumption that it is a sage with incremental storage of 9 cf for 0.1 ft depth, 140 cf for 0.4 ft depth, and 785 cf for 0.9 ft depth. The result of onsite storage and street storage is that post-development peak runoff can be reduced below existing volume (13.0 cf at existing conditions and 11.22 at post development). Please note, the project does not volunteer to reduce below existing volume. The intent of parcel 4 storage modeling is to show that project development impacts can be completely achieved. Hydrology Studio report is provided in Appendix 6.

NOTE: If parcel 4 will not be subdivided and no onsite storage will be placed on parcel 4, current project development still does not have a significant impact on downstream properties as described in Section 3.3. However, if parcel 4 will be subdivided in future, then there is a potential possibility to mitigate development impacts completely, which will have to be further analyzed under level 4 study.

## **4.0 REGIONAL FLOOD CONTROL**

### **4.1 OVERLAND RELEASE DISCHARGE LOCATION**

Project will have two overland release discharge locations as in existing conditions. The first discharge location will be located in the middle of the west boundary line at the east corner of parcels 261-0250-041 and 051. 100-yr runoff should be conveyed to Windshire Lane via an existing 15' wide drainage easement per subdivision map for Windsor Park (259 RM 10). The second discharge location is Kevmich Way as shown in the Overland Release Exhibit in Appendix 4.

### **4.2 UPSTREAM AND DOWNSTREAM IMPACTS**

No obstruction of flow is proposed for the offsite upstream drainage patterns therefore no impact to the upstream properties is anticipated. Also, there is no downstream impact to the west discharge location. There is an increase in flow to the south discharge location at Kevmich Way but it will not increase WSE significantly. However, there is a potential to reduce or lower post-development runoff via street storage or future onsite storage on parcel 4.

### **4.3 MINIMUM FINISH FLOOR**

The minimum finish floor elevation for the proposed parcels should be set to provide 1.5 ft clearance above flooding elevation. The minimum certified pads elevation should be set at 1.2 ft above the flooding elevations. For the proposed parcels 1, 2, 3 and 4 will be determined by either future road design or by overland release discharge elevation. Future level 4 study, that will be required to supplement the improvement plans, will set min elevation for the future houses.

## **5.0 WATER QUALITY**

### **5.1 PROPOSED LAND USE**

No rezoning is proposed for the project. The existing vacant project site will be split into residential lots. RD-4 land use is assumed for all parcels which will allow future parcels subdivision into RD-4 lots without any impact assessment. Proposed Tentative Parcel Map in Appendix 1.

### **5.2 LOW IMPACT DEVELOPMENT**

Single family residential projects with more than 1 acre of impervious area are required to include Low Impact Development (LID) control measures. The proposed Project impervious area includes 10,150 square feet of public right of way. About 30,000 sqft is reserved for future four houses, axillary structures and driveways. Since the total area is below 1 acre, the LID control measures are not applicable for the current project.

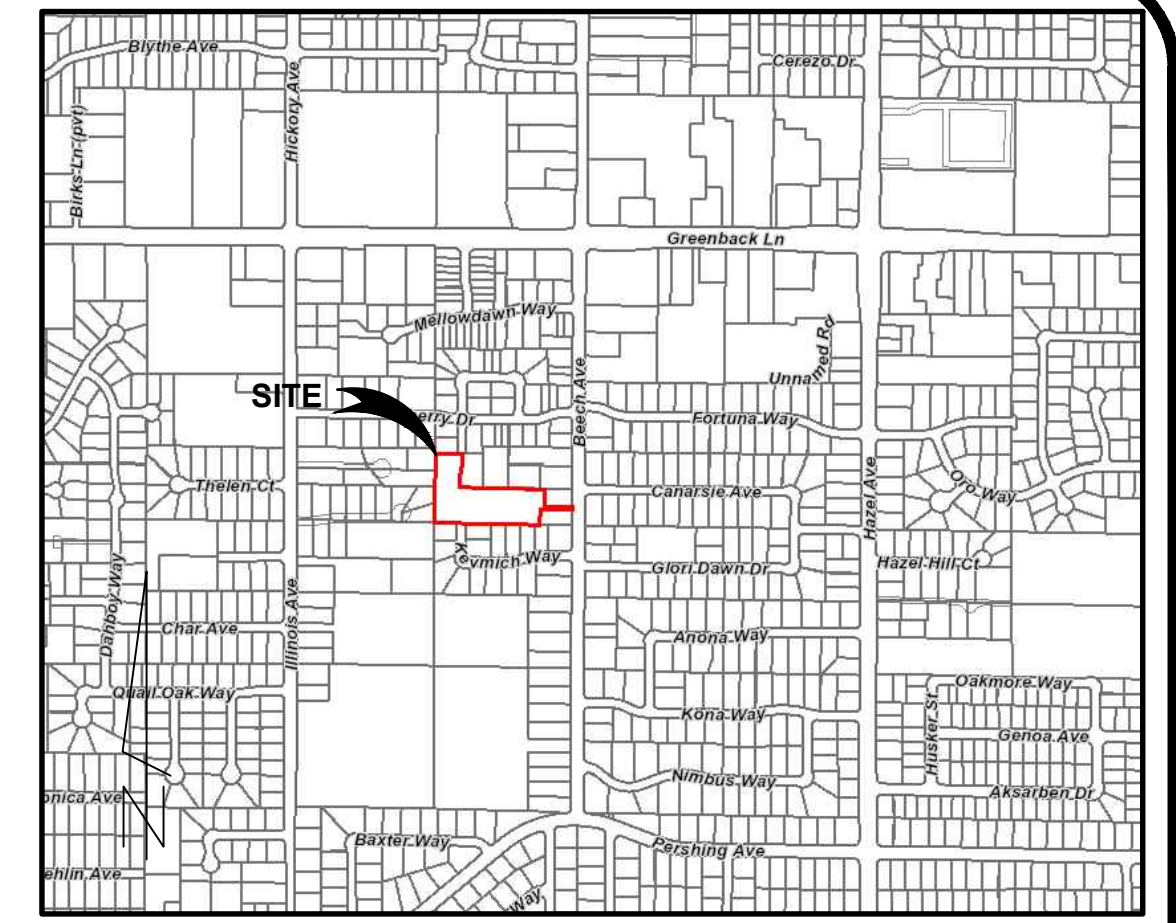
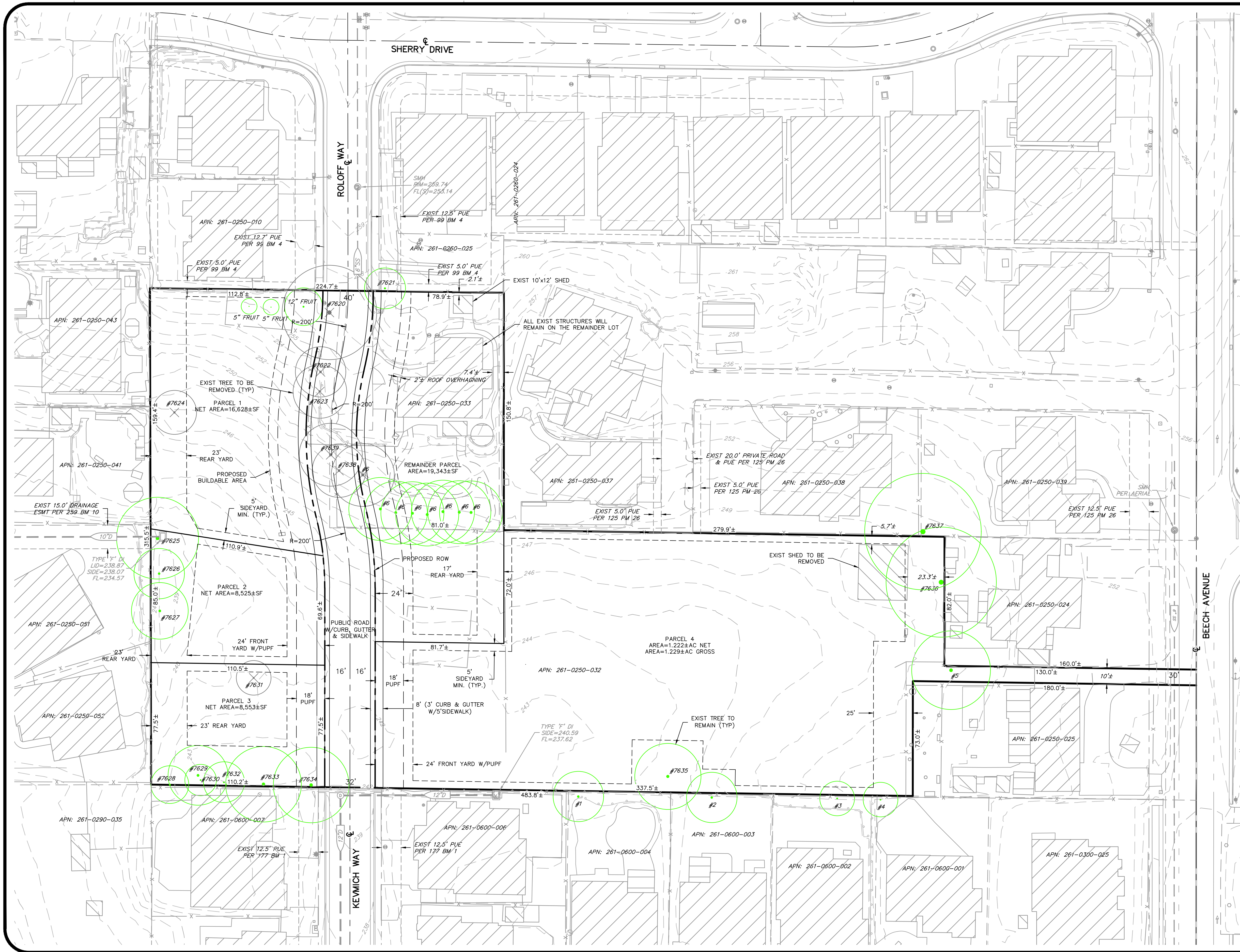
## **6.0 SUMMARY OF FINDINGS AND CONCLUSIONS**

Current drainage study was prepared to support Beech Avenue and Roloff Way TPM application. The proposed project does not have significant impacts to the existing storm drain systems or properties upstream or downstream from the development. Water surface elevation downstream or upstream will not increase beyond the County acceptable range. However, at the ultimate conditions, implementation of the runoff storage via proposed roads or onsite storage can reduce runoff to pre-existing conditions. LID requirements are not applicable to the proposed project. No grading prosed at this time since the project owner does not have plans to build houses right now. Land split is done for the future and separate grading plan for all parcels or for each parcel individually will be submitted separately for the review in the future. All supported calculations and reference documents are provided in the Appendices below.



**Appendix 1 - Project Vicinity Map and TPM Exhibit**





VICINITY MAP (NOT TO SCALE)	
TENTATIVE MAP INFORMATION	
APPLICANT:	TONI CONNERS P.O. BOX 725 ORANGEVALE, CA 95662
OWNER:	TONI CONNERS P.O. BOX 725 ORANGEVALE, CA 95662
ENGINEER:	AREA WEST ENGINEERS, INC. 7478 SANDALWOOD DRIVE, SUITE 400 CITRUS HEIGHTS, CA 95621 ATTN: RICHARD ROZUMOWICZ
ASSESSOR'S PARCEL NO:	261-0250-032 & 261-0250-033
EXISTING ZONING:	RD-4
PROPOSED ZONING:	RD-4
EXISTING PROJECT AREA:	2.69± AC GROSS 2.689± AC NET
EXISTING NUMBER OF LOTS:	2
EXISTING DENSITY:	2 PARCELS ON 2.69± AC OR 0.74 PARCELS PER 1 AC GROSS
PROPOSED NUMBER OF LOTS:	4 + 1 REMAINDER
PROPOSED DENSITY:	5 PARCELS ON 2.69± AC OR 2 PARCELS PER 1 AC GROSS
PROPOSED PARCEL 1 AREA:	16,628± SF (NET)
PROPOSED PARCEL 2 AREA:	8,525± SF (NET)
PROPOSED PARCEL 3 AREA:	8,553± SF (NET)
PROPOSED PARCEL 4 AREA:	1.22± AC (NET); 1.23± AC (GR)
PROPOSED REMAINDER AREA:	19,343± SF (NET)
SERVICE PROVIDERS:	SCHOOL DISTRICT: SAN JUAN UNIFIED PARKS DISTRICT: ORANGEVALE FIRE DISTRICT: SACRAMENTO METROPOLITAN FIRE STORM DRAIN: SACRAMENTO COUNTY SEWER DISTRICT: SACRAMENTO AREA SEWER DISTRICT WATER DISTRICT: ORANGE VALE WATER COMPANY ELECTRICITY: SMUD
LEGAL DESCRIPTION	A PORTION OF LOT 264 AS SHOWN ON 3 BM 20, SACRAMENTO COUNTY RECORDS.
BOUNDARY NOTE:	THIS MAP IS NOT AN OFFICIAL BOUNDARY SURVEY OF THE LAND DEPICTED HEREON AND SHOULD NOT BE RELIED ON FOR ANY PURPOSE OTHER THAN PLANNING PURPOSES. ALL BOUNDARY MARKERS, PROPERTY LINES AND EASEMENTS, IF SHOWN, ARE DRAWN USING EXISTING COUNTY RECORDED MAPS AND/OR DOCUMENTS.
TENTATIVE MAP NOTES	1. PARCEL LINES AND PARCEL AREAS MAY BE ADJUSTED AT THE TIME OF THE PARCEL MAP, PROVIDED NO ADDITIONAL PARCELS ARE CREATED AND SUBJECT TO THE APPROVAL OF THE SACRAMENTO COUNTY.



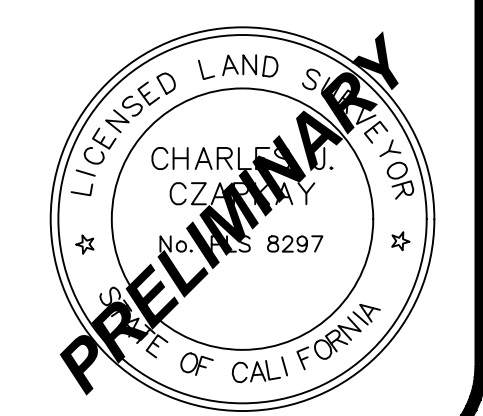
**AREA WEST ENGINEERS, INC.**  
ENGINEERING - SURVEYING - PLANNING  
7478 SANDALWOOD DRIVE, SUITE 400  
CITRUS HEIGHTS, CA 95621  
(916) 725-5551 (916) 725-5808 (FAX)  
AWE@AREAWESTENG.COM

TENTATIVE PARCEL MAP  
FOR  
6001 BEECH AVENUE & 6018 ROLOFF WAY  
APN's: 261-0250-032 & 033  
SACRAMENTO COUNTY  
STATE OF CALIFORNIA

REVISIONS

02/09/2023	- ADDED PUBLIC CUL-DE-SAC FROM ROLOFF WAY
03/15/2023	- REVISED PARCEL AREAS, ADDED SETBACK LINES
05/05/2023	- REVISED SETBACK LINE FOR PARCELS 3 AND 4
08/15/2023	- MINOR LOT LINE ADJUSTMENT BETWEEN PARCELS 1 & 2
06/05/2024	- REVISED LOT CONFIGURATION, RETAIN EXISTING TREES ON PARCEL 2

DATE: JUNE 2024	JOB NO. 21068
SCALE: 1"=30'	SHEET 1 OF 1



FOR REVIEW AND COMMENT ONLY

C:\UDL\_28\PROJECTS\108\Drawings\Final Parcel Map.dwg 6/6/2024 3:02:40 PM 1:1 AREA WEST ENGINEERS, INC.



**Appendix 2 – Project Soil Map Exhibit**



United States  
Department of  
Agriculture

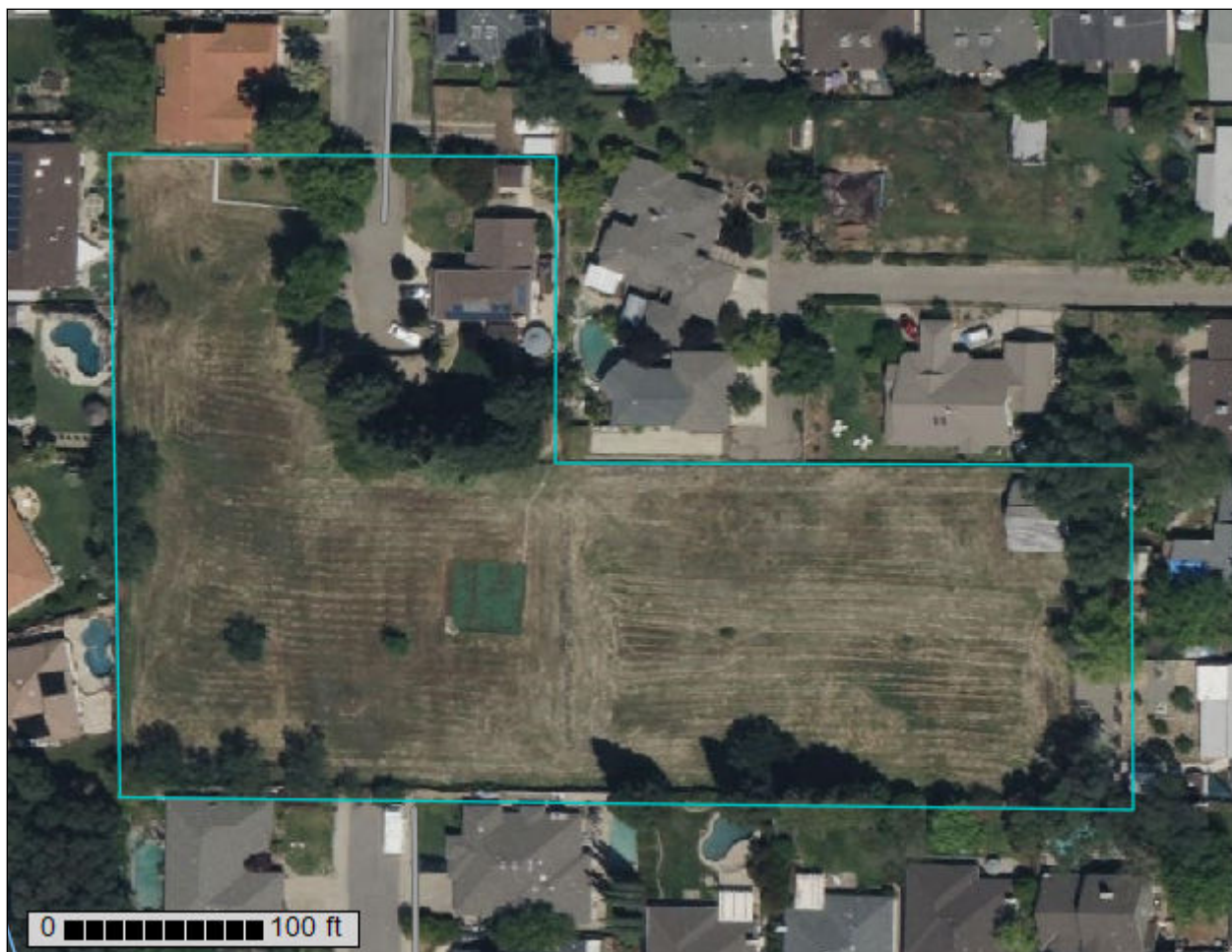
NRCS

Natural  
Resources  
Conservation  
Service

A product of the National  
Cooperative Soil Survey,  
a joint effort of the United  
States Department of  
Agriculture and other  
Federal agencies, State  
agencies including the  
Agricultural Experiment  
Stations, and local  
participants

# Custom Soil Resource Report for **Sacramento County, California**

**6001 Beech Ave (21068)**



September 2, 2022

# Preface

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Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist ([http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2\\_053951](http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951)).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

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# Soil Map

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The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



Custom Soil Resource Report

Soil Map



# Custom Soil Resource Report

## MAP LEGEND

### Area of Interest (AOI)

 Area of Interest (AOI)

### Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

### Special Point Features

 Blowout

 Borrow Pit


 Clay Spot


 Closed Depression

 Gravel Pit


 Gravelly Spot


 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water


 Perennial Water

 Rock Outcrop


 Saline Spot

 Sandy Spot

 Severely Eroded Spot

 Sinkhole

 Slide or Slip

 Sodic Spot

 Spoil Area

 Stony Spot

 Very Stony Spot

 Wet Spot

 Other


 Special Line Features

### Water Features

 Streams and Canals


### Transportation

 Rails


 Interstate Highways

 US Routes

 Major Roads

 Local Roads

### Background

 Aerial Photography

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
Web Soil Survey URL:  
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Sacramento County, California  
Survey Area Data: Version 20, Sep 3, 2021

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Apr 23, 2022—Apr 24, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
148	Fiddymment-Orangevale-Urban land complex, 2 to 8 percent slopes	2.8	100.0%
<b>Totals for Area of Interest</b>		<b>2.8</b>	<b>100.0%</b>

## Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

## Custom Soil Resource Report

onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## Sacramento County, California

### 148—Fiddymment-Orangevale-Urban land complex, 2 to 8 percent slopes

#### Map Unit Setting

*National map unit symbol:* hhmr

*Elevation:* 50 to 280 feet

*Mean annual precipitation:* 19 to 22 inches

*Mean annual air temperature:* 61 degrees F

*Frost-free period:* 230 to 300 days

*Farmland classification:* Not prime farmland

#### Map Unit Composition

*Fiddymment and similar soils:* 40 percent

*Orangevale and similar soils:* 25 percent

*Urban land:* 20 percent

*Minor components:* 15 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Fiddymment

##### Setting

*Landform:* Hills

*Landform position (two-dimensional):* Backslope

*Landform position (three-dimensional):* Side slope

*Down-slope shape:* Linear

*Across-slope shape:* Linear

*Parent material:* Residuum weathered from sedimentary rock

##### Typical profile

*H1 - 0 to 8 inches:* fine sandy loam

*H2 - 8 to 15 inches:* loam

*H3 - 15 to 28 inches:* sandy clay loam

*H4 - 28 to 40 inches:* indurated

*H5 - 40 to 44 inches:* weathered bedrock

##### Properties and qualities

*Slope:* 2 to 8 percent

*Depth to restrictive feature:* 28 to 40 inches to duripan; 40 to 44 inches to paralithic bedrock

*Drainage class:* Well drained

*Runoff class:* Very high

*Capacity of the most limiting layer to transmit water (Ksat):* Very low (0.00 to 0.00 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Available water supply, 0 to 60 inches:* Very low (about 2.7 inches)

##### Interpretive groups

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 4e

*Hydrologic Soil Group:* D

*Ecological site:* R017XY902CA - Duripan Vernal Pools

*Hydric soil rating:* No

## Description of Orangevale

### Setting

*Landform:* Terraces  
*Landform position (two-dimensional):* Toeslope  
*Landform position (three-dimensional):* Tread  
*Down-slope shape:* Concave  
*Across-slope shape:* Linear  
*Parent material:* Alluvium derived from granite

### Typical profile

*H1 - 0 to 15 inches:* coarse sandy loam  
*H2 - 15 to 20 inches:* coarse sandy loam  
*H3 - 20 to 72 inches:* sandy clay loam  
*H4 - 72 to 80 inches:* coarse sandy loam

### Properties and qualities

*Slope:* 2 to 8 percent  
*Depth to restrictive feature:* More than 80 inches  
*Drainage class:* Well drained  
*Runoff class:* Medium  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high to high  
(0.57 to 1.98 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Available water supply, 0 to 60 inches:* Low (about 5.8 inches)

### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 4e  
*Hydrologic Soil Group:* B  
*Hydric soil rating:* No

## Description of Urban Land

### Typical profile

*H1 - 0 to 6 inches:* variable

### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 8  
*Hydric soil rating:* No

## Minor Components

### Xerarents

*Percent of map unit:* 6 percent  
*Hydric soil rating:* No

### Kaseberg

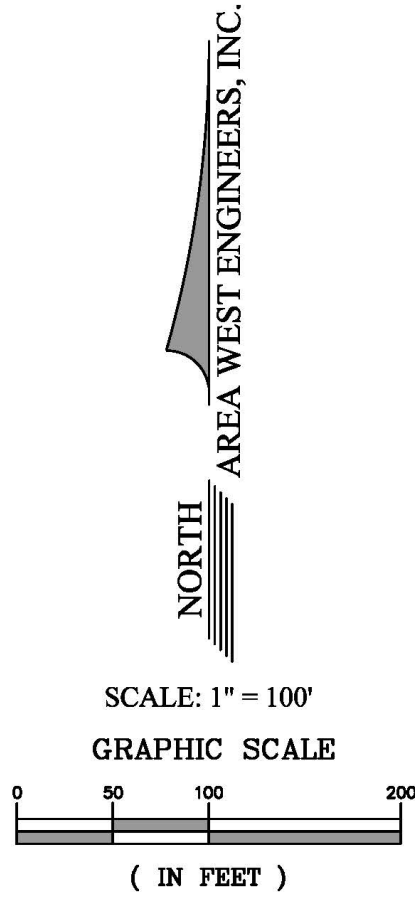
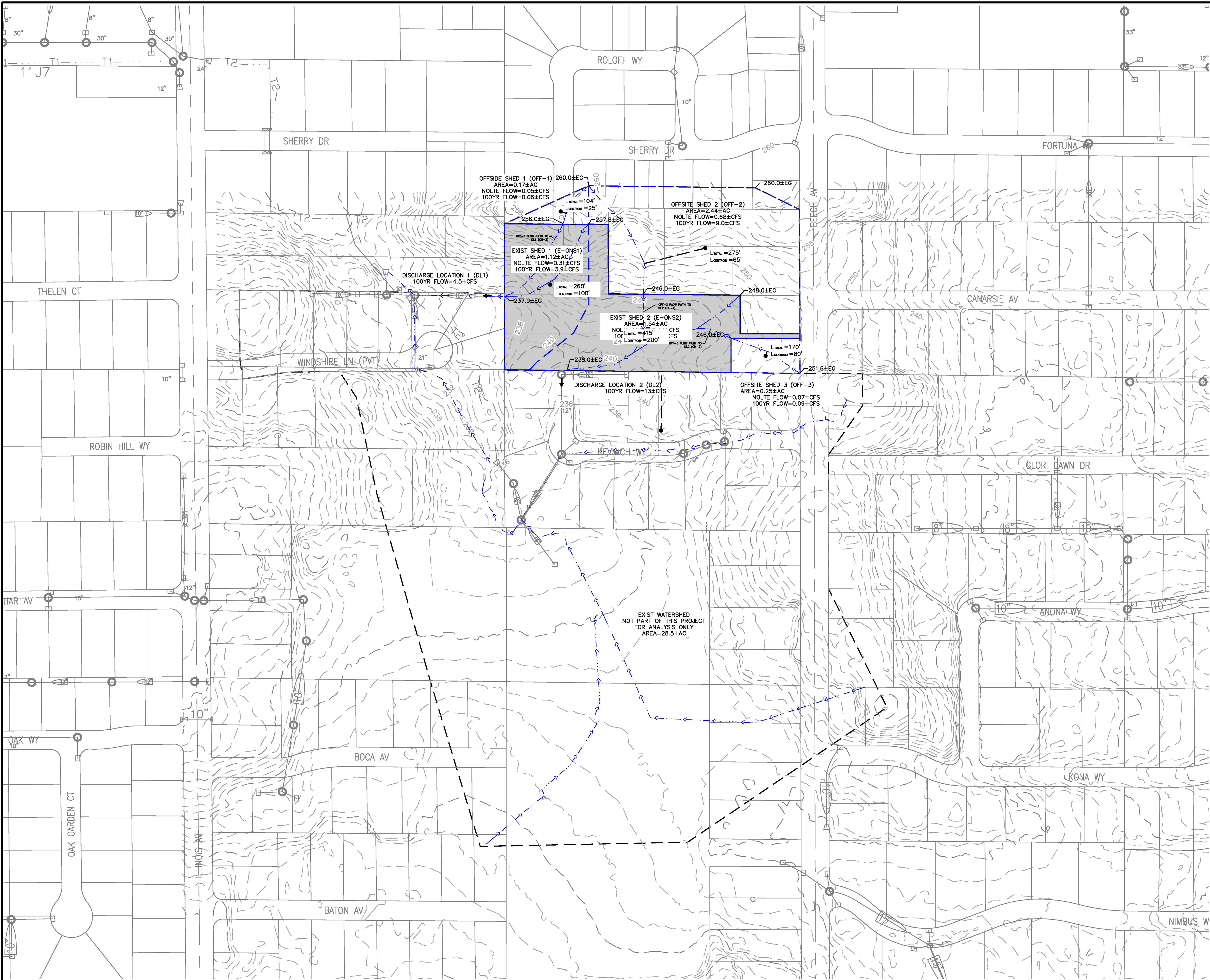
*Percent of map unit:* 6 percent  
*Hydric soil rating:* No

### Unnamed, steeper slopes

*Percent of map unit:* 3 percent  
*Hydric soil rating:* No

**Appendix 3 – Existing and Proposed Shed Maps**



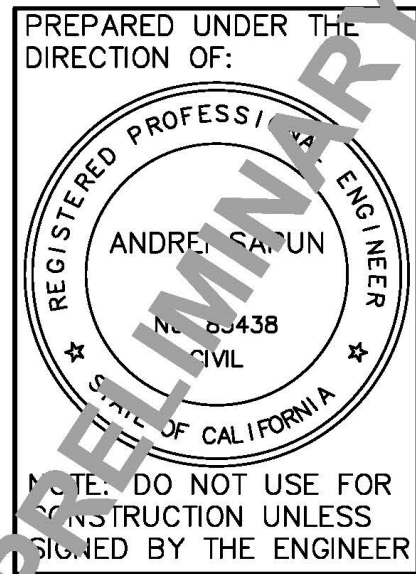


Nolte method results (Project: 21068) (Hydrologic zone 1)			
ID	Drainage area (acres)	Impervious area (%)	Design Q (cfs)
E-ONS1	1.12	23.40	0.31
OFF-1	0.17	55.00	0.05
DL1	1.29	27.56	0.36
E-ONS2	1.54	21.80	0.43
OFF-2	2.44	45.70	0.68
OFF-3	0.25	40.00	0.07
DL2	4.23	36.66	1.18

Sacramento method results (Project: 21068) (100-year, 1-day rainfall)						
ID	Peak flow (cfs)	Time of peak (hours)	Basin area (sq. mi)	Peak stage (feet)	Peak storage (ac-ft)	Diversion volume (ac-ft)
E-ONS1	3.9	12:03	.00			
OFF-1	.6	12:02	.00			
CH-3	.6	12:04	.00			
DL1	4.5	12:03	.00			
E-ONS2	4.1	12:07	.00			
OFF-2	9.0	12:02	.00			
CH1	8.8	12:04	.00			
OFF-3	.9	12:02	.00			
CH2	.9	12:07	.00			
DL2	13.	12:04	.01			

WATERSHEDS LAND USE TABLE						
LAND USE	IMPERV. AREA	E-ONS1	E-ONS2	OFF1	OFF2	OFF3
RESIDENTIAL, RD-8 TO RD-10	60%	0%	0%	50%	19%	0%
RESIDENTIAL, RD-4	40%	17%	9%	50%	81%	100%
OPEN SPACE	2%	83%	91%	0%	0%	0%
TOTAL		100%	100%	100%	100%	100%

SHED MAP LEGEND	
	SHED BOUNDARY
	LONGEST WATER PATH
	PROJECT SITE
	SHED CENTROID



BENCHMARK N/A ELEVATION N/A  
SACRAMENTO COUNTY LIDAR CONTOURS.

RECORD DRAWINGS

ENGR. INIT. DATE

DESIGNED BY: AS

DRAFTED BY: AS

CHECKED BY: RR

DRAINAGE STUDY  
FOR  
BEECH AVE & ROLOFF WAY TPM  
EXISTING SHED MAP  
SACRAMENTO COUNTY STATE OF CALIFORNIA

**AREA WEST ENGINEERS, INC.**  
ENGINEERING - SURVEYING - PLANNING  
7478 SANDALWOOD DRIVE, SUITE 400  
CITRUS HEIGHTS, CA 95621  
(916) 725-5551 (916) 725-5808 (FAX)  
AW@AREAWESTENG.COM

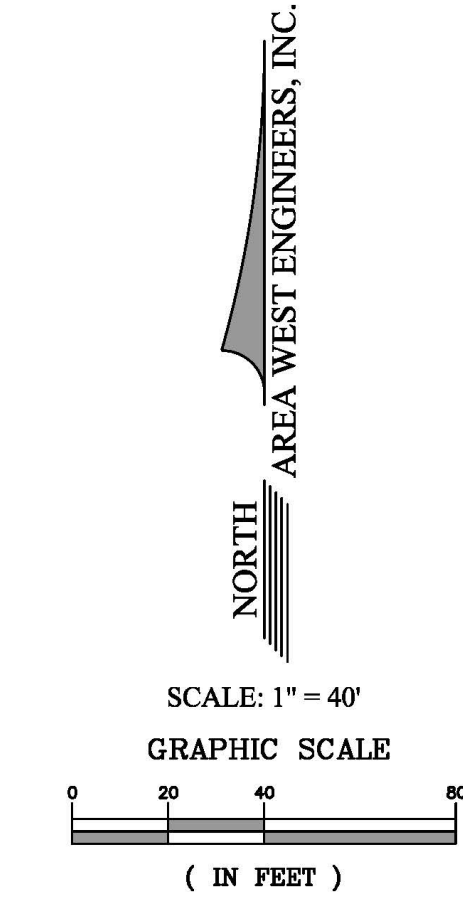
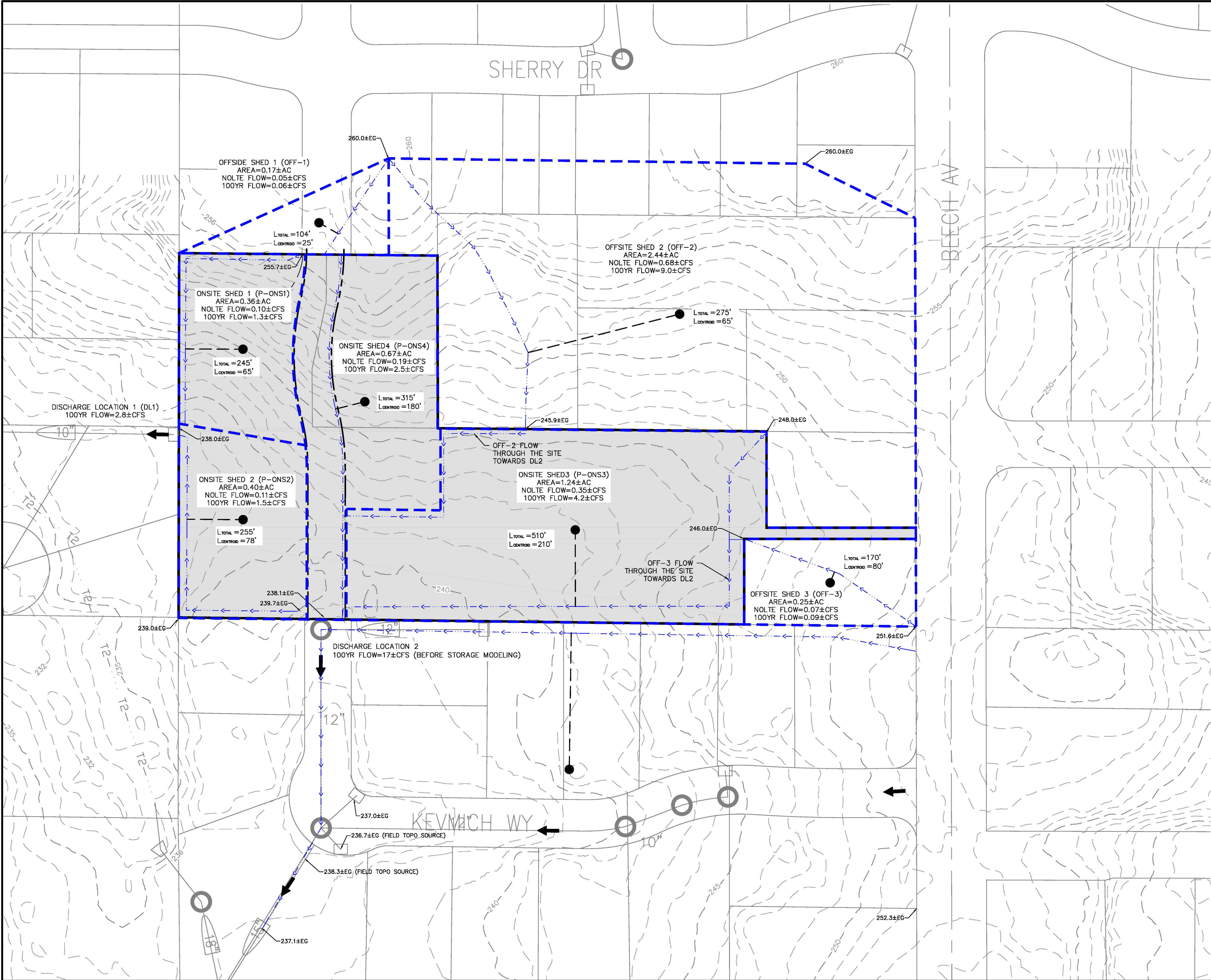
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			APPROVED BY	DATE		
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					VERT: N/A	
					DATE: AUGUST 2024	

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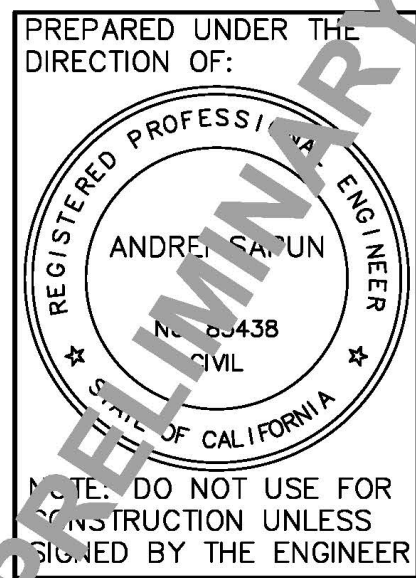
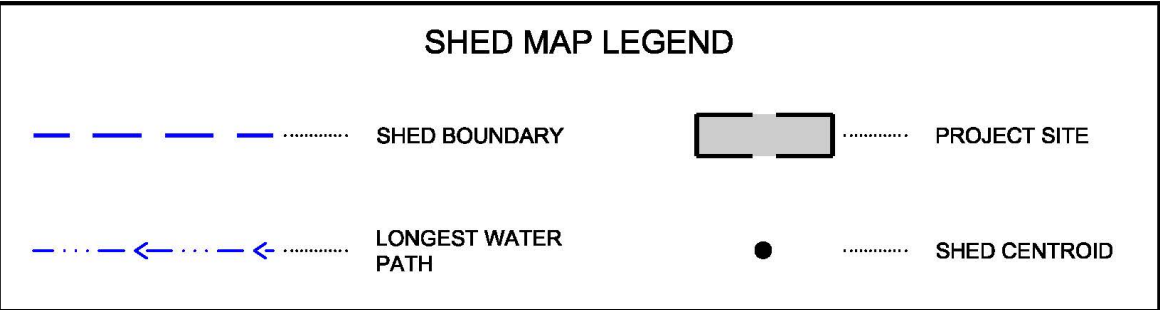
G:\LDD\_20\PROJ\21068\Drainage\CAD Drawings\EXISTING SHED MAP.dwg 8/15/2024 3:28:30 PM 1:1 AREA WEST ENGINEERS, INC.





Nolte method results (Project: 21068) (Hydrologic zone 1)		
ID	Drainage area (acres)	Design Q (cfs)
P-ONS1	0.36	0.10
P-ONS2	0.40	0.11
DL1	0.76	0.21
P-ONS3	1.24	0.35
OFF-3	0.25	0.07
OFF-2	2.44	0.68
JN-3	3.93	1.10
OFF-1	0.17	0.05
P-ONS4	0.67	0.19
JN-4	0.84	0.24
DL-2	4.77	1.34

Sacramento method results (Project: 21068) (100-year, 1-day rainfall)					
ID	Peak flow (cfs)	Time of peak (hours)	Basin area (sq. mi)	Peak stage (feet)	Diversion volume (ac-ft)
P-ONS1	1.3	12:02	.00		
P-ONS2	1.5	12:02	.00		
DL1	2.8	12:02	.00		
P-ONS3	4.2	12:03	.00		
OFF-3	.9	12:02	.00		
CH-2	.9	12:04	.00		
OFF-2	9.0	12:02	.00		
CH-3	8.9	12:03	.00		
JN-3	14.	12:03	.01		
OFF-1	.6	12:02	.00		
CH-4	.6	12:03	.00		
P-ONS4	2.5	12:02	.00		
JN-4	3.1	12:02	.00		
DL-2	17.	12:03	.01		

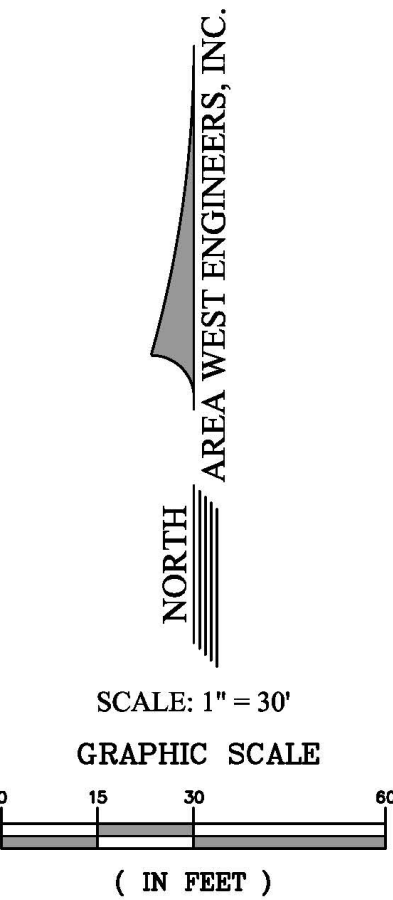
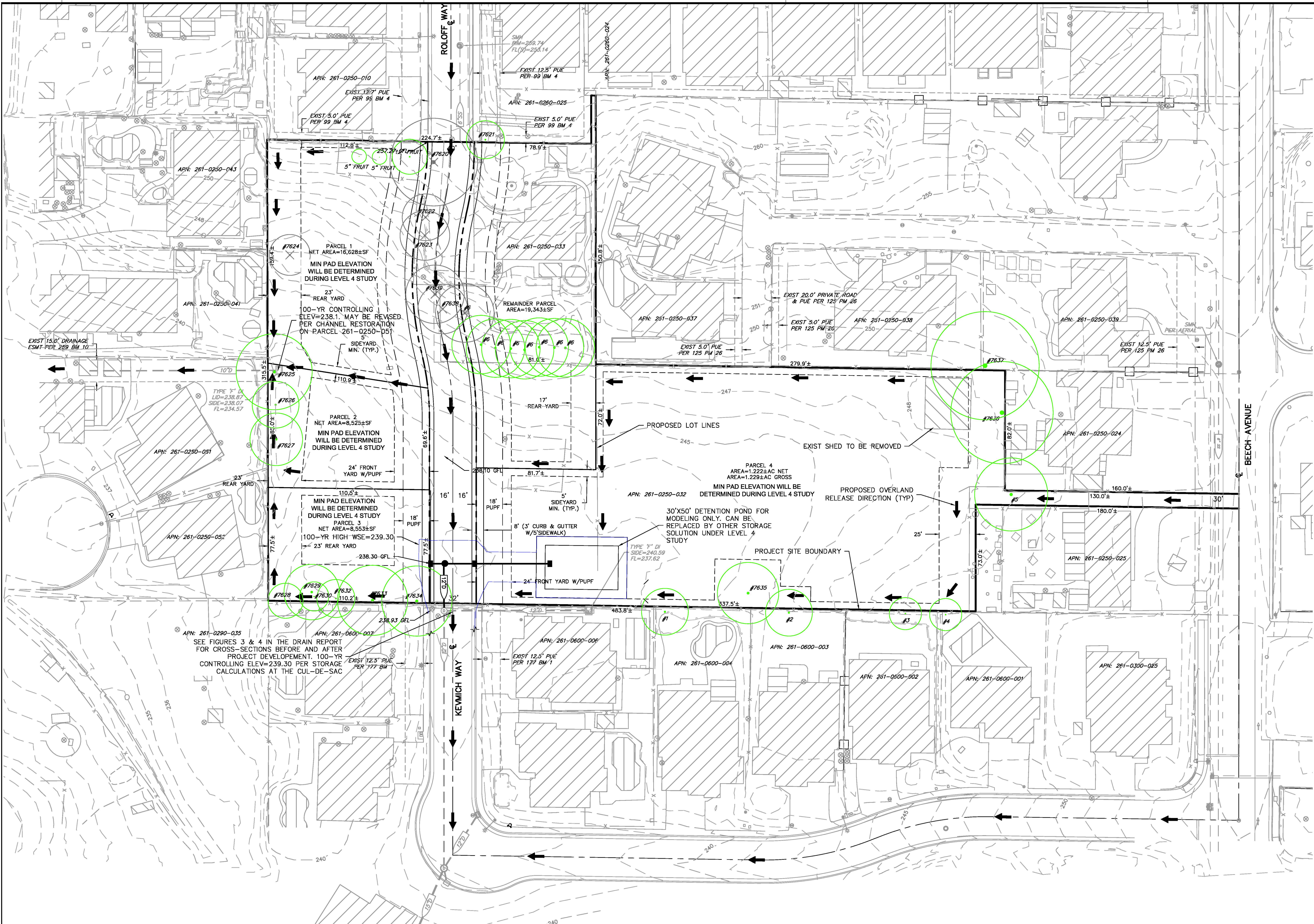


BENCHMARK N/A SACRAMENTO COUNTY LIDAR CONTOURS.	RECORD DRAWINGS ENGR. INIT.      DATE	DESIGNED BY: AS	DRAINAGE STUDY FOR BEECH AVE & ROLOFF WAY TPM PROPOSED SHED MAP SACRAMENTO COUNTY      STATE OF CALIFORNIA		#	DESCRIPTION	ENGR. INIT.	APPROVAL		SCALE HORIZ.: 1"=40' VERT.: N/A DATE: AUGUST 2024	JOB NO. 21068 SHEET 1 OF 1
							APPROVED BY	DATE			
CHECKED BY: RR											



**Appendix 4 – Overland Release Exhibit**






PREPARED UNDER THE  
DIRECTION OF:

REGISTERED PROFESSIONAL ENGINEER  
ANDREW CAPUN  
No. 64438  
CIVIL  
STATE OF CALIFORNIA

NOTE: DO NOT USE FOR  
CONSTRUCTION UNLESS  
SIGNED BY THE ENGINEER

BENCHMARK _____	ELEVATION _____	RECORD DRAWINGS	DESIGNED BY: AS	DRAINAGE STUDY FOR BEECH AVE & ROLOFF WAY TPM OVERLAND RELEASE EXHIBIT	 ENGINEERING - SURVEYING - PLANNING 7478 SANDALWOOD DRIVE, SUITE 400 CITRUS HEIGHTS, CA 95621 (916) 725-5551 (916) 725-5808 (FAX) AWE@AREAWESTENG.COM	REVISIONS	#	DESCRIPTION	ENGR. INIT.	APPROVAL		SCALE	JOB NO.
			DRAFTED BY: AS							APPROVED BY	DATE	HORIZ.: 1"=30'	21068
			CHECKED BY: RR									VERT.: N/A	SHEET
		ENGR. INIT.      DATE										DATE: AUGUST 2024	1 OF 1

SACRAMENTO COUNTYSTATE OF CALIFORNIA



**Appendix 5 – SacCalc Design Reports and Results**

[View HEC-1 output](#)

**Sacramento method results**  
**(Project: 21068)**  
**(100-year, 1-day rainfall)**

ID	Peak flow (cfs)	Time of peak (hours)	Basin area (sq. mi)	Peak stage (feet)	Peak storage (ac-ft)	Diversion volume (ac-ft)
E-ONS1	3.9	12:03	.00			
OFF-1	.6	12:02	.00			
CH-3	.6	12:04	.00			
DL1	4.5	12:03	.00			
E-ONS2	4.1	12:07	.00			
OFF-2	9.0	12:02	.00			
CH1	8.8	12:04	.00			
OFF-3	.9	12:02	.00			
CH2	.9	12:07	.00			
DL2	13.	12:04	.01			

### Sacramento Hydrologic Calculator Report

December 29, 2023 13:55

Project Title: 21068 Method: Sacramento County HEC-1 method  
 Comments: EXISTING MODEL (Beech Ave and Roloff Way). REVISION DATE - Date: 9/1/2022  
 Prepared by: AWE

#### Watershed Hydrologic Summary Data

Watershed	Area (acres)	Mean Elevation (ft)	Lag Times		Basin "n"		Loss Rates		Percent Impervious	
			Method	Lag Time (min)	Method	Basin "n"	Method	Loss Rate (in/hr)	Method	Impervious Area (%)
E-ONS1	1.12	248	Basin "n"	-	Computed	-	Computed	-	Computed	-
E-ONS2	1.54	243	Basin "n"	-	Computed	-	Computed	-	Computed	-
OFF-1	0.17	258	Basin "n"	-	Computed	-	Computed	-	Computed	-
OFF-2	2.44	253	Basin "n"	-	Computed	-	Computed	-	Computed	-
OFF-3	0.25	249	Basin "n"	-	Computed	-	Computed	-	Computed	-

Basin "n" Method Data for Lag Time Computation

Watershed	Channel Length (ft)	Centroid Length (ft)	Slope (ft/ft)	Channelization	Land Use Impervious Area Percent (% or acres)																	
					95	90	85	80	75	70	60	50	40	30	25	20	15	10	5	2	1	1*
E-ONS1	234	100	0.08	Undeveloped									0							83		
				Developed								17								0		
E-ONS2	375	200	0.024	Undeveloped									0							91		
				Developed								9								0		
OFF-1	94	25	0.035	Undeveloped							0		0									
				Developed							50		50									
OFF-2	248	59	0.05	Undeveloped							0		0									
				Developed							19		81									
OFF-3	153	80	0.035	Undeveloped									0									
				Developed									100									

Refer to the Drainage manual for Land Use Impervious Area Percent

\*Dense Oaks, Shrubs, Vines

## Infiltration Loss Rate Data

Watershed	Soil Cover Group	Land Use Impervious Area Percent (% or acres)																	
		95	90	85	80	75	70	60	50	40	30	25	20	15	10	5	2	1	1*
E-ONS1	B																		
	C																		
	D									17							83		
E-ONS2	B																		
	C																		
	D									9							91		
OFF-1	B																		
	C																		
	D							50		50									
OFF-2	B																		
	C																		
	D							19		81									
OFF-3	B																		
	C																		
	D									100									

Refer to the help file for Land Use Impervious Area Percent

\*Dense Oaks, Shrubs, Vines



## Hydrograph Routing – Kinematic Wave

Routing ID	Route From	Route To	Channel Type	Length (ft)	Slope (ft/ft)	Width or Diameter (ft)	Side Slope (H:V)	Mannings "n"
CH1	OFF-2	DL2	Trapezoidal	285	0.028	15	10:1	0.060
CH2	OFF-3	DL2	Trapezoidal	380	0.021	15	10:1	0.060
CH-3	OFF-1	DL1	Trapezoidal	205	0.08	15	10:1	0.060

[View HEC-1 output](#)

**Sacramento method results**  
**(Project: 21068)**  
**(100-year, 1-day rainfall)**

ID	Peak flow (cfs)	Time of peak (hours)	Basin area (sq. mi)	Peak stage (feet)	Peak storage (ac-ft)	Diversion volume (ac-ft)
P-ONS1	1.3	12:02	.00			
P-ONS2	1.5	12:02	.00			
DL1	2.8	12:02	.00			
P-ONS3	4.2	12:03	.00			
OFF-3	.9	12:02	.00			
CH-2	.9	12:04	.00			
OFF-2	9.0	12:02	.00			
CH-3	8.9	12:03	.00			
JN-3	14.	12:03	.01			
OFF-1	.6	12:02	.00			
CH-4	.6	12:03	.00			
P-ONS4	2.5	12:02	.00			
JN-4	3.1	12:02	.00			
DL-2	17.	12:03	.01			

### Sacramento Hydrologic Calculator Report

August 19, 2024 13:14

Project Title: 21068  
 Comments: PROPOSED MODEL (Beech Ave and Roloff Way). REVISION DATE - 08/16/2024  
 Prepared by: AWE

Method: Sacramento County HEC-1 method

Date: 9/1/2022

#### Watershed Hydrologic Summary Data

Watershed	Area (acres)	Mean Elevation (ft)	Lag Times		Basin "n"		Loss Rates		Percent Impervious	
			Method	Lag Time (min)	Method	Basin "n"	Method	Loss Rate (in/hr)	Method	Impervious Area (%)
P-ONS1	0.36	247	Basin "n"	-	Computed	-	Computed	-	Computed	-
P-ONS3	1.24	243	Basin "n"	-	Computed	-	Computed	-	Computed	-
OFF-1	0.17	258	Basin "n"	-	Computed	-	Computed	-	Computed	-
OFF-2	2.44	253	Basin "n"	-	Computed	-	Computed	-	Computed	-
OFF-3	0.25	249	Basin "n"	-	Computed	-	Computed	-	Computed	-
P-ONS2	0.4	239	Basin "n"	-	Computed	-	Computed	-	Computed	-
P-ONS4	0.67	243	Basin "n"	-	Computed	-	Computed	-	Computed	-

Basin "n" Method Data for Lag Time Computation

Watershed	Channel Length (ft)	Centroid Length (ft)	Slope (ft/ft)	Channelization	Land Use Impervious Area Percent (% or acres)																	
					95	90	85	80	75	70	60	50	40	30	25	20	15	10	5	2	1	1*
P-ONS1	220	65	0.07	Undeveloped									0									
				Developed								100										
P-ONS3	460	210	0.020	Undeveloped									0									
				Developed								100										
OFF-1	94	25	0.035	Undeveloped							0		0									
				Developed							50		50									
OFF-2	248	65	0.05	Undeveloped							0		0									
				Developed							19		81									
OFF-3	153	80	0.035	Undeveloped									0									
				Developed									100									
P-ONS2	230	78	0.01	Undeveloped									0									
				Developed									100									
P-ONS4	284	180	0.05	Undeveloped									0									
				Developed									100									

Refer to the Drainage manual for Land Use Impervious Area Percent

\*Dense Oaks, Shrubs, Vines

## Infiltration Loss Rate Data

Watershed	Soil Cover Group	Land Use Impervious Area Percent (% or acres)																	
		95	90	85	80	75	70	60	50	40	30	25	20	15	10	5	2	1	1*
P-ONS1	B																		
	C																		
	D									100									
P-ONS3	B																		
	C																		
	D									100									
OFF-1	B																		
	C																		
	D							50		50									
OFF-2	B																		
	C																		
	D							19		81									
OFF-3	B																		
	C																		
	D									100									
P-ONS2	B																		
	C																		
	D									100									
P-ONS4	B																		
	C																		
	D									100									

Refer to the help file for Land Use Impervious Area Percent

\*Dense Oaks, Shrubs, Vines

## Hydrograph Routing – Kinematic Wave

Routing ID	Route From	Route To	Channel Type	Length (ft)	Slope (ft/ft)	Width or Diameter (ft)	Side Slope (H:V)	Mannings "n"
CH-2	OFF-3	JN-3	Trapezoidal	410	.019	10	2:1	0.030
CH-3	OFF-2	JN-3	Trapezoidal	310	.027	5	3:1	0.060
CH-4	OFF-1	JN-4	Trapezoidal	315	.06	30	50:1	0.016

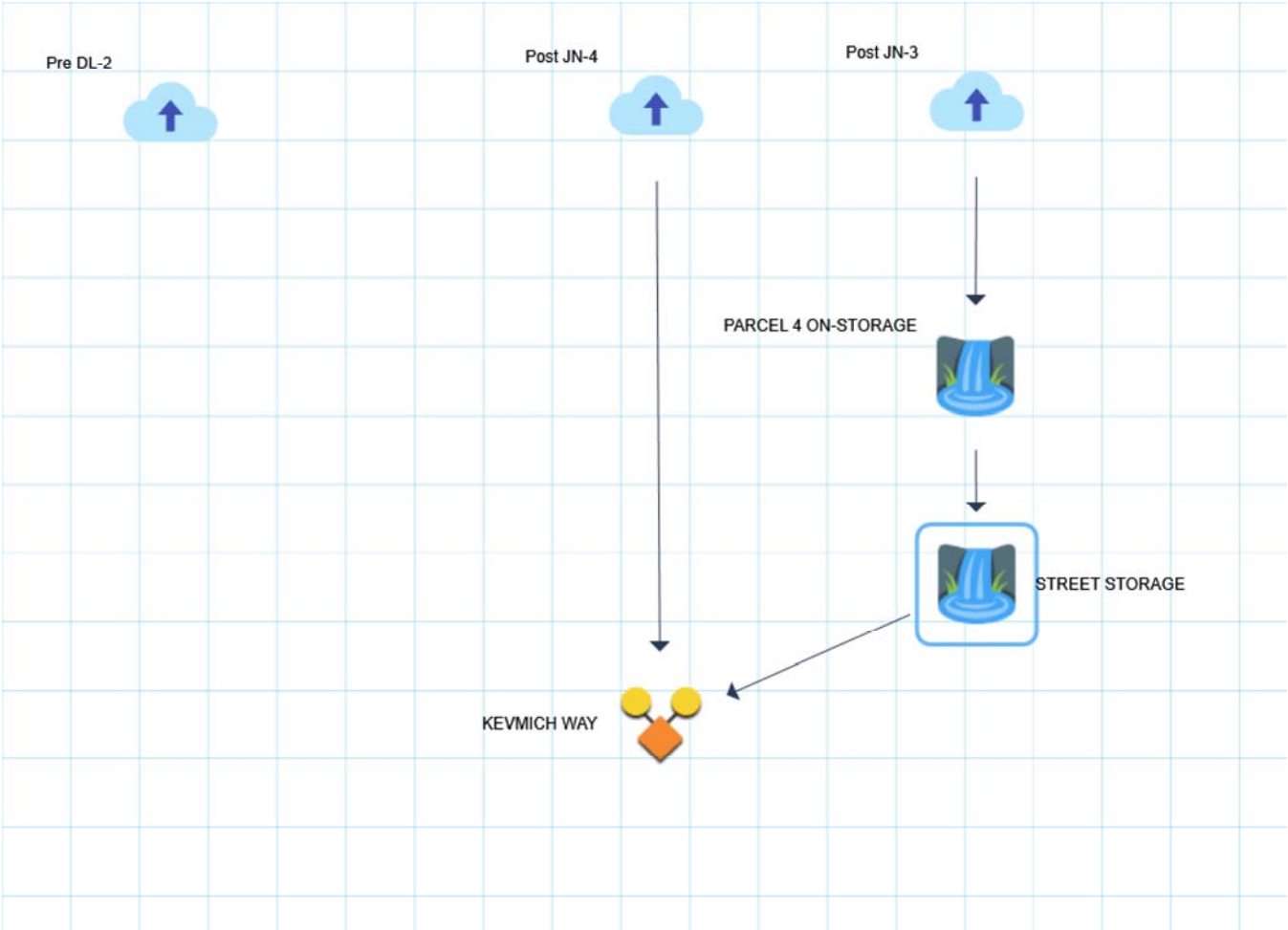
**Appendix 6 – Hydrology Studio Reports On-site Storage**

# Basin Model

Hydrology Studio v 3.0.0.32

Project Name:

08-16-2024





# Hydrograph Report

Project Name:

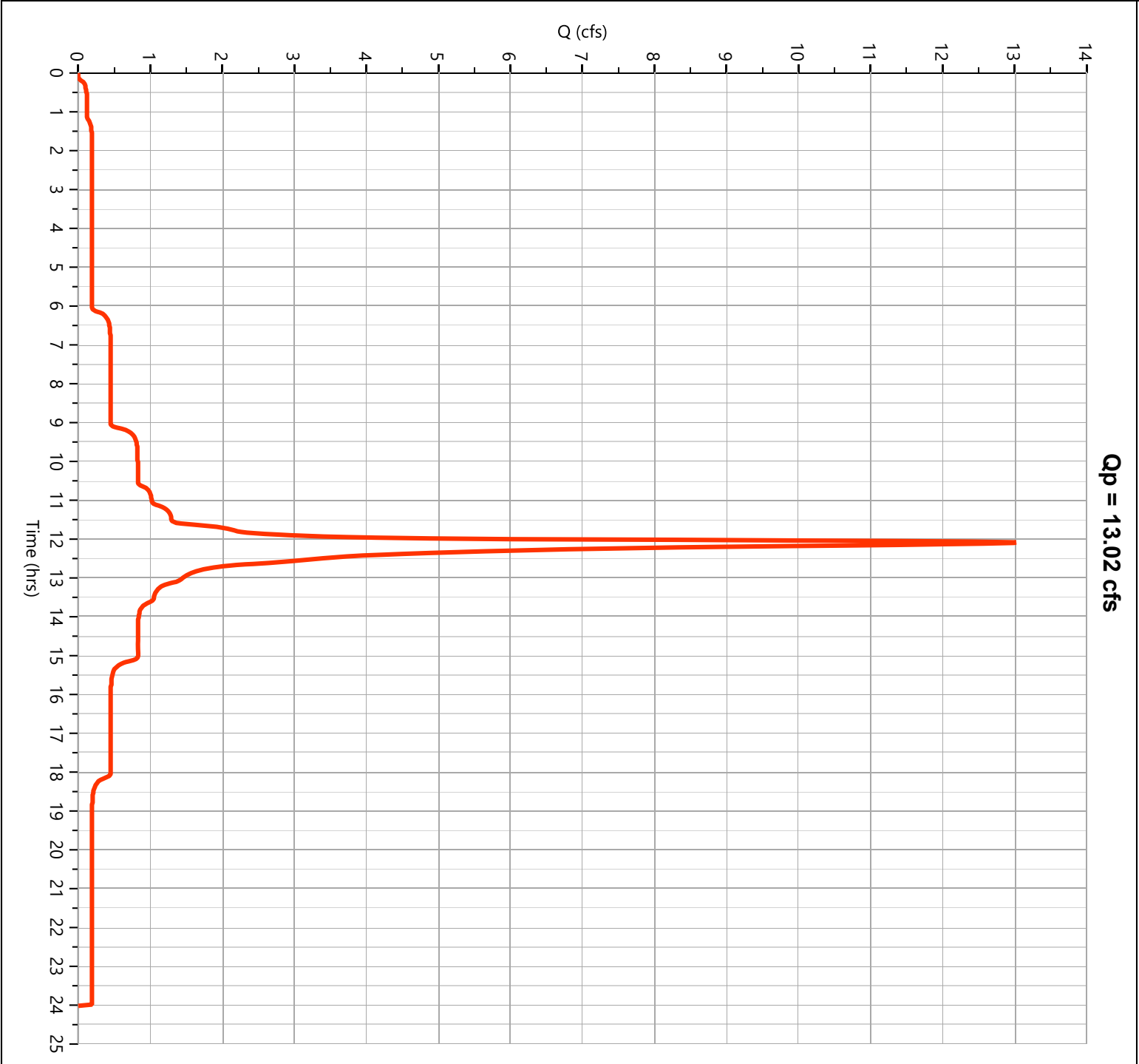
Hydrology Studio v 3.0.0.32

08-16-2024

Pre DL-2

Hyd. No. 1

Hydrograph Type	= Manual	Peak Flow	= 13.02 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.08 hrs
Time Interval	= 1 min	Hydrograph Volume	= 54,151 cuft



# Hydrograph Report

Project Name:

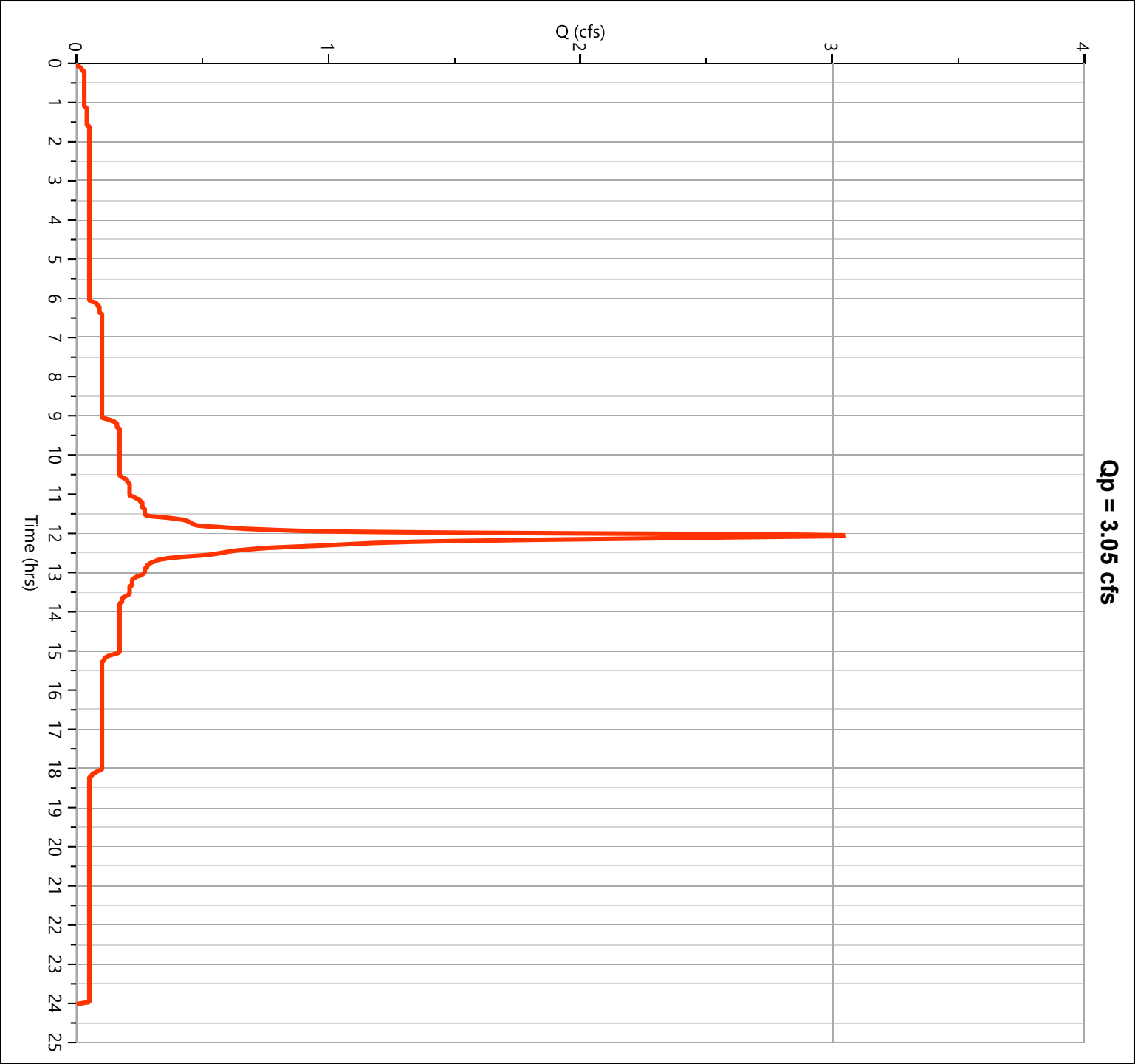
Hydrology Studio v 3.0.0.32

08-16-2024

Post JN-4

Hyd. No. 2

Hydrograph Type	= Manual	Peak Flow	= 3.050 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.05 hrs
Time Interval	= 1 min	Hydrograph Volume	= 11,611 cuft



# Hydrograph Report

Project Name:

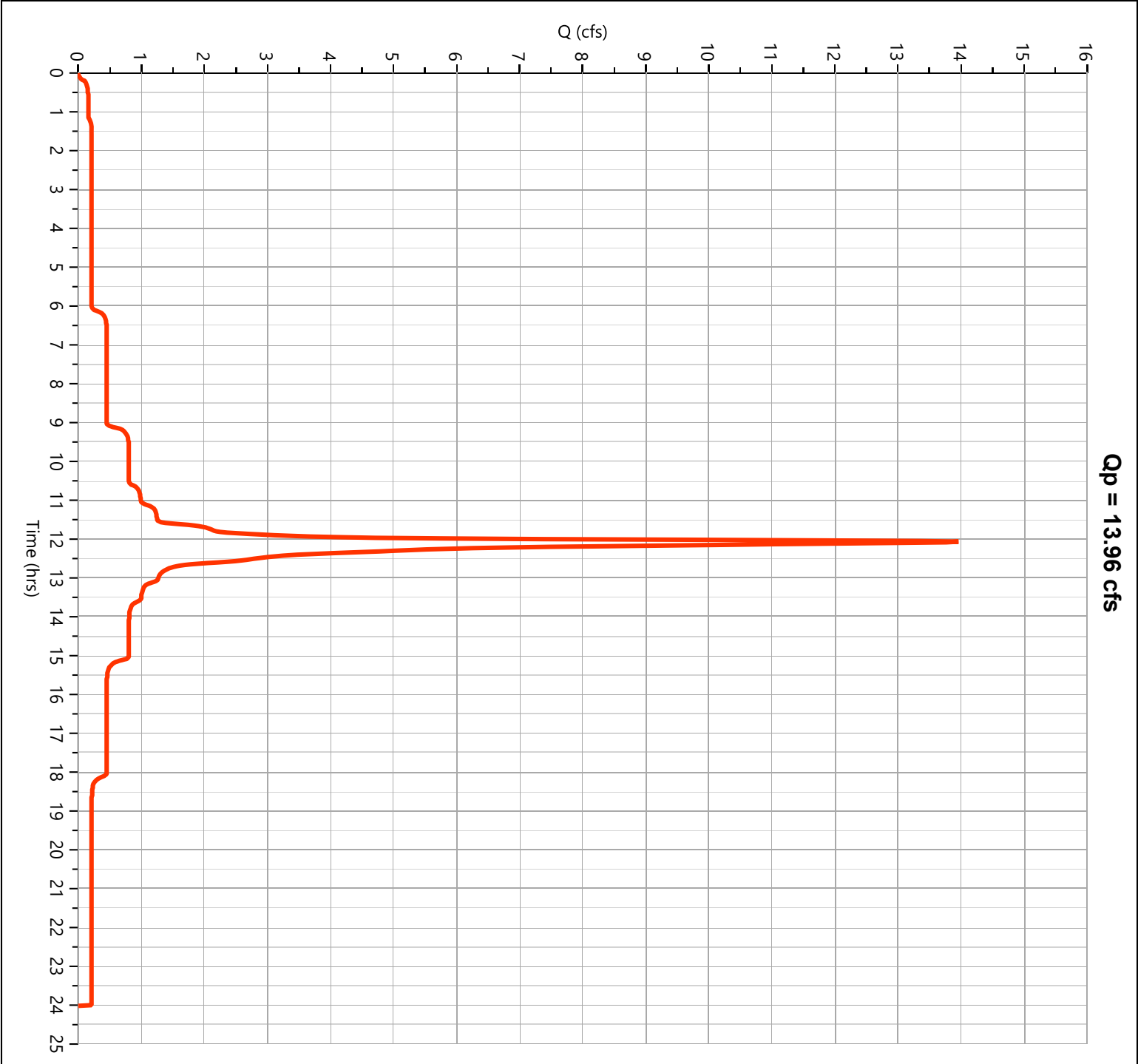
Hydrology Studio v 3.0.0.32

08-16-2024

Post JN-3

Hyd. No. 3

Hydrograph Type	= Manual	Peak Flow	= 13.96 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.07 hrs
Time Interval	= 1 min	Hydrograph Volume	= 53,250 cuft



# Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.32

08-16-2024

## PARCEL 4 ON-STORAGE

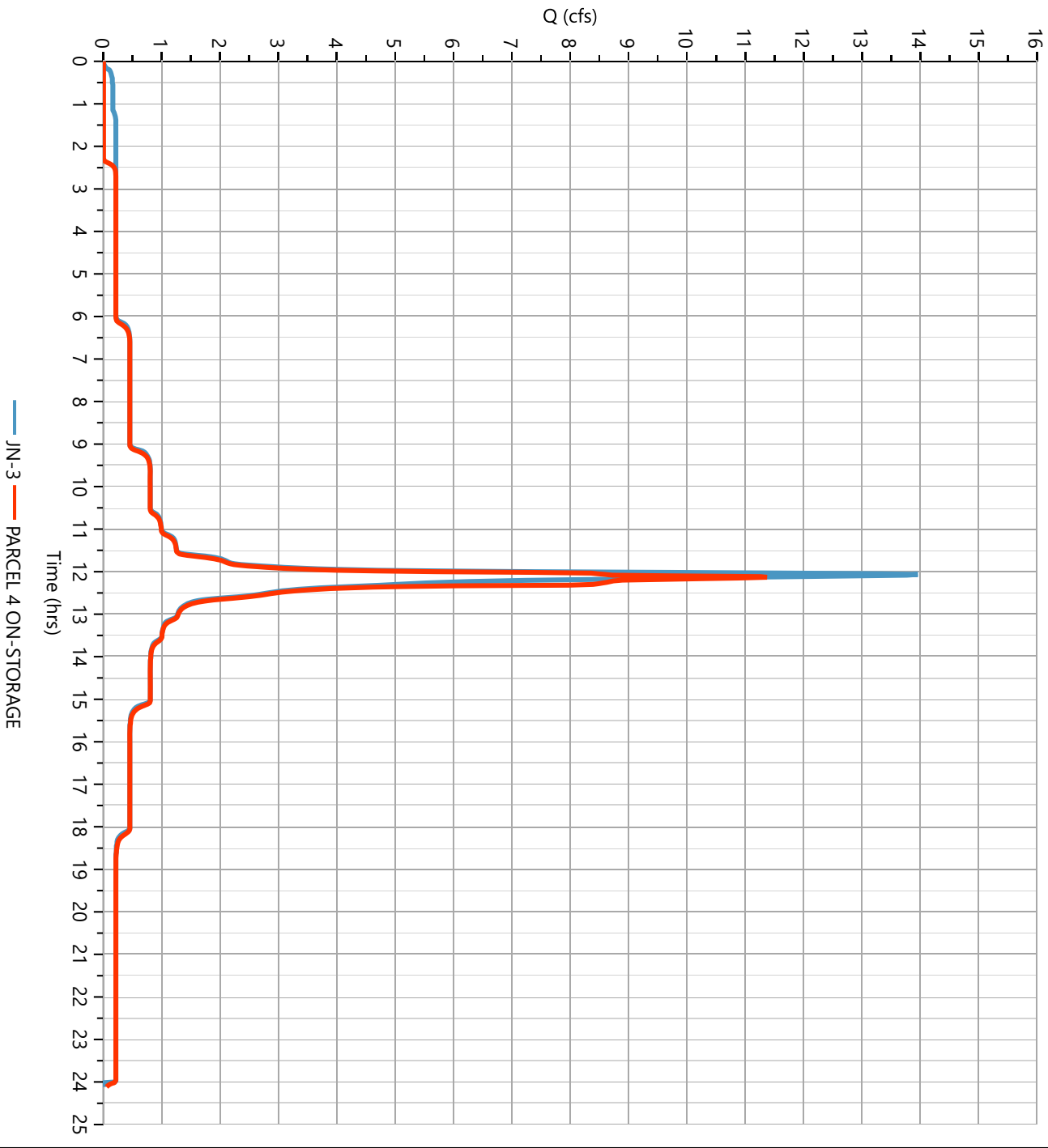
Hyd. No. 4

Hydrograph Type	= Pond Route	Peak Flow	= 11.37 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Hydrograph Volume	= 51,827 cuft
Inflow Hydrograph	= 3 - JN-3	Max. Elevation	= 239.12 ft
Pond Name	= PARCEL 4 STORAGE	Max. Storage	= 3,587 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 21 min

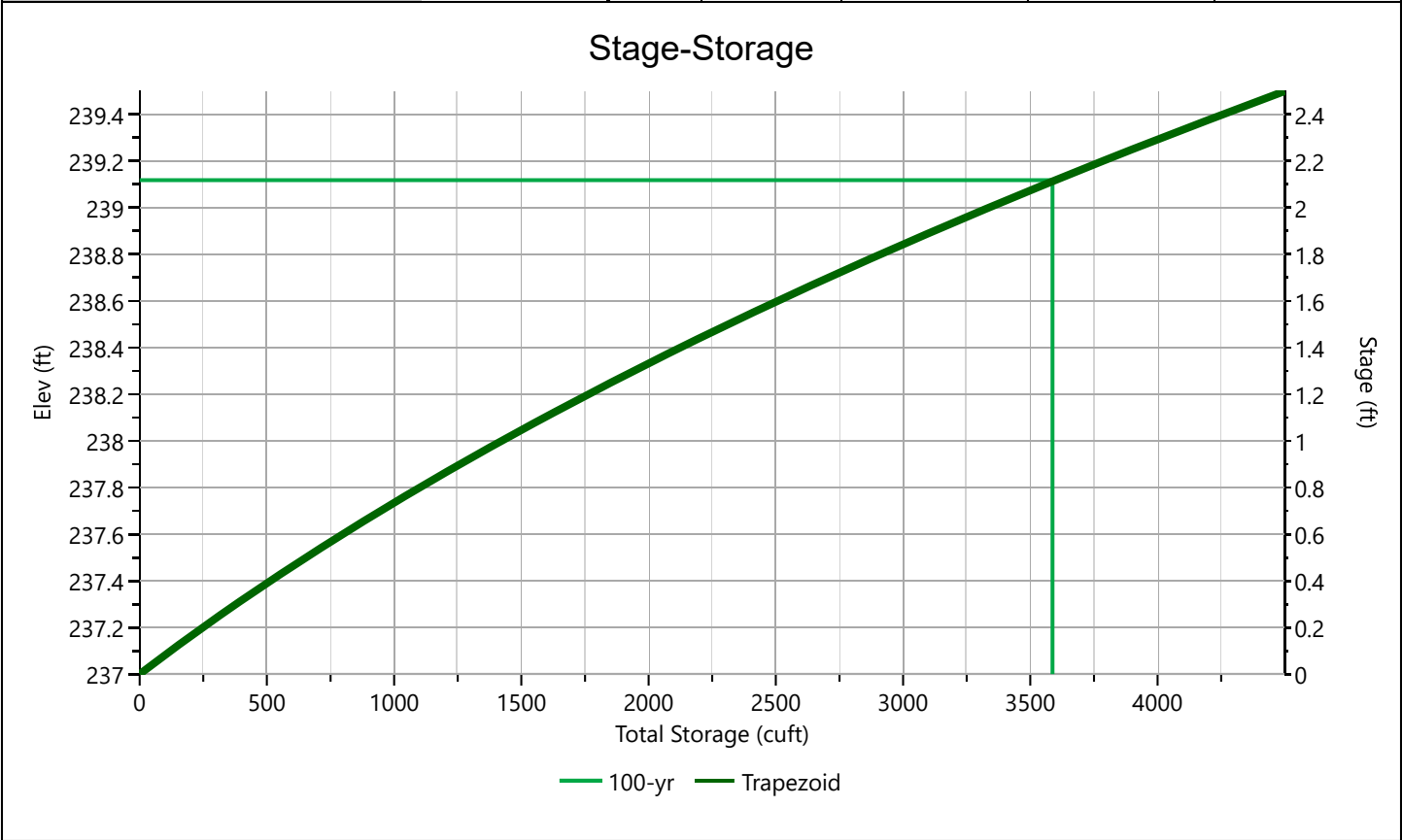
**Qp = 11.37 cfs**



PARCEL 4 STORAGE

Stage-Storage

Trapezoid		Stage / Storage Table				
Description	Input	Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
Bottom Elevation, ft	237.00	0.00	237.00	1,200	0.000	0.000
Bottom Length, ft	40.00	0.13	237.13	1,253	153	153
Bottom Width, ft	30.00	0.25	237.25	1,307	160	313
Side Slope, H:1	3.00	0.38	237.38	1,363	167	480
Total Depth, ft	2.50	0.50	237.50	1,419	174	654
Voids (%)	100.00	0.63	237.63	1,477	181	835
		0.75	237.75	1,535	188	1,023
		0.88	237.88	1,595	196	1,219
		1.00	238.00	1,656	203	1,422
		1.13	238.13	1,718	211	1,633
		1.25	238.25	1,781	219	1,852
		1.38	238.38	1,846	227	2,078
		1.50	238.50	1,911	235	2,313
		1.63	238.63	1,978	243	2,556
		1.75	238.75	2,045	251	2,807
		1.88	238.88	2,114	260	3,067
		2.00	239.00	2,184	269	3,336
		2.13	239.13	2,255	277	3,613
		2.25	239.25	2,327	286	3,900
		2.38	239.38	2,401	295	4,195
		2.50	239.50	2,475	305	4,500

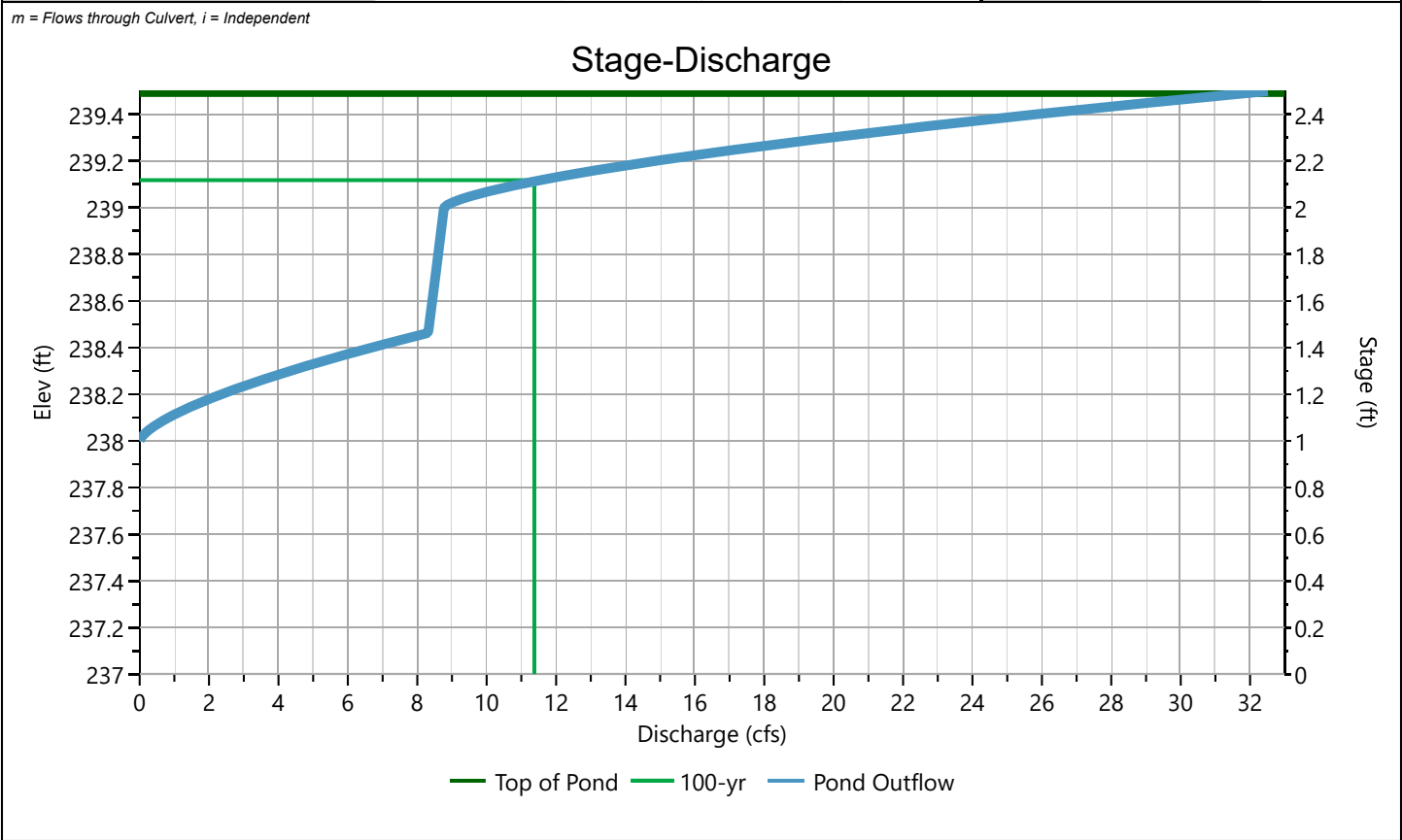


PARCEL 4 STORAGE

Stage-Discharge

Culvert / Orifices	Culvert	Orifice			Orifice Plate
		1	2	3	
Rise, in	12				Orifice Dia, in
Span, in	12				No. Orifices
No. Barrels	1				Invert Elevation, ft
Invert Elevation, ft	233.00				Height, ft
Orifice Coefficient, Co	0.60				Orifice Coefficient, Co
Length, ft	30				
Barrel Slope, %	1				
N-Value, n	0.015				
Weirs	Riser	Weir			Ancillary
		1 (i)	2	3	
Shape / Type	Box	Rectangular			Exfiltration, in/hr
Crest Elevation, ft	238	239			
Crest Length, ft	8	20			
Angle, deg					
Weir Coefficient, Cw	3.3	3.3			

m = Flows through Culvert, i = Independent



# Pond Report

Project Name:

Hydrology Studio v 3.0.0.32

08-16-2024

## PARCEL 4 STORAGE

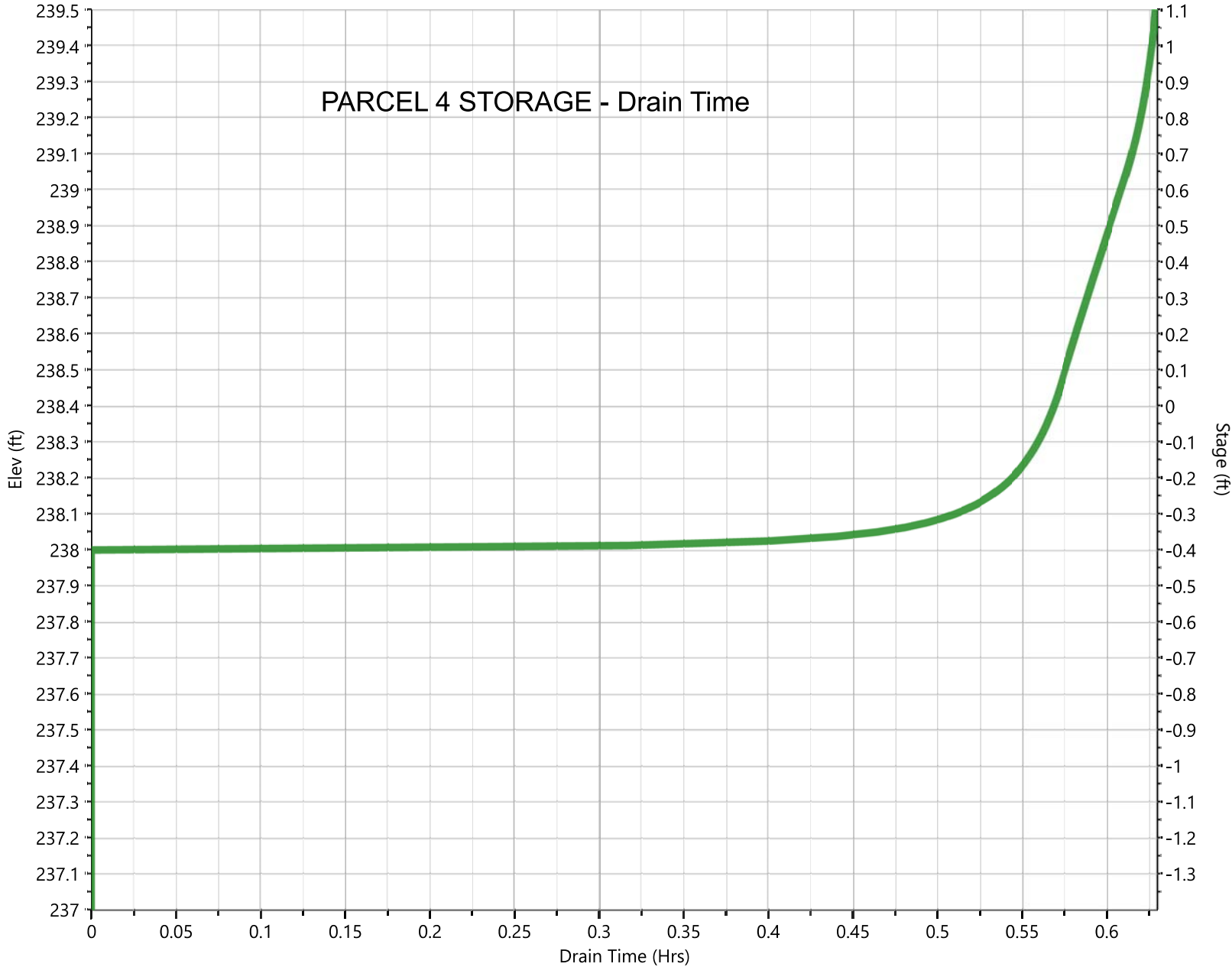
## Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.00	237.00	0.000	0.000				0.000	0.000						0.000
0.13	237.13	153	0.000 oc				0.000	0.000						0.000
0.25	237.25	313	0.000 oc				0.000	0.000						0.000
0.38	237.38	480	0.000 oc				0.000	0.000						0.000
0.50	237.50	654	0.000 oc				0.000	0.000						0.000
0.63	237.63	835	0.000 oc				0.000	0.000						0.000
0.75	237.75	1,023	0.000 oc				0.000	0.000						0.000
0.88	237.88	1,219	0.000 oc				0.000	0.000						0.000
1.00	238.00	1,422	0.000 oc				0.000	0.000						0.000
1.13	238.13	1,633	1.167 oc				1.167	0.000						1.167
1.25	238.25	1,852	3.300 oc				3.300	0.000						3.300
1.38	238.38	2,078	6.062 oc				6.062	0.000						6.062
1.50	238.50	2,313	8.345 oc				0.000	0.000						8.345
1.63	238.63	2,556	8.453 oc				0.000	0.000						8.453
1.75	238.75	2,807	8.560 oc				0.000	0.000						8.560
1.88	238.88	3,067	8.665 oc				0.000	0.000						8.665
2.00	239.00	3,336	8.769 oc				0.000	0.000						8.769
2.13	239.13	3,613	8.872 oc				0.000	2.917						11.79
2.25	239.25	3,900	8.973 oc				0.000	8.250						17.22
2.38	239.38	4,195	9.074 oc				0.000	15.16						24.23
2.50	239.50	4,500	9.173 oc				0.000	23.33						32.51

Suffix key: ic = inlet control, oc = outlet control, s = submerged weir

PARCEL 4 STORAGE

Pond Drawdown



— Stage vs. Drain Time



# Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.32

08-16-2024

## STREET STORAGE

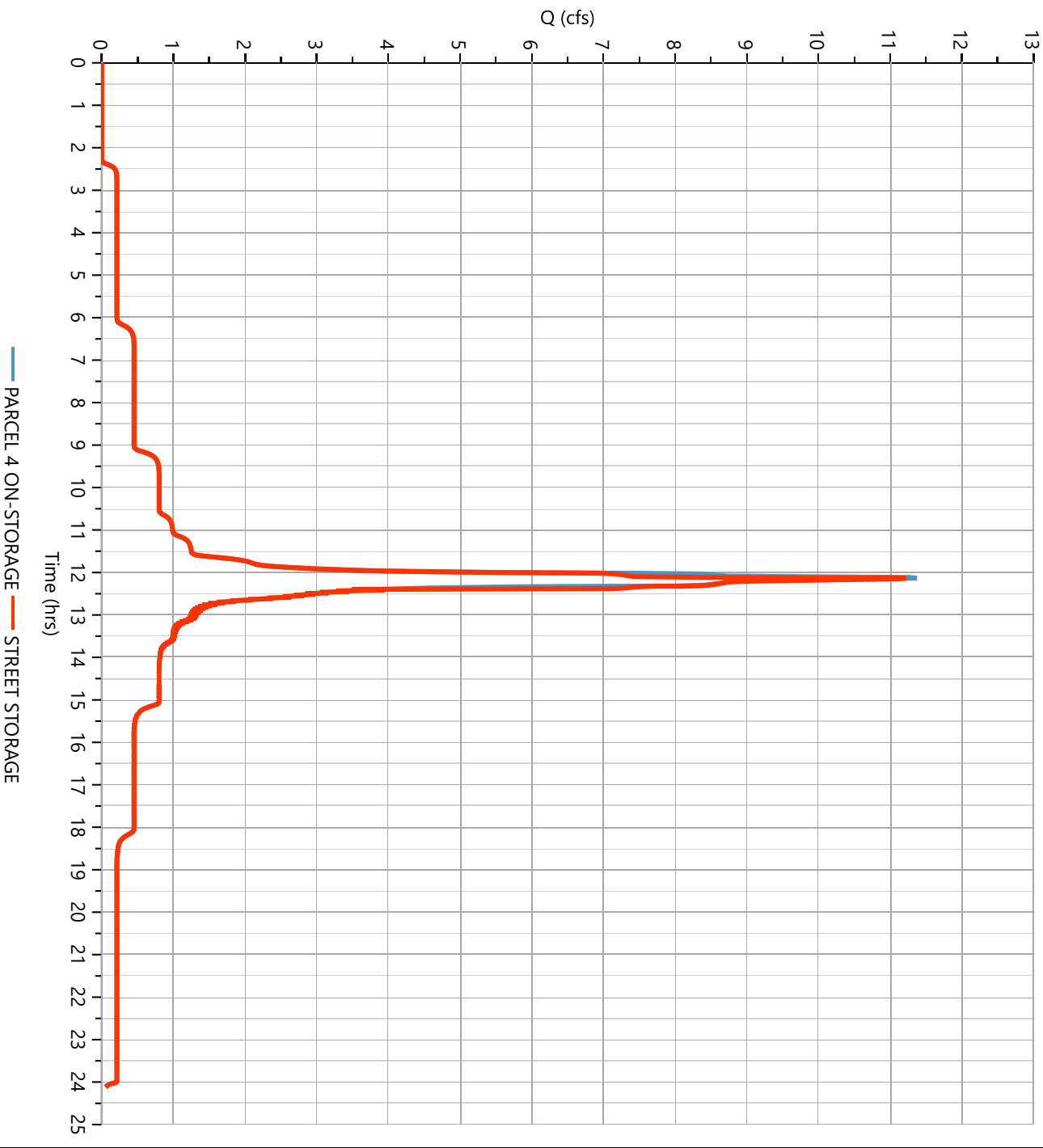
Hyd. No. 5

Hydrograph Type	= Pond Route	Peak Flow	= 11.22 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.13 hrs
Time Interval	= 1 min	Hydrograph Volume	= 51,827 cuft
Inflow Hydrograph	= 4 - PARCEL 4 ON-STORAGE	Max. Elevation	= 239.21 ft
Pond Name	= STREET STORAGE	Max. Storage	= 668 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 0 min

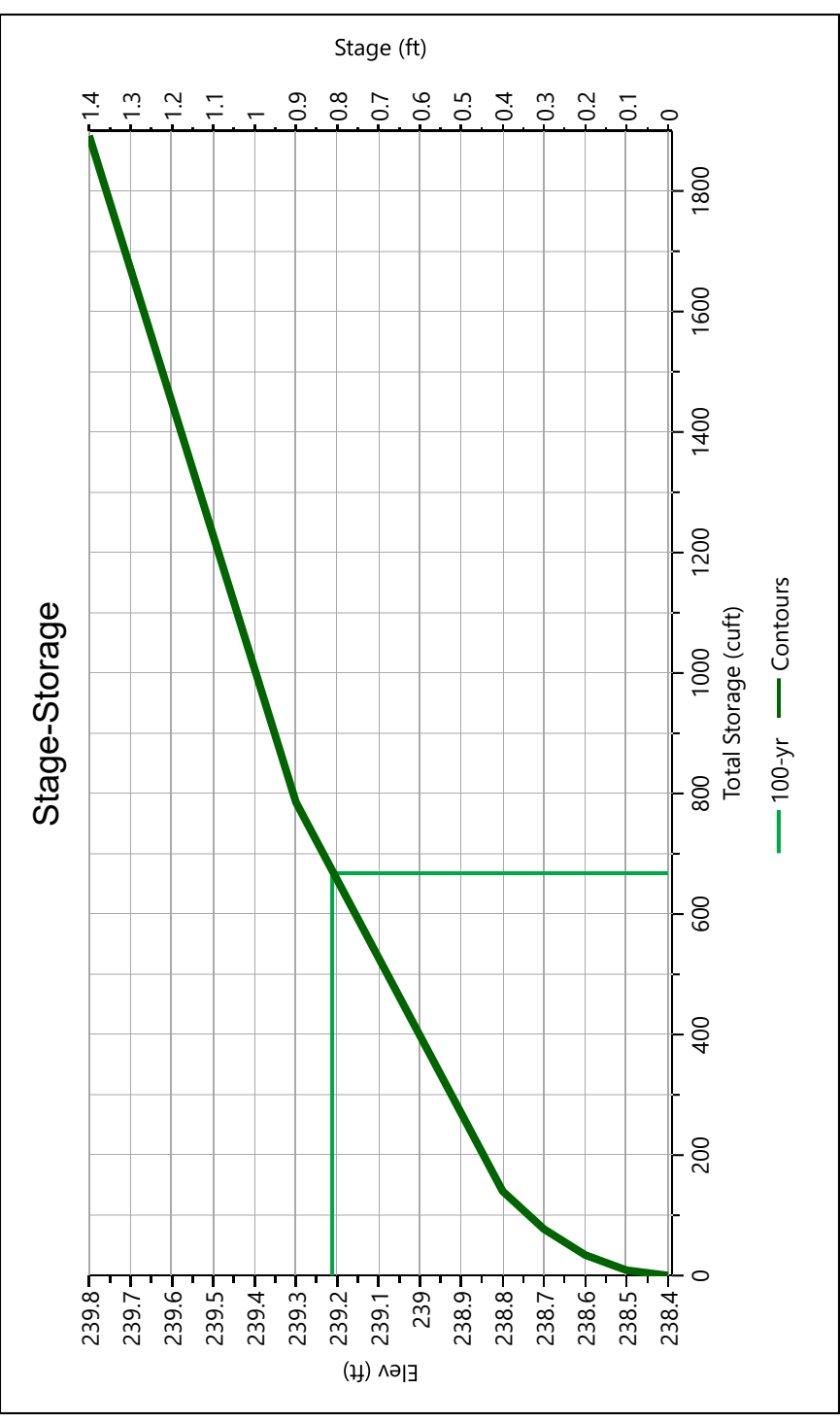
**Qp = 11.22 cfs**



## Project Name:

08-16-2024

## Stage-Storage

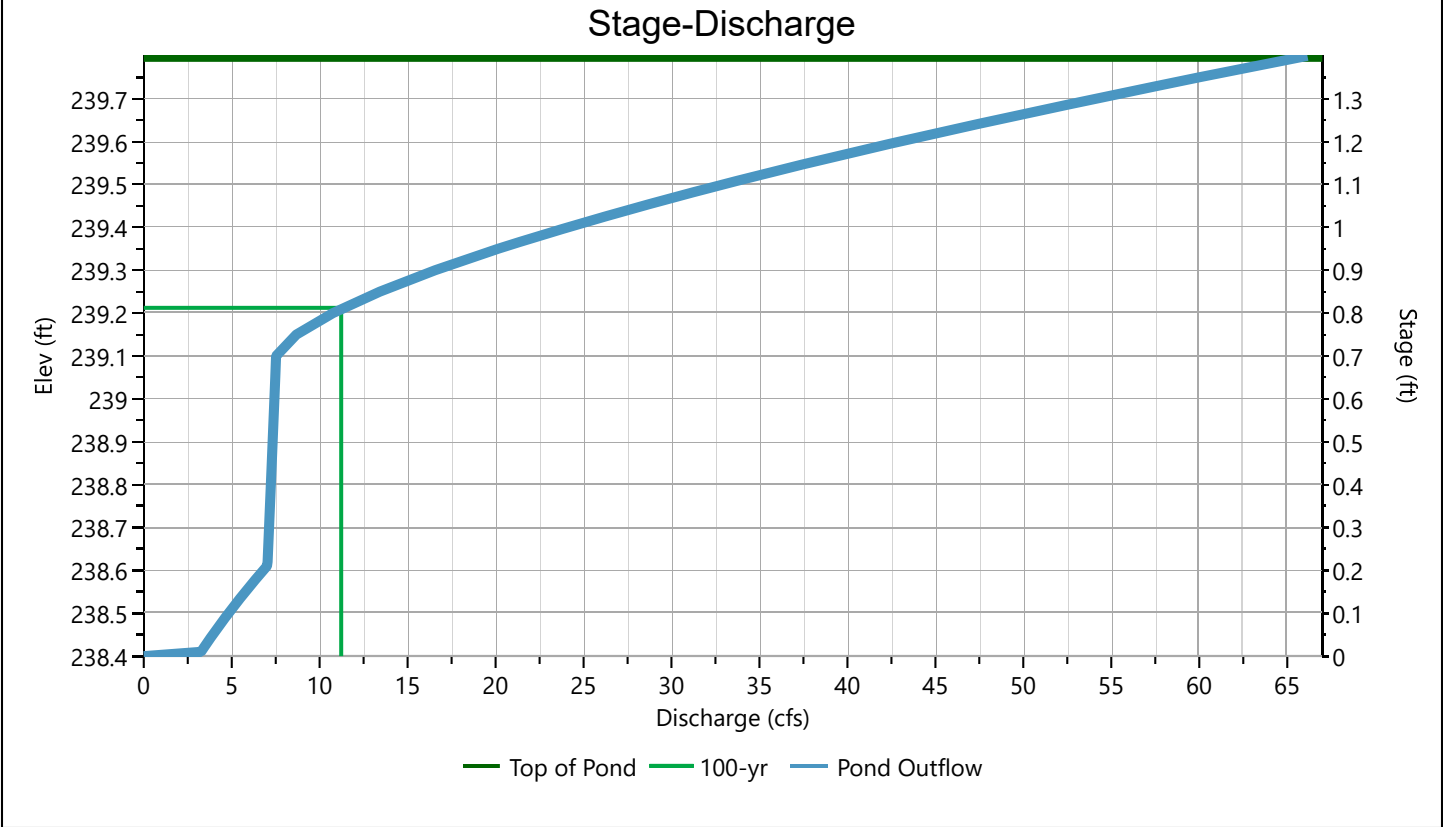
[illegible]

STREET STORAGE

Stage-Discharge

Culvert / Orifices	Culvert	Orifice			Orifice Plate
		1	2	3	
Rise, in	12				Orifice Dia, in
Span, in	12				No. Orifices
No. Barrels	1				Invert Elevation, ft
Invert Elevation, ft	234.50				Height, ft
Orifice Coefficient, Co	0.60				Orifice Coefficient, Co
Length, ft	30				
Barrel Slope, %	1				
N-Value, n	0.015				
Weirs	Riser	Weir			Ancillary
		1 (i)	2	3	
Shape / Type	Box	Rectangular			Exfiltration, in/hr
Crest Elevation, ft	238.11	239.1			
Crest Length, ft	6	30			
Angle, deg					
Weir Coefficient, Cw	3.3	3.3			

m = Flows through Culvert, i = Independent



# Pond Report

Project Name:

Hydrology Studio v 3.0.0.32

08-16-2024

## STREET STORAGE

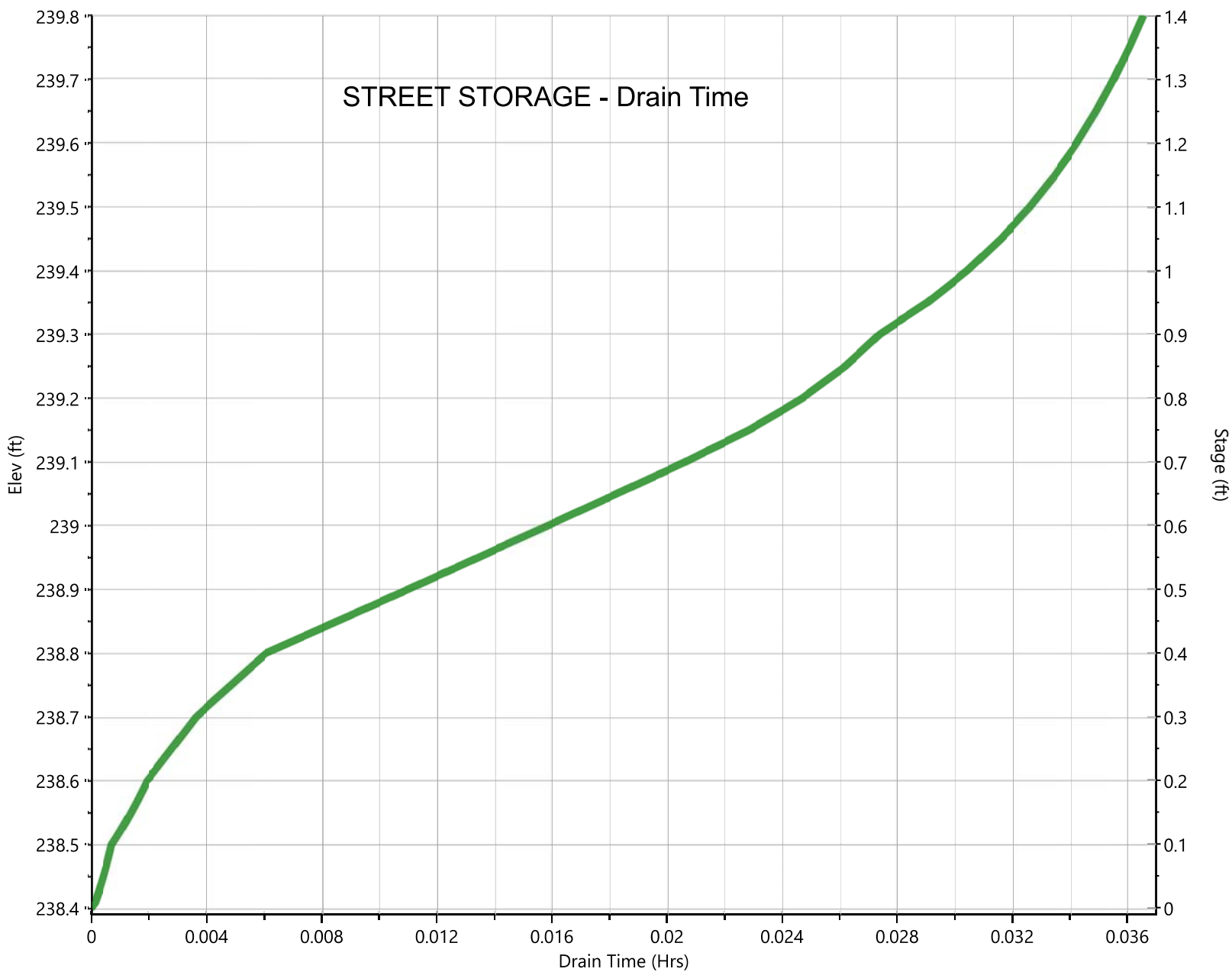
## Stage-Storage-Discharge Summary

[illegible]

Suffix key: ic = inlet control, oc = outlet control, s = submerged weir

STREET STORAGE

Pond Drawdown



— Stage vs. Drain Time

# Hydrograph Report

Project Name:

Hydrology Studio v 3.0.0.32

08-16-2024

## KEVMICH WAY

Hyd. No. 6

Hydrograph Type	= Junction	Peak Flow	= 11.24 cfs
Storm Frequency	= 100-yr	Time to Peak	= 12.18 hrs
Time Interval	= 1 min	Hydrograph Volume	= 63,107 cuft
Inflow Hydrographs	= 2, 5	Total Contrib. Area	= 0.0 ac

