Appendix B

Appendix B1

Grace Solar Geotechnical Engineering and Pile Load Testing Report

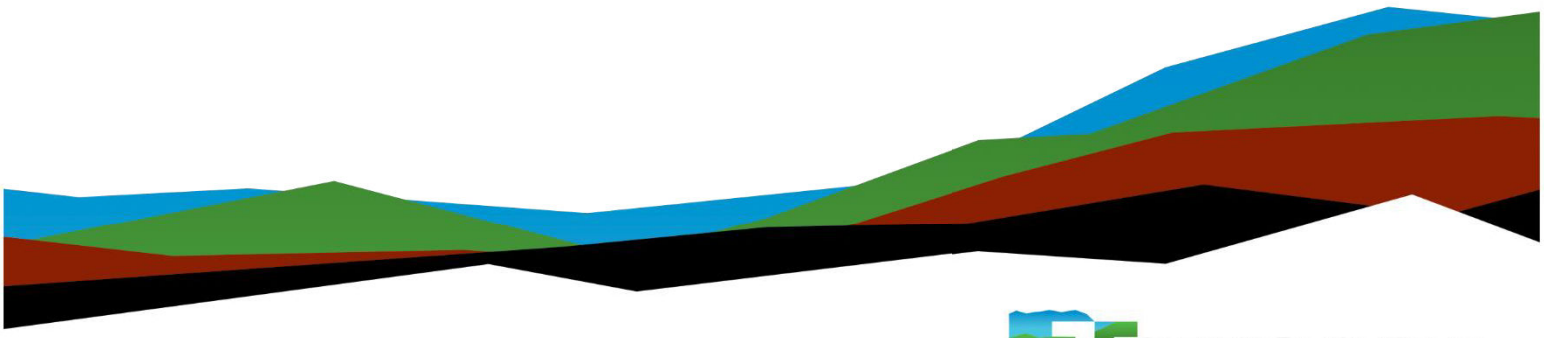
Grace Solar

Geotechnical Engineering and Pile Load Testing Report

January 21, 2026 | Terracon Project No. CB255143

Prepared for:

Cupertino Electric, Inc.
1132 N. 7th Street
San Jose, CA 95112



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January 21, 2026

Cupertino Electric, Inc.
1132 N. 7th Street
San Jose, CA 95112

Attn: Mrs. Katie (Miller) Waddel – Preconstruction Project Manager
P: (772)
E: Yesenia.Diaz@nexteraenergy.com

Re: Geotechnical Engineering and Pile Load Testing Report
Grace Solar
Blythe, Riverside County, CA
Terracon Project No. CB255143 (formerly 60225164)

Dear Mrs. Waddel:

Terracon has completed the Geotechnical Engineering and Pile Load Testing services for the above-referenced project in general accordance with our scope and fee summary provided via email on October 10, 2025, with a final revision proposal dated November 11, 2025. This report presents the results of the subsurface exploration and provides design-level geotechnical recommendations for inverter, CAB, and LBD foundations, lightly loaded ancillary structures, and access roads within the proposed solar facility. Geotechnical recommendations for the PV array structures are not included within this report and are understood to be provided by others.

We appreciate the opportunity to be of service to you on this project. If you have any questions concerning this report or if we may be of further service, please contact us.

Sincerely,

Terracon



Victor V. Nguyen, P.E.
Project Engineer

Joshua R. Morgan, P.E.
Regional Geotechnical Manager

SME Review by Jimmy Jackson, P.E. (FL)

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Field Exploration Results
Laboratory Test Results
Field Soil Electrical Resistivity Results
Test Pile Driving Data
Pile Load Test Results
Supporting Information

Note: This report was originally delivered in a web-based format. **Blue Bold** text in the report indicates a referenced section heading. The PDF version also includes hyperlinks which direct the reader to that section and clicking on the terracon logo will bring you back to this page. For more interactive features, please view your project online at **client.terracon.com**.

Refer to each individual Attachment for a listing of contents.

Potential Geotechnical Constraints

Item	Overview Statement [1]
Pile Drivability	Pile driving refusal was not encountered during Terracon's preliminary pile load testing program, which evaluated embedment depths of 7 and 10 feet. In addition, supplemental pile load testing performed by ANS Geo, targeting embedment depths ranging from 5 to 8 feet, as documented in ANS Geo's pile load testing report dated November 7, 2025, did not encounter pile driving refusal. Based on the results of both testing programs and the subsurface conditions encountered, pile driving difficulty is not anticipated within the upper 10 feet at the site.
Shallow Bedrock	Shallow bedrock is not anticipated at the site.
Shallow Groundwater	Shallow groundwater is not anticipated at the site.
Liquefaction and Related Hazards	Based on review of the Riverside County GIS, the site is located within "Moderate" severity liquefaction hazard zone as designated by the County of Riverside. However, based on the anticipated depth to historic groundwater and encountered subsurface conditions, liquefaction hazard potential at the site is considered to be low. Other hazards related to liquefaction, such as lateral spreading, are also considered low.
Karst	Karst is not a concern at this site.
Frost Potential	Frost susceptibility is not anticipated at the site.
Expansive Soils	Expansive soils were not observed at the site.
Collapsible Soils	Near-surface collapsible soils were encountered at the site within the substation area. Mitigative measures including over-excavation and re-compaction have been provided in the text of this report to support shallow concrete foundations. Collapsible soils are not anticipated to adversely affect performance of driven steel piles.

1. This summary is for convenience only. It should be used in conjunction with the entire report for design purposes.

Introduction and Previous Reports

This report presents the results of our subsurface exploration and Geotechnical Engineering and Pile Load Testing Report performed for the proposed Grace Solar power facility to be located near Blythe, Riverside County, CA. The purpose of these services was to provide final design information and geotechnical engineering recommendations relative to foundations supporting inverter, CAB (cable management systems), LBD (local break disconnect) equipment, and other minor ancillary support structures within the proposed solar array development area. Recommendations for PV array foundations are being provided by others.

Prior to issuance of this report, Terracon prepared a preliminary geotechnical report titled *Grace Solar Preliminary Geotechnical Engineering and Pile Load Testing Services*, dated June 7, 2024 (Terracon Project No. 60225164). That report documented subsurface exploration and testing activities and provided preliminary geotechnical recommendations for construction of the Grace Solar Facility, including preliminary foundation and access road recommendations for PV and substation structures.

Subsequent to issuance of the preliminary report, Terracon prepared one geotechnical report addendum and one additional geotechnical report for the Grace Solar project, as summarized below:

- *Geotechnical Report Addendum 1 – Supplemental Array Exploration and Access Road Design, Grace Solar* dated June 5, 2025, Terracon Project No. 60225164. This addendum documented supplemental borings within the PV area and provided design-level recommendations for unpaved and concrete access roads along Buck Boulevard leading to the site entrance.

- *Grace Solar BESS and Substation, Geotechnical Engineering and Pile Load Testing Report – Rev. 1* dated October 8, 2025, Terracon Project No. 60225164. This report provided design-level foundation and access road recommendations for the final substation and battery energy storage system (BESS) area.

As initially mentioned, Terracon understands that a final, design-level geotechnical engineering and pile load testing report specific to inverter, CAB, and LBD foundations, lightly loaded ancillary equipment within solar fields, and associated access roads within the PV fields is requested. The design level driven pile recommendations are based on both preliminary pile load testing previously conducted by Terracon, and supplemental pile load testing conducted by ANS Geo, which was observed by Terracon. Design-level recommendations for these site elements are presented in this report. ANS Geo and GameChange Solar are providing recommendations and design of PV Array foundations. Recommendations for these elements are not included in this report.

The geotechnical engineering Scope of Services for this project included soil borings, test pits, field electrical resistivity testing, laboratory thermal resistivity testing, laboratory corrosion testing, and pile load testing. Additional details can be found in the **Exploration and Testing Procedures** section of this report. In addition, the ANS Geo pile load testing report is provided in the **Supporting Information** section of this report.

Project Description

Our initial understanding of the project was provided in our proposal and was discussed during project planning. A period of collaboration has transpired since the project was initiated, and our final understanding of the project conditions is as follows:

Item	Description
Information Provided	During the proposal stage, the following information was provided to Terracon: ■ Pull Testing Procedure (PTP) dated September 23, 2025 prepared by Gamechange Solar which details the pile load testing plan for the project ■ KMZ titled PTP_Cupertino-Grace_23.Sep.2025 which provided the latest PV boundary and pile load test locations associated with the PTP. ■ Gamechange Solar – Cupertino – Grace – Solar Project: Pile Load Testing Report, Blythe, CA prepared by ANS Geo dated November 7, 2025
Project Description	It is our understanding that the Client intends to develop a photovoltaic (PV) electric power plant with an array area of approximately 2,560 acres. The completed facility will consist of solar panels supported on steel structures, associated electrical equipment, a BESS yard, and a substation area. Geotechnical recommendations for the design and construction of the proposed BESS and substation facilities are provided in Terracon's report titled *Grace Solar BESS and Substation, Geotechnical Engineering and Pile Load Testing Report – Rev. 1*, dated October 8, 2025 (Terracon Project No. 60225164), and are therefore outside the scope of this report. This report is limited to geotechnical recommendations for inverters, CAB, LBD, lightly loaded ancillary structures, and access roads. Geotechnical recommendations for the PV array structures are not included herein and are understood to be provided by others.

Item	Description
Proposed Structures	The proposed project will include the construction of ground-mounted solar panels on steel racks, preferably founded on driven W-Section steel beams (W6x9 or similar). Electrical equipment and substation elements are anticipated to be supported on concrete slabs-on-grade, spread footings, or drilled piers.
Maximum Loads (assumed based on previous experience)	We have estimated the following foundation loads for the project: Inverters: ■ 50 kips MET Station Slabs: ■ Less than 100 psf contact pressure
Grading/Slopes	We anticipate that solar array fields will generally follow existing grades with minimal grading required to bring the area to finished grade.
Berms	Permanent berms with approximate heights of 4 feet and side slopes of 3:1 are planned within the perimeter of the substation and stormwater retention areas.
Access Roads	Unpaved access roads are planned for the site as described below: ■ Access roads are to support post-construction traffic which we understand will be primarily light maintenance vehicles. The roads will be required to support a maximum vehicle load of 80,000 pounds for fire truck access. ■ We understand it is acceptable for the access roads to require ongoing maintenance throughout their design life.

Terracon should be notified if any of the above information is inconsistent with the planned construction, as modifications to our recommendations may be necessary.

Site Conditions

The following description of site conditions is derived from our site visit in association with the field exploration and our review of publicly available geologic and topographic maps.

Item	Description
Parcel Information	The project site is located at Blythe, Riverside County, California and is approximately 2,560 acres in size. Coordinates near the approximate center of the project site are: 33.6539°N, - 114.6909°W. See **Site Location**

Item	Description
Existing Improvements	The site is primarily undeveloped. Nearby features include unpaved access roads, an agricultural area near the center of the site, solar array fields to the west and south of the site, and the Blythe Airport south of the site.
Current Ground Cover	The majority of the surface of the site is covered by exposed soils with sparse desert vegetation, several plots appear to be previously used for agricultural purposes but are now fallow.
Existing Topography (From Google Earth Pro™)	Site-specific topographic map was not available at the time of this report. Based on review of publicly available topographic data from Google Earth Pro™, elevations within the project area range approximately from 310 to 430 feet MSL and generally decreases in elevation towards the southeast direction.

Geotechnical Characterization

Site Geology

The site is located in the eastern portion of the Mojave Desert geomorphic province, a broad interior region characterized by isolated mountains separated by expanses of desert plains that include playa lakes. The site region is arid and topography includes hills with dissected older alluvial aprons and younger sediments as alluvial plains. Alluvium in the site area is derived from adjacent highlands (McCoy Mountains) and includes Pleistocene age sediments on Palo Verde Mesa and younger recent wash deposits. The Colorado River is approximately 7 miles to the east.

Surficial geologic materials are mapped as recent alluvium of valley plains and wash areas and Pleistocene age alluvium of Palo Verde Mesa that underlie the majority of the site (Stone, 2006)[1]. Materials encountered in site explorations include gravel layers and caliche horizons (an accumulation of carbonate derived from precipitation) typical of the arid Mojave environment. Caliche-rich and reddish soils typically reflect pre-Holocene age sediments consistent with published geologic mapping.

Based on examination of historical aerial imagery, the site surface was previously disturbed for agricultural use. As encountered in site explorations, soils materials include

[1] Stone, Paul, 2006, Geologic map of the west half of the Blythe 30' by 60' quadrangle, Riverside County, California and La Paz County, Arizona, U.S. Geological Survey, Scientific Investigations Map SIM-2922, 1:100,000

silty sand, gravelly sands, poorly- to well-graded sands and locally silt layers. These sediments are interpreted as locally less dense, disturbed surficial materials derived from and mantling older, more dense materials, consistent with the mapped older alluvial deposits.

Soil Survey

The Soil Survey for the Palo Verde Area of Riverside County, California, as prepared by the United States Department of Agriculture (USDA), Natural Resource Conservation Service - NRCS, identifies several soil types within the project area with the majority (~87%) being Carsitas-Rositas-Buzzard Springs complex 0% to 5% slopes. The soil survey presents shallow (typically upper 80 inches) soil stratification information.

The Carsitas-Rositas-Buzzard Springs complex is classified as somewhat excessively drained, with depth to restrictive feature more than 80 inches, and calcium carbonate content up to 25%. Additional soils units include Carrizo Gravelly Sand, Badland, Orita Gravelly Loamy Sand and Aco Sand Loam that comprise approximately 10% of the project area. These additional units occur in the southern portion of the site area within and adjacent to an unnamed dry wash. All soils types are typical of the arid desert environment.

Exploration Results

We have developed a general characterization of the subsurface conditions based upon our review of the subsurface exploration, laboratory data, geologic setting, and our understanding of the project.

Subsurface soils encountered at the site generally consisted of loose to very dense sand with varying amounts of silt, clay and, gravel with occasional interbedded pockets of very stiff to hard clay layers with varying amount of sand and gravel.

Conditions observed at each exploration point are indicated on the individual logs. The individual logs can be found in the **Exploration Results** attachment of this report.

Groundwater

The site is located within the Palo Verde Mesa Groundwater Basin. The Basin is bounded by mountains to the north and west and is drained by McCoy Wash that trends near the northeast portion of the site and drains to the Colorado River. According to the State of California Water Data Library (WDL) – Groundwater Data website, groundwater is deeper than 150 feet bgs in the site area.

Groundwater was not observed in the borings and test pits while drilling/excavation, or for the short duration the boring/test pits remained open. Groundwater is not anticipated to occur within the depth of excavations or foundation installations at the site.

These observations represent groundwater conditions at the time of the field exploration and may not be indicative of other times, or at other locations.

Groundwater conditions may change because of seasonal variations in rainfall, runoff, and other conditions not apparent at the time of exploration. Therefore, groundwater levels during construction or at other times may be higher or lower than expected.

Field Electrical Resistivity

Field measurements of soil electrical resistivity were performed by Terracon between October 24 to November 15, 2023. The location and results of the field electrical resistivity testing are included in the **Field Soil Electrical Resistivity Test Results** section of this report.

Laboratory Thermal Resistivity

Forty-two laboratory thermal resistivity tests were completed on remolded soil samples. Undisturbed thermal tests were not conducted due to the sandy conditions encountered and the high potential for disturbance during long distance transport to the GeoTherm testing lab. In lieu of this, an additional remolded thermal test was conducted on bulk samples at approximate depths of 4 to 6 feet bgs. The individual laboratory thermal resistivity dry-out curves are provided in the **Laboratory Test Results** section of this report and are summarized below. The compaction percentages shown in the table are relative to the soil's standard Proctor (ASTM D698) maximum dry density.

Thermal Resistivity Test Results Summary

Test Condition	Thermal Resistivity (°C-cm/W)				Number of Tests
	Parameter	Min.	Max.	Average	
Remolded to 85% (4 to 6 ft-bgs)	Remolded Wet	55	76	63	21
	Remolded Dry	142	228	178	
Remolded to 90% (1 to 4 ft-bgs)	Remolded Wet	43	56	48	21
	Remolded Dry	134	202	165	

Thermal Resistivity Test Results Summary

Test Condition	Thermal Resistivity (°C-cm/W)				Number of Tests
	Parameter	Min.	Max.	Average	

1. The "Remolded Wet" samples were tested near the higher of either the optimum or in-situ moisture content.
2. The "Dry" samples were tested at a moisture content of near 0%.

Laboratory Corrosion Testing

The table below lists the results of laboratory pH, soluble sulfate, sulfides, soluble chloride, total salts, oxidation-reduction potential (redox), and laboratory electrical resistivity testing. The values may be used to estimate potential corrosive characteristics of the on-site soils with respect to contact with the various underground materials which will be used for project construction.

Corrosivity Test Results Summary

Parameter	Number of Tests	Min.	Max.	Average
pH	31	8.29	9.73	9.23
Water Soluble Sulfate (percent %)	31	<0.01	0.02	0.01
Sulfides (mg/kg)	31	Nil	Nil	Nil
Soluble Chloride (mg/kg)	31	17	332	90
Total Salts (mg/kg)	31	229	2,632	660
Redox (mV)	31	712	734	735
Saturated Minimum Electrical Resistivity (Ω-cm)	31	348	5,092	1,690

Results of soluble sulfate testing can be classified in accordance with ACI 318 – Building Code Requirements for Structural Concrete. Based on review of ACI 318, soluble sulfate testing indicates samples of the on-site soils tested have sulfate concentrations ranging from exposure class S0 when classified in accordance with Table 19.3.1.1 of the ACI

Design Manual. Concrete should be designed in accordance with the exposure class provisions of the ACI Design Manual, Section 318, Chapter 19.

Numerous sources are available to characterize corrosion potential to buried metals using the parameters above. Section 10.7.5 of the AASHTO LRFD Bridge Manual, 8th Edition, 2017, states the following soil or site conditions should be considered as indicative of potential deterioration or corrosion situation for steel piles:

- Soil electrical resistivity less than 2,000 ohm-cm
- Ph less than 5.5
- Ph between 5.5 and 8.5 with high organic content
- Sulfate concentration greater than 1,000 ppm (mg/kg)

These test results are provided to assist in determining the type and degree of corrosion protection that may be required. We recommend that a NACE certified corrosion professional be retained to analyze the need for corrosion protection and to design appropriate protective measures, if required.

Imported fill materials may have significantly different properties than the site materials noted above and should be evaluated if expected to be in contact with metals used for construction.

Seismic Site Class

The 2022 California Building Code (CBC) Seismic Design Parameters have been generated using the ASCSE Hazard Tool. This web-based software application calculates seismic design parameters in accordance with ASCE 7-16 and 2022 CBC. The seismic design parameters are summarized in the following table.

Description	Value
2022 California Building Code Site Classification (CBC) [1]	D [2]
Site Latitude (°N) [3]	33.6539
Site Longitude (°W) [3]	114.6909
S_s Spectral Acceleration for a 0.2-Second Period	0.313
S_1 Spectral Acceleration for a 1-Second Period	0.162
F_a Site Coefficient for a 0.2-Second Period	1.54

Description	Value
F_V Site Coefficient for a 1-Second Period	2.27
S_{DS} Spectral Response Acceleration Parameter at Short Periods	0.324
S_{D1} Spectral Response Acceleration Parameter at 1-Second Period	0.245
Site Modified Peak Ground Acceleration PGA_M	0.213 g

1. Seismic site classification in general accordance with the *2022 California Building Code*.
2. The 2022 California Building Code (CBC) requires a site soil profile determination extending to a depth of 100 feet for seismic site classification. The current scope does not include the required 100-foot soil profile determination. Borings were extended to a maximum depth of 51½ feet, and this seismic site class definition considers that similar soils continue below the maximum depth of the subsurface exploration. Additional exploration to deeper depths would be required to confirm the conditions below the current depth of exploration.
3. A 50-foot boring was advanced near the substation area and was subsequently utilized to determine seismic site class for the site.

Faulting and Estimated Ground Motions

The site is located in eastern California, which is a region of moderate seismic activity. The type and magnitude of seismic hazards affecting the site are dependent on the distance to causative faults, the intensity, and the magnitude of the seismic event. As estimated using the USGS Unified Hazard Tool, the San Andreas fault and Brawley Seismic zone are the closest major faults to the site at a distance of approximately 56 miles. However, the majority of seismic hazard is 'grid based' background seismicity associated with non-fault sources in the seismic hazard model.

Based on the USGS Design Maps Summary Report, using the American Society of Civil Engineers (ASCE 7-16) standard, the peak ground acceleration (PGA_M) for design at the project site is 0.22 g. Based on the USGS Unified Hazard Tool, the project site has a mean magnitude of 6.8. Furthermore, the site is not located within an Alquist-Priolo Earthquake Fault Zone based on our review of the State Fault Hazard Maps and County of Riverside Fault Hazard Maps.[2]

[2] California Geological Survey (CGS), https://maps.conservation.ca.gov/cgs/informationwarehouse/regulatorymaps/

Liquefaction

Liquefaction is a mode of ground failure that results from the generation of high porewater pressures during earthquake ground shaking, causing loss of shear strength. Liquefaction is typically a hazard where loose sandy soils exist below groundwater. The California Geological Survey (CGS) has designated certain areas as potential liquefaction hazard zones. These are areas considered at a risk of liquefaction-related ground failure during a seismic event, based upon mapped surficial deposits and the presence of a relatively shallow water table.

The site has not been mapped for liquefaction hazard by the California Geologic Survey. Based on review of the Riverside County GIS, the site is located within "Moderate" severity liquefaction hazard zone as designated by the County of Riverside. However, based on the anticipated depth to historic groundwater, liquefaction hazard potential at the site is considered to be low. Other geologic hazards related to liquefaction, such as lateral spreading, are therefore also considered low.

Pile Load Testing

We completed a preliminary pile load testing program that included:

- Directing the installation of a group of two test piles at each of 35 locations. A third pile for compression testing was installed at all test locations.
- Performing pile load testing under axial compressive loads for one test pile at each group (35 tests).
- Performing pile load testing under axial tensile loads for two test piles in each group (70 tests).
- Performing pile load testing under lateral loads for two test piles in each group (70 tests).

In addition, ANS Geo conducted a supplemental pile load testing program that was observed by Terracon and included:

- Directing the installation of a group of two W6x9 test piles at each of 76 locations. A third W6x9 pile for compression testing was installed at 12 locations. In addition, one W6x12 test pile was installed at 16 locations.

- Performing pile load testing under axial compressive loads for one W6x9 test pile at 12 locations (12 tests).
- Performing pile load testing under axial tensile loads for two W6x9 test piles in each group (152 tests) and one W6x12 test piles at 16 locations (16 tests) for a total of 168 tests.
- Performing pile load testing under lateral loads for two test piles in each group (152 tests) and one W6x12 test piles at 16 locations (16 tests) for a total of 168 tests.

A summary of the total drive times and load test results is provided below. A summary of the installation procedures and drive time curves are included in the **Test Pile Driving Data** section.

Summary of Pile Load Test Results

Terracon's individual pile load test results are provided in the **Pile Load Test Results** section of this report. ANS Geo's individual pile load testing results are detailed in their pile load testing report dated November 7, 2025 provided in the **Supporting Information** section of this report. All Terracon pile load tests (denoted as PLT-X) were conducted with W6x9 piles while ANS Geo's pile load tests (denoted as PT-X) were conducted on a combination of W6x9 and W6x12 piles. The following table provides a summary of each test pile location, embedment depth, total drive time, uplift load at ¼-inch of vertical displacement, lateral load at ½-inch of lateral displacement, and compressive load at ¼-inch of vertical displacement:

Pile Load Test Results Summary

Pile ID	Pile Emb. Depth (ft-bgs)	Total Drive Time (seconds)	Uplift Load at ¼" Displacement (lbs) [1]	Lateral Load at ½" Displacement (lbs) [1]	Compression Load at ¼" Displacement (lbs) [1]
PLT-1A	7	32	>10,000	>7,000	---
PLT-1B	10	52	>10,000	>7,000	---
PLT-1C	7	34	---	---	>10,000
PLT-2A	7	24	>10,000	5,920	---
PLT-2B	10	42	>10,000	>7,000	---
PLT-2C	7	16	---	---	>10,000
PLT-3A	7	17	>10,000	5,990	---
PLT-3B	10	44	>10,000	4,680	---

Pile Load Test Results Summary

Pile ID	Pile Emb. Depth (ft-bgs)	Total Drive Time (seconds)	Uplift Load at ¼" Displacement (lbs) [1]	Lateral Load at ½" Displacement (lbs) [1]	Compression Load at ¼" Displacement (lbs) [1]
PLT-3C	7	26	---	---	>10,000
PLT-4A	7	20	>10,000	6,190	---
PLT-4B	10	46	>10,000	6,220	---
PLT-4C	7	21	---	---	>10,000
PLT-5A	7	26	>10,000	>7,000	---
PLT-5B	10	42	>10,000	>7,000	---
PLT-5C	7	19	---	---	>10,000
PLT-6A	7	20	>10,000	>7,000	---
PLT-6B	10	74	>10,000	>7,000	---
PLT-6C	7	35	---	---	>10,000
PLT-7A	7	21	>10,000	5,430	---
PLT-7B	10	45	>10,000	>7,000	---
PLT-7C	7	19	---	---	>10,000
PLT-8A	7	30	>10,000	>7,000	---
PLT-8B	10	45	>10,000	>7,000	---
PLT-8C	7	20	---	---	>10,000
PLT-9A	7	13	5,750	2,860	---
PLT-9B	10	32	>10,000	3,520	---
PLT-9C	7	19	---	---	>10,000
PLT-10A	7	16	4,520	3,470	---
PLT-10B	10	34	>10,000	5,390	---

Pile Load Test Results Summary

Pile ID	Pile Emb. Depth (ft-bgs)	Total Drive Time (seconds)	Uplift Load at ¼" Displacement (lbs) [1]	Lateral Load at ½" Displacement (lbs) [1]	Compression Load at ¼" Displacement (lbs) [1]
PLT-10C	7	13	---	---	>10,000
PLT-11A	7	23	>10,000	5,410	---
PLT-11B	10	46	>10,000	4,370	---
PLT-11C	7	19	---	---	>10,000
PLT-12A	7	19	>10,000	5,890	---
PLT-12B	10	44	>10,000	5,780	---
PLT-12C	7	27	---	---	>10,000
PLT-13A	7	19	>10,000	>7,000	---
PLT-13B	10	36	>10,000	>7,000	---
PLT-13C	7	14	---	---	>10,000
PLT-14A	7	16	>10,000	5,490	---
PLT-14B	10	40	>10,000	>7,000	---
PLT-14C	7	16	---	---	>10,000
PLT-15A	7	26	>10,000	>7,000	---
PLT-15B	10	40	>10,000	5,650	---
PLT-15C	7	28	---	---	>10,000
PLT-16A	7	11	>10,000	4,350	---
PLT-16B	10	32	5,580 [2]	5,520	---
PLT-16C	7	17	---	---	>10,000
PLT-17A	7	25	>10,000	4,550	---
PLT-17B	10	34	>10,000	4,370	---

Pile Load Test Results Summary

Pile ID	Pile Emb. Depth (ft-bgs)	Total Drive Time (seconds)	Uplift Load at ¼" Displacement (lbs) [1]	Lateral Load at ½" Displacement (lbs) [1]	Compression Load at ¼" Displacement (lbs) [1]
PLT-17C	7	14	---	---	>10,000
PLT-18A	7	24	>10,000	>7,000	---
PLT-18B	10	51	>10,000	6,700	---
PLT-18C	7	31	---	---	>10,000
PLT-19A	7	33	9,990	4,060	---
PLT-19B	10	66	>10,000	4,280	---
PLT-19C	7	29	---	---	>10,000
PLT-20A	7	29	5,550	2,560	---
PLT-20B	10	50	>10,000	3,510	---
PLT-20C	7	25	---	---	>10,000
PLT-21A	7	23	>10,000	5,790	---
PLT-21B	10	52	>10,000	5,650	---
PLT-21C	7	15	---	---	>10,000
PLT-22A	7	20	>10,000	4,910	---
PLT-22B	10	40	>10,000	5,670	---
PLT-22C	7	22	---	---	>10,000
PLT-23A	7	22	>10,000	5,500	---
PLT-23B	10	66	>10,000	5,640	---
PLT-23C	7	31	---	---	>10,000
PLT-24A	7	12	8,240	3,100	---
PLT-24B	10	16	>10,000	3,760	---

Pile Load Test Results Summary

Pile ID	Pile Emb. Depth (ft-bgs)	Total Drive Time (seconds)	Uplift Load at ¼" Displacement (lbs) [1]	Lateral Load at ½" Displacement (lbs) [1]	Compression Load at ¼" Displacement (lbs) [1]
PLT-24C	7	11	---	---	>10,000
PLT-25A	7	19	>10,000	4,640	---
PLT-25B	10	44	>10,000	4,990	---
PLT-25C	7	16	---	---	>10,000
PLT-26A	7	26	>10,000	>7,000	---
PLT-26B	10	61	>10,000	>7,000	---
PLT-26C	7	27	---	---	>10,000
PLT-27A	7	58	>10,000	>7,000	---
PLT-27B	10	79	>10,000	>7,000	---
PLT-27C	7	18	---	---	>10,000
PLT-28A	7	11	>10,000	5,570	---
PLT-28B	10	38	>10,000	>7,000	---
PLT-28C	7	16	---	---	>10,000
PLT-29A	7	16	>10,000	>7,000	---
PLT-29B	10	58	>10,000	>7,000	---
PLT-29C	7	38	---	---	>10,000
PLT-30A	7	20	>10,000	>7,000	---
PLT-30B	10	60	>10,000	>7,000	---
PLT-30C	7	25	---	---	>10,000
PLT-31A	7	36	>10,000	>7,000	---
PLT-31B	10	70	>10,000	>7,000	---

Pile Load Test Results Summary

Pile ID	Pile Emb. Depth (ft-bgs)	Total Drive Time (seconds)	Uplift Load at ¼" Displacement (lbs) [1]	Lateral Load at ½" Displacement (lbs) [1]	Compression Load at ¼" Displacement (lbs) [1]
PLT-31C	7	38	---	---	>10,000
PLT-32A	7	32	>10,000	4,580	---
PLT-32B	10	41	>10,000	4,970	---
PLT-32C	7	28	---	---	>10,000
PLT-33A	7	22	>10,000	5,560	---
PLT-33B	10	41	>10,000	5,260	---
PLT-33C	7	20	---	---	>10,000
PLT-34A	7	22	>10,000	4,320	---
PLT-34B	10	48	>10,000	5,590	---
PLT-34C	7	27	---	---	>10,000
PLT-35A	7	22	>10,000	5,650	---
PLT-35B	10	60	6,600 [2]	4,590	---
PLT-35C	7	19	---	---	>10,000

1. The ">" sign indicates the load was achieved prior to reaching the noted deflection.
2. The tension results for deeper test piles (10 ft embedment) at PLT-16 and PLT-35 indicated significant variability compared to shorter piles at the same test locations. For piles embedded greater than 7 ft at these two locations, we recommend additional pile load test to be conducted to confirm the available resistance.

Geotechnical Overview

The site appears suitable for the proposed construction based upon geotechnical conditions encountered in the test borings, provided that the findings and recommendations presented in this report are incorporated into project design and construction.

We understand that inverter, CAB, and LBD structures may be supported on driven piles or isolated mat/slab foundation systems.

Geotechnical engineering recommendations for pile foundation systems for this project are presented in the following sections. These recommendations are based on the results of field testing, the use of direct-drive embedment piles or mat/slab foundations to support the inverters, and our current understanding of the proposed project. If alternative installation methods or support systems are proposed, we should be contacted to review the design and/or to perform pile load testing using the proposed installation methods.

Driven pile axial and lateral recommendations presented below may also be applied to other non-PV panel structures (i.e. embedded poles) supported on driven piles. Other structures supported on driven piles may require piles to be driven to greater depths in order to achieve the required axial capacities.

Potentially compressible soils, which show significant tendency for hydro-compaction when elevated in moisture content were encountered on site. Such soils will require particular attention in the design and construction. Hydro-compactive soils, sometimes referred to as collapsible soils, are capable of supporting typical building loads at natural moisture contents, these same materials however, undergo volume decrease, including settlement and consolidation, when subjected to increases in moisture content under constant load. These collapsible materials are anticipated to impact structures supported on shallow foundation systems. Collapsible soils are not anticipated to adversely affect performance of structures supported on driven steel piles.

Inverters, CAB, LBD, and other non-PV panel structures supported on a shallow foundation system should bear on structural fill extending to a minimum depth of 3 feet below the bottom of foundations, or 6 feet below existing site grades, whichever is greater.

Estimated movements described in this report are based on effective drainage for the life of the structure and cannot be relied upon if effective drainage is not maintained. Exposed ground, extending at least 10 feet from the perimeter, should be sloped a minimum of 5% away from the structure to provide positive drainage away from the structure. Grades around the structure should be periodically inspected and adjusted as part of the structure's maintenance program.

Geotechnical engineering recommendations for foundation systems and other earth connected phases of the project are outlined below. The recommendations contained in this report are based upon the results of test borings, laboratory testing, engineering analyses, and our current understanding of the proposed project. The **General Comments** section provides an understanding of the report limitations.

Driven Steel Piles

Based on the results of the axial and lateral pile load testing program, we have separated the site into two groups for axial parameters and three groups for lateral parameters. Each

PLT location was assigned into either group 1 or 2 based on the axial tension test performance, and into either group A, B, or C based on the lateral test performance. The project site was then zoned by matching test locations by their axial tension and lateral group. The resulting zones are then A1, A2, B1, B2, and C1 where the zone numbers correspond to the axial tension parameter group and the zone letter corresponds to the lateral parameter group.

Significant variability was not encountered during compression testing and therefore, the compressive capacities provided are applicable for the entirety of the site.

The following table presents the results of the zoning on the site:

Axial Tension Zone	Lateral Zone	Tested By	Pile Locations
1	A	Terracon	PLT-17, PLT-19, PLT-25, PLT-32, PLT-34
		ANS	PT-13, PT-46, PT-75, PT-76
	B	Terracon	PLT-3 , PLT-11, PLT-12, PLT-15, PLT-21, PLT-22
		ANS	PT-58, PT-70
	C	Terracon	PLT-1, PLT-2, PLT-4, PLT-5 through PLT-8, PLT-13, PLT-14, PLT-26 through PLT-31
		ANS	PT-2 through PT-12, PT-18, PT-19, PT-20, PT-23 through PT-29, PT-31, PT-33 through PT-38, PT-40, PT-41, PT-42 PT-45, PT-47, PT-48, PT-50, PT-52, PT-54 through PT-57, PT-59, PT-60, PT-63 through PT-71, PT-73, PT-74 PT-14*, PT-15*. PT-30*, PT-32*, PT-44*, PT-62* PT-22**, PT-72**
2	A	Terracon	PLT-20, PLT-24
		ANS	--[1]

Axial Tension Zone	Lateral Zone	Tested By	Pile Locations
	B	Terracon	PLT-16, PLT-35
		ANS	PT-16 PLT-39*
	C	Terracon	--[2]
		ANS	PT-1, PT-21, PT-49, PT-51, PT-53, PT-61 PT-43*

1. No Pile Load Test within the Terracon data falls under zone A2
2. No Pile Load Test within the ANS data falls under zone C2
3. PT-17 and PT-69 within the ANS data should not be considered due to large anomalies in the axial tension data We recommend load testing within these areas during construction to verify the available capacity.
4. PT-7 within the ANS data should not be considered due to large anomalies in the axial compression data. We recommend load testing within these areas during construction to verify the available capacity.
5. The lateral data in the 7-foot pile at PT-73 within the ANS data should not be considered due to large anomalies. We recommend load testing within these areas during construction to verify the available capacity.
6. The "*" sign indicates that the "B" pile experienced twisting and/or bending during testing, resulting in unavailable failure deflection data. The "**" sign indicates that both pile A and B experienced twisting and/or bending during testing, resulting in unavailable failure deflection data.
7. PLT-9, PLT-10, PLT-18, PLT-23, PLT-33 are outside of the latest PV array footprint and therefore are not considered in any soil zones.

A map of these zones is provided on the attached **Pile Load Test Zoning Plan** in the **Test Pile Driving Data** section of this report.

It should be noted that the pile performance varied significantly across the site, and while an attempt has been made to quantify this variability using spatial zones, isolated areas of lower strength soils within zones of relatively high strength soils may exist, and vice versa. Additional testing should be conducted to assess variability within the array areas and delineate the zones. When carrying out the quality control program of testing production piles across the site, some of these relatively weaker soil areas may be encountered. Depending on the array layout, controlling loads, and minimum pile embedment, additional testing may be warranted to delineate or better characterize the subsurface and pile performance in these areas.

Axial Capacity

The axial uplift capacity of driven piles may be estimated based on skin friction developed along the perimeter of the pile, while the compression capacity may be estimated using the skin friction and end bearing. When determining embedment depths, the perimeter of a wide flange beam should be taken as twice the sum of the flange width and section depth. Axial capacity should be neglected in the upper 1 foot or to the depth of scour, whichever is greater.

Significant variability during tension testing was observed in the pile load test results by Terracon for the 10-foot embedded piles at locations of PLT-16 and PLT-35. If piles are to be embedded at depths greater than 7 feet at these locations, we recommend additional pile load tests to be conducted to evaluate the available resistance. In addition, significant variability was observed in ANS pile load test results for the 5-foot embedded piles tested in tension at locations of PT-17 and PT-69 and compression at PT-7. We recommend additional pile load tests to be conducted to evaluate the available resistance.

The table below summarizes the recommended axial capacities for the various zones. These zones were developed based on the results of tension and compression pile load testing and are applicable to inverter, CAB, and LBD foundations.

Terracon's tension and compression load tests were terminated at either 10,000 pounds or a maximum deflection of 0.75 inch, whichever occurred first. ANS Geo's tension load tests were terminated at either 8,000 pounds or a maximum deflection of 1.0 inch, whichever occurred first, while ANS Geo's compression load tests were terminated at either 15,000 pounds or 1.0 inch of deflection, whichever occurred first.

All Terracon compression tests reached the maximum applied load of 10,000 pounds with minimal deflections (less than 0.05 inch), with the exception of PLT-13, which resulted in a deflection of 0.11 inch at the maximum target load. With the exception of PT-7 (which is excluded from the analysis due to significant variability) all ANS Geo compression tests reached the maximum applied load of 15,000 pounds.

Based on the consistency of the compression test results across the site and the minimal deflections upon reaching the maximum compressive load in Terracon's compression load tests, it is our opinion that the compressive skin friction values derived from Terracon's pile load tests may be considered in conjunction with the ANS Geo compression test data for development of the recommended axial capacities.

Axial Design Parameters – Uplift

Zone	Layer Depth (feet-bgs)	Ultimate Uplift Skin Friction, q_s (psf)[1, 2]	Recommended Factor of Safety
1	1 to 7	650	1.5
	7 to 10	850[3]	1.5
	10 to 20	850	2.0
2	1 to 6	400	1.5
	7 to 10	850	1.5
	10 to 20	850	2.0

1. If the site is subject to scour or disturbance deeper than 1 foot below ground surface, then that disturbed zone should be neglected from the calculation
2. The values provided in this table are only applicable to piles with a minimum embedment depth of 5 feet.
3. The tension results for deeper test piles (10 ft embedment) at PLT-16 and PLT-35 indicated significant variability compared to shorter piles at the same test locations. For piles embedded greater than 7 ft at these two locations, we recommend additional pile load test to be conducted to confirm the available resistance.

Axial Design Parameters – Compression

Layer Depth (feet-bgs)	Ultimate Compressive Resistance, q_s (psf)[1, 2, 3]	Recommended Factor of Safety
1 to 5	1,800	1.5
5 to 20	1,800	2.0

1. If the site is subject to scour or disturbance deeper than 1 foot below ground surface, then that disturbed zone should be neglected from the calculation
2. The values provided in this table are only applicable to piles with a minimum embedment depth of 5 feet.
3. Ultimate compressive resistance considers both skin friction and end bearing.

The above values are to be used in the following equations to obtain the ultimate uplift or compression load capacity of a pile:

$$Q_{ult\ (compressive)} = \Sigma H \times P \times Q_{s(compression)}$$
$$Q_{ult\ (uplift)} = \Sigma H \times P \times q_{s(uplift)}$$

Q_{ult} = Ultimate uplift or compression capacity of pile (lbs.)
H = Depth of embedment of pile (ft)
P = Box perimeter area/ft. of pile. (i.e. W6x9 = 1.64 ft.)

$q_{s(uplift)}$ = Skin friction per table above (psf).
$Q_{s(compression)}$ = Ultimate compressive resistance per table above (psf)

For Allowable Stress Design (ASD), the allowable compression and tension capacities should be determined by applying the factors of safety provided in the tables above.

An example calculation to determine the allowable tension capacity for an 8-foot embedded pile assuming a neglect depth of 1 foot, in Zone 1, is shown below:

$Q_{allowable\ (uplift)}$ = (7-1) x 1.64 x $\frac{650}{1.5}$ + (8-7) x 1.64 x $\frac{850}{1.5}$ = 5,193 pounds

An example calculation to determine the allowable compression capacity for a 15-foot embedded pile assuming neglect depth of 1 foot is shown below:

$Q_{allowable\ (uplift)}$ = (5-1) x 1.64 x $\frac{1,800}{1.5}$ + (15-5) x 1.64 x $\frac{1,800}{2.0}$ = 22,632 pounds

The provided skin friction values are applicable for piles that are driven using equipment similar to a Gayk HRE 1000 or GAYK HRE 4000 pile driver with a hydraulically operated hammer.

Final pile design to be completed by an engineer licensed in the State of California based upon information contained in this geotechnical report.

Lateral Capacity

Lateral load response of pile foundations was calculated using the computer program *LPile 2022*, by Ensoft, Inc. The stiffness of the pile and the stress-strain properties of the surrounding soils determine the lateral resistance of the foundation. We modeled the lateral response of the tested piles to evaluate LPile input parameters that can be used for design of the production piles. Recommended LPile input parameters for lateral load analysis for driven pile foundations are shown in the following table:

LPile Parameters for Zones A, B, and C

Depth Interval (feet-bgs)	LPile Soil Type	Effective Unit Weight (pcf)	Friction Angle (degree)	Soil/subgrade Modulus (k, pci)
0 to 20	Elastic Subgrade	105	---	100

1. The resistance of the upper 1-foot of soil should be neglected when modeling the pile in LPile to account for potential scour depths. The full depth of scour should be accounted for if deeper than 1 foot bgs. To account for point scour condition a subgrade modulus of 1 should be utilized. For a global scour condition the soils should start at the bottom of the scour depth.

The lateral load test results were varied between the different locations and embedment depths at the site. Therefore, we are providing the following table of p-multiplier values that should be used for the corresponding zone and embedment depth:

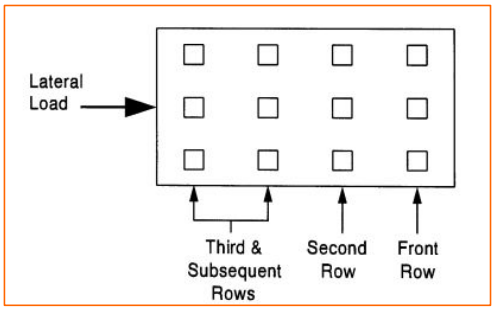
Zone	Pile Embedment Depth (feet-bgs)	P-Multiplier[1,2,3]
A	5	1.5
	10	1.5
B	5	3.0
	8	2.5
	10	
C	5	3.5
	8	
	10	4.5

1. Linear interpolation should be used to determine the p-multiplier for embedment depths between those shown in this table. The interpolated value should then be applied to the entire pile embedment depth.
2. The p-multiplier values provided in this table are only applicable to piles installed to a depth of at least 5 feet.
3. The p-multiplier value for the 10-foot-deep piles may be used for piles installed deeper than 10 feet.

Lateral analyses were performed by using LPile to generate a load vs. deflection curve that was generally consistent with the field load test results. The shear load was applied at approximately 2 feet above the ground surface for all Terracon tests whereas the shear load was applied at approximately 5 feet for all ANS tests. The effective unit weight and modulus of subgrade reaction (k-value) were based on the results of the SPT borings and laboratory results. The p- multiplier was then adjusted (by trial-and-error method) such that the applied load resulted in a deflection value that matched the load test results. Please note that this procedure was based on only one discrete set of data determined at about six inches from the ground surface during the field load testing. These results should be used for LPile analysis only using the 2022 version of LPile. These parameters are only applicable to piles installed a minimum of 5 feet bgs. In our evaluation, the piles were modeled as a Steel AISC Section Strong Axis.

The structural engineer should evaluate the moment capacity of the pile as part of their structural evaluation. Piles should have a minimum center-to-center spacing of at least five times their largest cross-sectional dimension in the direction of the lateral loads, or the lateral capacities should be reduced due to group effects.

Guidance for applying p-multiplier factors to the p values in the p-y curves for each row of pile foundations within a pile group are as follows:



Recommended P-Multipliers for Lateral Group Effects

Pile Center to Center Spacing (in the direction of loading)	P-Multipliers, P_M		
	Row 1	Row 2	Row 3
5.0B	1.0	0.85	0.70
4.0B	0.85	0.65	0.50
3.0B	0.7	0.50	0.35

Flowable Backfill Recommendations

It is our understanding that Controlled Low Strength Material (CLSM), also referred to as "flowable fill" will be utilized within the upper portion of driven piles supporting inverters, CAB, and LBD structures throughout the array areas. We understand that piles will be driven and soil surrounding the pile subsequently excavated to allow for installation of utilities and then backfilled with flowable fill. Flowable fill with the following properties is recommended:

- Unit Weight: 100 - 125 pcf
- Unconfined compressive strength at 28 days: ≥ 125 psi (excavatable)
- Flow consistency: >7 inches (per ASTM D6103)

If flowable fill is utilized around driven piles, we recommend bulkheads be used so that the flowable fill can be placed within the entire trench width and up to finished grade. We recommend neglecting the upper 1 foot of the CLSM backfill when determining the skin friction capacity on a pile. Below the upper 1 foot, an allowable skin friction of 150 psf should be used for the portion of the pile that is in contact with the flowable fill. The lateral parameters provided in the **Lateral Capacity Recommendations** section may be utilized to design the driven piles.

Construction Considerations

Based on the field exploration and pile load testing, it is our opinion that the soils on the site are suitable for direct drive pile installation into native soils.

The Engineering, Procurement, and Construction(EPC) contractor should perform production pile testing as part of a Quality Control plan. If unforeseen conditions are found during the pile installation and testing, the Geotechnical Engineer should be contacted to provide additional recommendations.

Shallow Foundations

Recommendations for foundations for the proposed structures and related structural elements are presented in the following paragraphs.

If the site has been prepared in accordance with the requirements noted in **Earthwork Considerations**, the following design parameters are applicable for shallow foundations.

Foundation Design Recommendations

Item	Description
Foundation System	Spread/strip footings, mat foundation, or support slab with thickened edges bearing on structural fill
Subgrade Preparation Requirements	Structural fill extending to a minimum depth of 3 foot below the bottom of foundations or 6 feet below existing site grades, whichever is greater.
Maximum Allowable Bearing pressure (based on settlement analysis) [1,2]	Spread Footings ■ 3,000 psf (up to 5 feet) ■ 2,900 psf (up to 10 feet) ■ 2,100 psf (up to 15 feet) Strip Footings ■ 3,000 psf (up to 5 feet) Mat Foundations or Support Slab with Thickened Edges ■ 2,300 psf (up to 10 by 20 feet) ■ 1,800 psf (up to 15 by 30 feet) ■ 1,500 psf (up to 20 by 40 feet) ■ 1,300 psf (up to 25 by 50 feet)
Design Modulus of Subgrade Reaction, k [3]	200 pounds per square inch per inch (psi/in). The modulus was obtained based on estimates obtained from NAVFAC 7.1 design charts. This value is for a small-loaded area (1 Sq. ft or less) such as for forklift wheel loads or point loads and should be adjusted for larger loaded areas.
Modulus Correction Factor [2]	$k_c = k_{v1} (B+1)^2/4B^2$
Minimum Embedment Below Finished Grade	18 inches
Minimum Dimensions	Square footings and mats: 24 inches Strip footings: 18 inches

Estimated Total Settlement from Structural Loads [4]	About 1-inch
Estimated Differential Settlement [4]	About ½ of total settlement over a horizontal distance of 50 feet

1. The maximum net allowable bearing pressure is the pressure in excess of the minimum surrounding overburden pressure at the foundation base elevation. A factor of safety of 3 has been applied to the bearing capacity calculation. These bearing pressures can be increased by 1/3 for transient loads unless those loads have been factored to account for transient conditions.
2. Settlement analysis is performed based on field and laboratory results obtained from substation boring location.
3. K_{v1} values should be reduced to account for dimensional effects of largely loaded areas. Where k_c is the corrected or design modulus value, K_{v1} is modulus of vertical subgrade reaction and B is the mat width in feet.
4. Settlement analysis considers that subgrade soils supporting foundations have been removed and replaced with structural fill in accordance with the *Subgrade Preparation* section of this report.

The allowable foundation bearing pressures apply to dead loads plus design live load conditions. The weight of the foundation concrete below grade may be neglected in dead load computations.

Foundation excavations should be observed by the Geotechnical Engineer. If the soil conditions encountered differ significantly from those presented in this report, Terracon should be contacted to provide additional evaluation and supplemental recommendations.

MET Station and Minor Equipment Slabs

We understand that lightly loaded exterior slabs supporting MET Stations and other minor appurtenant equipment are planned on site. These slabs, which are anticipated to have less than 100 psf contact pressure, may be supported on a minimum of 12 inches of scarified, moisture conditioned, and recompacted subgrade materials. We recommend slabs be designed with a minimum embedment of 3 inches below the anticipated scour depth at the site.

Lateral Earth Pressures

For structural fill comprised of on-site soils or imported low volume change materials above any free water surface, recommended equivalent fluid pressures for unrestrained foundation elements are:

ITEM	VALUE [1, 2]
Active Case	39 psf/ft
Passive Case	400 psf/ft
At-Rest Case	59 psf/ft

Coefficient of Friction	0.35

1. The values are based on s**tructural** fill materials used as backfill.
2. Uniform, horizontal backfill, compacted to at least 90% of the ASTM D 1557 maximum dry density, rendering a maximum unit weight of 125 pcf.

The lateral earth pressures herein do not include any factor of safety and are not applicable for submerged soils/hydrostatic loading. Additional recommendations may be necessary if such conditions are to be included in the design.

Fill against foundations should be compacted to densities specified in the Earthwork section of this report. Compaction of each lift adjacent to walls should be accomplished with hand-operated tampers or other lightweight compactors.

Earthwork Considerations

General

Grading plans were not available at the time of this report. Terracon should be given the opportunity to review grading plans when available so that we may revise any recommendations presented herein, if necessary. Based on the available information, we have assumed that earthwork for the project will include clearing and grubbing, remedial grading for shallow concrete foundations (on the order of 6 feet), trenching for cables and conduits, cutting and filling to achieve roadway grade. As the array area is expected to follow the natural topography, excavation or filling is anticipated to be less than 2 feet around piles supporting inverters, CAB, and LBD structures.

The site work conditions will be largely dependent on the weather conditions and the contractor's means and methods in controlling surface drainage and protecting the subgrade. Site preparation where shallow foundations are planned should include clearing and grubbing, installation of a site drainage system (where necessary), subgrade preparation, proof-rolling and vibratory densification, as necessary. Site preparation is not required for inverter, CAB, and LBD structures within the PV array fields where foundations consist of driven piles, except where grading is needed to improve site drainage; this is to preserve overall stability of ground surface and to minimize erosion in the area.

The recommendations presented for design and construction of earth supported elements including foundations, slabs, and access roadways are contingent upon following the recommendations outlined in this section.

Earthwork on the project should be observed and evaluated by Terracon. The evaluation of earthwork should include observation and testing of compacted fill, subgrade

preparation, foundation bearing soils, and other geotechnical conditions exposed during the construction of the project.

Site Preparation

Although no physical evidence of fills, utilities, or underground facilities such as septic tanks, cesspools, basements, and utilities was observed during the site reconnaissance, such features could be encountered during construction. If unexpected fills, utilities, or underground facilities are encountered, such features should be removed, and the excavation thoroughly cleaned prior to backfill placement and/or construction.

We recommend the surficial topsoil or deleterious material encountered during construction be removed from within the proposed on-site structure and access road limits to remove any organic material that may be present at the surface. Based on subsurface exploration, topsoil depth is anticipated to be less than 6 inches bgs.

The exposed subgrade should then be proof-rolled utilizing vibratory roller to delineate any loose areas. Following proof-rolling, the subgrade preparation for structures can begin.

Subgrade Preparation

Due to the collapse potential of near surface soils, structures supported on a shallow foundation system should bear on structural fill extending to a minimum depth of 3 feet below the bottom of foundation, or 6 feet below existing site grades, whichever is greater.

To provide lateral support, grading for the proposed foundations should incorporate the footprint of foundations plus a lateral distance of 8 inches beyond the outside perimeter of foundations of every foot of vertical over-excavation, where space is available.

Subgrade soils beneath exterior slabs and pavements should be scarified to a minimum depth of 12 inches, moisture conditioned, and compacted. The moisture content and compaction of subgrade soils should be maintained until non-structural slab or pavement construction.

Exposed areas which will receive compacted fill, once properly cleared and benched where necessary, should be scarified to a minimum depth of 10 inches, moisture conditioned, and compacted per the compaction requirements in this report.

Based upon the subsurface conditions determined from the geotechnical exploration, subgrade soils exposed during construction are anticipated to be relatively workable. However, the workability of the subgrade may be affected by precipitation, repetitive construction traffic or other factors. If unworkable conditions develop, workability may be improved by scarifying and moisture conditioning process.

Excavations

It is anticipated that excavations for the proposed construction can be generally accomplished with conventional earthmoving equipment.

The bottom of excavations should be thoroughly cleaned of loose soils and disturbed materials prior to backfill placement and/or construction.

Onsite soils consist of sandy soils. Such soils have the tendency to cave and slough during excavations. Therefore, formwork may be needed for foundation excavations.

The earthwork contractor is solely responsible for designing and constructing stable, temporary excavations as required to maintain stability of both the excavation sides and bottoms. Excavations should be sloped or shored in the interest of safety following local and federal regulations, including current OSHA excavation and trench safety standards. As a minimum, excavations should be performed in accordance with OSHA 29 CFR, Part 1926, Subpart P, "Excavations" and its appendices, and in accordance with any applicable local, and/or state regulations. Based on the currently available information, we have assumed that excavations deeper than 6 feet will not be required.

Fill Material Types

Compacted fill utilized on site is classified as either structural fill or non-structural fill. Structural fill is material used below and within a lateral distance of two feet from the perimeter edge of mat slab or shallow foundation and below access roadways. Non-structural fill is any fill utilized outside of these areas.

Imported fill materials should meet the following material property requirements. Regardless of its source, compacted fill should consist of approved materials that are free of organic matter and debris.

Fill Type[1]	USCS Classification	Acceptable Areas for Placement
Structural Fill	Sands with <30% P200, Organic Content (OC) <3%, LL<30, PI<15, and EI less than 20 (SP, SP-SM, SM, SC)	▪ Below shallow foundations and structural slabs ▪ Below roadways ▪ Berms

Fill Type[1]	USCS Classification	Acceptable Areas for Placement
Non-Structural Fill	Soils with OC <10% (Any USCS Classification)	■ Below areas outside of shallow foundations and structural slabs (utility trenches, non-structural slabs, etc.) ■ Berms

1. Structural fill should consist of approved materials that are free of organic matter and debris. A sample of each material type should be submitted to the Geotechnical Engineer for evaluation prior to use on this site.

Fill Material and Placement

Recommended compaction and moisture content criteria for compacted fill materials are as follows:

Material Type and Location	Per the Modified Proctor Test (ASTM D 1557)		
	Minimum Compaction Requirement	Range of Moisture Contents for Compaction Above Optimum[2]	
		Minimum	Maximum
On-site approved soils and/or low volume change imported fill:			
Beneath foundations:	90%	-3%	+3%
Berms less than 5 feet high:	90%	-3%	+3%
Fill greater than 5 feet in depth:	95%	-2%	+2%
Miscellaneous backfill:	90%	-3%	+3%
Utility trenches [1]:	90%	-3%	+3%
Bottom of excavation receiving fill:	90%	-3%	+3%
Beneath proposed pavements/roadways and exterior slabs:	95%	-3%	+3%
Aggregate base:	95%	-3%	+3%

1. Upper 12 inches should be compacted to 95% within structural areas. Compaction requirements within utility trenches should be verified with electrical engineer based on thermal resistivity and may be modified accordingly.
2. Provided compaction can be consistently met, moisture content range constraints can be modified.

Compacted fill should be placed and compacted in horizontal lifts, using equipment and procedures that will produce recommended moisture contents and densities throughout the lift. Fill lifts should not exceed 4 to 6 inches loose thickness when hand-guided equipment (i.e. jumping jack or plate compactor) is used. For self-propelled compaction equipment, fill lifts should not exceed 12 inches loose thickness.

Site Grading and Drainage

Positive drainage should be provided during construction and maintained throughout the life of the development. Infiltration of water into utility trenches or foundation excavations should be prevented during construction. Based on the location of the site, significant rainfall is not anticipated. Therefore, we recommend that protective slopes be provided with a minimum grade of approximately 2 percent for at least 10 feet from perimeter walls accordance with section 1804.4 of CBC 2022. Backfill against footings, exterior walls, and

in utility and sprinkler line trenches should be well compacted and free of all construction debris to reduce the possibility of moisture infiltration.

Shrinkage

For balancing grading on-site, estimated shrink factors of the on-site soils, when used as compacted fill, are provided in this section. Shrinkage factors are based on converting materials in its natural state before disturbance to materials after compaction. The earthwork factors are based on a range of target densities for varying compaction levels compared to the maximum dry density as determined by ASTM D1557. The estimated shrinkage of the near surface site soils when used as compacted fill is presented in the table below:

Percent Compaction (%)	Shrinkage (%)[1]
85	0 to 10
90	5 to 15
95	10 to 20

1. The degree of shrinkage is expected to vary across the site, however it our opinion that the midpoint of each range can be used to provide an estimated volume for import/export considerations.

These estimates are general in nature, and are based on our experience, limited data from our field exploration, results of laboratory tests, and the soil conditions we encountered at the site. Earthwork factors may vary dependent upon the actual subsurface conditions, which may include variations in soil gradations as well as gravel and fines content.

Berms

We understand that permanent berms with approximate heights of 4 feet and slopes of 3:1 are planned along the substation and stormwater retention areas. Furthermore, we understand berms will not retain water for a period longer than 72 hours. Based on this understanding, on-site soils are considered suitable for the construction of berms. Berms should be sodded to provide protection against erosion. However even with sodding and vegetating the proposed berm, some degree of sloughing should be anticipated. Berms should be inspected regularly to identify sloughing and dead patches which should be repaired immediately.

Utility Trenches

It is anticipated that the on-site soils will provide suitable support for underground utilities and piping that may be installed. Any soft and/or unsuitable material encountered at the

bottom of excavations should be removed and be replaced with an adequate bedding material. A non-expansive granular material with a sand equivalent greater than 30 should be used for bedding and shading of utilities, unless allowed or specified otherwise by the utility manufacturer.

On-site materials are considered suitable for backfill of utility and pipe trenches from one foot above the top of the pipe to the final ground surface, provided the material is free of organic matter and deleterious substances.

Trench backfill should be mechanically placed and compacted as discussed earlier in this report. Compaction of initial lifts should be accomplished with hand-operated tampers or other lightweight compactors. If trenches are placed beneath footings, the backfill should satisfy the gradation and expansion index requirements of structural fill discussed in this report. Flooding or jetting for placement and compaction of backfill is not recommended.

Earthwork Construction Considerations

We recommend that a Terracon geotechnical engineer or qualified representative be retained during the construction phase of the project to observe earthwork and to perform necessary tests and observations during stripping of topsoil or surficial organics (if any encountered), subgrade preparation, placement and compaction of controlled compacted fills, backfilling of excavations, and just prior to construction of any proposed foundations, slabs, or roadways.

Care should be taken to avoid disturbance of prepared subgrade soils. The near surface soils can be easily disturbed, especially by construction traffic. Construction traffic should not operate directly on saturated or low strength soils. If the subgrade becomes saturated, desiccated, or disturbed, the affected materials should either be scarified and compacted, or be removed and replaced as previously discussed. Subgrades should be observed and tested by Terracon prior to construction.

Excavations for utility installations or shallow foundations are not expected to encounter shallow groundwater near-surface. The contractor is responsible for employing appropriate dewatering methods to control seepage and facilitate construction, if needed. In our experience, dewatering of excavations with perched water in granular soils above the water table can be accomplished with typical sump pits and pumps.

As a minimum, excavations should be performed in accordance with OSHA 29 CFR, Part 1926, Subpart P, "Excavations" and its appendices as well as other applicable codes, and in accordance with any applicable local, state, and federal safety regulations. The contractor should be aware that slope height, slope inclination, and excavation depth should in no instance exceed those specified by these safety regulations. Flatter slopes than those dictated by these regulations may be required depending upon the soil conditions encountered and other external factors. These regulations are strictly enforced

and if they are not followed, the owner, the contractor, and/or earthwork and utility subcontractor could be liable and subject to substantial penalties. Under no circumstances should the information provided in this report be interpreted to mean that Terracon is responsible for construction site safety or the contractor's activities. Construction site safety is the sole responsibility of the contractor who shall also be solely responsible for the means, methods, and sequencing of the construction operations.

Construction Observation and Testing

The geotechnical engineer should be retained during the construction phase of the project to observe earthwork and to perform necessary tests and observations during subgrade preparation, proof-rolling, placement and compaction of controlled compacted fills, backfilling of excavations to the completed subgrade.

The earthwork efforts should be monitored under the direction of the Geotechnical Engineer. Field density tests should be conducted during placement and compaction of compacted fill. The testing frequency should be in accordance with the following table.

Fill Placement Area	Recommended Testing Frequency (ASTM D6938)
Shallow Foundations and Slabs	A minimum of 1 test per slab per vertical foot of fill placed
Solar Array Field	Each vertical foot of fill placed should be tested at a frequency of 1 test per every 20,000 square feet of fill placed, or a minimum of 1 test per solar array block quadrant per vertical foot of fill placed
Access Roads	A minimum of 1 test per 500 linear feet for each vertical lift of base, subbase, subgrade, or structural fill
Berms	A minimum of 1 test per 300 linear feet of berm per foot of fill
Utility Trench Backfill	Each vertical foot of fill placed should be tested at an interval of every 1,000 linear feet of fill placed for load bearing areas, and every 5,000 linear feet for non-load bearing areas

The Geotechnical Engineer may require additional tests as considered necessary to check on the uniformity of compaction. No additional layers of fill should be placed until the field density test results indicate that the specified density has been obtained.

In addition to the documentation of the essential parameters necessary for construction, the continuation of the Geotechnical Engineer into the construction phase of the project

provides the continuity to maintain the Geotechnical Engineer's evaluation of subsurface conditions, including assessing variations and associated design changes.

Gravel-Surfaced Drives and Parking

General Comments

Roadway designs are provided for the traffic conditions and pavement life conditions as noted in the **Project Description** and in the following sections of this report. A critical aspect of pavement performance is site preparation. Roadway sections noted in this section are contingent upon the site being adequately prepared. Additionally, our recommendations are based on *Chapter 4 Low-Volume Road Design* found in AASHTO 1993.

Roadway Subgrades

Laboratory testing conducted in the referenced Terracon Addendum 1 for the site dated June 5, 2025 resulted in a California Bearing Ratio (CBR) value of 20 corresponding to 95 percent of the maximum dry density as determined using the Modified Proctor method (ASTM D1557). To account for variability across the site, a CBR of 10 was assumed for the PV fields.

Design Parameters

We understand unpaved access roads are planned throughout the site. The unpaved road sections for post-construction use have been developed under the following assumptions:

Aggregate Roadway Design Parameters		
Parameter	**Design Value**	**Comments**
Traffic Loading	Array Area = 2,500 ESALs	Assumed
Design Life	30 years	Assumed
Design CBR	10 (based on previous experience and testing in similar geology)	Assumed
Resilient Modulus	11,800 psi (all-weather)	Based on CBR of 10
Aggregate Base Elastic Modulus	36,000 psi	Assumed

Aggregate Roadway Design Parameters		
Parameter	**Design Value**	**Comments**
Allowable Rut Depth	2.0 inches	Assumed
Design Serviceability Loss	2.0 inches	Assumed
Vehicle Tire Pressure	80 psi	Assumed

1. ESAL = 18 kips Equivalent Single Axle Load

Access Road Sections

As a minimum, we recommend the following options for unpaved access roads:

Typical Unpaved Road Section – Post Construction Traffic	
Base Course Thickness (inches)	**Traffic (ESALs)**
4[1, 2]	2,500

1. Minimum section thickness is anticipated to support fire trucks and pick-up trucks associated with on-going maintenance. Trucks containing heavy equipment may require localized repairs.
2. Base materials shall consist of Class II Base meeting requirements of the Caltrans Standard Specifications.

Roadway section should be constructed over a minimum of 12 inches of scarified, moisture conditioned, and compacted native soils to 95% of the maximum dry density using ASTM D1557. The recommended thicknesses should be measured after full compaction. The width of the roadway should extend a minimum distance of 1 foot on each side of the desired surface width.

Aggregate materials should conform to the specifications of Class II aggregate base in accordance with the requirements and specifications of the State of California Department of Transportation (Caltrans), or other approved local governing specifications.

Positive drainage should be provided during construction and maintained throughout the life of the roadways. Proposed roadway design should maintain the integrity of the road and eliminate ponding.

Roadway Design and Construction Considerations

Regardless of the design, un-surfaced roadways will display varying levels of wear and deterioration. We recommend an implementation of a site inspection program at a frequency of at least once per year to verify the adequacy of the roadways. Preventative measures should be applied as needed for erosion control and re-grading. An initial site inspection should be completed approximately three months following construction.

Surfacing materials should not be placed when the surface is wet. Surface drainage should be provided away from the edge of roadways to reduce lateral moisture transmission into the subgrade.

If rut depths become excessive as construction work progresses, re-grading and re-compaction should be performed as necessary. Care should be taken to reduce or eliminate trafficking of the unpaved access road when the subgrade is wet as this will result in accelerated rutting conditions. Scarification, moisture treatment as necessary, and re-compaction of the roadways will likely be necessary as the roadways deteriorate.

Materials and construction of roadways for the project should be in accordance with the requirements and specifications of the California Department of Transportation or the applicable local governing body.

General Comments

Our analysis and opinions are based upon our understanding of the project, the geotechnical conditions in the area, and the data obtained from our site exploration. Variations will occur between exploration point locations or due to the modifying effects of construction or weather. The nature and extent of such variations may not become evident until during or after construction. Terracon should be retained as the Geotechnical Engineer, where noted in this report, to provide observation and testing services during pertinent construction phases. If variations appear, we can provide further evaluation and supplemental recommendations. If variations are noted in the absence of our observation and testing services on-site, we should be immediately notified so that we can provide evaluation and supplemental recommendations.

Our Scope of Services does not include either specifically or by implication any environmental or biological (e.g., mold, fungi, bacteria) assessment of the site or identification or prevention of pollutants, hazardous materials or conditions. If the owner is concerned about the potential for such contamination or pollution, other studies should be undertaken.

Our services and any correspondence are intended for the sole benefit and exclusive use of our client for specific application to the project discussed and are accomplished in

accordance with generally accepted geotechnical engineering practices with no third-party beneficiaries intended. Any third-party access to services or correspondence is solely for information purposes to support the services provided by Terracon to our client. Reliance upon the services and any work product is limited to our client and is not intended for third parties. Any use or reliance of the provided information by third parties is done solely at their own risk. No warranties, either express or implied, are intended or made.

Site characteristics as provided are for design purposes and not to estimate excavation cost. Any use of our report in that regard is done at the sole risk of the excavating cost estimator as there may be variations on the site that are not apparent in the data that could significantly effect excavation cost. Any parties charged with estimating excavation costs should seek their own site characterization for specific purposes to obtain the specific level of detail necessary for costing. Site safety and cost estimating including excavation support and dewatering requirements/design are the responsibility of others. Construction and site development have the potential to affect adjacent properties. Such impacts can include damages due to vibration, modification of groundwater/surface water flow during construction, foundation movement due to undermining or subsidence from excavation, as well as noise or air quality concerns. Evaluation of these items on nearby properties are commonly associated with contractor means and methods and are not addressed in this report. The owner and contractor should consider a preconstruction/precondition survey of surrounding development. If changes in the nature, design, or location of the project are planned, our conclusions and recommendations shall not be considered valid unless we review the changes and either verify or modify our conclusions in writing.

Attachments

Field Exploration Results

Contents:

Site Location
Site Layout
Exploration Location Plans (6 pages)
Exploration and Testing Procedures (2 pages)
General Notes
Unified Soil Classification System
Boring Logs (74 pages)
Test Pit Logs (36 pages)
Test Pit Photo Log (6 pages)

Site Location

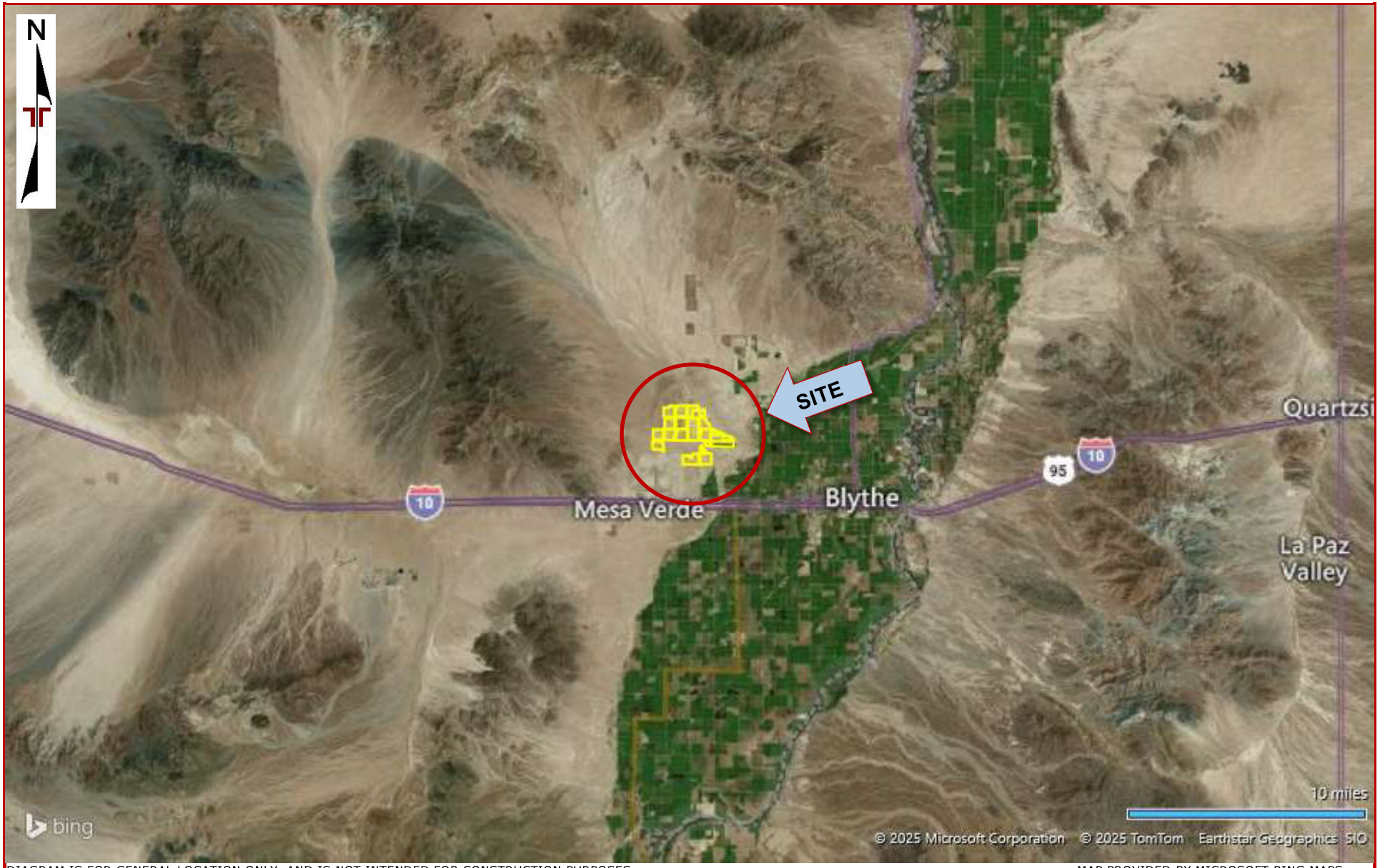


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Site Overview

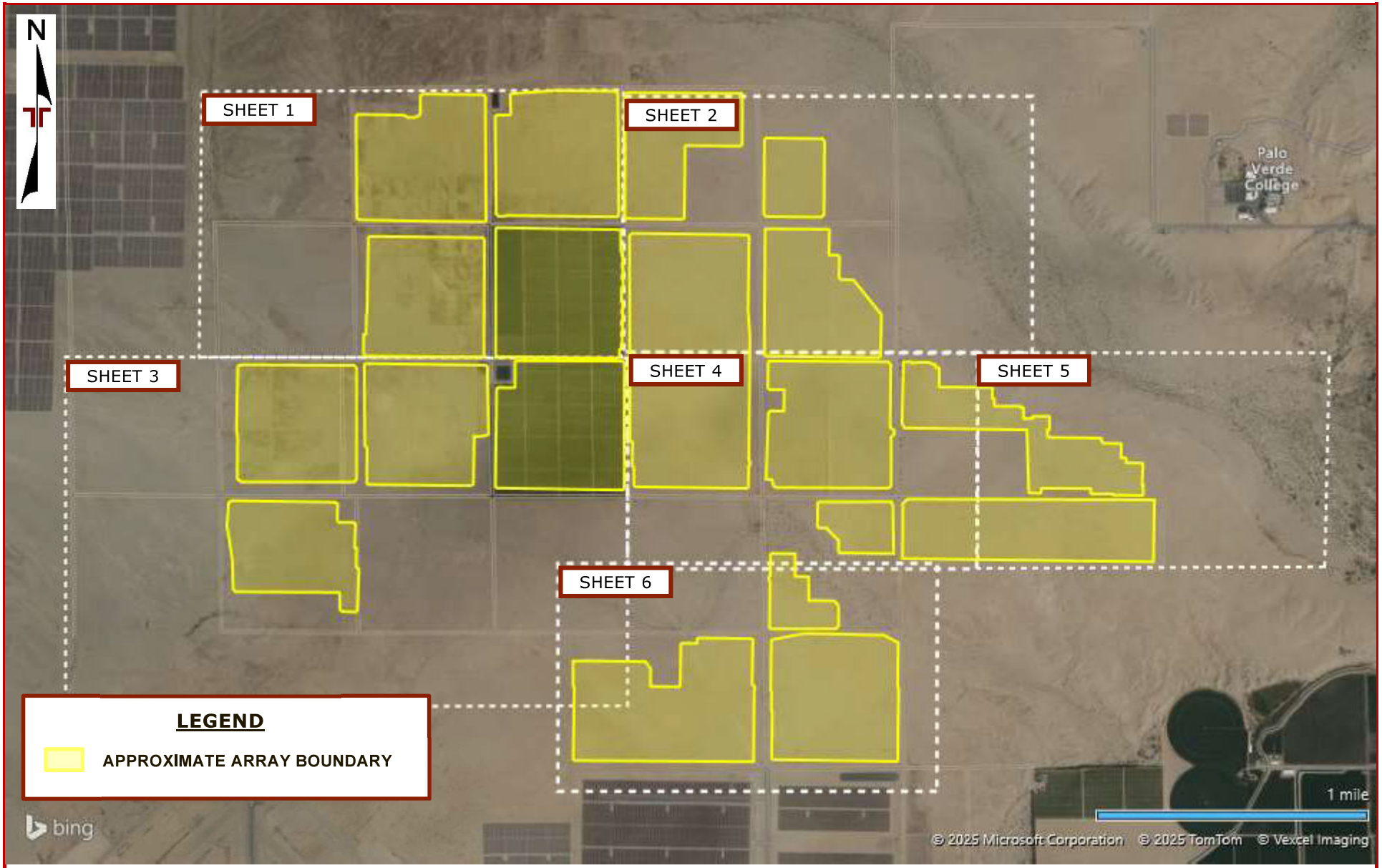


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MAP PROVIDED BY MICROSOFT BING MAPS

Exploration Location Plan (Sheet 1)

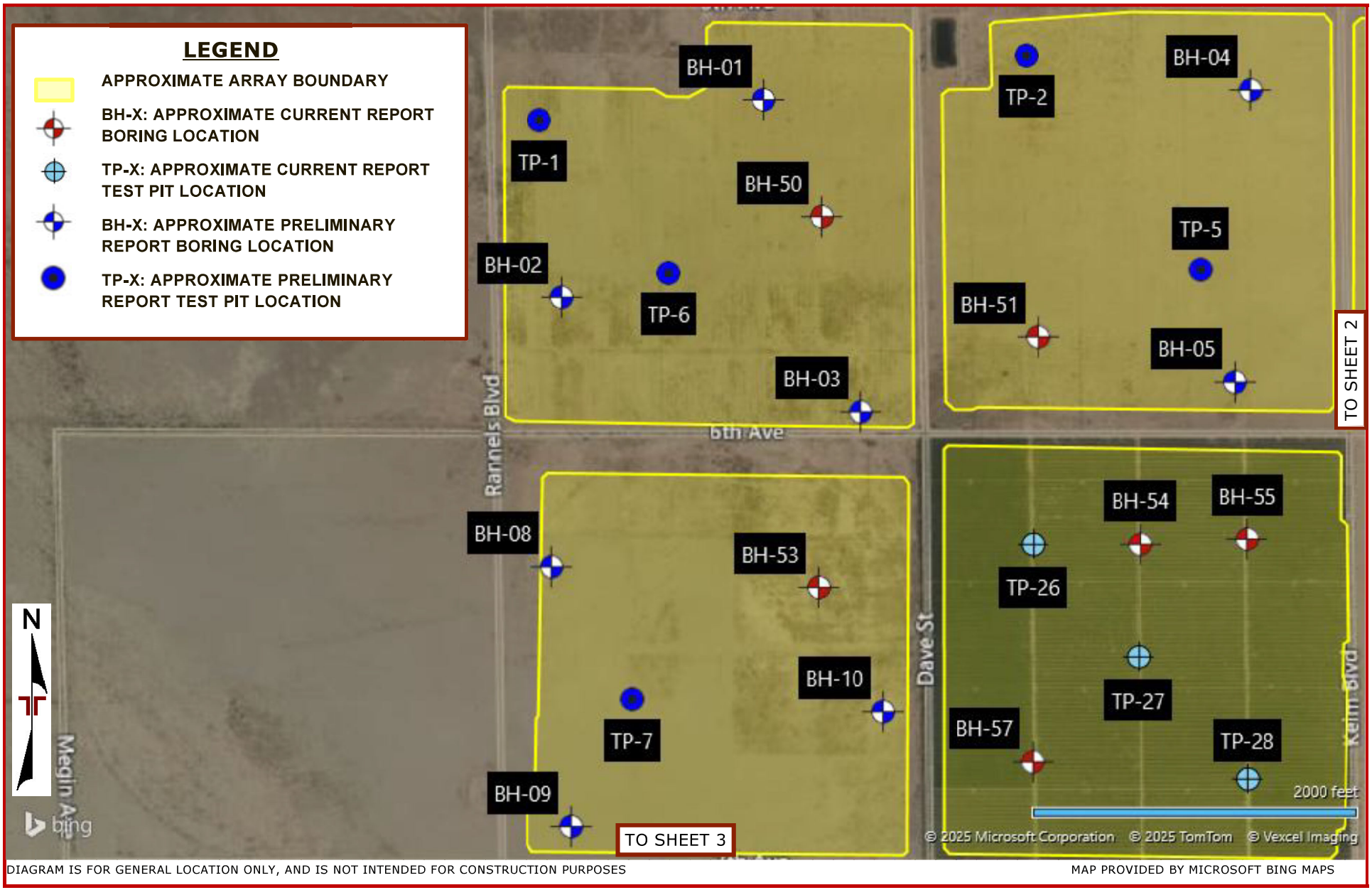


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MAP PROVIDED BY MICROSOFT BING MAPS

Exploration Location Plan (Sheet 2)

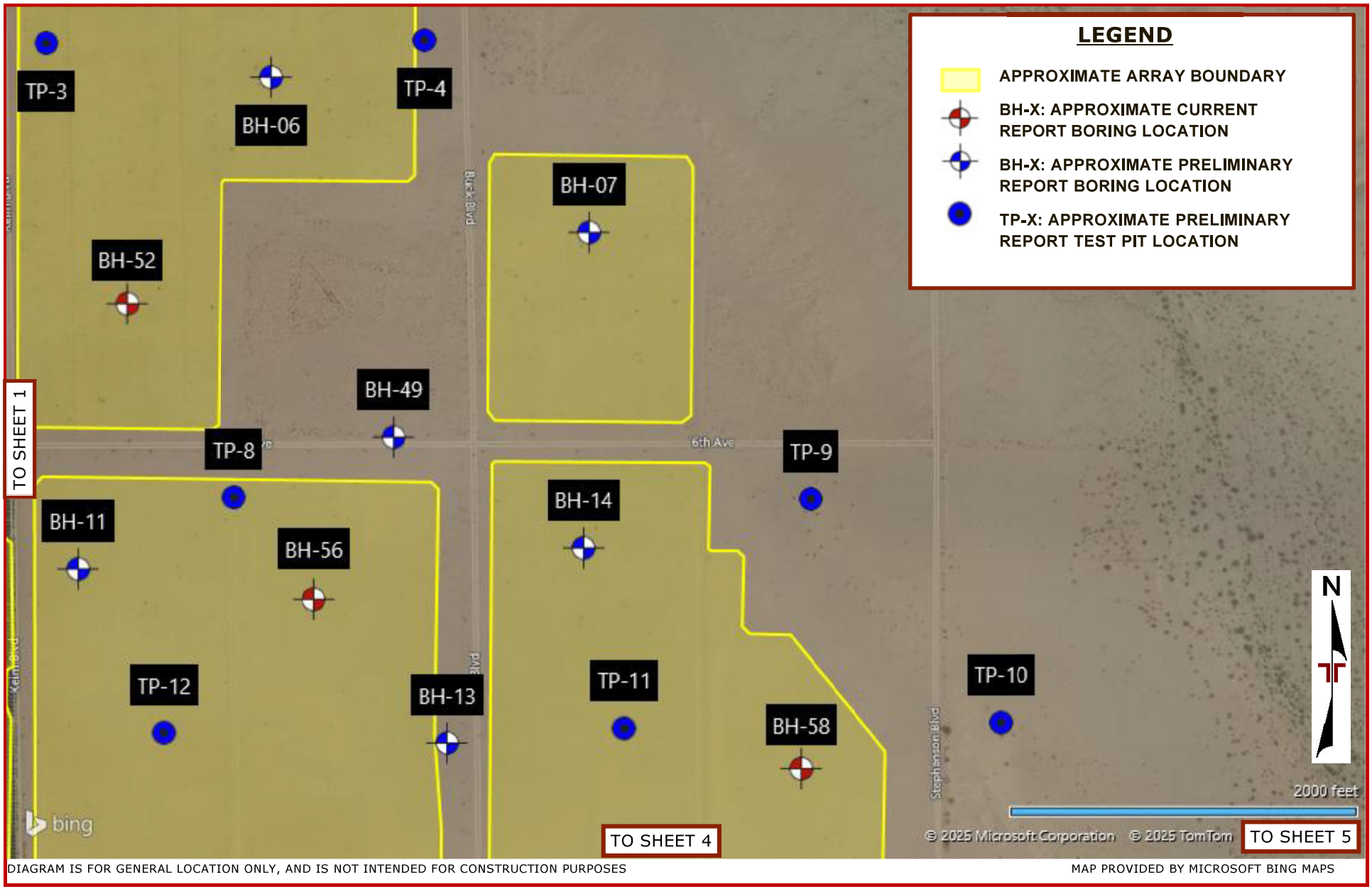


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MAP PROVIDED BY MICROSOFT BING MAPS

Exploration Location Plan (Sheet 3)

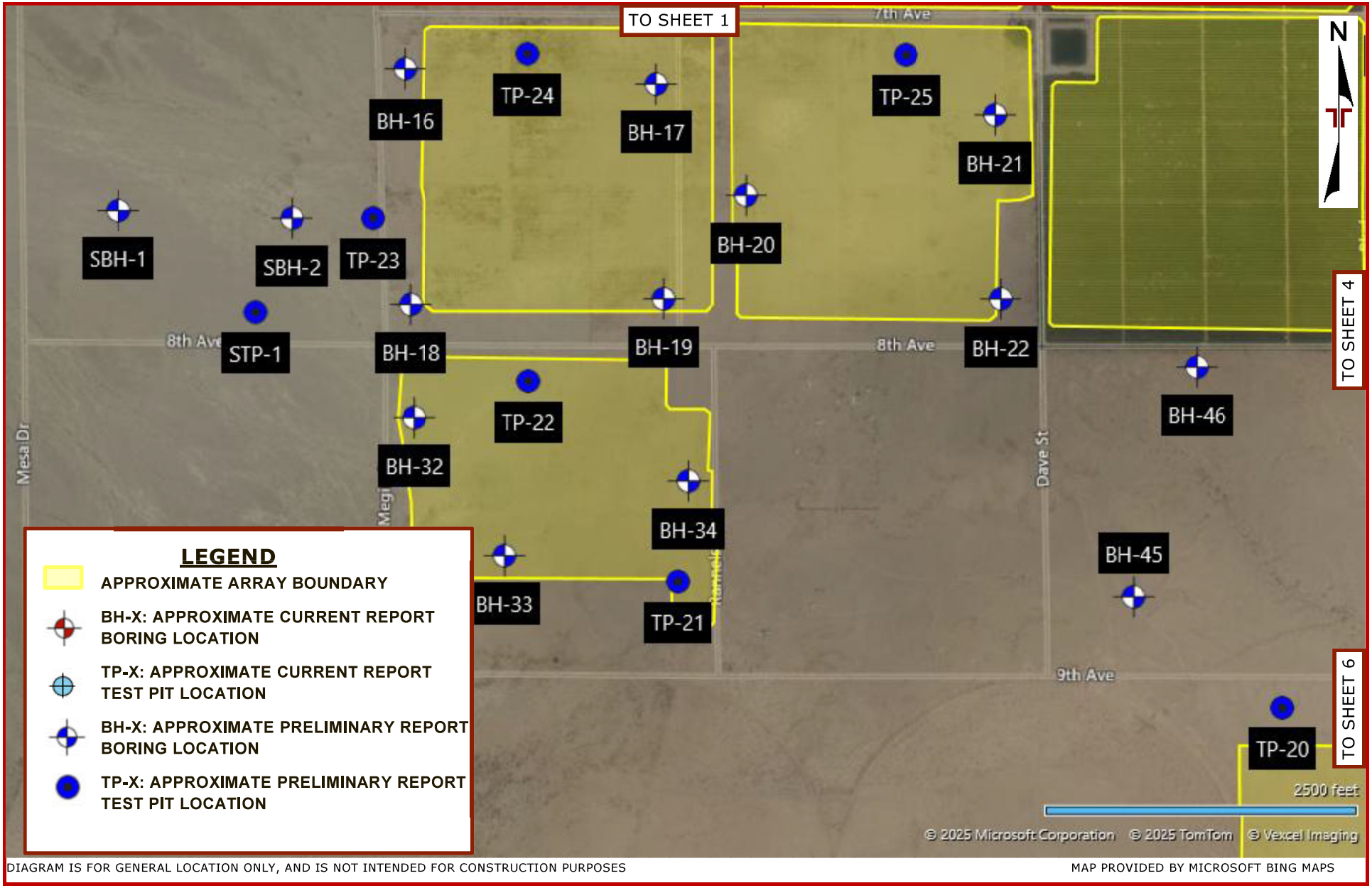


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MAP PROVIDED BY MICROSOFT BING MAPS

Exploration Location Plan (Sheet 4)

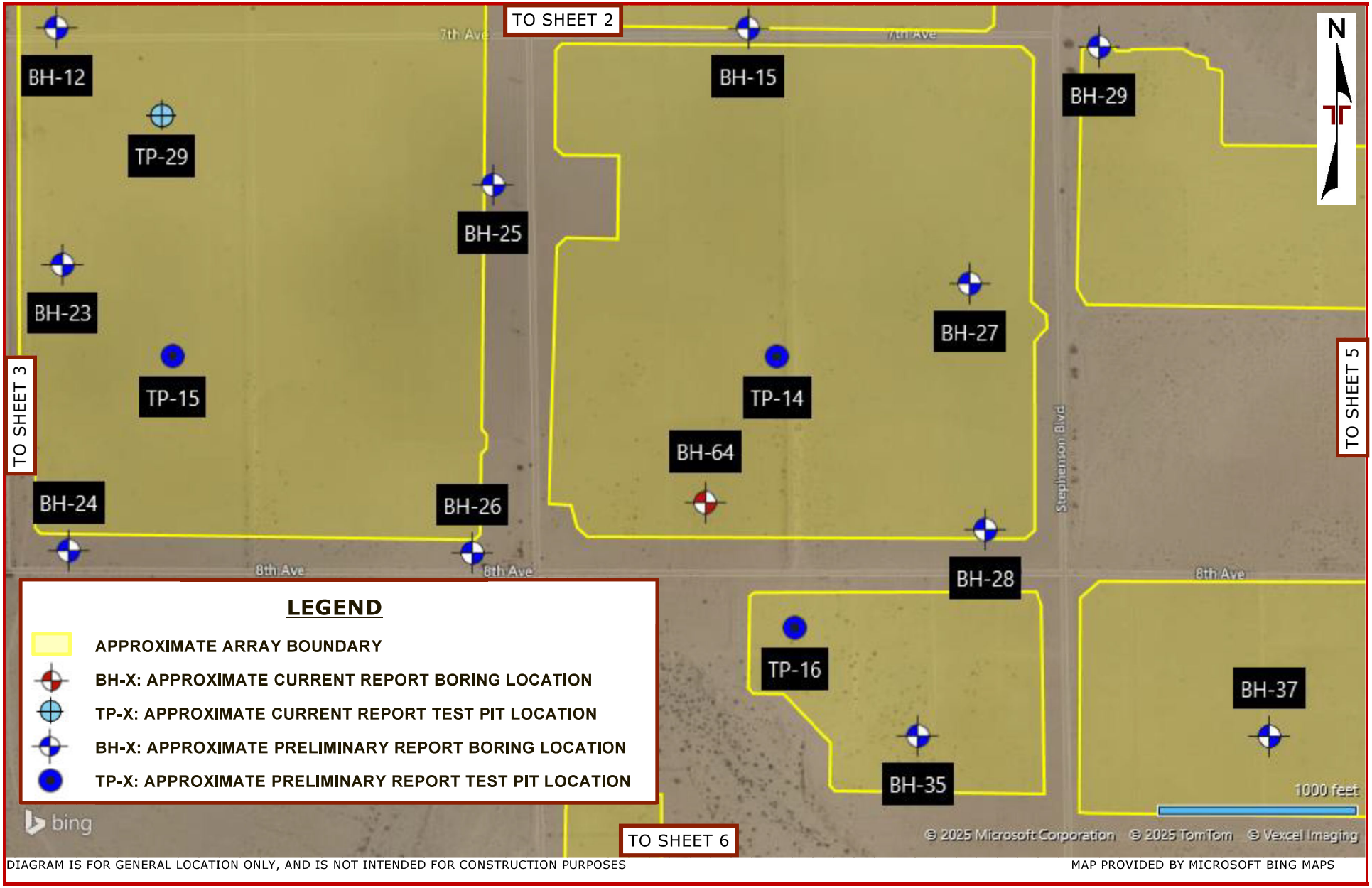


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Exploration Location Plan (Sheet 5)

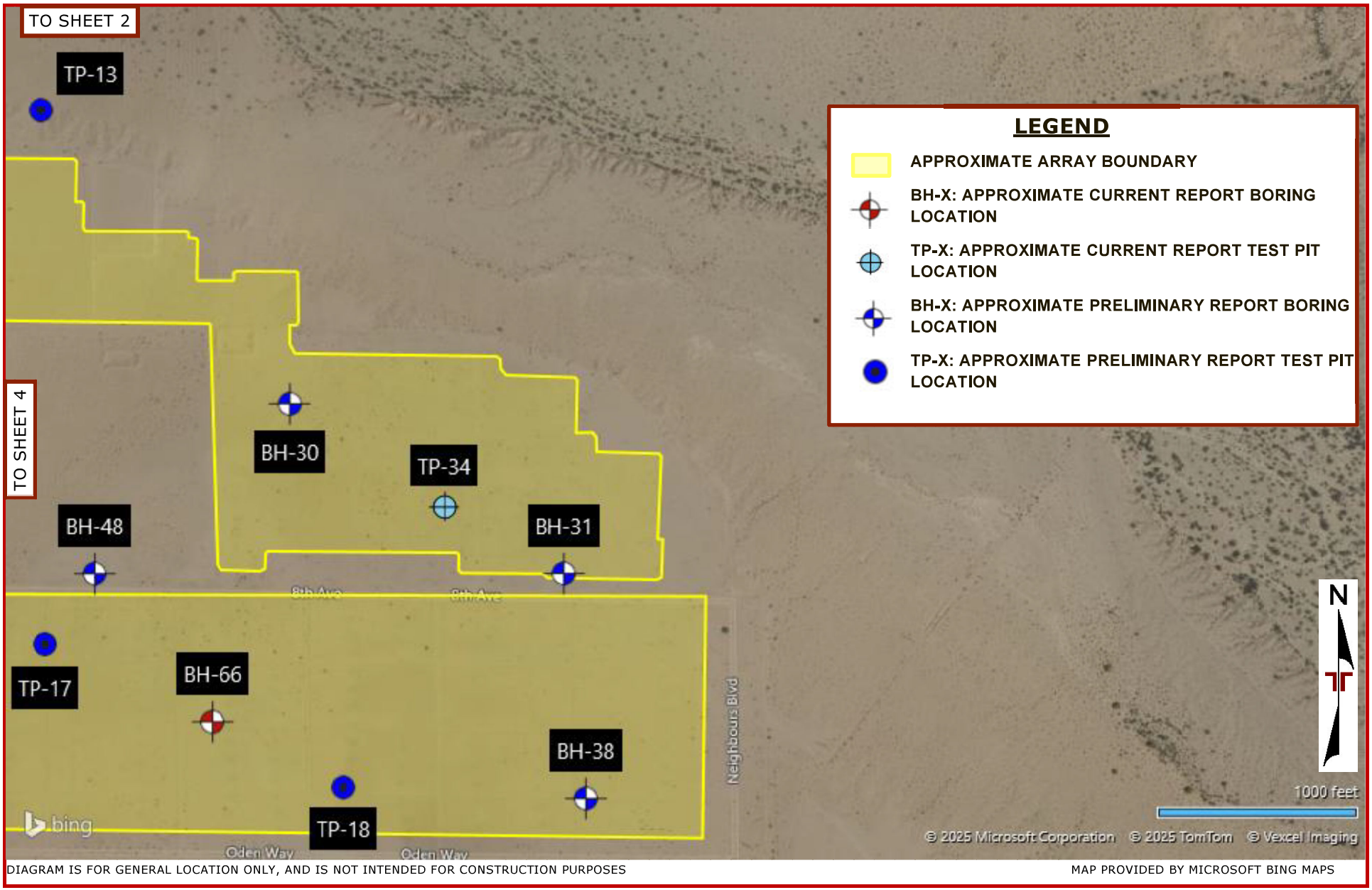


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Exploration Location Plan (Sheet 6)

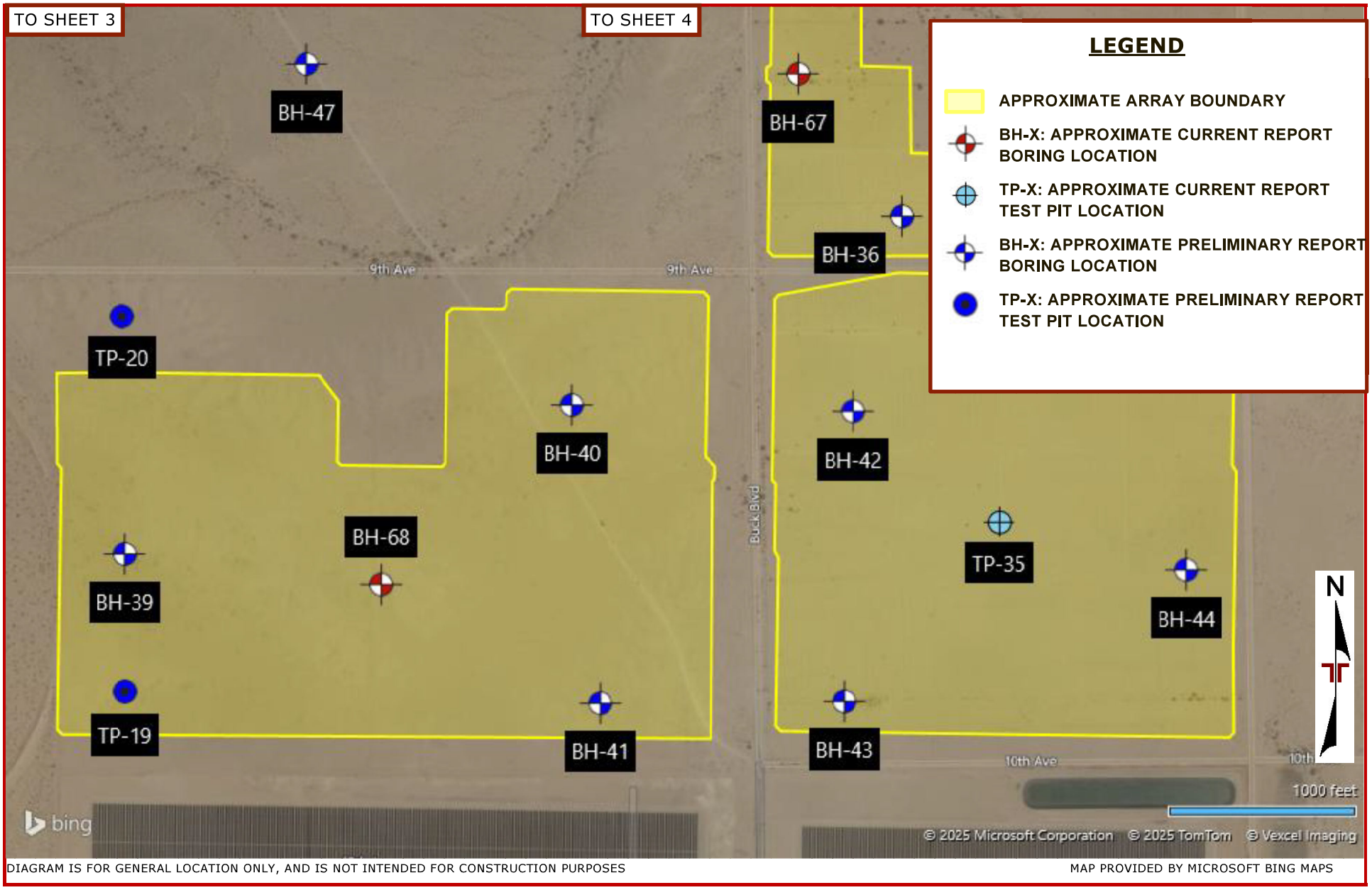


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Exploration and Testing Procedures

Field Exploration

The following table provides a summary of our geotechnical exploration completed in the array area.

Number of Explorations	Type of Exploration	Depth or Description
68	Soil Boring	21½ feet
35	Test Pit	10 feet

The following table provides a summary of our geotechnical exploration completed near the substation area.

Number of Explorations	Type of Exploration	Depth or Description
2	Soil Boring	51½
1	Test Pit	10

Boring Layout and Elevations: Boring layout was provided by NEER and adjusted by Terracon based on accessibility. Borings were located using handheld GPS equipment (estimated horizontal accuracy of about ±10 to 15 feet) and referencing existing site features. Approximate ground surface elevations were estimated using Google Earth. If elevations and a more precise boring layout are desired, we recommend borings be surveyed.

Subsurface Exploration Procedures: We advanced the borings with a track-mounted, drill rig using continuous flight augers (hollow stem). Four samples were generally obtained in the upper 10 feet of each boring and at intervals of 5 feet thereafter. Test samples were collected during drilling in general accordance with the appropriate ASTM methods using Standard Penetration Testing (SPT) and sampling using either standard split-spoon or Modified California samplers. A sampling spoon was driven into the ground by a 140-pound automatic hammer falling a distance of 30 inches. The number of blows required to advance the sampling spoon the last 12 inches of a normal 18-inch penetration was recorded as the Standard Penetration Test (SPT) resistance value, also referred to as N-values. The N-values are indicated on the boring logs at the test depths.

We also observed the boreholes while drilling and at the completion of drilling for the presence of groundwater. The groundwater levels are shown on the attached boring logs.

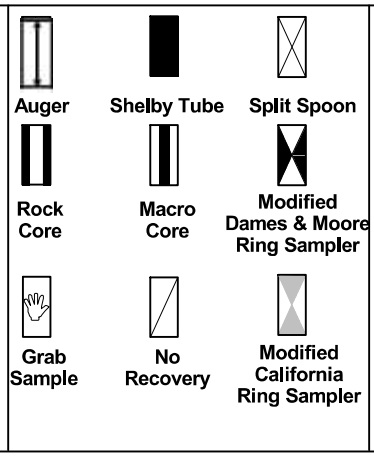
The sampling depths, penetration distances, and other sampling information was recorded on the field boring logs. The samples were placed in appropriate containers and taken to our soil laboratory for testing and classification by a Geotechnical Engineer. Our exploration team prepared field boring logs as part of the drilling operations. These field logs included visual classifications of the materials observed during drilling and our interpretation of the subsurface conditions between samples. Final boring logs were prepared from the field logs. The final boring logs represent the Geotechnical Engineer's interpretation of the field logs and include modifications based on observations and tests of the samples in our laboratory.

Test Pits: Test pits were performed using a metal track-mounted excavator and observed by a Terracon field engineer. Samples and intact sidewalls of the pits were classified in the field by the field engineer with photos at each location. In addition, we observed and recorded groundwater levels (if present) during excavations. Test pits were backfilled with the excavated materials upon completion.

GENERAL NOTES

DESCRIPTION OF SYMBOLS AND ABBREVIATIONS

SAMPLING



WATER LEVEL

- Water Initially Encountered
- Water Level After a Specified Period of Time
- Water Level After a Specified Period of Time

Water levels indicated on the soil boring logs are the levels measured in the borehole at the times indicated. Groundwater level variations will occur over time. In low permeability soils, accurate determination of groundwater levels is not possible with short term water level observations.

FIELD TESTS

(HP)	Hand Penetrometer
(T)	Torvane
(b/f)	Standard Penetration Test (blows per foot)
N	N value
(PID)	Photo-Ionization Detector
(OVA)	Organic Vapor Analyzer
(WOH)	Weight of Hammer

DESCRIPTIVE SOIL CLASSIFICATION

Soil classification is based on the Unified Soil Classification System. Coarse Grained Soils have more than 50% of their dry weight retained on a #200 sieve; their principal descriptors are: boulders, cobbles, gravel or sand. Fine Grained Soils have less than 50% of their dry weight retained on a #200 sieve; they are principally described as clays if they are plastic, and silts if they are slightly plastic or non-plastic. Major constituents may be added as modifiers and minor constituents may be added according to the relative proportions based on grain size. In addition to gradation, coarse-grained soils are defined on the basis of their in-place relative density and fine-grained soils on the basis of their consistency.

LOCATION AND ELEVATION NOTES

Unless otherwise noted, Latitude and Longitude are approximately determined using a hand-held GPS device. The accuracy of such devices is variable. Surface elevation data annotated with +/- indicates that no actual topographical survey was conducted to confirm the surface elevation. Instead, the surface elevation was approximately determined from topographic maps of the area.

STRENGTH TERMS

RELATIVE DENSITY OF COARSE-GRAINED SOILS (More than 50% retained on No. 200 sieve.) Density determined by Standard Penetration Resistance Includes gravels, sands and silts.			CONSISTENCY OF FINE-GRAINED SOILS (50% or more passing the No. 200 sieve.) Consistency determined by laboratory shear strength testing, field visual-manual procedures or standard penetration resistance			
Descriptive Term (Density)	**Standard Penetration or N-Value Blows/Ft.**	**Ring Sampler Blows/Ft.**	**Descriptive Term (Consistency)**	**Unconfined Compressive Strength, Qu, psf**	**Standard Penetration or N-Value Blows/Ft.**	**Ring Sampler Blows/Ft.**
Very Loose	0 - 3	0 - 6	Very Soft	less than 500	0 - 1	< 3
Loose	4 - 9	7 - 18	Soft	500 to 1,000	2 - 4	3 - 4
Medium Dense	10 - 29	19 - 58	Medium-Stiff	1,000 to 2,000	4 - 8	5 - 9
Dense	30 - 50	59 - 98	Stiff	2,000 to 4,000	8 - 15	10 - 18
Very Dense	> 50	≥ 99	Very Stiff	4,000 to 8,000	15 - 30	19 - 42
			Hard	> 8,000	> 30	> 42

RELATIVE PROPORTIONS OF SAND AND GRAVEL

Descriptive Term(s) of other constituents	Percent of Dry Weight
Trace	< 15
With	15 - 29
Modifier	> 30

GRAIN SIZE TERMINOLOGY

Major Component of Sample	Particle Size
Boulders	Over 12 in. (300 mm)
Cobbles	12 in. to 3 in. (300mm to 75mm)
Gravel	3 in. to #4 sieve (75mm to 4.75 mm)
Sand	#4 to #200 sieve (4.75mm to 0.075mm
Silt or Clay	Passing #200 sieve (0.075mm)

RELATIVE PROPORTIONS OF FINES

Descriptive Term(s) of other constituents	Percent of Dry Weight
Trace	< 5
With	5 - 12
Modifier	> 12

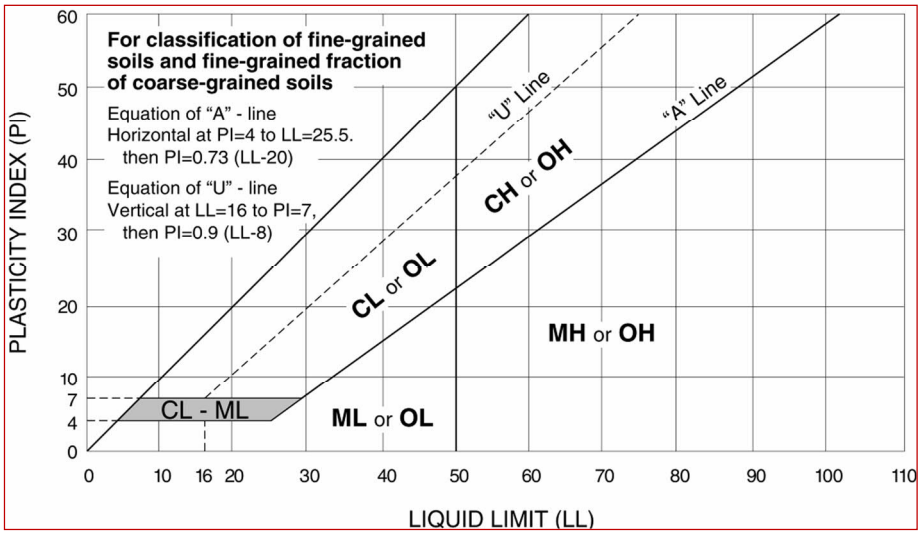
PLASTICITY DESCRIPTION

Term	Plasticity Index
Non-plastic	0
Low	1 - 10
Medium	11 - 30
High	> 30

Unified Soil Classification System

Criteria for Assigning Group Symbols and Group Names Using Laboratory Tests [A]				Soil Classification	
				Group Symbol	Group Name [B]
Coarse-Grained Soils: More than 50% retained on No. 4 sieve	**Gravels:** More than 50% of coarse fraction retained on No. 4 sieve	**Clean Gravels:** Less than 5% fines [C]	Cu≥4 and 1≤Cc≤3 [E]	GW	Well-graded gravel [F]
			Cu<4 and/or [Cc<1 or Cc>3.0] [E]	GP	Poorly graded gravel [F]
		Gravels with Fines: More than 12% fines [C]	Fines classify as ML or MH	GM	Silty gravel [F, G, H]
			Fines classify as CL or CH	GC	Clayey gravel [F, G, H]
	Sands: 50% or more of coarse fraction passes No. 4 sieve	**Clean Sands:** Less than 5% fines [D]	Cu≥6 and 1≤Cc≤3 [E]	SW	Well-graded sand [I]
			Cu<6 and/or [Cc<1 or Cc>3.0] [E]	SP	Poorly graded sand [I]
		Sands with Fines: More than 12% fines [D]	Fines classify as ML or MH	SM	Silty sand [G, H, I]
			Fines classify as CL or CH	SC	Clayey sand [G, H, I]
Fine-Grained Soils: 50% or more passes the No. 200 sieve	**Silts and Clays:** Liquid limit less than 50	**Inorganic:**	PI > 7 and plots above "A" line [J]	CL	Lean clay [K, L, M]
			PI < 4 or plots below "A" line [J]	ML	Silt [K, L, M]
		Organic:	$\frac{LL\ oven\ dried}{LL\ not\ dried} < 0.75$	OL	Organic clay [K, L, M, N]
					Organic silt [K, L, M, O]
	Silts and Clays: Liquid limit 50 or more	**Inorganic:**	PI plots on or above "A" line	CH	Fat clay [K, L, M]
			PI plots below "A" line	MH	Elastic silt [K, L, M]
		Organic:	$\frac{LL\ oven\ dried}{LL\ not\ dried} < 0.75$	OH	Organic clay [K, L, M, P]
					Organic silt [K, L, M, Q]
Highly organic soils:	Primarily organic matter, dark in color, and organic odor			PT	Peat

[A] Based on the material passing the 3-inch (75-mm) sieve.
[B] If field sample contained cobbles or boulders, or both, add "with cobbles or boulders, or both" to group name.
[C] Gravels with 5 to 12% fines require dual symbols: GW-GM well-graded gravel with silt, GW-GC well-graded gravel with clay, GP-GM poorly graded gravel with silt, GP-GC poorly graded gravel with clay.
[D] Sands with 5 to 12% fines require dual symbols: SW-SM well-graded sand with silt, SW-SC well-graded sand with clay, SP-SM poorly graded sand with silt, SP-SC poorly graded sand with clay.
[E] $Cu = D_{60}/D_{10}$ $\quad Cc = \frac{(D_{30})^2}{D_{10} \times D_{60}}$
[F] If soil contains ≥ 15% sand, add "with sand" to group name.
[G] If fines classify as CL-ML, use dual symbol GC-GM, or SC-SM.
[H] If fines are organic, add "with organic fines" to group name.
[I] If soil contains ≥ 15% gravel, add "with gravel" to group name.
[J] If Atterberg limits plot in shaded area, soil is a CL-ML, silty clay.
[K] If soil contains 15 to 29% plus No. 200, add "with sand" or "with gravel," whichever is predominant.
[L] If soil contains ≥ 30% plus No. 200 predominantly sand, add "sandy" to group name.
[M] If soil contains ≥ 30% plus No. 200, predominantly gravel, add "gravelly" to group name.
[N] PI ≥ 4 and plots on or above "A" line.
[O] PI < 4 or plots below "A" line.
[P] PI plots on or above "A" line.
[Q] PI plots below "A" line.



Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-01

Graphic Log	Location: See Exploration Plan Latitude: 33.6665° Longitude: -114.7049° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	SILTY SAND (SM), trace gravel, brown								
	dense				21-29-35	2.0 2.1	101		29
	brownish tan, medium dense	5			5-8-9 N=17				
					10-22-29	0.9	96		
	10.0 **POORLY GRADED SAND (SP)**, tan, medium dense	10			5-9-11 N=20				
	15.0 **SILTY SAND (SM)**, trace gravel, tannish brown, dense	15			10-18-19 N=37				
		20			9-15-20 N=35				
	21.5 ***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-26-2023

Boring Completed
10-26-2023

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-02

Graphic Log	Location: See Exploration Plan Latitude: 33.6631° Longitude: -114.7090° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	CLAYEY SAND (SC), trace gravel, brown								
	medium dense				3-8-15 N=23				
	very dense	5			12-45-50/2"	2.9	102		28
	tannish brown, medium dense				4-9-13 N=22				
	10.0 **POORLY GRADED SAND (SP)**, trace gravel, tan, dense	10			14-34-45	0.3	101		
	medium dense	15			6-12-14 N=26				
	21.5	20			6-5-13 N=18				
	Boring Terminated at 21.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-26-2023

Boring Completed
10-26-2023

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-03

Location: See Exploration Plan

Latitude: 33.6612° Longitude: -114.7029°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
SILTY SAND (SM), trace gravel, brown								
dense				27-40-43	4.7 1.6	100		36
	5			8-13-17 N=30				
				19-29-50/6"	1.3	100		
10.0 **POORLY GRADED SAND (SP)**, trace clay, tan, medium dense	10			8-13-15 N=28				
tannish brown, very dense	15			32-40-36 N=76				4
	20			34-39-41 N=80				
21.5 ***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-26-2023

Boring Completed
10-26-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-04

Location: See Exploration Plan

Latitude: 33.6666° Longitude: -114.6950°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
	SILTY SAND (SM), trace gravel, brown medium dense			4-11-13 N=24	1.6			21
5.0	**POORLY GRADED SAND (SP)**, trace clay and gravel, tan to gray, dense			17-39-39	0.4	98		
5				10-15-15 N=30				
10				12-28-36	0.8	95		
15	medium dense			6-11-17 N=28				
20	dense			8-14-17 N=31				
21.5	***Boring Terminated at 21.5 Feet***							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-26-2023

Boring Completed
10-26-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-05

Location: See Exploration Plan

Latitude: 33.6617° Longitude: -114.6953°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
								23
				7-8-8	4.7	96		
5.0								
				5-7-9 N=16				
				9-26-30	0.4	96		
10.0								
				7-15-18 N=33				
				9-15-18 N=33				
				13-19-22 N=41				
21.5								

CLAYEY SAND (SC), trace gravel, brown

loose

POORLY GRADED SAND (SP), trace clay and gravel, tannish brown, medium dense

SILTY SAND (SM), tannish gray, dense

Boring Terminated at 21.5 Feet

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-26-2023

Boring Completed
10-26-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
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Ste 350
Laguna Hills, CA

Boring Log No. BH-06

Location: See Exploration Plan

Latitude: 33.6666° Longitude: -114.6881°

Graphic Log	Depth (Ft.)	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	0	**SILTY CLAYEY SAND (SC-SM)**, trace gravel, brown								
		medium dense				4-8-11 N=19	2.7			20
	5		5			11-24-25	1.1	97		
	7.5	**POORLY GRADED SAND (SP)**, trace gravel, tannish brown, medium dense				6-9-13 N=22				
		dense	10			16-30-50/6"	0.6	99		
		tan, medium dense	15			7-12-12 N=24				
		tannish gray, very dense	20			14-30-49 N=79				
	21.5	***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-26-2023

Boring Completed
10-26-2023

Grace Solar
Blythe | Riverside County, CA
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Terracon
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Ste 350
Laguna Hills, CA

Boring Log No. BH-07

Location: See Exploration Plan

Latitude: 33.6642° Longitude: -114.6821°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
CLAYEY SAND (SC), trace gravel, brown tan, medium dense	0							
				10-16-22	0.3	96		
	5			7-10-12 N=22				
				5-15-27	2.3	102		
10.0 **POORLY GRADED SAND (SP)**, trace clay, tan, medium dense	10			5-9-10 N=19				
brown	15			6-11-15 N=26				
tan	20			5-10-14 N=24				
21.5 ***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-24-2023

Boring Completed
10-24-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
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Laguna Hills, CA

Boring Log No. BH-08

Location: See Exploration Plan

Latitude: 33.6586° Longitude: -114.7092°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	CLAYEY SAND (SC), trace gravel, brown dense			12-16-18 N=34				31
	7.5			27-30-34	2.6	95		
	POORLY GRADED SAND (SP), trace clay and gravel, tan, medium dense			5-8-10 N=18				
				12-22-26	1.6	98		
	20.0			7-12-17 N=29				
	SILTY SAND (SM), tan, dense			10-19-23 N=42				
	21.5							
	Boring Terminated at 21.5 Feet							

5
10
15
20

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-26-2023

Boring Completed
10-26-2023

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Grace Solar
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Terracon
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Boring Log No. BH-09

Graphic Log	Location: See Exploration Plan Latitude: 33.6542° Longitude: -114.7088° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	SILTY SAND (SM), trace gravel, brown								
	dense				12-30-40	3.2	103		26
	tannish brown, medium dense	5			8-11-14 N=25				
					10-26-32	1.1	98		
	tan	10			6-10-11 N=21				
	15.0 **POORLY GRADED SAND (SP)**, trace gravel, tannish gray, medium dense	15			5-8-11 N=19				
		20			8-12-17 N=29				
	21.5 ***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-26-2023

Boring Completed
10-26-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
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Laguna Hills, CA

Boring Log No. BH-10

Graphic Log	Location: See Exploration Plan Latitude: 33.6561° Longitude: -114.7024° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	CLAYEY SAND (SC), trace gravel, brown loose				2-3-3 N=6				
	dense	5			10-14-17	5.6	95		
	7.5 **POORLY GRADED SAND (SP)**, trace gravel, tannish gray, medium dense				5-10-16 N=26				
	tannish gray	10			5-20-24	0.8	96		
	15.0 **SILTY SAND (SM)**, brown, medium dense	15			6-8-10 N=18				15
	tannish gray, dense	20			10-16-17 N=33				
	21.5 ***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-26-2023

Boring Completed
10-26-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-11

Location: See Exploration Plan

Latitude: 33.6589° Longitude: -114.6918°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
	SILTY SAND (SM), brown							
	medium dense			17-21-25	1.0 0.6	97		20
5				5-7-9 N=16				
	brown			9-13-20	9.0	85		
10				5-8-11 N=19				
	tannish brown			6-10-12 N=22				
15								
	20.0							
20	**WELL GRADED SAND (SW)**, tannish brown, medium dense			6-9-11 N=20				
	21.5							
	Boring Terminated at 21.5 Feet							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-18-2023

Boring Completed
10-18-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-12

Graphic Log	Location: See Exploration Plan Latitude: 33.6537° Longitude: -114.6920° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	CLAYEY SAND (SC), brown medium dense				5-7-8 N=15	1.6			16
	5.0 **WELL GRADED SAND (SW)**, tannish brown, medium dense	5			8-12-14	0.7	92		
	7.5 **SILTY SAND (SM)**, brown, medium dense				3-5-9 N=14				
	10.0 **WELL GRADED SAND (SW)**, tannish brown, medium dense	10			12-18-24	1.0	94		
		15			6-5-18 N=23				
	dense	20			7-13-18 N=31				
	21.5 ***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-18-2023

Boring Completed
10-18-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-13

Graphic Log	Location: See Exploration Plan Latitude: 33.6561° Longitude: -114.6848° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	CLAYEY SAND (SC), brown								
	medium dense				21-23-24	2.3	93		19
	loose	5			3-3-4 N=7				
	7.5								
	WELL GRADED SAND (SW), brown, medium dense				7-10-14	1.1	94		
		10			6-7-10 N=17				
	dense	15			8-15-20 N=35				
	20.0								
	SILTY SAND (SM), brown, medium dense	20			9-13-15 N=28				
	21.5								
	Boring Terminated at 21.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-18-2023

Boring Completed
10-18-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-14

Location: See Exploration Plan

Latitude: 33.6592° Longitude: -114.6822°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	CLAYEY SAND (SC), brown medium dense			5-7-7 N=14	2.6			18
	5.0 **POORLY GRADED SAND (SP)**, tan to brown, medium dense			16-12-20	0.7	106		
				5-9-13 N=22				
				8-16-23				
				5-10-13 N=23				
				5-9-11 N=20				
	21.5 ***Boring Terminated at 21.5 Feet***							

5
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15
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See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-24-2023

Boring Completed
10-24-2023

Grace Solar
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Terracon
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Laguna Hills, CA

Boring Log No. BH-15

Location: See Exploration Plan

Latitude: 33.6537° Longitude: -114.6806°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	CLAYEY SAND (SC), brown medium dense			8-12-15	1.0	96		12
				4-7-8 N=15				
				9-18-22	1.5	92		
	10.0 **POORLY GRADED SAND (SP)**, brown, medium dense			5-9-11 N=20				
				5-8-9 N=17				
	21.5			7-11-16 N=27				
	Boring Terminated at 21.5 Feet							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-20-2023

Boring Completed
10-20-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-16

Location: See Exploration Plan

Latitude: 33.6524° Longitude: -114.7182°

Graphic Log	Depth (Ft.)	Material Description	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	0	**CLAYEY SAND (SC)**, trace gravel, brown							
		very dense			10-32-32 N=64				22
	5	dense			26-43-42	1.9	94		
	7.5	**POORLY GRADED SAND (SP)**, tan, medium dense			6-10-11 N=21				
	10				24-19-25				
	15	very dense			17-23-35 N=58				
	20.0								
	20	**SILTY SAND (SM)**, trace gravel, very dense			6-29-45 N=74				
	21.5	***Boring Terminated at 21.5 Feet***							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-26-2023

Boring Completed
10-26-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-17

Location: See Exploration Plan

Latitude: 33.6521° Longitude: -114.7116°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	CLAYEY SAND (SC), trace gravel, brown medium dense			15-25-33	4.9	98		27
	7.5			10-11-10 N=21				
	POORLY GRADED SAND (SP), tan, medium dense			9-20-25	0.9	88		
				5-9-10 N=19				
				6-9-13 N=22				
	tan			7-12-14 N=26				
	21.5							
	Boring Terminated at 21.5 Feet							

5
10
15
20

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-24-2023

Boring Completed
10-24-2023

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Grace Solar
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Terracon
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Boring Log No. BH-18

Location: See Exploration Plan

Latitude: 33.6472° Longitude: -114.7181°

Graphic Log	Depth (Ft.)	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
		CLAYEY SAND (SC), brown dense				12-19-21 N=40				20
	5.0	**POORLY GRADED SAND (SP)**, tannish brown, medium dense, no recovery trace gravel	5			13-19-27 4-8-15 N=23				
	10.0	**CLAYEY SAND (SC)**, trace gravel, tannish brown, dense	10			11-30-37	0.3	110		
	15.0	**POORLY GRADED SAND (SP)**, tannish gray, medium dense	15			4-6-9 N=15				
	21.5		20			5-10-12 N=22				
		Boring Terminated at 21.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-25-2023

Boring Completed
10-25-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-19

Location: See Exploration Plan

Latitude: 33.6473° Longitude: -114.7114°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
					4.8			26
				6-6-7	4.9	101		
5				2-2-5 N=7				
				5-7-10	2.8	103		
10				4-7-10 N=17				
15				6-13-18 N=31				
20				7-15-21 N=36				

SILTY SAND (SM), brown

loose

tannish brown

10.0

POORLY GRADED SAND (SP), tannish brown, medium dense

tan, dense

21.5

Boring Terminated at 21.5 Feet

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-24-2023

Boring Completed
10-24-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-20

Graphic Log	Location: See Exploration Plan Latitude: 33.6496° Longitude: -114.7092° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	CLAYEY SAND (SC), brown								
	loose				2-1-3 N=4				17
	medium dense	5			4-9-12	1.0	95		
	7.5 **POORLY GRADED SAND (SP)**, tan, medium dense				4-8-11 N=19				
	dense	10			10-26-44	0.4	92		
	trace gravel, tannish brown	15			5-17-23 N=40				
	medium dense	20			9-13-16 N=29				
	21.5 ***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-24-2023

Boring Completed
10-24-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-21

Graphic Log	Location: See Exploration Plan Latitude: 33.6514° Longitude: -114.7026° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	SILTY SAND (SM), trace gravel, brown								
	dense				14-47-45	1.7 3.8	107		24
	tannish brown, medium dense	5			7-10-12 N=22				
					6-18-23	1.1	101		
		10			4-6-9 N=15				
	15.0 **POORLY GRADED SAND (SP)**, tannish brown, medium dense	15			4-8-11 N=19				
	trace gravel, dense	20			11-17-21 N=38				
	21.5 ***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-23-2023

Boring Completed
10-23-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-22

Graphic Log	Location: See Exploration Plan Latitude: 33.6473° Longitude: -114.7025° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	SANDY LEAN CLAY (CL), trace gravel, reddish brown very stiff				8-15-12 N=27				
	5.0 **CLAYEY SAND (SC)**, brown, medium dense	5			10-15-17	1.7	106		
					4-6-7 N=13				
	trace gravel, brown	10			11-19-22	1.2	111		
	15.0 **POORLY GRADED SAND (SP)**, tannish brown, medium dense	15			6-11-14 N=25				
	21.5	20			7-13-16 N=29				
	Boring Terminated at 21.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-23-2023

Boring Completed
10-23-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-23

Location: See Exploration Plan

Latitude: 33.6505° Longitude: -114.6919°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
	CLAYEY SAND (SC), brown medium dense			12-23-25				14
5				3-7-8 N=15	3.0	89		
7.5	**POORLY GRADED SAND (SP)**, brown, medium dense			12-20-28	0.9	95		
10				7-10-11 N=21				
10.0	**SILTY SAND (SM)**, brown, medium dense							
15				6-8-9 N=17				
15.0	**POORLY GRADED SAND (SP)**, brown, medium dense							
20				5-9-10 N=19				
21.5	***Boring Terminated at 21.5 Feet***							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-18-2023

Boring Completed
10-18-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-24

Location: See Exploration Plan

Latitude: 33.6466° Longitude: -114.6918°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
	SILTY SAND (SM), brown medium dense			4-4-6 N=10	2.1			12
5.0	**POORLY GRADED SAND (SP)**, brown, medium dense			7-15-19	0.4	96		
				3-5-7 N=12				
10				15-22-25	0.8	94		
15				5-8-10 N=18				
20								
20.0	**SILTY SAND (SM)**, brown, medium dense			7-11-13 N=24				
21.5	***Boring Terminated at 21.5 Feet***							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-18-2023

Boring Completed
10-18-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-25

Location: See Exploration Plan

Latitude: 33.6516° Longitude: -114.6848°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
SILTY SAND (SM), brown								
loose				4-5-8	0.8 1.6	87		19
medium dense	5			4-5-7 N=12				
tannish brown				10-12-16	0.5	89		
	10			4-4-6 N=10				
	15			5-6-10 N=16				
20.0 **WELL GRADED SAND (SW)**, brown, dense	20			9-14-18 N=32				
21.5 ***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-20-2023

Boring Completed
10-20-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-26

Location: See Exploration Plan

Latitude: 33.6465° Longitude: -114.6851°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
								13
				12-27-39	1.5	95		
5				4-6-7 N=13				
				13-23-27	0.8	96		
10				4-9-13 N=22				
15				6-11-15 N=26				
20				7-12-16 N=28				

CLAYEY SAND (SC), brown

dense

medium dense

10.0

POORLY GRADED SAND (SP), trace silt, brown, medium dense

15.0

WELL GRADED SAND (SW), trace silt, brown, medium dense

21.5

Boring Terminated at 21.5 Feet

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-18-2023

Boring Completed
10-18-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-27

Graphic Log	Location: See Exploration Plan Latitude: 33.6502° Longitude: -114.6769° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	SILTY SAND (SM), trace gravel, brown medium dense				22-16-25	2.4 2.2	108		16
		5			5-6-8 N=14				
	7.5 **POORLY GRADED SAND (SP)**, tan, medium dense				6-12-7	1.1	99		
		10			4-7-10 N=17				
		15			4-9-11 N=20				
	tannish brown, loose 21.5	20			3-4-5 N=9				
	Boring Terminated at 21.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-24-2023

Boring Completed
10-24-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-28

Location: See Exploration Plan

Latitude: 33.6469° Longitude: -114.6767°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
								17
				5-12-15 N=27				
5				14-19-29	1.8	103		
7.5								
				5-10-12 N=22				
10				5-19-30	0.7	103		
15				3-6-7 N=13				
20				4-9-11 N=20				
20.0								
21.5								

CLAYEY SAND (SC), trace gravel, brown

medium dense

WELL GRADED SAND WITH SILT (SW-SM), tan to brown, medium dense

WELL GRADED SAND (SW), tan to brown, medium dense

Boring Terminated at 21.5 Feet

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-24-2023

Boring Completed
10-24-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-29

Graphic Log	Location: See Exploration Plan Latitude: 33.6535° Longitude: -114.6748° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	SILTY SAND (SM), trace gravel, brown								
	dense				18-28-31	1.7 1.7	100		15
	5.0	5			5-8-11 N=19				
	POORLY GRADED SAND (SP), tannish brown, medium dense				6-18-29	1.0	102		
		10			5-7-8 N=15				
	trace gravel, tan	15			5-10-13 N=23				
	21.5	20			4-7-10 N=17				
	Boring Terminated at 21.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-24-2023

Boring Completed
10-24-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-30

Graphic Log	Location: See Exploration Plan Latitude: 33.6488° Longitude: -114.6659° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	CLAYEY SAND (SC), trace gravel, brown								
	medium dense				3-6-10 N=16				19
	loose	5			5-6-9	4.2	91		
	medium dense				5-5-9 N=14				
	tannish brown, dense	10			19-25-36	0.6	98		
	medium dense	15			5-10-11 N=21				
	20.0 **WELL GRADED SAND (SW)**, tannish brown, medium dense 21.5	20			7-10-12 N=22				
	Boring Terminated at 21.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-17-2023

Boring Completed
10-17-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-31

Location: See Exploration Plan

Latitude: 33.6464° Longitude: -114.6614°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
	SILTY SAND (SM), brown							
	dense			24-26-39	1.5 1.3	98		21
5	medium dense			6-10-12 N=22				
				13-16-27	0.7	100		
10	10.0 **WELL GRADED SAND (SW)**, brown, medium dense			7-8-12 N=20				
15	15.0 **SILTY SAND (SM)**, brown, medium dense			5-10-15 N=25				
20				6-8-11 N=19				
	21.5 ***Boring Terminated at 21.5 Feet***							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-17-2023

Boring Completed
10-17-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-32

Location: See Exploration Plan

Latitude: 33.6447° Longitude: -114.7180°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
	CLAYEY SAND (SC), brown medium dense			4-7-9 N=16				34
5				11-14-15	0.9	99		
7.5	**POORLY GRADED SAND (SP)**, tannish gray, medium dense			6-11-13 N=24				
10	dense			17-29-32	0.3	103		
15	tannish yellow			6-13-21 N=34				
20	medium dense			9-14-15 N=29				
21.5	***Boring Terminated at 21.5 Feet***							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-25-2023

Boring Completed
10-25-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-33

Location: See Exploration Plan

Latitude: 33.6417° Longitude: -114.7156°

Graphic Log / Depth (Ft.)		Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0	**SILTY SAND (SM)**, trace gravel, brown								
	dense				21-31-40	0.5	101		21
	tannish brown	5			8-19-20 N=39				
7.5	**POORLY GRADED SAND (SP)**, trace gravel, tan, medium dense				16-31-31				
		10			6-10-12 N=22				
		15			3-8-12 N=20				
	dense	20			6-15-21 N=36				
21.5	***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-25-2023

Boring Completed
10-25-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-34

Location: See Exploration Plan

Latitude: 33.6433° Longitude: -114.7107°

Graphic Log	Depth (Ft.)	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	0	**CLAYEY SAND (SC)**, trace gravel, brown dense				9-16-17 N=33				
	5	tan	5			15-34-37	0.7	95		
	7.5	**POORLY GRADED SAND (SP)**, tan, medium dense				5-8-10 N=18				
	10		10			25-21-23	0.3	101		
	15	no recovery	15			5-8-9 N=17				
	20.0		20							
		SILTY SAND (SM), tan, medium dense				7-8-12 N=20				
	21.5	***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-25-2023

Boring Completed
10-25-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-35

Graphic Log	Location: See Exploration Plan Latitude: 33.6440° Longitude: -114.6778° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	SILTY SAND (SM), brown medium dense				6-12-18	1.9 1.8	99		19
	5.0 **CLAYEY SAND (SC)**, brown, medium dense tannish brown	5			10-14-15 N=29				
					10-15-19	0.7	89		
		10			5-7-9 N=16				
		15			6-10-12 N=22				
		20			6-9-10 N=19				
	21.5 ***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-17-2023

Boring Completed
10-17-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-36

Graphic Log	Location: See Exploration Plan Latitude: 33.6398° Longitude: -114.6813° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	CLAYEY SAND (SC), brown medium dense	5			7-13-14 N=27				15
					14-19-26	1.9	98		
					9-14-14 N=28				
		10			15-23-28	0.7	91		
		15			5-7-8 N=15				
		20			7-10-13 N=23				
	21.5								
	Boring Terminated at 21.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-20-2023

Boring Completed
10-20-2023

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-37

Location: See Exploration Plan

Latitude: 33.6440° Longitude: -114.6720°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
	CLAYEY SAND (SC), brown medium dense			10-13-16	1.6	100		13
5				5-7-7 N=14				
				9-15-19	1.3	94		
10				6-10-12 N=22				
15				7-10-13 N=23				
20				6-9-16 N=25				
20.0	**WELL GRADED SAND (SW)**, tannish brown, medium dense							
21.5	***Boring Terminated at 21.5 Feet***							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-17-2023

Boring Completed
10-17-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-38

Location: See Exploration Plan

Latitude: 33.6434° Longitude: -114.6610°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
								11
				5-10-17 N=27				
5				15-24-25	1.2	92		
				5-7-10 N=17				
10				8-12-16	1.9	91		
15				3-6-8 N=14				
20				6-9-10 N=19				

CLAYEY SAND (SC), brown

medium dense

20.0

WELL GRADED SAND (SW), tannish brown, medium dense

21.5

Boring Terminated at 21.5 Feet

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-17-2023

Boring Completed
10-17-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-39

Location: See Exploration Plan

Latitude: 33.6349° Longitude: -114.6950°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0	**SILTY SAND (SM)**, brown							
	dense			16-29-33	1.6 1.6	97		19
5	medium dense			6-8-9 N=17				
7.5	**WELL GRADED SAND (SW)**, brown, medium dense			8-20-23	1.1	99		
10				6-10-13 N=23				
15				8-10-14 N=24				
20				7-11-18 N=29				
21.5	***Boring Terminated at 21.5 Feet***							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-19-2023

Boring Completed
10-19-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-40

Location: See Exploration Plan

Latitude: 33.6371° Longitude: -114.6871°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
								6
				4-7-7 N=14				
5				9-13-16				
7.5				3-5-6 N=11				
10				10-17-21	0.4	99		
15				6-10-11 N=21				
20				6-12-15 N=27				
21.5								

POORLY GRADED SAND WITH CLAY (SP-SC), brown

medium dense

no recovery

WELL GRADED SAND (SW), tannish brown, medium dense

Boring Terminated at 21.5 Feet

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-19-2023

Boring Completed
10-19-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-41

Location: See Exploration Plan

Latitude: 33.6327° Longitude: -114.6866°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
								11
				10-13-18	0.7	94		
5.0								
5				3-4-6 N=10				
				9-11-16	0.7	92		
10				6-9-13 N=22				
15				3-9-11 N=20				
20				7-9-13 N=22				
21.5								

POORLY GRADED SAND WITH CLAY (SP-SC), brown

medium dense

WELL GRADED SAND (SW), brown, medium dense

Boring Terminated at 21.5 Feet

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-19-2023

Boring Completed
10-19-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-42

Location: See Exploration Plan

Latitude: 33.6370° Longitude: -114.6822°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND (SP), trace clay, tannish brown medium dense	0							
				5-5-8 N=13	1.0 0.8			3
	5.0							
WELL GRADED SAND (SW), brown, medium dense	5			7-13-17	0.4	91		
	7.5							
SILTY SAND (SM), tannish brown, medium dense				5-7-8 N=15				
	10			19-23-26	0.6	94		
	15			7-10-12 N=22				
	20			6-10-12 N=22				
	21.5							
Boring Terminated at 21.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-19-2023

Boring Completed
10-19-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-43

Location: See Exploration Plan

Latitude: 33.6327° Longitude: -114.6823°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
	POORLY GRADED SAND WITH CLAY (SP-SC), tannish brown							
	medium dense			5-9-13 NO RECOVERY				
5	very dense			36-35-18 N=53				
7.5	**CLAYEY SAND (SC)**, trace gravel, tannish brown, medium dense			11-18-22	0.3	102		
10				6-8-9 N=17				
15.0	**FAT CLAY WITH SAND (CH)**, tannish brown, very stiff			8-9-15 N=24			66-16-50	70
15								
20.0								
20	**SILTY SAND (SM)**, tannish brown, dense			9-13-17 N=30				
21.5	***Boring Terminated at 21.5 Feet***							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-16-2023

Boring Completed
10-16-2023

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-44

Location: See Exploration Plan

Latitude: 33.6346° Longitude: -114.6763°

Depth (Ft.)	Graphic Log		Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0		**CLAYEY SAND (SC)**, brown							
		very dense			6-25-26 N=51			21-11-10	20
5		dense			20-30-35	1.1	98		
		medium dense			8-11-11 N=22				
10		10.0 **WELL GRADED SAND (SW)**, tannish brown, medium dense			10-20-23	0.8	103		
15					6-11-13 N=24				
20		20.0 **SILTY SAND (SM)**, tannish brown, medium dense			7-9-10 N=19				
		21.5 ***Boring Terminated at 21.5 Feet***							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CR

Boring Started
10-17-2023

Boring Completed
10-17-2023

Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-45

Location: See Exploration Plan

Latitude: 33.6458° Longitude: -114.6973°

Depth (Ft.)	Graphic Log	Material Description	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0.1		**TOPSOIL TO 0.1'**, poorly graded sand with gravel								
		SILTY SAND WITH GRAVEL (SM), light brown, gravel up to 3/4" in diameter, medium dense, sample disturbed			20-20-25				NP	17
5					3-7-9 N=16					
7.5		**POORLY GRADED SAND WITH SILT (SP-SM)**, trace gravel, light brown, medium dense, gravel up to 3/4" in diameter, sample disturbed			10-19-23					
10					6-6-9 N=15	C-5				
15					7-12-15 N=27					
20					5-10-12 N=22					
21.5		***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Logged by
DD

Boring Started
02-11-2025

Boring Completed
02-11-2025

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-46

Location: See Exploration Plan

Latitude: 33.6421° Longitude: -114.6918°

Depth (Ft.)	Graphic Log	Material Description	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0.1		**TOPSOIL TO 0.1'**, poorly graded sand with gravel								
		SILTY SAND WITH GRAVEL (SM), light brown, gravel up to 1" in diameter, medium dense			6-6-6 N=12				NP	17
5.0		**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown, medium dense, sample disturbed			10-14-16					
					6-11-12 N=23					
10.0		**POORLY GRADED SAND WITH SILT AND GRAVEL (SP-SM)**, light brown, medium dense, gravel up to 3/4" in diameter			11-18-24	C-5	2.0	108		
15.0		**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown to brown, medium dense			7-11-14 N=25					
		dense			8-17-22 N=39	C-4				
21.5		***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Logged by
DD

Boring Started
02-11-2025

Boring Completed
02-11-2025

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-47

Location: See Exploration Plan

Latitude: 33.6464° Longitude: -114.6691°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0.1	**TOPSOIL TO 0.1'**, poorly graded sand with gravel								
	POORLY GRADED SAND WITH SILT (SP-SM), trace gravel, light brown								
	medium dense			5-11-12	C-5	1.6	100	NP	7
5	loose			3-2-2 N=4	C-6				
	medium dense, no recovery			5-8-14	C-5				
10				3-5-7 N=12					
15				4-8-9 N=17					
20				8-11-15 N=26					
21.5	***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Logged by
DD

Boring Started
02-11-2025

Boring Completed
02-11-2025

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-48

Location: See Exploration Plan

Latitude: 33.6610° Longitude: -114.6858°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0.1	**TOPSOIL TO 0.1'**, poorly graded sand with gravel **SILTY SAND (SM)**, light brown, moderate cementation dense			14-18-18 N=36	C-4				
5.0	**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown, medium dense, sample disturbed			11-18-22		1.2			
				5-6-13 N=19					
10.0	**POORLY GRADED SAND WITH SILT AND GRAVEL (SP-SM)**, light brown, medium dense, moderate cementation			18-22-30	C-5				
15.0	**POORLY GRADED SAND WITH SILT (SP-SM)**, trace gravel, light brown to white, medium dense, gravel up to 3/4" in diameter			7-10-13 N=23					
	light brown			6-10-14 N=24					
21.5	***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Logged by
DD

Boring Started
02-10-2025

Boring Completed
02-10-2025

Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. BH-49

Location: See Exploration Plan

Latitude: 33.6610° Longitude: -114.6858°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0.1	**TOPSOIL TO 0.1'**, poorly graded sand with gravel								
	POORLY GRADED SAND WITH SILT (SP-SM), light brown, moderate cementation								
	medium dense, sample disturbed			6-13-17				NP	11
5				4-6-8 N=14					
	sample disturbed			7-13-20					
10				4-7-7 N=14	C-5				
15				7-10-14 N=24					
20	dense			7-14-20 N=34	C-4				
21.5	***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Logged by
DD

Boring Started
02-11-2025

Boring Completed
02-11-2025

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

Terracon
1355 E Cooley Dr
Colton, CA

Boring Log No. BH-50

Location: See Exploration Plan

Latitude: 33.6645° Longitude: -114.7037°

Depth (Ft.) Approximate Elevation: 399 (Ft.)

Depth (Ft.)	Material Description	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0	**SILTY SAND (SM)**, brown dense	12-14-16 N=30			NP	42.0
5	medium dense	16-20-30	2.4	104		
		8-11-14 N=25				
10	no recovery	5-15-30				
15.0 (384)	**SILTY SAND WITH GRAVEL (SM)**, light brown, very dense	50/6"				
20.0 (379)	**SILTY SAND (SM)**, light brown, very dense	22-32-34 N=66				36.3
21.5 (377.5)	***Boring Terminated at 21.5 Feet***					

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
AC

Boring Started
11-20-2025

Boring Completed
11-20-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Boring Log No. BH-51

Location: See Exploration Plan

Latitude: 33.6625° Longitude: -114.6993°

Depth (Ft.) Approximate Elevation: 397 (Ft.)

Depth (Ft.)	Material Description	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0	**SILTY SAND (SM)**, brown					
	medium dense, trace clay	7-14-18	1.6	105		
5		5-10-15 N=25				
		5-15-30	1.1	108		
10	dense	18-15-22 N=37			NP	34.0
15	very dense	12-24-30 N=54				
20	light brown, dense	15-20-25 N=45				
21.5	***Boring Terminated at 21.5 Feet***					
	375.5					

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
AC

Boring Started
11-20-2025

Boring Completed
11-20-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Boring Log No. BH-52

Location: See Exploration Plan

Latitude: 33.6631° Longitude: -114.6908°

Depth (Ft.) Approximate Elevation: 398 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
SILTY SAND (SM), brown medium dense				6-9-12 N=21				
no recovery	5			5-11-19				
trace clay				5-9-14 N=23				
10.0 388								
SILTY CLAYEY SAND (SC-SM), dark brown, medium dense	10			5-10-20	4.8	105	18-14-4	12.5
15.0 383								
SILTY SAND (SM), brown, medium dense	15			4-6-9 N=15				
dense, trace gravel	20			5-18-25 N=43				
21.5 376.5								
Boring Terminated at 21.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
AC

Boring Started
11-20-2025

Boring Completed
11-20-2025

1355 E Cooley Dr
Colton, CA

Boring Log No. BH-53

Location: See Exploration Plan

Latitude: 33.6582° Longitude: -114.7037°

Depth (Ft.) | Approximate Elevation: 397 (Ft.)

Depth (Ft.)	Material Description	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0	**SANDY LEAN CLAY (CL)**, brown, very stiff			8-15-15	6.1	106	25-14-11	50.3
5.0 (392)	**SILTY SAND (SM)**, brown, medium dense			2-5-7 N=12				
				3-10-16	1.6	109		
10.0 (387)	**POORLY GRADED SAND (SP)**, trace silt, brown, medium dense			6-7-15 N=22			NP	2.8
15.0 (382)	**SANDY LEAN CLAY (CL)**, light brown, hard			50/6"				
20.0 (377)	**SILTY SAND WITH GRAVEL (SM)**, light brown, very dense			15-26-36 N=62				
21.5 (375.5)	***Boring Terminated at 21.5 Feet***							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
AC

Boring Started
11-20-2025

Boring Completed
11-20-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Boring Log No. BH-54

Location: See Exploration Plan

Latitude: 33.6590° Longitude: -114.6972°

Depth (Ft.) Approximate Elevation: 401 (Ft.)

Depth (Ft.)	Material Description	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0	**SILTY SAND (SM)**, trace gravel, light brown					
	medium dense	5-8-10 N=18				12.8
5	sample disturbed	8-15-22	0.8			
7.5 / 393.5	**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown, medium dense	5-11-17 N=28				
10	dense	16-30-36	2.1	103		
15	trace gravel	6-16-28 N=44				
20	very dense	7-20-44 N=64				
21.5 / 379.5	***Boring Terminated at 21.5 Feet***					

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
MB

Boring Started
12-05-2025

Boring Completed
12-05-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

Terracon
1355 E Cooley Dr
Colton, CA

Boring Log No. BH-55

Location: See Exploration Plan

Latitude: 33.6591° Longitude: -114.6951°

Depth (Ft.) Approximate Elevation: 401 (Ft.)

Depth (Ft.)	Graphic Log	Material Description	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0		**SILTY SAND WITH GRAVEL (SM)**, light brown, medium dense		9-18-24	1.2	105		21.0
5		poor recovery		4-7-6 N=13				
		no recovery		8-18-26				
10.0		391						
10		**POORLY GRADED SAND WITH SILT (SP-SM)**, trace gravel, light brown, dense		7-14-20 N=34				
15				9-16-22 N=38				
20				7-16-20 N=36				
21.5		379.5						
		Boring Terminated at 21.5 Feet						

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
MB

Boring Started
12-05-2025

Boring Completed
12-05-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Boring Log No. BH-56

Location: See Exploration Plan

Latitude: 33.6584° Longitude: -114.6873°

Depth (Ft.) Approximate Elevation: 397 (Ft.)

Depth (Ft.)	Material Description	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0	**SILTY SAND (SM)**, brown, medium dense	6-9-13 N=22				
5		10-15-20	2.8	87		
7.5 / 389.5	**POORLY GRADED SAND (SP)**, trace silt, brown, medium dense	5-8-12 N=20				2.9
10	sample disturbed	7-13-24	0.9			
15	dense	7-12-20 N=32				
20		8-15-22 N=37	2.7			1.7
21.5 / 375.5	***Boring Terminated at 21.5 Feet***					

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
AC

Boring Started
11-19-2025

Boring Completed
11-19-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Boring Log No. BH-57

Location: See Exploration Plan

Latitude: 33.6553° Longitude: -114.6994°

Depth (Ft.) Approximate Elevation: 402 (Ft.)

Depth (Ft.)	Description	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0	**SILTY SAND (SM)**, trace gravel, brown to light brown					
	medium dense, sample disturbed	11-15-17	1.3		NP	19.0
5	no recovery	5-7-10 N=17				
7.5	394.5					
	POORLY GRADED SAND (SP), trace silt and gravel, light brown, medium dense, no recovery	7-12-20				
10		5-9-14 N=23				
	dense	8-13-18 N=31				
15						
20	medium dense	5-10-13 N=23				2.8
21.5	380.5					
	Boring Terminated at 21.5 Feet					

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
MB

Boring Started
12-05-2025

Boring Completed
12-05-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Boring Log No. BH-58

Location: See Exploration Plan

Latitude: 33.6557° Longitude: -114.6781°

Depth (Ft.) Approximate Elevation: 398 (Ft.)

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
	SILTY SAND (SM), light brown medium dense			2-7-8 N=15			NP	18.8
5				5-10-20	0.7	99		
				5-10-15 N=25				
10				10-20-30	0.9	91		
15				10-10-17 N=27				1.6
20				7-10-15 N=25				
21.5								
376.5	***Boring Terminated at 21.5 Feet***							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
AC

Boring Started
11-19-2025

Boring Completed
11-19-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Boring Log No. BH-59

Location: See Exploration Plan

Latitude: 33.6531° Longitude: -114.6971°

Depth (Ft.) Approximate Elevation: 402 (Ft.)

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0	**SILTY SAND (SM)**, trace gravel, brown to light brown loose			2-5-9	3.9	109	19-17-2	16.3
5.0 / 397	**POORLY GRADED SAND WITH SILT (SP-SM)**, trace gravel, light brown, medium dense no recovery			8-9-11 N=20				
5				5-12-28				
10				4-8-13 N=21				
15				6-11-15 N=26				
20.0 / 382								
20	**POORLY GRADED SAND (SP)**, trace silt, light brown, medium dense			8-8-12 N=20				
21.5 / 380.5	***Boring Terminated at 21.5 Feet***							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
MB

Boring Started
12-04-2025

Boring Completed
12-04-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Boring Log No. BH-60

Location: See Exploration Plan

Latitude: 33.6518° Longitude: -114.6950°

Depth (Ft.) Approximate Elevation: 400 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	0							
SILTY SAND (SM), trace gravel, light brown medium dense				4-5-7 N=12				
5.0 / 395	5			7-12-15	1.4			
POORLY GRADED SAND WITH GRAVEL (SP), light brown, medium dense, sample disturbed				4-8-11 N=19				2.6
	10			10-18-25	2.6	106		
15.0 / 385	15			5-10-12 N=22				1.7
POORLY GRADED SAND (SP), trace silt and gravel, light brown, medium dense								
21.5 / 378.5	20			4-9-11 N=20				
Boring Terminated at 21.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Logged by
MB

Boring Started
12-04-2025

Boring Completed
12-04-2025

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Boring Log No. BH-61

Location: See Exploration Plan

Latitude: 33.6501° Longitude: -114.6972°

Depth (Ft.) Approximate Elevation: 398 (Ft.)

Depth (Ft.)	Description	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Percent Fines
0	**SILTY SAND WITH GRAVEL (SM)**, light brown				
2.5 / 395.5	**SILTY SAND (SM)**, light brown, medium dense, sample disturbed	12-16-20	1.0		14.7
5		3-7-9 N=16			
7.5 / 390.5	**POORLY GRADED SAND (SP)**, trace silt, light brown, medium dense	10-20-30	0.8	99	
10		8-12-16 N=28			
15	trace gravel, dense	6-14-17 N=31			
20		6-14-20 N=34			
21.5 / 376.5	***Boring Terminated at 21.5 Feet***				

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
MB

Boring Started
12-04-2025

Boring Completed
12-04-2025

Terracon
1355 E Cooley Dr
Colton, CA

Boring Log No. BH-62

Location: See Exploration Plan

Latitude: 33.6479° Longitude: -114.6992°

Depth (Ft.) Approximate Elevation: 394 (Ft.)

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0								
	SILTY SAND WITH GRAVEL (SM), light brown							
	loose			5-5-3 N=8				
5								
	medium dense			8-12-16	2.4	104		
7.5	386.5							
	POORLY GRADED SAND (SP), some gravel, light brown, medium dense			4-6-8 N=14				
10								
				9-15-17	1.5	101		
15								
				5-7-12 N=19				
20								
				7-9-11 N=20				2.1
21.5	372.5							
	Boring Terminated at 21.5 Feet							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
MB

Boring Started
12-04-2025

Boring Completed
12-04-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

Terracon
1355 E Cooley Dr
Colton, CA

Boring Log No. BH-63

Location: See Exploration Plan

Latitude: 33.6472° Longitude: -114.6970°

Depth (Ft.) Approximate Elevation: 396 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
CLAYEY SAND (SC), trace gravel, light brown medium dense	0							
				10-11-12	3.6	105	20-12-8	20.7
5.0 391	5							
POORLY GRADED SAND WITH SILT (SP-SM), trace gravel, medium dense sample disturbed				4-7-9 N=16				
				5-12-16	0.8			
10.0 386	10							
POORLY GRADED SAND (SP), trace silt and gravel, medium dense				5-7-8 N=15				
	15			2-8-9 N=17				
	20			6-11-16 N=27				
21.5 374.5								
Boring Terminated at 21.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
MB

Boring Started
12-04-2025

Boring Completed
12-04-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Boring Log No. BH-64

Location: See Exploration Plan

Latitude: 33.6472° Longitude: -114.6813°

Depth (Ft.) Approximate Elevation: 387 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	SILTY SAND (SM), brown medium dense			5-7-8 N=15				
				5-11-19	1.5	87		
	dense			7-14-18 N=32				
	10.0 377 **CLAYEY SAND (SC)**, brown, dense			13-40-50/6"	9.9	109	28-13-15	28.6
	15.0 372 **SILTY SAND (SM)**, brown, medium dense			5-12-16 N=28				
	20.0 367 **POORLY GRADED SAND WITH SILT (SP-SM)**, brown, medium dense			4-6-10 N=16				7.7
	21.5 365.5 ***Boring Terminated at 21.5 Feet***							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
AC

Boring Started
11-18-2025

Boring Completed
11-18-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Boring Log No. BH-65

Location: See Exploration Plan

Latitude: 33.6437° Longitude: -114.7128°

Depth (Ft.) Approximate Elevation: 398 (Ft.)

Depth (Ft.)	Graphic Log	Material Description	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0		**SILTY SAND (SM)**, brown, trace gravel							
		medium dense, no recovery			40-26-30				
5					6-7-4 N=11	4.5			20.8
7.5		390.5							
		POORLY GRADED SAND (SP), trace silt, brown, medium dense			11-16-30	0.6	95		3.6
10					8-12-14 N=26				
15.0		383							
15		**SILTY SAND (SM)**, brown, dense			11-20-14 N=34			NP	31.7
20		light brown, medium dense			6-7-15 N=22				
21.5		376.5							
		Boring Terminated at 21.5 Feet							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
AC

Boring Started
11-20-2025

Boring Completed
11-20-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Boring Log No. BH-66

Location: See Exploration Plan

Latitude: 33.6444° Longitude: -114.6671°

Depth (Ft.) Approximate Elevation: 363 (Ft.)

Depth (Ft.)	Material Description	Elevation	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0	**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown, trace gravel						
	medium dense		6-10-12 N=22				11.8
5			5-9-20	1.6	107		
7.5		355.5					
	SANDY SILTY CLAY (CL-ML), reddish brown, very stiff		6-8-11 N=19			24-17-7	68.0
10.0		353					
10	**CLAYEY SAND (SC)**, brown, medium dense		2-7-12	2.6	101		
15.0		348					
15	**SILTY CLAY (CL-ML)**, brown, hard, trace sand		6-13-20 N=33				
	very stiff		4-7-12 N=19				
20							
21.5		341.5					
	Boring Terminated at 21.5 Feet						

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
AC

Boring Started
11-19-2025

Boring Completed
11-19-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

Terracon
1355 E Cooley Dr
Colton, CA

Boring Log No. BH-67

Location: See Exploration Plan

Latitude: 33.6419° Longitude: -114.6831°

Depth (Ft.) Approximate Elevation: 376 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
SILTY SAND (SM), light brown, dense							18-15-3	18.2
				20-30-50/6"	3.0	112		
brown, trace clay and gravel	5			10-15-23 N=38	1.7			
7.5 368.5								
CLAYEY SAND (SC), light brown, very dense				18-50/6"				
10.0 366								
SILTY SAND (SM), light brown, dense	10			8-16-22 N=38				
medium dense	15			6-8-12 N=20				
	20			5-8-12 N=20				
21.5 354.5								
Boring Terminated at 21.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
AC

Boring Started
11-18-2025

Boring Completed
11-18-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Boring Log No. BH-68

Location: See Exploration Plan

Latitude: 33.6344° Longitude: -114.6905°

Depth (Ft.) Approximate Elevation: 388 (Ft.)

Depth (Ft.)	Material Description	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0	**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown, trace gravel					
	medium dense	3-6-7 N=13				11.7
5		20-21-22	4.8	92		
		6-9-15 N=24				
10		6-24-32	1.2	95		
	dense	8-15-18 N=33				
15						
	no recovery	6-12-18 N=30				
20						
21.5	***Boring Terminated at 21.5 Feet***					
366.5						

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Drill Rig
GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
AC

Boring Started
11-18-2025

Boring Completed
11-18-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. SBH-01

Location: See Exploration Plan

Latitude: 33.6493° Longitude: -114.7258°

Depth (Ft.)	Graphic Log	Material Description	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0		**SILTY SAND (SM)**, trace gravel, brown							
		dense			15-29-47	1.7 1.9	104		20
5		tannish brown, medium dense			11-14-15 N=29				
		dense			13-27-33				
10		**POORLY GRADED SAND (SP)**, tan, very dense			7-23-33 N=56				
		dense			7-21-28 N=49				
20		**SILTY SAND (SM)**, tan, very dense			20-36-40 N=76				28
25									

10.0

20.0

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-25-2023

Boring Completed
10-25-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. SBH-01

Graphic Log	Location: See Exploration Plan Latitude: 33.6493° Longitude: -114.7258° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	SILTY SAND (SM), tan, very dense *(continued)*				17-38-44 N=82				
		30			24-40-46 N=86				25
		35			17-50/6"				
	dense	40			14-21-22 N=43				
	very dense	45			16-45-50/5"				
		50							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-25-2023

Boring Completed
10-25-2023

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. SBH-01

Graphic Log	Location: See Exploration Plan Latitude: 33.6493° Longitude: -114.7258° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	SILTY SAND (SM), tan, very dense *(continued)* 51.5				19-42-42 N=84				
	Boring Terminated at 51.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-25-2023

Boring Completed
10-25-2023

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. SBH-02

Location: See Exploration Plan

Latitude: 33.6491° Longitude: -114.7212°

Depth (Ft.)	Graphic Log	Material Description	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0		**CLAYEY SAND (SC)**, trace gravel, brown							
		medium dense			8-8-13 N=21			21-11-10	37
5.0		**LEAN CLAY WITH SAND (CL)**, brown, hard			13-21-27	7.6	97		76
7.5		**SILTY SAND (SM)**, brown, medium dense			5-9-9 N=18				
10					12-18-21	0.2	98		
		trace gravel, very dense			33-50/4"				
20.0		**POORLY GRADED SAND (SP)**, trace gravel, brown, medium dense			9-11-11 N=22				
25.0									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-25-2023

Boring Completed
10-25-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. SBH-02

Location: See Exploration Plan

Latitude: 33.6491° Longitude: -114.7212°

Graphic Log	Depth (Ft.)	Material Description	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
		SILTY SAND (SM), trace gravel, tannish gray very dense			6-33-50/2"				
	30	dense			11-17-15 N=32				14
	35				11-22-28 N=50				
	40	very dense			18-43-50/2"				
	45				15-31-27 N=58				
	50								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-25-2023

Boring Completed
10-25-2023

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Boring Log No. SBH-02

Graphic Log	Location: See Exploration Plan Latitude: 33.6491° Longitude: -114.7212° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	SILTY SAND (SM), trace gravel, tannish gray *(continued)* dense				16-24-25 N=49				
	51.5 ***Boring Terminated at 51.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Drill Rig
Track

Hammer Type
Automatic

Driller
2R Drilling

Logged by
CG

Boring Started
10-25-2023

Boring Completed
10-25-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-01

Graphic Log	Location: See Exploration Plan Latitude: 33.6661° Longitude: -114.7094° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	CLAYEY SAND, trace gravel, brown	5							
	10.0 ***Test Pit Terminated at 10 Feet***	10							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
TA

Test Pit Started
11-13-2023

Test Pit Completed
11-13-2023

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-02

Location: See Exploration Plan

Latitude: 33.6672° Longitude: -114.6995°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND WITH CLAY (SP-SC), brown	5				0.8			6
10.0	10							
Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-13-2023

Test Pit Completed
11-13-2023

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-03

Graphic Log	Location: See Exploration Plan Latitude: 33.6672° Longitude: -114.6924° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	CLAYEY SAND (SC), brown	5							
	10.0 ***Test Pit Terminated at 10 Feet***	10							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-13-2023

Test Pit Completed
11-13-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-04

Location: See Exploration Plan

Latitude: 33.6672° Longitude: -114.6852°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND (SP), trace clay, brown	5				0.6			4
10.0	10							
Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-13-2023

Test Pit Completed
11-13-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-05

Location: See Exploration Plan

Latitude: 33.6636° Longitude: -114.6960°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND (SP), trace clay and gravel, brown	5				0.9			4
10.0	10							
Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-13-2023

Test Pit Completed
11-13-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-06

Location: See Exploration Plan

Latitude: 33.6635° Longitude: -114.7068°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
CLAYEY SAND (SC), trace gravel, brown	0							
	5				2.4			18
10.0	10							
Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-14-2023

Test Pit Completed
11-14-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-07

Location: See Exploration Plan

Latitude: 33.6564° Longitude: -114.7075°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
CLAYEY SAND (SC), trace gravel, brown	0							
	5				3.4			12
10.0	10							
Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-14-2023

Test Pit Completed
11-14-2023

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-08

Location: See Exploration Plan

Latitude: 33.6600° Longitude: -114.6888°

Graphic Log	Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	POORLY GRADED SAND WITH CLAY (SP-SC), grayish brown trace gravel	5							9
	10.0 ***Test Pit Terminated at 10 Feet***	10							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-09-2023

Test Pit Completed
11-09-2023

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-09

Graphic Log	Location: See Exploration Plan Latitude: 33.6600° Longitude: -114.6779° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	POORLY GRADED SAND WITH SILT (SP-SM), reddish brown	5							9
	10.0 ***Test Pit Terminated at 10 Feet***	10							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-08-2023

Test Pit Completed
11-08-2023

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-10

Graphic Log	Location: See Exploration Plan Latitude: 33.6565° Longitude: -114.6743° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	CLAYEY SAND (SC), brown	5							
	10.0 ***Test Pit Terminated at 10 Feet***	10							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-08-2023

Test Pit Completed
11-08-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-11

Location: See Exploration Plan

Latitude: 33.6564° Longitude: -114.6814°

Graphic Log	Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	POORLY GRADED SAND WITH CLAY (SP-SC), brown								
	10.0	5							11
	Test Pit Terminated at 10 Feet	10							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-09-2023

Test Pit Completed
11-09-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-12

Location: See Exploration Plan

Latitude: 33.6563° Longitude: -114.6901°

Graphic Log	Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	POORLY GRADED SAND WITH CLAY (SP-SC), brown								
	10.0	5							5
	Test Pit Terminated at 10 Feet	10							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-09-2023

Test Pit Completed
11-09-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-13

Location: See Exploration Plan

Latitude: 33.6528° Longitude: -114.6700°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND (SP), trace clay, olive brown	0							
	5							2
10.0	10							
Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-08-2023

Test Pit Completed
11-08-2023

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-14

Location: See Exploration Plan

Latitude: 33.6492° Longitude: -114.6801°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND (SP), trace gravel and clay, grayish brown	0							
	5							5
10.0	10							
Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-09-2023

Test Pit Completed
11-09-2023

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-15

Location: See Exploration Plan

Latitude: 33.6492° Longitude: -114.6901°

Graphic Log	Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	POORLY GRADED SAND WITH SILT (SP-SM), trace gravel, reddish brown								
		5							9
	10.0	10							
	Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-09-2023

Test Pit Completed
11-09-2023

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-16

Location: See Exploration Plan

Latitude: 33.6455° Longitude: -114.6798°

Graphic Log	Depth (Ft.)	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
		SILTY SAND (SM), trace gravel, grayish brown								
	4.0									
		CLAYEY SAND (SC), trace gravel, grayish brown	5							
	10.0		10							
		Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-07-2023

Test Pit Completed
11-07-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-17

Location: See Exploration Plan

Latitude: 33.6455° Longitude: -114.6699°

Graphic Log	Depth (Ft.)	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	0	**SILTY SAND (SM)**, trace gravel, grayish brown								
	2.0	**SILTY CLAYEY SAND (SC-SM)**, trace gravel, grayish brown	5							53
	10.0	***Test Pit Terminated at 10 Feet***	10							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-08-2023

Test Pit Completed
11-08-2023

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-18

Graphic Log	Location: See Exploration Plan Latitude: 33.6435° Longitude: -114.6650° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	SILTY SAND (SM), trace gravel, grayish brown								
	reddish brown	5							27
	10.0	10							
	Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-08-2023

Test Pit Completed
11-08-2023

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-19

Location: See Exploration Plan

Latitude: 33.6328° Longitude: -114.6950°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND (SP), trace clay and gravel, light brown	5				0.5			4
10.0	10							
Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-02-2023

Test Pit Completed
11-02-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-20

Location: See Exploration Plan

Latitude: 33.6384° Longitude: -114.6951°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
CLAYEY SAND (SM), trace gravel, light brown								
4.0								
POORLY GRADED SAND WITH SILT (SP-SM), trace gravel and clay, brown	5				0.6			8
10.0								
Test Pit Terminated at 10 Feet	10							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-03-2023

Test Pit Completed
11-03-2023

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-21

Graphic Log	Location: See Exploration Plan Latitude: 33.6411° Longitude: -114.7110° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	SILTY SAND (SM), brown	5							
	10.0	10							
	Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-02-2023

Test Pit Completed
11-02-2023

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-22

Location: See Exploration Plan

Latitude: 33.6455° Longitude: -114.7150°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND WITH CLAY (SP-SC), trace gravel, light brown to brown	5				0.9			6
10.0	10							
Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-02-2023

Test Pit Completed
11-02-2023

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-23

Location: See Exploration Plan

Latitude: 33.6491° Longitude: -114.7191°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND WITH CLAY (SP-SC), trace gravel, yellowish brown	0							
	5				1.0			8
10.0	10							
Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-14-2023

Test Pit Completed
11-14-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-24

Location: See Exploration Plan

Latitude: 33.6527° Longitude: -114.7150°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND (SP), trace clay, brown	5				4.0			5
10.0	10							
Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-14-2023

Test Pit Completed
11-14-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. TP-25

Location: See Exploration Plan

Latitude: 33.6527° Longitude: -114.7050°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
SILTY CLAYEY SAND (SC-SM), trace gravel, light brown	5				2.3			16
10.0	10							
Test Pit Terminated at 10 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-02-2023

Test Pit Completed
11-02-2023

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

Terracon
1355 E Cooley Dr
Colton, CA

Test Pit Log No. TP-26

Location: See Exploration Plan

Latitude: 33.6590° Longitude: -114.6994°

Depth (Ft.) Approximate Elevation: 402 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND (SP), light brown, trace silt and gravel, light to moderate cementation	0							
	5							4.4
6.0 396								
SILTY SAND (SM), light brown, trace gravel, light to moderate cementation								
10.0 392	10							
Test Pit Terminated at 10 Feet								





See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Operator
Lourenco Backhoe

Notes

Advancement Method
Bucket

Abandonment Method
Test Pit backfilled with soil spoils upon completion

Logged by
AM

Test Pit Started
11-20-2025

Test Pit Completed
11-20-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Test Pit Log No. TP-27

Location: See Exploration Plan

Latitude: 33.6571° Longitude: -114.6973°

Depth (Ft.) Approximate Elevation: 401 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND (SP), light brown, trace gravel, light to moderate cementation	0							
	5							
10.0 391	10							
Test Pit Terminated at 10 Feet								





See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Operator
Lourenco Backhoe

Notes

Advancement Method
Bucket

Abandonment Method
Test Pit backfilled with soil spoils upon completion

Logged by
AM

Test Pit Started
11-21-2025

Test Pit Completed
11-21-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Test Pit Log No. TP-28

Location: See Exploration Plan

Latitude: 33.6550° Longitude: -114.6951°

Depth (Ft.) Approximate Elevation: 400 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
WELL GRADED SAND WITH SILT (SW), light brown, clumps caused by moisture	0							6.2
10.0 390	5							
Test Pit Terminated at 10 Feet	10							





See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Operator
Lourenco Backhoe

Notes

Advancement Method
Bucket

Abandonment Method
Test Pit backfilled with soil spoils upon completion

Logged by
AM

Test Pit Started
11-20-2025

Test Pit Completed
11-20-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

Terracon
1355 E Cooley Dr
Colton, CA

Test Pit Log No. TP-29

Location: See Exploration Plan

Latitude: 33.6525° Longitude: -114.6902°

Depth (Ft.) Approximate Elevation: 394 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND (SP), trace silt, light brown, light to moderate cementation	0							2.2
	5							
10.0 384	10							
Test Pit Terminated at 10 Feet								





See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Operator
Lourenco Backhoe

Notes

Advancement Method
Bucket

Abandonment Method
Test Pit backfilled with soil spoils upon completion

Logged by
AM

Test Pit Started
11-19-2025

Test Pit Completed
11-19-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

Terracon
1355 E Cooley Dr
Colton, CA

Test Pit Log No. TP-30

Location: See Exploration Plan

Latitude: 33.6515° Longitude: -114.6993°

Depth (Ft.) Approximate Elevation: 401 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	0							
POORLY GRADED SAND (SP), trace silt, trace gravel, light brown, light cementation								1.8
	5							
10.0 391	10							
Test Pit Terminated at 10 Feet								





See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Operator
Lourenco Backhoe

Notes

Advancement Method
Bucket

Logged by
AM

Abandonment Method
Test Pit backfilled with soil spoils upon completion

Test Pit Started
11-20-2025

Test Pit Completed
11-20-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

Terracon
1355 E Cooley Dr
Colton, CA

Test Pit Log No. TP-31

Location: See Exploration Plan

Latitude: 33.6481° Longitude: -114.7143°

Depth (Ft.) Approximate Elevation: 399 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND WITH SILT (SP), light brown, trace gravel, light cementation	0							
	5							11.8
10.0 389	10							
Test Pit Terminated at 10 Feet								





See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Operator
Lourenco Backhoe

Notes

Advancement Method
Bucket

Logged by
AM

Abandonment Method
Test Pit backfilled with soil spoils upon completion

Test Pit Started
11-20-2025

Test Pit Completed
11-20-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Test Pit Log No. TP-32

Location: See Exploration Plan

Latitude: 33.6491° Longitude: -114.7049°

Depth (Ft.) Approximate Elevation: 394 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND WITH SILT (SP), light brown, trace gravel	0							6.9
	5							
10.0 384	10							
Test Pit Terminated at 10 Feet								





See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Operator
Lourenco Backhoe

Notes

Advancement Method
Bucket

Logged by
AM

Abandonment Method
Test Pit backfilled with soil spoils upon completion

Test Pit Started
11-20-2025

Test Pit Completed
11-20-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Test Pit Log No. TP-33

Location: See Exploration Plan

Latitude: 33.6481° Longitude: -114.6949°

Depth (Ft.) Approximate Elevation: 397 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	0							
POORLY GRADED SAND (SP), light brown, trace gravel, clumps caused by moisture								1.5
	5							
one side of trench completely collapsed								
10.0 387	10							
Test Pit Terminated at 10 Feet								





See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Operator
Lourenco Backhoe

Notes

Advancement Method
Bucket

Logged by
AM

Abandonment Method
Test Pit backfilled with soil spoils upon completion

Test Pit Started
11-20-2025

Test Pit Completed
11-20-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

Terracon
1355 E Cooley Dr
Colton, CA

Test Pit Log No. TP-34

Location: See Exploration Plan

Latitude: 33.6474° Longitude: -114.6633°

Depth (Ft.) Approximate Elevation: 370 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND WITH SILT (SP-SM), light brown, trace gravel, sand in large chunks, light to moderate cementation	0							
	5							9.5
10.0 360	10							
Test Pit Terminated at 10 Feet								





See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Operator
Lourenco Backhoe

Notes

Advancement Method
Bucket

Abandonment Method
Test Pit backfilled with soil spoils upon completion

Logged by
AM

Test Pit Started
11-20-2025

Test Pit Completed
11-20-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. CB255143

1355 E Cooley Dr
Colton, CA

Test Pit Log No. TP-35

Location: See Exploration Plan

Latitude: 33.6353° Longitude: -114.6796°

Depth (Ft.) Approximate Elevation: 366 (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
POORLY GRADED SAND WITH SILT (SP), light brown, trace gravel, light to moderate cementation	0							6.0
	5							
10.0 356	10							
Test Pit Terminated at 10 Feet								





See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation Reference: Elevation obtained using Google Earth Pro

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Operator
Lourenco Backhoe

Notes

Advancement Method
Bucket

Logged by
AM

Abandonment Method
Test Pit backfilled with soil spoils upon completion

Test Pit Started
11-19-2025

Test Pit Completed
11-19-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Test Pit Log No. STP-1

Location: See Exploration Plan

Latitude: 33.6470° Longitude: -114.7222°

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
CLAYEY SAND (SC), light brown								
	4.0							
SILTY CLAY WITH SAND (CL-ML), light brown	5				10.9			79
	10.0							
Test Pit Terminated at 10 Feet	10							

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Logged by
CG

Test Pit Started
11-14-2023

Test Pit Completed
11-14-2023



STP-1



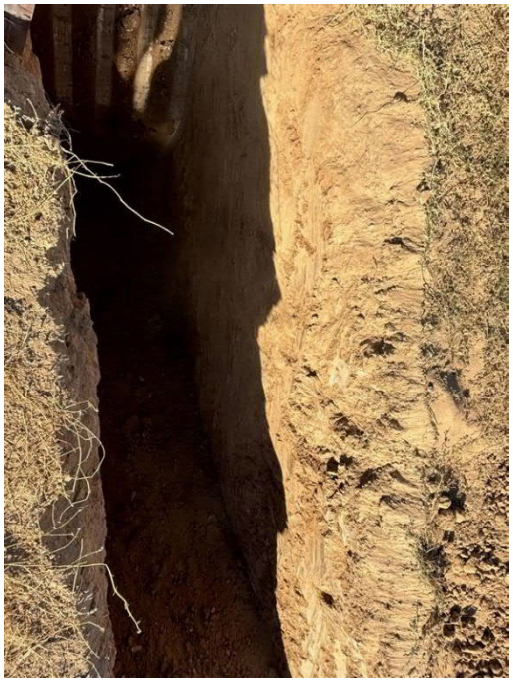
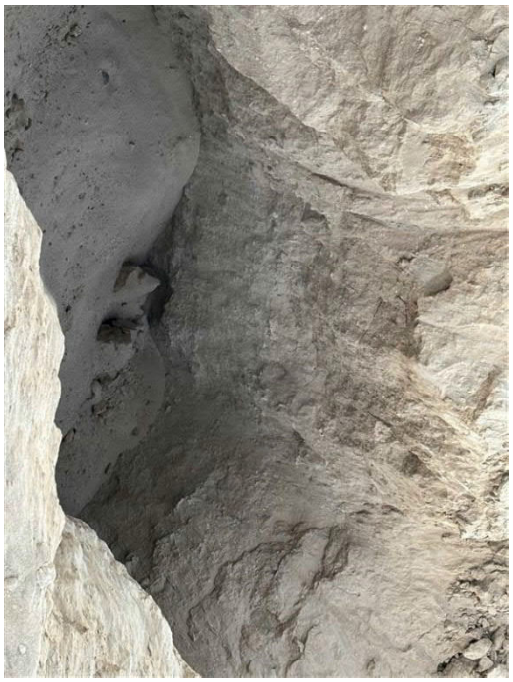
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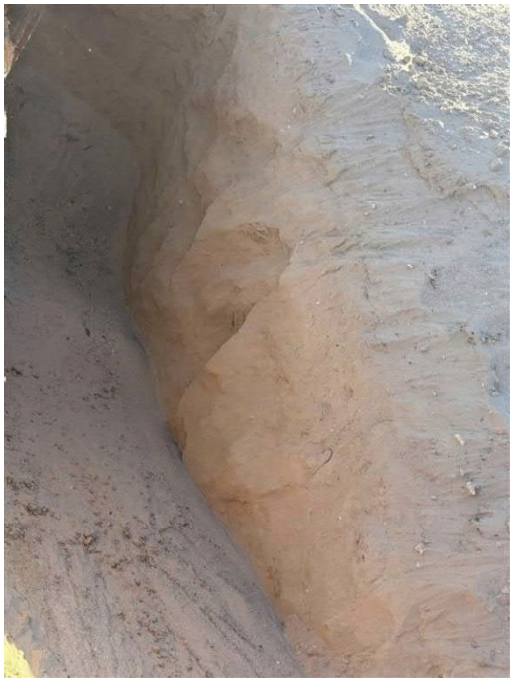
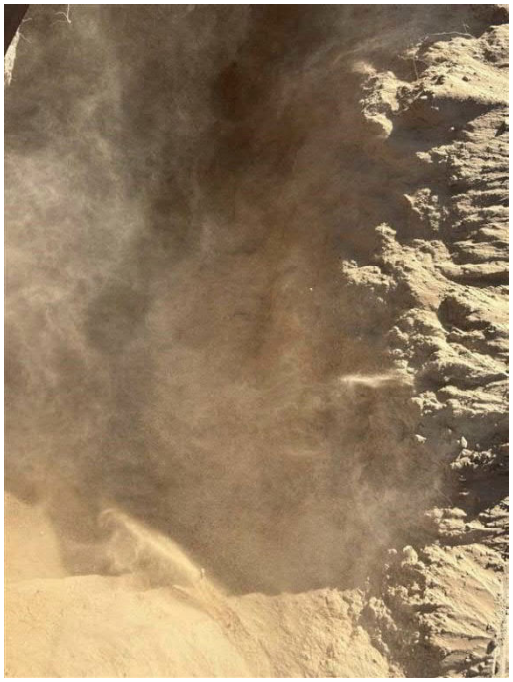
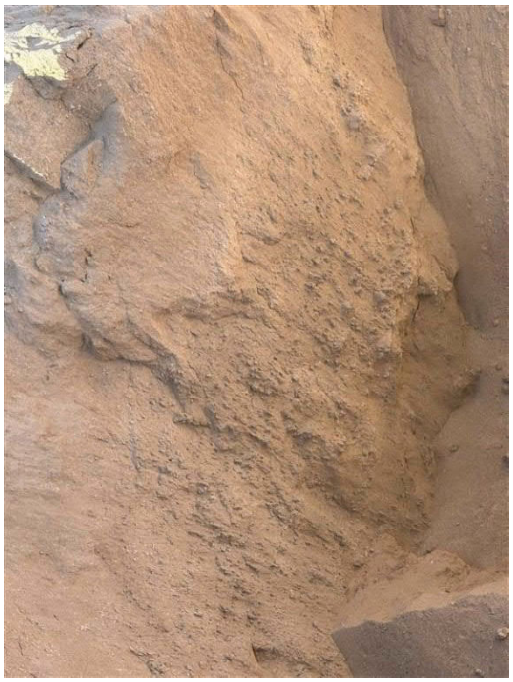


TP-2



TP-4

	
TP-5	**TP-6**
	
TP-7	**TP-8**

	
TP-9	**TP-10**
	
TP-13	**TP-15**



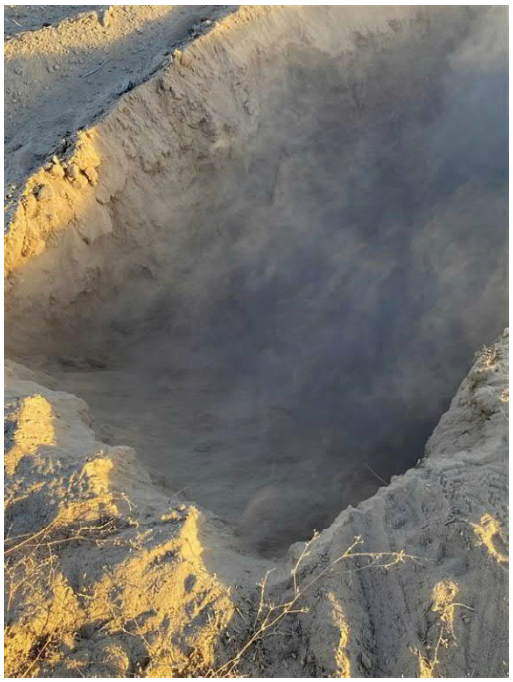
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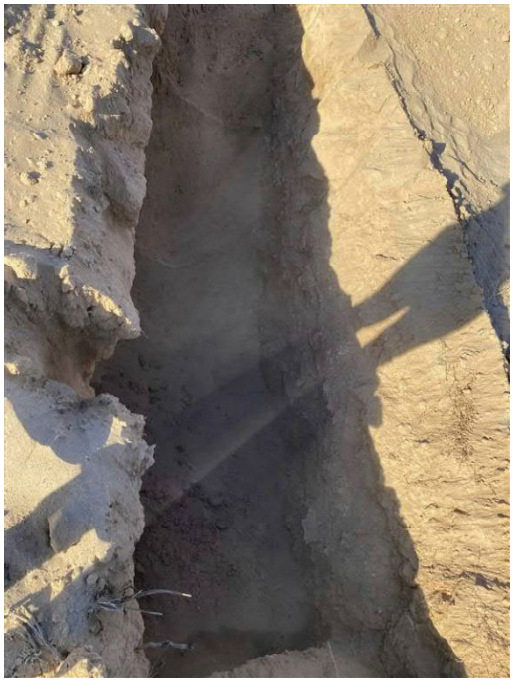
TP-17



TP-18



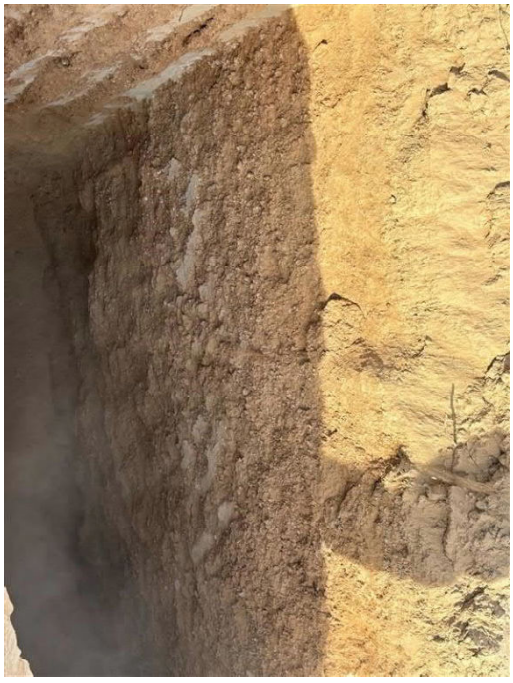
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TP-20



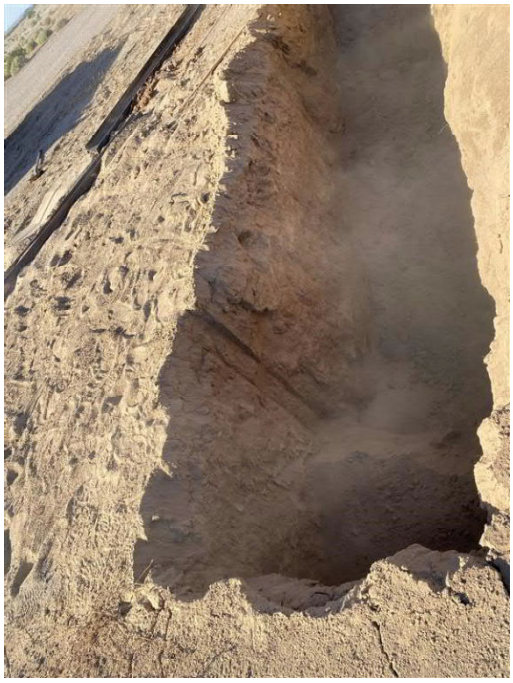
TP-22



TP-23



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Laboratory Test Results

Contents:

Laboratory Testing Procedures (2 pages)
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Grain Size Distribution (9 pages)
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Standard Proctor Test Results (22 pages)
Laboratory Thermal Resistivity Test Results (46 pages)
Laboratory Corrosion Series Test Results (10 pages)

Laboratory Testing Procedures

The project engineer reviewed the field data and assigned laboratory tests. The laboratory testing program included the following types of tests:

- ASTM D2216 Standard Test Methods for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass
- ASTM D7263 Standard Test Methods for Laboratory Determination of Dry Density (Unit Weight) of Soil Specimens
- ASTM D4318 Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils
- ASTM D1140 Standard Test Methods for Determining the Amount of Material Finer than 75-µm (No. 200) Sieve in Soils by Washing
- ASTM D422 Standard Test Methods for Particle-Size Analysis of Soils (including hydrometer)
- ASTM D3080 Standard Test Method for Direct Shear of Soils
- ASTM D698 Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort
- ASTM D4546 Standard Test Methods for Standard Test Methods for One-Dimensional Swell or Collapse of Soils

The laboratory testing program also included review of soil samples by an engineer. Based on the results of our field and laboratory programs, we described and classified the soil samples in accordance with the Unified Soil Classification System.

Thermal Resistivity: Thermal resistivity tests were analyzed by Geotherm USA. Terracon collected two bulk samples of subsurface materials obtained within proposed array and substation areas. Each bulk sample had a Standard Proctor test performed, and each bulk sample was tested for thermal resistivity tests on samples remolded to content to 90% (1 to 4 feet samples) and 85% (4 to 6 feet sample) of its respective Standard Proctor maximum dry density. A total of 20 locations were sampled within array areas and 1 location within the substation area. The percent compaction is based on comparing to the material's maximum dry density as determined by test method ASTM D698 (Standard Proctor). Tests included a minimum of 4 readings, including optimum moisture content or as-received moisture (whichever is higher), totally dry condition, and 2 intermediate moisture contents.

Corrosivity Testing: Bulk samples of near surface soils were tested in the laboratory for the following properties in general accordance with the corresponding standards:

- pH Analysis (ASTM G51)
- Chloride (ASTM D512)
- Sulfate (ASTM C1580)

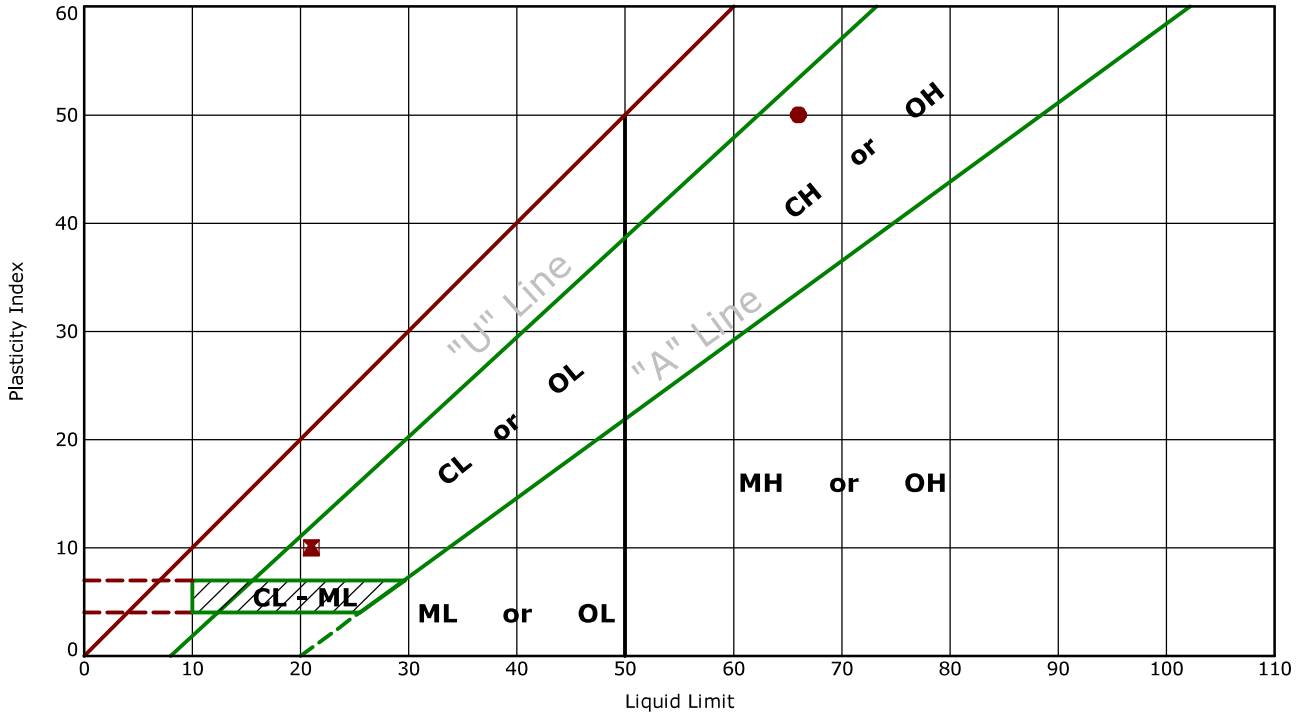
- Sulfide Content (AWWA 4500-S D)
- Oxidation-Reduction Potential (ASTM G200)
- Electrical Resistivity Testing (ASTM G187)

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Atterberg Limit Results

ASTM D4318

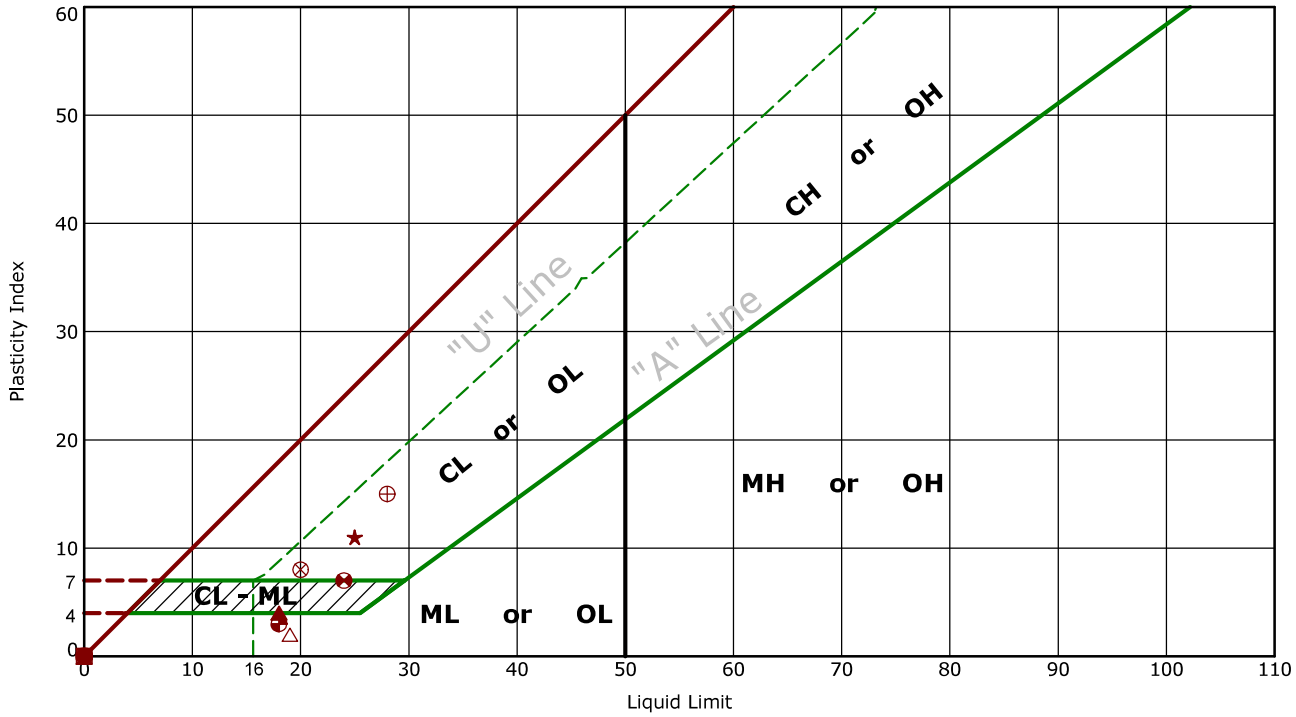


	Boring ID	Depth (Ft)	LL	PL	PI	Fines	USCS	Description
●	BH-43	15 - 16.5	66	16	50	70.2	CH	FAT CLAY WITH SAND
⊠	BH-44	1 - 4	21	11	10	19.5	SC	CLAYEY SAND
▲	SBH-02	1 - 4	21	11	10	37.3	SC	CLAYEY SAND

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Atterberg Limit Results

ASTM D4318



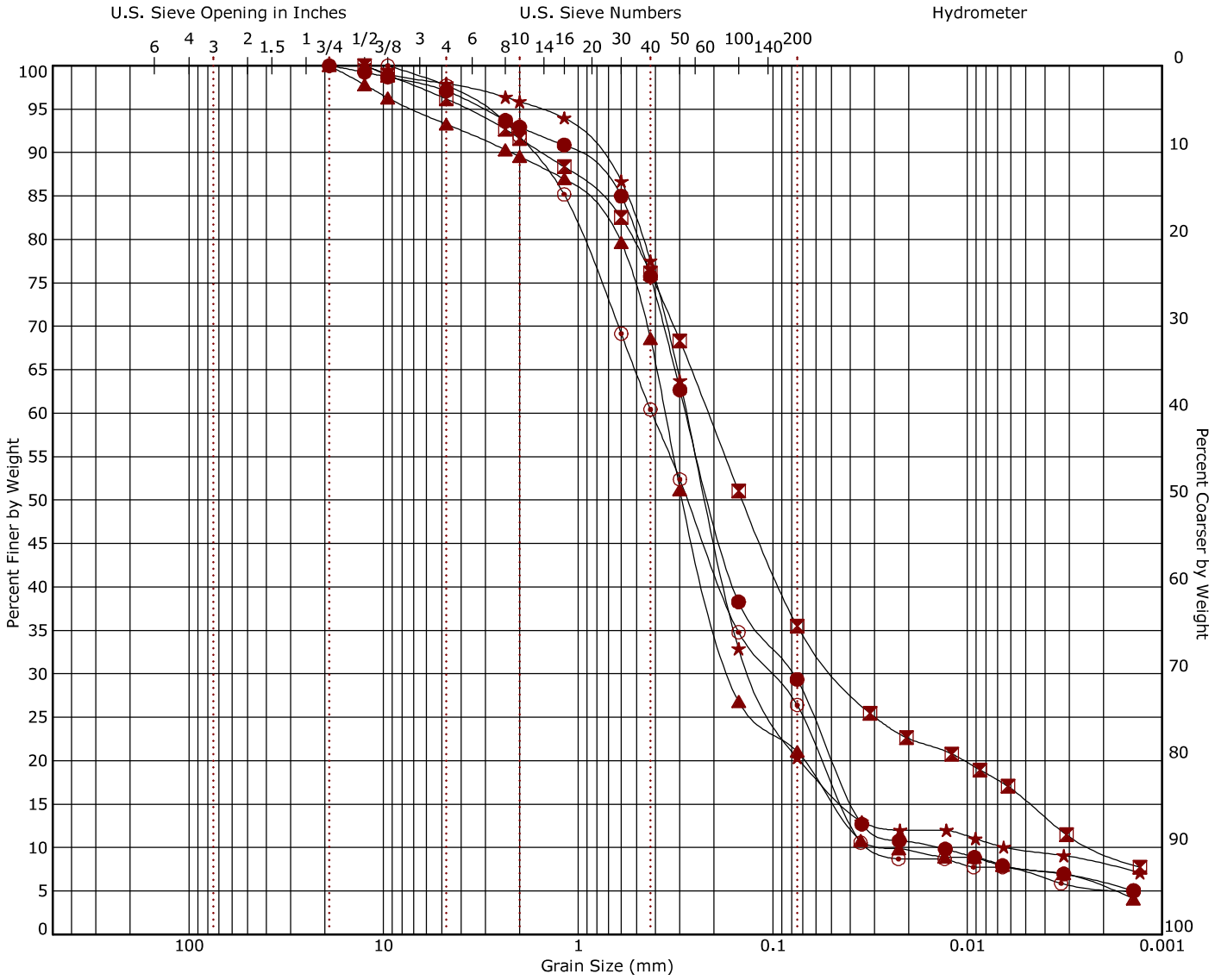
	Boring ID	Depth (Ft)	LL	PL	PI	Fines	USCS	Description
●	BH-50	0 - 5	NP	NP	NP	42.0	SM	SILTY SAND
⊠	BH-51	10 - 11.5	NP	NP	NP	34.0	SM	SILTY SAND
▲	BH-52	10 - 11.5	18	14	4	12.5	SC-SM	SILTY CLAYEY SAND
★	BH-53	0 - 5	25	14	11	50.3	CL	SANDY LEAN CLAY
⊙	BH-53	10 - 11.5	NP	NP	NP	2.8	SP	POORLY GRADED SAND
✿	BH-57	0 - 5	NP	NP	NP	19.0	SM	SILTY SAND
○	BH-58	0 - 5	NP	NP	NP	18.8	SM	SILTY SAND
△	BH-59	0 - 5	19	17	2	16.3	SM	SILTY SAND
⊗	BH-63	0 - 5	20	12	8	20.7	SC	CLAYEY SAND
⊕	BH-64	10 - 11.5	28	13	15	28.6	SC	CLAYEY SAND
□	BH-65	15 - 16.5	NP	NP	NP	31.7	SM	SILTY SAND
⊗	BH-66	7.5 - 9	24	17	7	68.0	CL-ML	SANDY SILTY CLAY
◒	BH-67	0 - 5	18	15	3	18.2	SM	SILTY SAND

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Grain Size Distribution

ASTM D422 / ASTM C136



Cobbles	Gravel		Sand			Silt or Clay
	coarse	fine	coarse	medium	fine	

Boring ID		Depth (Ft)	Description	USCS	LL	PL	PI	Cc	Cu
●	BH-01	1 - 4	SILTY SAND	SM				1.56	19.44
⊠	BH-03	1 - 4	SILTY SAND	SM				4.62	97.38
▲	BH-04	1 - 4	SILTY SAND	SM				3.16	15.03
★	BH-06	1 - 4	SILTY CLAYEY SAND	SC-SM				9.50	44.25
⊙	BH-09	1 - 4	SILTY SAND	SM				0.79	13.50

Boring ID		Depth (Ft)	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
●	BH-01	1 - 4	19	0.278	0.079	0.014	0.0	2.9	67.7		21.8	7.6
⊠	BH-03	1 - 4	12.5	0.215	0.047	0.002	0.0	3.8	60.7		20.2	15.3
▲	BH-04	1 - 4	19	0.358	0.164	0.024	0.0	6.7	72.2		13.5	7.6
★	BH-06	1 - 4	12.5	0.276	0.128	0.006	0.0	2.0	77.6		10.7	9.7
⊙	BH-09	1 - 4	9.5	0.417	0.101	0.031	0.0	2.3	71.3		19.4	7.0

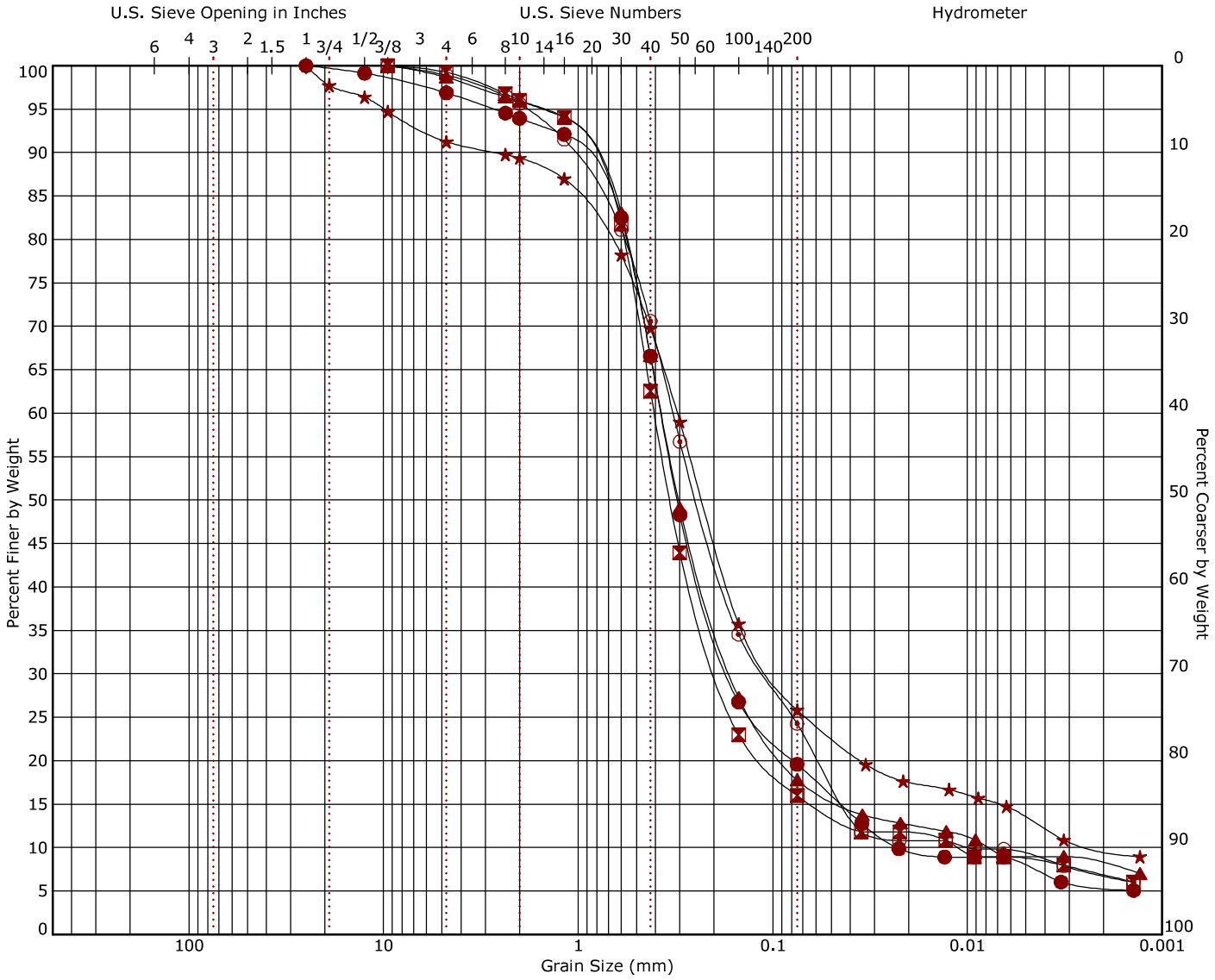
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Laguna Hills, CA

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Cobbles	Gravel		Sand			Silt or Clay
	coarse	fine	coarse	medium	fine	

Boring ID		Depth (Ft)	Description	USCS	LL	PL	PI	Cc	Cu
●	BH-11	1 - 4	SILTY SAND	SM				3.22	16.35
⊠	BH-12	1 - 4	CLAYEY SAND	SC				7.95	36.54
▲	BH-14	1 - 4	CLAYEY SAND	SC				9.20	47.81
★	BH-19	1 - 4	CLAYEY SAND	SC				15.35	146.56
⊙	BH-21	1 - 4	SILTY SAND	SM				3.82	33.28

Boring ID		Depth (Ft)	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
●	BH-11	1 - 4	25	0.375	0.166	0.023	0.0	3.1	77.3		11.8	7.8
⊠	BH-12	1 - 4	9.5	0.405	0.189	0.011	0.0	0.8	83.3		7.4	8.6
▲	BH-14	1 - 4	9.5	0.373	0.163	0.008	0.0	1.3	80.9		8.8	9.0
★	BH-19	1 - 4	25	0.31	0.1	0.002	0.0	8.8	65.4		12.4	13.4
⊙	BH-21	1 - 4	9.5	0.326	0.11	0.01	0.0	1.1	74.6		15.2	9.1

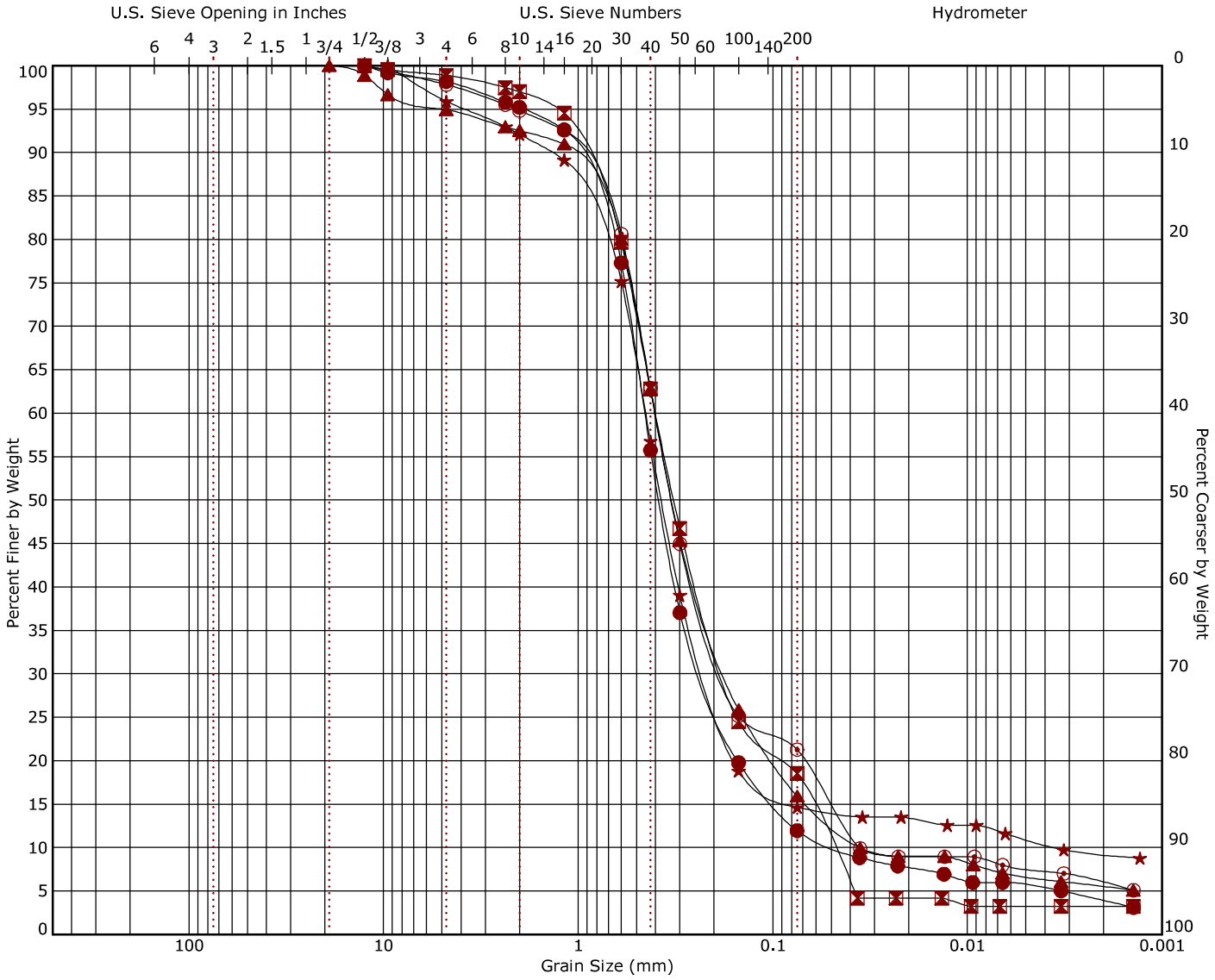
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Grain Size Distribution

ASTM D422 / ASTM C136



Cobbles	Gravel		Sand			Silt or Clay
	coarse	fine	coarse	medium	fine	

Boring ID		Depth (Ft)	Description	USCS	LL	PL	PI	Cc	Cu
●	BH-24	1 - 4	SILTY SAND	SM				2.38	9.65
⊠	BH-25	1 - 4	SILTY SAND	SM				1.61	8.15
▲	BH-27	1 - 4	SILTY SAND	SM				2.10	11.25
★	BH-29	1 - 4	SILTY SAND	SM				30.54	128.53
⊙	BH-31	1 - 4	SILTY SAND	SM				2.20	11.22

Boring ID		Depth (Ft)	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
●	BH-24	1 - 4	12.5	0.455	0.226	0.047	0.0	1.8	86.2		6.4	5.6
⊠	BH-25	1 - 4	12.5	0.4	0.178	0.049	0.0	1.1	80.4		15.3	3.2
▲	BH-27	1 - 4	19	0.402	0.174	0.036	0.0	5.1	79.0		9.3	6.7
★	BH-29	1 - 4	9.5	0.451	0.22	0.004	0.0	4.1	81.3		3.7	11.0
⊙	BH-31	1 - 4	12.5	0.402	0.178	0.036	0.0	2.2	76.5		13.7	7.6

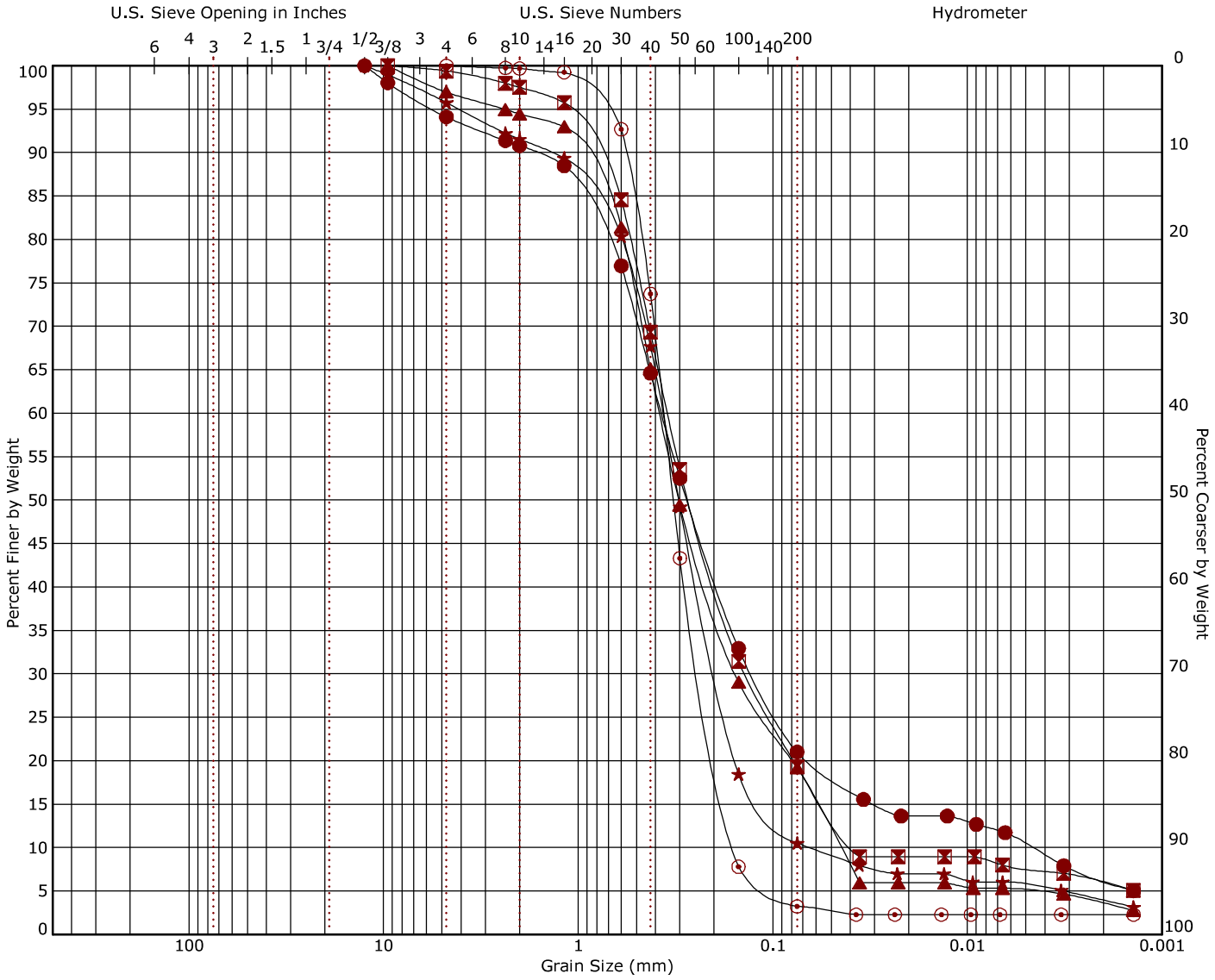
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Laguna Hills, CA

Grain Size Distribution

ASTM D422 / ASTM C136



Cobbles	Gravel		Sand			Silt or Clay
	coarse	fine	coarse	medium	fine	

Boring ID		Depth (Ft)	Description	USCS	LL	PL	PI	Cc	Cu
●	BH-33	1 - 4	SILTY SAND	SM				9.19	79.53
⊠	BH-35	1 - 4	SILTY SAND	SM				1.43	8.97
▲	BH-39	1 - 4	SILTY SAND	SM				1.41	8.45
★	BH-41	1 - 4	POORLY GRADED SAND WITH CLAY	SP-SC				1.60	5.71
⊙	BH-42	1 - 4	POORLY GRADED SAND	SP				0.94	2.32

Boring ID		Depth (Ft)	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
●	BH-33	1 - 4	12.5	0.372	0.127	0.005	0.0	5.9	73.1		10.6	10.4
⊠	BH-35	1 - 4	9.5	0.346	0.138	0.039	0.0	0.6	80.1		11.7	7.6
▲	BH-39	1 - 4	9.5	0.38	0.155	0.045	0.0	3.0	77.8		14.1	5.1
★	BH-41	1 - 4	12.5	0.367	0.195	0.064	0.0	4.2	85.2		4.9	5.7
⊙	BH-42	1 - 4	4.75	0.363	0.231	0.157	0.0	0.0	96.8		0.9	2.3

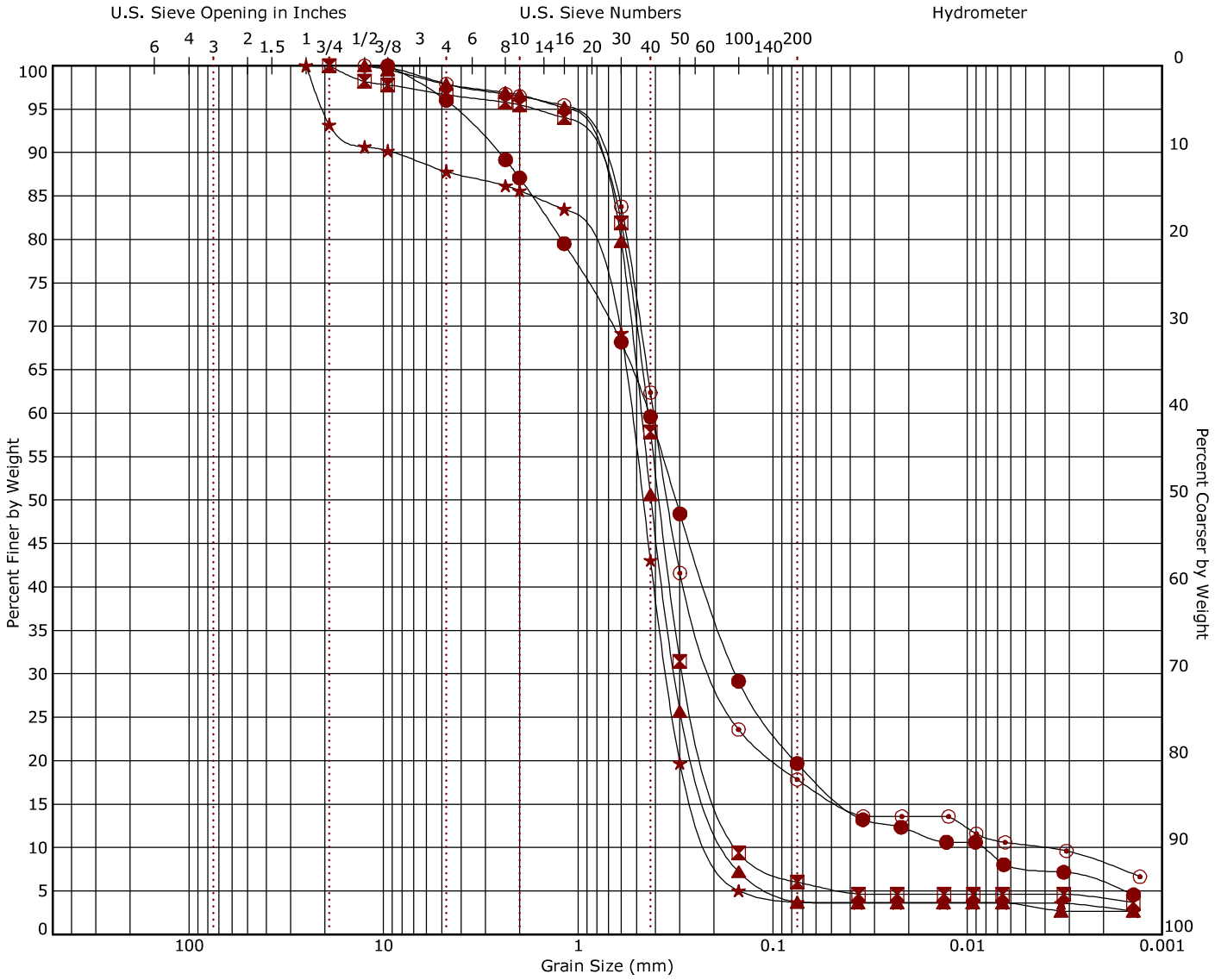
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ASTM D422 / ASTM C136



Cobbles	Gravel		Sand			Silt or Clay
	coarse	fine	coarse	medium	fine	

Boring ID	Depth (Ft)	Description	USCS	LL	PL	PI	Cc	Cu
● SBH-01	1 - 4	SILTY SAND	SM				6.59	51.44
⊠ TP-02	4 - 6	POORLY GRADED SAND WITH CLAY	SP-SC				1.23	2.87
▲ TP-04	4 - 6	POORLY GRADED SAND	SP				1.29	2.85
★ TP-05	4 - 6	POORLY GRADED SAND	SP				1.21	2.80
⊙ TP-06	4 - 6	CLAYEY SAND	SC				21.98	99.50

Boring ID	Depth (Ft)	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
● SBH-01	1 - 4	9.5	0.432	0.155	0.008	0.0	4.0	76.3		12.0	7.7
⊠ TP-02	4 - 6	19	0.438	0.287	0.153	0.0	3.3	90.6		1.4	4.7
▲ TP-04	4 - 6	12.5	0.475	0.319	0.166	0.0	2.1	94.2		0.4	3.3
★ TP-05	4 - 6	25	0.531	0.35	0.19	0.0	12.2	84.1		0.1	3.6
⊙ TP-06	4 - 6	12.5	0.408	0.192	0.004	0.0	2.1	80.1		7.6	10.3

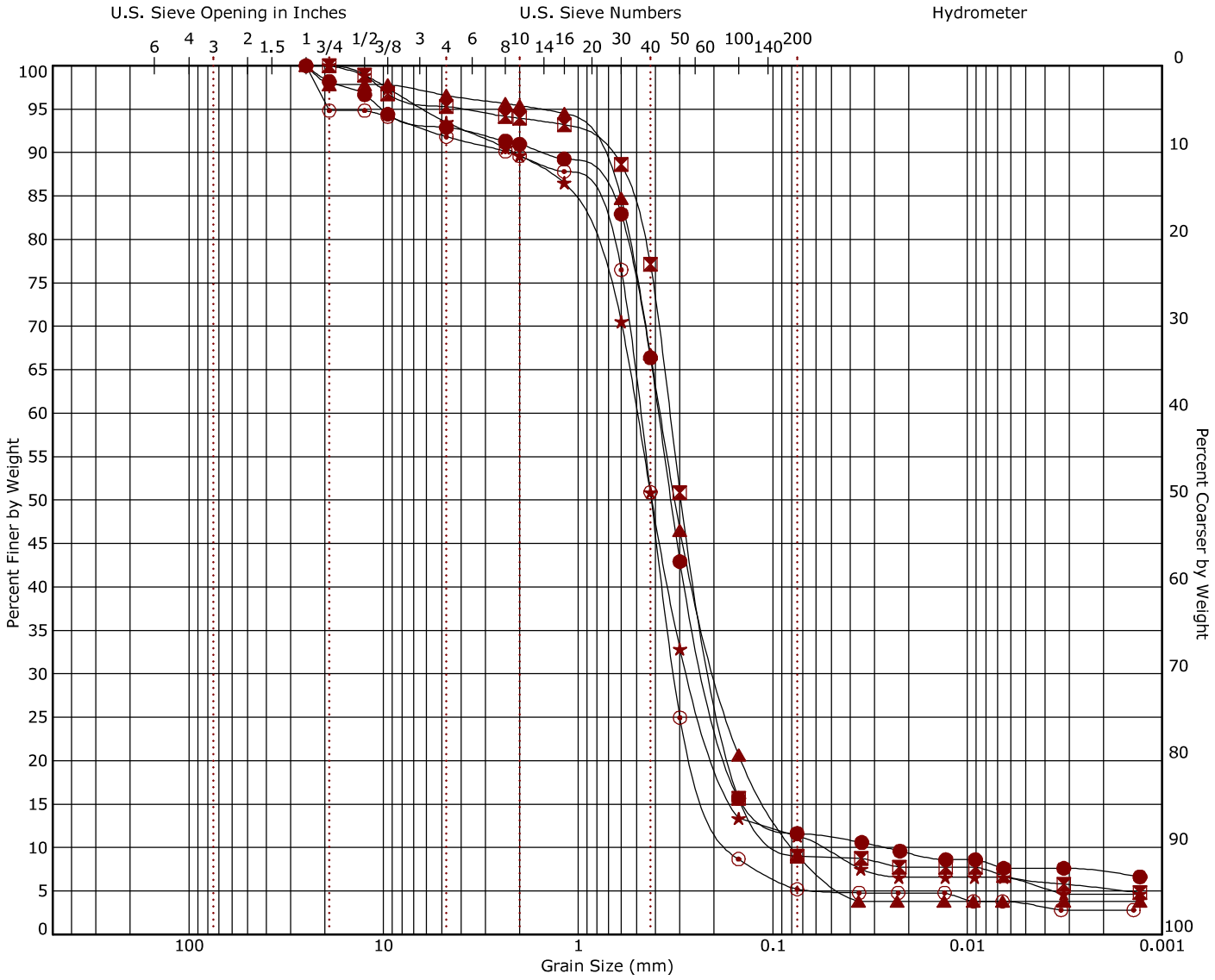
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Grain Size Distribution

ASTM D422 / ASTM C136



Cobbles	Gravel		Sand			Silt or Clay
	coarse	fine	coarse	medium	fine	

Boring ID		Depth (Ft)	Description	USCS	LL	PL	PI	Cc	Cu
●	TP-07	4 - 6	CLAYEY SAND	SC				4.52	14.48
⊠	TP-08	4 - 6	POORLY GRADED SAND WITH CLAY	SP-SC				1.40	4.07
▲	TP-09	4 - 6	POORLY GRADED SAND WITH SILT	SP-SM				1.25	4.80
★	TP-11	4 - 6	POORLY GRADED SAND WITH CLAY	SP-SC				2.56	8.67
⊙	TP-12	4 - 6	POORLY GRADED SAND WITH CLAY	SP-SC				1.35	3.03

Boring ID		Depth (Ft)	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
●	TP-07	4 - 6	25	0.387	0.216	0.027	0.0	7.1	81.3		4.0	7.6
⊠	TP-08	4 - 6	19	0.339	0.199	0.083	0.0	4.7	86.3		2.6	6.4
▲	TP-09	4 - 6	25	0.378	0.193	0.079	0.0	3.4	87.4		5.4	3.8
★	TP-11	4 - 6	19	0.499	0.271	0.058	0.0	6.5	82.1		5.5	5.9
⊙	TP-12	4 - 6	25	0.48	0.321	0.159	0.0	8.2	86.6		1.8	3.4

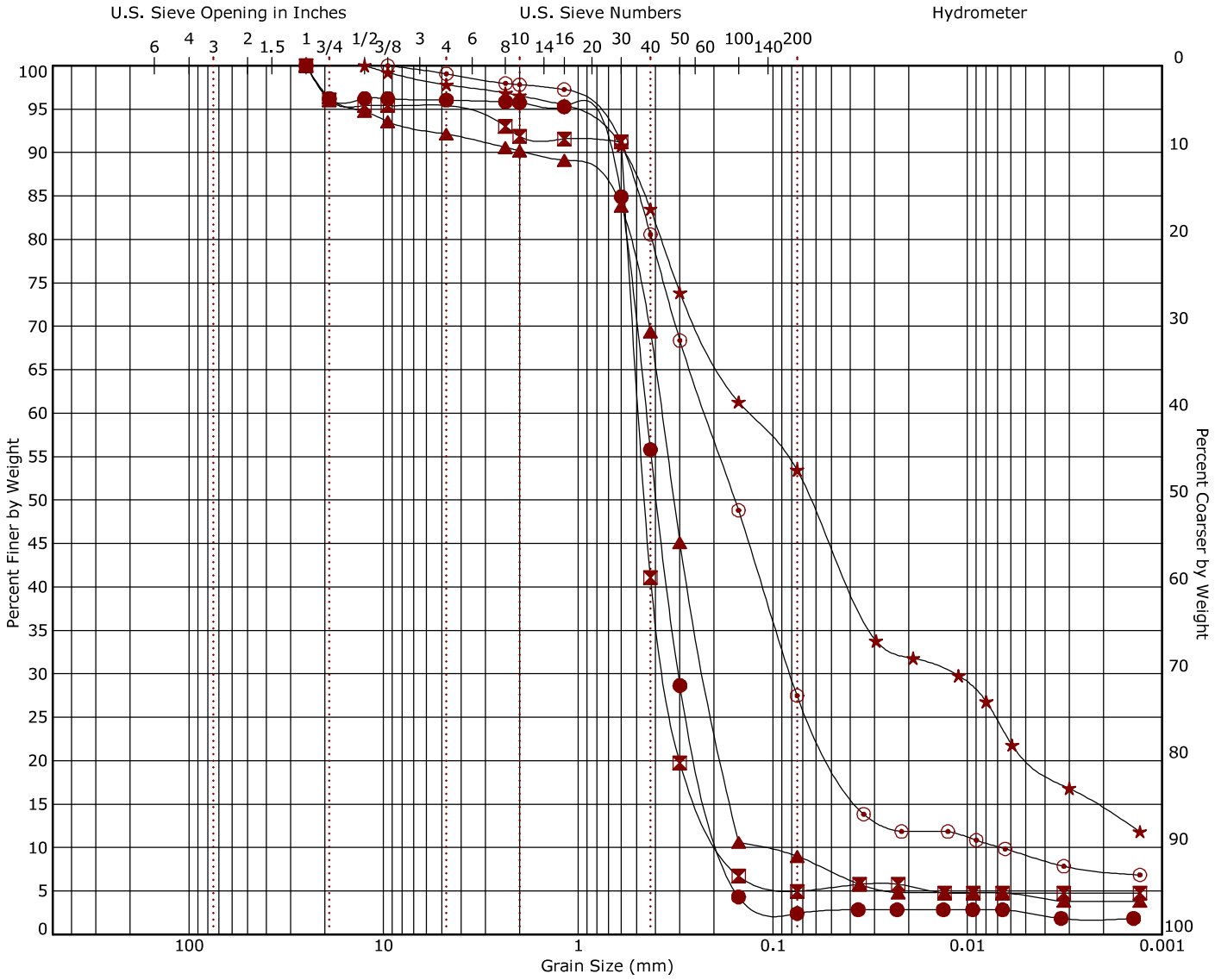
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Grain Size Distribution

ASTM D422 / ASTM C136



Cobbles	Gravel		Sand			Silt or Clay
	coarse	fine	coarse	medium	fine	

Boring ID		Depth (Ft)	Description	USCS	LL	PL	PI	Cc	Cu
●	TP-13	4 - 6	POORLY GRADED SAND	SP				1.18	2.53
⊠	TP-14	4 - 6	POORLY GRADED SAND	SP				1.45	2.71
▲	TP-15	4 - 6	POORLY GRADED SAND WITH SILT	SP-SM				1.14	3.22
★	TP-17	4 - 6	SILTY CLAYEY SAND	SC-SM					
⊙	TP-18	4 - 6	SILTY SAND	SM				4.41	33.14

Boring ID		Depth (Ft)	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
●	TP-13	4 - 6	25	0.447	0.305	0.176	0.0	4.0	93.6		0.0	2.4
⊠	TP-14	4 - 6	25	0.484	0.355	0.179	0.0	5.8	89.2		0.2	4.8
▲	TP-15	4 - 6	25	0.371	0.221	0.115	0.0	7.9	83.1		4.6	4.4
★	TP-17	4 - 6	12.5	0.134	0.012		0.0	2.2	44.3		32.9	20.6
⊙	TP-18	4 - 6	9.5	0.223	0.081	0.007	0.0	0.9	71.6		18.3	9.1

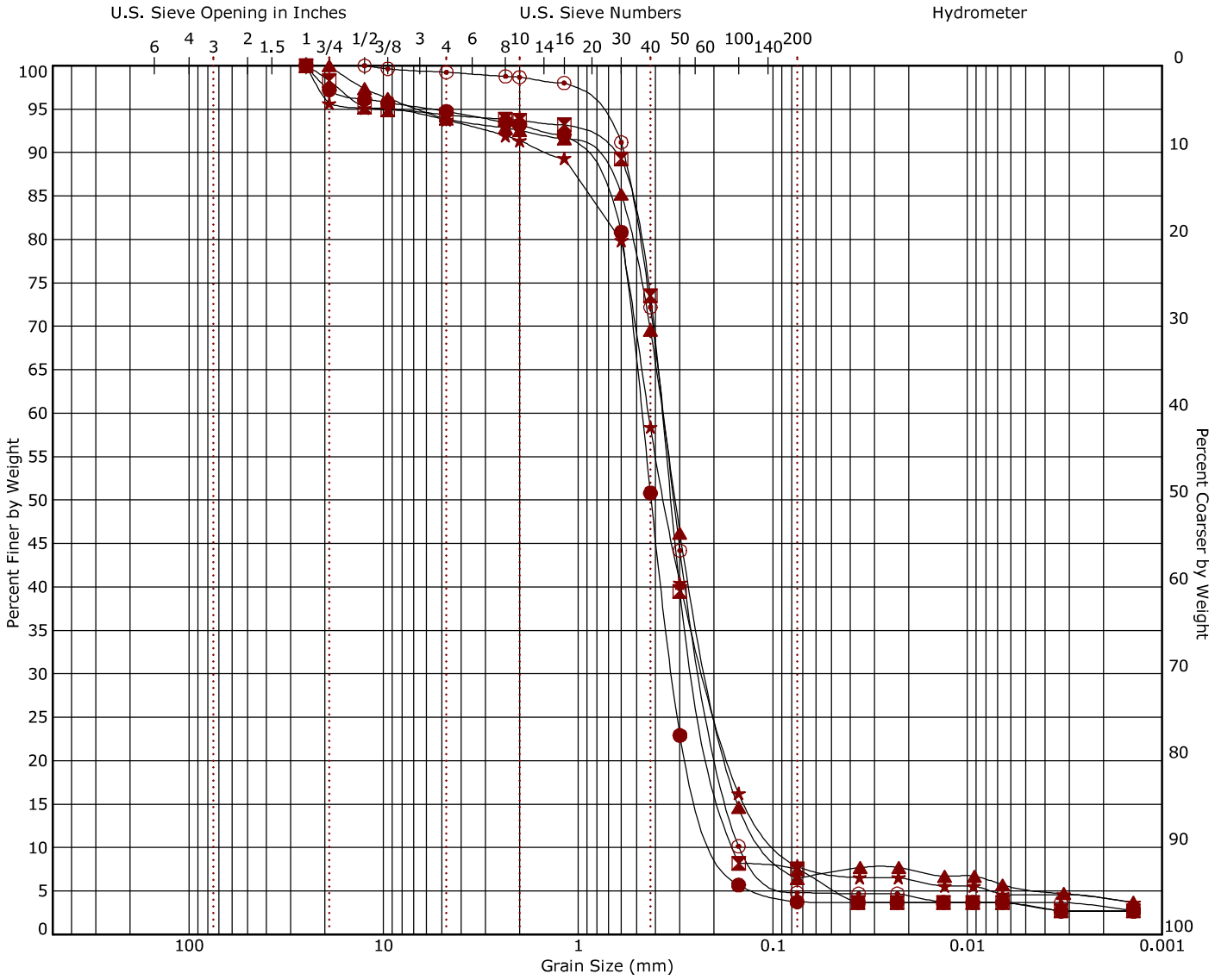
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Grain Size Distribution

ASTM D422 / ASTM C136



Cobbles	Gravel		Sand			Silt or Clay
	coarse	fine	coarse	medium	fine	

Boring ID		Depth (Ft)	Description	USCS	LL	PL	PI	Cc	Cu
●	TP-19	4 - 6	POORLY GRADED SAND	SP				1.27	2.65
⊠	TP-20	4 - 6	POORLY GRADED SAND WITH SILT	SP-SM				1.02	2.37
▲	TP-22	4 - 6	POORLY GRADED SAND WITH CLAY	SP-SC				2.09	6.43
★	TP-23	4 - 6	POORLY GRADED SAND WITH CLAY	SP-SC				1.27	4.86
⊙	TP-24	4 - 6	POORLY GRADED SAND	SP				0.94	2.48

Boring ID		Depth (Ft)	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
●	TP-19	4 - 6	25	0.472	0.328	0.178	0.0	5.3	91.0		0.5	3.3
⊠	TP-20	4 - 6	25	0.37	0.243	0.156	0.0	5.7	86.8		4.2	3.3
▲	TP-22	4 - 6	19	0.369	0.21	0.057	0.0	6.1	87.4		1.1	5.3
★	TP-23	4 - 6	25	0.436	0.222	0.09	0.0	6.3	85.9		3.2	4.6
⊙	TP-24	4 - 6	12.5	0.365	0.225	0.147	0.0	0.7	94.4		1.1	3.7

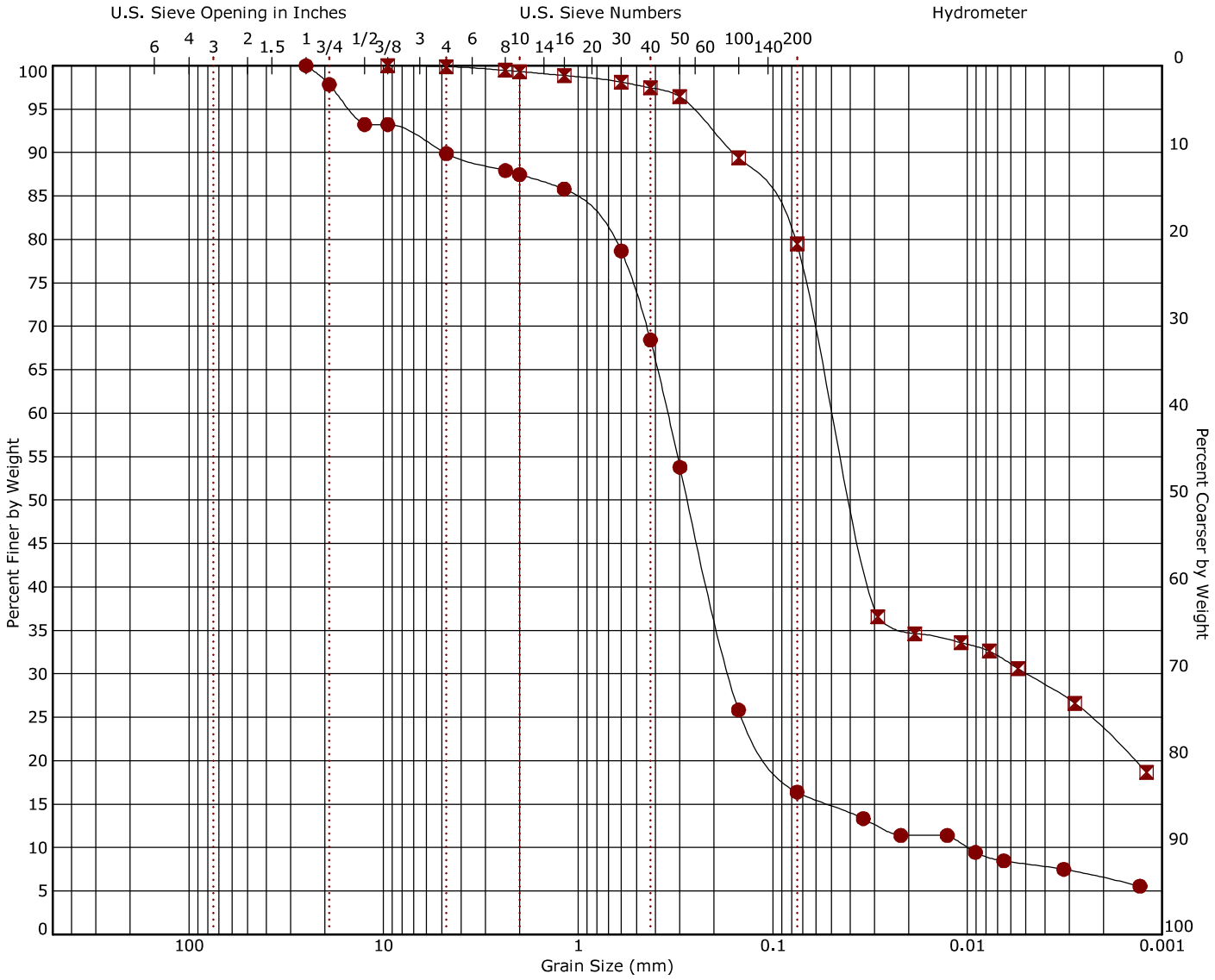
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ASTM D422 / ASTM C136



Cobbles	Gravel		Sand			Silt or Clay
	coarse	fine	coarse	medium	fine	

Boring ID	Depth (Ft)	Description	USCS	LL	PL	PI	Cc	Cu
TP-25	4 - 6	SILTY CLAYEY SAND	SC-SM				7.96	34.79
STP-1	4 - 6	SILTY CLAY WITH SAND	CL-ML					

Boring ID	Depth (Ft)	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
TP-25	4 - 6	25	0.348	0.166	0.01	0.0	10.1	73.5		8.3	8.1
STP-1	4 - 6	9.5	0.049	0.005		0.0	0.1	20.4		49.4	30.0

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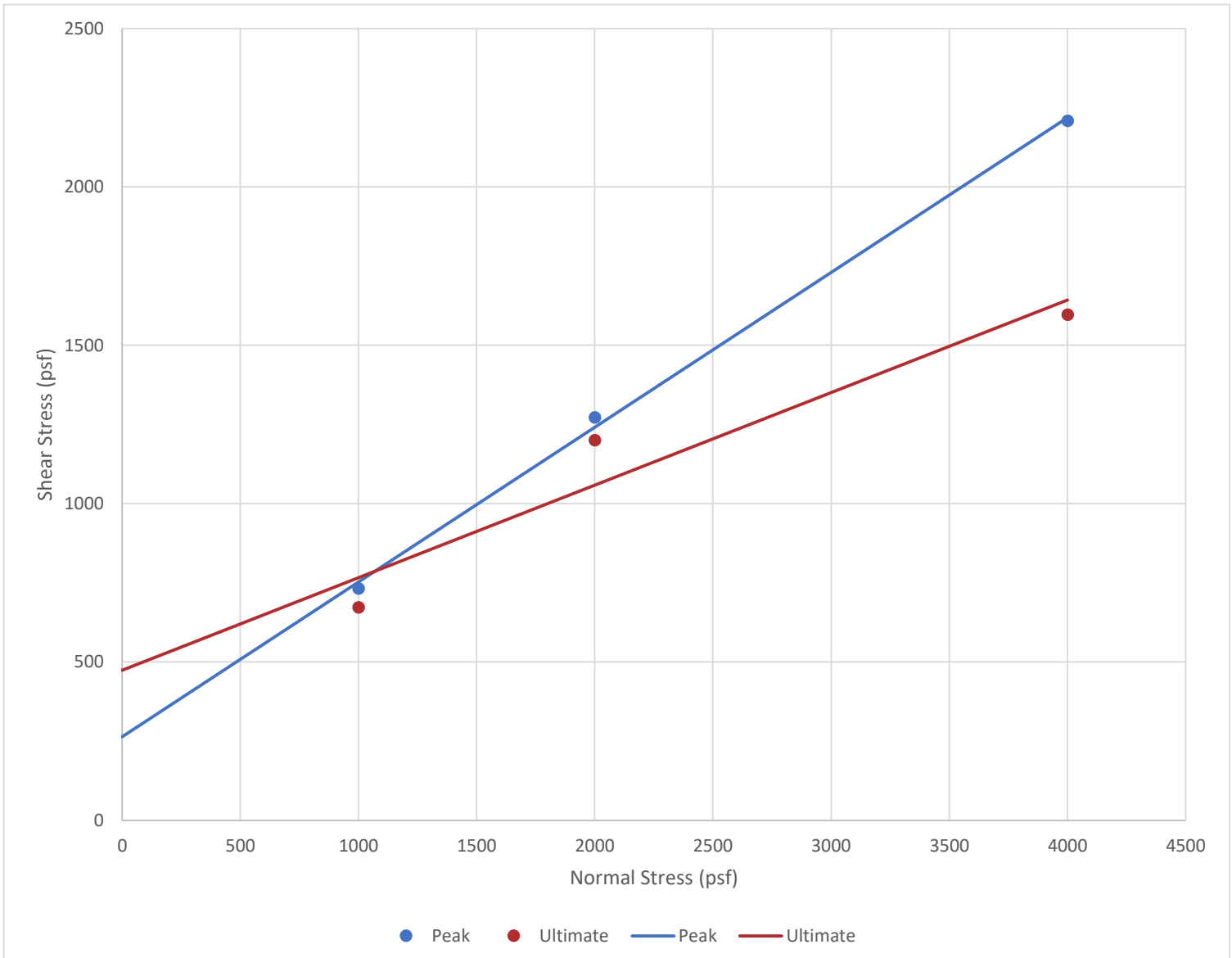
Terracon
145 W Walnut St
Carson, CA

Direct Shear Test

ASTM D3080

Boring ID	Depth	Description	USCS	γ_d (pcf)	W(%)
BH-20	5'-6.5'	Clayey Sand	SC	95	1.0

Normal Stress (psf)	Peak Shear Stress (psf)	Ultimate Shear Stress (psf)	Peak		Ultimate	
			φ°	C (psf)	φ°	C (psf)
1000	732	672	**26.0**	**260**	**16.0**	**470**
2000	1272	1200				
4000	2208	1596				



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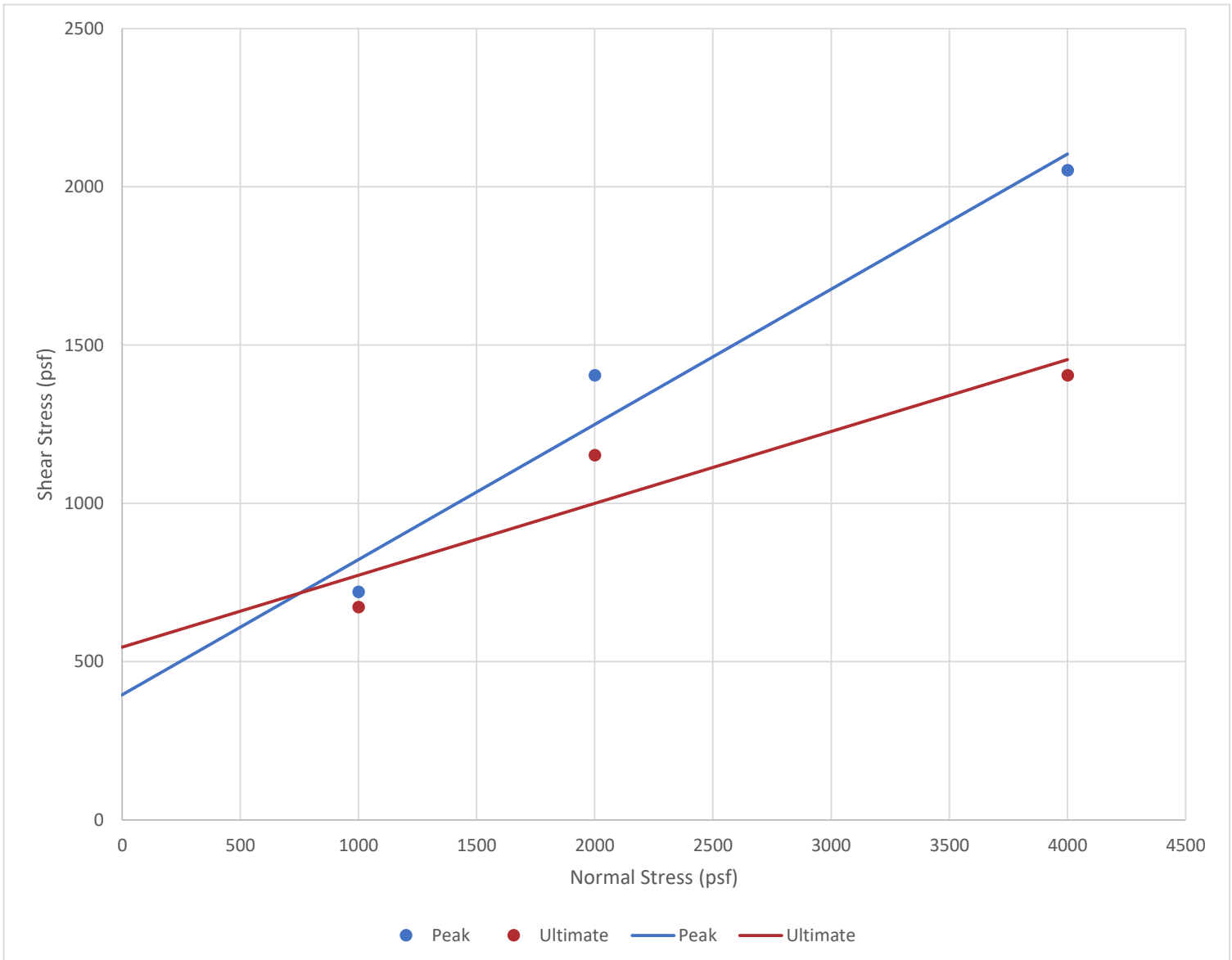
Terracon
145 W Walnut St
Carson, CA

Direct Shear Test

ASTM D3080

Boring ID	Depth	Description	USCS	γ_d (pcf)	W(%)
BH-28	5'-6.5'	Clayey Sand	SC	103	1.8

Normal Stress (psf)	Peak Shear Stress (psf)	Ultimate Shear Stress (psf)	Peak		Ultimate	
			φ°	C (psf)	φ°	C (psf)
1000	720	672	**23.0**	**400**	**13.0**	**550**
2000	1404	1152				
4000	2052	1404				



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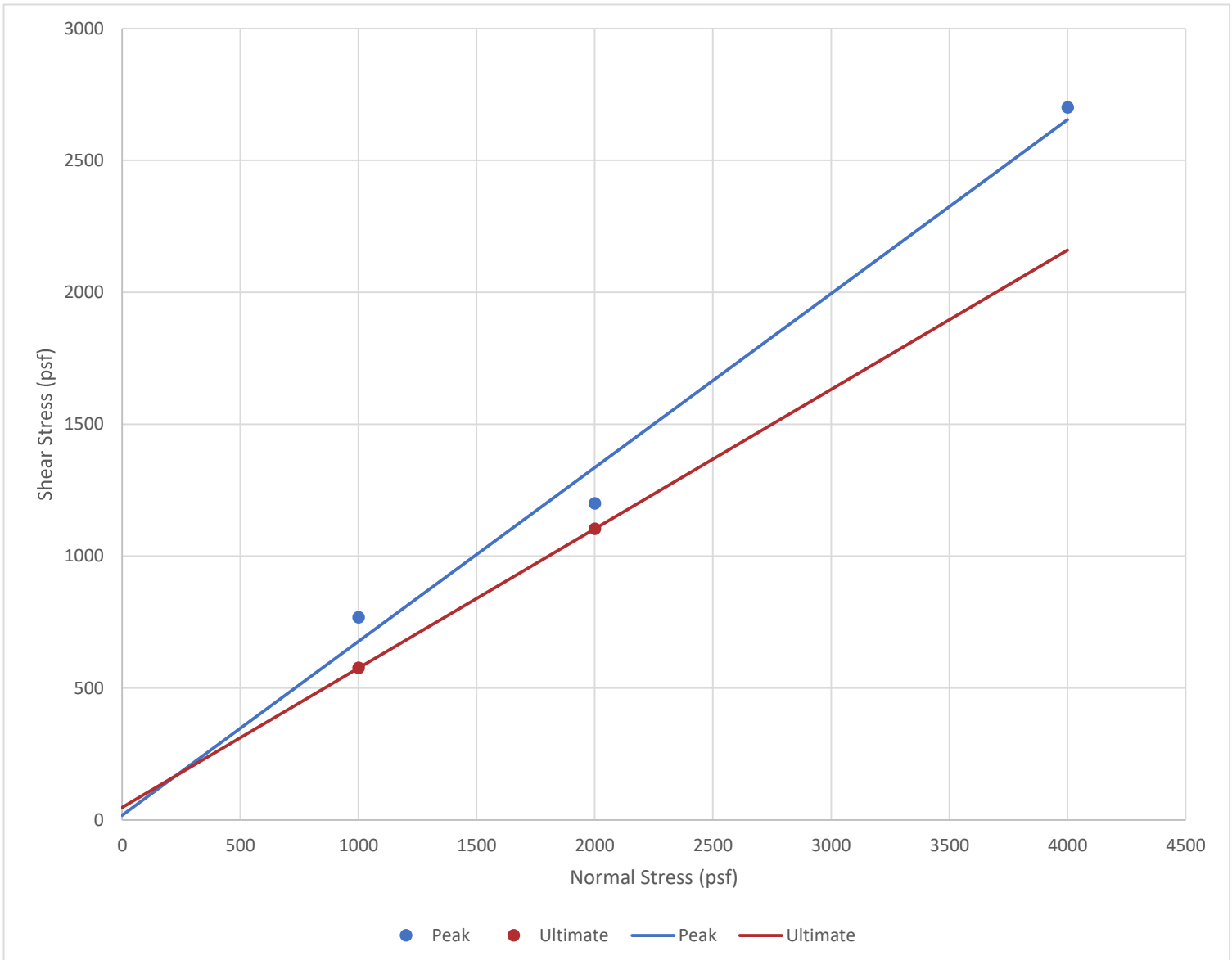
Terracon
145 W Walnut St
Carson, CA

Direct Shear Test

ASTM D3080

Boring ID	Depth	Description	USCS	γ_d (pcf)	W(%)
BH-40	10-11.5'	Well Graded Sand	SW	99	0.4

Normal Stress (psf)	Peak Shear Stress (psf)	Ultimate Shear Stress (psf)	Peak		Ultimate	
			φ°	C (psf)	φ°	C (psf)
1000	768	576	**33.0**	**20**	**28.0**	**50**
2000	1200	1104				
4000	2700					



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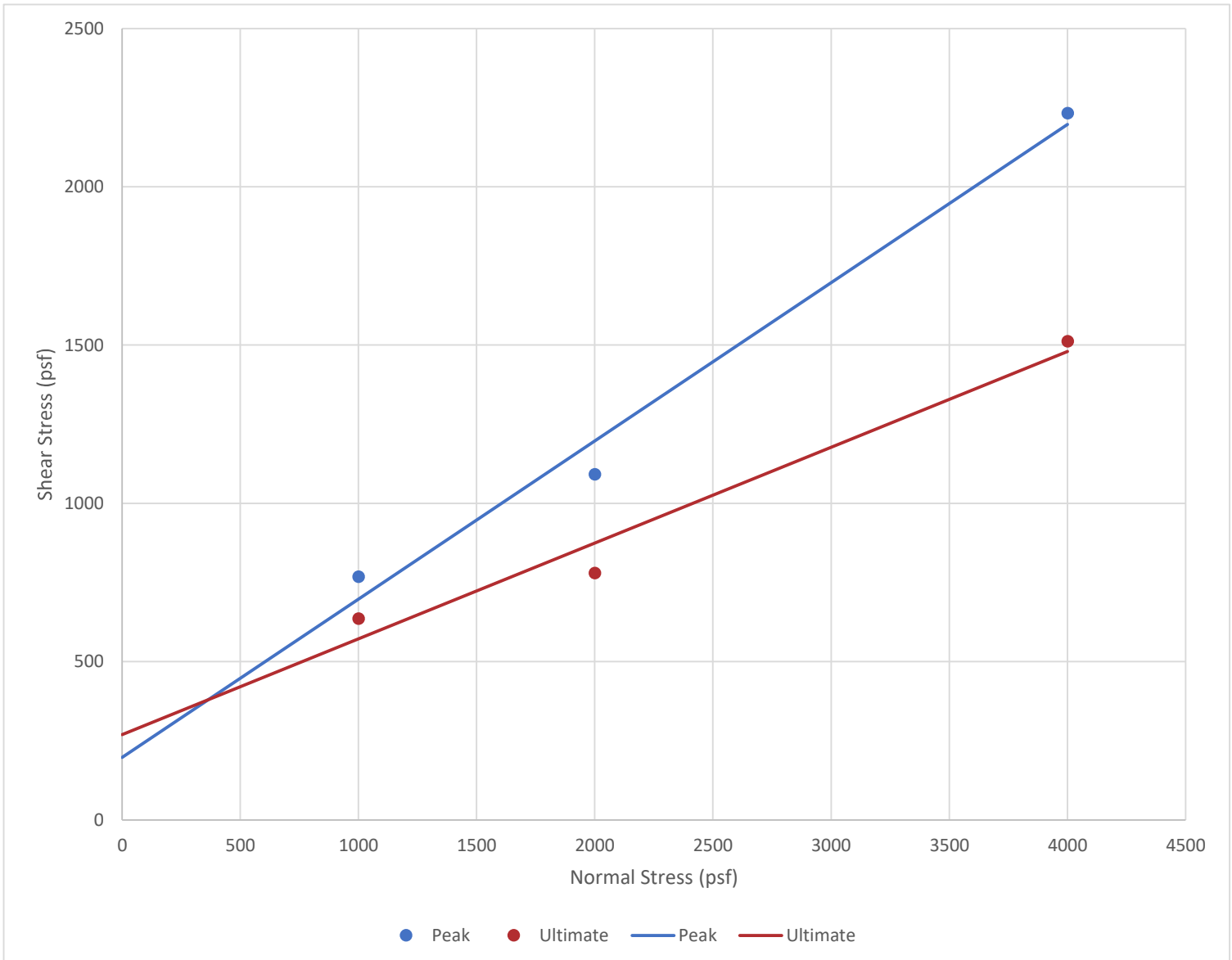
145 W Walnut St
Carson, CA

Direct Shear Test

ASTM D3080

Boring ID	Depth	Description	USCS	γ_d (pcf)	W(%)
BH-11	7.5-9'	Clayey Sand	SC	85	9.0

Normal Stress (psf)	Peak Shear Stress (psf)	Ultimate Shear Stress (psf)	Peak		Ultimate	
			φ°	C (psf)	φ°	C (psf)
1000	768	636	**26.6**	**200**	**16.8**	**270**
2000	1092	780				
4000	2232	1512				



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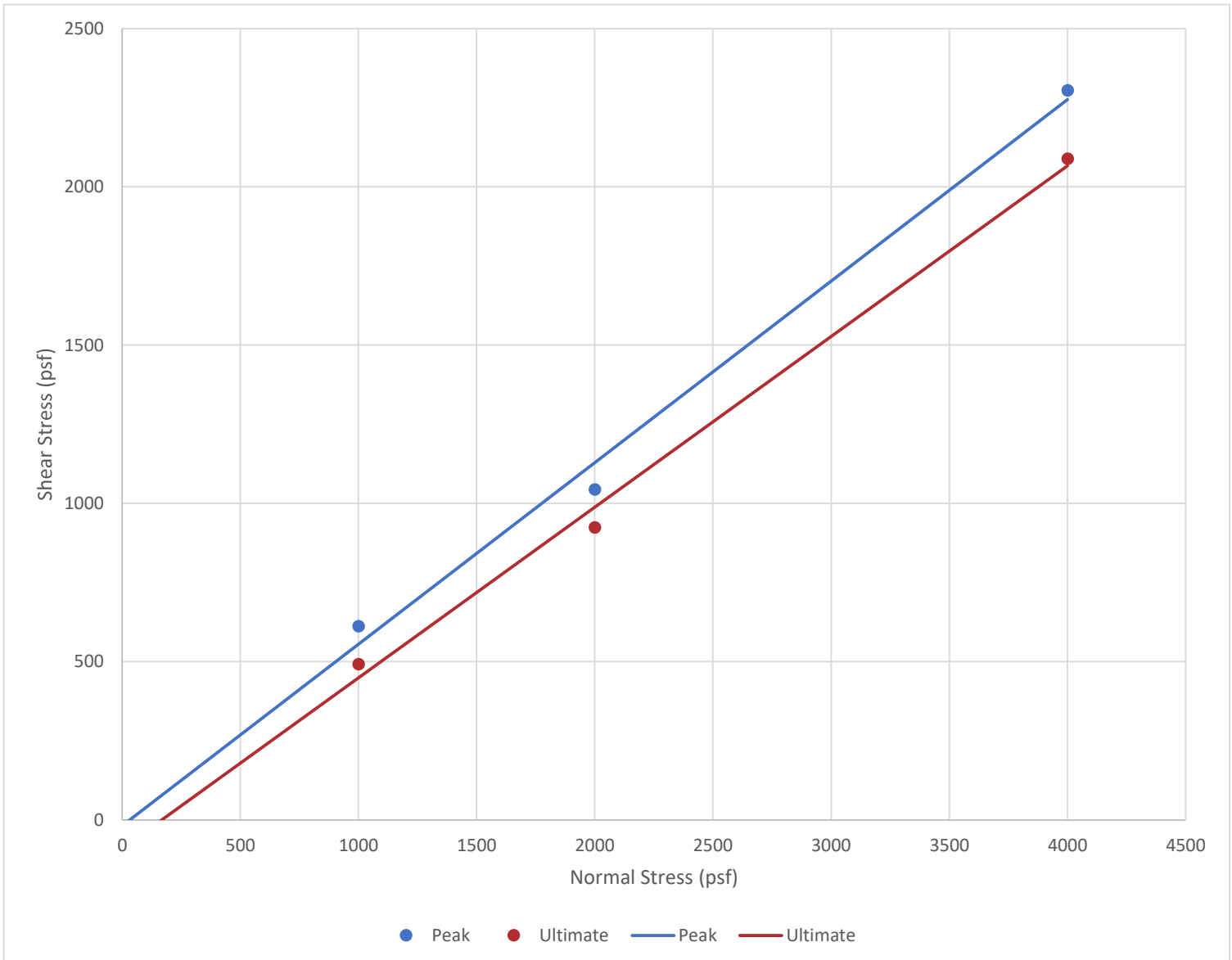
Terracon
145 W Walnut St
Carson, CA

Direct Shear Test

ASTM D3080

Boring ID	Depth	Description	USCS	γ_d (pcf)	W(%)
BH-19	2.5-4'	Silty Sand	SM	101	4.9

Normal Stress (psf)	Peak Shear Stress (psf)	Ultimate Shear Stress (psf)	Peak		Ultimate	
			φ°	C (psf)	φ°	C (psf)
1000	612	492	**30.0**	**0**	**28.0**	**0**
2000	1044	924				
4000	2304	2088				

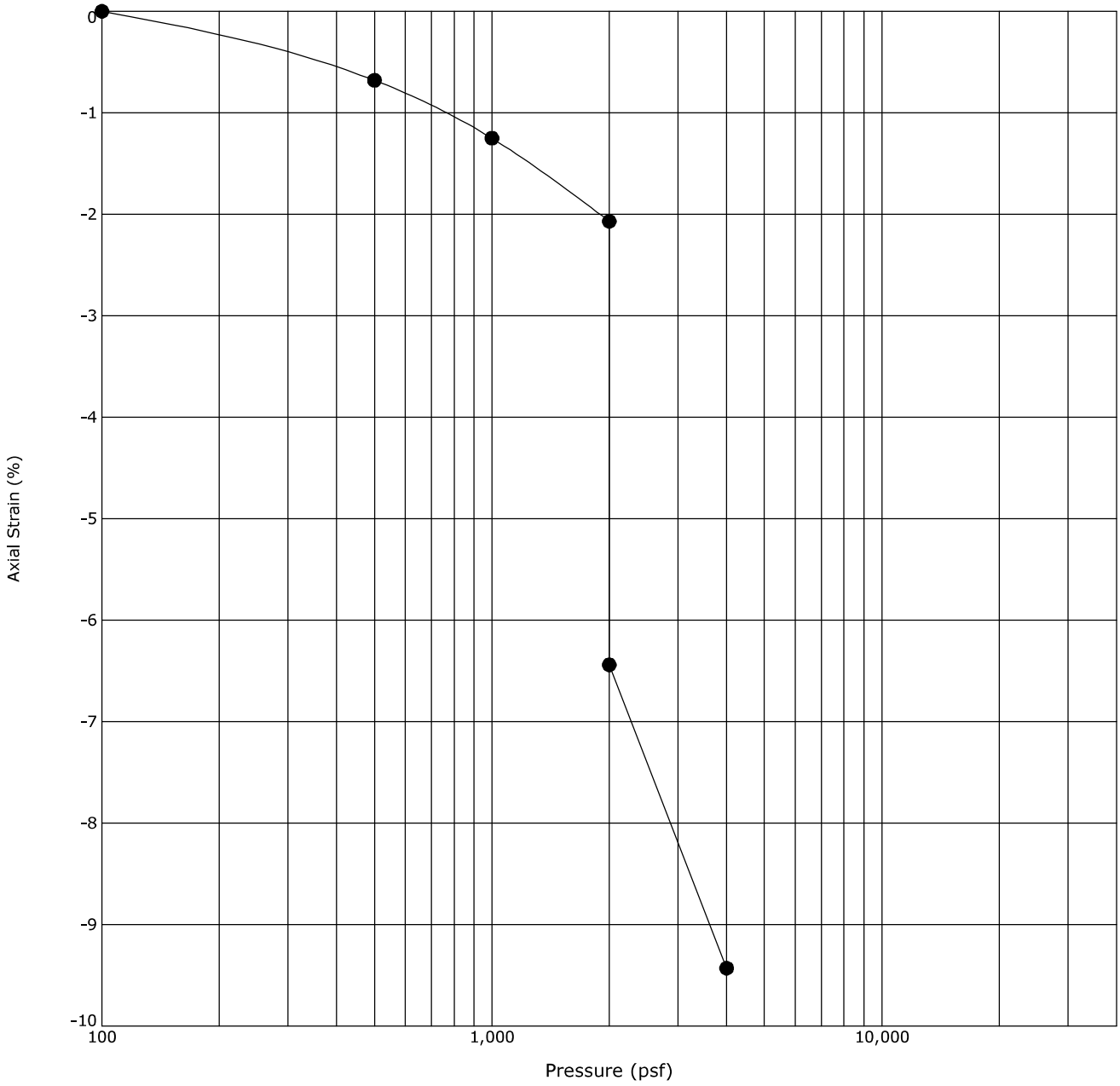


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Swell Consolidation Test

ASTM D2435



Boring ID		Depth (Ft)	Description	USCS	γ_d(pcf)	WC (%)
●	SBH-02	5 - 6.5	Lean Clay with Sand	CL	97	7.6

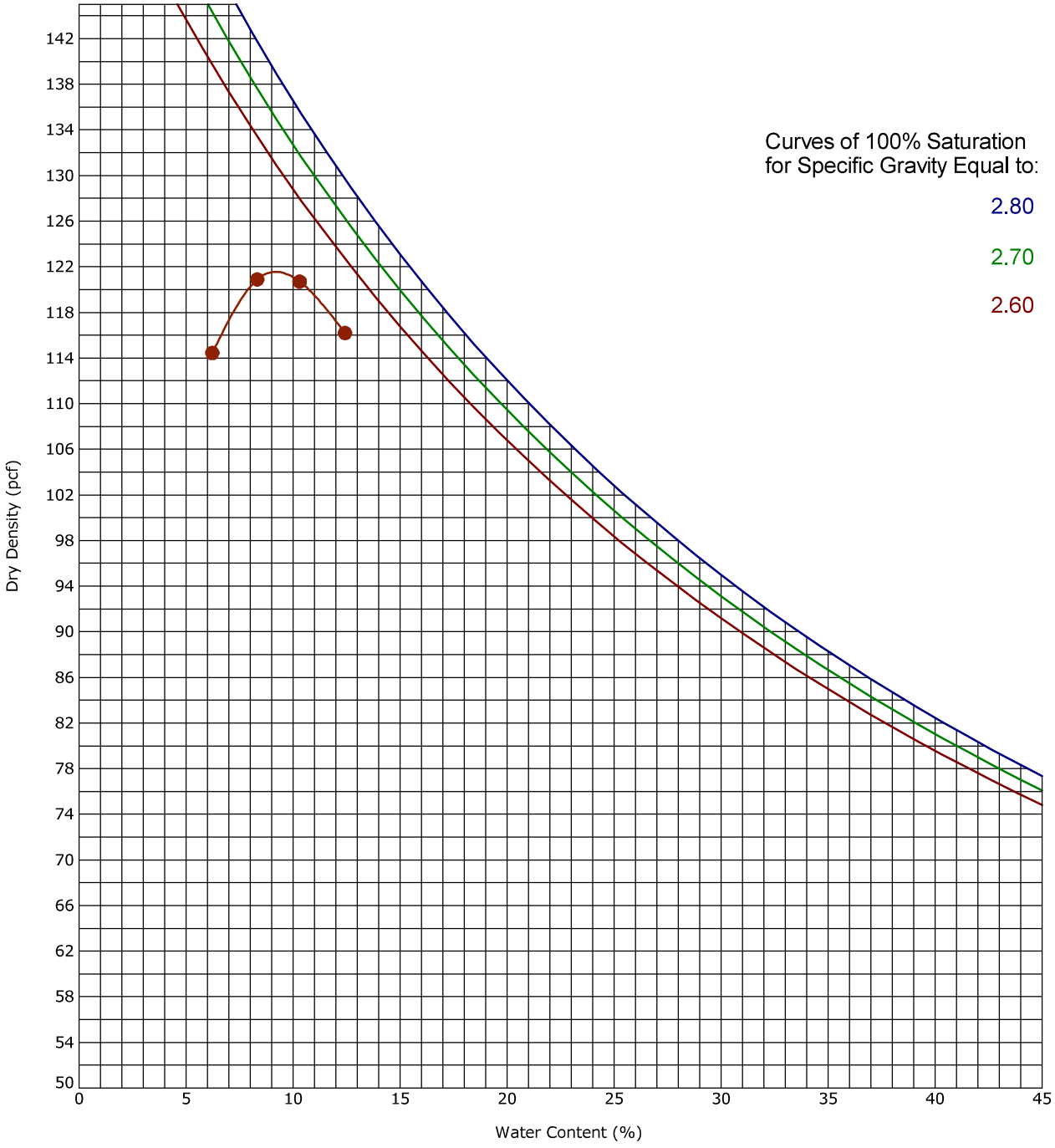
Notes: Water added at 2,000psf

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Moisture-Density Relationship

ASTM D698/D1557



Boring ID	Depth (Ft)	Description of Materials
BH-01	1 - 4	CLAYEY SAND

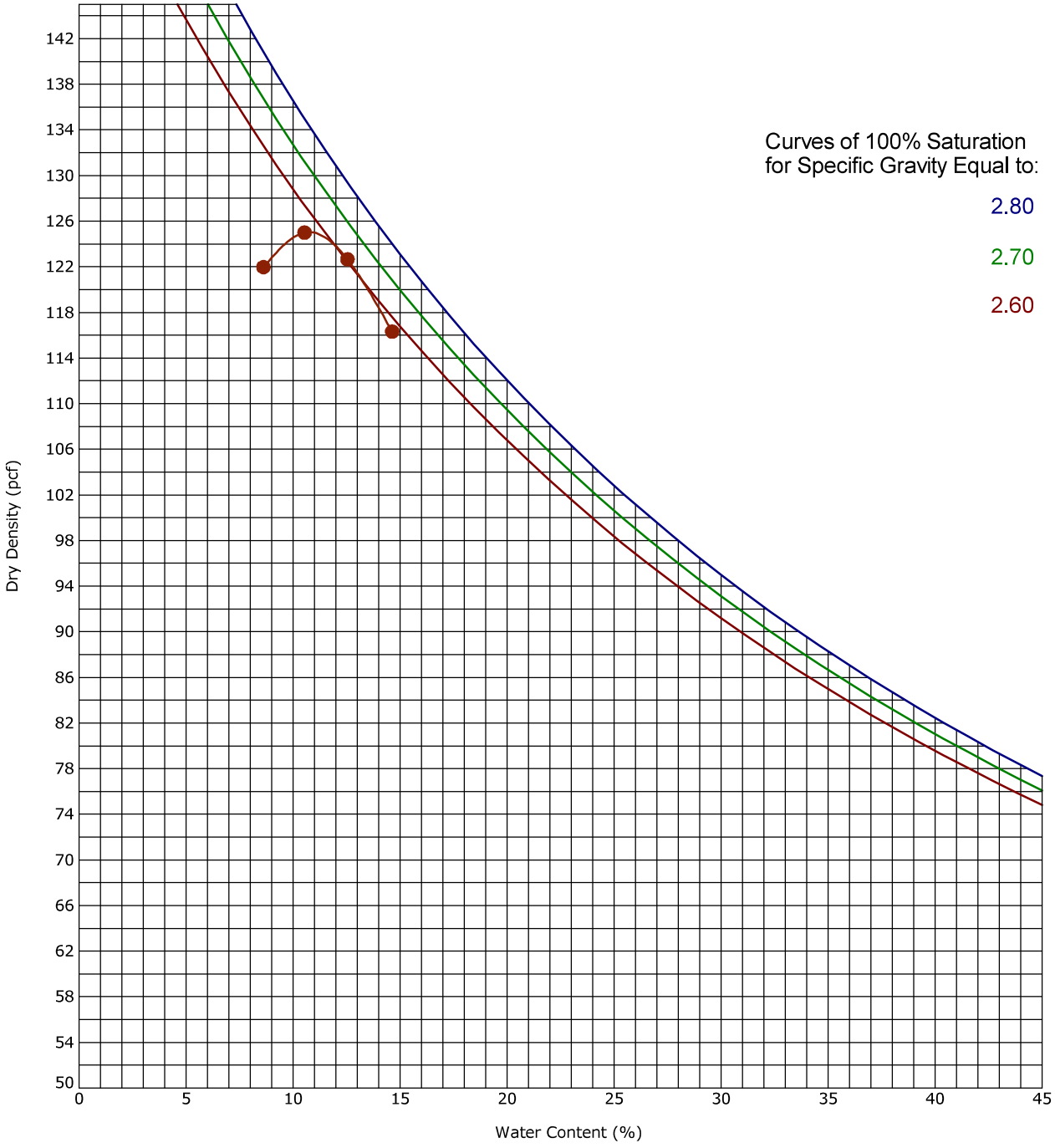
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
					ASTM D698 Method A	121.6	9.2

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Boring ID	Depth (Ft)	Description of Materials
BH-03	1 - 4	CLAYEY SAND

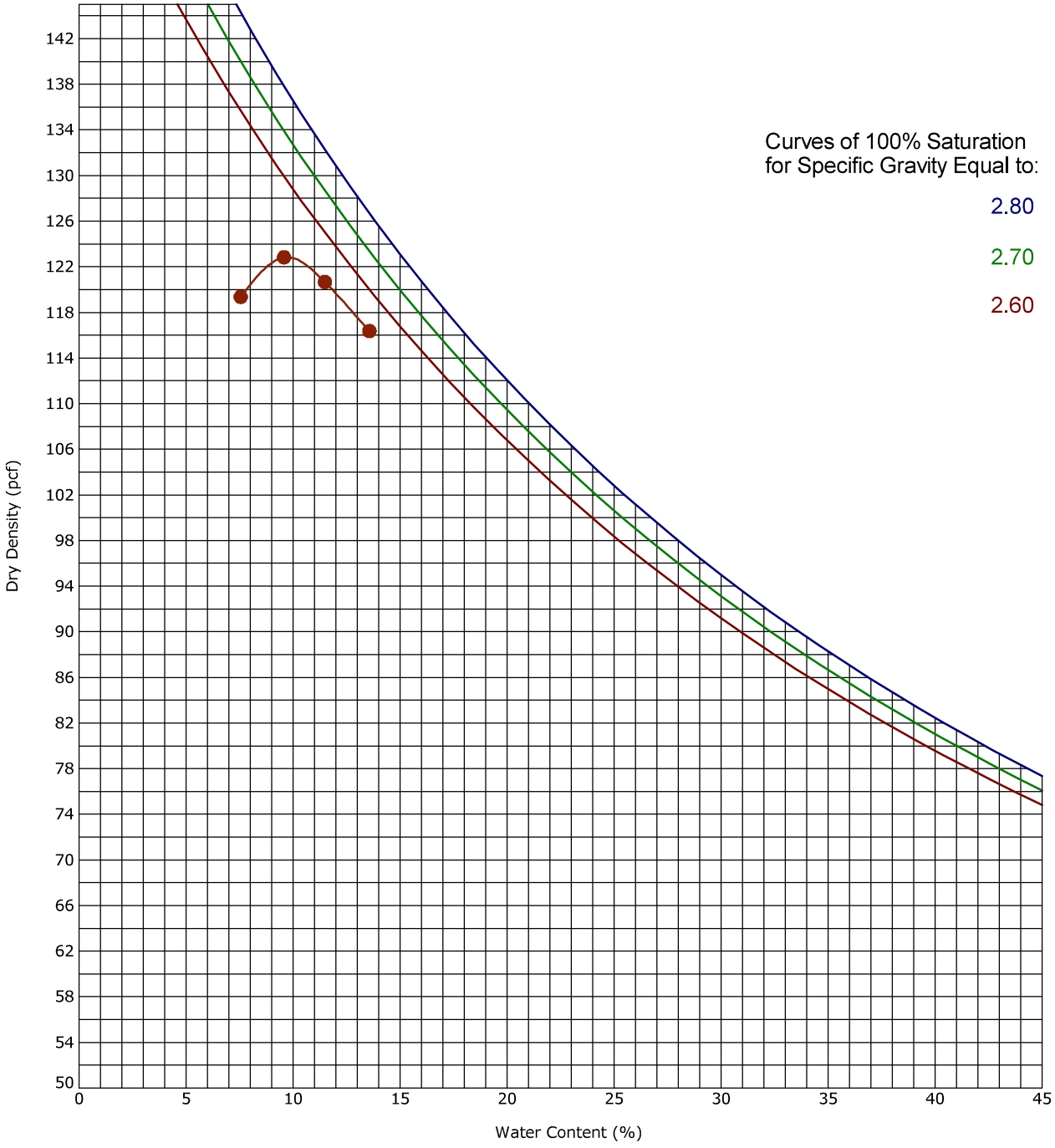
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
36					ASTM D698 Method A	125.0	10.7

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Boring ID	Depth (Ft)	Description of Materials
BH-04	1 - 4	CLAYEY SAND

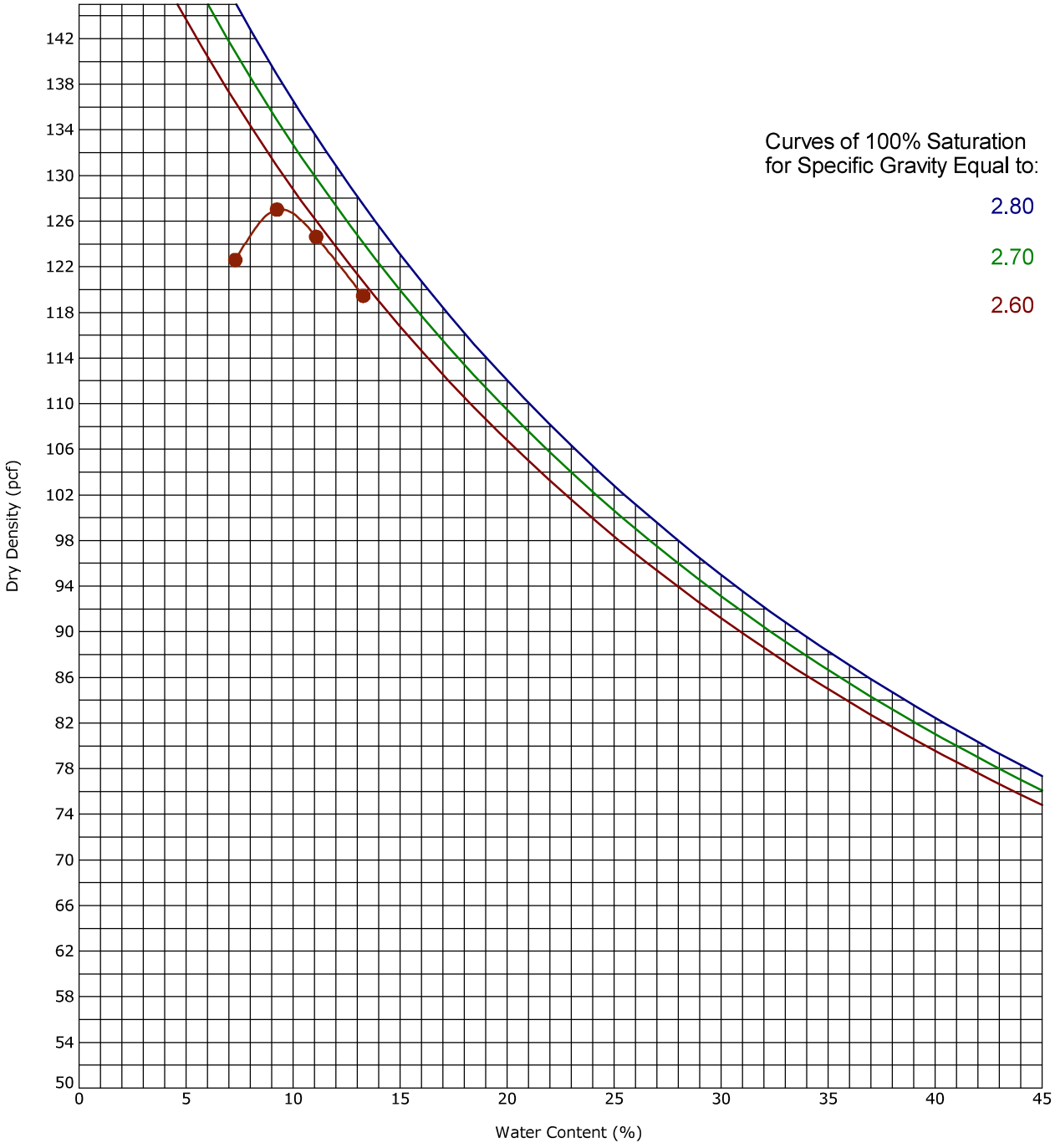
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
21					ASTM D698 Method A	122.8	9.7

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Boring ID	Depth (Ft)	Description of Materials
BH-06	1 - 4	CLAYEY SAND

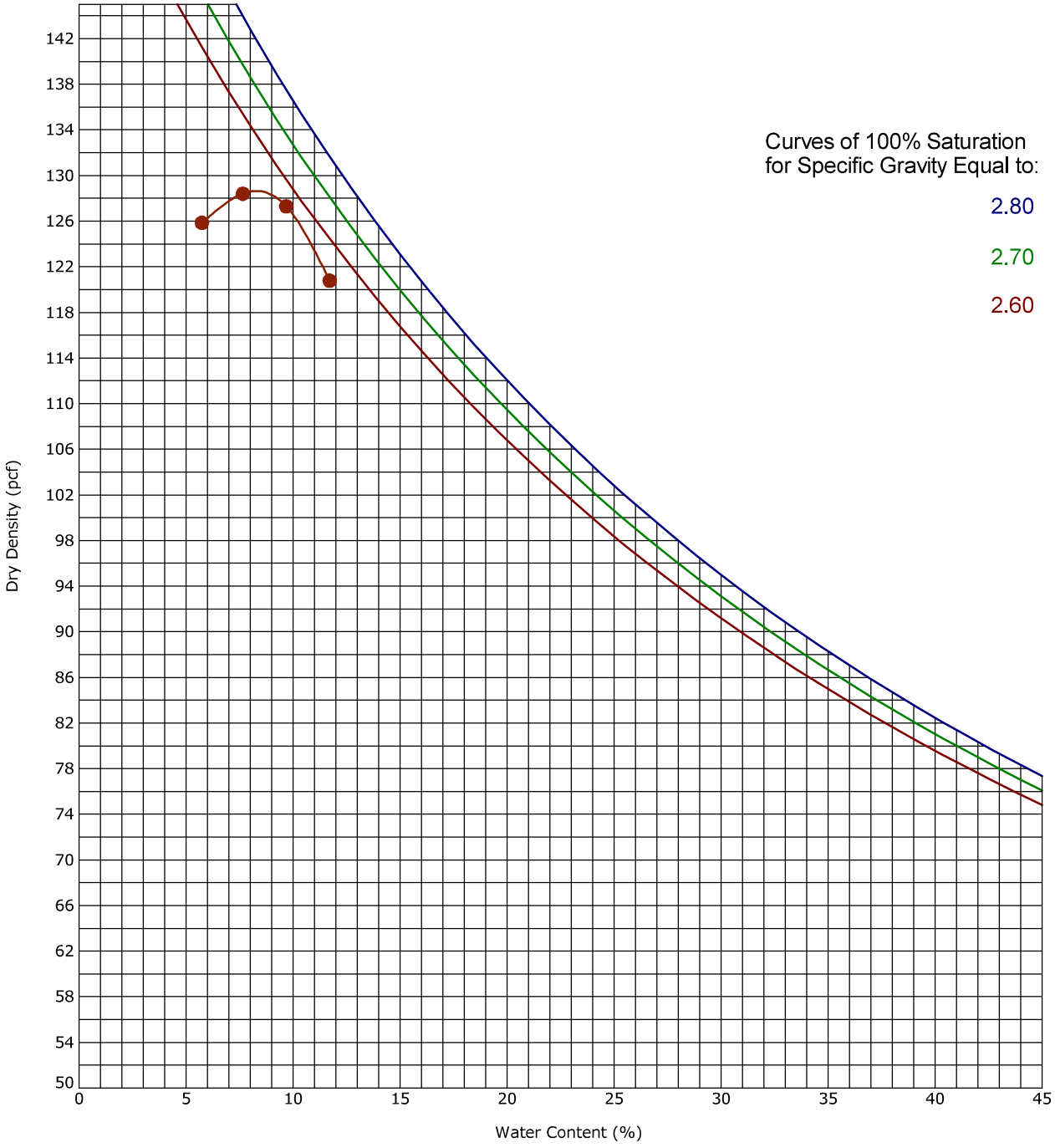
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
20					ASTM D698 Method A	127.1	9.4

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Boring ID	Depth (Ft)	Description of Materials
BH-09	1 - 4	CLAYEY SAND

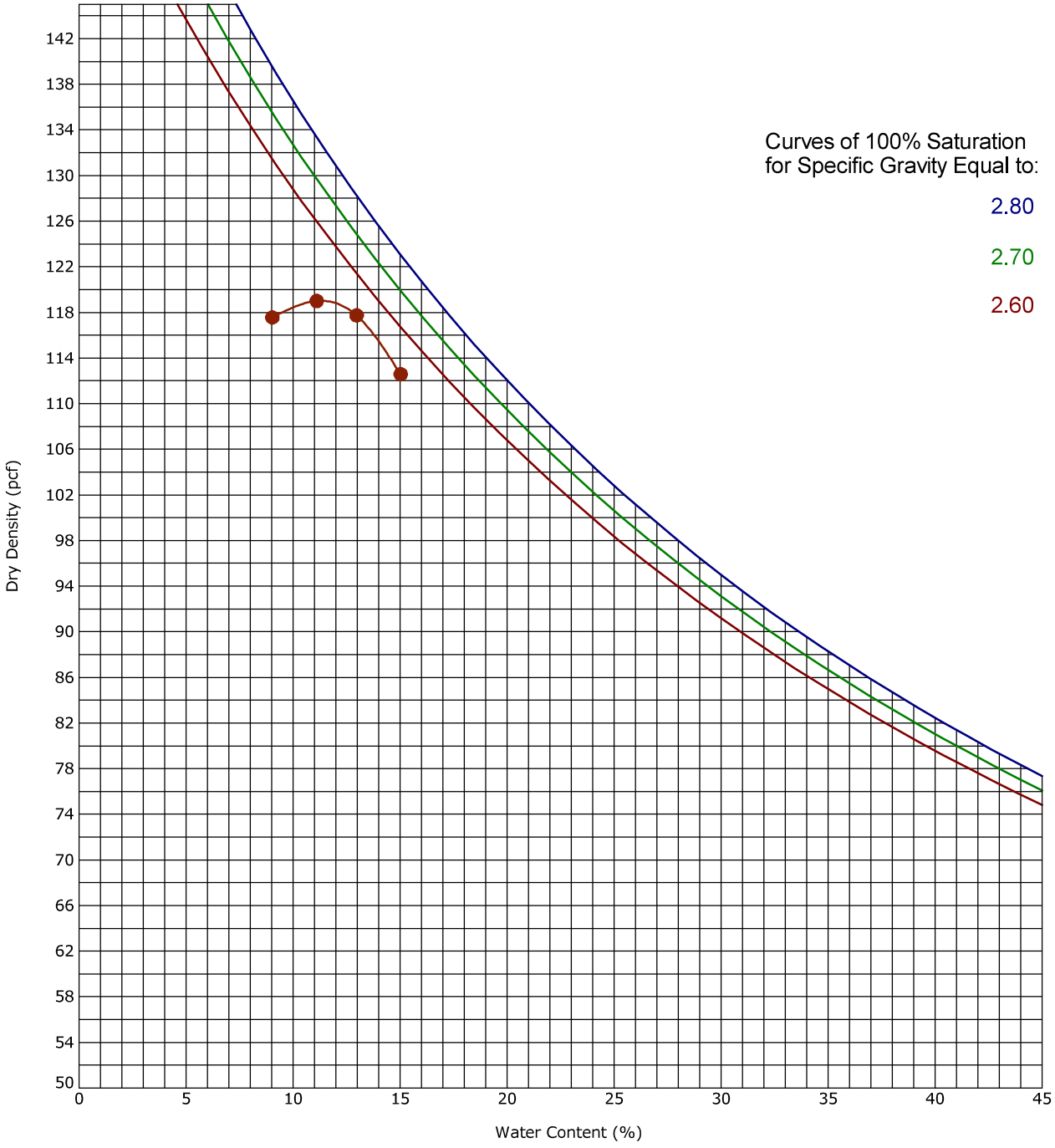
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
26					ASTM D698 Method A	128.6	8.3

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Boring ID	Depth (Ft)	Description of Materials
BH-11	1 - 4	CLAYEY SAND

Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
20					ASTM D698 Method A	119.0	11.3

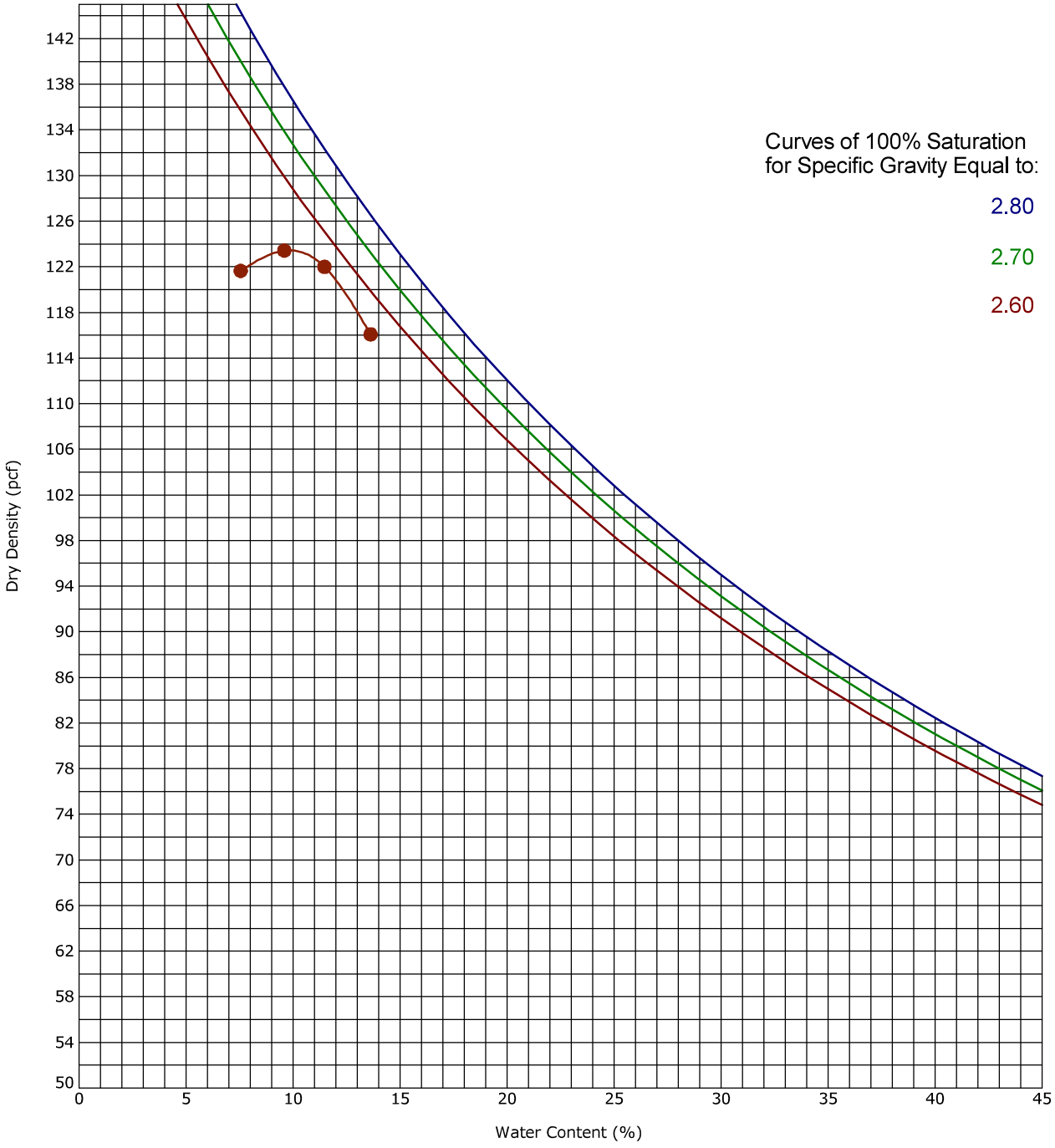
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Moisture-Density Relationship

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Boring ID	Depth (Ft)	Description of Materials
BH-12	1 - 4	CLAYEY SAND

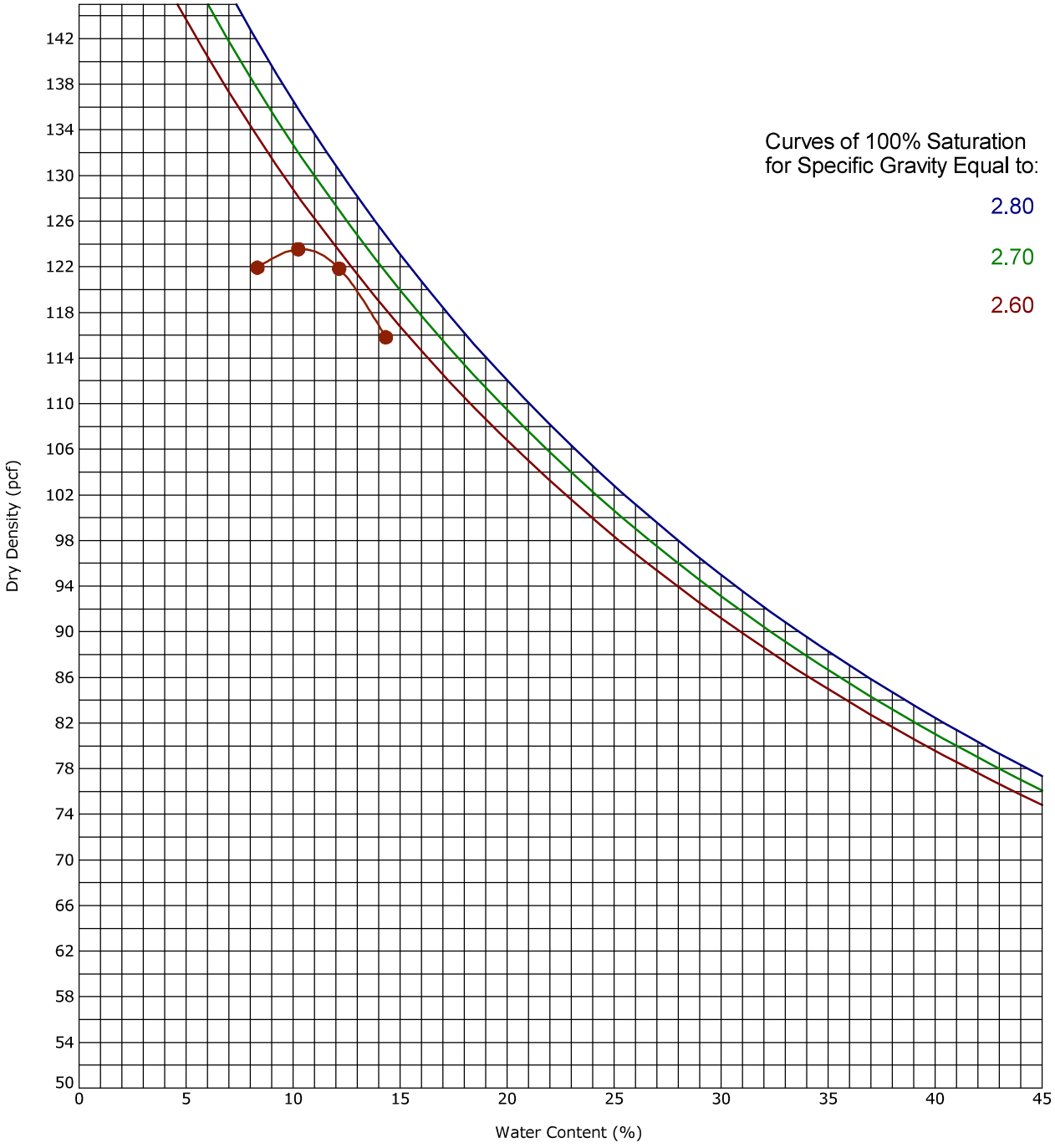
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
16					ASTM D698 Method A	123.4	9.8

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Boring ID	Depth (Ft)	Description of Materials
BH-14	1 - 4	CLAYEY SAND

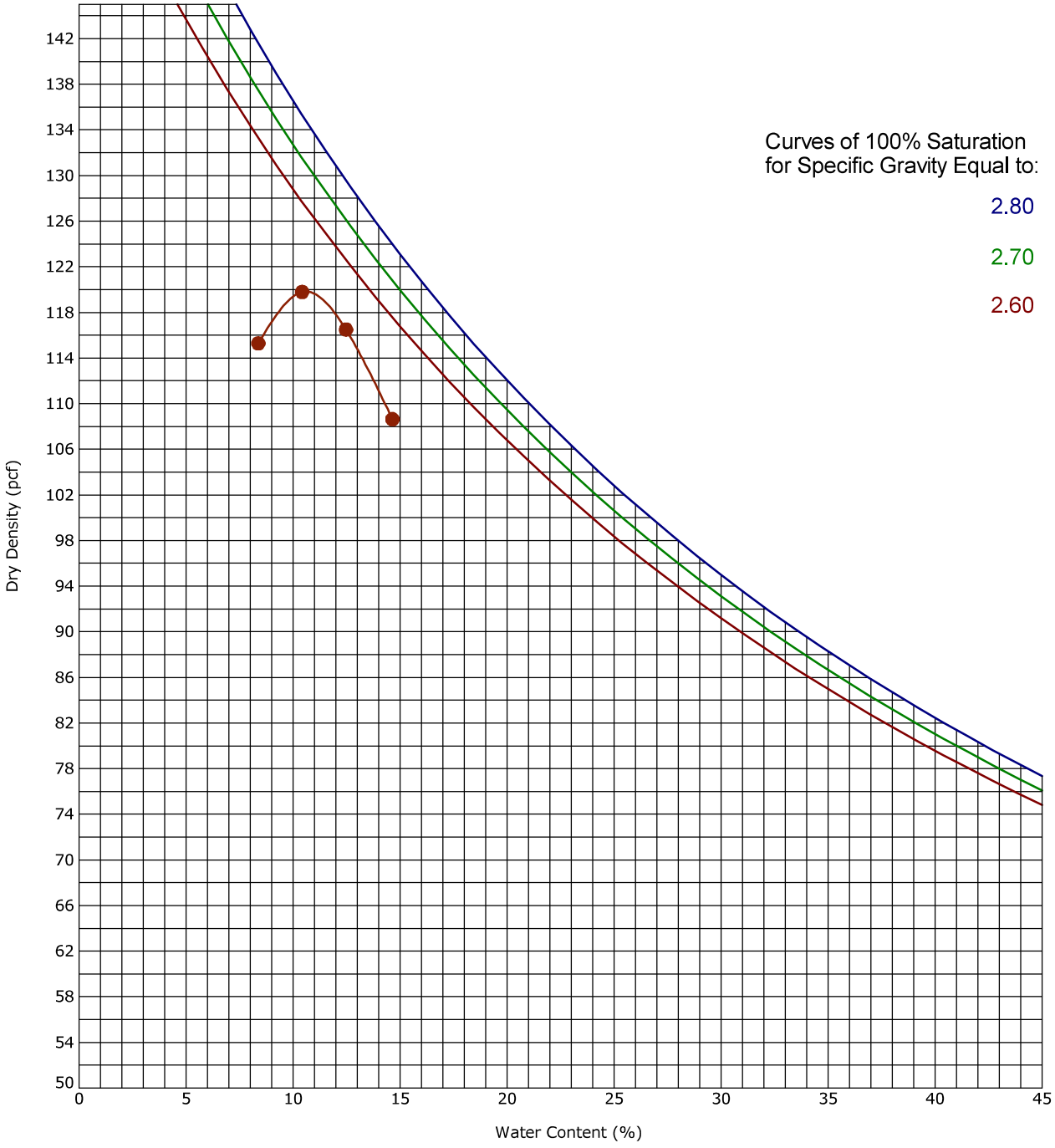
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
18					ASTM D698 Method A	123.5	10.3

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Boring ID	Depth (Ft)	Description of Materials
BH-19	1 - 4	CLAYEY SAND

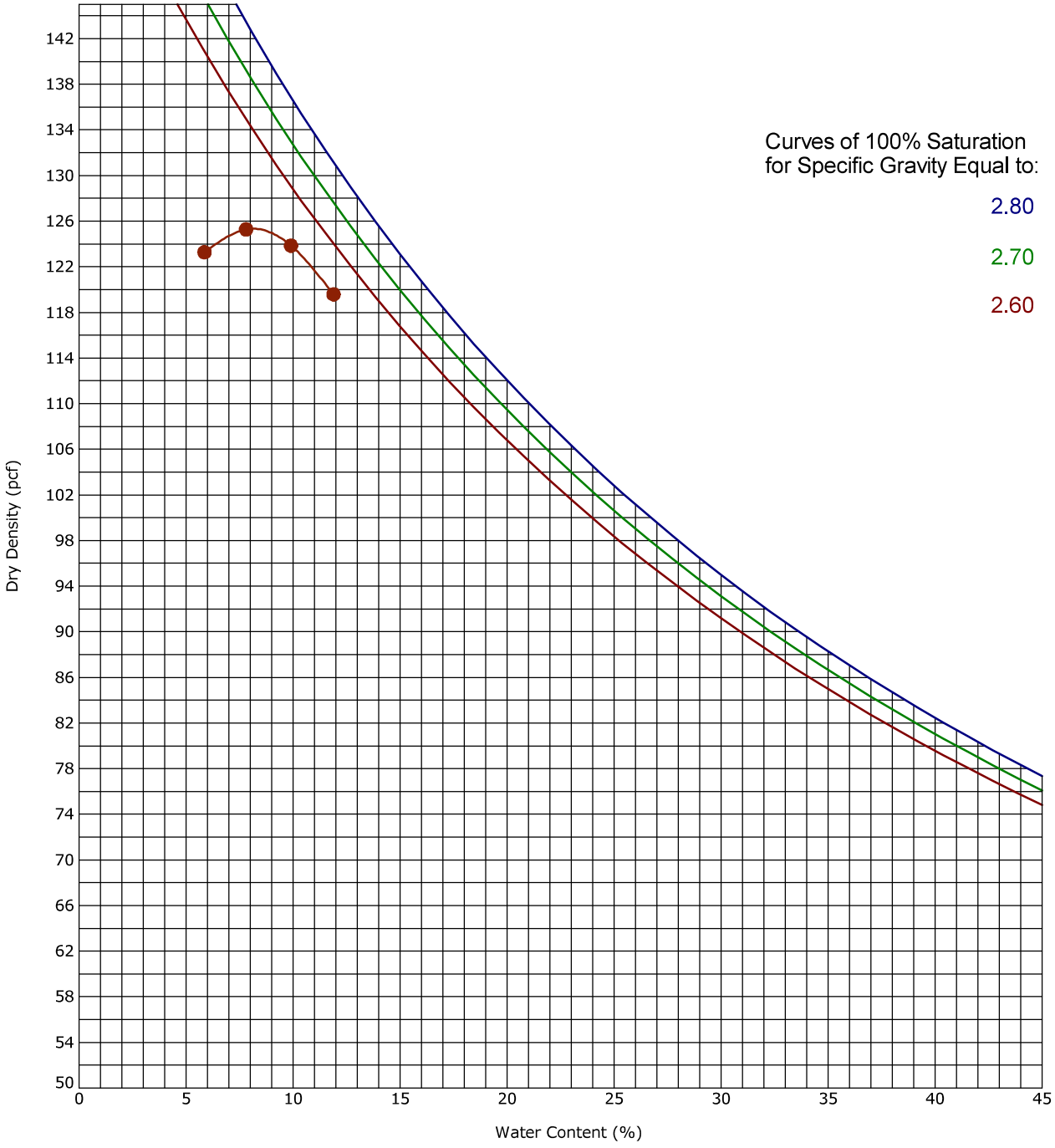
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
26					ASTM D698 Method A	119.8	10.6

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Boring ID	Depth (Ft)	Description of Materials
BH-21	1 - 4	CLAYEY SAND

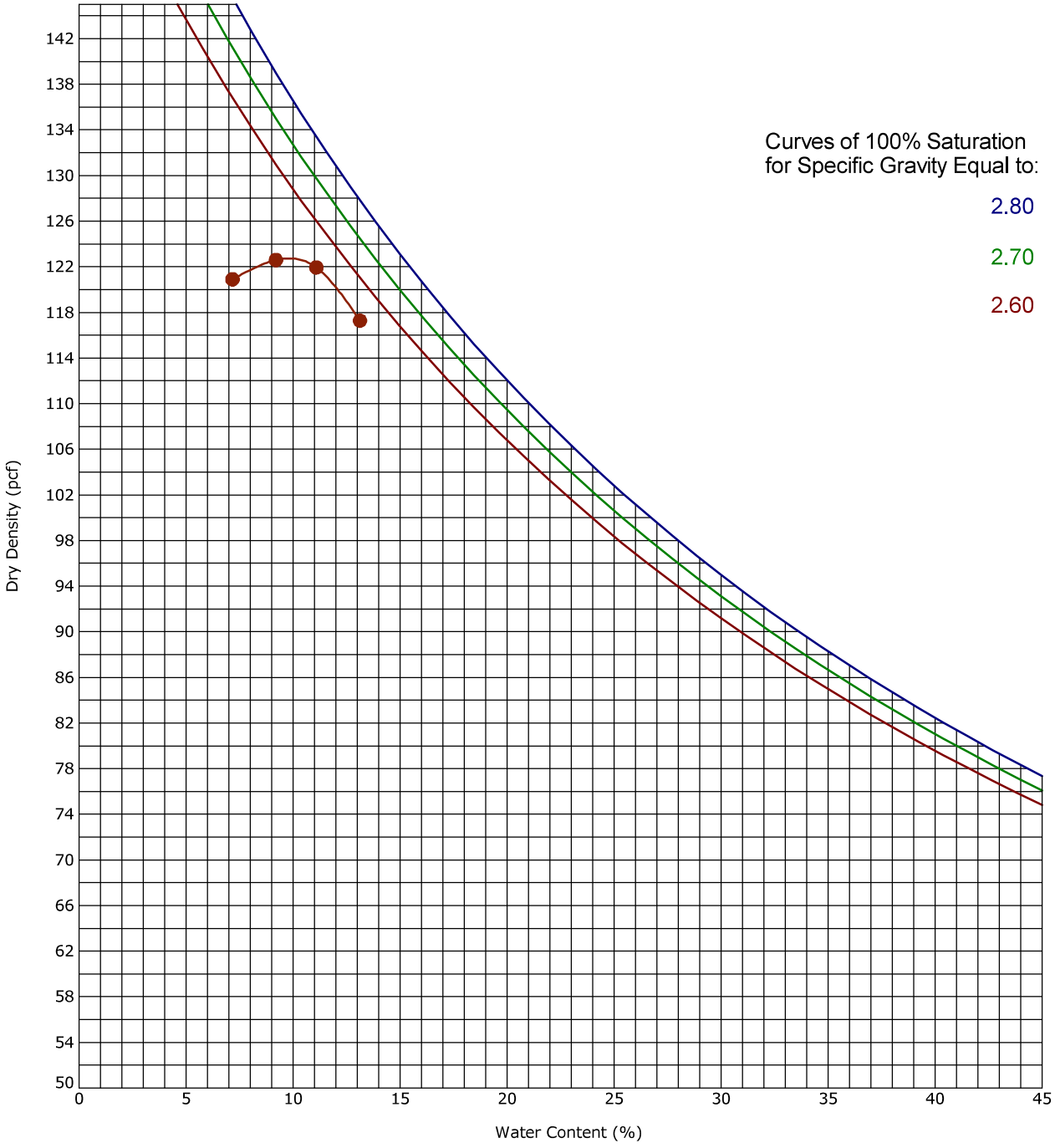
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
24					ASTM D698 Method A	125.3	8.1

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Boring ID	Depth (Ft)	Description of Materials
BH-24	1 - 4	CLAYEY SAND

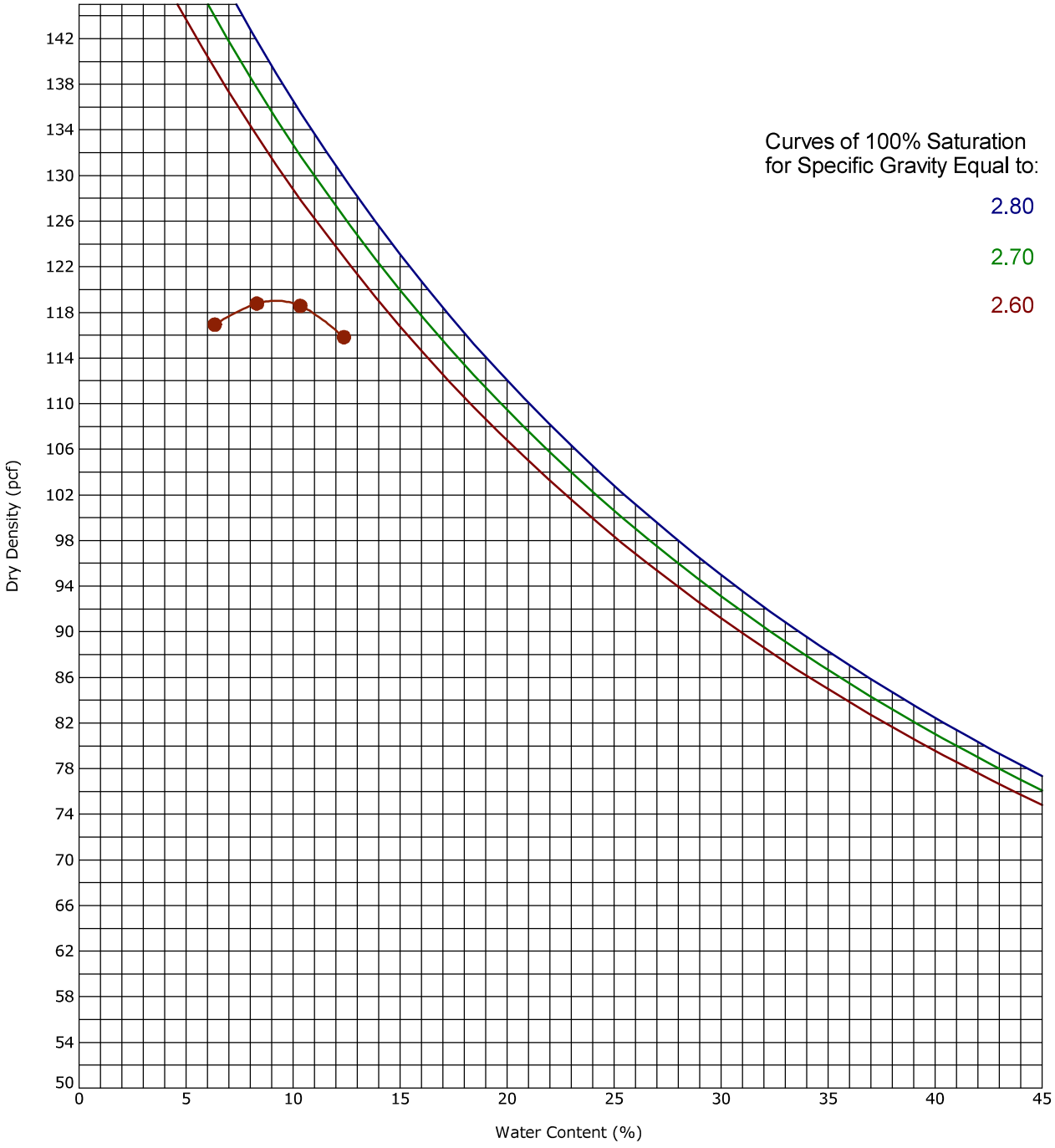
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
12					ASTM D698 Method A	122.7	9.8

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Boring ID	Depth (Ft)	Description of Materials
BH-25	1 - 4	CLAYEY SAND

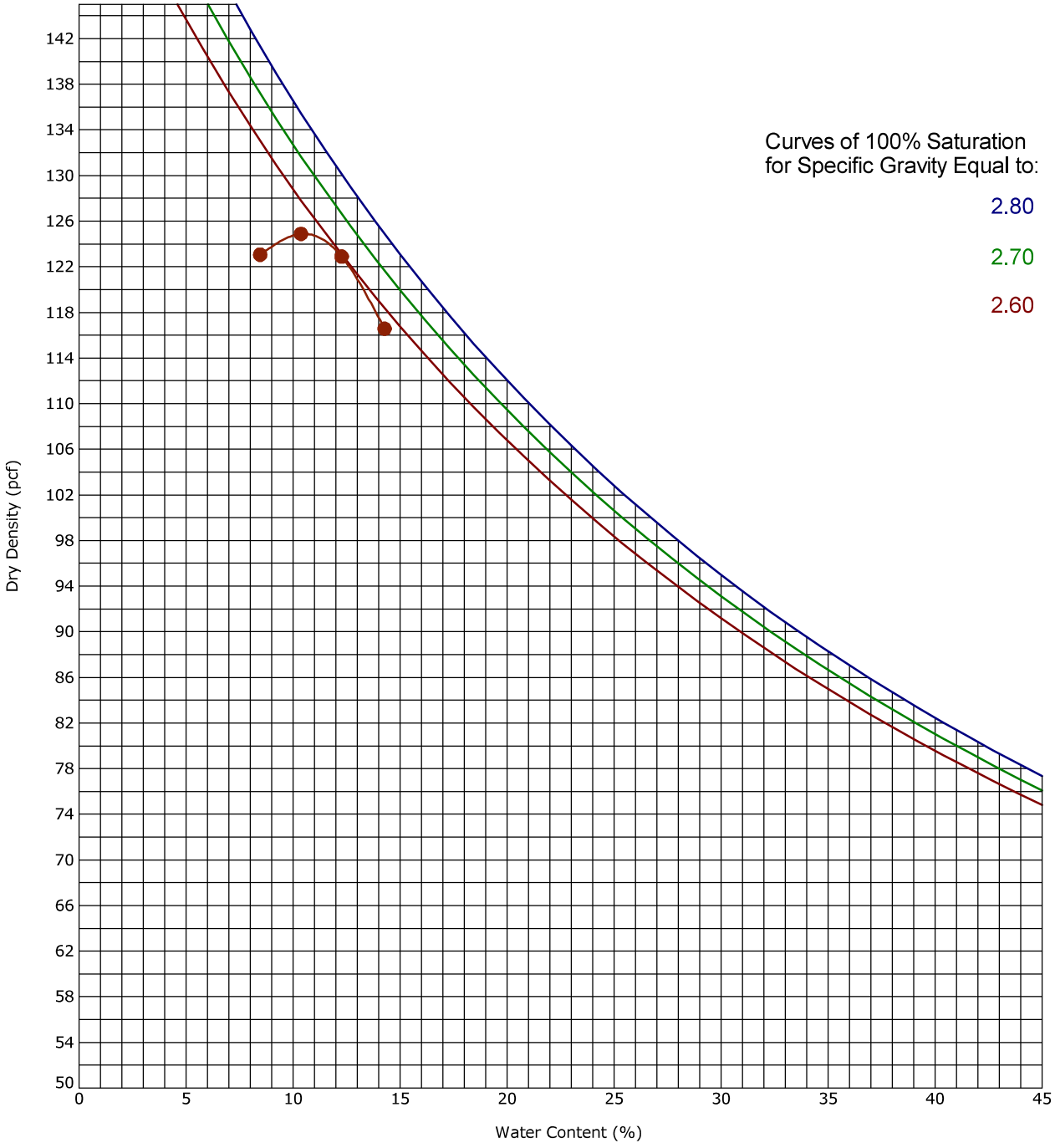
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
19					ASTM D698 Method A	119.0	9.2

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Boring ID	Depth (Ft)	Description of Materials
BH-27	1 - 4	CLAYEY SAND

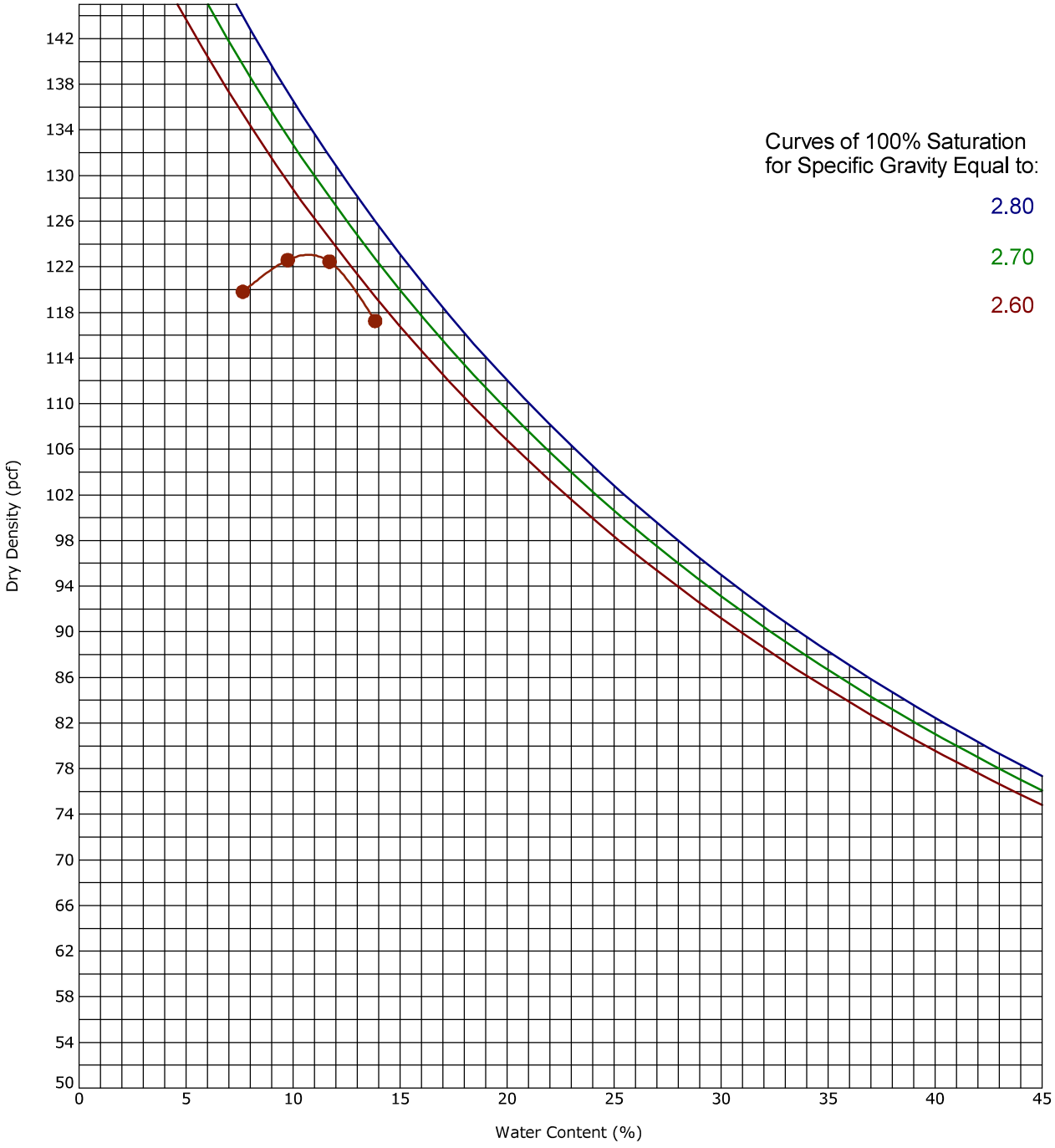
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
16					ASTM D698 Method A	124.9	10.5

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Boring ID	Depth (Ft)	Description of Materials
BH-29	1 - 4	CLAYEY SAND

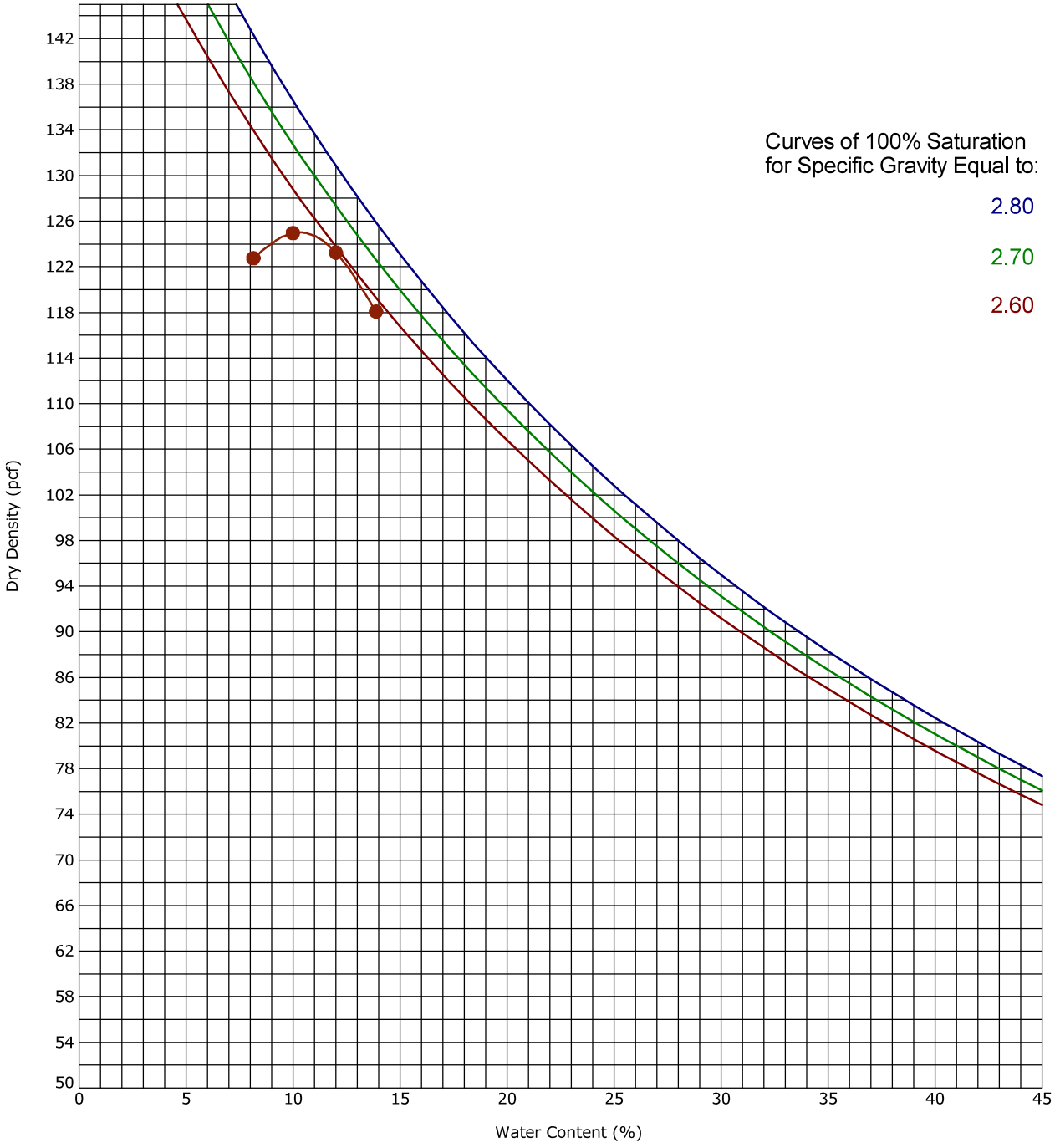
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
15					ASTM D698 Method A	123.0	10.8

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Moisture-Density Relationship

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Boring ID	Depth (Ft)	Description of Materials
BH-31	1 - 4	CLAYEY SAND

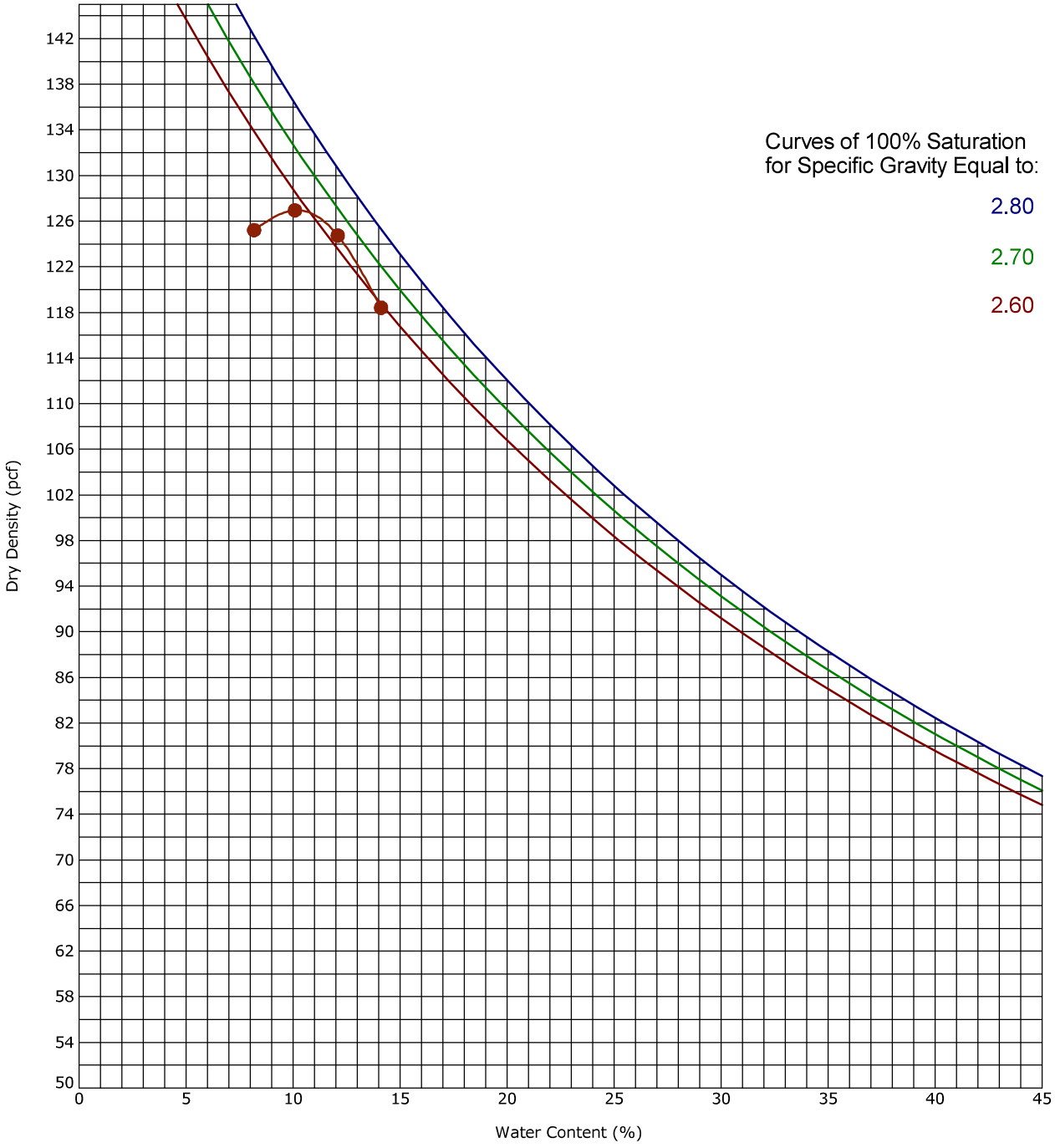
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
21					ASTM D698 Method A	125.0	10.3

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Moisture-Density Relationship

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Boring ID	Depth (Ft)	Description of Materials
BH-33	1 - 4	CLAYEY SAND

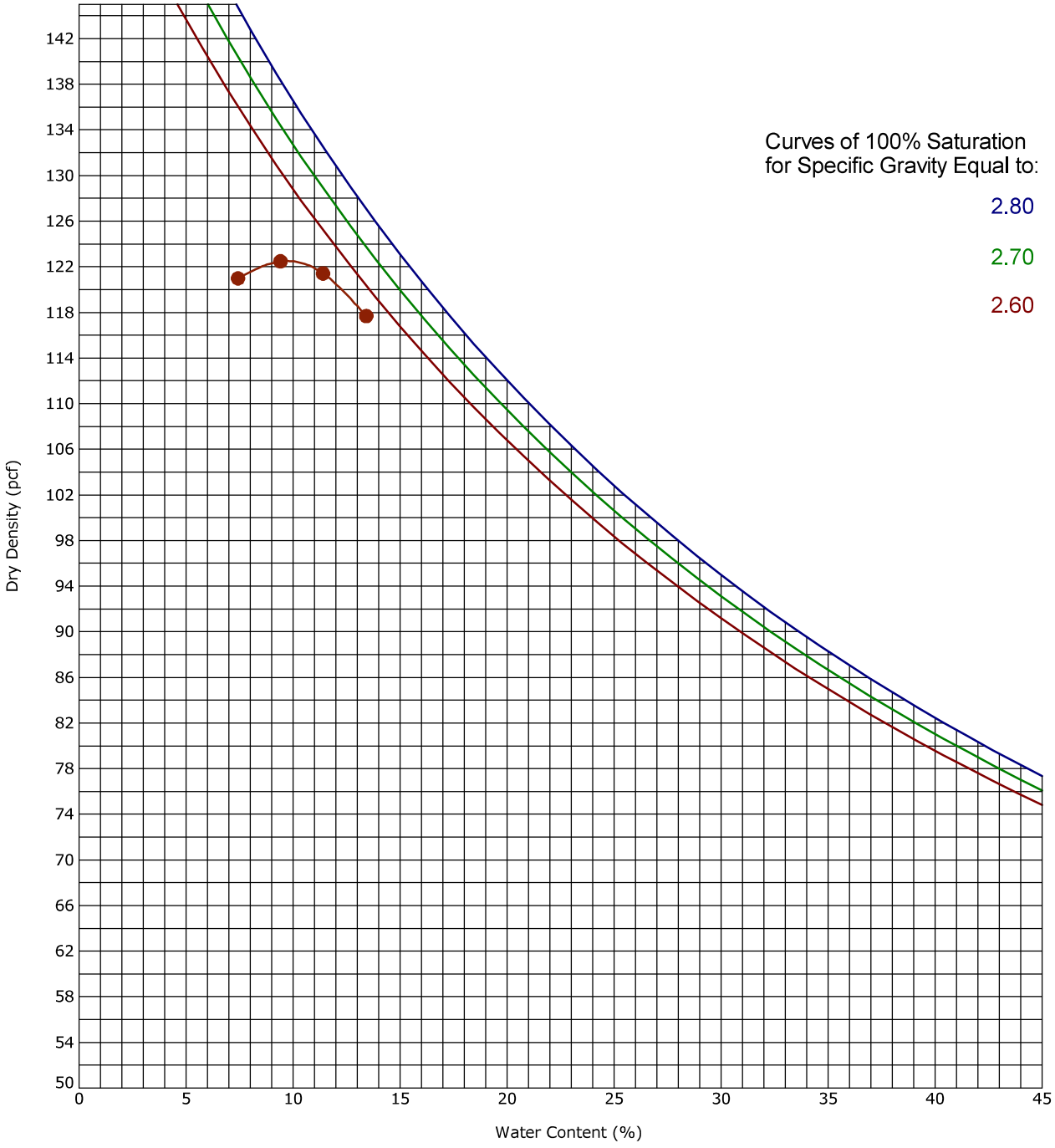
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
21					ASTM D698 Method A	127.0	10.2

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Boring ID	Depth (Ft)	Description of Materials
BH-35	1 - 4	CLAYEY SAND

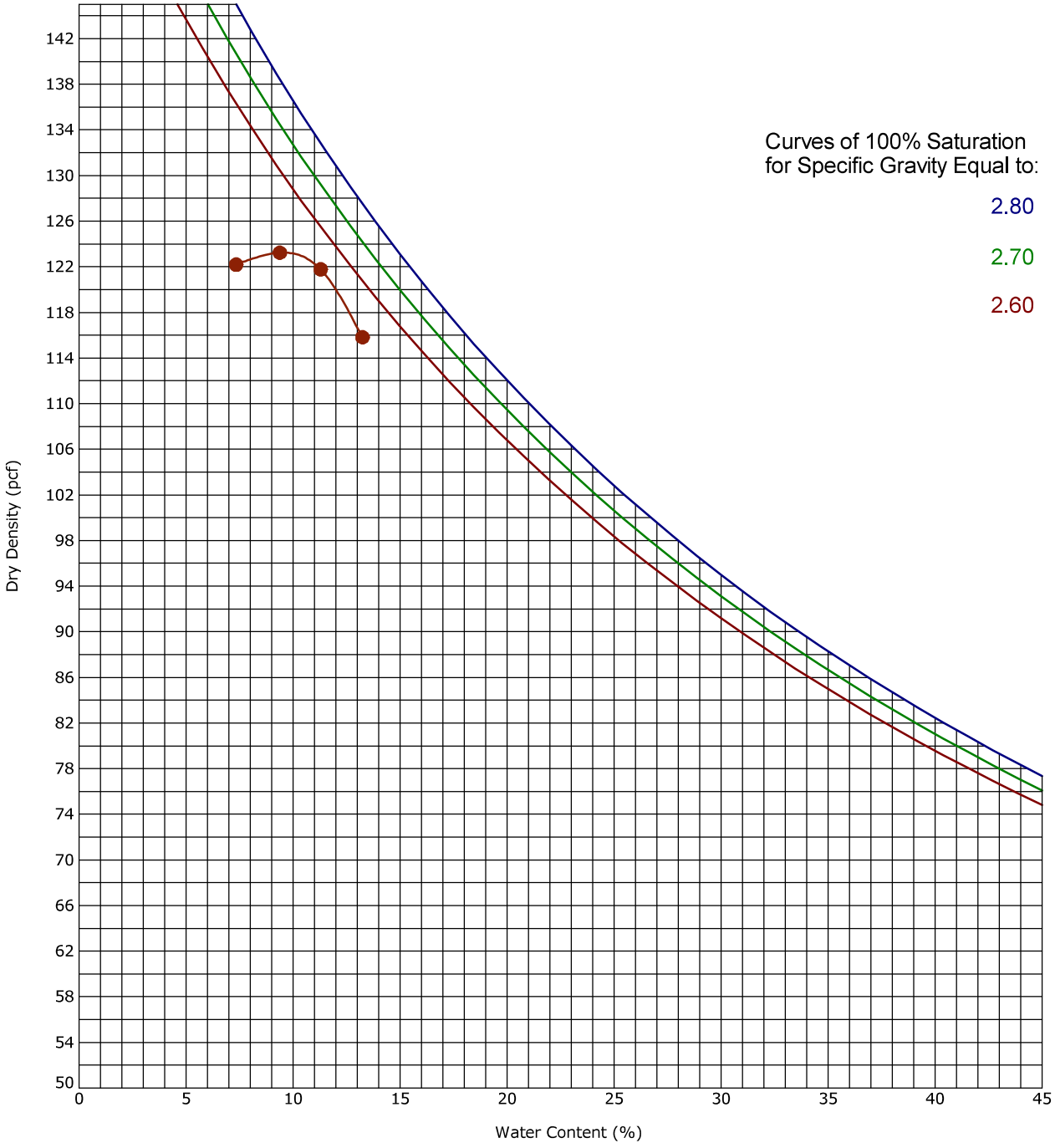
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
19					ASTM D698 Method A	122.5	9.7

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Boring ID	Depth (Ft)	Description of Materials
BH-39	1 - 4	CLAYEY SAND

Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
19					ASTM D698 Method A	123.2	9.6

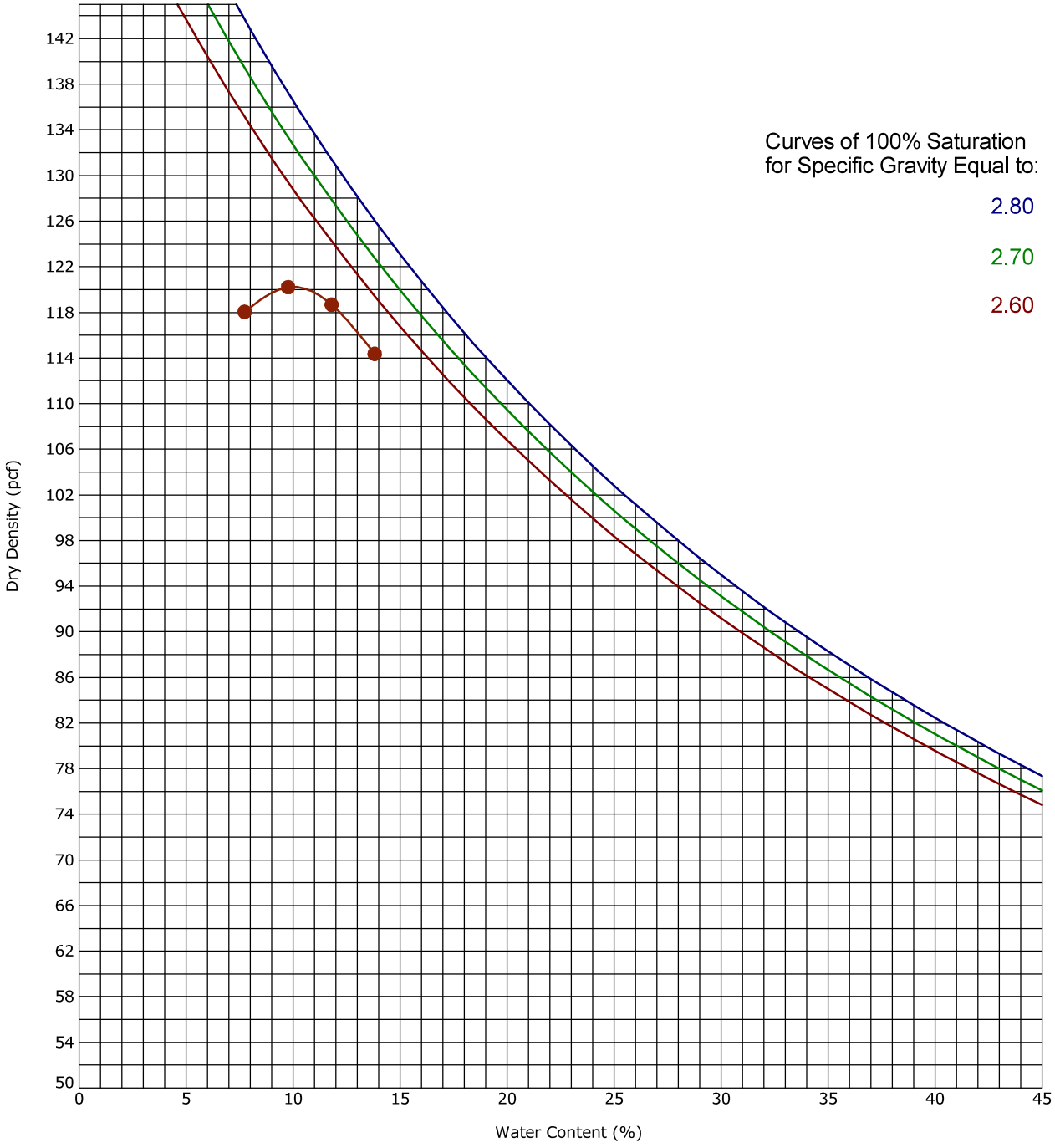
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Moisture-Density Relationship

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Boring ID	Depth (Ft)	Description of Materials
BH-41	1 - 4	POORLY GRADED SAND WITH CLAY

Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
11					ASTM D698 Method A	120.2	10.0

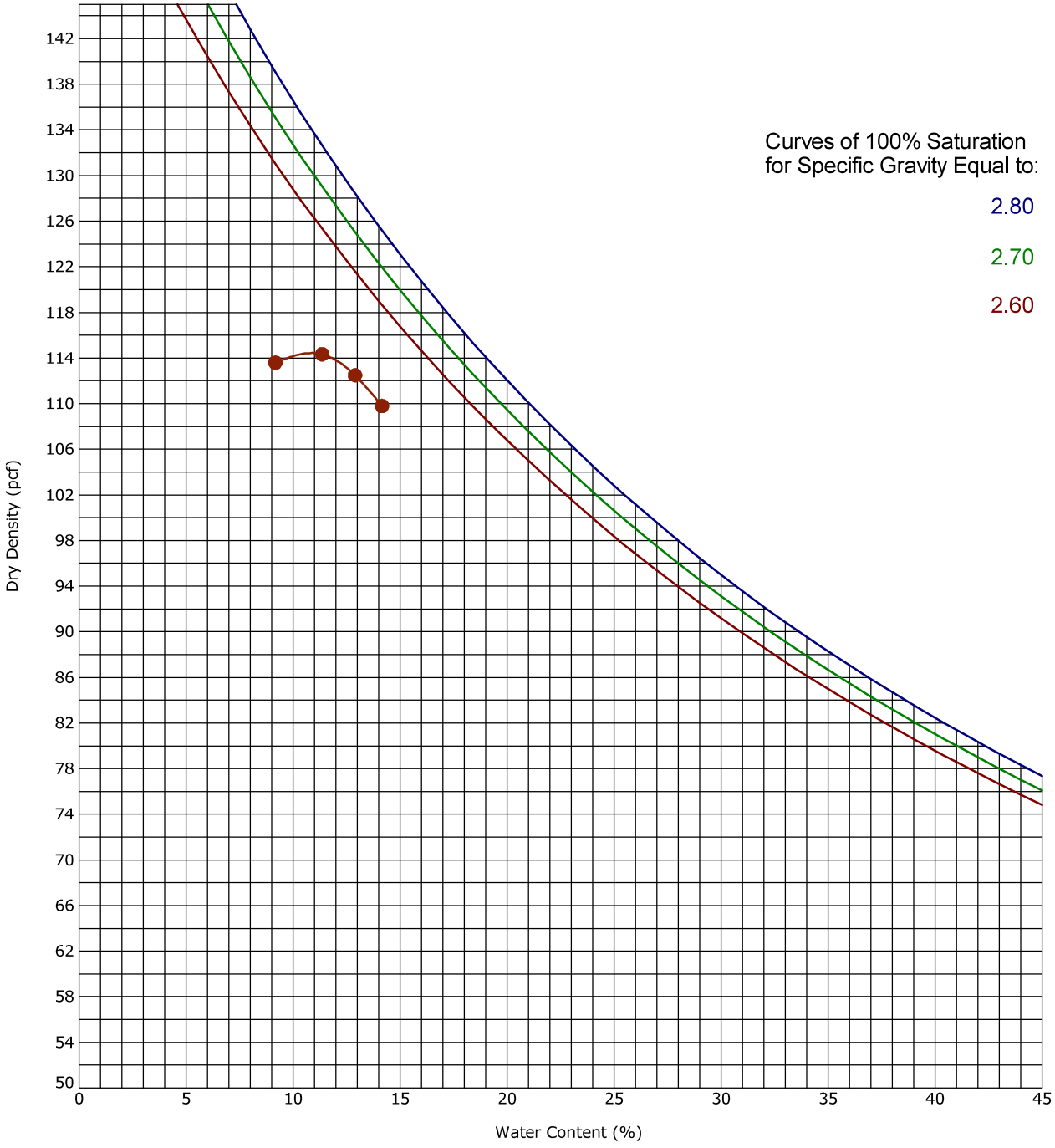
Laboratory tests are not valid if separated from original report.

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Boring ID	Depth (Ft)	Description of Materials
BH-42	1 - 4	POORLY GRADED SAND

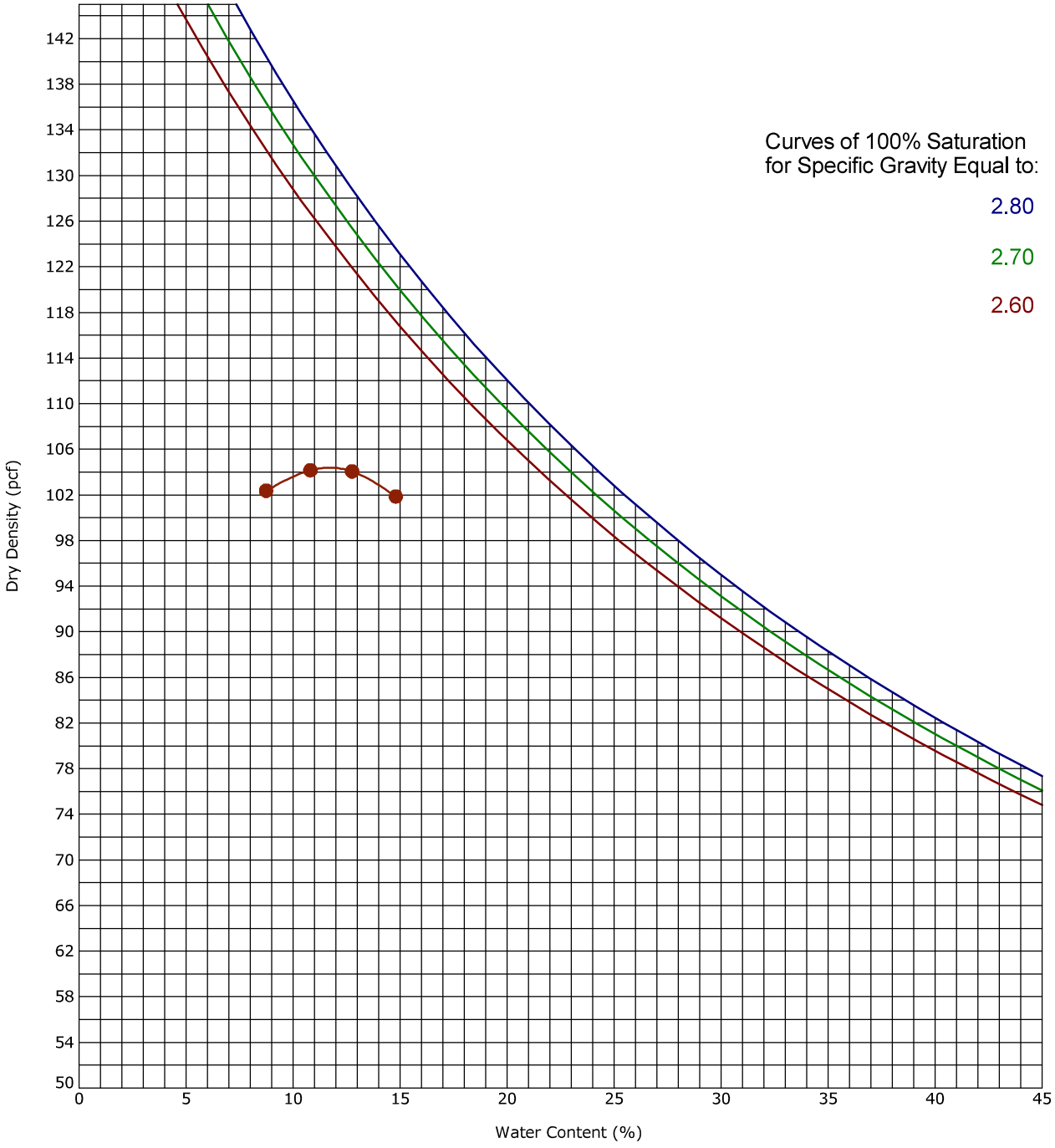
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
3					ASTM D1557 Method D	114.4	10.9

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

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23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Moisture-Density Relationship

ASTM D698/D1557



Boring ID	Depth (Ft)	Description of Materials
BH-42	2 - 4	POORLY GRADED SAND

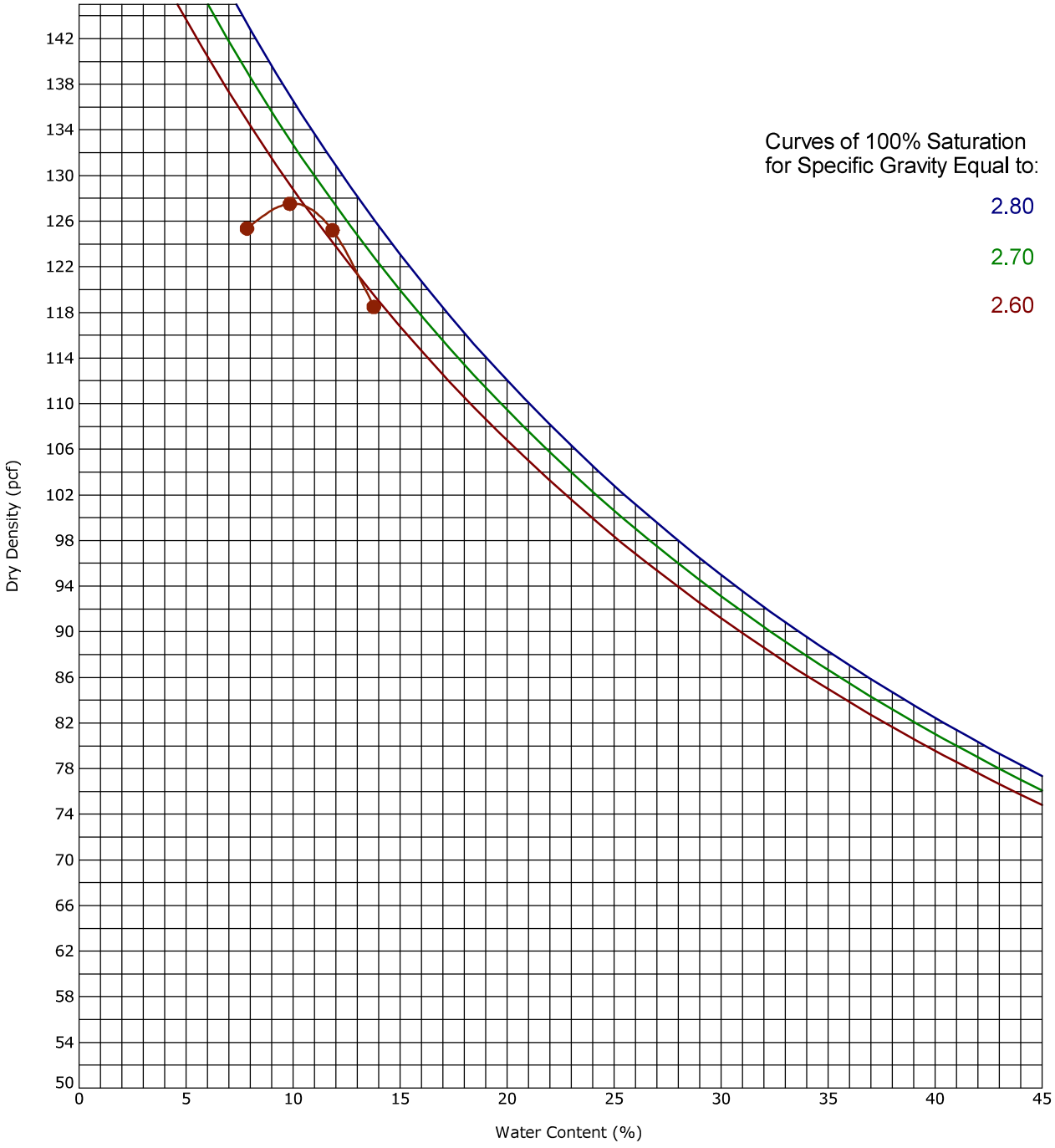
Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
					ASTM D698 Method A	104.4	11.7

Grace Solar
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Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Moisture-Density Relationship

ASTM D698/D1557



Boring ID	Depth (Ft)	Description of Materials
SBH-01	1 - 4	CLAYEY SAND

Fines (%)	Fraction >19mm size (%)	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
					ASTM D698 Method A	127.5	10.0

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January 29, 2024

Terracon
23041 Avenida de la Carlota, Suite 350
Laguna Hills, CA 92653
Attn: Victor V. Nguyen

Re: Thermal Analysis of Native Soil Samples
Grace Solar – Blythe, CA (Project No. 60225164)

The following is the report of thermal dryout characterization tests conducted on twenty-two (22) native soil samples from the referenced project sent to our laboratory.

Thermal Resistivity Tests: Per instructions, the samples were tested at the 'optimum' moisture content and at 85% of the standard Proctor dry density ***provided by Terracon***. In addition, sample BH-42 was tested at 90% of the standard Proctor dry density. The tests were conducted in accordance with the IEEE standard 442-2017. The results are tabulated below and the thermal dryout curves are presented in **Figure 1 to 22.**

Sample ID, Description, Thermal Resistivity, Moisture Content and Density

Sample ID	Depth (ft)	Effort (%)	Description (Terracon)	Thermal Resistivity (°C-cm/W)		Moisture Content (%)	Dry Density (lb/ft^3)
				Wet	Dry		
BH-42	2 - 4	90	--	52	202	12	94
TP-02	4 - 6	85	--	69	205	11	90
TP-04	4 - 6	85	--	66	193	14	91
TP-05	4 - 6	85	--	65	186	14	91
TP-06	4 - 6	85	--	62	150	11	103
TP-07	4 - 6	85	--	64	174	13	97
TP-08	4 - 6	85	--	65	179	13	94
TP-09	4 - 6	85	--	64	196	12	97

Sample ID, Description, Thermal Resistivity, Moisture Content and Density

Sample ID	Depth (ft)	Effort (%)	Description (Terracon)	Thermal Resistivity (°C-cm/W)		Moisture Content (%)	Dry Density (lb/ft^3)
				Wet	Dry		
TP-11	4 - 6	85	--	63	188	11	98
TP-12	4 - 6	85	--	64	179	15	92
TP-13	4 - 6	85	--	76	228	14	87
TP-14	4 - 6	85	--	63	174	13	93
TP-15	4 - 6	85	--	67	198	13	89
TP-17	4 - 6	85	--	60	142	14	101
TP-18	4 - 6	85	--	55	163	12	101
TP-19	4 - 6	85	--	71	203	15	90
TP-20	4 - 6	85	--	62	178	15	90
TP-22	4 - 6	85	--	57	151	12	96
TP-23	4 - 6	85	--	55	148	13	96
TP-24	4 - 6	85	--	60	167	14	91
TP-25	4 - 6	85	--	61	169	14	91
STP-1	4 - 6	85	--	67	174	19	90

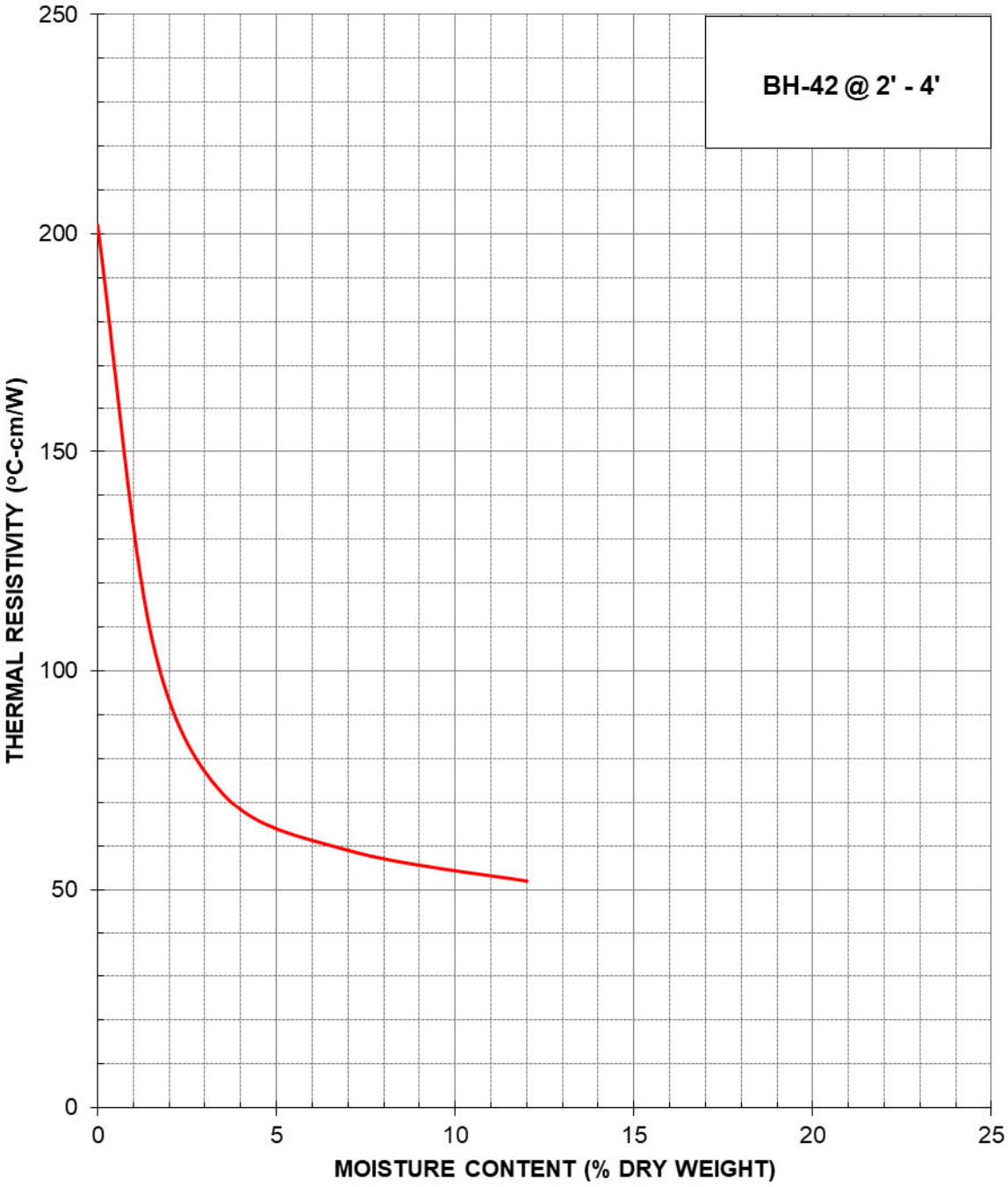
Please contact us if you have any questions or if we can be of further assistance.

Geotherm USA



Nimesh Patel

THERMAL DRYOUT CURVE



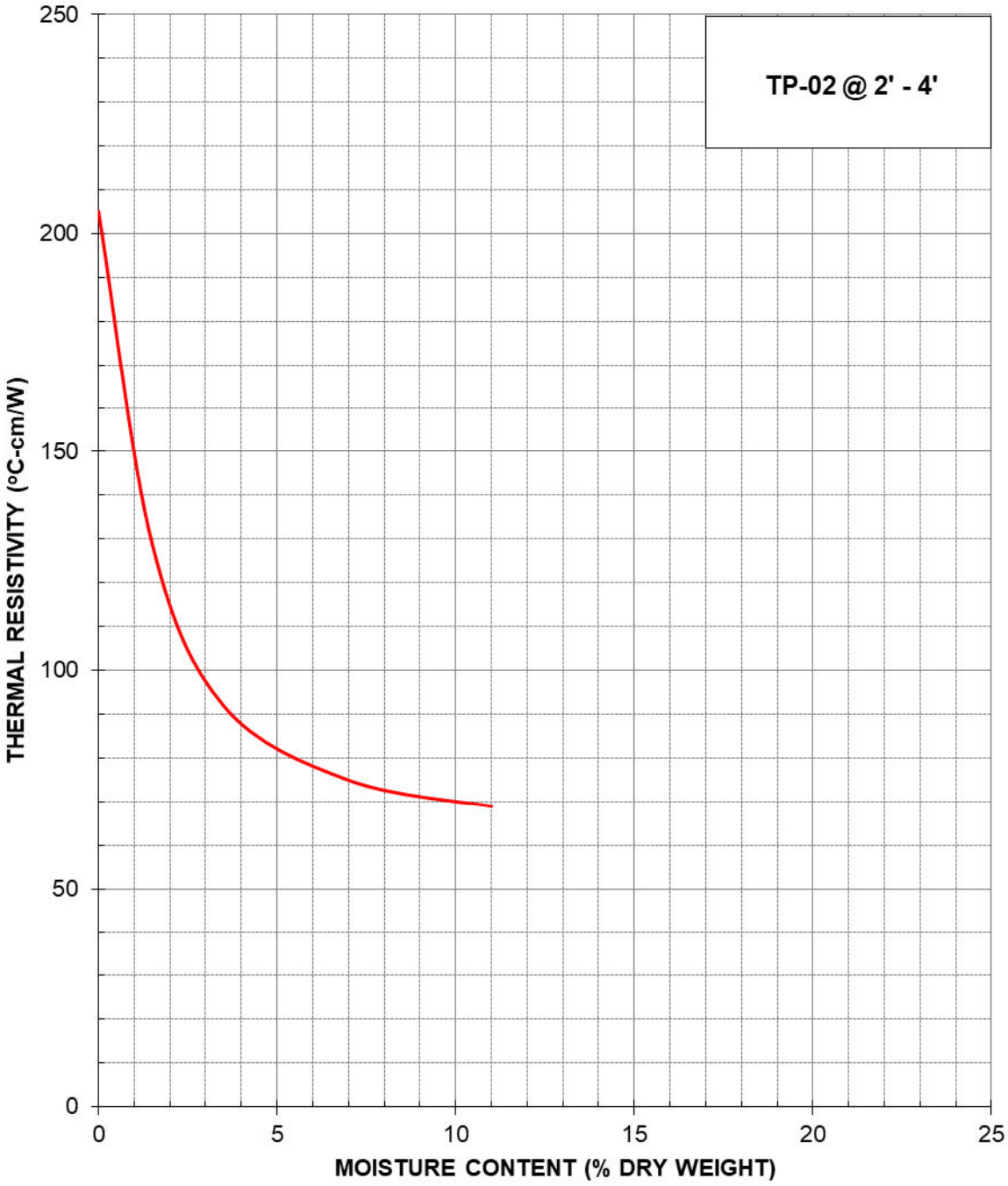
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 1**

THERMAL DRYOUT CURVE



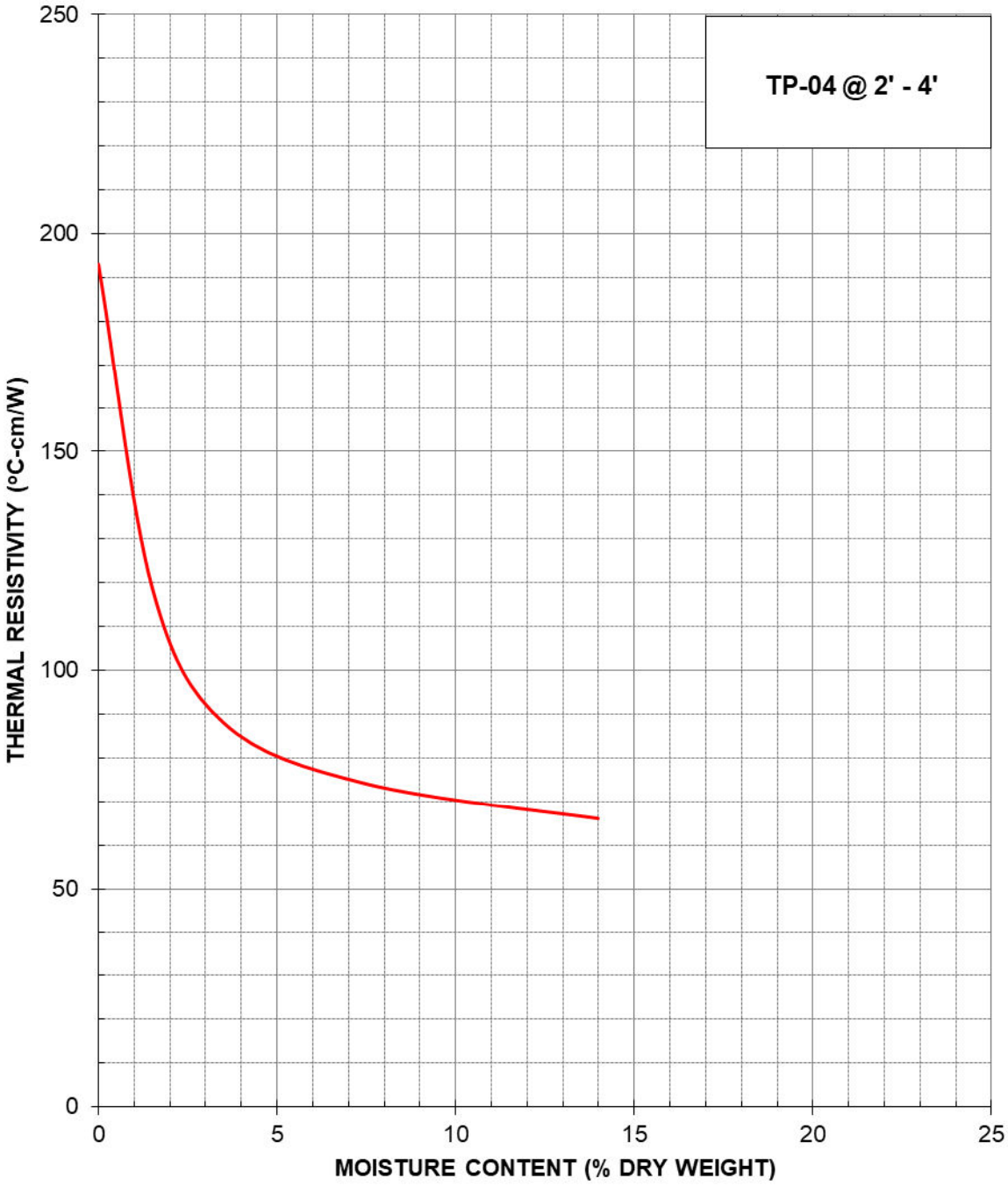
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 2**

THERMAL DRYOUT CURVE



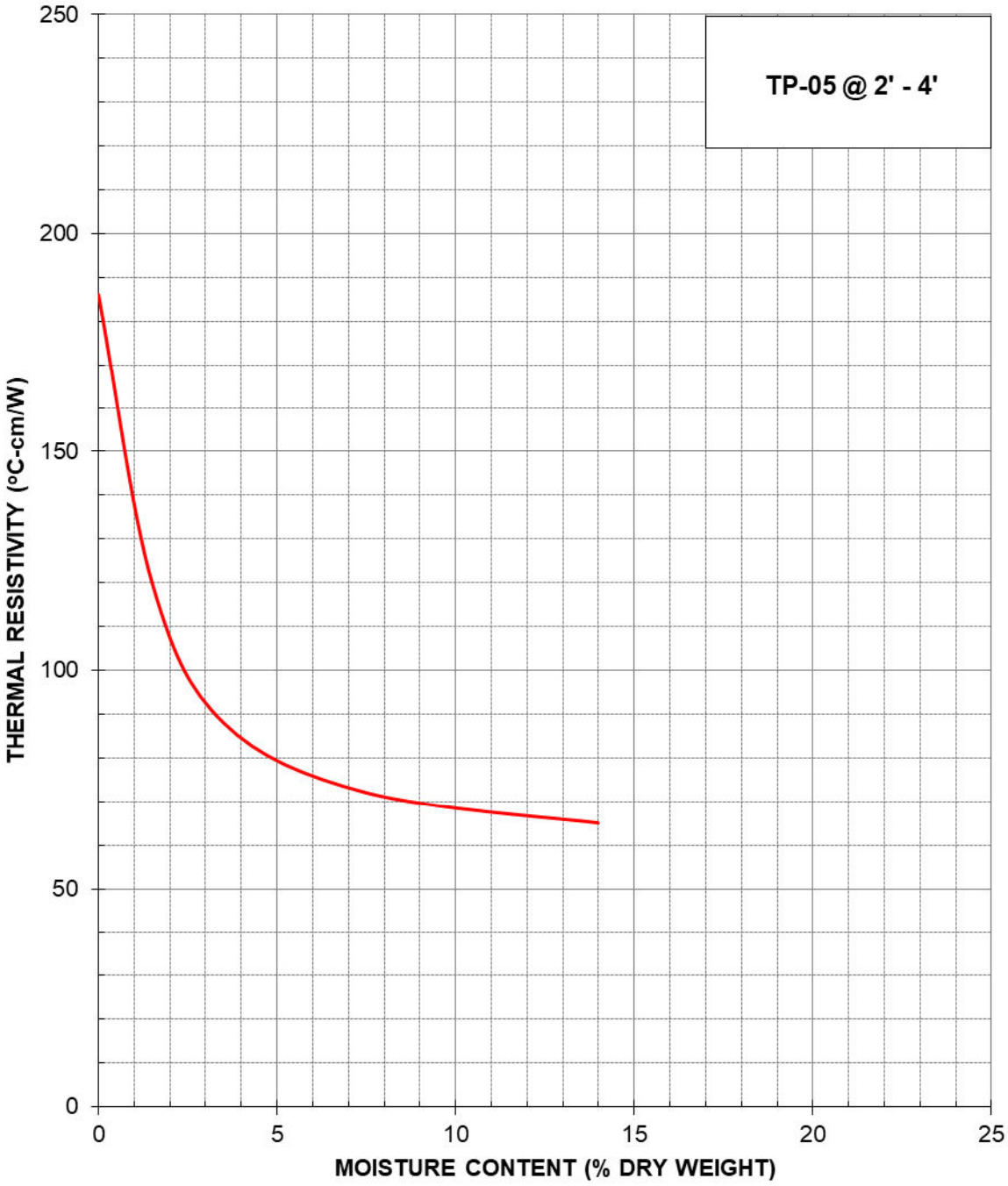
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 3**

THERMAL DRYOUT CURVE



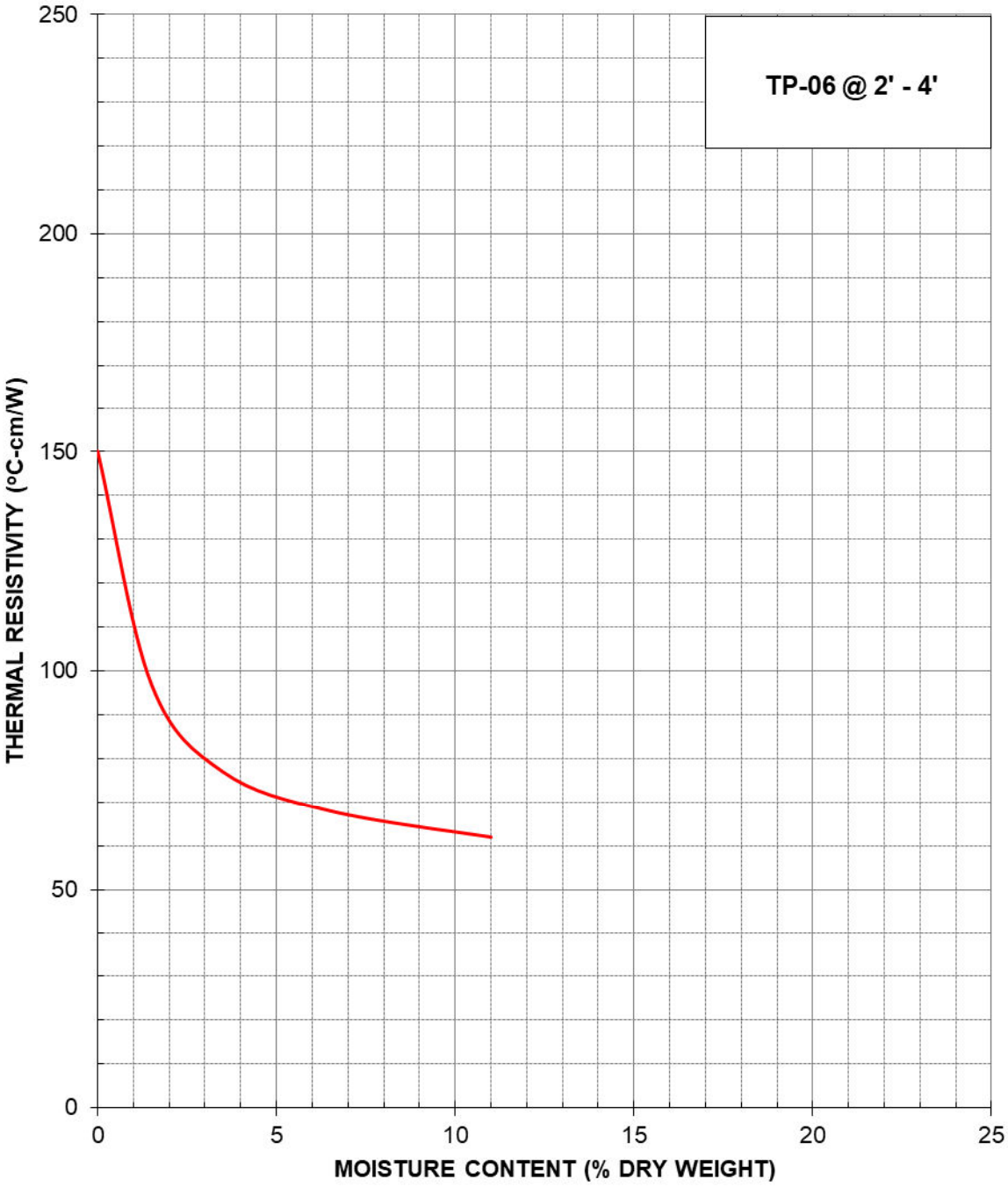
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 4**

THERMAL DRYOUT CURVE



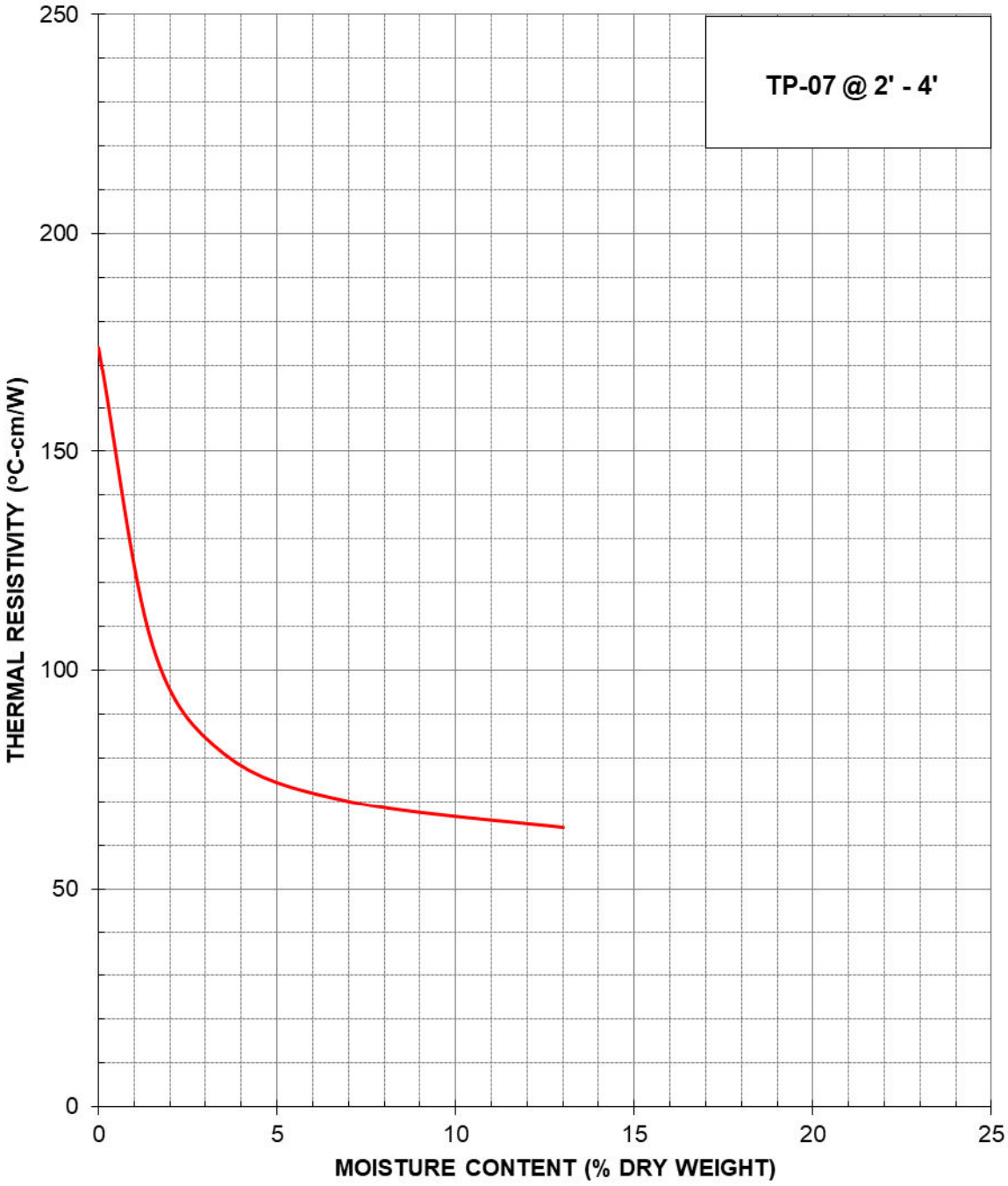
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 5**

THERMAL DRYOUT CURVE



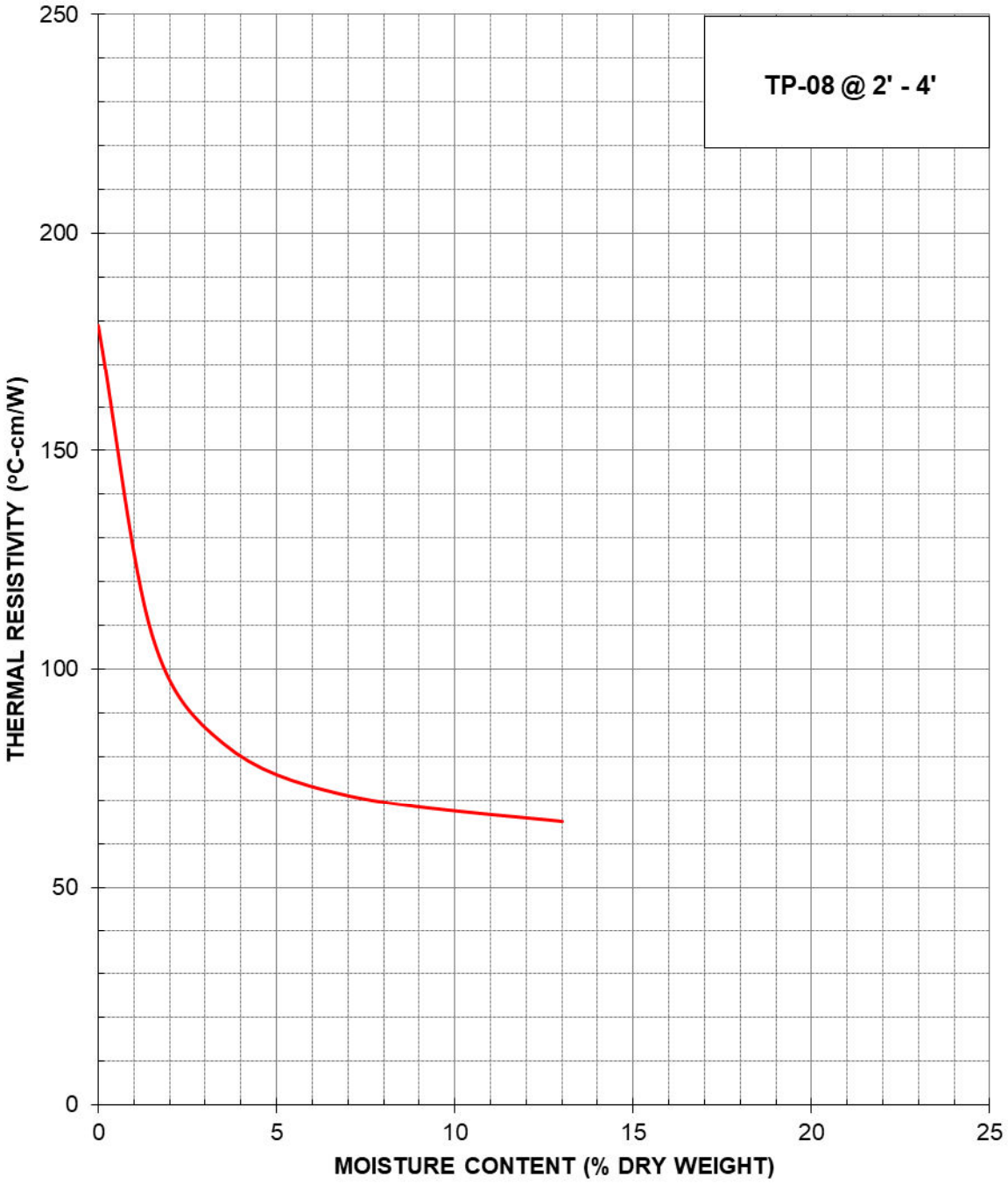
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 6**

THERMAL DRYOUT CURVE



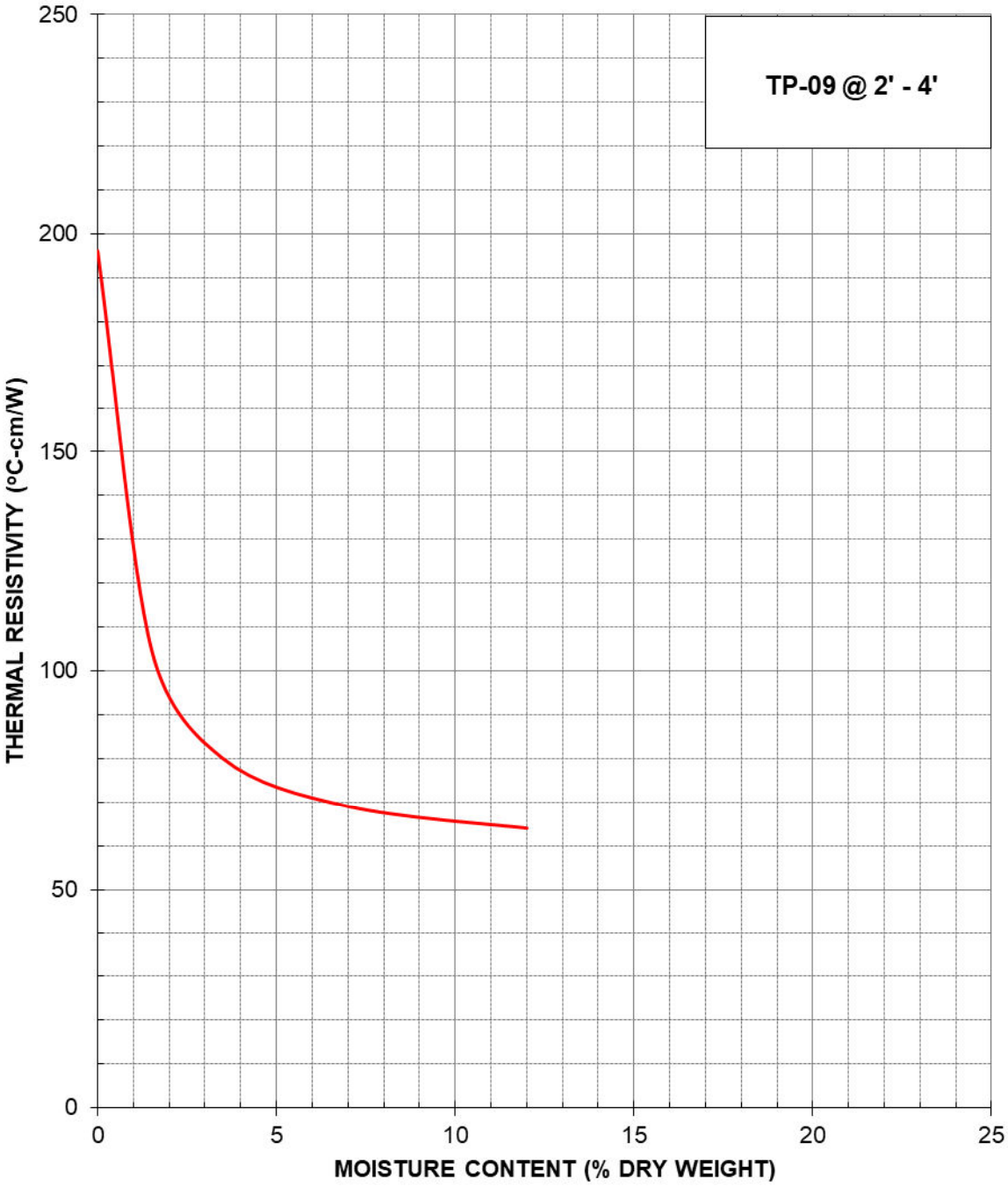
Terracon (Project No. 60225164)

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Thermal Analysis of Native Soil Sample

January 2024 **Figure 7**

THERMAL DRYOUT CURVE



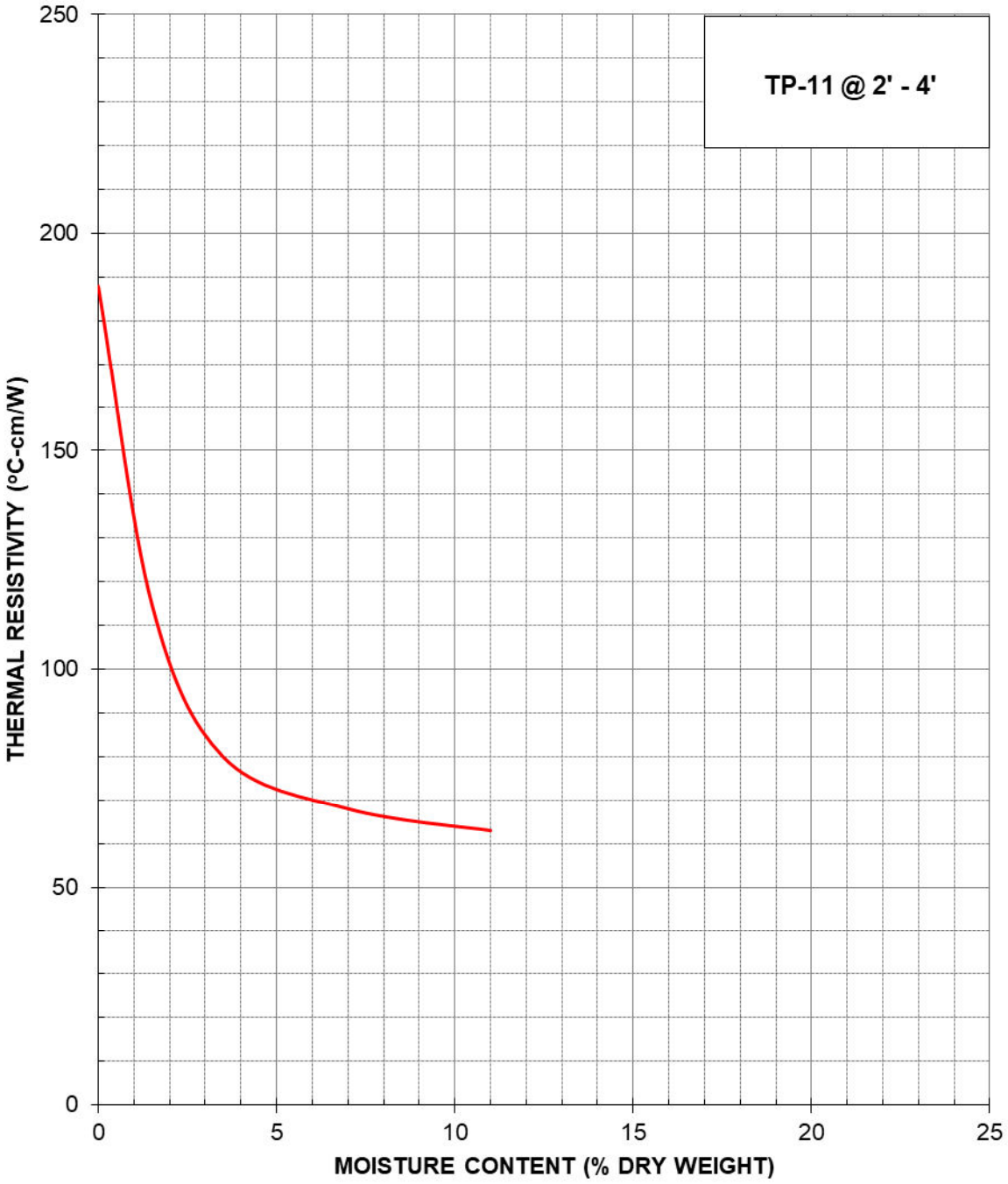
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 8**

THERMAL DRYOUT CURVE



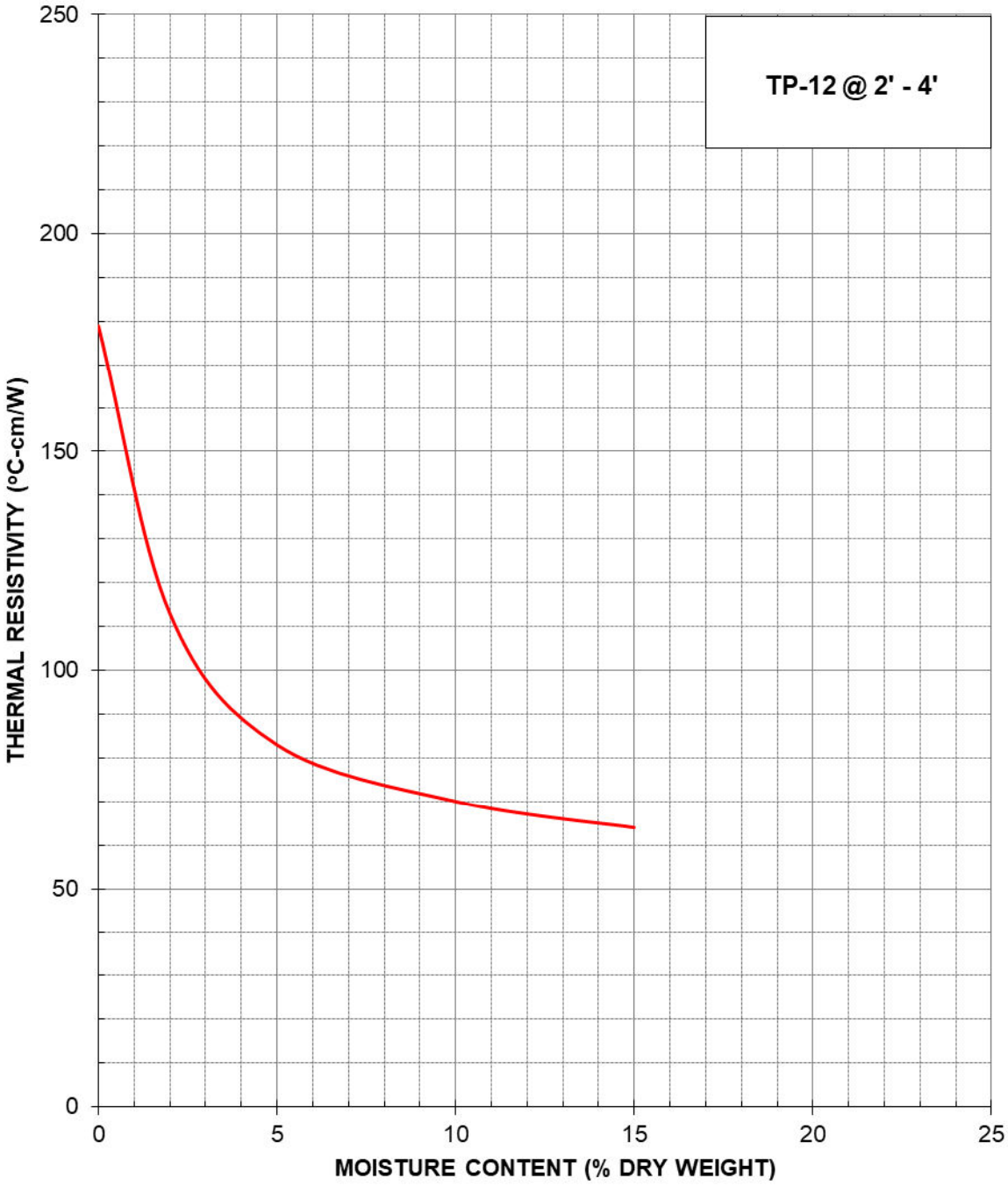
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 9**

THERMAL DRYOUT CURVE



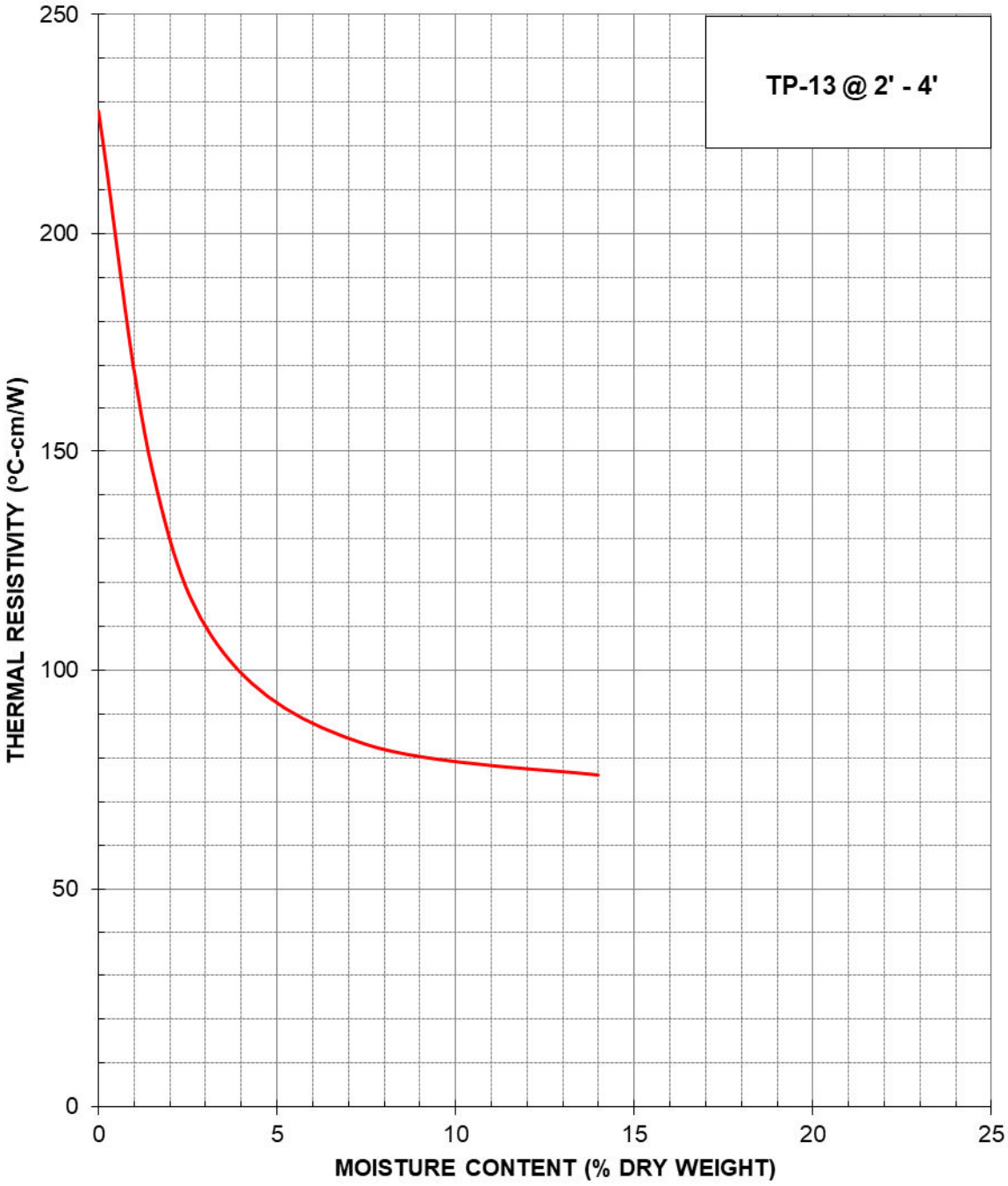
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 10**

THERMAL DRYOUT CURVE



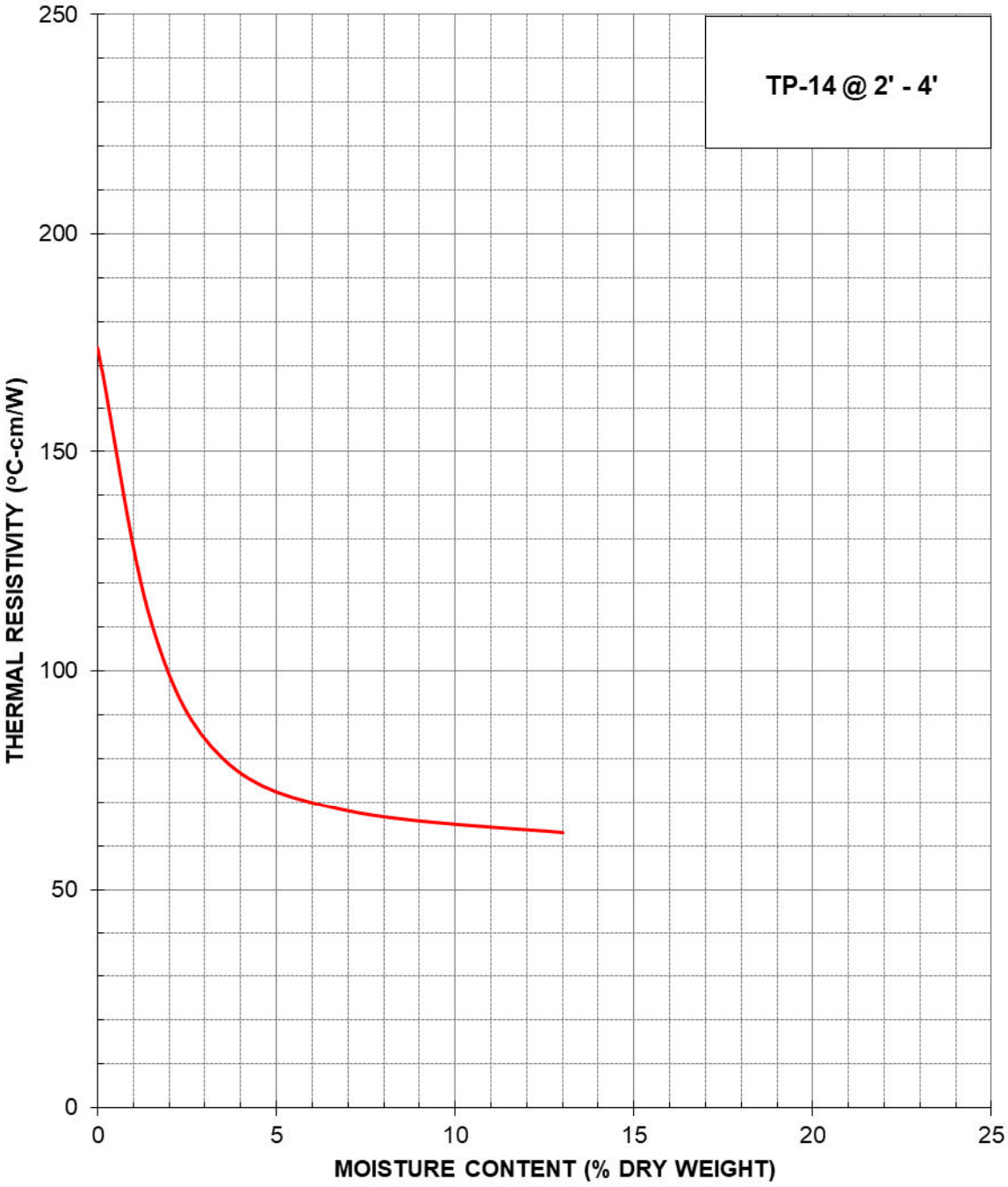
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 11**

THERMAL DRYOUT CURVE



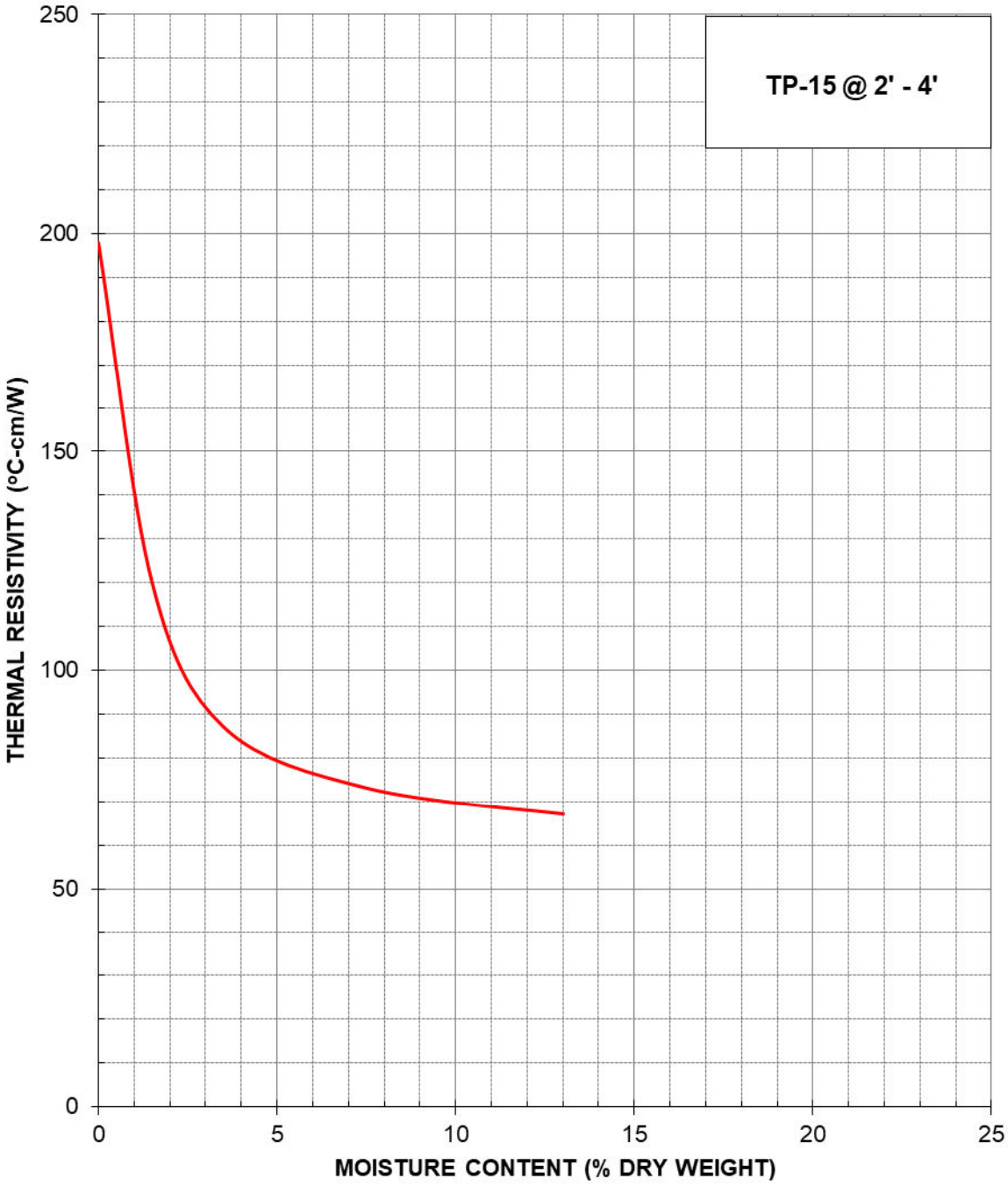
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 12**

THERMAL DRYOUT CURVE



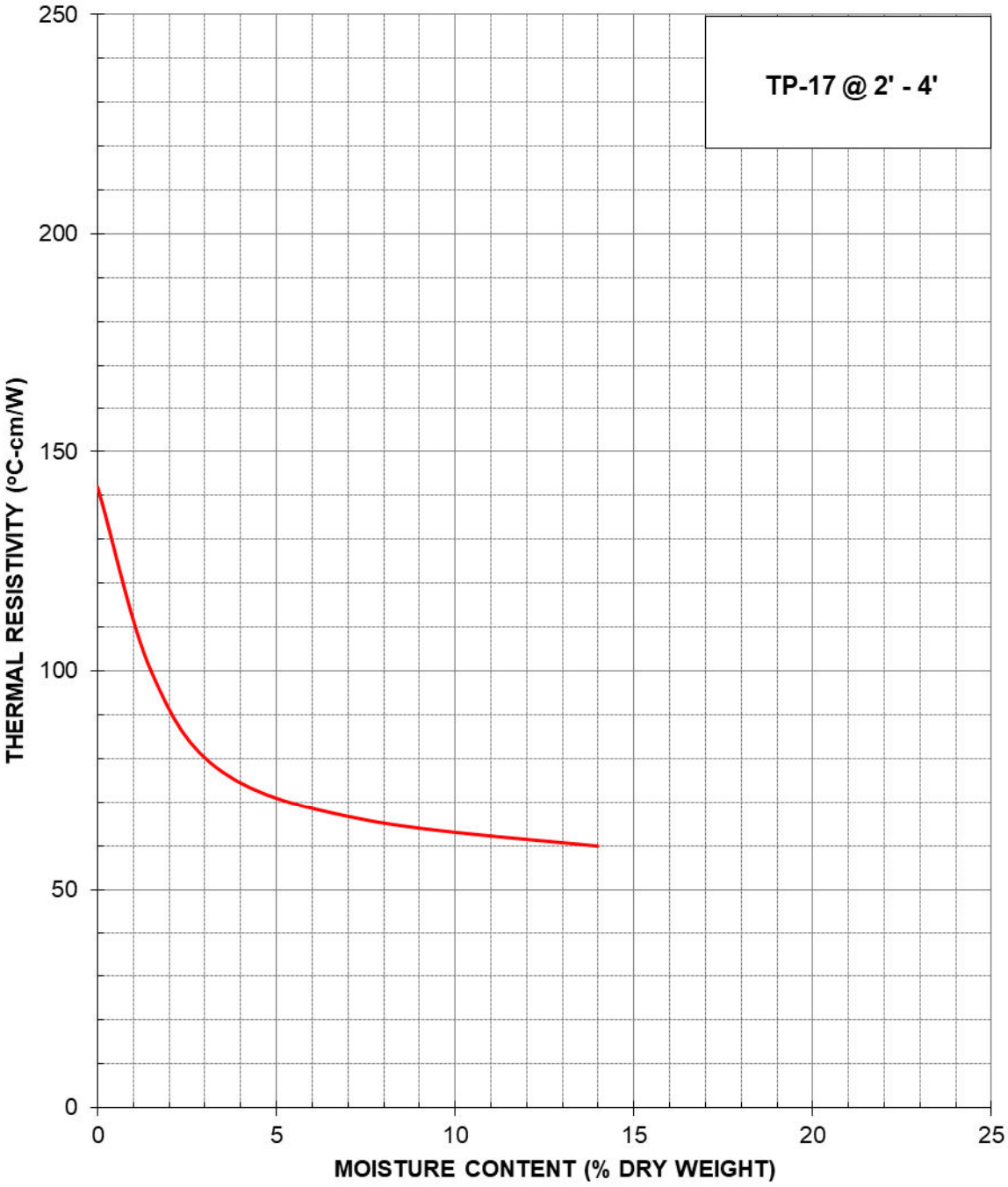
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 13**

THERMAL DRYOUT CURVE



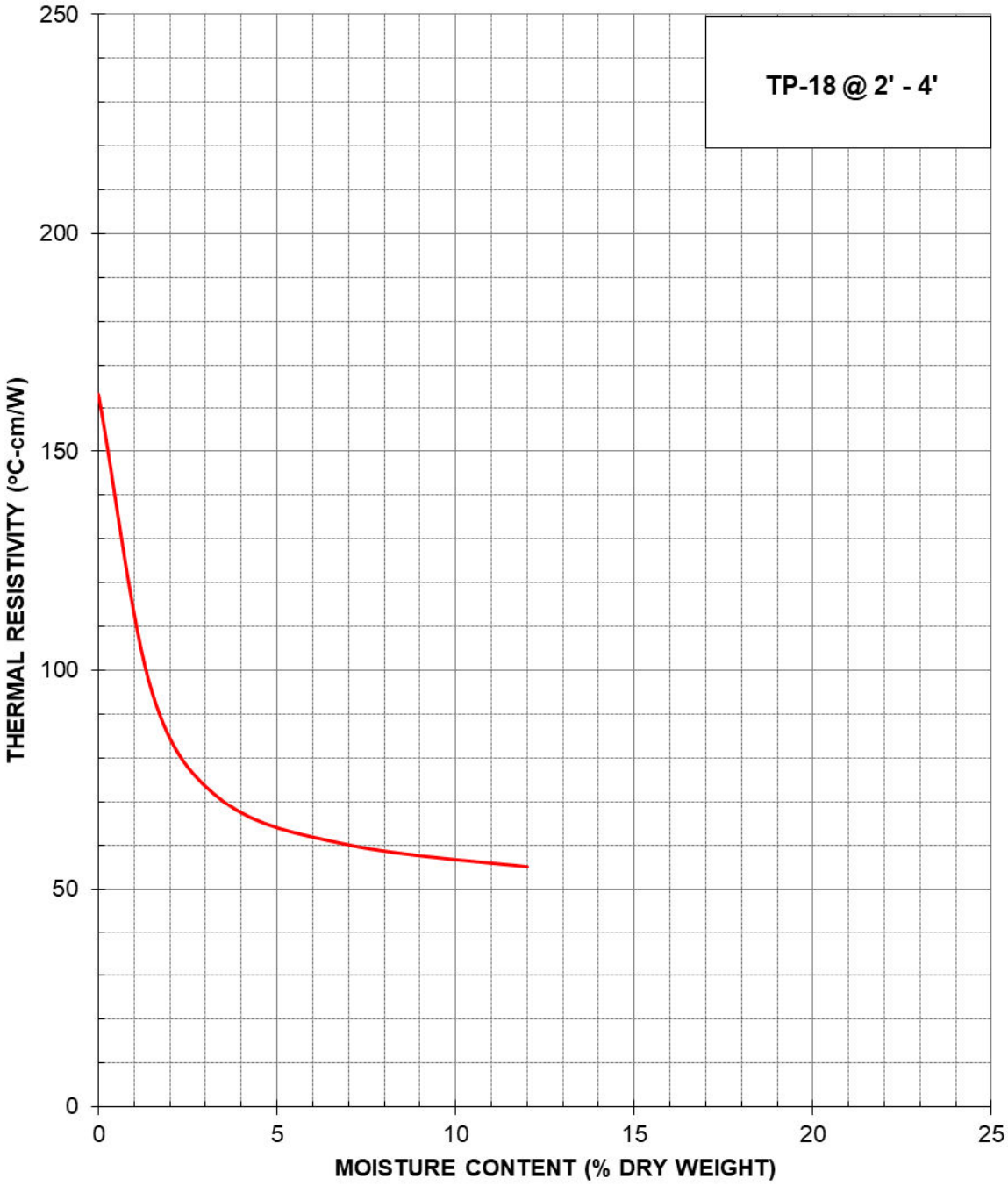
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 14**

THERMAL DRYOUT CURVE



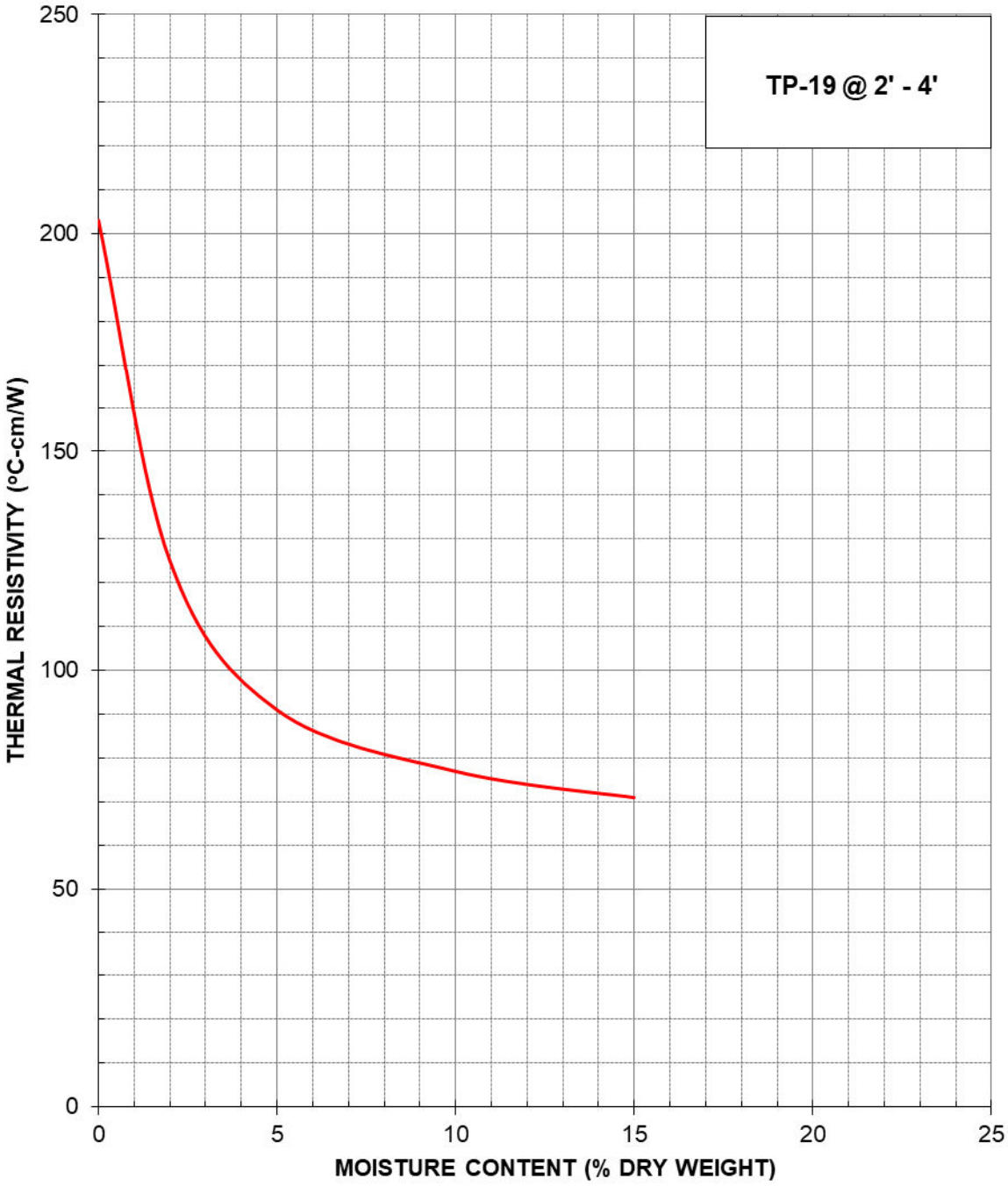
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 15**

THERMAL DRYOUT CURVE



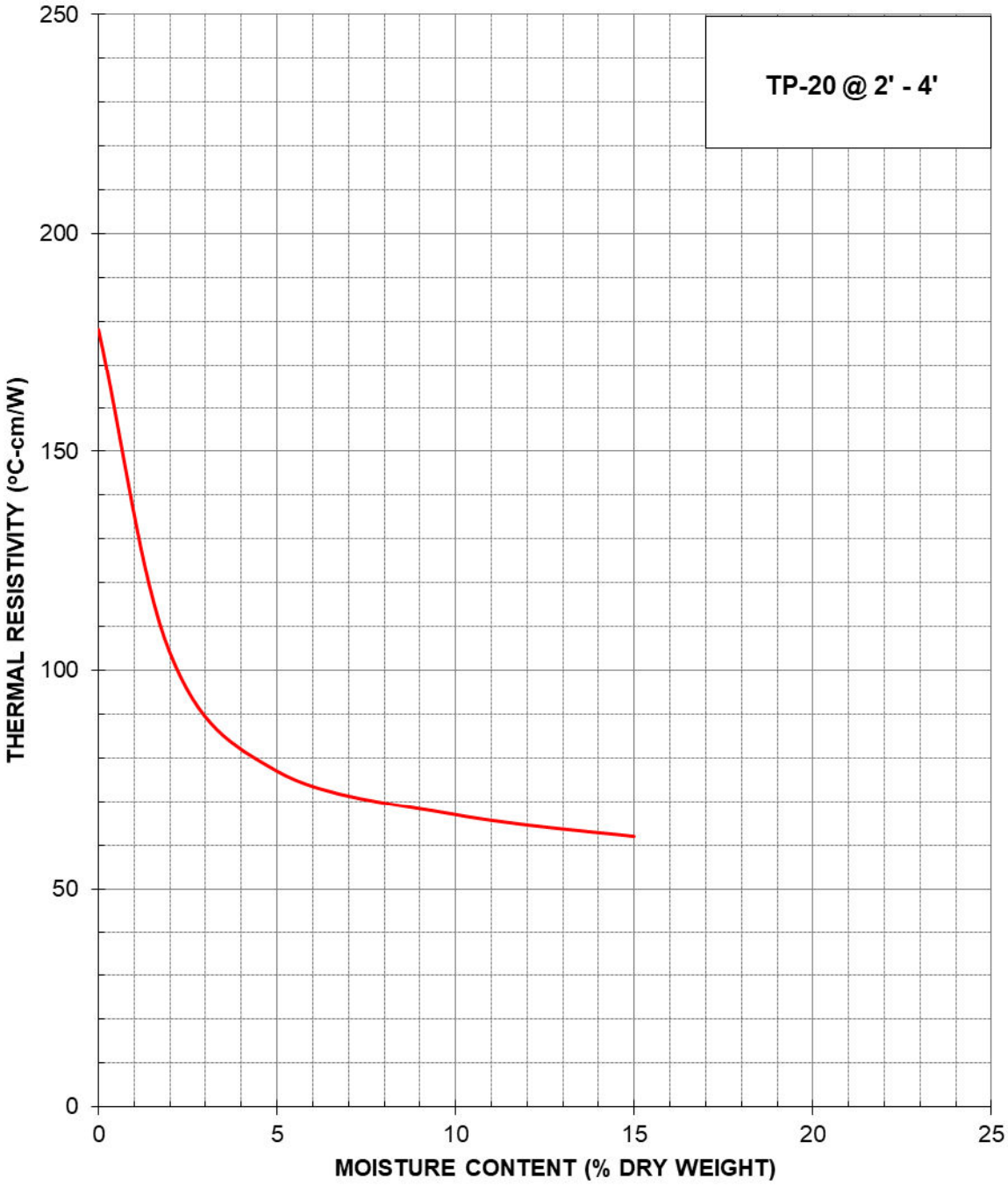
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 16**

THERMAL DRYOUT CURVE



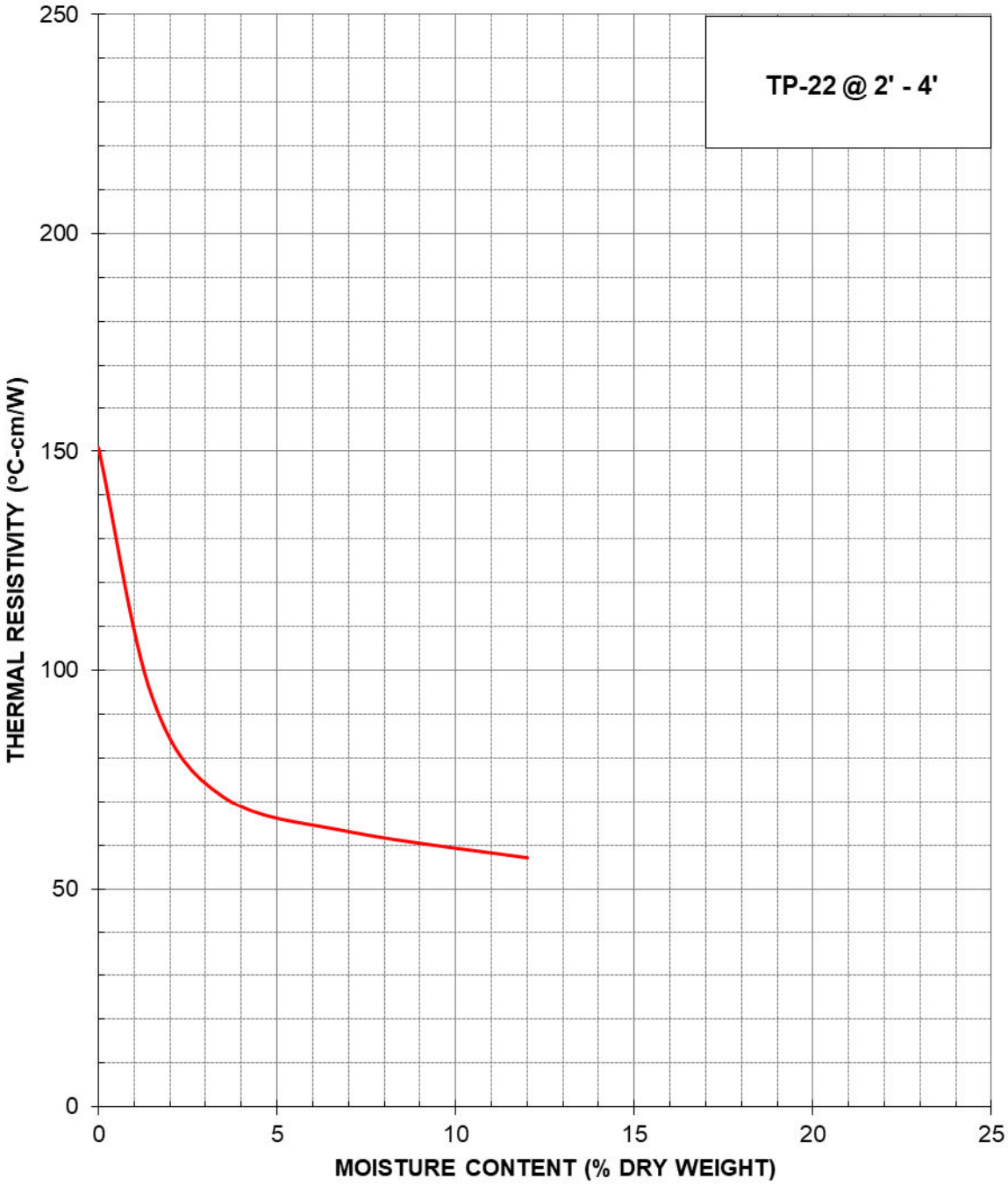
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 17**

THERMAL DRYOUT CURVE



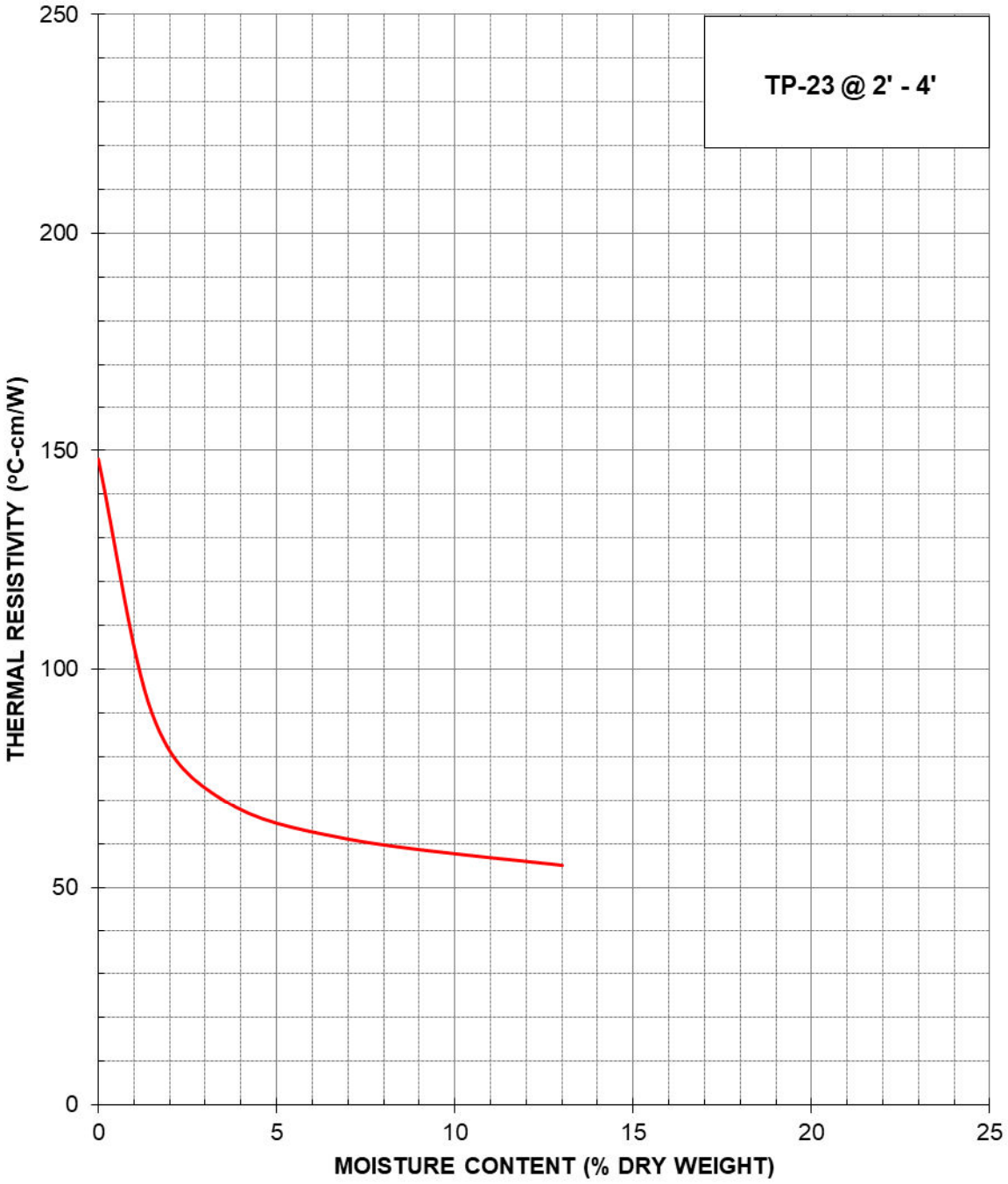
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 18**

THERMAL DRYOUT CURVE



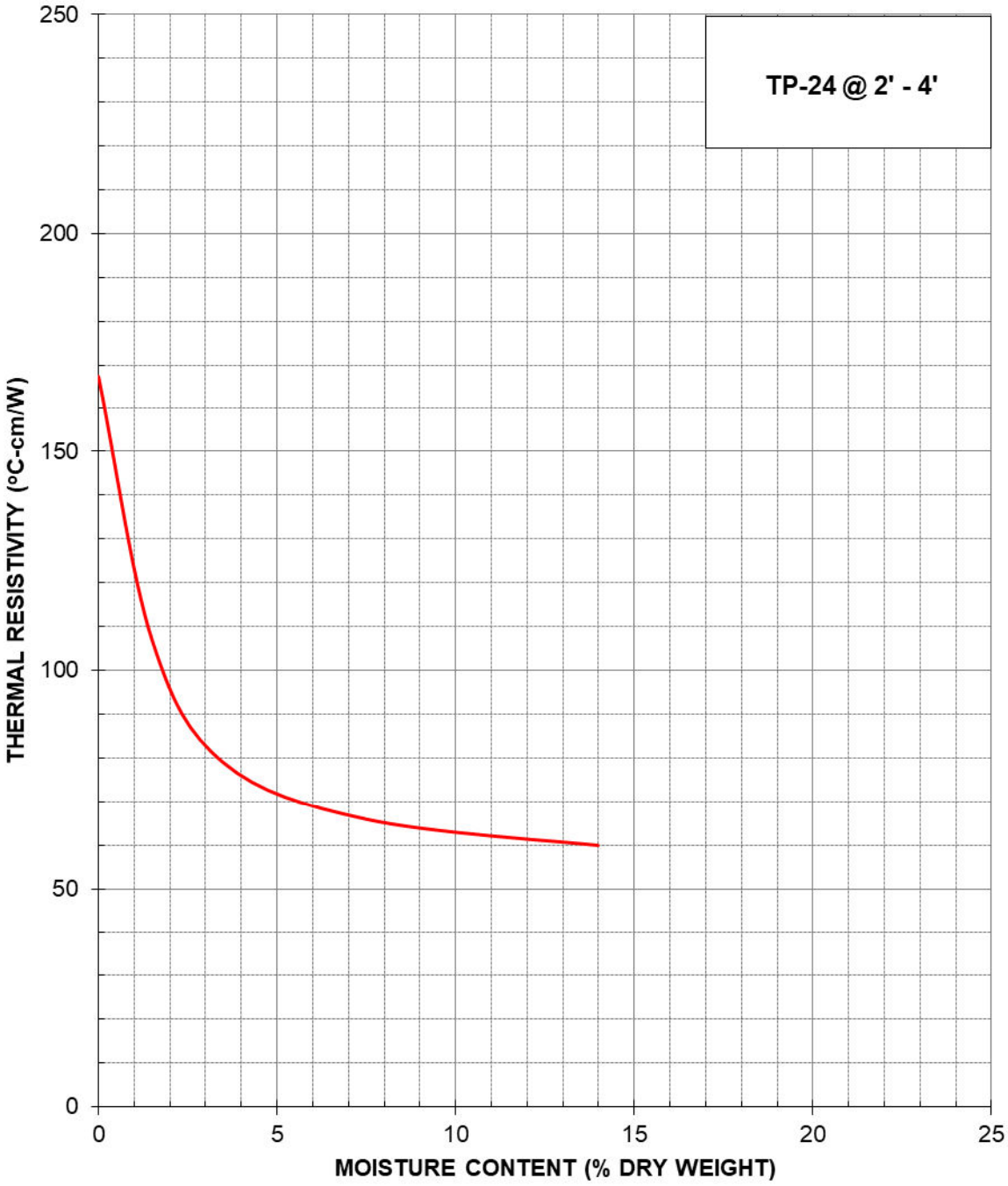
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 19**

THERMAL DRYOUT CURVE



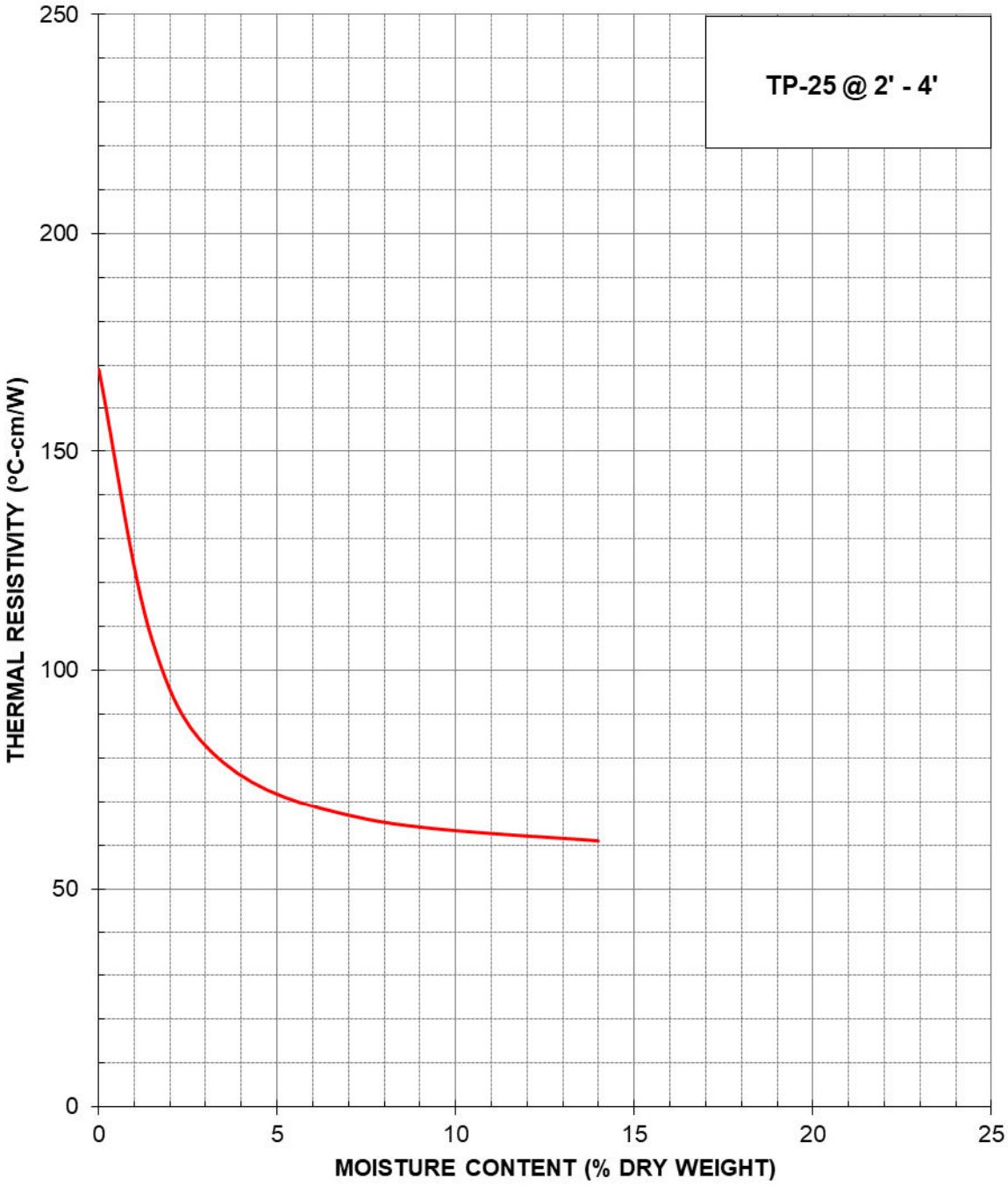
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 20**

THERMAL DRYOUT CURVE



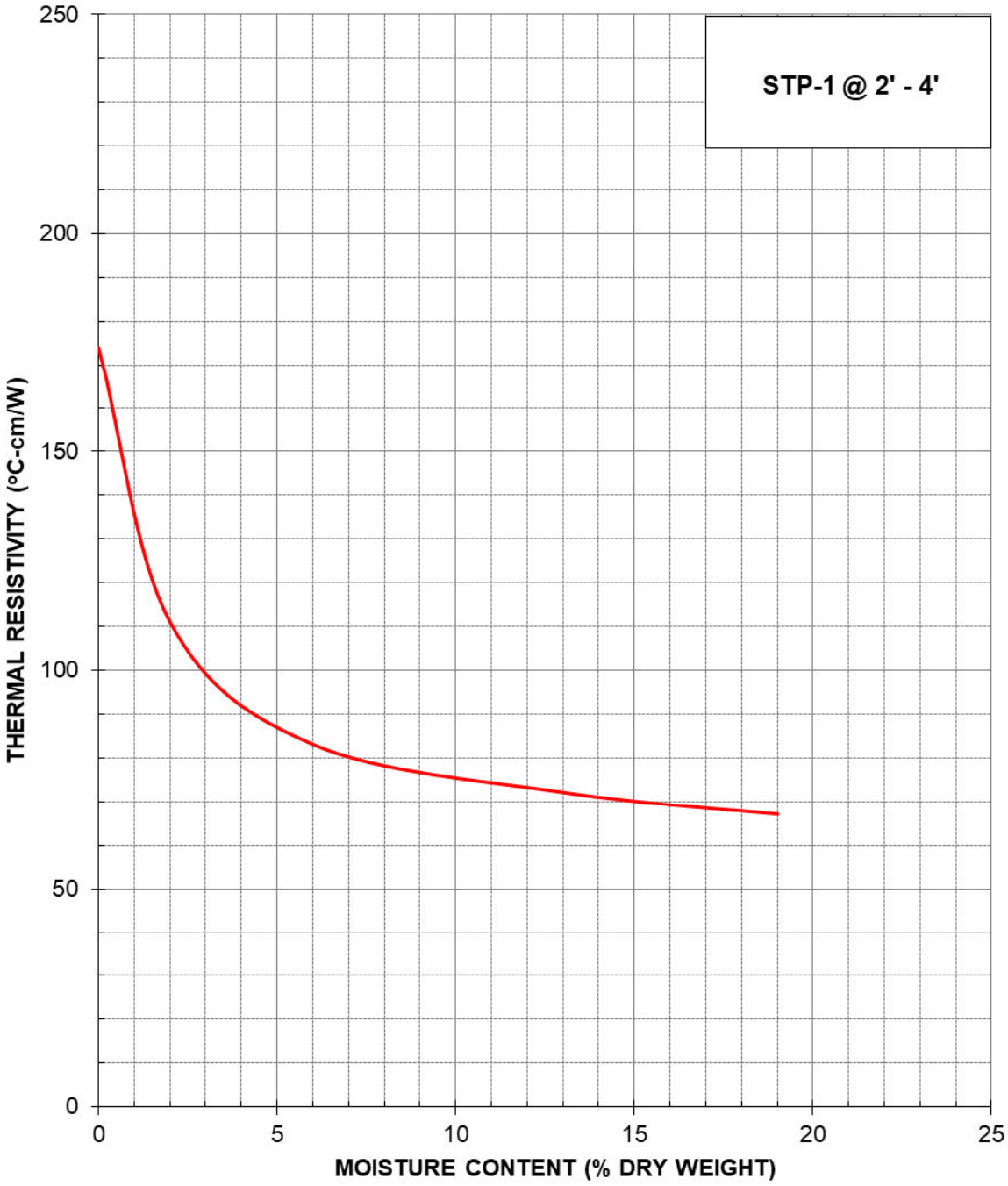
Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 21**

THERMAL DRYOUT CURVE



Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Analysis of Native Soil Sample

January 2024 **Figure 22**

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January 3, 2024 (Revised December 25, 2025)

Terracon
23041 Avenida de la Carlota, Suite 350
Laguna Hills, CA 92653
Attn: Victor V. Nguyen

Re: Thermal Analysis of Native Soil Samples
Grace Solar – Blythe, Riverside County, CA (Project No. 60225164)

The following is the report of thermal dryout characterization tests conducted on twenty (20) native soil samples from the referenced project sent to our laboratory.

Thermal Resistivity Tests The bulk samples were tested at either the optimum or in-situ moisture content, whichever was greater and at 90% of the standard Proctor dry density ***provided by Terracon***. The tests were conducted in accordance with the IEEE standard 442-2017. The results are tabulated below and the thermal dryout curves are presented in **Figures 1 to 20.**

Sample ID, Description, Thermal Resistivity, Moisture Content and Density

Sample ID	Depth (ft)	Effort (%)	Description (Terracon)	Thermal Resistivity (°C-cm/W)		Moisture Content (%)	Dry Density (lb/ft^3)
				Wet	Dry		
BH-01	1 - 4	90	Clayey Sand	53	156	9	109
BH-03	1 - 4	90	Silty Clayey Sand, Brown	44	160	11	113
BH-04	1 - 4	90	Silty F-C Sand trace of gravel, Brown	50	171	10	111
BH-06	1 - 4	90	Silty fine to medium Sand, Brown	49	154	9	114
BH-09	1 - 4	90	Silty F-C Sand trace of gravel, Brown	48	142	8	116
BH-11	1 - 4	90	Silty F-C Sand, Brown	56	186	11	107

Sample ID, Description, Thermal Resistivity, Moisture Content and Density

Sample ID	Depth (ft)	Effort (%)	Description (Terracon)	Thermal Resistivity (°C-cm/W)		Moisture Content (%)	Dry Density (lb/ft^3)
				Wet	Dry		
BH-12	1 - 4	90	Silty F-C Sand, Brown	46	164	10	111
BH-14	1 - 4	90	Silty F-M Sand, Brown	47	168	10	111
BH-19	1 - 4	90	Silty Sand trace of Clay, Brown	53	181	11	108
BH-21	1 - 4	90	Silty F-C Sand, Brown	54	164	8	113
BH-24	1 - 4	90	Silty F-M Sand, Brown	48	170	10	110
BH-25	1 - 4	90	Silty Sand, Brown	52	192	9	107
BH-27	2 - 4	90	Silty F-M Sand, Brown	43	151	10	112
BH-29	1 - 4	90	Silty Sand trace of Clay, Brown	47	165	11	111
BH-31	1 - 4	90	Silty F-C Sand, Brown	49	147	10	113
BH-33	1 - 4	90	Silty F-C Sand trace of gravel, Brown	49	141	10	114
BH-35	1 - 4	90	Silty F-M Sand, Brown	46	169	10	110
BH-39	1 - 4	90	Silty F-C Sand, Brown	44	162	10	111
BH-41	1 - 4	90	Silty Sand, Brown	45	187	10	108
SBH-01	1 - 4	90	Clayey Sand	43	134	10	115

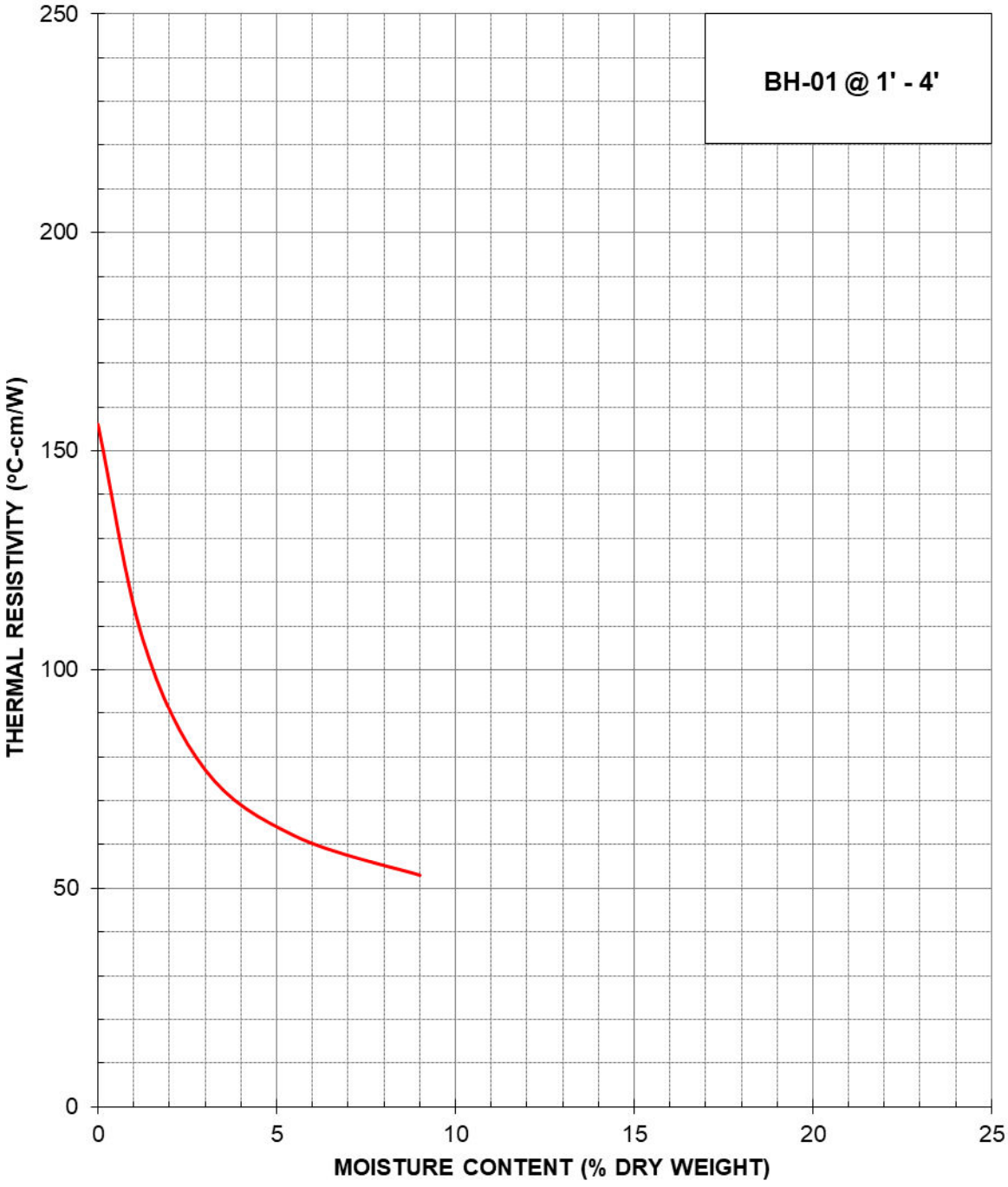
Please contact us if you have any questions or if we can be of further assistance.

Geotherm USA



Nimesh Patel

THERMAL DRYOUT CURVE



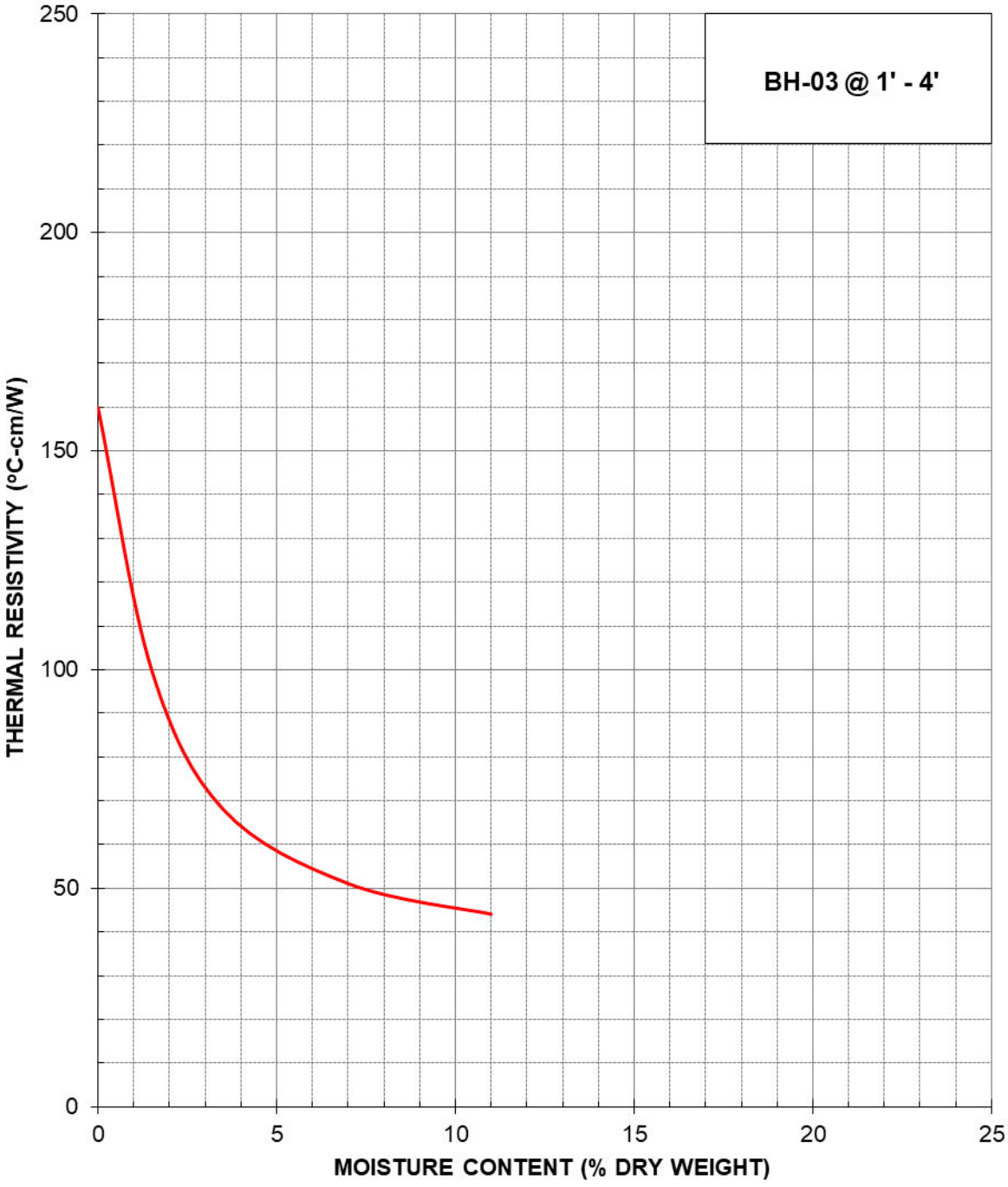
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 1**

THERMAL DRYOUT CURVE



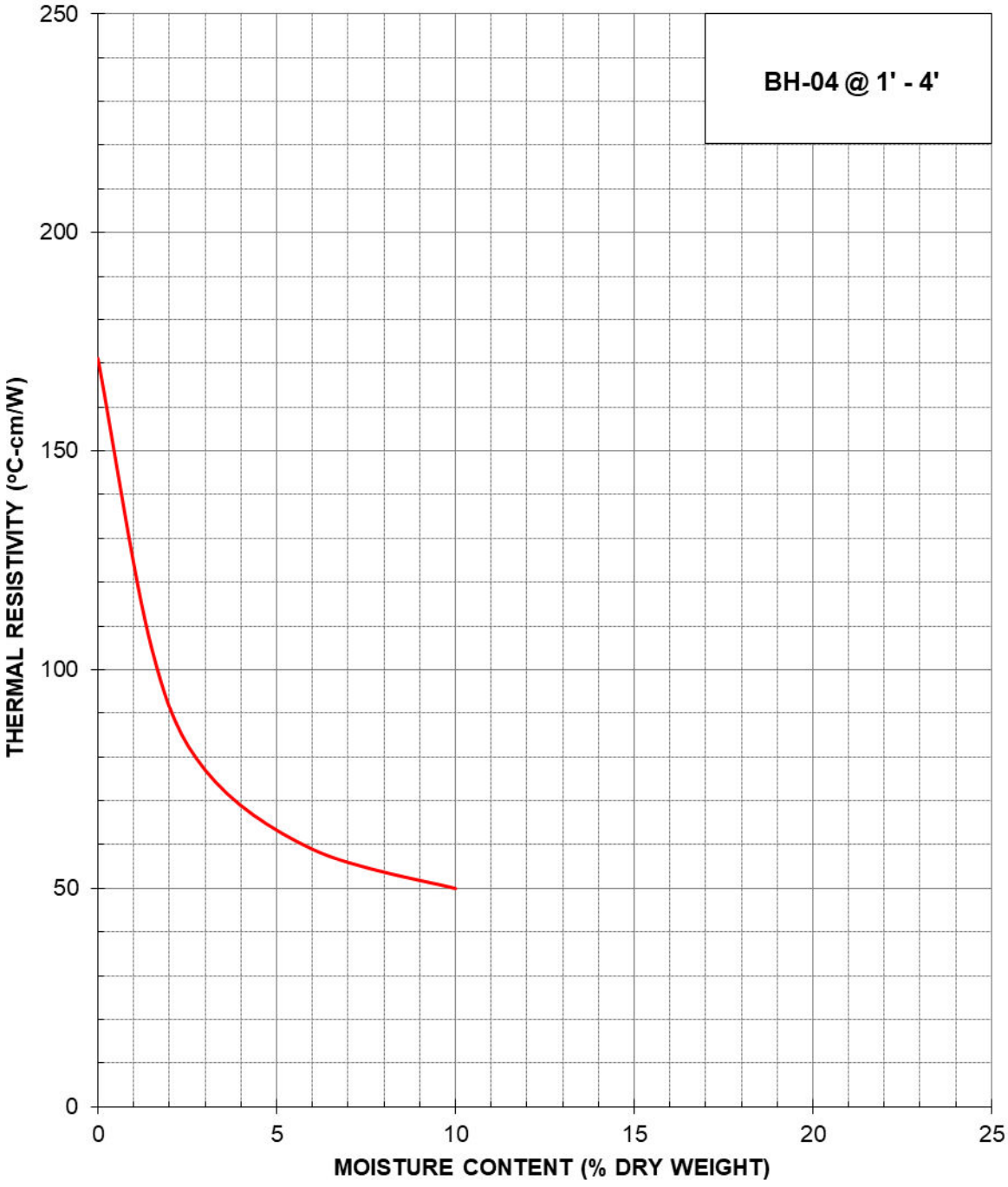
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 2**

THERMAL DRYOUT CURVE



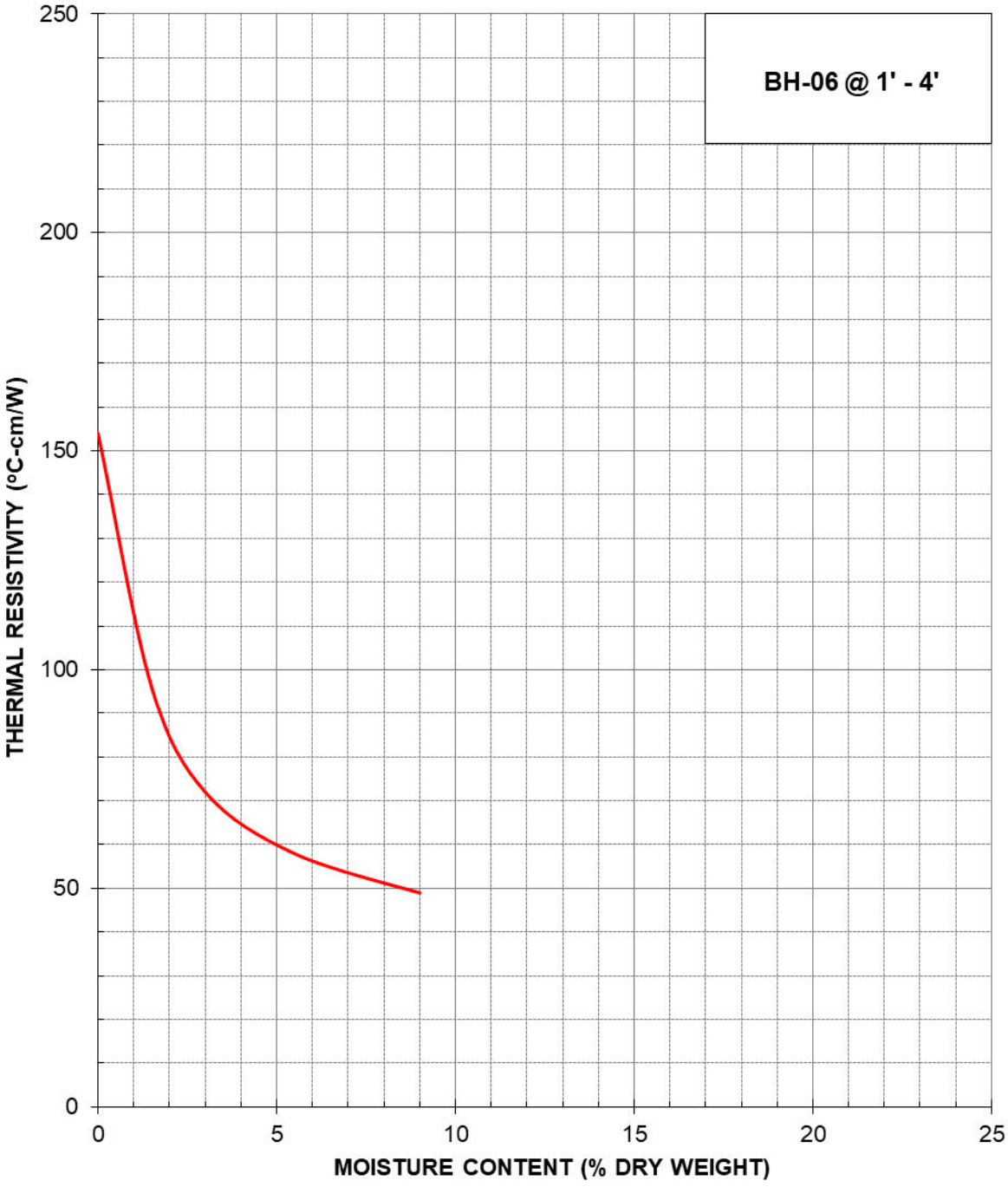
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 3**

THERMAL DRYOUT CURVE



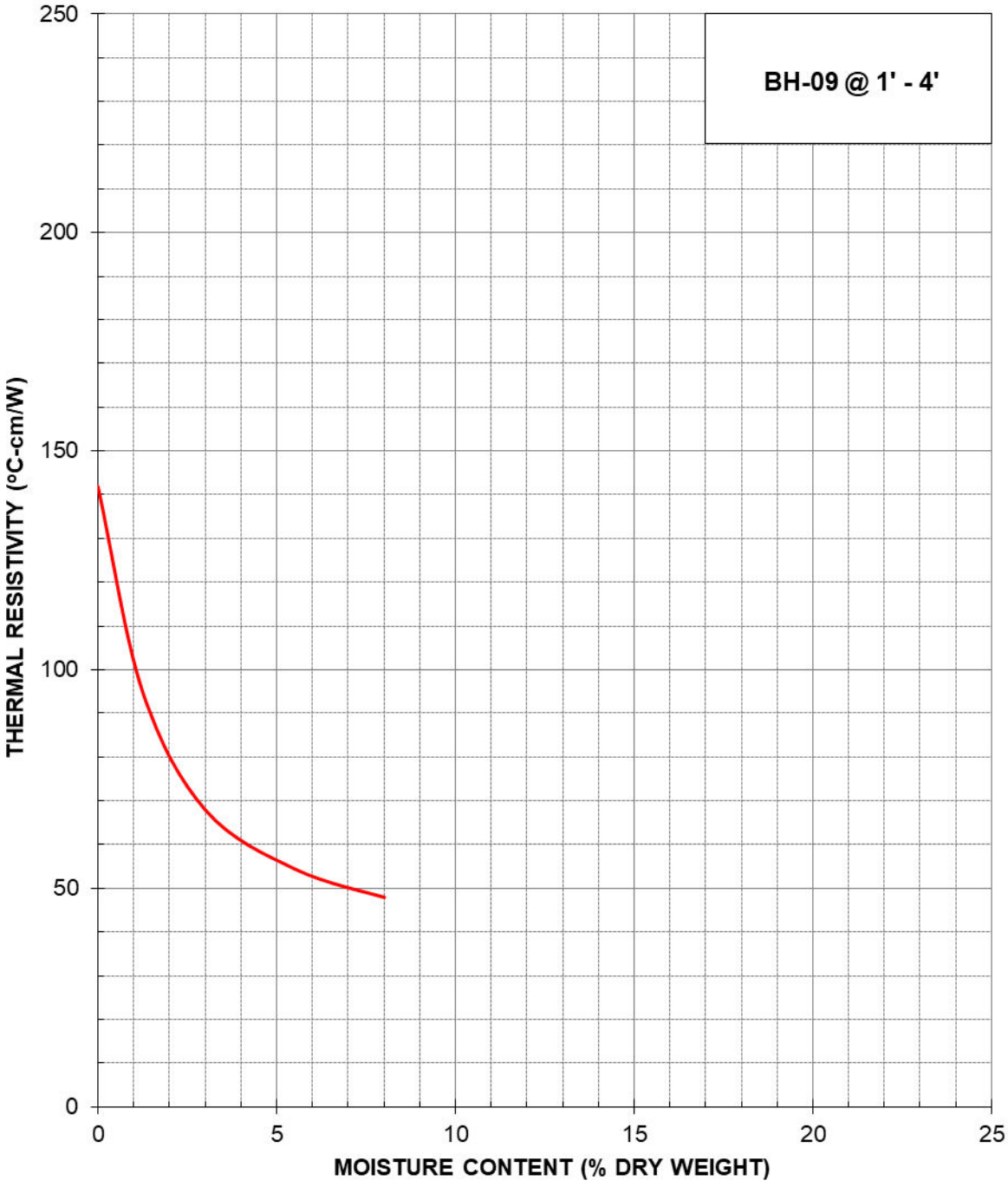
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 4**

THERMAL DRYOUT CURVE



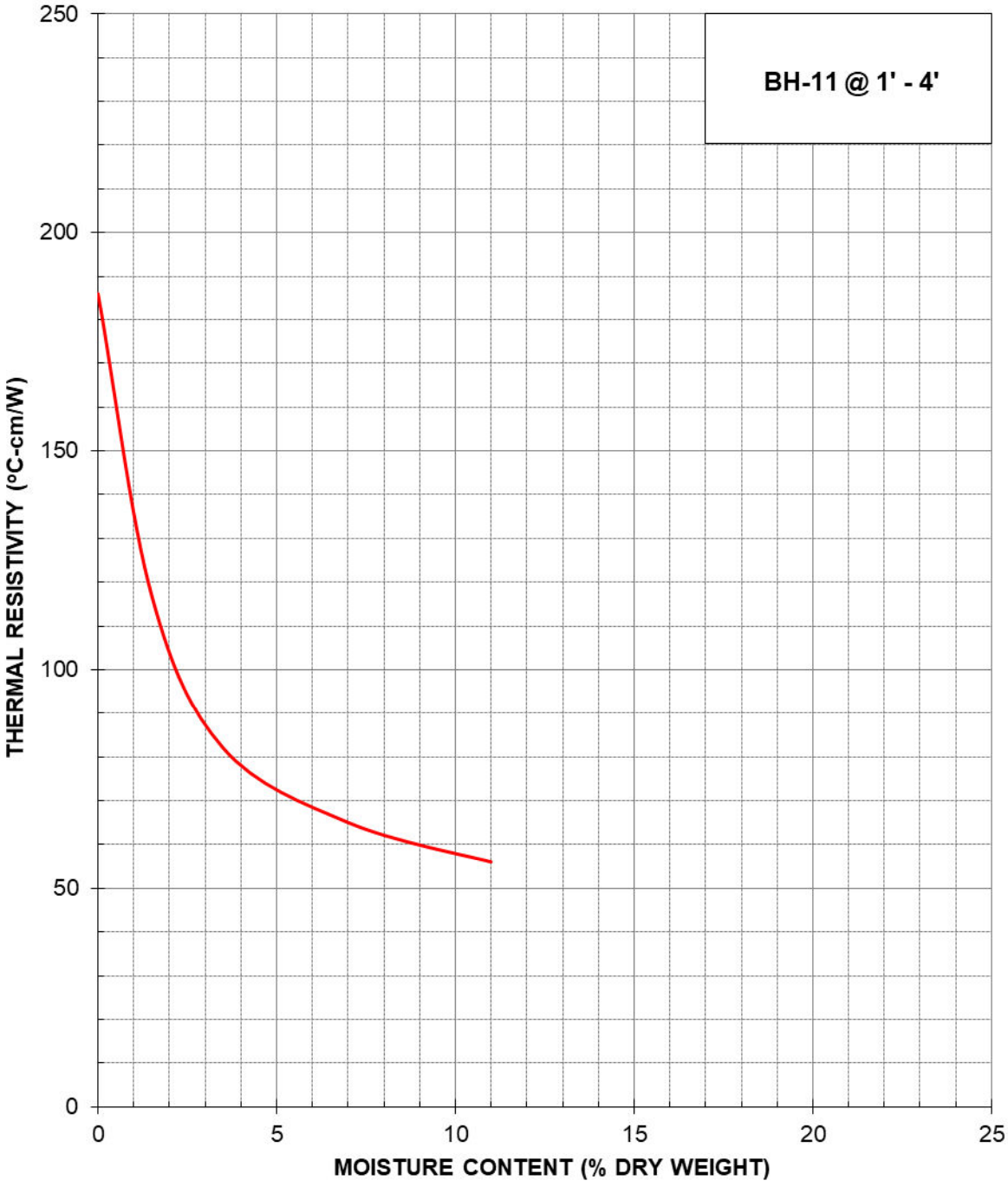
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 5**

THERMAL DRYOUT CURVE



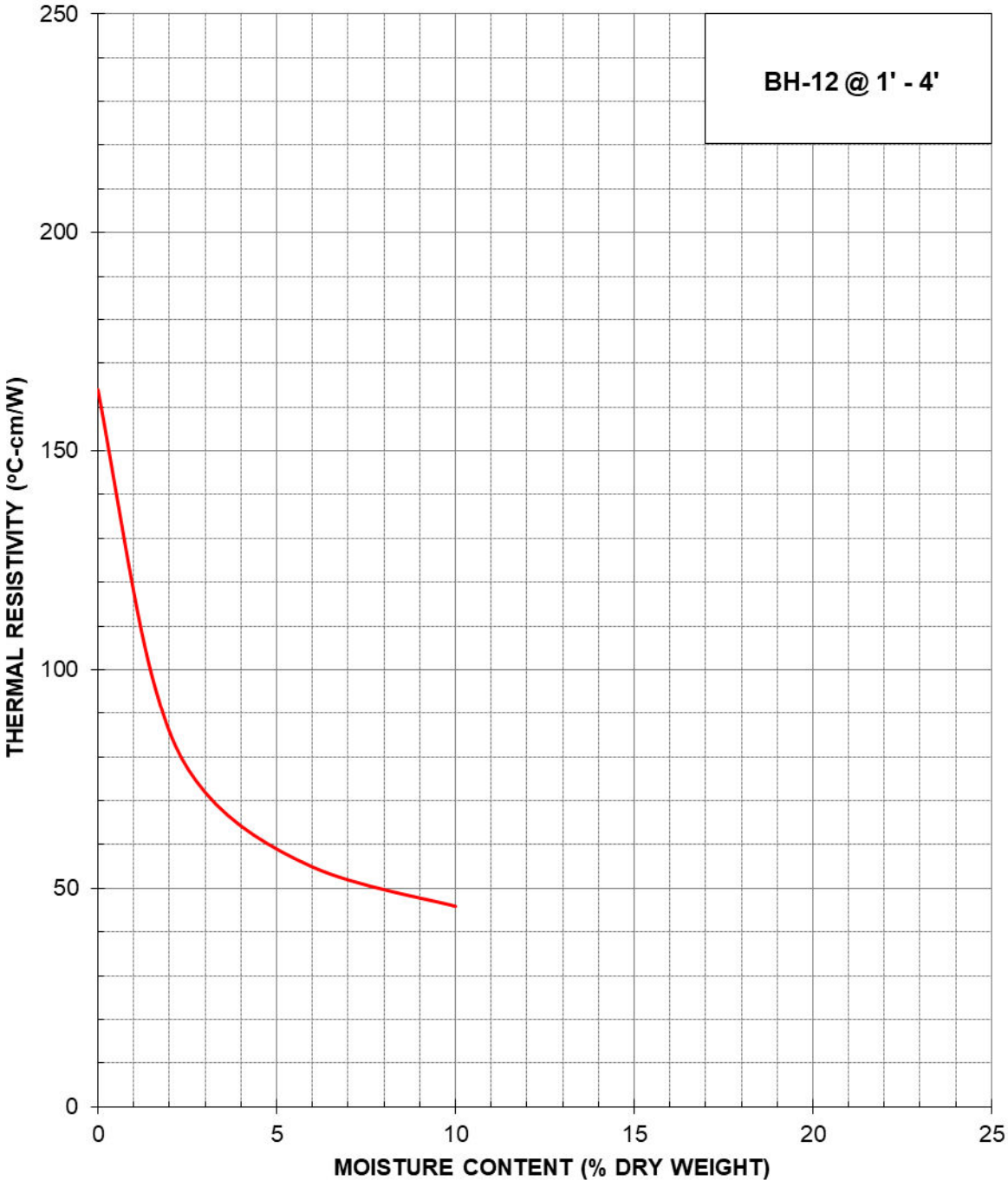
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 6**

THERMAL DRYOUT CURVE



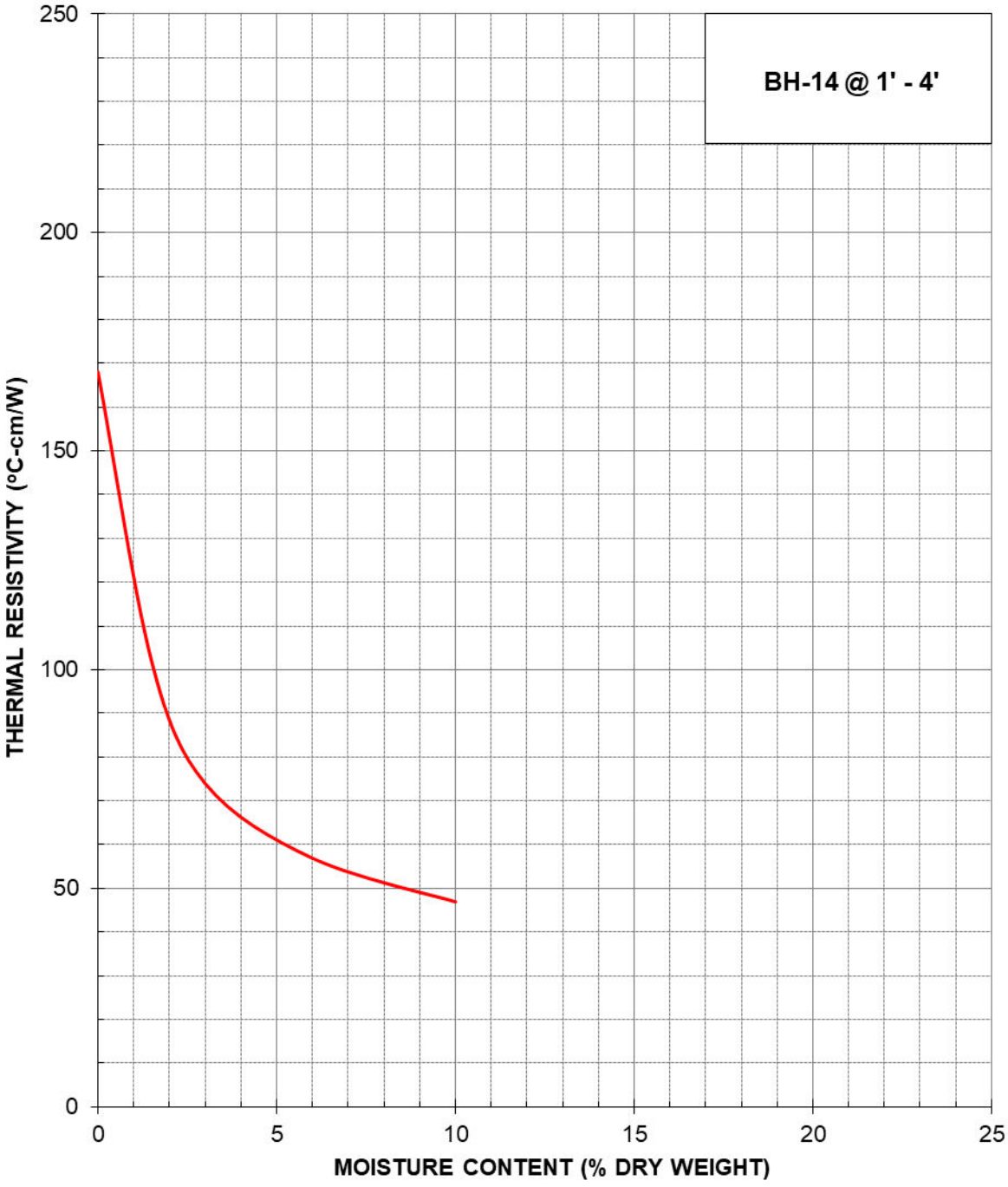
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 7**

THERMAL DRYOUT CURVE



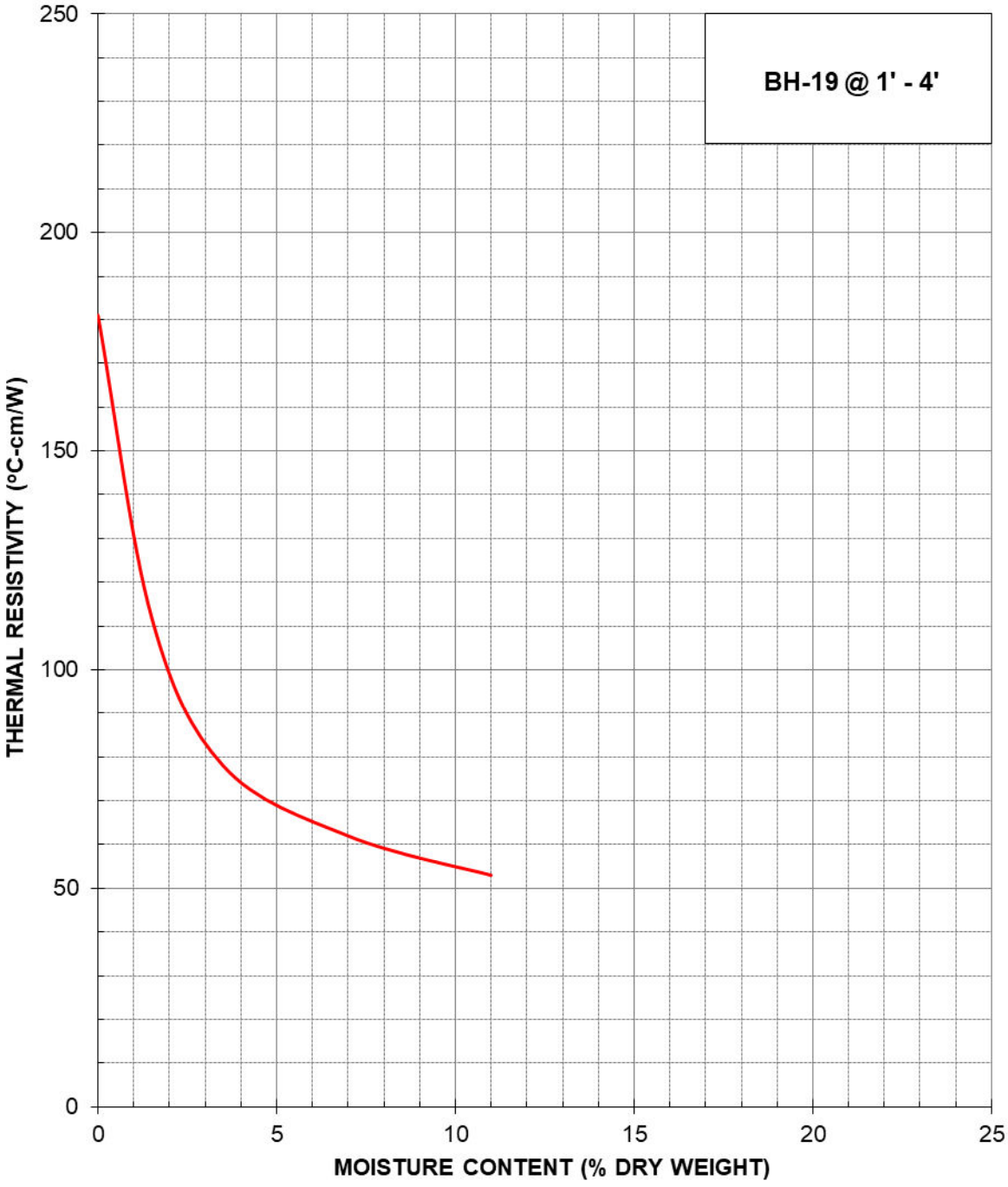
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 8**

THERMAL DRYOUT CURVE



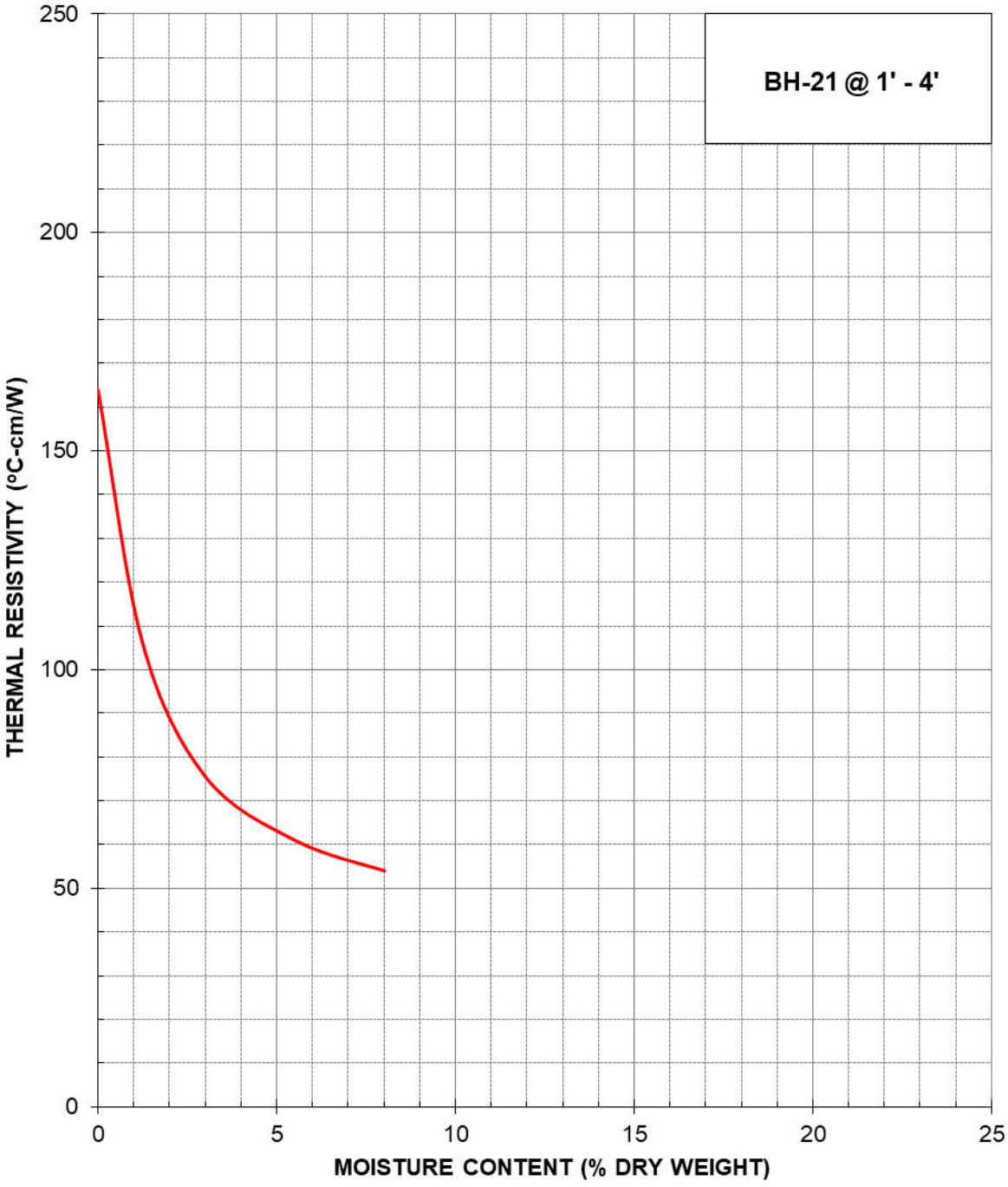
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 9**

THERMAL DRYOUT CURVE



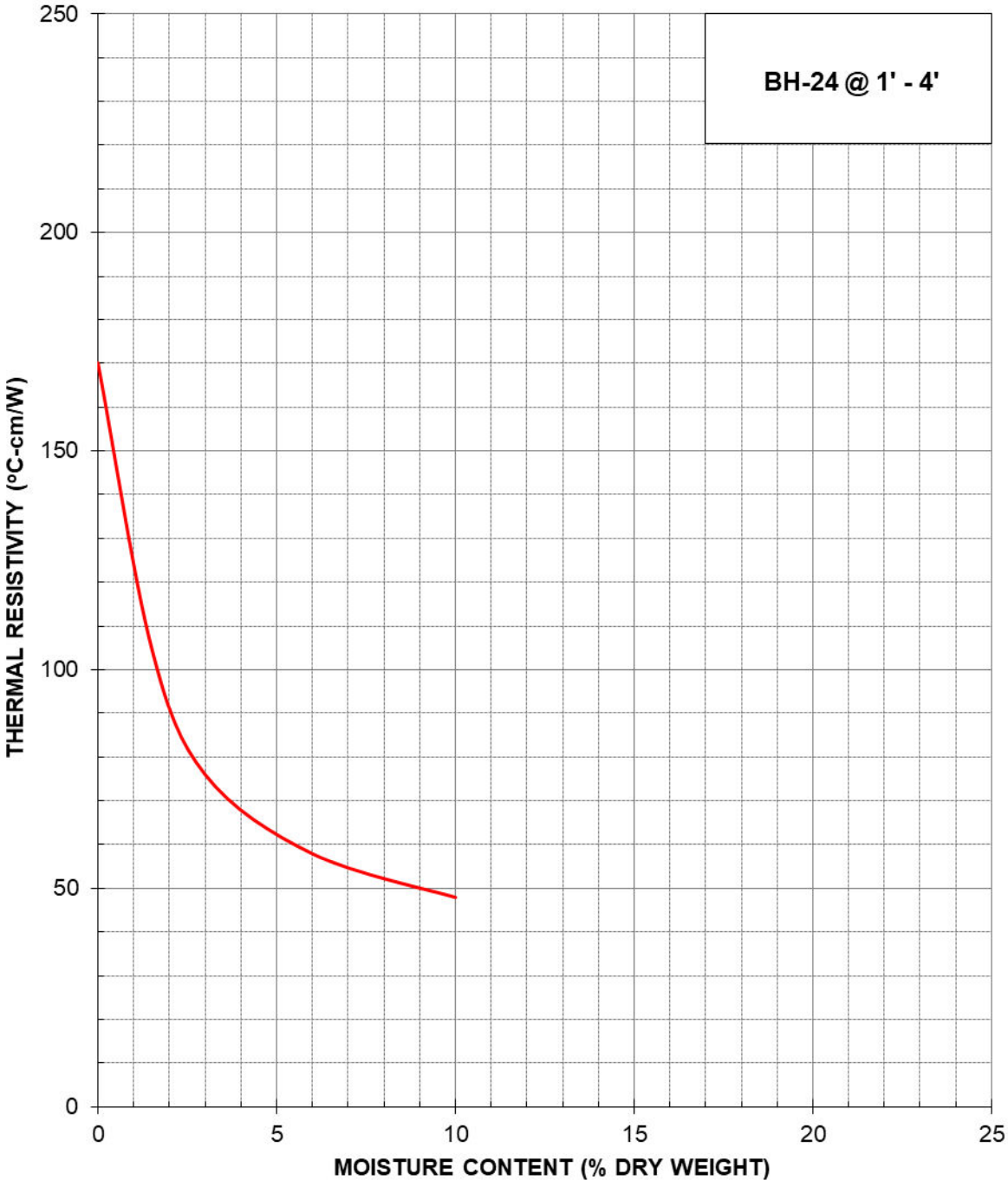
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 10**

THERMAL DRYOUT CURVE



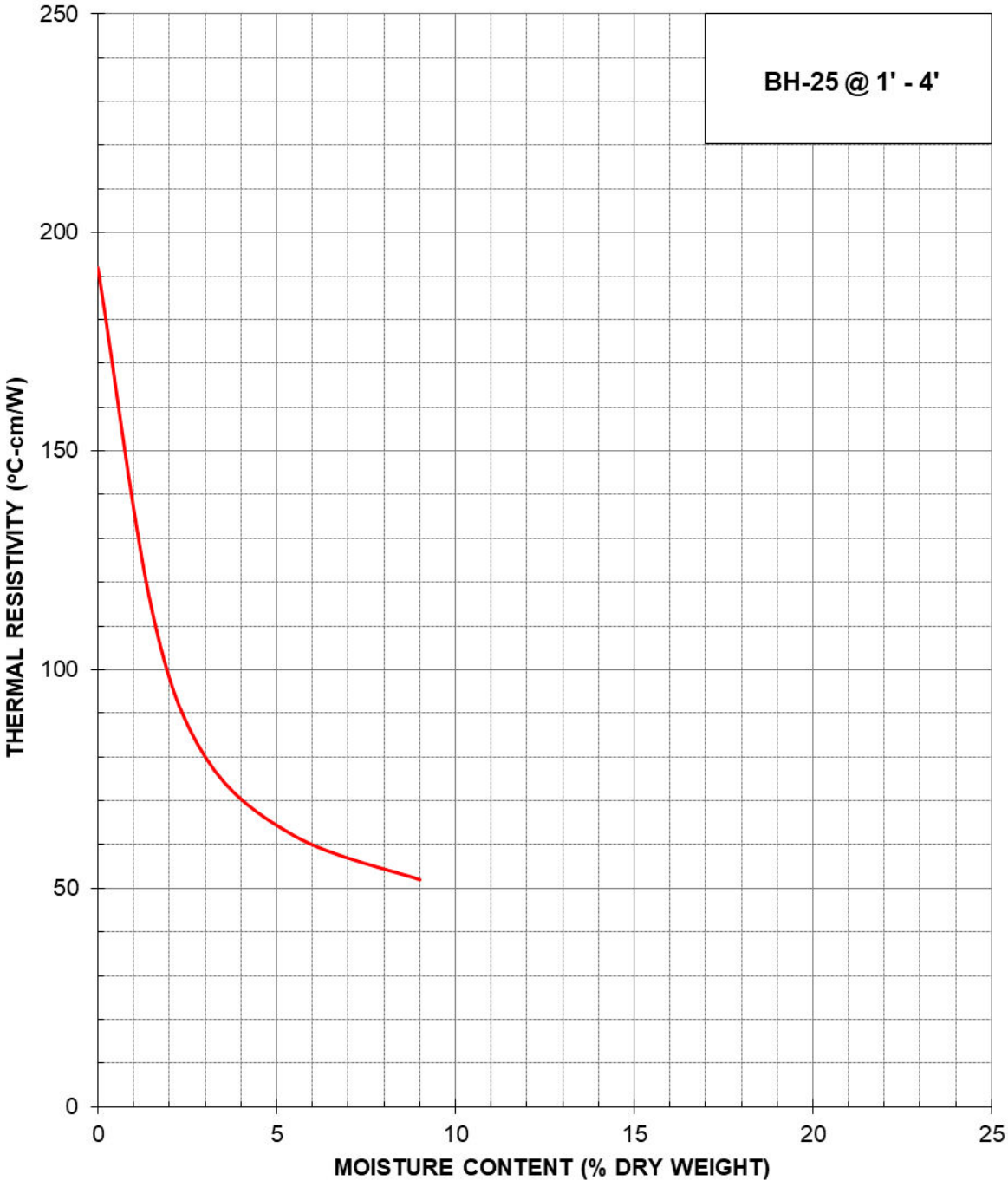
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 11**

THERMAL DRYOUT CURVE



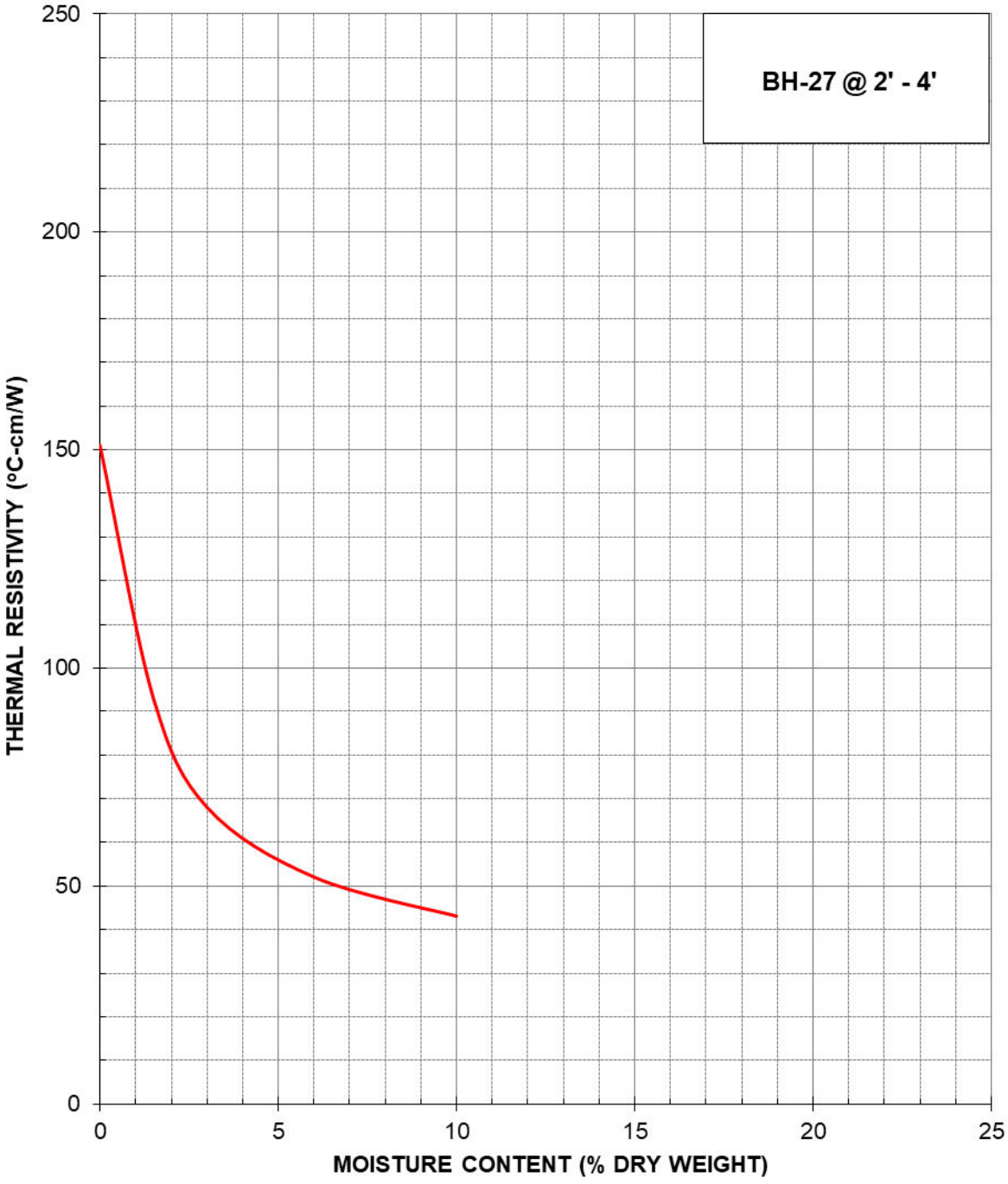
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 12**

THERMAL DRYOUT CURVE



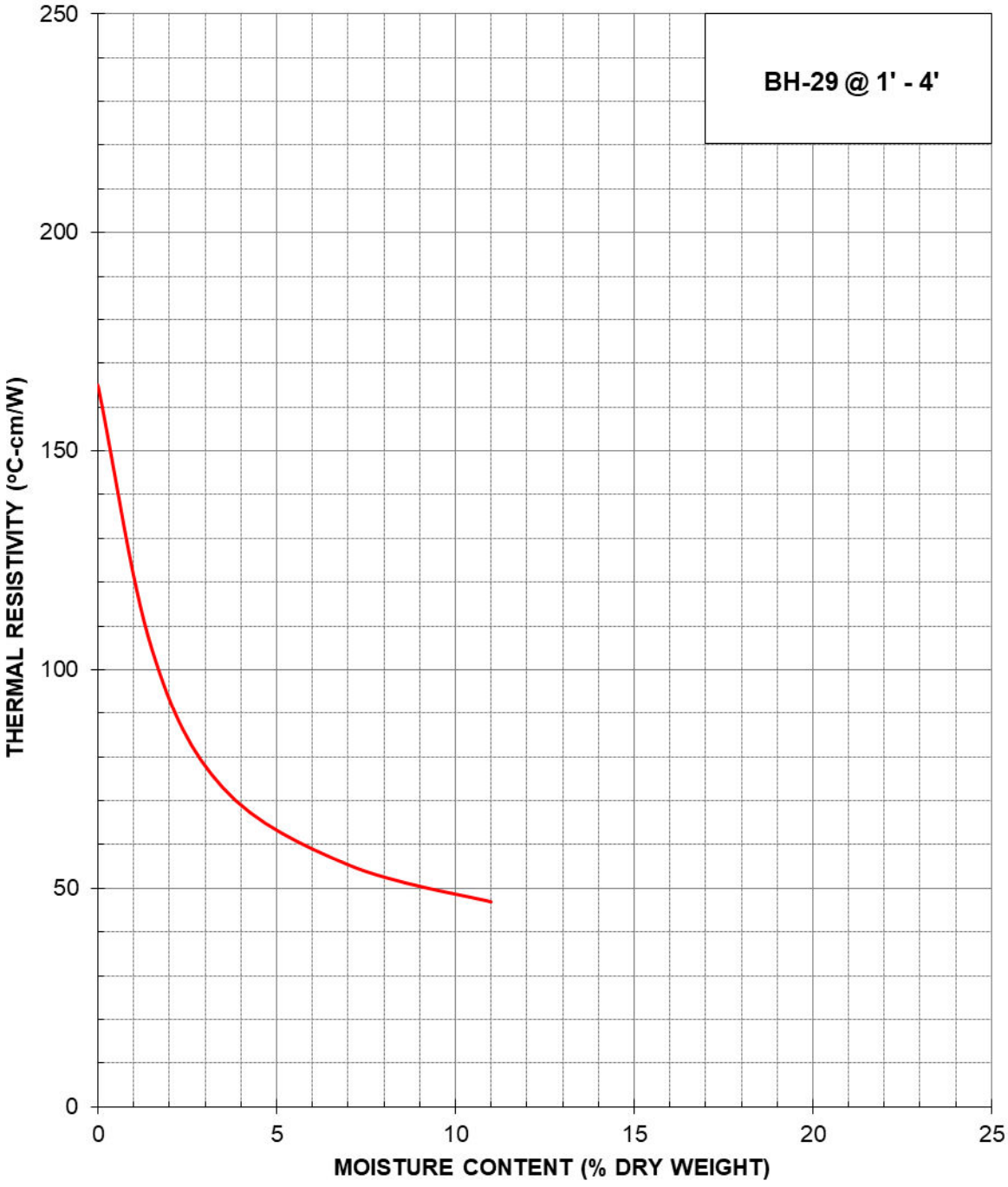
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 13**

THERMAL DRYOUT CURVE



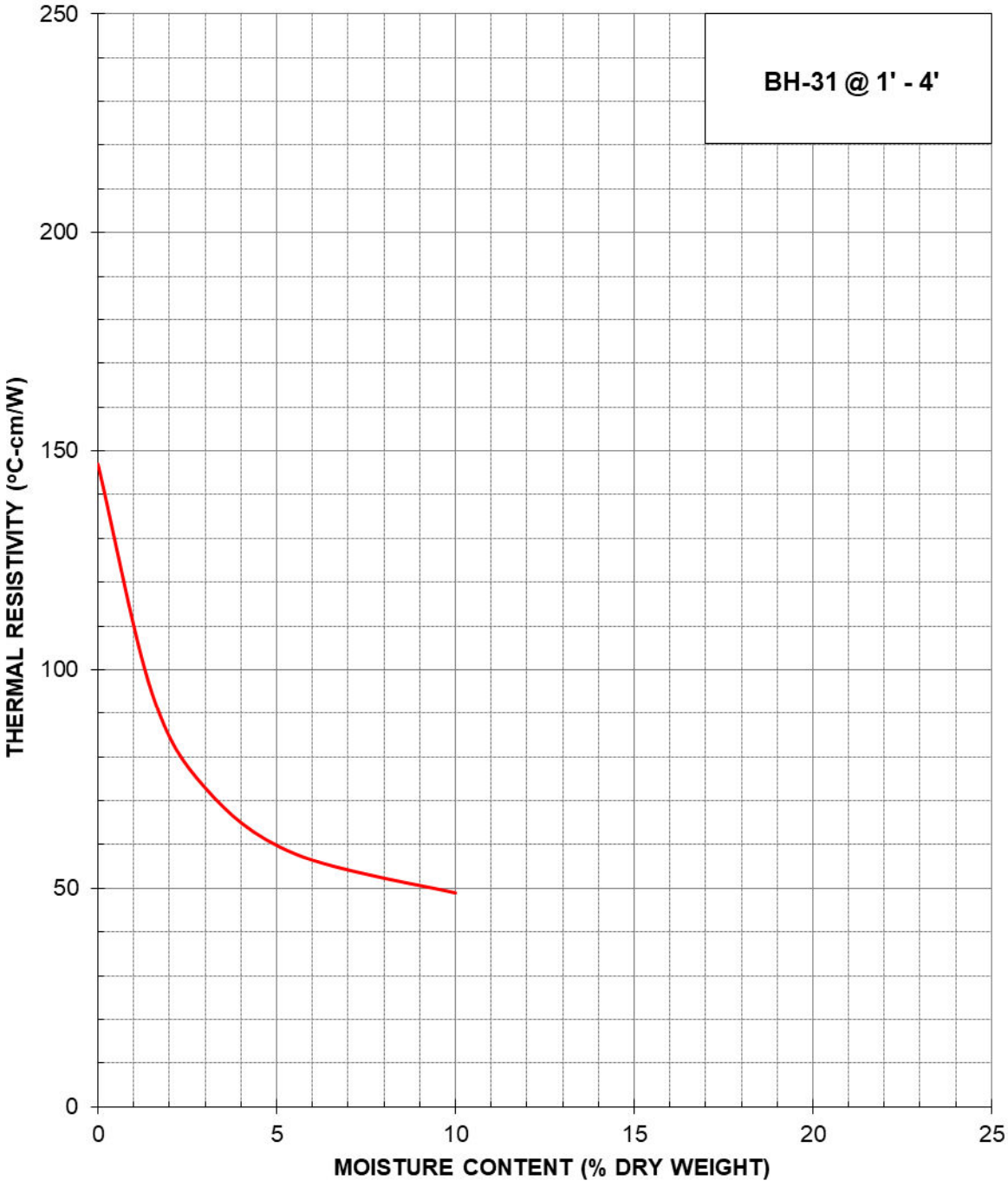
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 14**

THERMAL DRYOUT CURVE



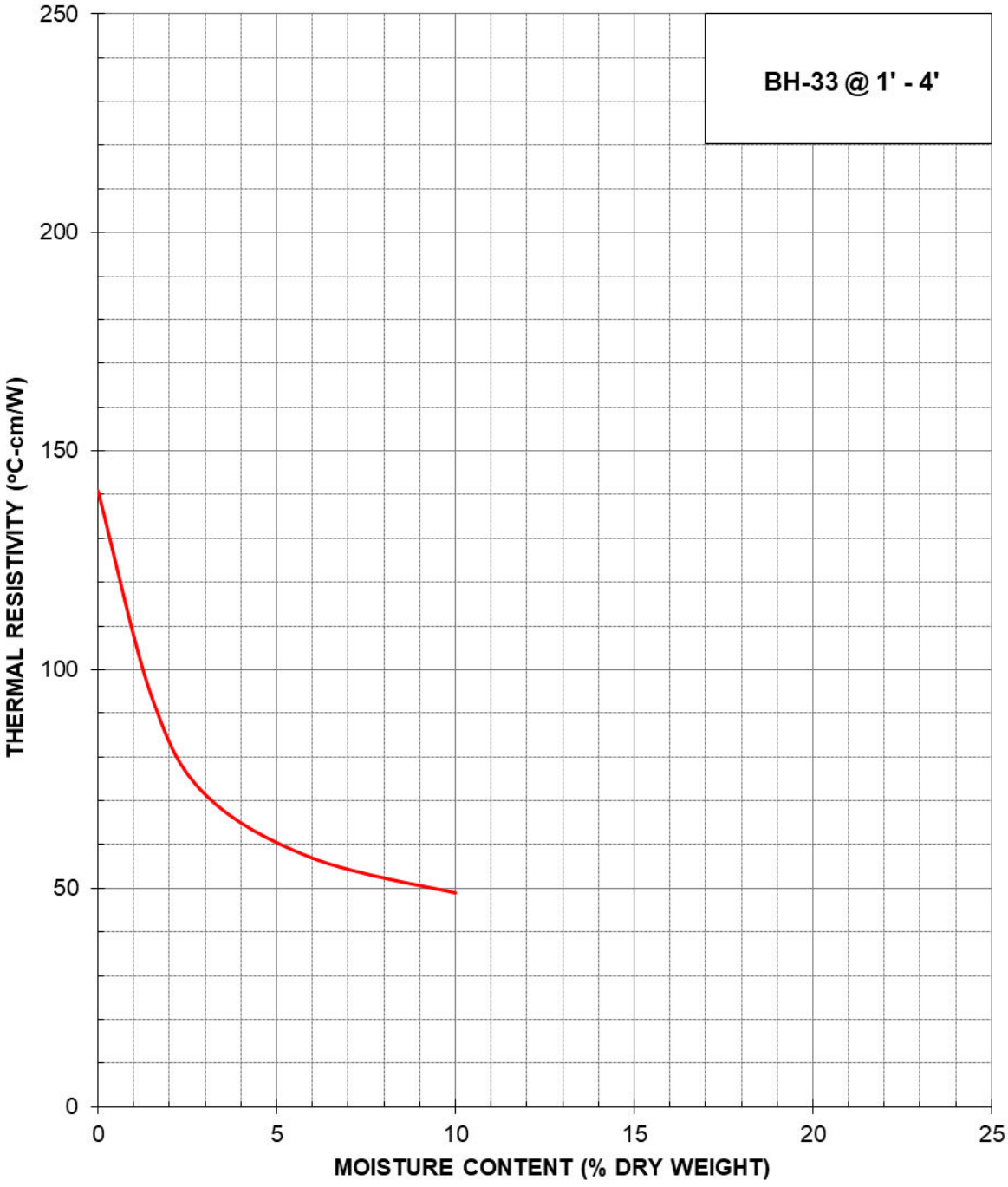
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 15**

THERMAL DRYOUT CURVE



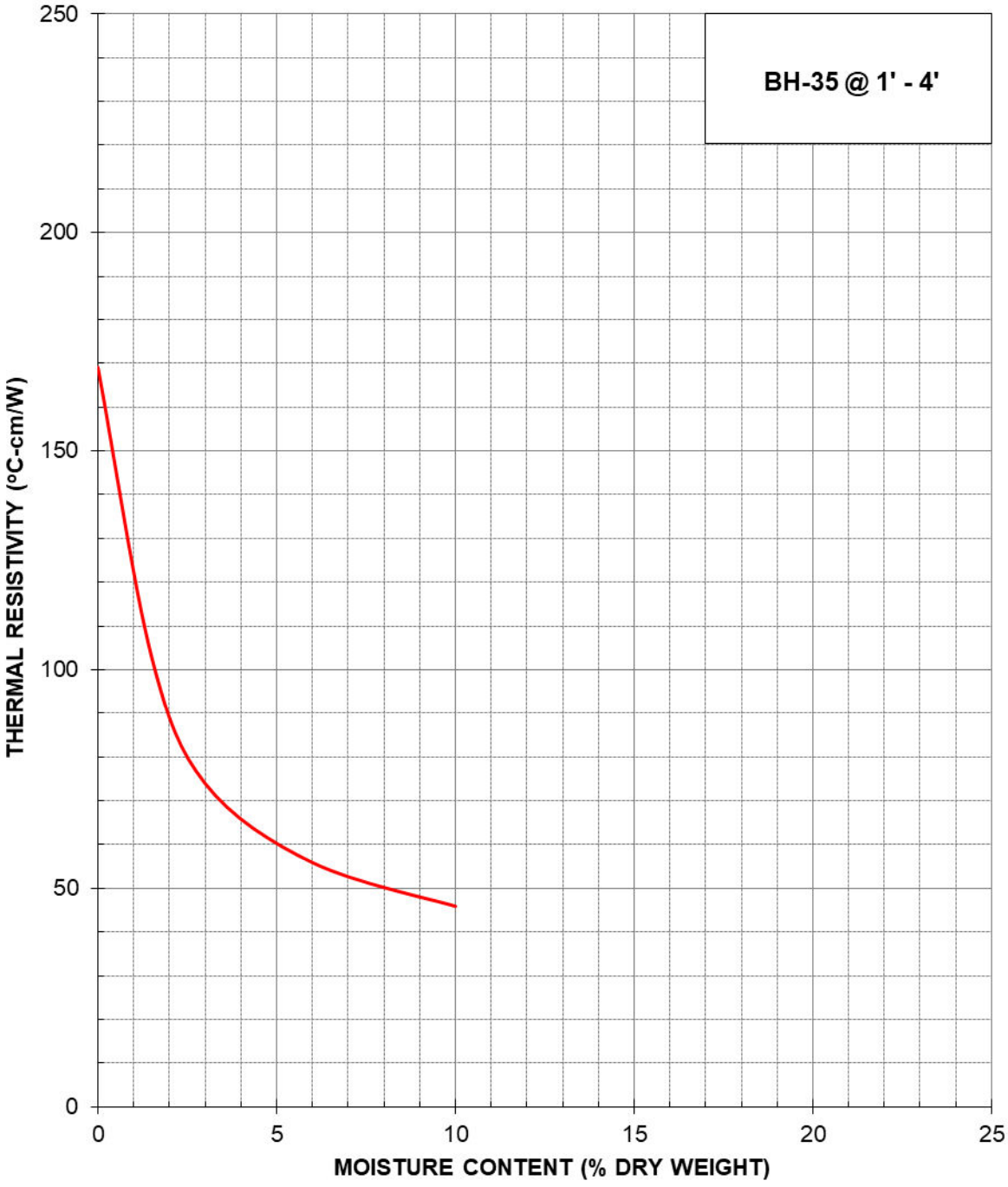
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 16**

THERMAL DRYOUT CURVE



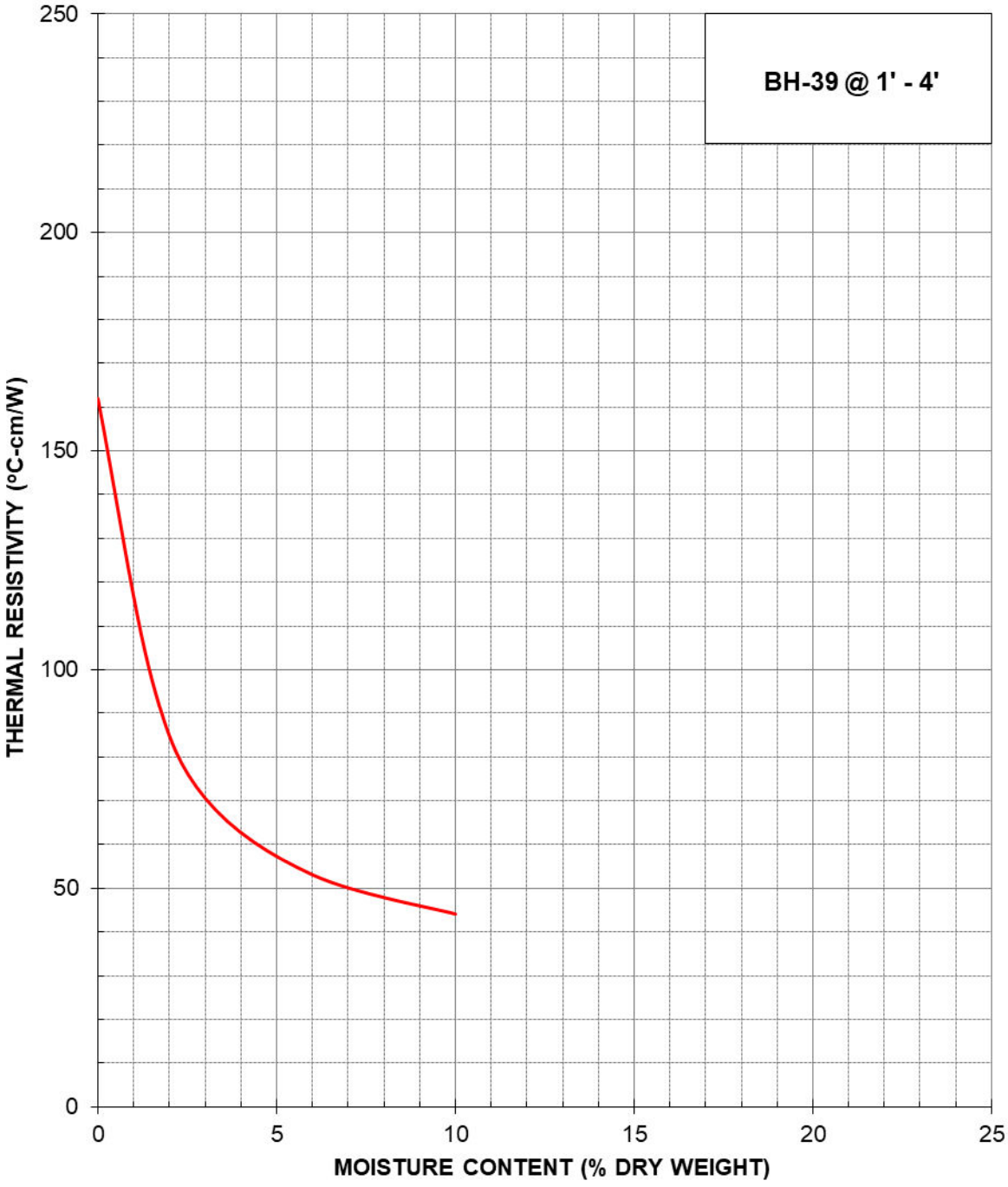
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 17**

THERMAL DRYOUT CURVE



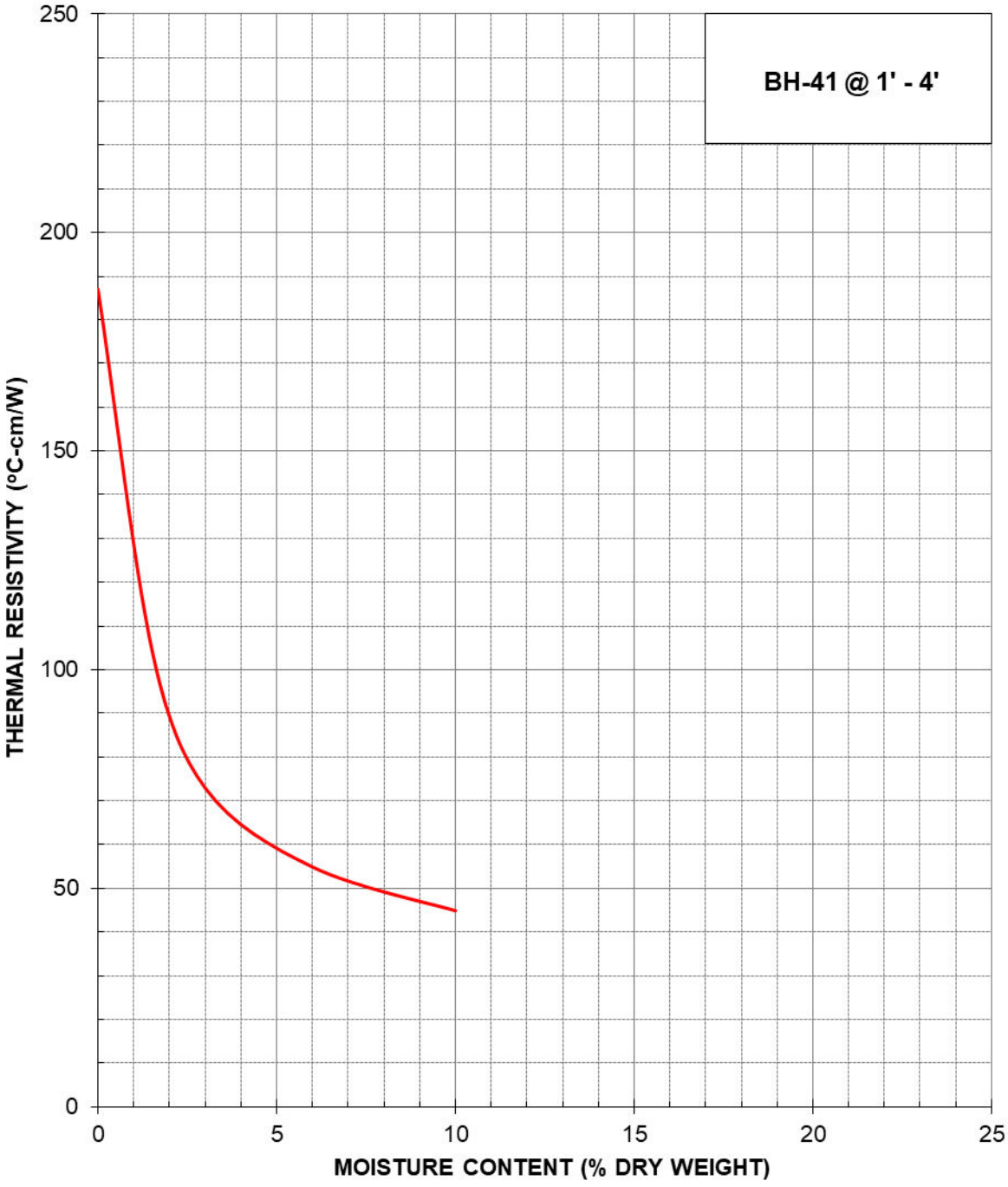
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 18**

THERMAL DRYOUT CURVE



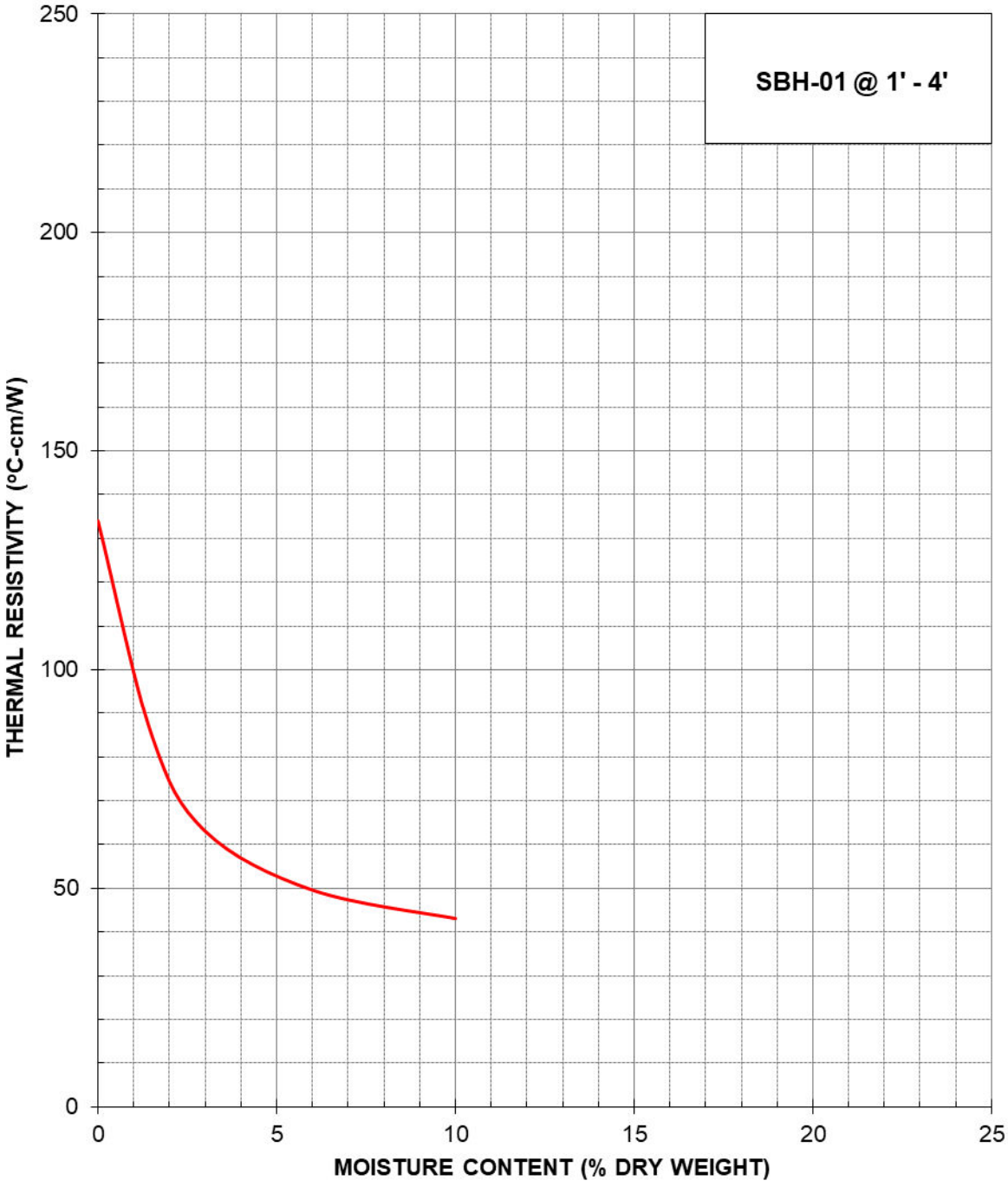
Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 19**

THERMAL DRYOUT CURVE



Terracon (Project No. 60225164)

Grace Solar – Blythe, Riverside County, CA

Thermal Analysis of Native Soil Samples

January 2024 **Figure 20**

750 Pilot Road, Suite F
Las Vegas, Nevada 89119
(702) 597-9393

Client
NextEra Energy Constructors, LLC

Project
NextEra-Grace Solar

Sample Submitted By: Terracon (60) **Date Received:** 11/20/2023 **Lab No.: 23-0621**

Results of Corrosion Analysis

Sample Number	--	--	--	--
Sample Location	BH-01	BH-03	BH-04	BH-06
Sample Depth (ft.)	1.0-4.0	1.0-4.0	1.0-4.0	1.0-4.0
pH Analysis, ASTM G51	9.57	9.30	9.47	9.5
Water Soluble Sulfate (SO4), ASTM C 1580 (Percent %)	<0.01	<0.01	0.01	0.01
Sulfides, AWWA 4500-S D, (mg/Kg)	Nil	Nil	Nil	Nil
Chlorides, ASTM D512, (mg/kg)	80	72	70	67
Red-Ox, ASTM G200, (mV)	+729	+725	+733	+730
Total Salts, AWWA 2520 B, (mg/Kg)	436	744	286	400
Saturated Minimum Resistivity, ASTM G-187, (ohm-cm)	737	609	837	871

Analyzed By
Nathan Campo
Engineering Technician III

The tests were performed in general accordance with applicable ASTM and AWWA test methods. This report is exclusively for the use of the client indicated above and shall not be reproduced except in full without the written consent of our company. Test results transmitted herein are only applicable to the actual samples tested at the location(s) referenced and are not necessarily indicative of the properties of other apparently similar or identical materials.

750 Pilot Road, Suite F
Las Vegas, Nevada 89119
(702) 597-9393

Client
NextEra Energy Constructors, LLC

Project
NextEra-Grace Solar

Sample Submitted By: Terracon (60) **Date Received:** 11/20/2023 **Lab No.: 23-0621**

Results of Corrosion Analysis

Sample Number	--	--	--	--
Sample Location	BH-09	BH-11	BH-12	BH-14
Sample Depth (ft.)	1.0-4.0	1.0-4.0	1.0-4.0	1.0-4.0
pH Analysis, ASTM G51	9.40	9.37	9.63	9.36
Water Soluble Sulfate (SO4), ASTM C 1580 (Percent %)	<0.01	<0.01	<0.01	0.01
Sulfides, AWWA 4500-S D, (mg/Kg)	Nil	Nil	Nil	Nil
Chlorides, ASTM D512, (mg/kg)	17	140	35	90
Red-Ox, ASTM G200, (mV)	+734	+729	+730	+729
Total Salts, AWWA 2520 B, (mg/Kg)	229	508	411	519
Saturated Minimum Resistivity, ASTM G-187, (ohm-cm)	1038	938	1139	1038

Analyzed By N. Campo

Nathan Campo
Engineering Technician III

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Las Vegas, Nevada 89119
(702) 597-9393

Client
NextEra Energy Constructors, LLC

Project
NextEra-Grace Solar

Sample Submitted By: Terracon (60) **Date Received:** 11/20/2023 **Lab No.: 23-0621**

Results of Corrosion Analysis

Sample Number	--	--	--	--
Sample Location	BH-15	BH-17	BH-18	BH-19
Sample Depth (ft.)	1.0-4.0	1.0-4.0	1.0-4.0	1.0-4.0
pH Analysis, ASTM G51	9.20	9.18	9.30	9.27
Water Soluble Sulfate (SO4), ASTM C 1580 (Percent %)	<0.01	0.01	<0.01	0.01
Sulfides, AWWA 4500-S D, (mg/Kg)	Nil	Nil	Nil	Nil
Chlorides, ASTM D512, (mg/kg)	65	77	42	95
Red-Ox, ASTM G200, (mV)	+725	+726	+728	+734
Total Salts, AWWA 2520 B, (mg/Kg)	761	599	459	235
Saturated Minimum Resistivity, ASTM G-187, (ohm-cm)	1005	1507	1306	2747

Analyzed By N. Campo

Nathan Campo
Engineering Technician III

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Las Vegas, Nevada 89119
(702) 597-9393

Client
NextEra Energy Constructors, LLC

Project
NextEra-Grace Solar

Sample Submitted By: Terracon (60) **Date Received:** 11/20/2023 **Lab No.: 23-0621**

Results of Corrosion Analysis

Sample Number	--	--	--	--
Sample Location	BH-21	BH-23	BH-24	BH-25
Sample Depth (ft.)	1.0-4.0	1.0-4.0	1.0-4.0	1.0-4.0
pH Analysis, ASTM G51	8.87	9.50	9.45	8.9
Water Soluble Sulfate (SO4), ASTM C 1580 (Percent %)	<0.01	0.01	0.01	<0.01
Sulfides, AWWA 4500-S D, (mg/Kg)	Nil	Nil	Nil	Nil
Chlorides, ASTM D512, (mg/kg)	82	85	37	45
Red-Ox, ASTM G200, (mV)	+727	+729	+730	+733
Total Salts, AWWA 2520 B, (mg/Kg)	561	664	493	309
Saturated Minimum Resistivity, ASTM G-187, (ohm-cm)	804	904	2278	2948

Analyzed By _____
Nathan Campo
Engineering Technician III

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(702) 597-9393

Client
NextEra Energy Constructors, LLC

Project
NextEra-Grace Solar

Sample Submitted By: Terracon (60) **Date Received:** 11/20/2023 **Lab No.: 23-0621**

Results of Corrosion Analysis

Sample Number	--	--	--	--
Sample Location	BH-27	BH-29	BH-31	BH-33
Sample Depth (ft.)	1.0-4.0	1.0-4.0	1.0-4.0	1.0-4.0
pH Analysis, ASTM G51	9.58	8.59	9.73	8.29
Water Soluble Sulfate (SO4), ASTM C 1580 (Percent %)	<0.01	0.01	<0.01	0.01
Sulfides, AWWA 4500-S D, (mg/Kg)	Nil	Nil	Nil	Nil
Chlorides, ASTM D512, (mg/kg)	52	185	140	332
Red-Ox, ASTM G200, (mV)	+722	+715	+728	+712
Total Salts, AWWA 2520 B, (mg/Kg)	1079	2049	681	2632
Saturated Minimum Resistivity, ASTM G-187, (ohm-cm)	1172	348	904	348

Analyzed By

Nathan Campo
Engineering Technician III

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(702) 597-9393

Client
NextEra Energy Constructors, LLC

Project
NextEra-Grace Solar

Sample Submitted By: Terracon (60) **Date Received:** 11/20/2023 **Lab No.: 23-0621**

Results of Corrosion Analysis

Sample Number	--	--	--	--
Sample Location	BH-35	BH-37	BH-39	BH-40
Sample Depth (ft.)	1.0-4.0	1.0-4.0	1.0-4.0	1.0-4.0
pH Analysis, ASTM G51	9.23	9.25	9.16	9
Water Soluble Sulfate (SO4), ASTM C 1580 (Percent %)	<0.01	0.02	0.01	0.01
Sulfides, AWWA 4500-S D, (mg/Kg)	Nil	Nil	Nil	Nil
Chlorides, ASTM D512, (mg/kg)	82	85	42	132
Red-Ox, ASTM G200, (mV)	+728	+733	+733	+728
Total Salts, AWWA 2520 B, (mg/Kg)	424	257	301	675
Saturated Minimum Resistivity, ASTM G-187, (ohm-cm)	3283	3450	3618	1340

Analyzed By N. Campo

Nathan Campo
Engineering Technician III

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Las Vegas, Nevada 89119
(702) 597-9393

Client
NextEra Energy Constructors, LLC

Project
NextEra-Grace Solar

Sample Submitted By: Terracon (60) **Date Received:** 11/20/2023 **Lab No.: 23-0621**

Results of Corrosion Analysis

Sample Number	--	--	--	--
Sample Location	BH-41	BH-42	BH-43	BH-44
Sample Depth (ft.)	1.0-4.0	1.0-4.0	1.0-4.0	1.0-4.0
pH Analysis, ASTM G51	8.80	9.57	9.44	8.55
Water Soluble Sulfate (SO4), ASTM C 1580 (Percent %)	<0.01	0.02	0.01	<0.01
Sulfides, AWWA 4500-S D, (mg/Kg)	Nil	Nil	Nil	Nil
Chlorides, ASTM D512, (mg/kg)	102	57	65	55
Red-Ox, ASTM G200, (mV)	+728	+732	+731	+727
Total Salts, AWWA 2520 B, (mg/Kg)	436	341	353	560
Saturated Minimum Resistivity, ASTM G-187, (ohm-cm)	2747	5092	4087	1943

Analyzed By N. Campo

Nathan Campo
Engineering Technician III

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750 Pilot Road, Suite F
Las Vegas, Nevada 89119
(702) 597-9393

Client
NextEra Energy Constructors, LLC

Project
NextEra-Grace Solar

Sample Submitted By: Terracon (60) **Date Received:** 11/20/2023 **Lab No.: 23-0621**

Results of Corrosion Analysis

Sample Number	--
Sample Location	SBH-01
Sample Depth (ft.)	1.0-4.0
pH Analysis, ASTM G51	9.05
Water Soluble Sulfate (SO4), ASTM C 1580 (Percent %)	<0.01
Sulfides, AWWA 4500-S D, (mg/Kg)	Nil
Chlorides, ASTM D512, (mg/kg)	152
Red-Ox, ASTM G200, (mV)	+723
Total Salts, AWWA 2520 B, (mg/Kg)	944
Saturated Minimum Resistivity, ASTM G-187, (ohm-cm)	938

Analyzed By N. Campo

Nathan Campo
Engineering Technician III

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Las Vegas, Nevada 89119
(702) 597-9393

Client
NextEra Energy Constructors, LLC

Project
NextEra-Grace Solar

Sample Submitted By: Terracon (60) **Date Received:** 11/20/2023 **Lab No.: 23-0621**

Results of Corrosion Analysis

Sample Number	--
Sample Location	TP-06
Sample Depth (ft.)	1.0-4.0
pH Analysis, ASTM G51	9.21
Water Soluble Sulfate (SO4), ASTM C 1580 (Percent %)	0.01
Sulfides, AWWA 4500-S D, (mg/Kg)	Nil
Chlorides, ASTM D512, (mg/kg)	132
Red-Ox, ASTM G200, (mV)	+721
Total Salts, AWWA 2520 B, (mg/Kg)	1327
Saturated Minimum Resistivity, ASTM G-187, (ohm-cm)	1474

Analyzed By N. Campo

Nathan Campo
Laboratory Coordinator

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Las Vegas, Nevada 89119
(702) 597-9393

Client
NextEra Energy Constructors, LLC

Project
NextEra-Grace Solar

Sample Submitted By: Terracon (60) **Date Received:** 11/20/2023 **Lab No.: 23-0647**

Results of Corrosion Analysis

Sample Number	--
Sample Location	TP-18
Sample Depth (ft.)	0.0-4.0
pH Analysis, ASTM G51	9.34
Water Soluble Sulfate (SO4), ASTM C 1580 (Percent %)	0.01
Sulfides, AWWA 4500-S D, (mg/Kg)	Nil
Chlorides, ASTM D512, (mg/kg)	77
Red-Ox, ASTM G200, (mV)	+724
Total Salts, AWWA 2520 B, (mg/Kg)	791
Saturated Minimum Resistivity, ASTM G-187, (ohm-cm)	2144

Analyzed By N. Campo

Nathan Campo
Laboratory Coordinator

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Field Soil Electrical Resistivity Test Results

Contents:

Electrical Resistivity Test Procedures
Electrical Resistivity Test Location Plan (6 pages)
Electrical Resistivity Results (21 pages)

Field Electrical Resistivity Test Procedures

Field measurements of soil electrical resistivity were performed by Terracon between October 24 to November 15, 2023. Field measurements of soil electrical resistivity were performed in general accordance with ASTM Test Method G 57, and IEEE Standard 81, using the Wenner Four-Electrode. The Wenner arrangement (equal electrode spacing) was used with the following "a" spacings:

- 1, 2, 3, 6, 10, 20, 30, 60, 100, 200 and 300 feet (or to the maximum spacing permitted by site constraints) at twenty locations within the solar array area
- 0.5, 1, 1.5, 2, 3, 5, 7, 10, 15, 20, 30, 45, 70, 100, 150, 250, 350, and 450 feet (or to the maximum spacing permitted by site constraints) at one location within the substation area

The "a" spacing is generally considered to be the depth of influence of the test. The testing was performed in both a north-south and an east-west orientation at each location.

Electrical Resistivity Test Location Plan (Sheet 1)



DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Electrical Resistivity Test Location Plan (Sheet 2)

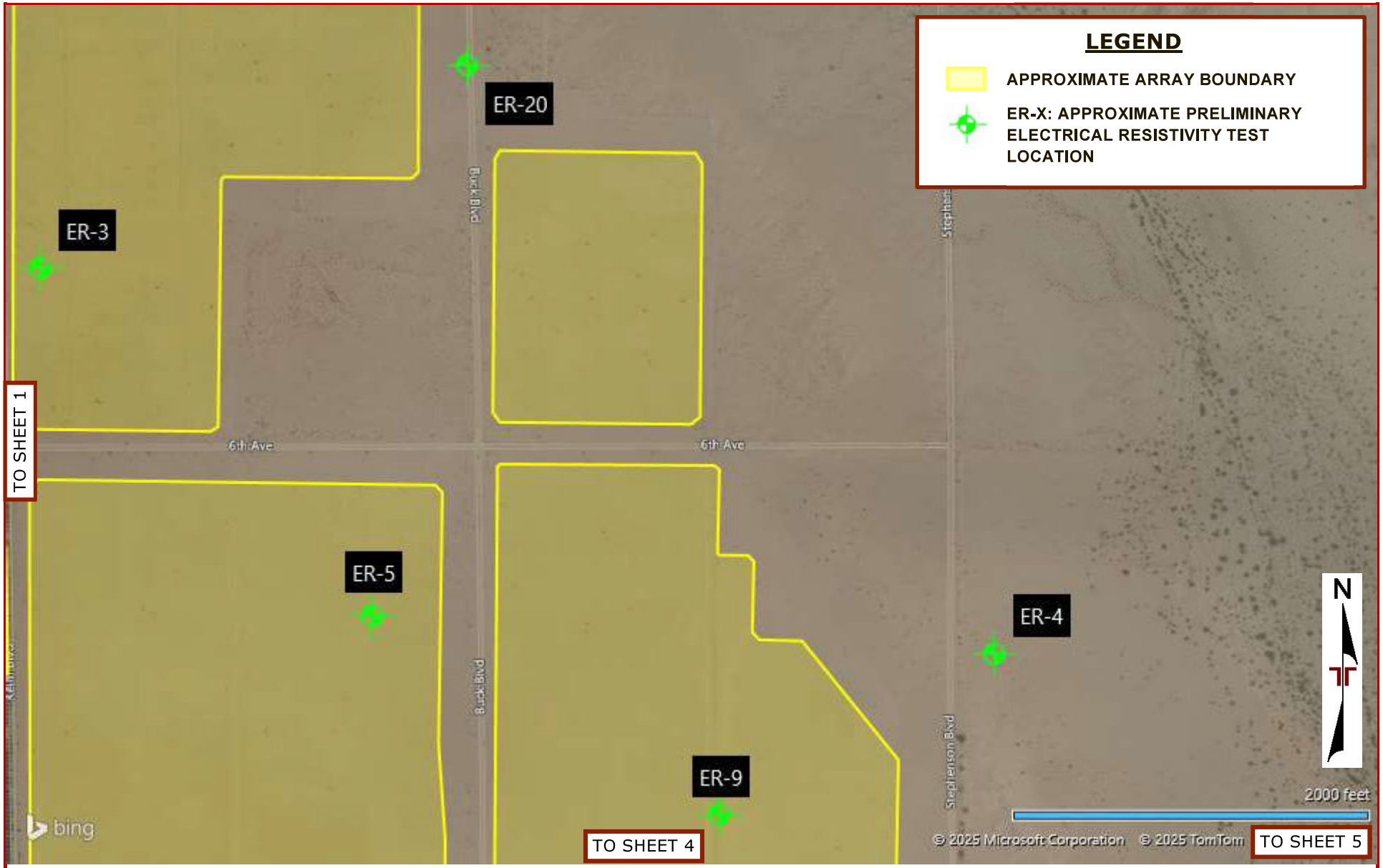


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Exploration Location Plan (Sheet 3)

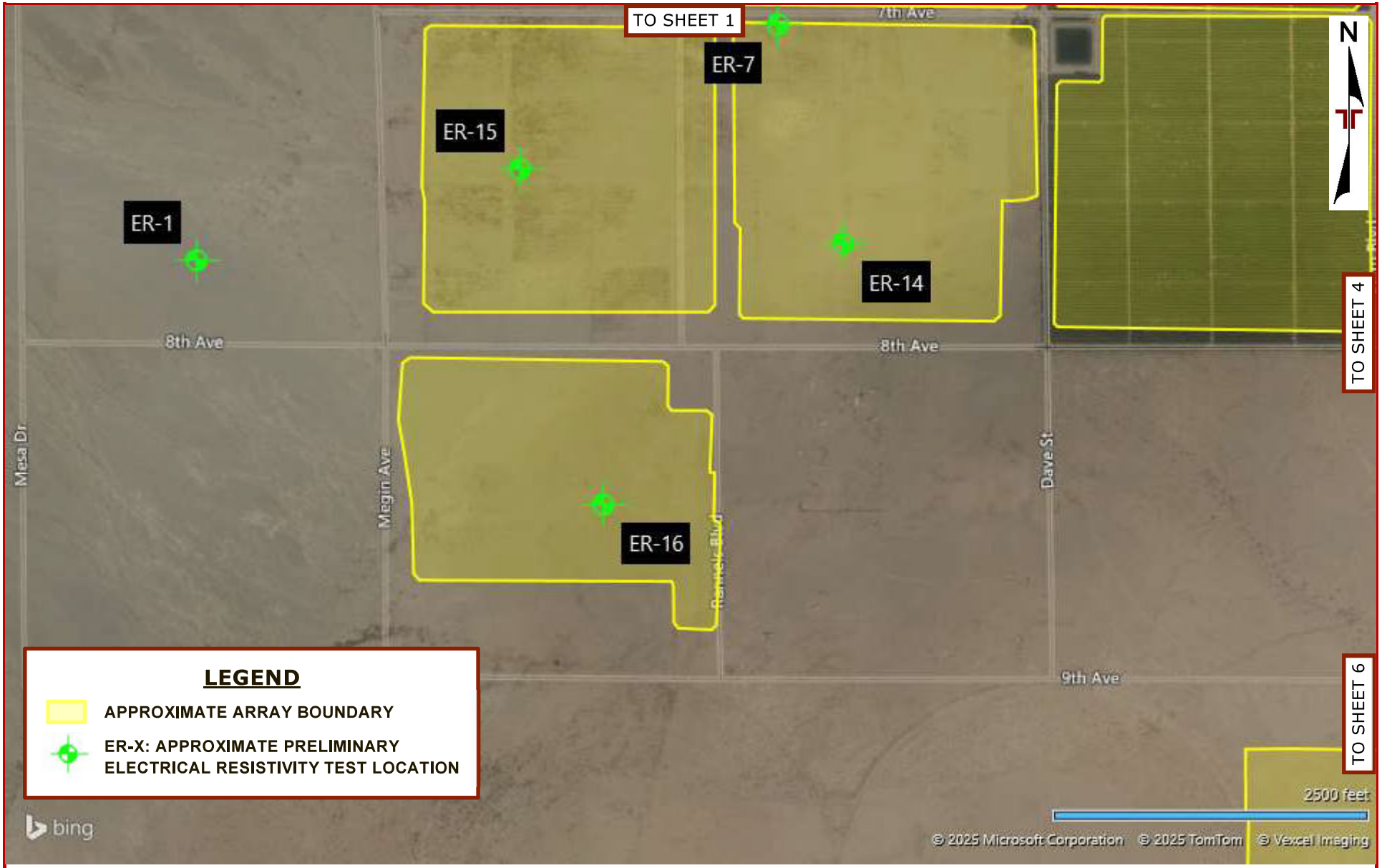


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Exploration Location Plan (Sheet 4)



DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Exploration Location Plan (Sheet 5)

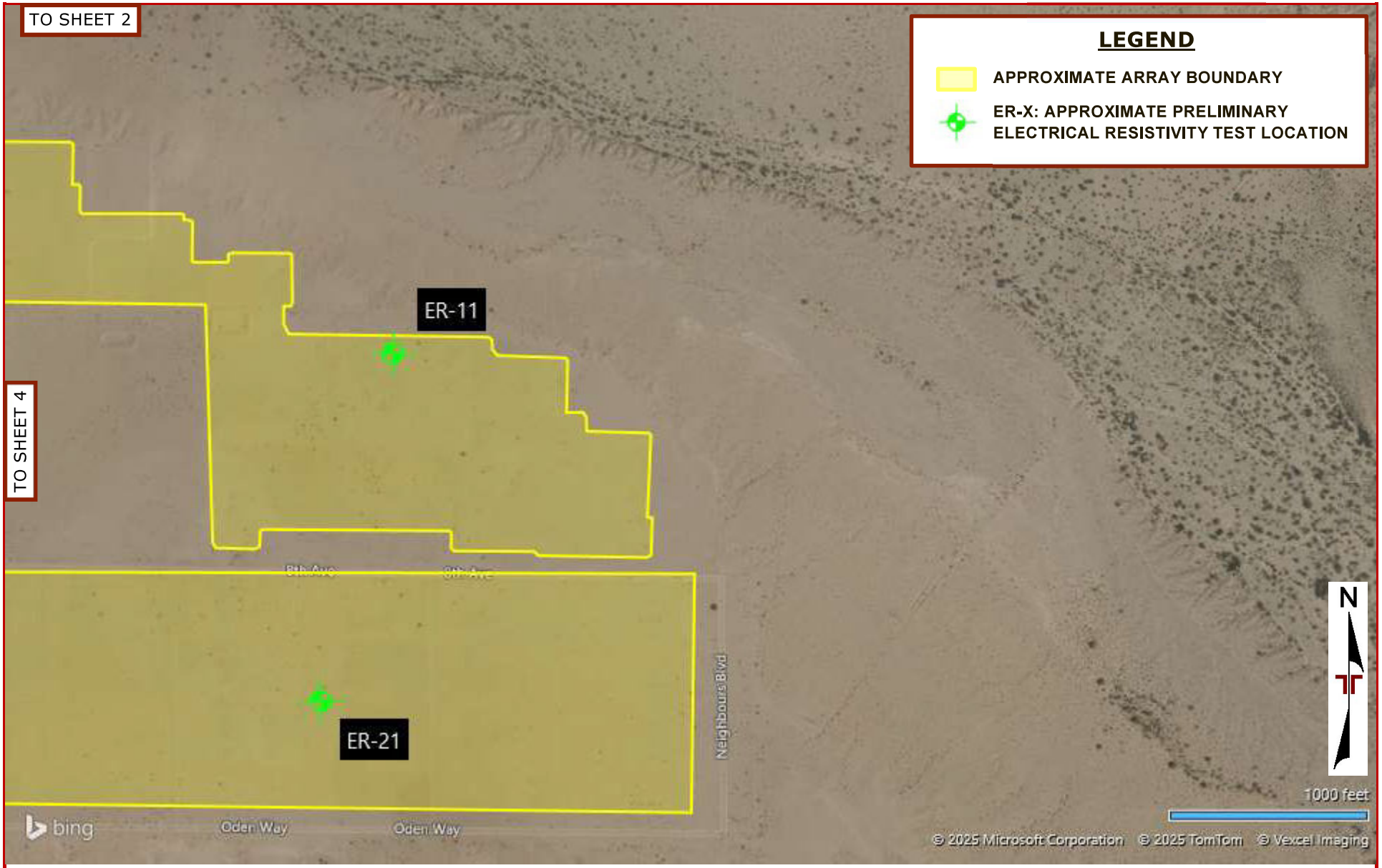


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Exploration Location Plan (Sheet 6)

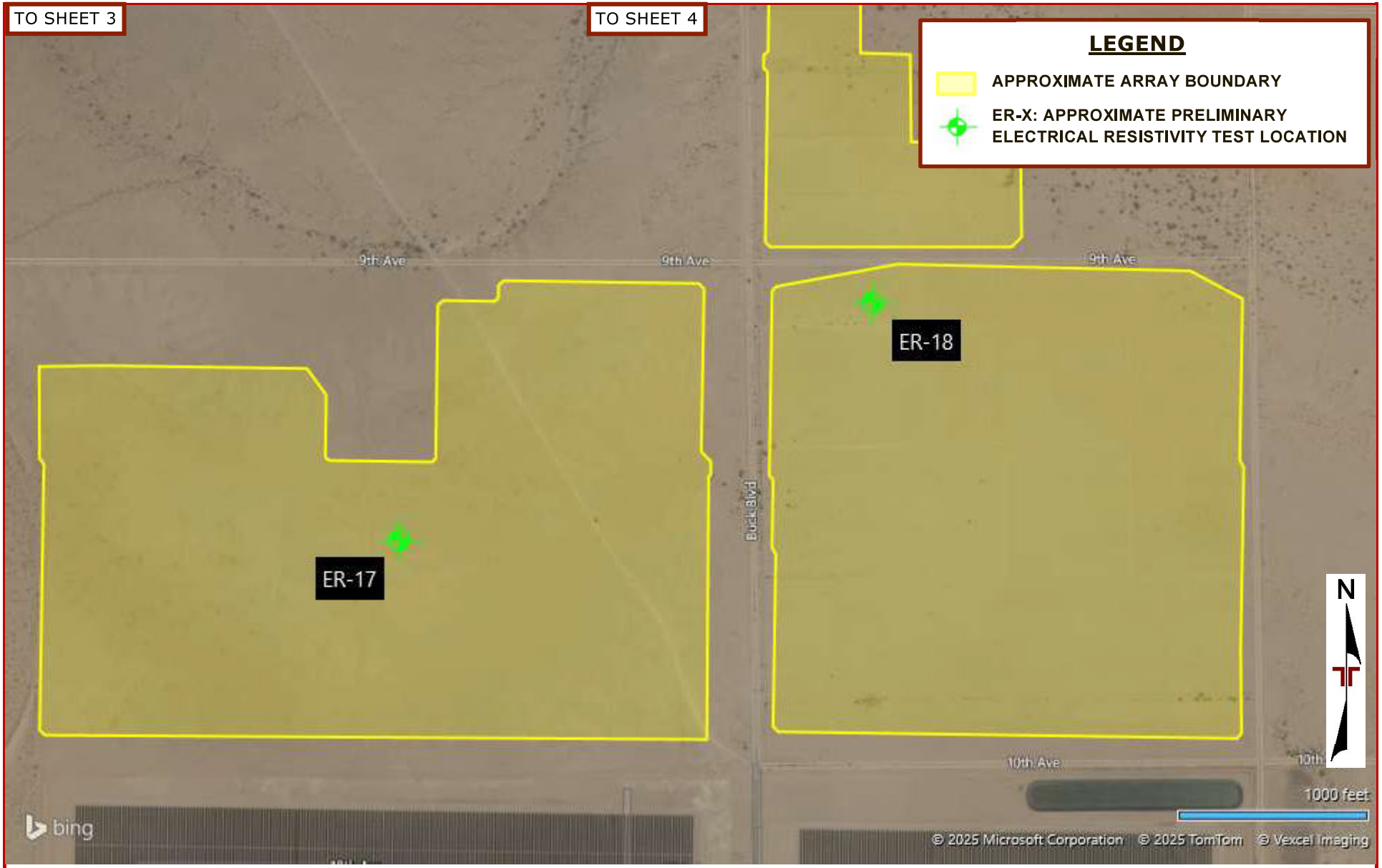


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

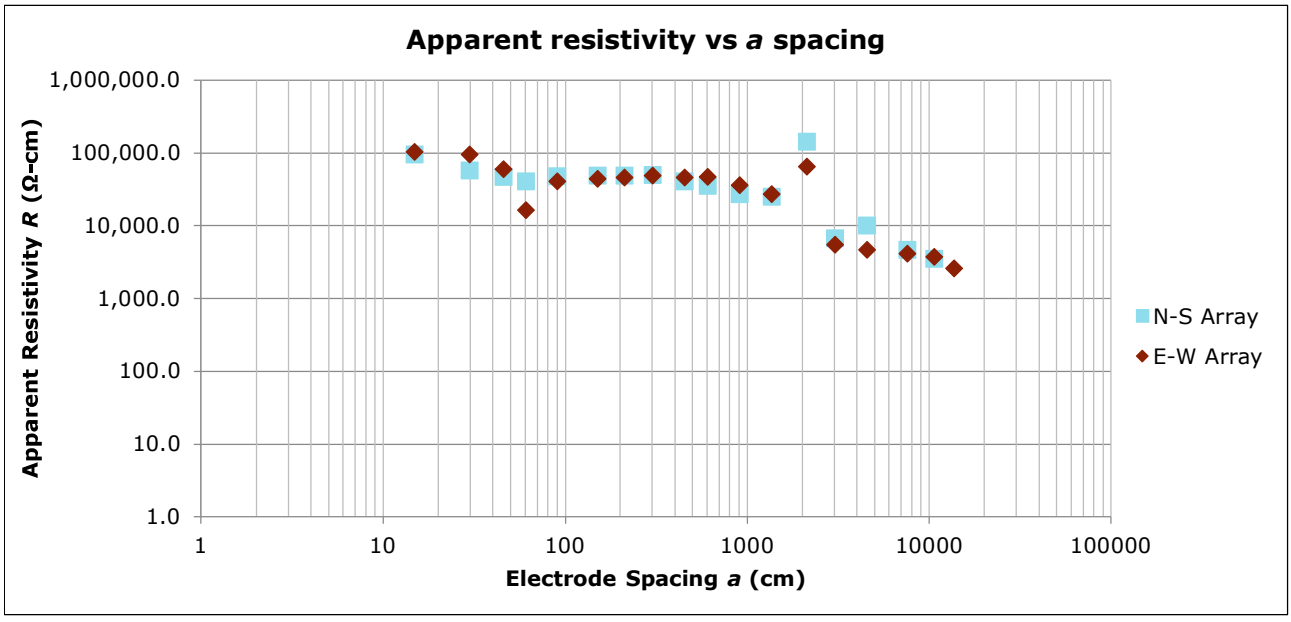
MAP PROVIDED BY MICROSOFT BING MAPS

FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-1 (33.6482, -114.7236)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MM
Test Date	November 15, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Maximum 'a' spacing along N-S array limited to 350-feet due to nearby access road.		

Apparent resistivity ρ is calculated as : $$\rho = \frac{4\pi aR}{1 + \frac{2a}{\sqrt{a^2+4b^2}} - \frac{a}{\sqrt{a^2+b^2}}}$$

Electrode Spacing *a*		Electrode Depth *b*		N-S Test		E-W Test	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
0.5	15	9	23	539	94550	582	102110
1	30	10	25	190	56900	312	93260
1.5	46	10	25	119	46420	151	59270
2	61	10	25	84	39840	34	16190
3	91	11	28	72	47210	62	40330
5	152	11	28	48	48380	43	43230
7	213	12	30	35	48320	33	45020
10	305	12	30	25	49270	25	48220
15	457	12	30	14	40240	16	45270
20	610	12	30	9.0	34620	12	46150
30	914	12	30	4.7	27030	6.2	35670
45	1372	12	30	2.9	24860	3.1	26820
70	2134	12	30	10	140170	4.8	64250
100	3048	12	30	0.35	6620	0.28	5430
150	4572	12	30	0.35	9930	0.16	4570
250	7620	12	30	0.10	4600	0.09	4110
350	10668	12	30	0.05	3490	0.05	3670
450	13716	12	30			0.03	2570

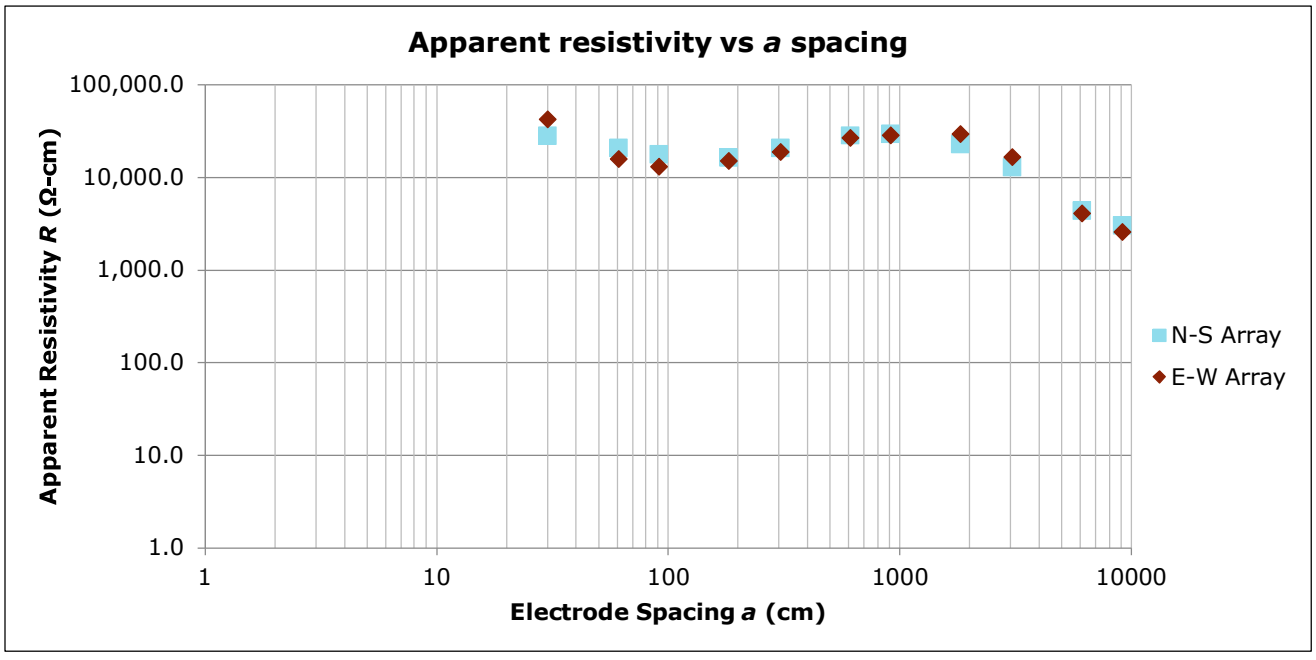


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-2 (33.6647, -114.7071)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MP
Test Date	October 26, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts			

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	8	20	100	27670	154	42390
2	61	8	20	46	20690	36	15850
3	91	8	20	28	17490	21	13190
6	183	9	23	14	16450	13	15270
10	305	9	23	11	20590	10	18960
20	610	9	23	7.4	28340	7.0	26710
30	914	10	25	5.2	29680	4.9	28250
60	1829	10	25	2.0	23090	2.6	29450
100	3048	10	25	0.68	12930	0.87	16700
200	6096	11	28	0.11	4390	0.11	4040
300	9144	11	28	0.05	3030	0.04	2570

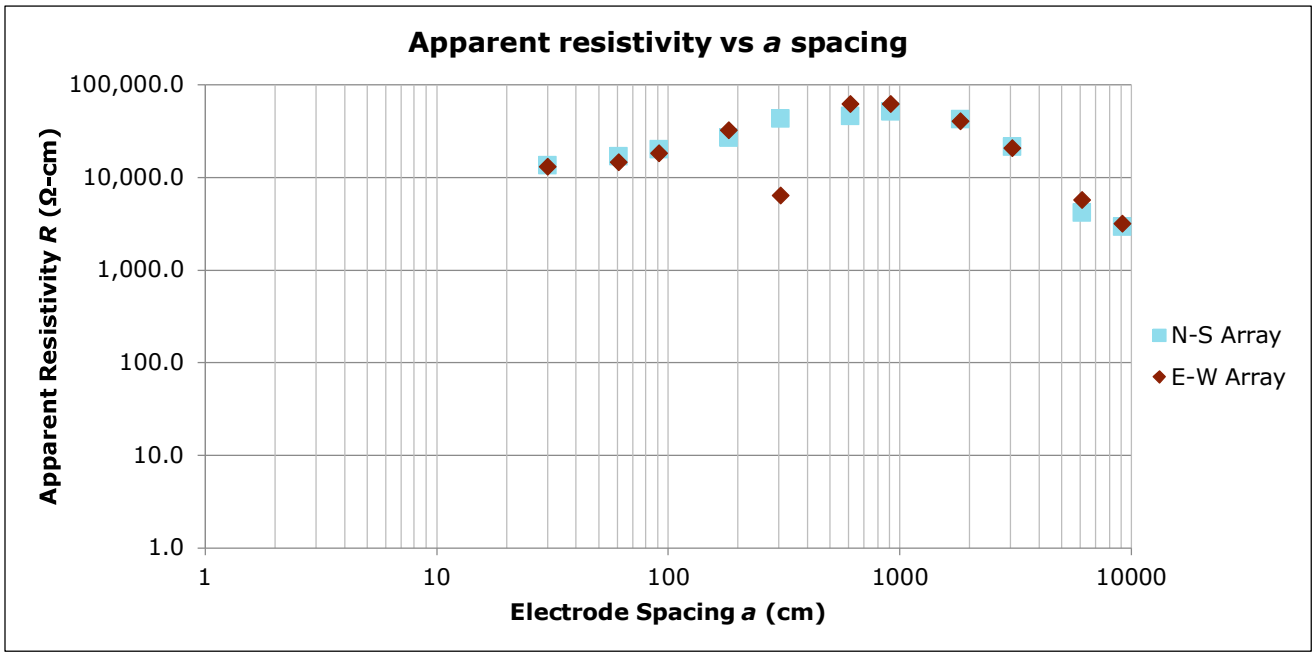


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-3 (33.6636, -114.6924)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MP
Test Date	October 24, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts			

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	6	15	54	13480	53	13150
2	61	6	15	40	16860	35	14520
3	91	6	15	34	20050	30	18110
6	183	7	18	23	26580	27	32140
10	305	8	20	23	43520	3.3	6380
20	610	9	23	12	46070	16	61980
30	914	10	25	8.9	51100	11	61870
60	1829	11	28	3.6	41950	3.5	40340
100	3048	11	28	1.1	21390	1.1	20610
200	6096	12	30	0.11	4140	0.15	5700
300	9144	12	30	0.05	2940	0.05	3150

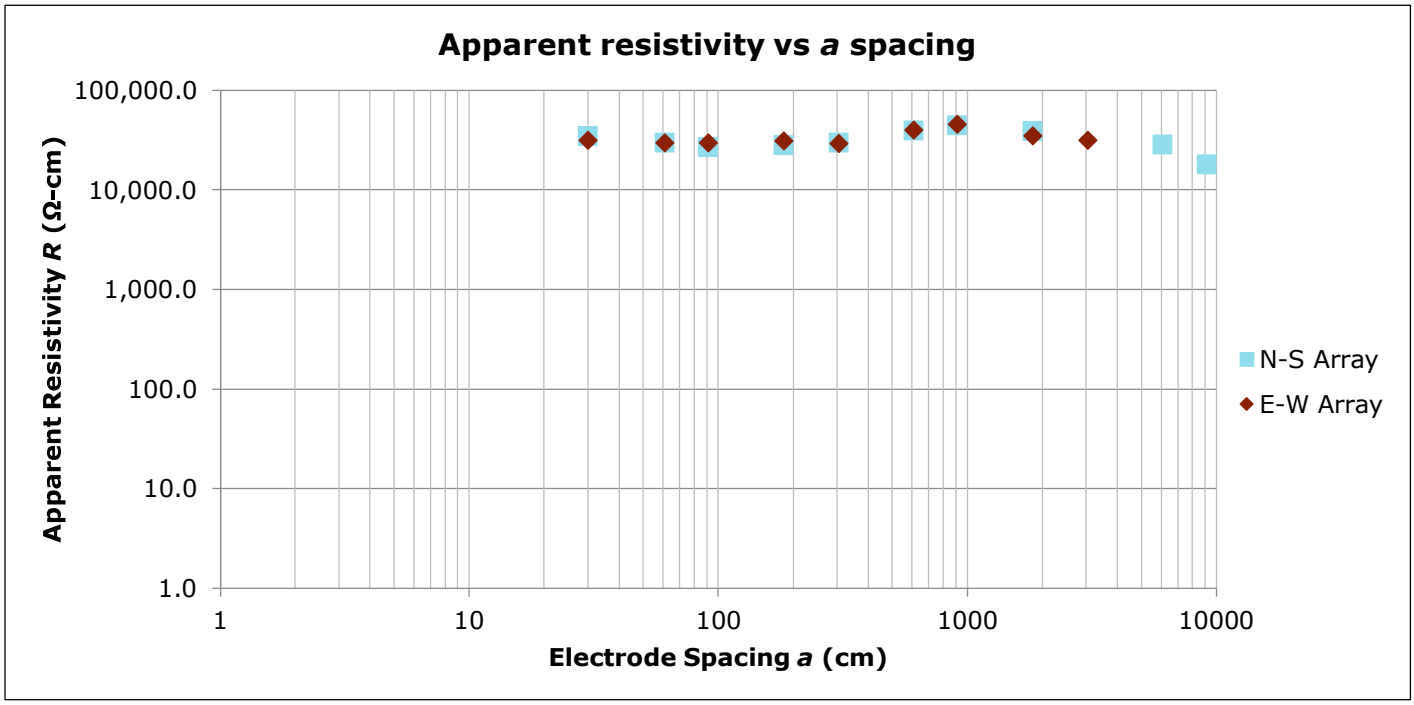


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-4 (33.6576, -114.6747)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MM
Test Date	November 6, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Maximum 'a' spacing along E-W array limited to 100-foot due to nearby access road. 100' test on N-S array omitted due to observed interference		

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		Electrode Depth *b*		N-S Test		E-W Test	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	6	15	139	34410	127	31400
2	61	7	18	68	29720	68	29720
3	91	8	20	43	26650	48	29500
6	183	8	20	24	27740	27	31130
10	305	8	20	15	29580	15	29170
20	610	9	23	10	39540	10	40150
30	914	11	28	7.6	43960	7.9	45300
60	1829	11	28	3.4	38700	3.0	35010
100	3048	11	28			1.6	31360
200	6096	11	28	0.75	28540		
300	9144	11	28	0.31	17890		

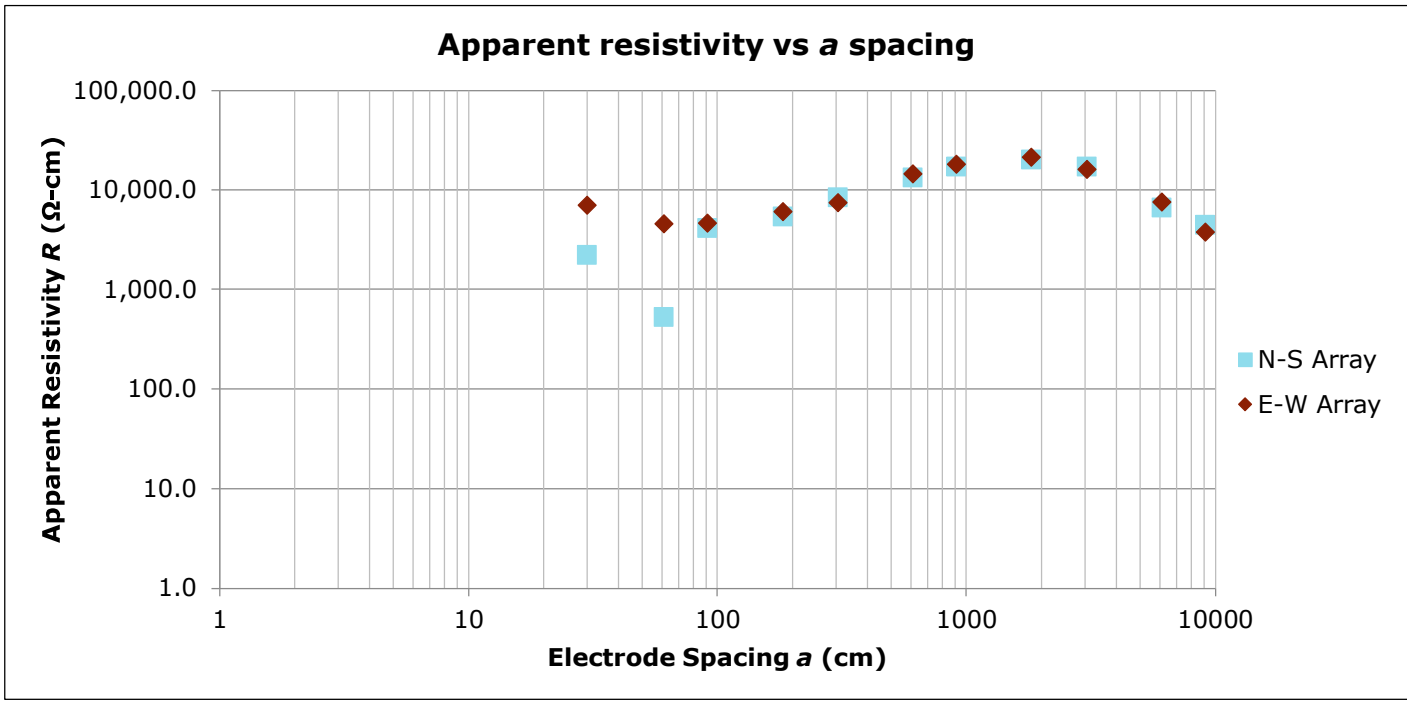


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-5 (33.6582, -114.6862)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MM
Test Date	November 14, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts			

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	8	20	8.1	2240	26	7090
2	61	9	23	1.1	530	10	4650
3	91	10	25	6.5	4160	7.3	4700
6	183	10	25	4.6	5410	5.1	6100
10	305	11	28	4.4	8500	3.9	7550
20	610	11	28	3.5	13450	3.8	14680
30	914	11	28	3.0	17330	3.2	18420
60	1829	12	30	1.8	20370	1.9	21670
100	3048	12	30	0.90	17270	0.86	16380
200	6096	12	30	0.17	6680	0.20	7620
300	9144	12	30	0.08	4520	0.07	3800

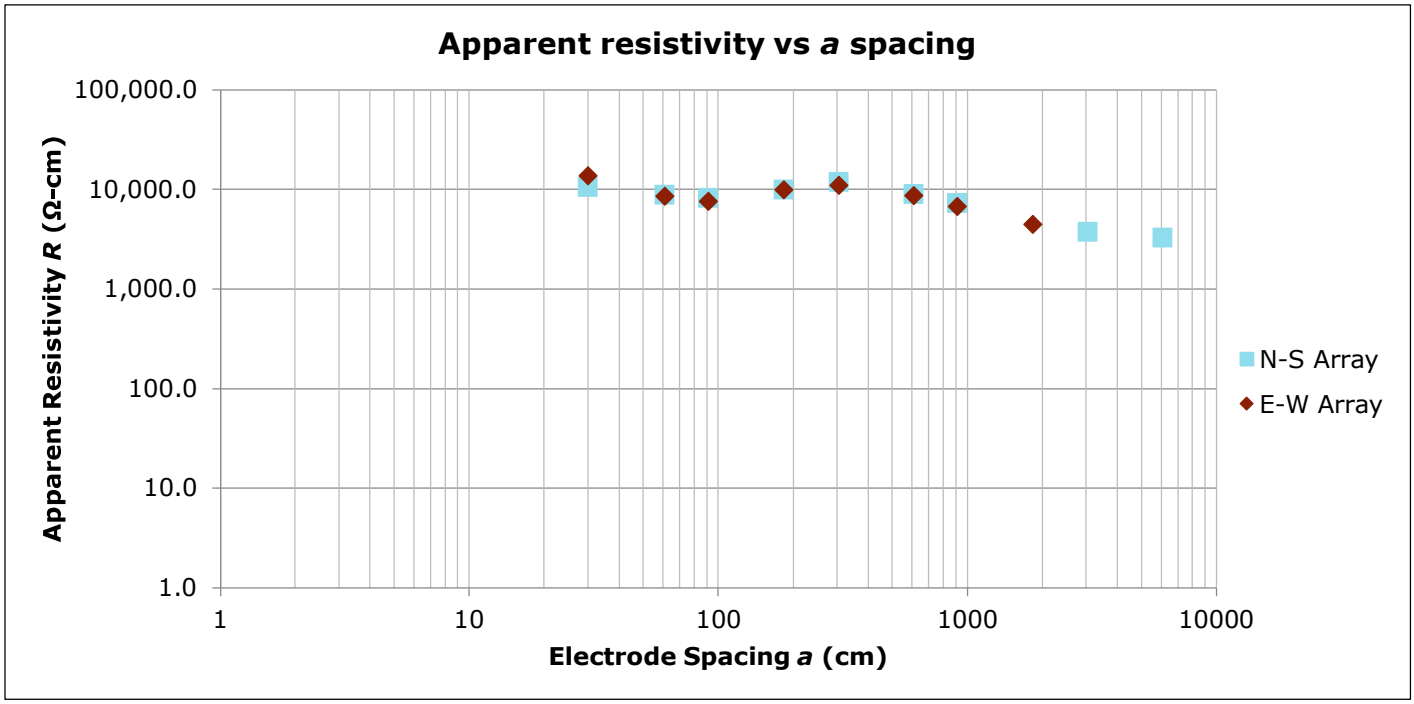


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-6 (33.6574, -114.7060)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MP
Test Date	October 26, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Maximum 'a' spacing along E-W and N-S array could not be reached due to site constraints. Interference observed at 60' on N-S array and a reading could not be obtained.		

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	7	18	40	10580	52	13730
2	61	7	18	20	8770	20	8540
3	91	8	20	13	8200	12	7610
6	183	8	20	8.4	9890	8.4	9880
10	305	8	20	6.2	11920	5.7	11040
20	610	9	23	2.3	8980	2.3	8650
30	914	9	23	1.3	7270	1.2	6770
60	1829	10	25		0	0.39	4490
100	3048	11	28	0.20	3760		
200	6096	11	28	0.09	3260		
300	9144	10	25		0		

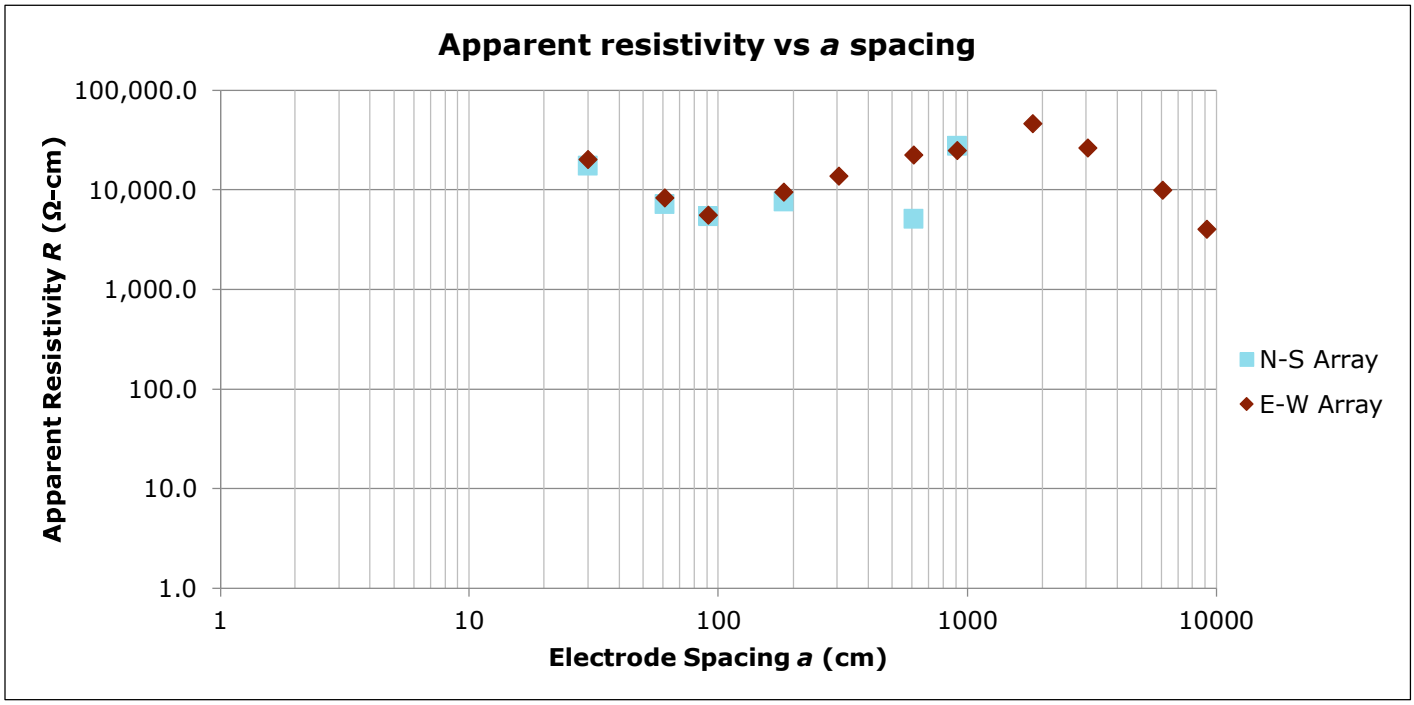


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-7 (33.6533, -114.7084)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MP
Test Date	November 2, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Maximum 'a' spacing along N-S array could not be reached due to nearby access road. Interference observed at 10' reading on N-S array and a reading could not be obtained.		

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	7	18	66	17480	76	20140
2	61	7	18	17	7170	19	8290
3	91	8	20	8.7	5390	9.0	5550
6	183	8	20	6.4	7560	8.1	9500
10	305	8	20			7.2	13830
20	610	9	23	1.3	5080	5.9	22490
30	914	10	25	4.8	27390	4.3	24970
60	1829	10	25			4.0	45950
100	3048	11	28			1.4	26360
200	6096	12	30			0.26	9920
300	9144	10	25			0.07	4030

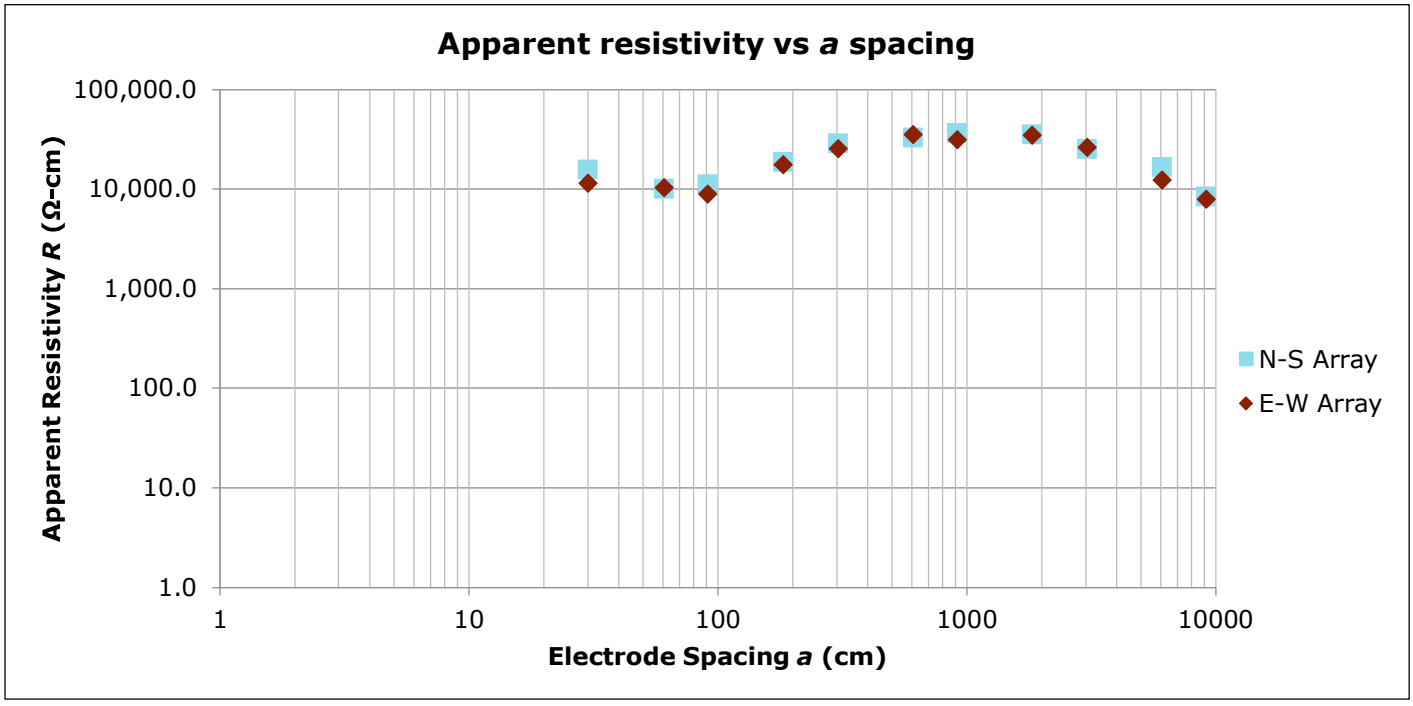


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-8 (33.6592, -114.6881)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MM
Test Date	November 8, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts			

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	6	15	63	15540	46	11480
2	61	6	15	24	9950	24	10280
3	91	6	15	19	11100	15	8920
6	183	6	15	16	18510	15	17670
10	305	8	20	15	28830	13	25560
20	610	8	20	8.5	32590	9.1	34970
30	914	9	23	6.3	36290	5.4	31090
60	1829	9	23	3.1	35060	3.0	34570
100	3048	9	23	1.3	25070	1.4	26260
200	6096	9	23	0.43	16520	0.32	12400
300	9144	10	25	0.15	8430	0.14	7920

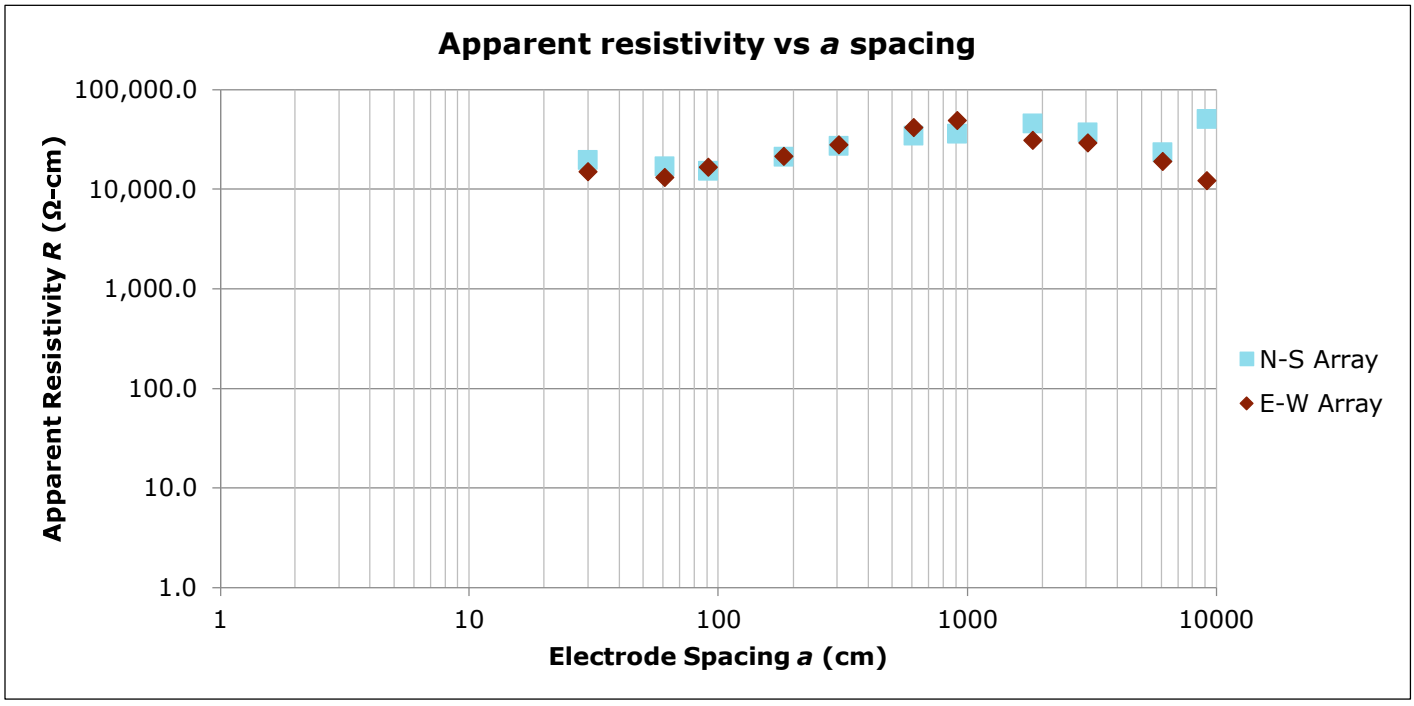


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-9 (33.6552, -114.6798)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MM
Test Date	November 14, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts			

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	6	15	80	19920	62	15290
2	61	6	15	41	17220	32	13400
3	91	6	15	26	15370	28	16870
6	183	7	18	18	21430	19	21880
10	305	7	18	14	27420	15	28280
20	610	7	18	9.1	34860	11	42190
30	914	8	20	6.4	36690	8.7	49820
60	1829	8	20	4.0	46020	2.7	31240
100	3048	8	20	2.0	37600	1.6	29800
200	6096	9	23	0.62	23720	0.51	19390
300	9144	10	25	0.89	51000	0.21	12320

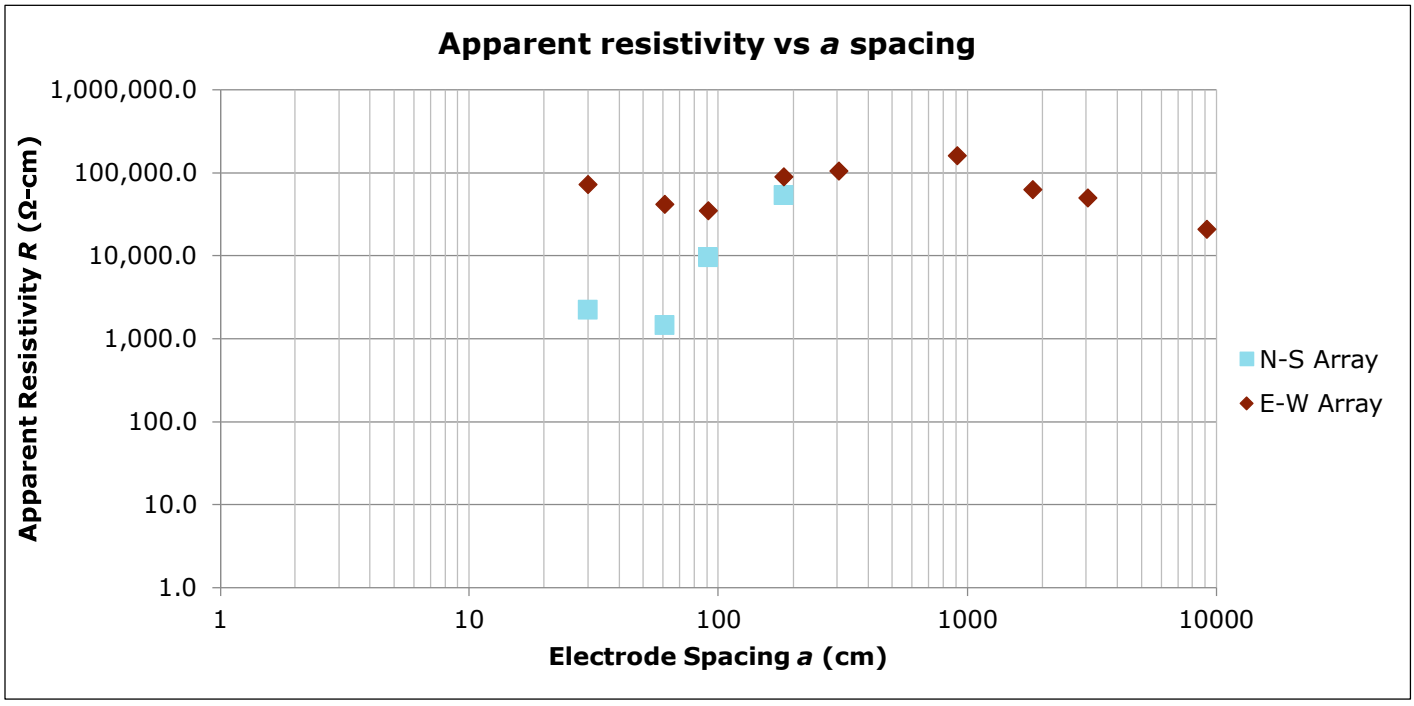


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-10 (33.6528, -114.6710)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MP
Test Date	November 7, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Test is situated within a steep sloped terrain and limited N-S array to a maximum spacing of 6'. Similarly, 20' and 200' readings along E-W array due to the sloped terrain.		

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	12	30	7.1	2270	229	72770
2	61	12	30	2.9	1470	85	42380
3	91	12	30	15	9640	53	35170
6	183	12	30	45	54430	76	90920
10	305	11	28			55	106880
20	610	11	28				
30	914	12	30			28	163460
60	1829	12	30			5.6	63930
100	3048	12	30			2.6	50450
200	6096	12	30				
300	9144	12	30			0.37	21200

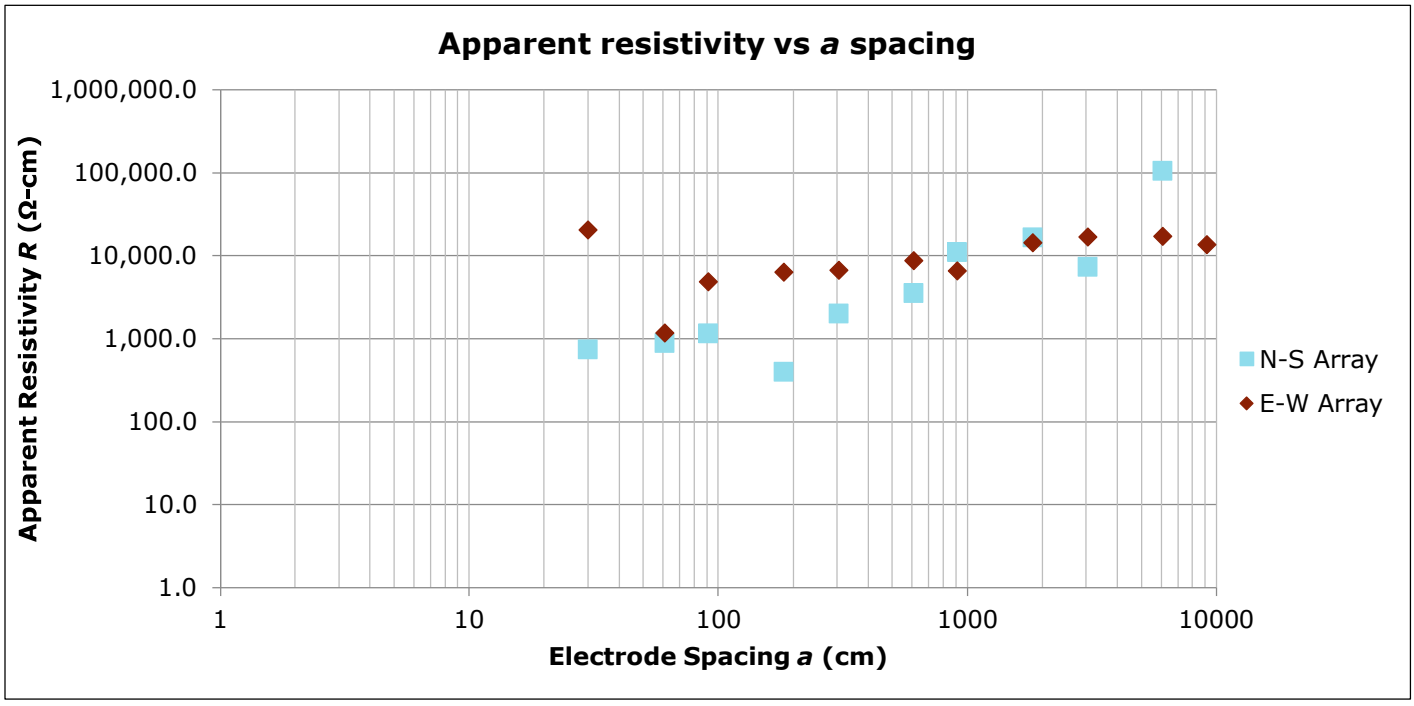


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-11 (33.6492, -114.6640)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MM
Test Date	November 7, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	300' spacing on N-S array could not be obtained due to sloped terrain.		

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	10	25	3	750	70	20920
2	61	10	25	1.9	890	2.5	1190
3	91	10	25	1.8	1180	7.7	4910
6	183	11	28	0.34	400	5.4	6410
10	305	12	30	1.0	2040	3.5	6830
20	610	12	30	0.9	3570	2.3	8820
30	914	12	30	2.0	11230	1.2	6730
60	1829	12	30	1.5	16760	1.3	14610
100	3048	12	30	0.39	7460	0.89	17060
200	6096	12	30	2.8	106520	0.45	17310
300	9144	12	30			0.24	13880

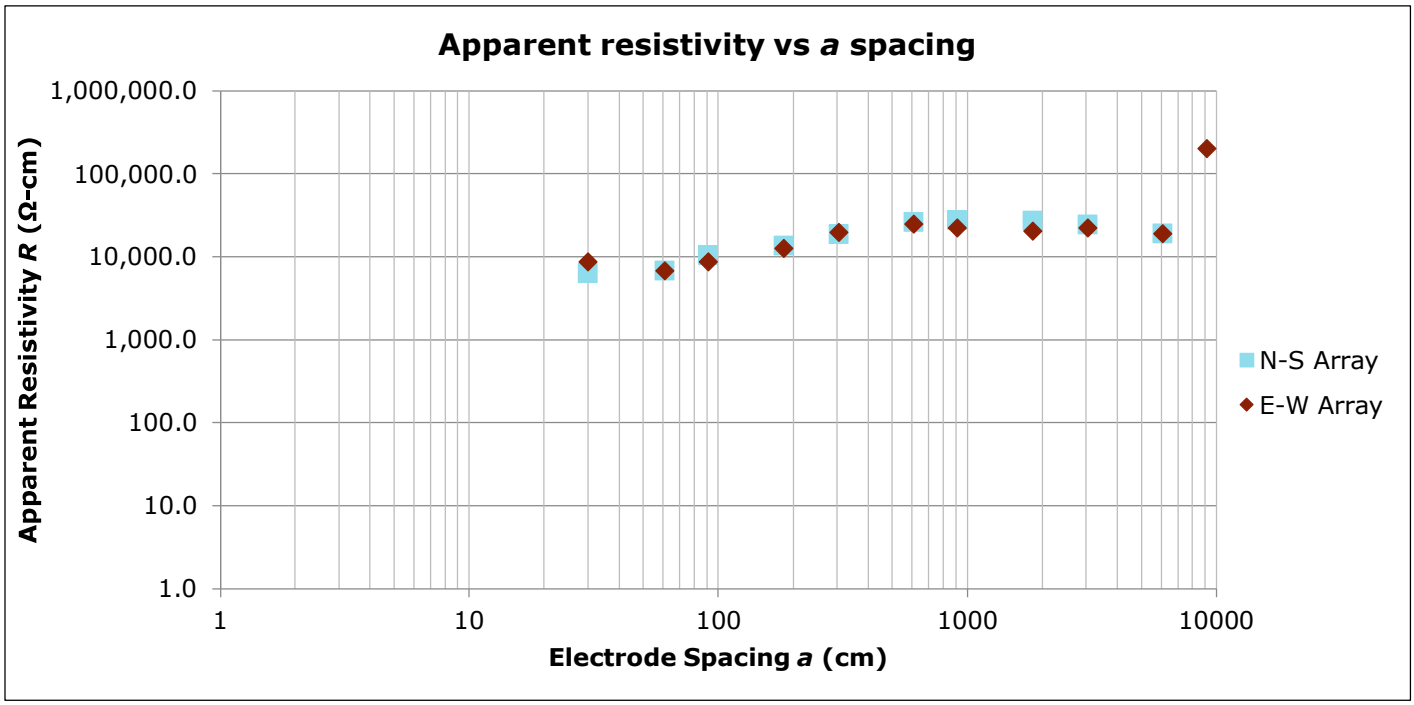


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-12 (33.6492, -114.6819)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MM
Test Date	November 12, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Interference observed at 300' spacing on N-S array and as such, a reading could not be obtained.		

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	6	15	26	6340	35	8710
2	61	6	15	16	6800	16	6830
3	91	6	15	17	10410	14	8630
6	183	7	18	12	13640	11	12680
10	305	8	20	9.6	18510	10	19710
20	610	8	20	6.8	26290	6.5	24860
30	914	9	23	4.8	27370	3.9	22270
60	1829	9	23	2.4	27250	1.8	20500
100	3048	9	23	1.3	24170	1.2	22430
200	6096	9	23	0.50	19120	0.50	19070
300	9144	11	28			3.5	200110

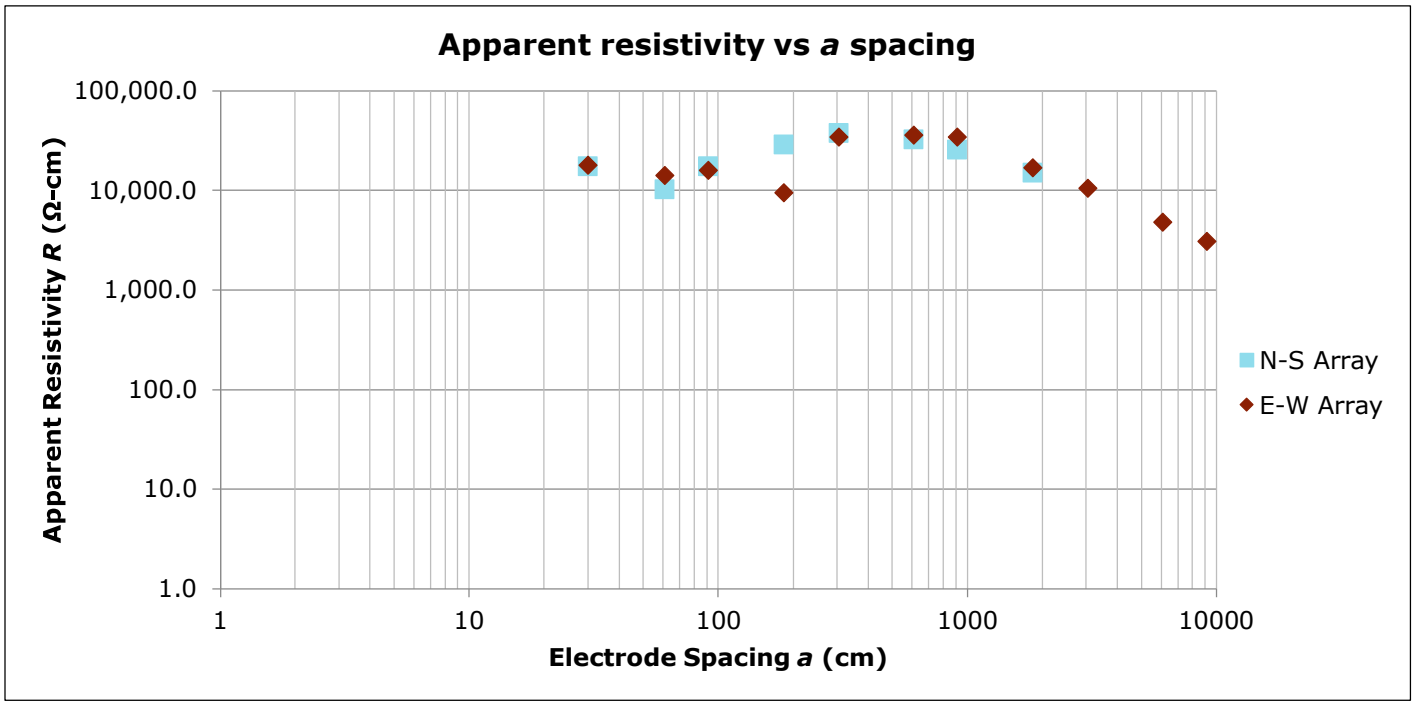


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-13 (33.6660, -114.6973)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MP
Test Date	October 25, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Intereference observed at 100, 200, and 300' spacings on N-S array and as such, a reading could not be obtained.		

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	6	15	70	17360	72	17940
2	61	6	15	24	10220	34	14220
3	91	6	15	29	17300	27	15940
6	183	7	18	25	28690	8.1	9510
10	305	7	18	19	37480	17.8	34360
20	610	8	20	8.4	32420	9.3	35890
30	914	8	20	4.4	25500	6.0	34300
60	1829	9	23	1.3	14900	1.5	17010
100	3048	9	23			0.55	10470
200	6096	9	23			0.13	4830
300	9144	10	25			0.05	3070

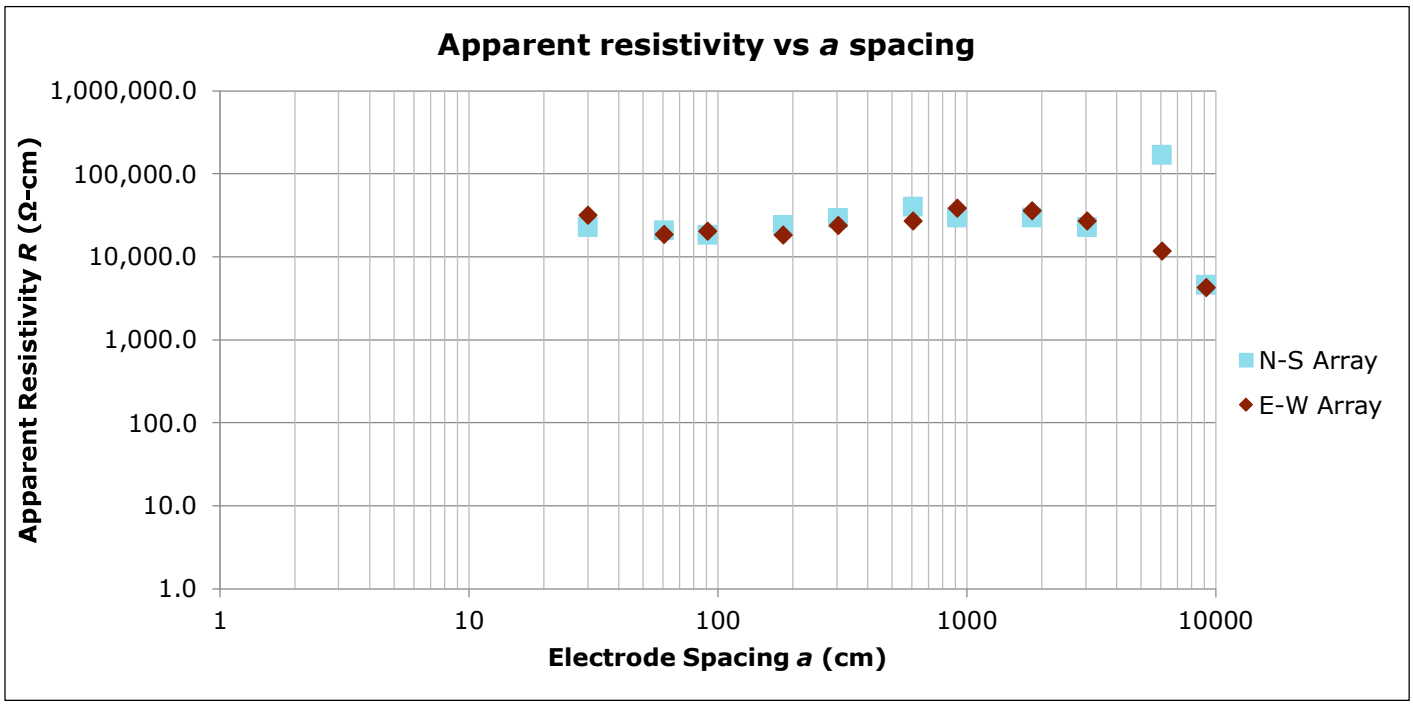


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-14 (33.6486, -114.7067)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MP
Test Date	November 3, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts			

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	8	20	83	22930	118	32520
2	61	9	23	46	21070	41	18830
3	91	9	23	30	18790	33	20700
6	183	9	23	20	24180	16	18790
10	305	9	23	15	29690	13	24350
20	610	9	23	11	41070	7.2	27550
30	914	10	25	5.2	29870	6.9	39670
60	1829	11	28	2.6	29870	3.2	36940
100	3048	11	28	1.2	23200	1.4	27410
200	6096	12	30	4.5	172830	0.31	12000
300	9144	12	30	0.081	4650	0.076	4370

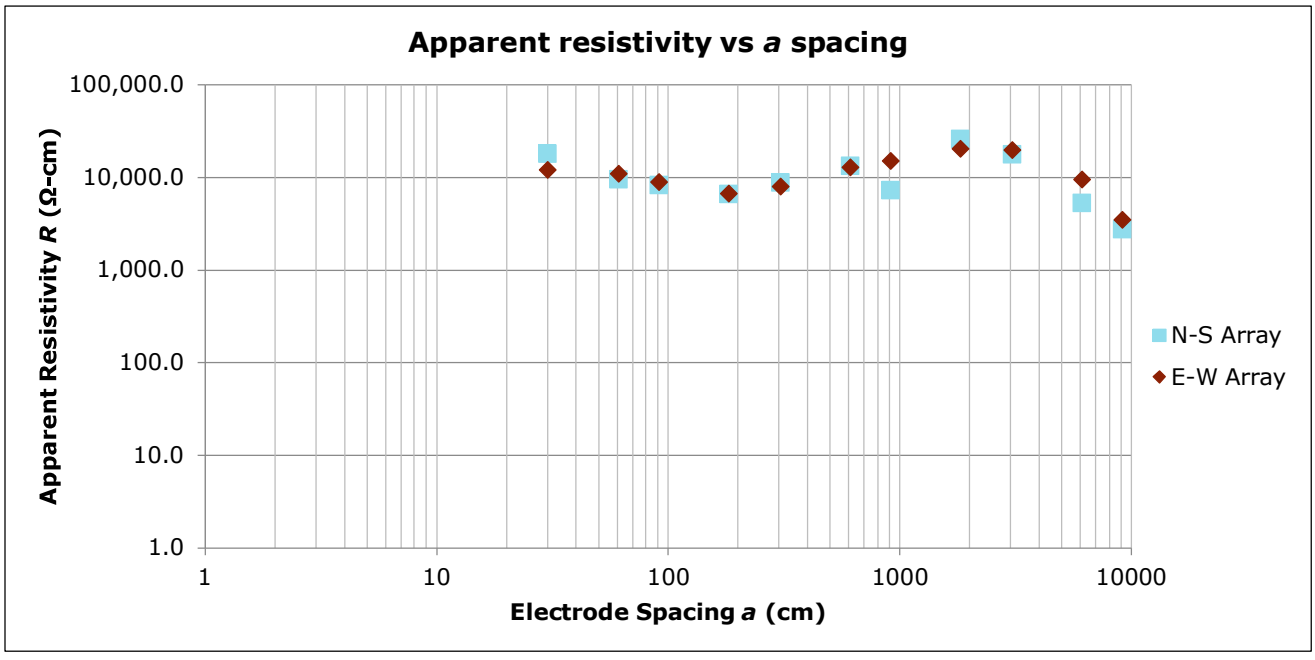


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-15 (33.6502, -114.7152)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MP
Test Date	November 2, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts			

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	7	18	68	17990	45	12030
2	61	8	20	21	9490	25	10930
3	91	8	20	13	8270	14	8920
6	183	8	20	5.7	6660	5.7	6720
10	305	8	20	4.5	8740	4	7970
20	610	8	20	3.5	13300	3.4	12970
30	914	8	20	1.3	7290	2.7	15300
60	1829	8	20	2.2	25760	1.8	20520
100	3048	9	23	0.92	17630	1.0	19840
200	6096	9	23	0.14	5350	0.25	9480
300	9144	9	23	0.05	2790	0.06	3510
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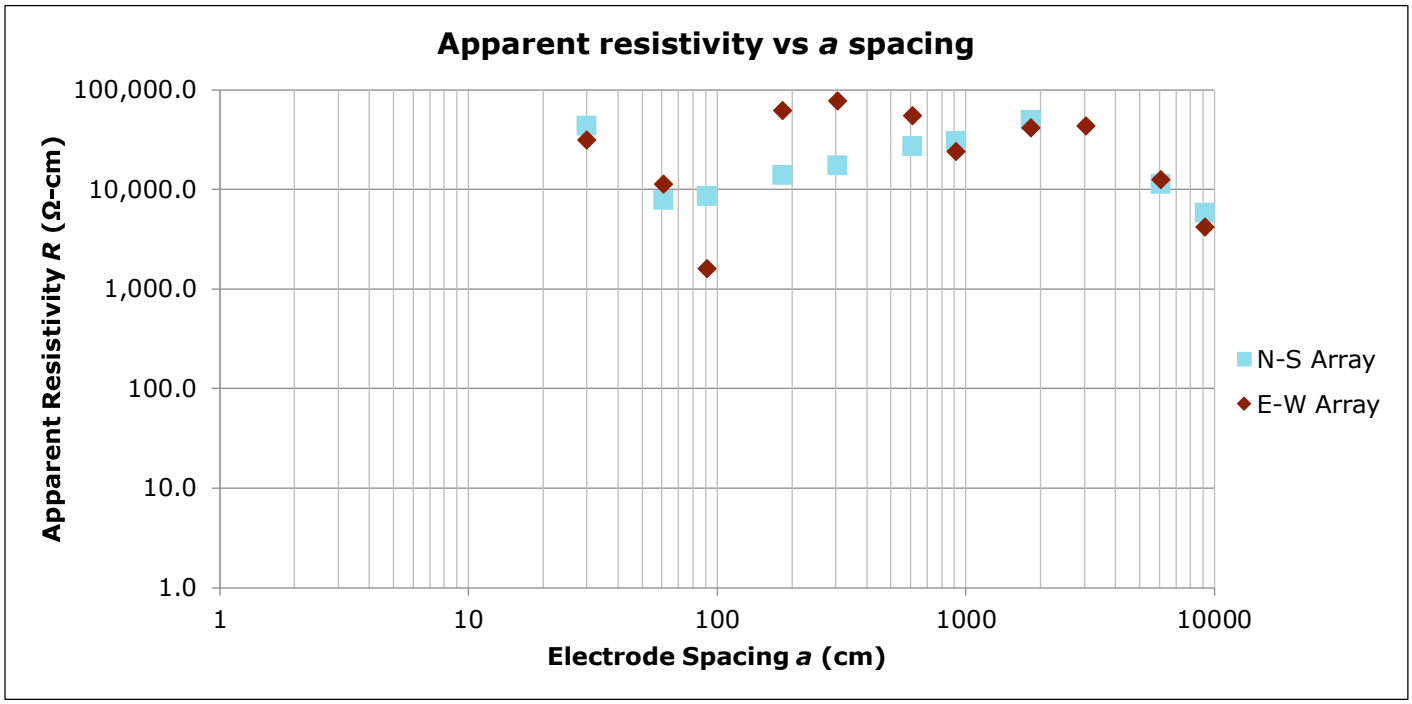


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-16 (33.6660, -114.6973)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MM
Test Date	November 13, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Interference was observed at 100' on N-S array and as such, a reading could not be obtained.		

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	6	15	179	44400	128	31850
2	61	6	15	19	7910	27	11530
3	91	7	18	14	8610	2.7	1630
6	183	8	20	12	14030	54	63410
10	305	8	20	9.1	17560	41	78850
20	610	9	23	7.2	27510	15	55790
30	914	9	23	5.4	31160	4.3	24510
60	1829	10	25	4.4	50400	3.7	42190
100	3048	10	25			2.3	44470
200	6096	11	28	0.30	11530	0.33	12790
300	9144	12	30	0.10	5890	0.07	4270

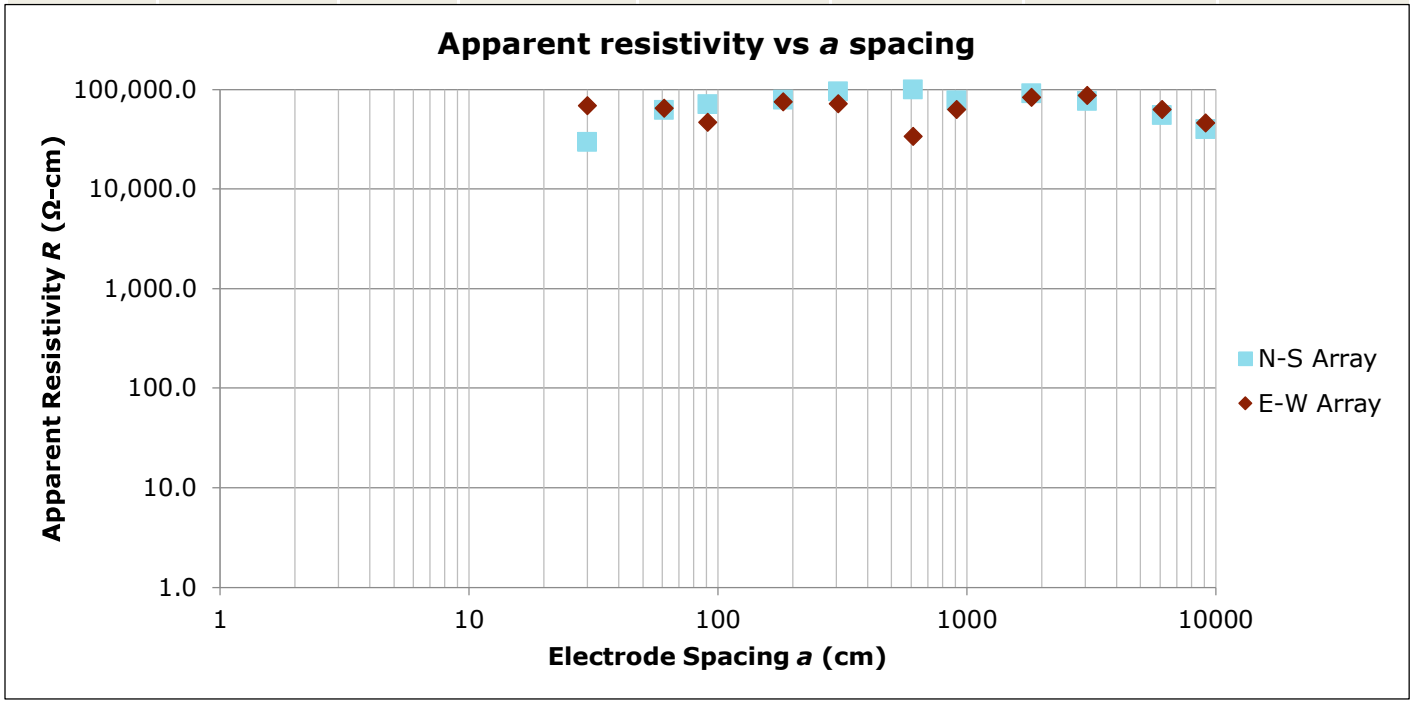


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-17 (33.6350, -114.6900)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MM
Test Date	November 13, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts			

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	8	20	107	29460	249	68510
2	61	9	23	133	61470	140	64740
3	91	9	23	112	70680	74	46680
6	183	10	25	66	78090	63	74260
10	305	11	28	49	95110	37	71800
20	610	12	30	26	99220	8.7	33470
30	914	12	30	13	76750	11	62430
60	1829	12	30	7.9	90280	7.2	82450
100	3048	12	30	4.0	76080	4.5	86160
200	6096	12	30	1.4	55000	1.6	62170
300	9144	12	30	0.69	39430	0.80	45820

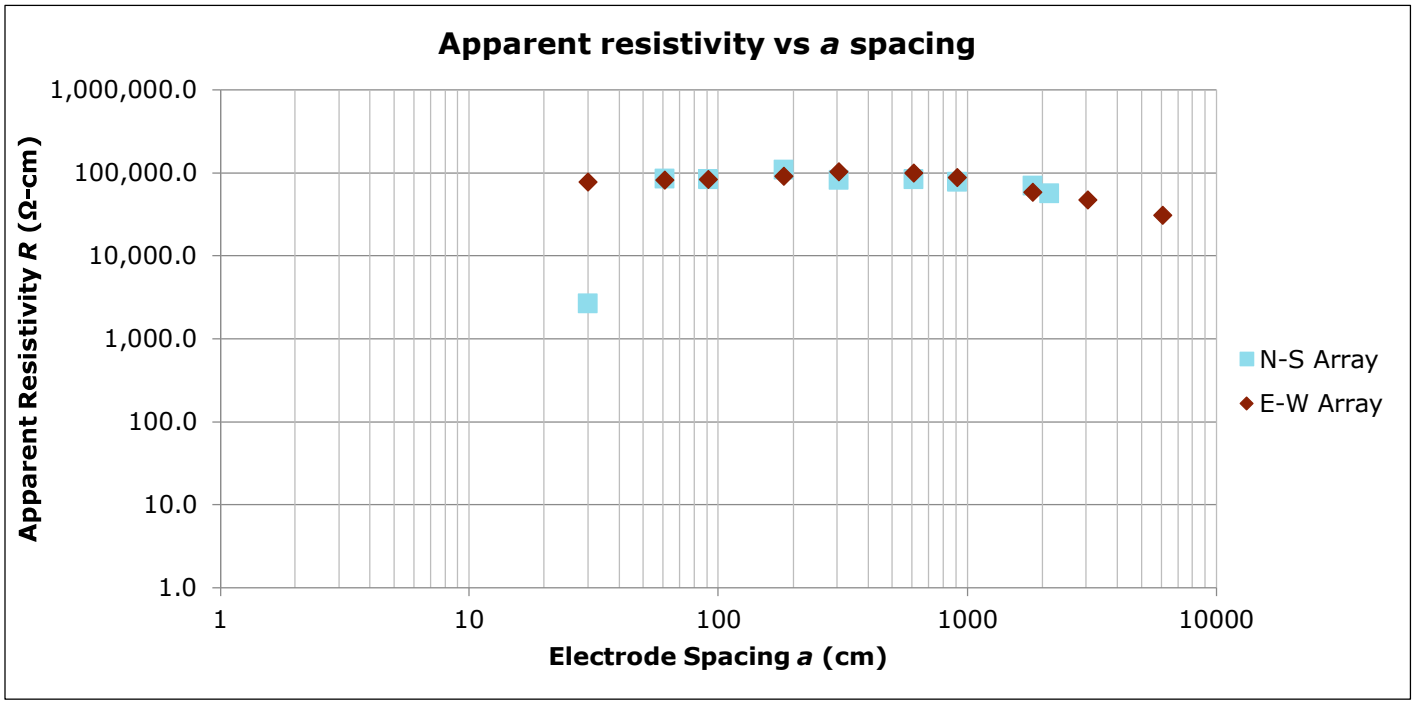


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-18 (33.6384, -114.6818)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MP
Test Date	November 15, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Maximum 'a' spacing could not be reached in N-S and E-W array due to site limitations. ER spacing of 70' was added to N-S array as this was the maximum spacing permitted by site constraints.		

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	7	18	10	2690	299	79250
2	61	8	20	192	85410	185	82380
3	91	8	20	138	85170	138	85170
6	183	8	20	95	110900	78	91900
10	305	8	20	43	83370	54	104840
20	610	9	23	22	84840	26	101550
30	914	9	23	14	79050	16	89170
60	1829	9	23	6.1	70180	5.2	59530
70	2134	9	23	4.3	57590		
100	3048	8	20			2.5	47790
200	6096	8	20			0.81	31040
300	9144	12	30				

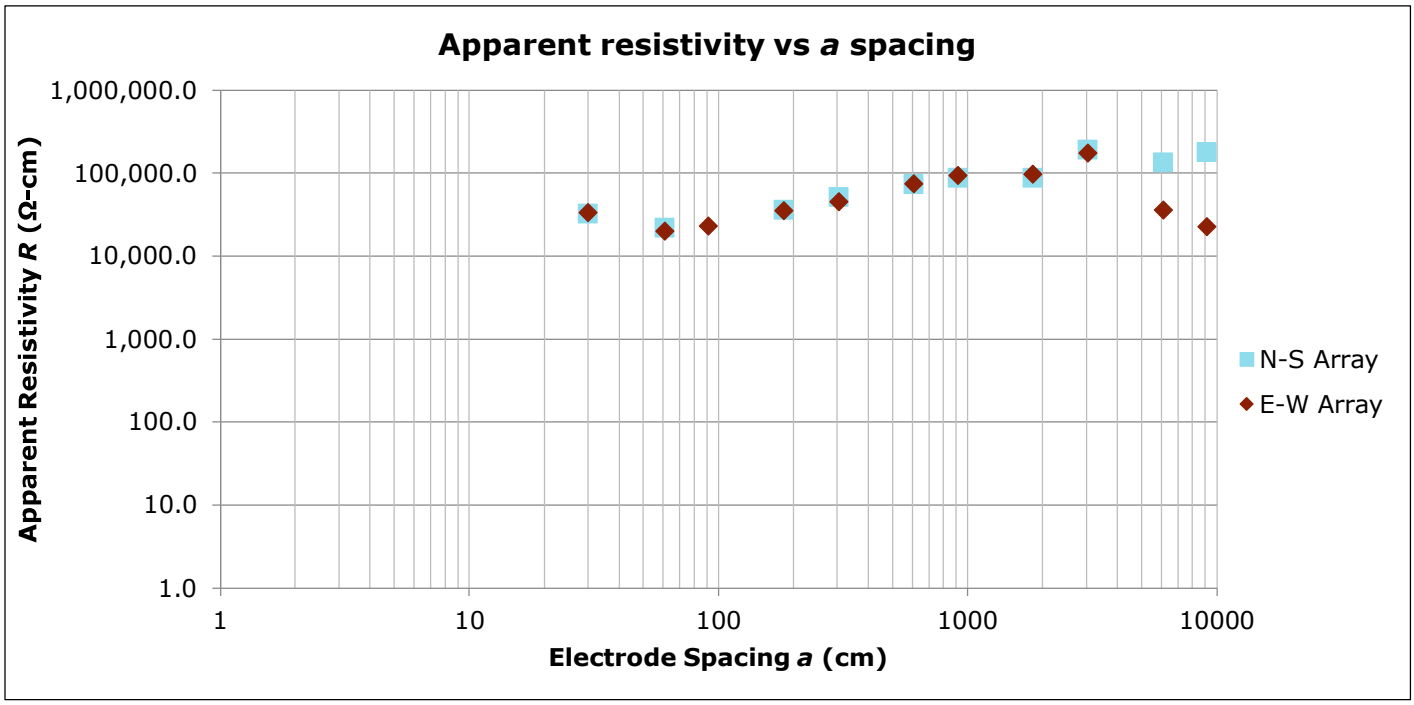


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-19 (33.6457, -114.6784)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MP
Test Date	November 14, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Interference observed at 3' on N-S array and as such, a reading could not be obtained.		

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	7	18	124	32720	129	34230
2	61	7	18	51	22130	47	20450
3	91	9	23			37	23550
6	183	9	23	31	36480	31	36100
10	305	9	23	27	52600	24	46080
20	610	10	25	19	74150	20	76300
30	914	11	28	16	89790	17	95430
60	1829	11	28	7.7	88640	8.6	98810
100	3048	11	28	10	195950	9.2	176640
200	6096	11	28	3.6	137090	1.0	36540
300	9144	11	28	3.2	181210	0.40	23150

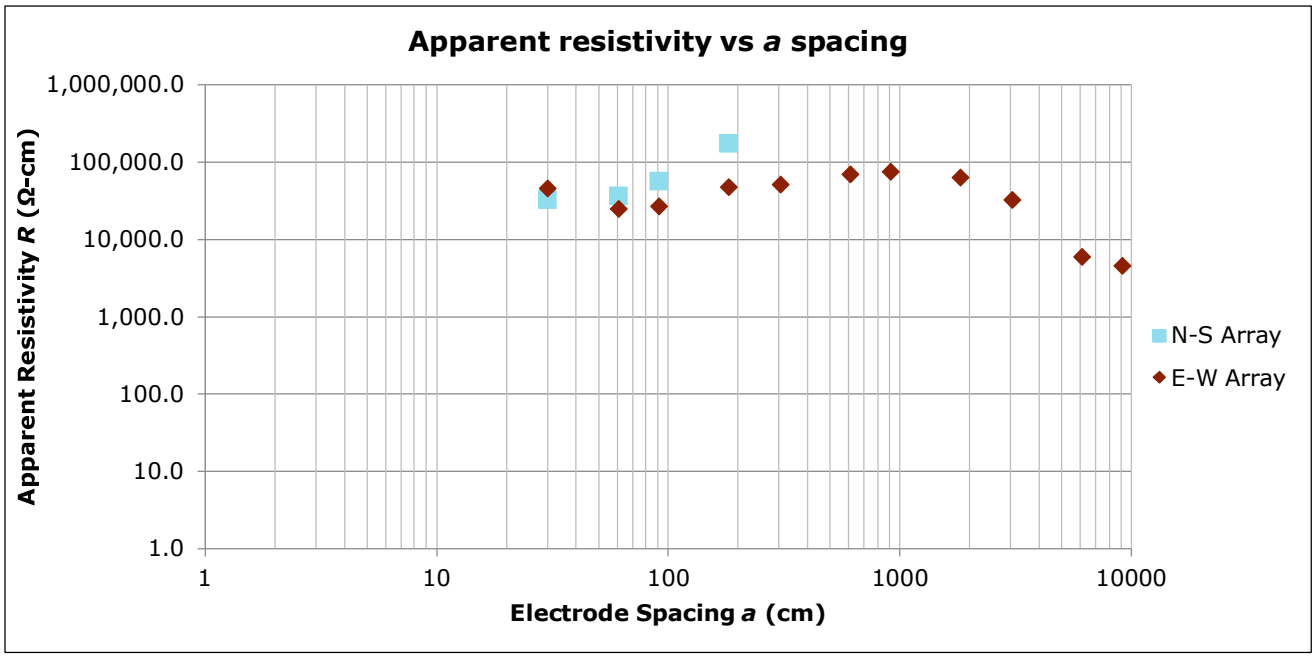


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-20 (33.6667, -114.6845)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MP
Test Date	October 24, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Overhead powerline runs parallel to N-S array and as such, interference was observed within N-S array readings. Test concluded early.		

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	4	10	149	32750	209	46000
2	61	5	13	89	36550	61	25170
3	91	5	13	96	56590	45	26830
6	183	5	13	151	175140	41	47430
10	305	6	15			27	51330
20	610	6	15			18	70170
30	914	8	20			13	75640
60	1829	8	20			5.4	62450
100	3048	8	20			1.7	32370
200	6096	9	23			0.15	5920
300	9144	10	25			0.08	4580
							#N/A

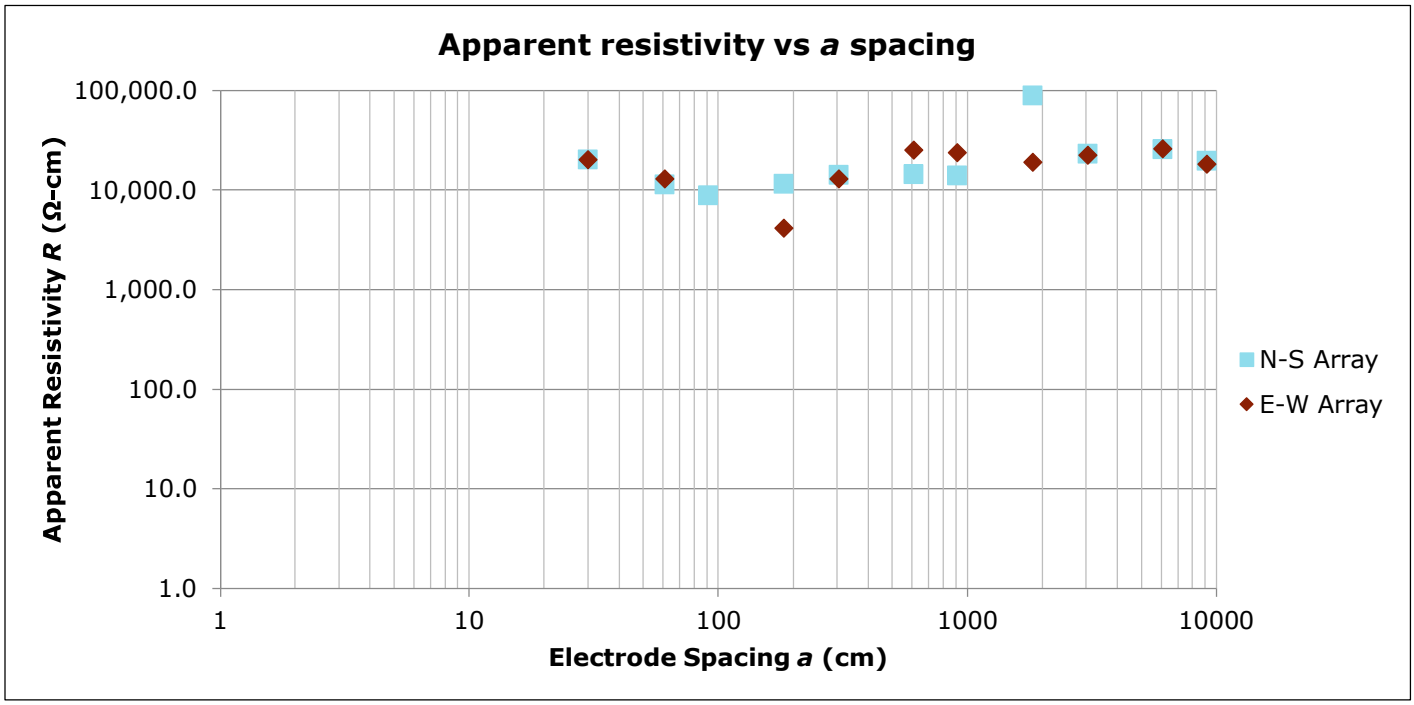


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace | Blythe, California
Terracon Project No. 60225164

Array Loc.	ER-21 (33.6636, -114.6924)		
Instrument	MiniSting	**Weather**	Sunny
Serial #	SZ107129	**Ground Cond.**	Exposed Soils
Cal. Check		**Tested By**	DM/MM
Test Date	November 7, 2023	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Intereference observed at 3' on E-W array and as such, a reading could not be obtained.		

Apparent resistivity ρ is calculated as : $\rho = \dfrac{4\pi aR}{1 + \dfrac{2a}{\sqrt{a^2+4b^2}} - \dfrac{a}{\sqrt{a^2+b^2}}}$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
1	30	4	10	92	20290	92	20160
2	61	6	15	27	11400	31	12870
3	91	7	18	14	8760		
6	183	7	18	9.9	11540	3.6	4160
10	305	8	20	7.3	14170	6.7	12950
20	610	9	23	3.7	14250	6.6	25370
30	914	9	23	2.4	13870	4.1	23630
60	1829	10	25	7.7	88240	1.7	19050
100	3048	10	25	1.2	22930	1.2	22490
200	6096	10	25	0.67	25670	0.67	25760
300	9144	10	25	0.34	19680	0.32	18200



Test Pile Driving Data

Contents:

Test Pile Installation Details (2 Pages)
Pile Load Testing Zoning Plan (1 Page)
Pile Load Test Location Plan (6 Pages)
Test Pile Driving Records (12 Pages)

Pile Location Procedures

The test locations were established in the field by using a hand-held GPS (accurate to about ±10 to 15 feet) and existing site features as reference points. The mapped test locations should be considered accurate only to the degree implied by the means and methods used to define them.

Test Pile Details

Terracon provided the steel piles to the job site. The piles were driven to facilitate performing tension (pull-out), lateral, and compression tests. The bare steel sections have the following properties[3]:

Parameter	W6x9
Depth	5.900 in
Flange Width, b_f	3.940 in
Flange Thickness, t_f	0.215 in
Web Thickness, t_w	0.170 in
Moment of Inertia, I_x	16.40 in^4
Section Area, A	2.68 in^2
Young's Modulus, E_s	29,000 ksi
Yield Stress, F_y	50 ksi
Hot Dip Galvanization	None

Test Pile Installation

The test piles consisted of wide-flange, bare steel W6x9 sections. A group of two test piles were installed at each of the thirty-five test locations and a third pile for compression testing was installed at each location. The pile identification system for each location begins with "PLT" and is followed by the number corresponding to the test pile group location and a letter identifying the pile. The "A" piles were installed to 7 feet below grade and the "B" piles were installed to 10 feet below grade. "C" piles were installed to 7 feet below grade and were solely for compression load testing.

[3] American Institute of Steel Construction (AISC), *"Steel Construction Manual – Fourteenth Edition"* February 2012.

The piles were advanced with a track mounted Gayk HRE equipped with a hydraulic hammer to embedment depths ranging from approximately 7 to 10 feet below the ground surface (bgs). The time rate of installation was recorded with a stopwatch. The total time required to advance each pile to its specified embedment depth was recorded and is summarized in the following installation plots.

Pile Load Testing Zoning Plan

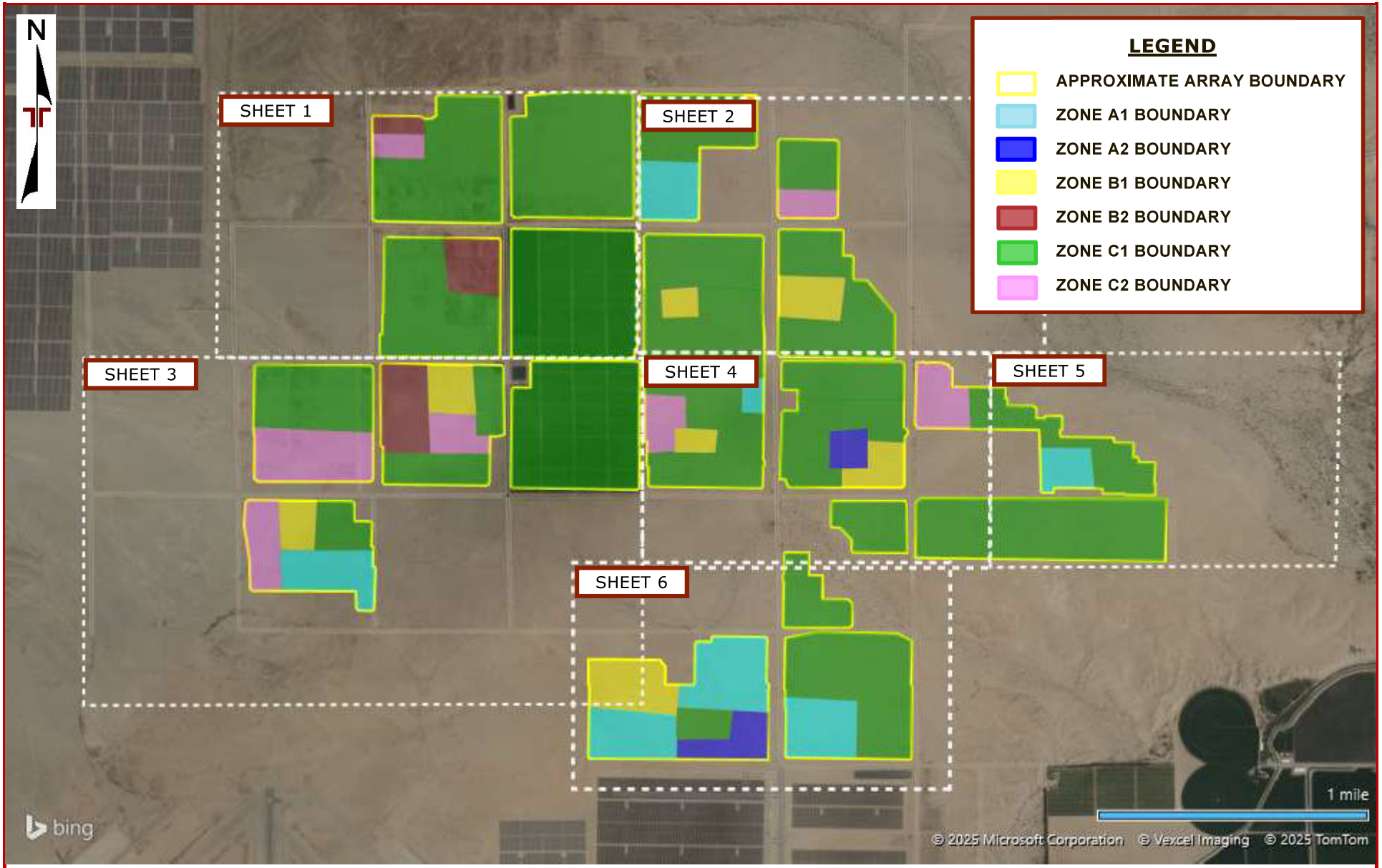


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Pile Load Test Location Plan (Sheet 1)



DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Pile Load Test Location Plan (Sheet 2)

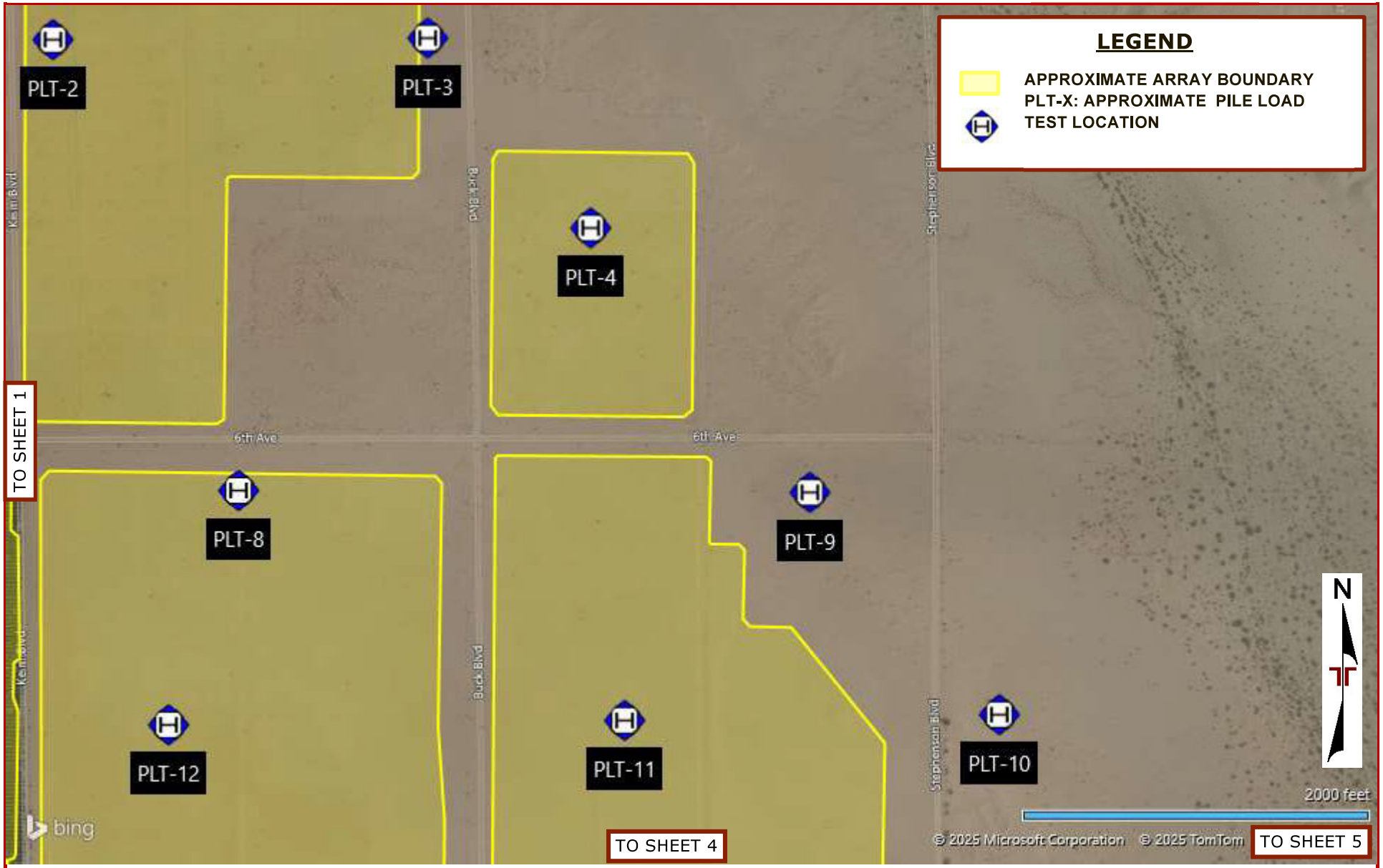


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Pile Load Test Location Plan (Sheet 3)

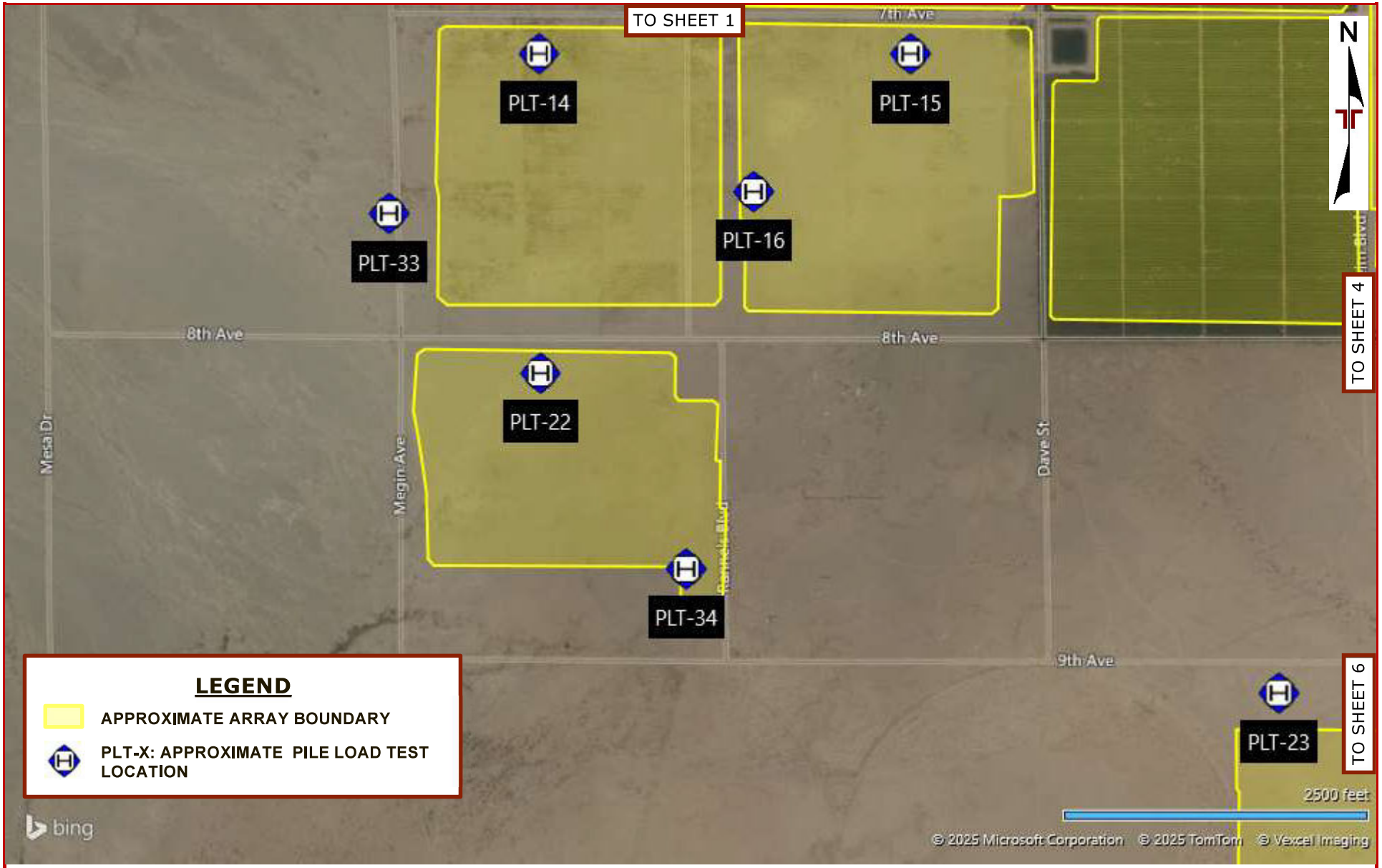


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Pile Load Test Location Plan (Sheet 4)

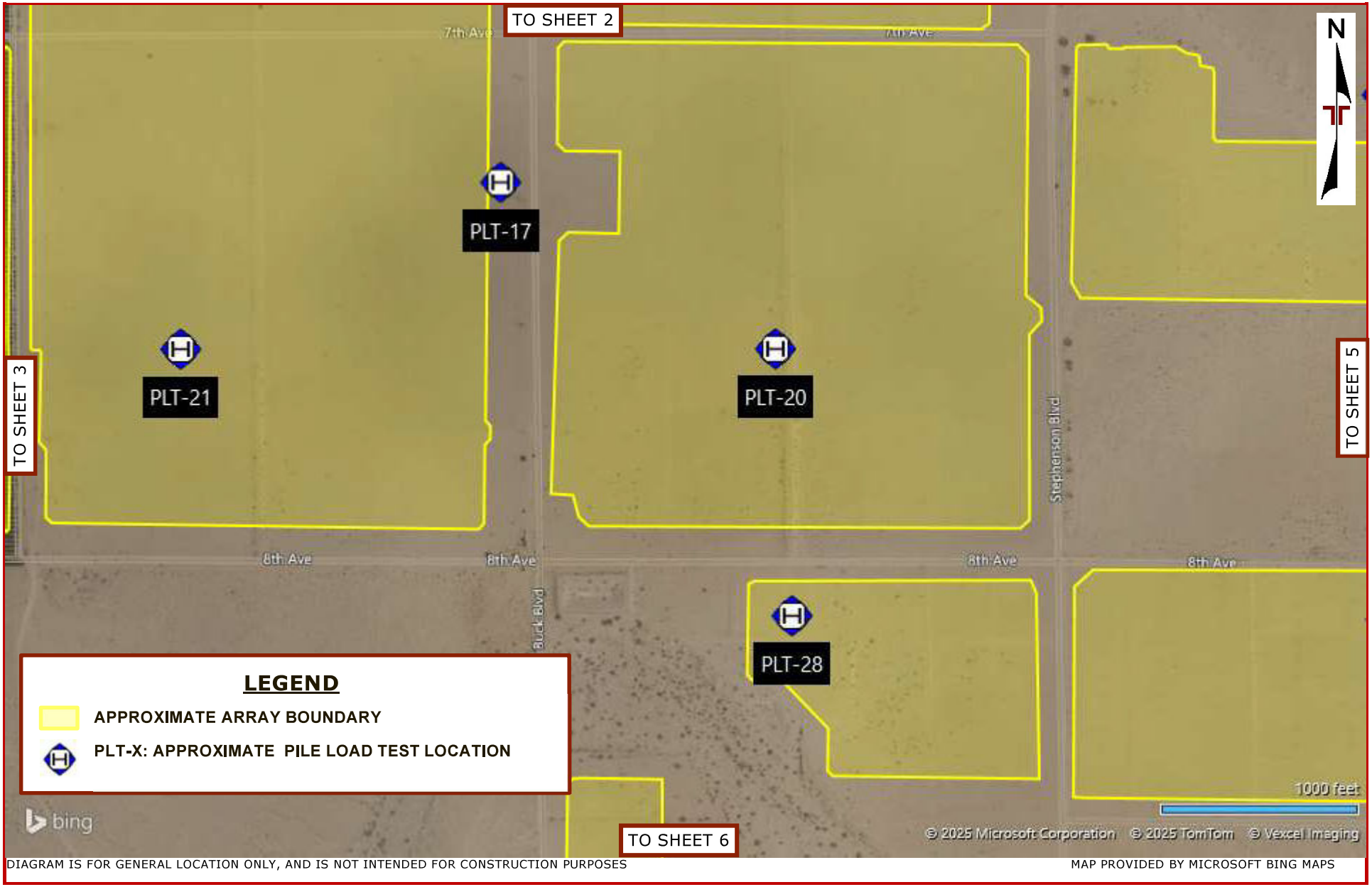


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Pile Load Test Location Plan (Sheet 5)

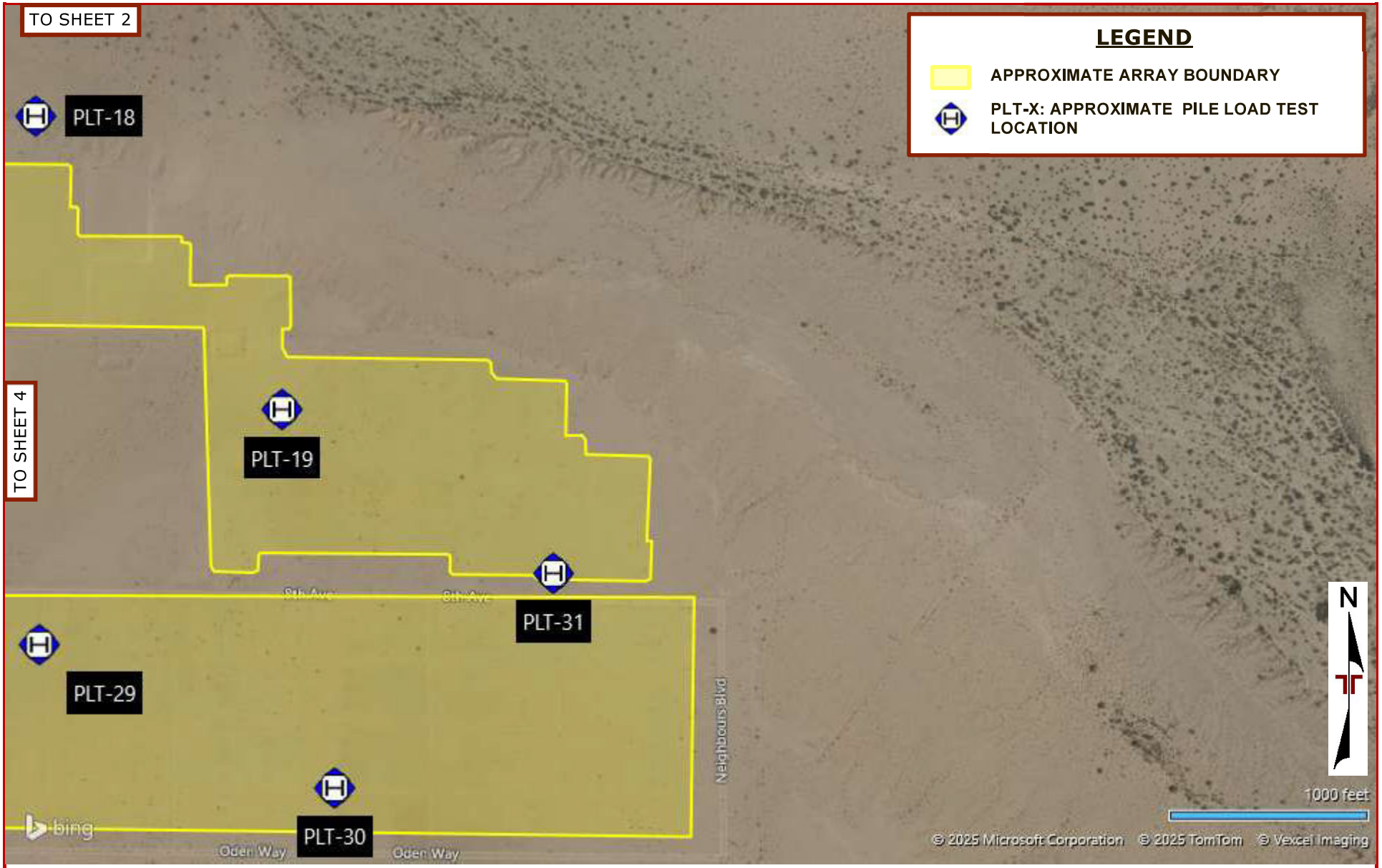


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Pile Load Test Location Plan (Sheet 6)

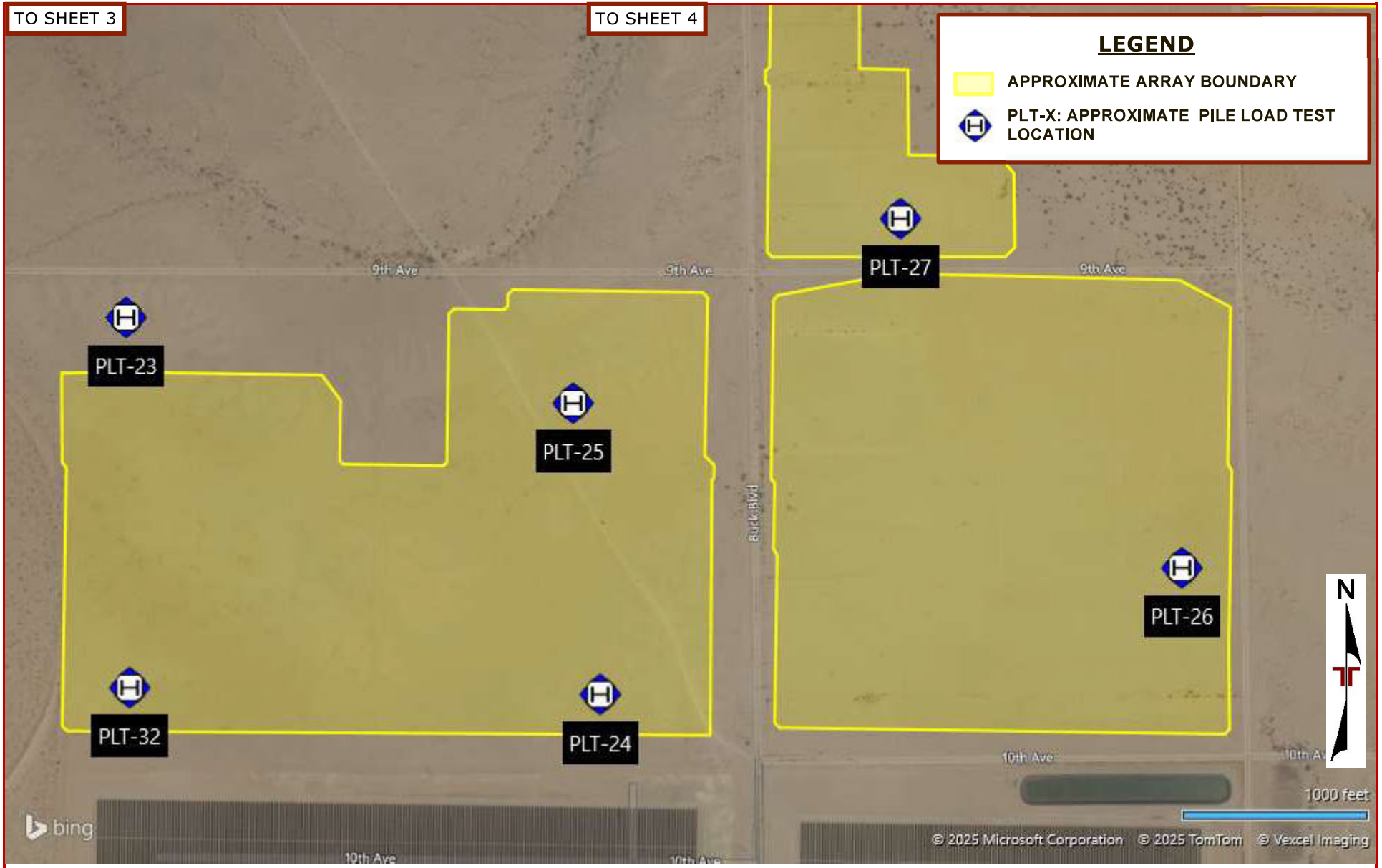


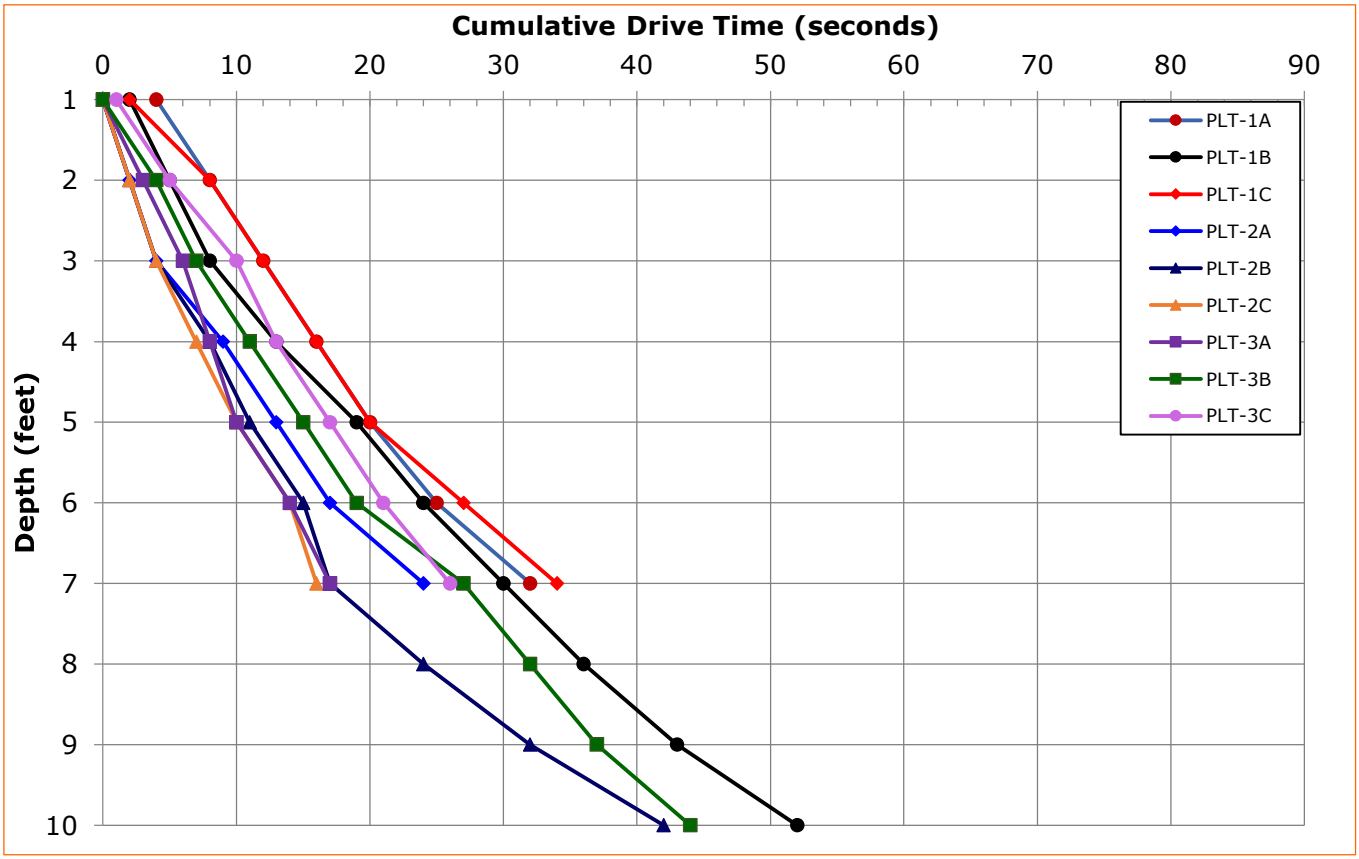
DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

TEST PILE DRIVING RECORDS

Grace Solar

Terracon Project No. 60225164



Depth Range, feet	Cumulative Driving Time, seconds (See Note 1)								
	PLT-1A	PLT-1B	PLT-1C	PLT-2A	PLT-2B	PLT-2C	PLT-3A	PLT-3B	PLT-3C
Pile Push (in.) (See Note 2)	-	-	-	-	-	-	-	-	12
0' to 1'	4	2	2	0	0	0	0	0	1
1' to 2'	8	5	8	2	2	2	3	4	5
2' to 3'	12	8	12	4	4	4	6	7	10
3' to 4'	16	13	16	9	8	7	8	11	13
4' to 5'	20	19	20	13	11	10	10	15	17
5' to 6'	25	24	27	17	15	14	14	19	21
6' to 7'	32	30	34	24	17	16	17	27	26
7' to 8'	x	36	x	x	24	x	x	32	x
8' to 9'	x	43	x	x	32	x	x	37	x
9' to 10'	x	52	x	x	42	x	x	44	x
10' to 11'	x	x	x	x	x	x	x	x	x
11' to 12'	x	x	x	x	x	x	x	x	x
Refusal	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**
Embedment Depth, ft.	**7**	**10**	**7**	**7**	**10**	**7**	**7**	**10**	**7**
Total Drive Time, sec.	**32**	**52**	**34**	**24**	**42**	**16**	**17**	**44**	**26**
Average, sec./ft.	**4.6**	**5.2**	**4.9**	**3.4**	**4.2**	**2.3**	**2.4**	**4.4**	**3.7**

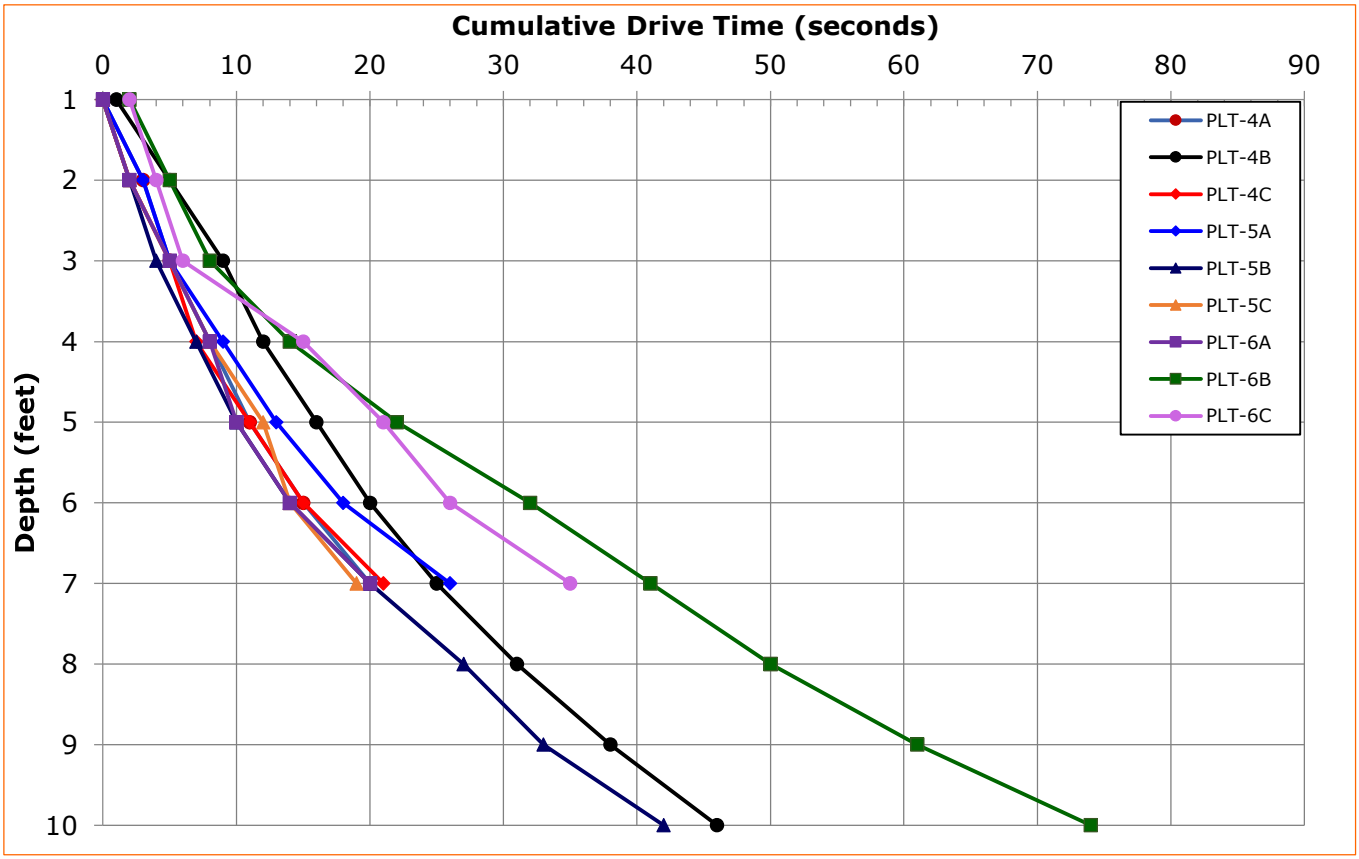
NOTES:

1. Piles advanced with a track mounted Gayk HRE 1000 hydraulic hammer.
2. Pile driving started at the end of pile push depth in inches.
3. Piles were installed between October 2 and October 6, 2023.

TEST PILE DRIVING RECORDS

Grace Solar

Terracon Project No. 60225164



Depth Range, feet	Cumulative Driving Time, seconds (See Note 1)								
	PLT-4A	PLT-4B	PLT-4C	PLT-5A	PLT-5B	PLT-5C	PLT-6A	PLT-6B	PLT-6C
Pile Push (in.) (See Note 2)	-	-	-	-	-	-	-	-	-
0' to 1'	0	1	0	0	0	0	0	2	2
1' to 2'	3	5	2	3	2	2	2	5	4
2' to 3'	5	9	5	5	4	5	5	8	6
3' to 4'	8	12	7	9	7	8	8	14	15
4' to 5'	11	16	11	13	10	12	10	22	21
5' to 6'	15	20	15	18	14	14	14	32	26
6' to 7'	20	25	21	26	20	19	20	41	35
7' to 8'	x	31	x	x	27	x	x	50	x
8' to 9'	x	38	x	x	33	x	x	61	x
9' to 10'	x	46	x	x	42	x	x	74	x
10' to 11'	x	x	x	x	x	x	x	x	x
11' to 12'	x	x	x	x	x	x	x	x	x
Refusal	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**
Embedment Depth, ft.	**7**	**10**	**7**	**7**	**10**	**7**	**7**	**10**	**7**
Total Drive Time, sec.	**20**	**46**	**21**	**26**	**42**	**19**	**20**	**74**	**35**
Average, sec./ft.	**2.9**	**4.6**	**3.0**	**3.7**	**4.2**	**2.7**	**2.9**	**7.4**	**5.0**

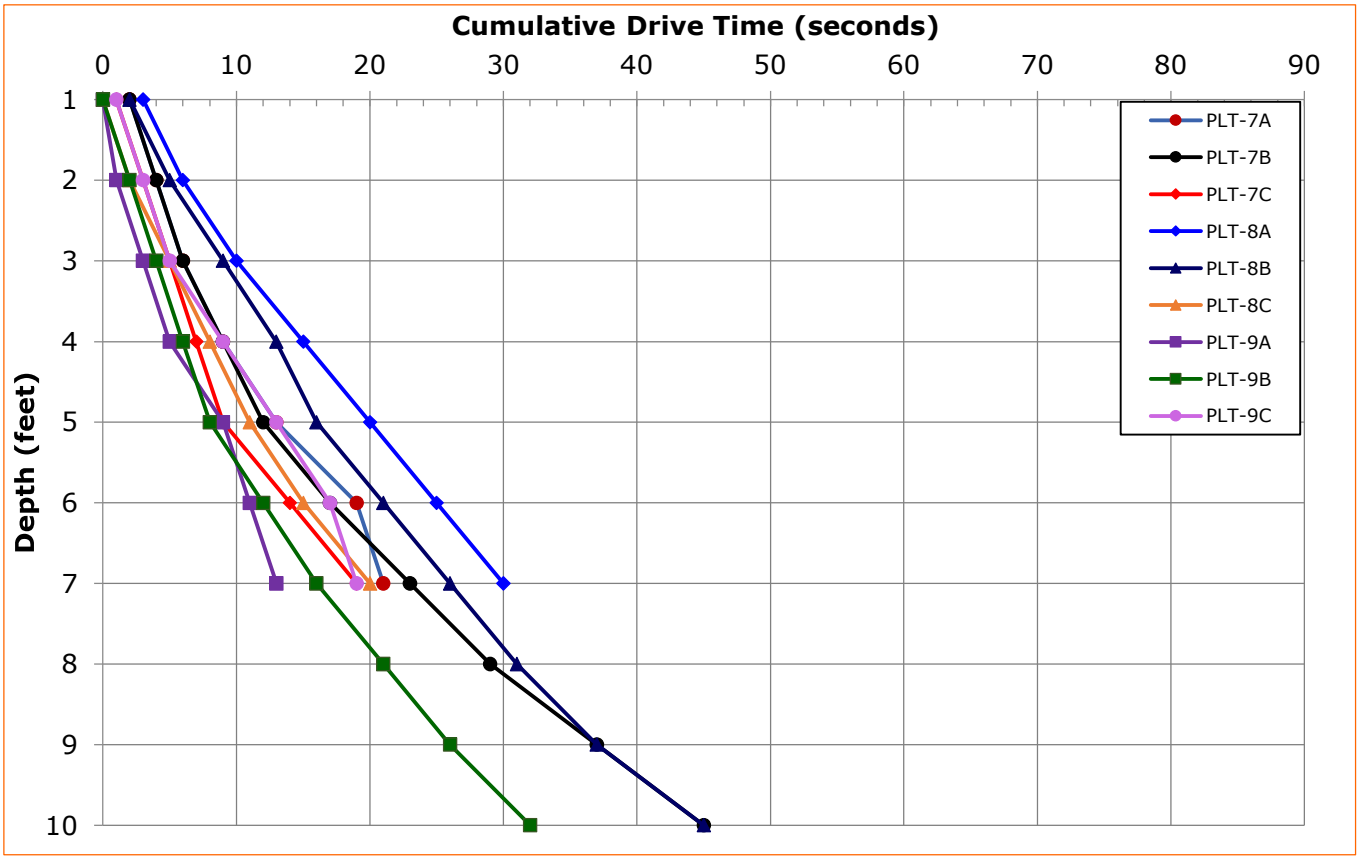
NOTES:

1. Piles advanced with a track mounted Gayk HRE 1000 hydraulic hammer.
2. Pile driving started at the end of pile push depth in inches.
3. Piles were installed between October 2 and October 6, 2023.

TEST PILE DRIVING RECORDS

Grace Solar

Terracon Project No. 60225164



Depth Range, feet	Cumulative Driving Time, seconds (See Note 1)								
	PLT-7A	PLT-7B	PLT-7C	PLT-8A	PLT-8B	PLT-8C	PLT-9A	PLT-9B	PLT-9C
Pile Push (in.) (See Note 2)	-	-	-	-	-	-	-	12	12
0' to 1'	2	2	1	3	2	0	0	0	1
1' to 2'	4	4	3	6	5	2	1	2	3
2' to 3'	6	6	5	10	9	5	3	4	5
3' to 4'	9	9	7	15	13	8	5	6	9
4' to 5'	13	12	9	20	16	11	9	8	13
5' to 6'	19	17	14	25	21	15	11	12	17
6' to 7'	21	23	19	30	26	20	13	16	19
7' to 8'	x	29	x	x	31	x	x	21	x
8' to 9'	x	37	x	x	37	x	x	26	x
9' to 10'	x	45	x	x	45	x	x	32	x
10' to 11'	x	x	x	x	x	x	x	x	x
11' to 12'	x	x	x	x	x	x	x	x	x
Refusal	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**
Embedment Depth, ft.	**7**	**10**	**7**	**7**	**10**	**7**	**7**	**10**	**7**
Total Drive Time, sec.	**21**	**45**	**19**	**30**	**45**	**20**	**13**	**32**	**19**
Average, sec./ft.	**3.0**	**4.5**	**2.7**	**4.3**	**4.5**	**2.9**	**1.9**	**3.2**	**2.7**

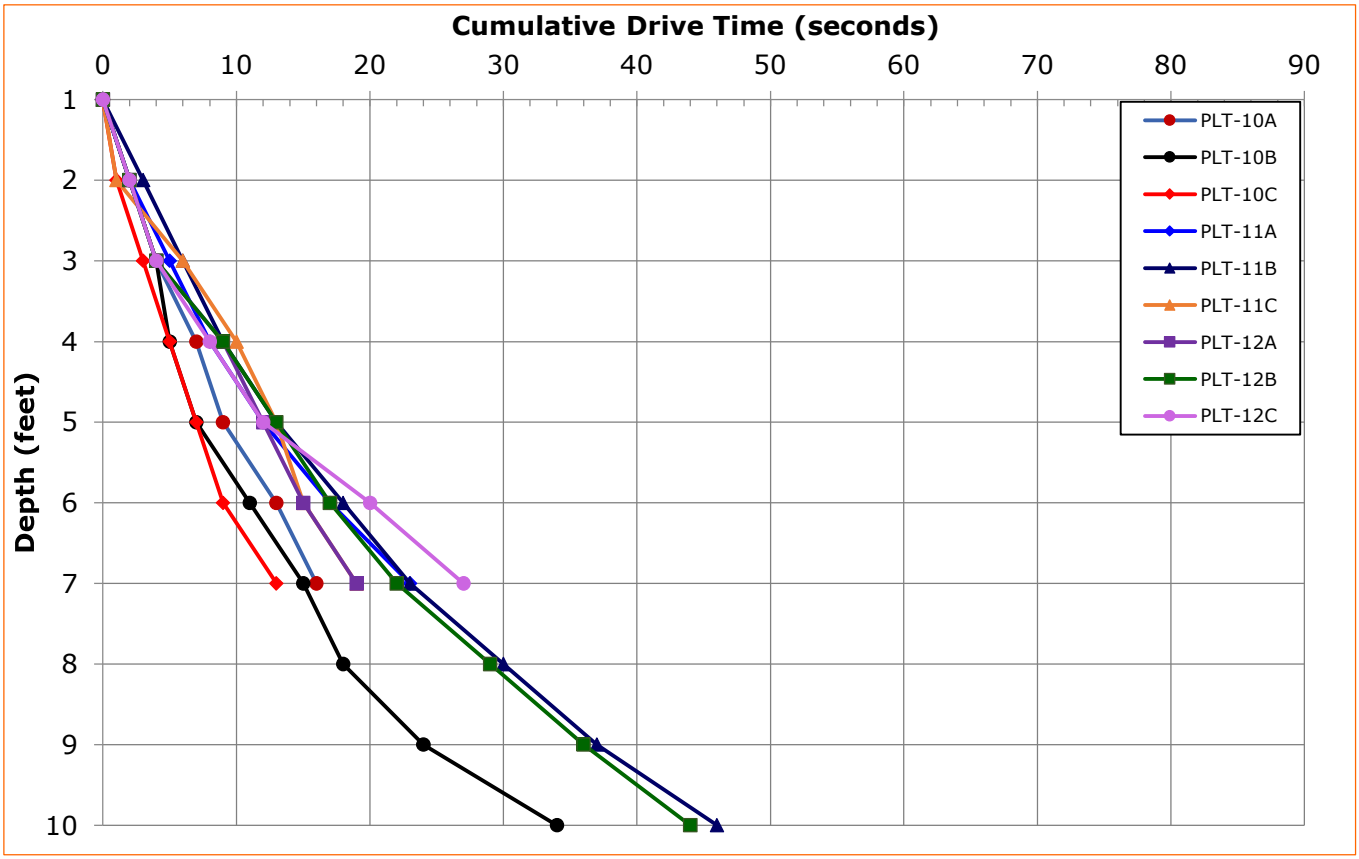
NOTES:

1. Piles advanced with a track mounted Gayk HRE 1000 hydraulic hammer.
2. Pile driving started at the end of pile push depth in inches.
3. Piles were installed between October 2 and October 6, 2023.

TEST PILE DRIVING RECORDS

Grace Solar

Terracon Project No. 60225164



Depth Range, feet	Cumulative Driving Time, seconds (See Note 1)								
	PLT-10A	PLT-10B	PLT-10C	PLT-11A	PLT-11B	PLT-11C	PLT-12A	PLT-12B	PLT-12C
Pile Push (in.) (See Note 2)	-	-	-	-	-	-	-	-	-
0' to 1'	0	0	0	0	0	0	0	0	0
1' to 2'	2	2	1	2	3	1	2	2	2
2' to 3'	4	4	3	5	6	6	4	4	4
3' to 4'	7	5	5	8	9	10	9	9	8
4' to 5'	9	7	7	12	13	13	12	13	12
5' to 6'	13	11	9	17	18	15	15	17	20
6' to 7'	16	15	13	23	23	19	19	22	27
7' to 8'	x	18	x	x	30	x	x	29	x
8' to 9'	x	24	x	x	37	x	x	36	x
9' to 10'	x	34	x	x	46	x	x	44	x
10' to 11'	x	x	x	x	x	x	x	x	x
11' to 12'	x	x	x	x	x	x	x	x	x
Refusal	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**
Embedment Depth, ft.	**7**	**10**	**7**	**7**	**10**	**7**	**7**	**10**	**7**
Total Drive Time, sec.	**16**	**34**	**13**	**23**	**46**	**19**	**19**	**44**	**27**
Average, sec./ft.	**2.3**	**3.4**	**1.9**	**3.3**	**4.6**	**2.7**	**2.7**	**4.4**	**3.9**

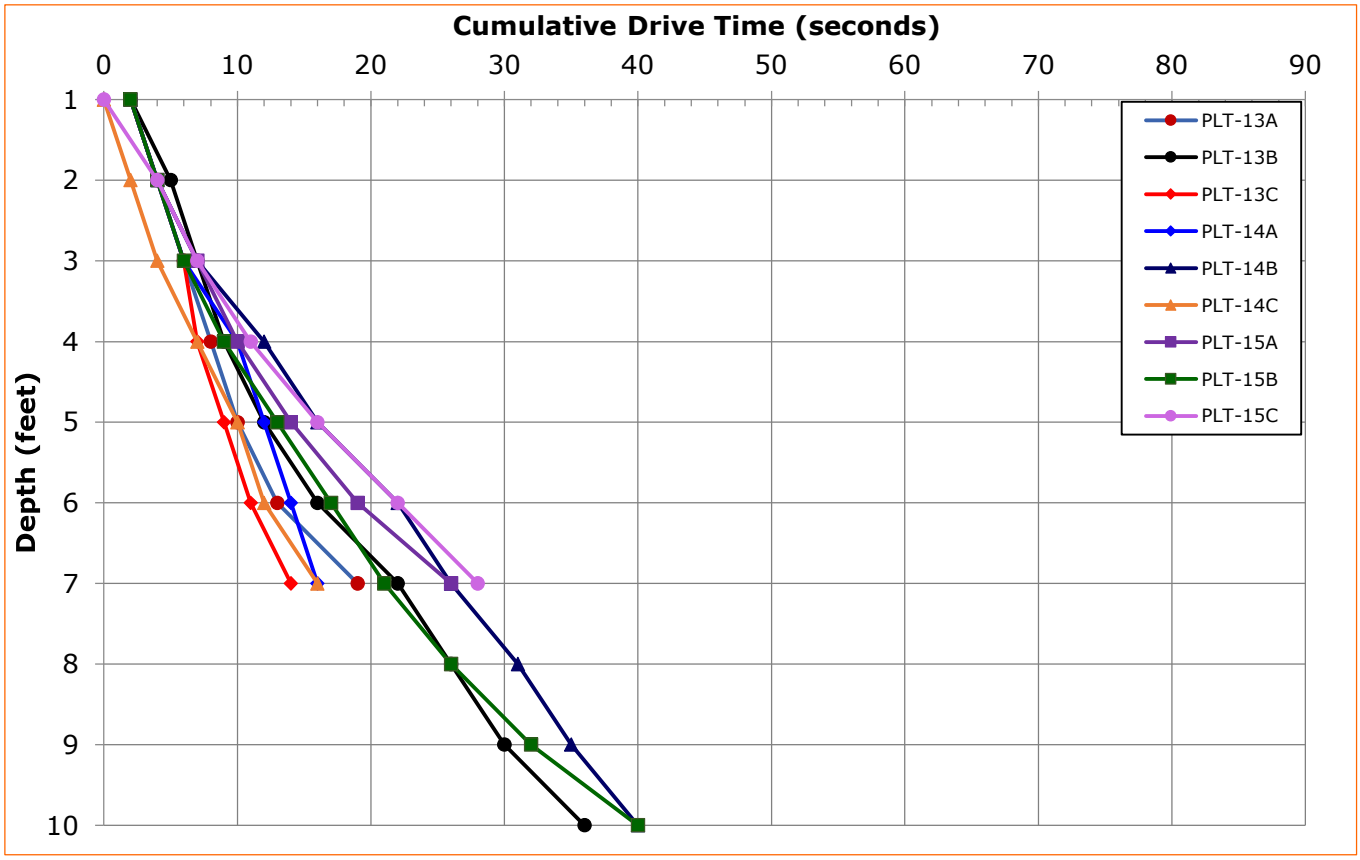
NOTES:

1. Piles advanced with a track mounted Gayk HRE 1000 hydraulic hammer.
2. Pile driving started at the end of pile push depth in inches.
3. Piles were installed between October 2 and October 6, 2023.

TEST PILE DRIVING RECORDS

Grace Solar

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Depth Range, feet	Cumulative Driving Time, seconds (See Note 1)								
	PLT-13A	PLT-13B	PLT-13C	PLT-14A	PLT-14B	PLT-14C	PLT-15A	PLT-15B	PLT-15C
Pile Push (in.) (See Note 2)	-	-	-	-	-	-	-	-	-
0' to 1'	2	2	2	2	2	0	2	2	0
1' to 2'	4	5	4	4	4	2	4	4	4
2' to 3'	6	7	6	6	7	4	7	6	7
3' to 4'	8	9	7	10	12	7	10	9	11
4' to 5'	10	12	9	12	16	10	14	13	16
5' to 6'	13	16	11	14	22	12	19	17	22
6' to 7'	19	22	14	16	26	16	26	21	28
7' to 8'	x	26	x	x	31	x	x	26	x
8' to 9'	x	30	x	x	35	x	x	32	x
9' to 10'	x	36	x	x	40	x	x	40	x
10' to 11'	x	x	x	x	x	x	x	x	x
11' to 12'	x	x	x	x	x	x	x	x	x
Refusal	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**
Embedment Depth, ft.	**7**	**10**	**7**	**7**	**10**	**7**	**7**	**10**	**7**
Total Drive Time, sec.	**19**	**36**	**14**	**16**	**40**	**16**	**26**	**40**	**28**
Average, sec./ft.	**2.7**	**3.6**	**2.0**	**2.3**	**4.0**	**2.3**	**3.7**	**4.0**	**4.0**

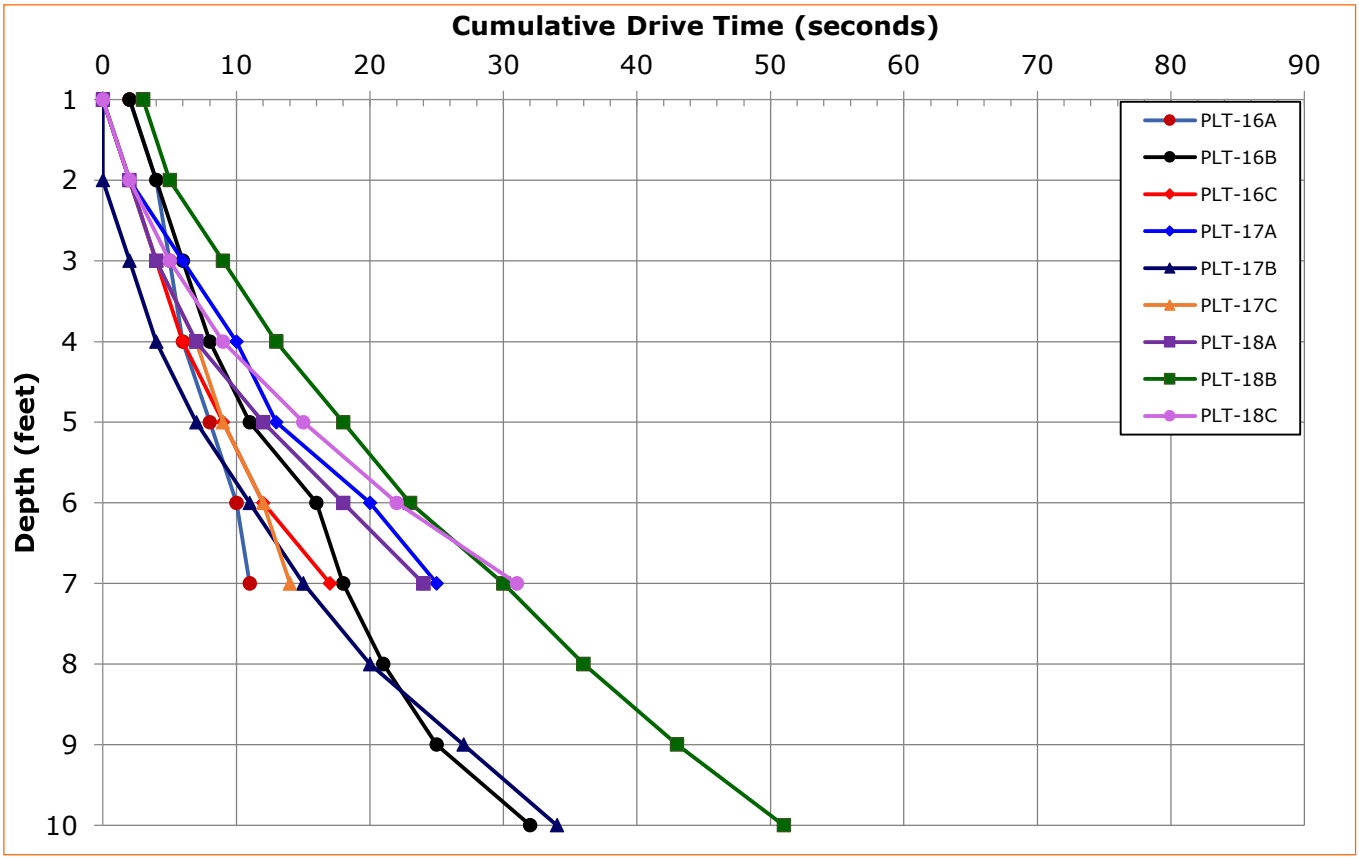
NOTES:

1. Piles advanced with a track mounted Gayk HRE 1000 hydraulic hammer.
2. Pile driving started at the end of pile push depth in inches.
3. Piles were installed between October 2 and October 6, 2023.

TEST PILE DRIVING RECORDS

Grace Solar

Terracon Project No. 60225164



Depth Range, feet	Cumulative Driving Time, seconds (See Note 1)								
	PLT-16A	PLT-16B	PLT-16C	PLT-17A	PLT-17B	PLT-17C	PLT-18A	PLT-18B	PLT-18C
Pile Push (in.) (See Note 2)	-	-	-	-	-	-	-	-	-
0' to 1'	2	2	0	0	0	0	0	3	0
1' to 2'	4	4	2	2	0	2	2	5	2
2' to 3'	5	6	4	6	2	4	4	9	5
3' to 4'	6	8	6	10	4	7	7	13	9
4' to 5'	8	11	9	13	7	9	12	18	15
5' to 6'	10	16	12	20	11	12	18	23	22
6' to 7'	11	18	17	25	15	14	24	30	31
7' to 8'	x	21	x	x	20	x	x	36	x
8' to 9'	x	25	x	x	27	x	x	43	x
9' to 10'	x	32	x	x	34	x	x	51	x
10' to 11'	x	x	x	x	x	x	x	x	x
11' to 12'	x	x	x	x	x	x	x	x	x
Refusal	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**
Embedment Depth, ft.	**7**	**10**	**7**	**7**	**10**	**7**	**7**	**10**	**7**
Total Drive Time, sec.	**11**	**32**	**17**	**25**	**34**	**14**	**24**	**51**	**31**
Average, sec./ft.	**1.6**	**3.2**	**2.4**	**3.6**	**3.4**	**2.0**	**3.4**	**5.1**	**4.4**

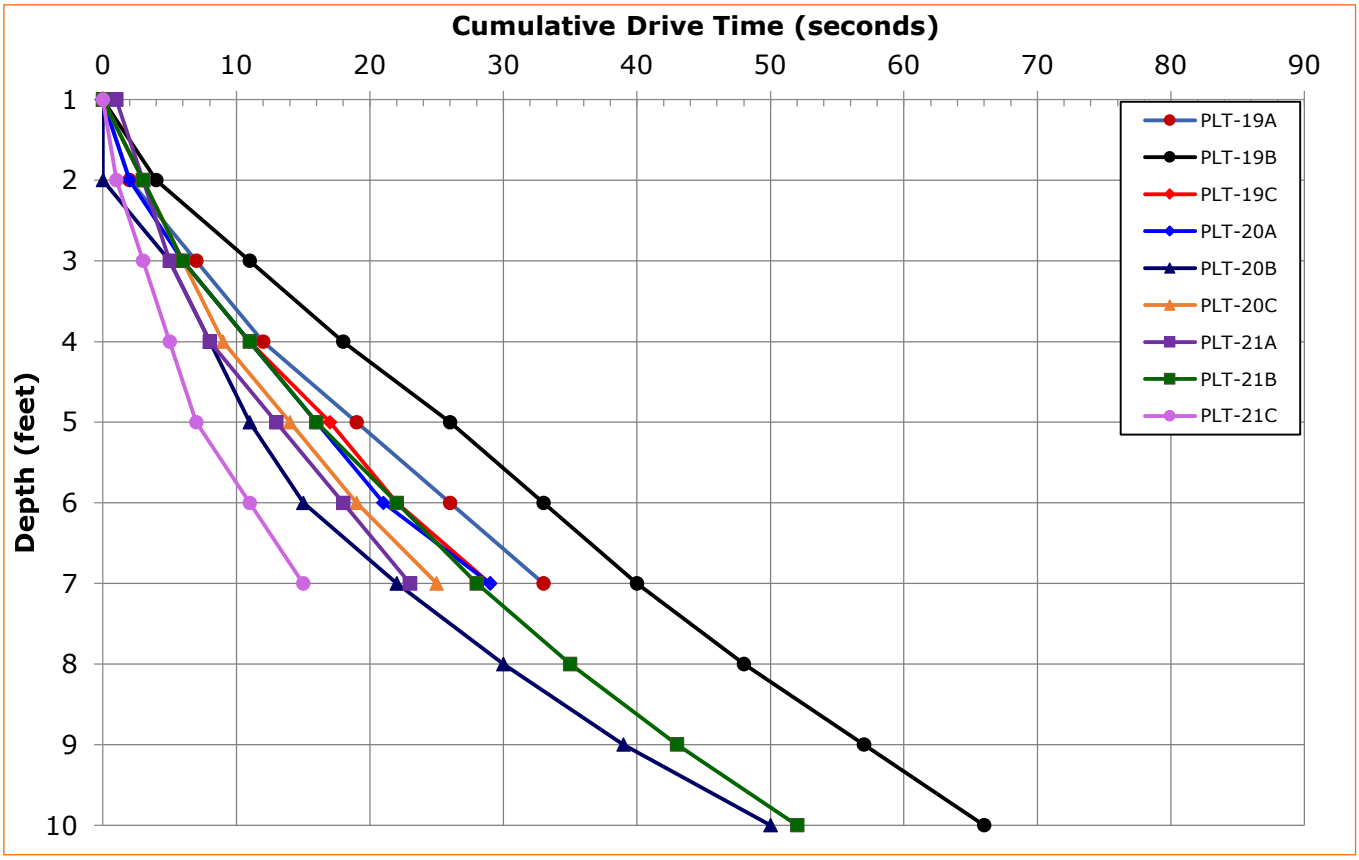
NOTES:

1. Piles advanced with a track mounted Gayk HRE 1000 hydraulic hammer.
2. Pile driving started at the end of pile push depth in inches.
3. Piles were installed between October 2 and October 6, 2023.

TEST PILE DRIVING RECORDS

Grace Solar

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Depth Range, feet	Cumulative Driving Time, seconds (See Note 1)								
	PLT-19A	PLT-19B	PLT-19C	PLT-20A	PLT-20B	PLT-20C	PLT-21A	PLT-21B	PLT-21C
Pile Push (in.) (See Note 2)	-	-	-	-	-	-	-	-	-
0' to 1'	0	0	0	0	0	0	1	0	0
1' to 2'	2	4	3	2	0	3	3	3	1
2' to 3'	7	11	6	6	5	6	5	6	3
3' to 4'	12	18	11	11	8	9	8	11	5
4' to 5'	19	26	17	16	11	14	13	16	7
5' to 6'	26	33	22	21	15	19	18	22	11
6' to 7'	33	40	29	29	22	25	23	28	15
7' to 8'	x	48	x	x	30	x	x	35	x
8' to 9'	x	57	x	x	39	x	x	43	x
9' to 10'	x	66	x	x	50	x	x	52	x
10' to 11'	x	x	x	x	x	x	x	x	x
11' to 12'	x	x	x	x	x	x	x	x	x
Refusal	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**
Embedment Depth, ft.	**7**	**10**	**7**	**7**	**10**	**7**	**7**	**10**	**7**
Total Drive Time, sec.	**33**	**66**	**29**	**29**	**50**	**25**	**23**	**52**	**15**
Average, sec./ft.	**4.7**	**6.6**	**4.1**	**4.1**	**5.0**	**3.6**	**3.3**	**5.2**	**2.1**

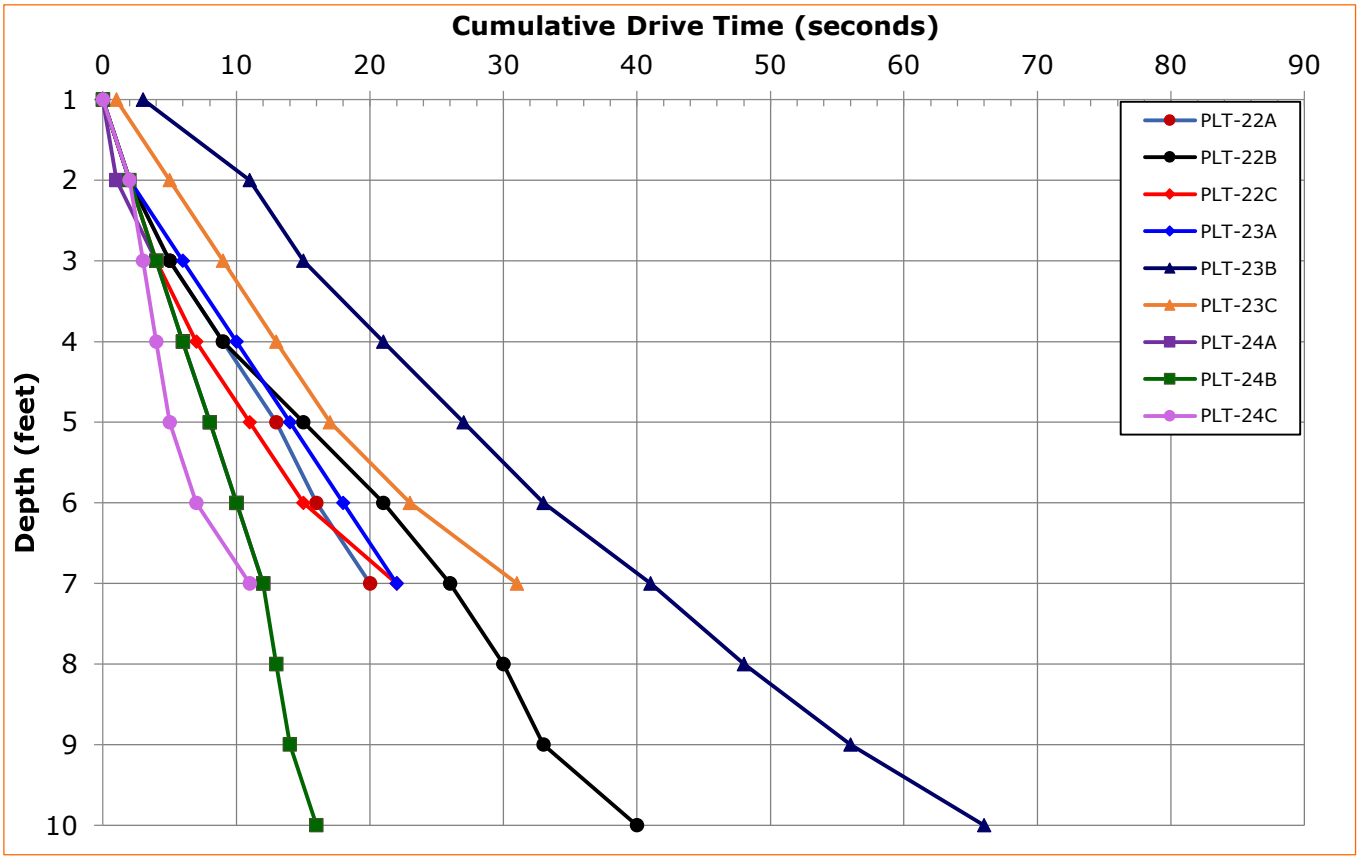
NOTES:

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2. Pile driving started at the end of pile push depth in inches.
3. Piles were installed between October 2 and October 6, 2023.

TEST PILE DRIVING RECORDS

Grace Solar

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Depth Range, feet	Cumulative Driving Time, seconds (See Note 1)								
	PLT-22A	PLT-22B	PLT-22C	PLT-23A	PLT-23B	PLT-23C	PLT-24A	PLT-24B	PLT-24C
Pile Push (in.) (See Note 2)	-	-	-	-	-	-	-	-	-
0' to 1'	0	0	0	0	3	1	0	0	0
1' to 2'	2	2	2	2	11	5	1	2	2
2' to 3'	5	5	4	6	15	9	4	4	3
3' to 4'	9	9	7	10	21	13	6	6	4
4' to 5'	13	15	11	14	27	17	8	8	5
5' to 6'	16	21	15	18	33	23	10	10	7
6' to 7'	20	26	22	22	41	31	12	12	11
7' to 8'	x	30	x	x	48	x	x	13	x
8' to 9'	x	33	x	x	56	x	x	14	x
9' to 10'	x	40	x	x	66	x	x	16	x
10' to 11'	x	x	x	x	x	x	x	x	x
11' to 12'	x	x	x	x	x	x	x	x	x
Refusal	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**
Embedment Depth, ft.	**7**	**10**	**7**	**7**	**10**	**7**	**7**	**10**	**7**
Total Drive Time, sec.	**20**	**40**	**22**	**22**	**66**	**31**	**12**	**16**	**11**
Average, sec./ft.	**2.9**	**4.0**	**3.1**	**3.1**	**6.6**	**4.4**	**1.7**	**1.6**	**1.6**

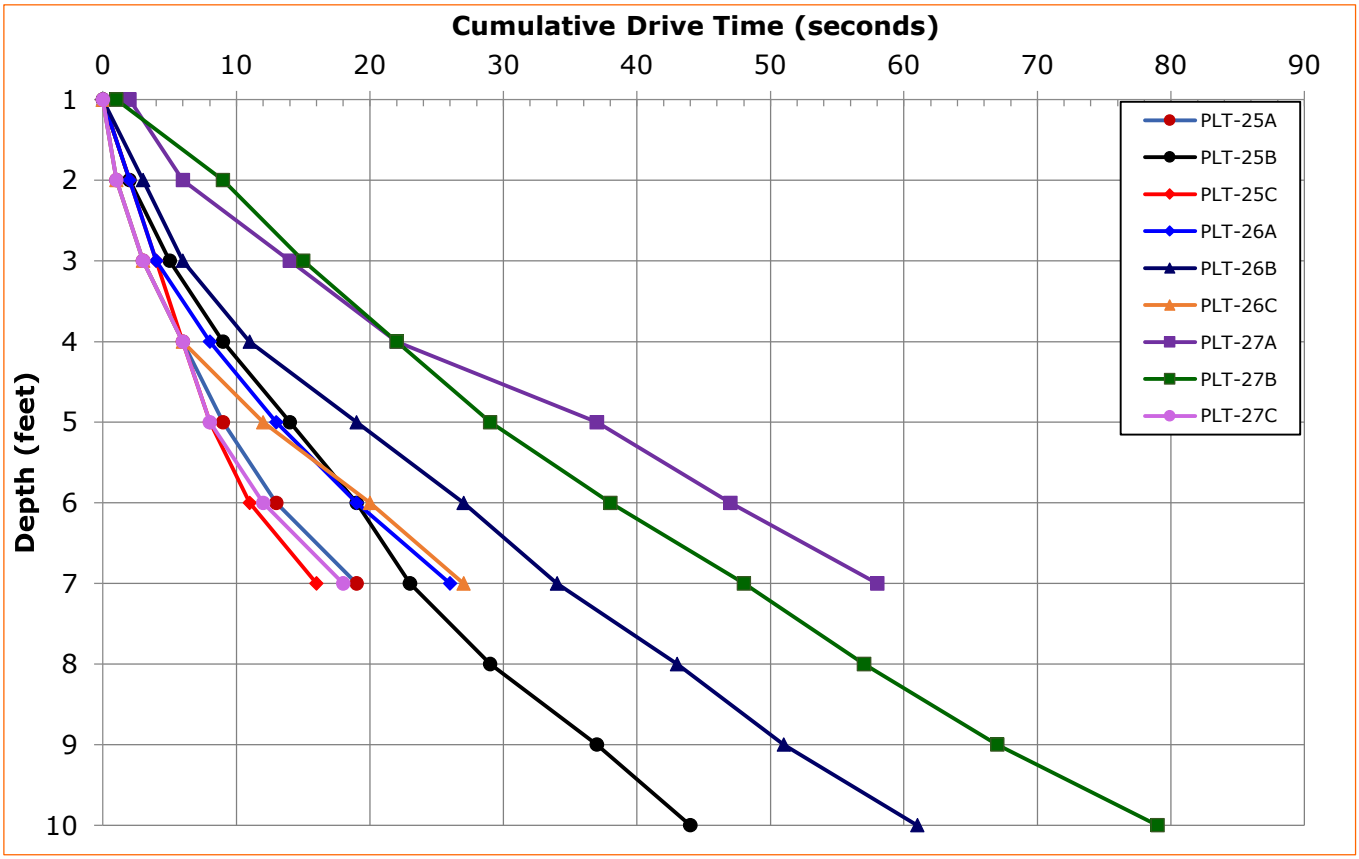
NOTES:

1. Piles advanced with a track mounted Gayk HRE 1000 hydraulic hammer.
2. Pile driving started at the end of pile push depth in inches.
3. Piles were installed between October 2 and October 6, 2023.

TEST PILE DRIVING RECORDS

Grace Solar

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Depth Range, feet	Cumulative Driving Time, seconds (See Note 1)								
	PLT-25A	PLT-25B	PLT-25C	PLT-26A	PLT-26B	PLT-26C	PLT-27A	PLT-27B	PLT-27C
Pile Push (in.) (See Note 2)	-	-	-	-	-	-	-	-	-
0' to 1'	0	0	0	0	0	0	2	1	0
1' to 2'	1	2	2	2	3	1	6	9	1
2' to 3'	3	5	4	4	6	3	14	15	3
3' to 4'	6	9	6	8	11	6	22	22	6
4' to 5'	9	14	8	13	19	12	37	29	8
5' to 6'	13	19	11	19	27	20	47	38	12
6' to 7'	19	23	16	26	34	27	58	48	18
7' to 8'	x	29	x	x	43	x	x	57	x
8' to 9'	x	37	x	x	51	x	x	67	x
9' to 10'	x	44	x	x	61	x	x	79	x
10' to 11'	x	x	x	x	x	x	x	x	x
11' to 12'	x	x	x	x	x	x	x	x	x
Refusal	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**
Embedment Depth, ft.	**7**	**10**	**7**	**7**	**10**	**7**	**7**	**10**	**7**
Total Drive Time, sec.	**19**	**44**	**16**	**26**	**61**	**27**	**58**	**79**	**18**
Average, sec./ft.	**2.7**	**4.4**	**2.3**	**3.7**	**6.1**	**3.9**	**8.3**	**7.9**	**2.6**

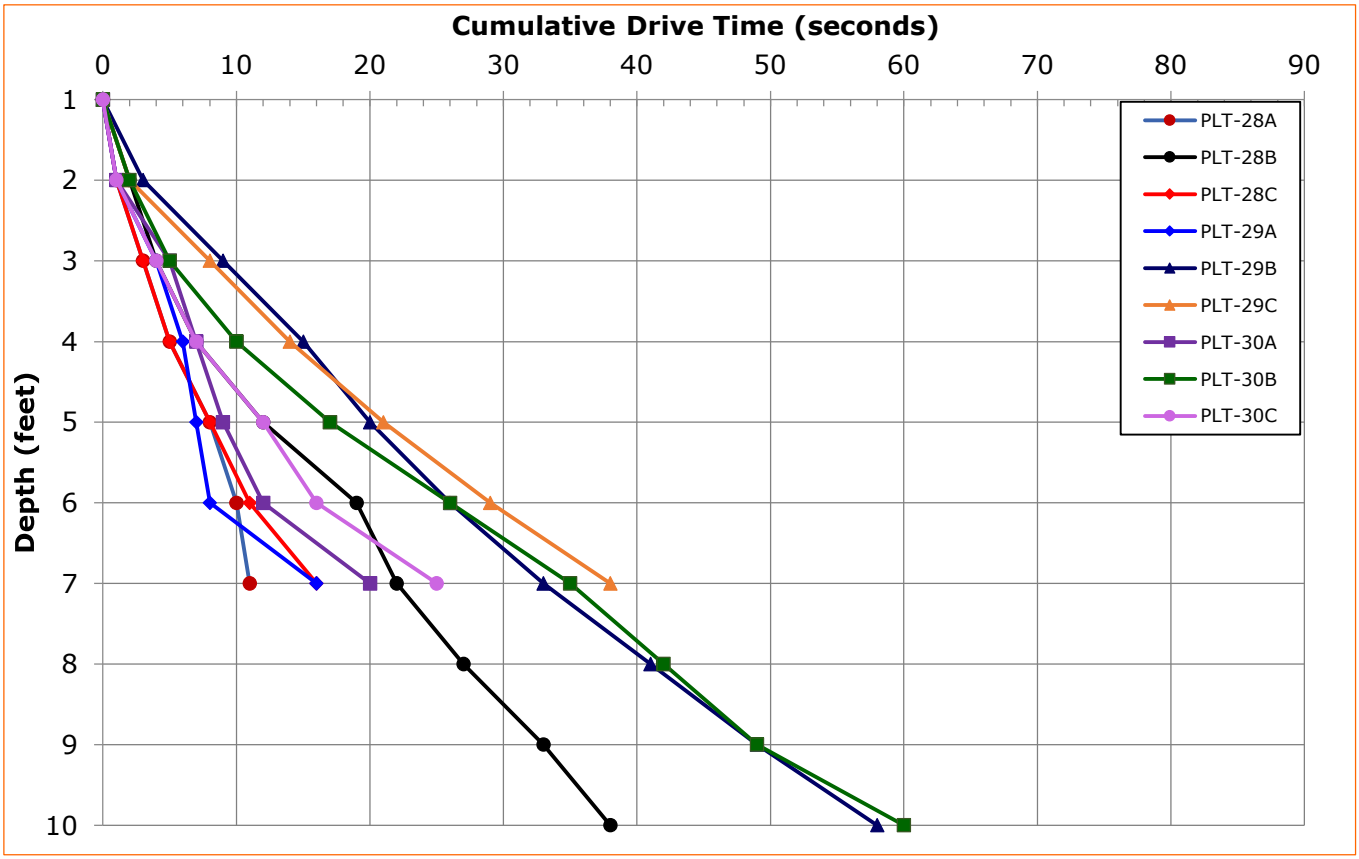
NOTES:

1. Piles advanced with a track mounted Gayk HRE 1000 hydraulic hammer.
2. Pile driving started at the end of pile push depth in inches.
3. Piles were installed between October 2 and October 6, 2023.

TEST PILE DRIVING RECORDS

Grace Solar

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Depth Range, feet	Cumulative Driving Time, seconds (See Note 1)								
	PLT-28A	PLT-28B	PLT-28C	PLT-29A	PLT-29B	PLT-29C	PLT-30A	PLT-30B	PLT-30C
Pile Push (in.) (See Note 2)	-	-	-	-	-	-	-	-	-
0' to 1'	0	0	0	0	0	0	0	0	0
1' to 2'	1	2	1	1	3	2	1	2	1
2' to 3'	3	4	3	4	9	8	5	5	4
3' to 4'	5	7	5	6	15	14	7	10	7
4' to 5'	8	12	8	7	20	21	9	17	12
5' to 6'	10	19	11	8	26	29	12	26	16
6' to 7'	11	22	16	16	33	38	20	35	25
7' to 8'	x	27	x	x	41	x	x	42	x
8' to 9'	x	33	x	x	49	x	x	49	x
9' to 10'	x	38	x	x	58	x	x	60	x
10' to 11'	x	x	x	x	x	x	x	x	x
11' to 12'	x	x	x	x	x	x	x	x	x
Refusal	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**
Embedment Depth, ft.	**7**	**10**	**7**	**7**	**10**	**7**	**7**	**10**	**7**
Total Drive Time, sec.	**11**	**38**	**16**	**16**	**58**	**38**	**20**	**60**	**25**
Average, sec./ft.	**1.6**	**3.8**	**2.3**	**2.3**	**5.8**	**5.4**	**2.9**	**6.0**	**3.6**

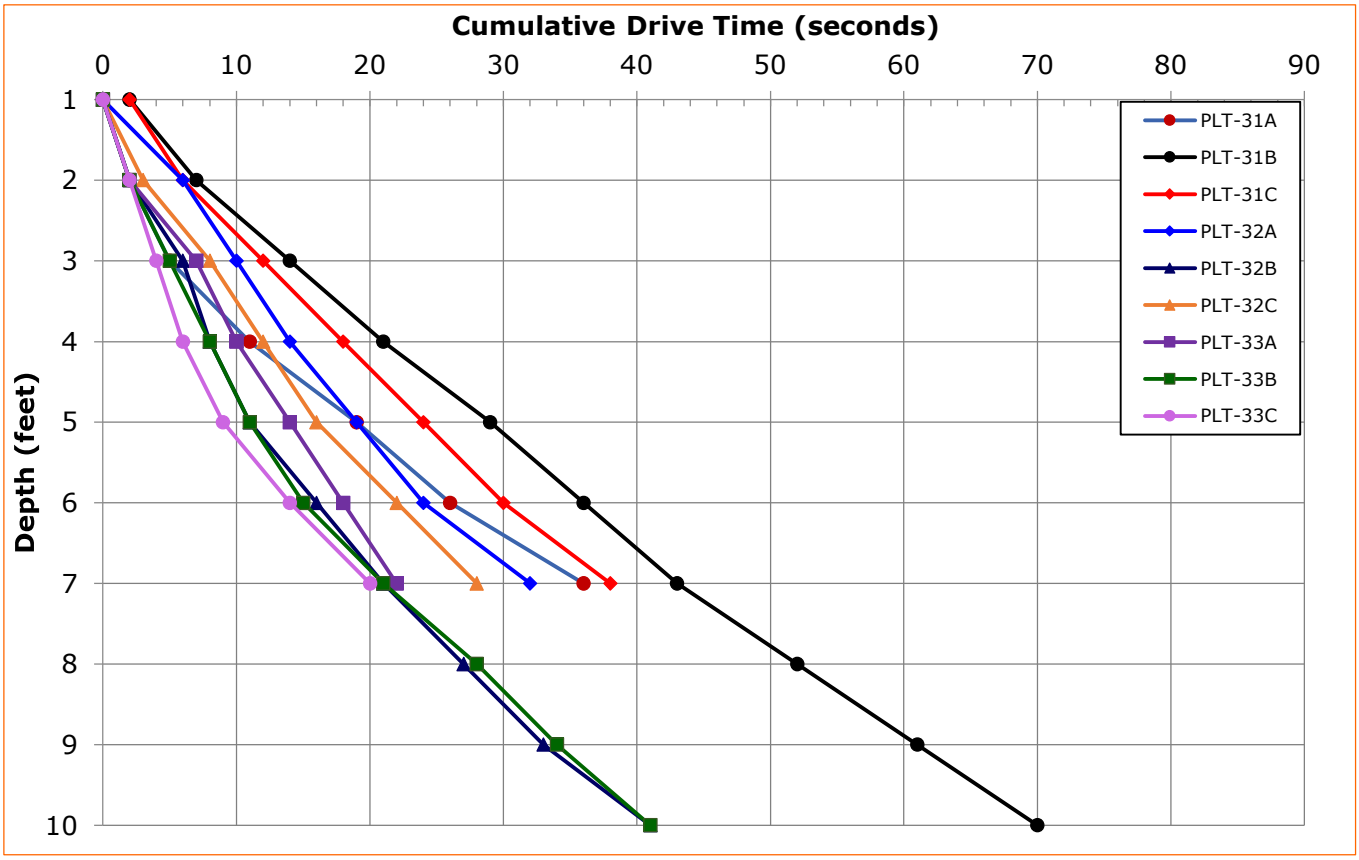
NOTES:

1. Piles advanced with a track mounted Gayk HRE 1000 hydraulic hammer.
2. Pile driving started at the end of pile push depth in inches.
3. Piles were installed between October 2 and October 6, 2023.

TEST PILE DRIVING RECORDS

Grace Solar

Terracon Project No. 60225164



Depth Range, feet	Cumulative Driving Time, seconds (See Note 1)								
	PLT-31A	PLT-31B	PLT-31C	PLT-32A	PLT-32B	PLT-32C	PLT-33A	PLT-33B	PLT-33C
Pile Push (in.) (See Note 2)	-	-	-	-	-	-	-	-	-
0' to 1'	0	2	2	0	0	0	0	0	0
1' to 2'	2	7	6	6	2	3	2	2	2
2' to 3'	5	14	12	10	6	8	7	5	4
3' to 4'	11	21	18	14	8	12	10	8	6
4' to 5'	19	29	24	19	11	16	14	11	9
5' to 6'	26	36	30	24	16	22	18	15	14
6' to 7'	36	43	38	32	21	28	22	21	20
7' to 8'	x	52	x	x	27	x	x	28	x
8' to 9'	x	61	x	x	33	x	x	34	x
9' to 10'	x	70	x	x	41	x	x	41	x
10' to 11'	x	x	x	x	x	x	x	x	x
11' to 12'	x	x	x	x	x	x	x	x	x
Refusal	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**
Embedment Depth, ft.	**7**	**10**	**7**	**7**	**10**	**7**	**7**	**10**	**7**
Total Drive Time, sec.	**36**	**70**	**38**	**32**	**41**	**28**	**22**	**41**	**20**
Average, sec./ft.	**5.1**	**7.0**	**5.4**	**4.6**	**4.1**	**4.0**	**3.1**	**4.1**	**2.9**

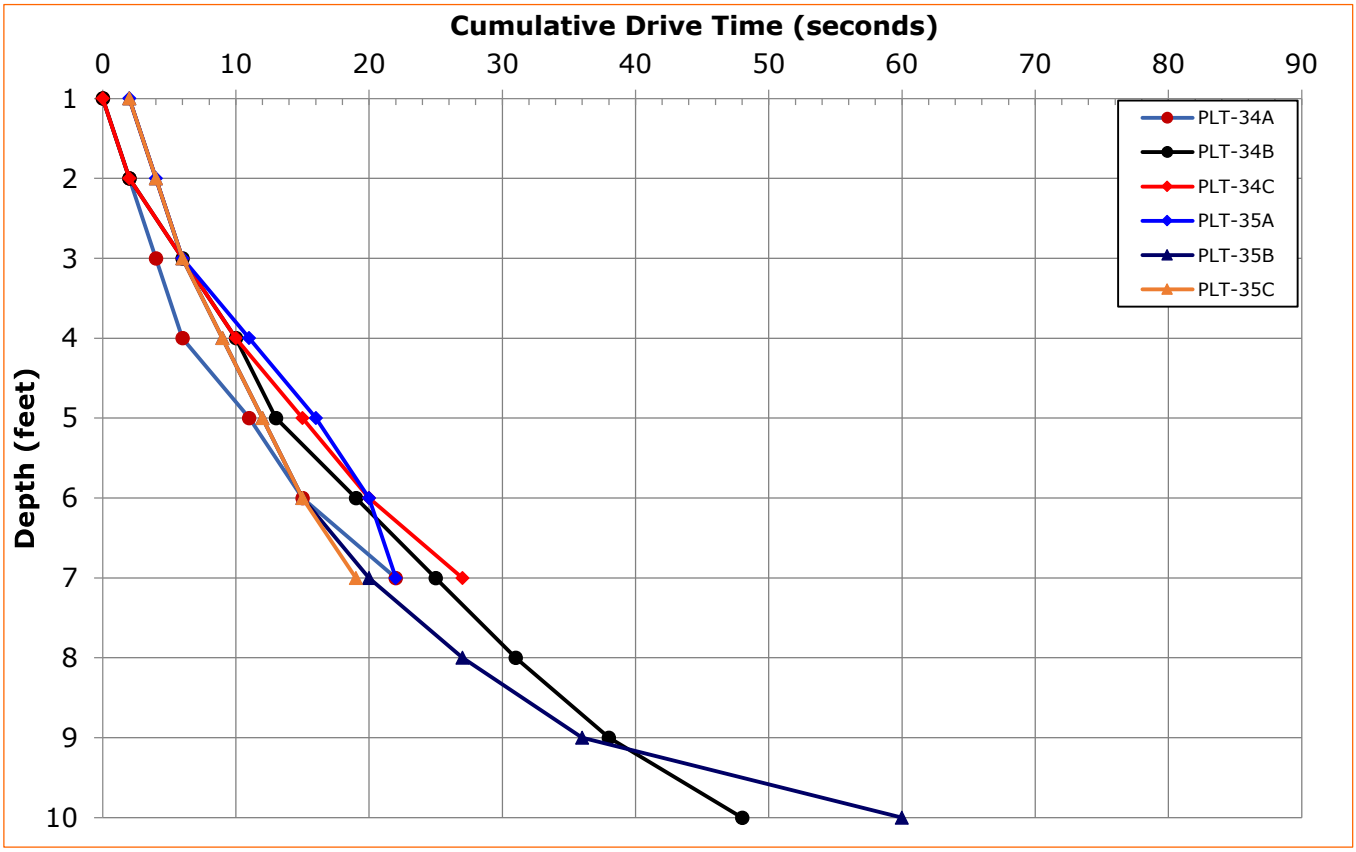
NOTES:

1. Piles advanced with a track mounted Gayk HRE 1000 hydraulic hammer.
2. Pile driving started at the end of pile push depth in inches.
3. Piles were installed between October 2 and October 6, 2023.

TEST PILE DRIVING RECORDS

Grace Solar

Terracon Project No. 60225164



Depth Range, feet	Cumulative Driving Time, seconds (See Note 1)								
	PLT-34A	PLT-34B	PLT-34C	PLT-35A	PLT-35B	PLT-35C			
Pile Push (in.) (See Note 2)	-	-	-	-	-	-			
0' to 1'	0	0	0	2	2	2			
1' to 2'	2	2	2	4	4	4			
2' to 3'	4	6	6	6	6	6			
3' to 4'	6	10	10	11	9	9			
4' to 5'	11	13	15	16	12	12			
5' to 6'	15	19	20	20	15	15			
6' to 7'	22	25	27	22	20	19			
7' to 8'	x	31	x	x	27	x			
8' to 9'	x	38	x	x	36	x			
9' to 10'	x	48	x	x	60	x			
10' to 11'	x	x	x	x	x	x			
11' to 12'	x	x	x	x	x	x			
Refusal	**No**	**No**	**No**	**No**	**No**	**No**			
Embedment Depth, ft.	**7**	**10**	**7**	**7**	**10**	**7**			
Total Drive Time, sec.	**22**	**48**	**27**	**22**	**60**	**19**			
Average, sec./ft.	**3.1**	**4.8**	**3.9**	**3.1**	**6.0**	**2.7**			

NOTES:

1. Piles advanced with a track mounted Gayk HRE 1000 hydraulic hammer.
2. Pile driving started at the end of pile push depth in inches.
3. Piles were installed between October 2 and October 6, 2023.

Pile Load Test Results

Contents:

Load Test Procedure Details (2 Pages)
Axial Tension Test Results (70 Pages)
Lateral Test Results (70 Pages)
Axial Compression Test Results (35 Pages)

Pile Load Testing

The pile load testing was performed in general accordance with ASTM D3689 Test Methods for Deep Foundations under Static Axial Tensile Load, ASTM D3966 Test Methods for Deep Foundations under Lateral Load, and ASTM D 1143, Standard Test Method for Deep Foundations under Static Axial Compressive Load.

Testing Under Axial Tensile ("pull-out") Load

We performed testing under axial tensile load for the piles at each location using the procedures generally outlined below.

Thirty-five piles, two piles at each PLT location, were tested under axial tensile ("pull-out") load. The test piles with the designations "A" and "B" were tested under axial tensile load with the designation "A" being embedded 7 feet below the ground surface, and the designation "B" being embedded 10 feet below the ground surface.

Terracon's proprietary tripod system was used to develop the vertical tension reaction. A locking "E"- plate clamp was used to grip the top of the web for the tension tests. A load cell was used to record the applied load, and deflections were recorded using a pair of calibrated indicators (dial or digital) secured to each flange of the test pile with magnetic mounting brackets. The indicators rested on reference beams supported at least five feet from the pile. Terracon applied loads in approximately 500-lb. increments up to 10,000 lbs. (our equipment's maximum safe working load) or to the target failure criteria (0.75-inch). Terracon recorded deflections after the load was brought back to zero from the target load.

Deflections were recorded using a pair of displacement indicators secured to either side of the test pile flange with a magnetic mounting bracket. The needle of each indicator rested on a reference beam supported at each end with wood and/or masonry blocks at the surface.

Testing Under Lateral Load

After testing under axial tensile load, the piles at each location were then tested under lateral load as described below.

Each pair of test piles was connected using a system of appropriately rated shackles and chain to allow each pile to effectively serve as a reaction for the other. Simultaneous load displacement testing across the strong axis was performed for each pair of piles. The test procedure is generally outlined in the following paragraphs.

A chain system was connected to the test piles using a flange clamp. The load was recorded using a load cell or equivalent and applied with a hydraulic pull cylinder or chain fall in cyclical increments of 500 pounds. The loads were applied in 500-pound increments

and cycled such that five (5) cycles of loading occurred if the maximum lateral load of 7,000 pounds. was obtained during the test. The loads were applied at an approximate height of 24 inches above ground surface (ags). The test was unloaded, and the test ended after the conclusion of the test load schedule or after the pile reached 1-inch of lateral displacement measured at 6 inches above the ground surface. Displacement indicators and reference beams supported at each end with wood blocks was used to measure deflection.

Testing Under Axial Compressive Load

Three piles were tested under axial compressive load. Please note that test piles with the designation "C" were tested under axial compressive load and were embedded 7 feet below the ground surface.

We performed tests under axial compressive loads as generally described below. These procedures were developed with reference to ASTM D1143, *Test Methods for Deep Foundations under Static Axial Compressive Load.*

The load reaction was developed with heavy equipment such as an excavator located an appropriate distance from the test pile. The load was recorded using a pancake-type load cell and applied with a hydraulic cylinder in increments of 500 pounds up to a maximum of 10,000 pounds. Displacement indicators and reference beams consisting of long square tube sections supported at each end with wood blocks were used to measure deflection. The test was unloaded, and the test ended after the conclusion of the test load schedule or after the pile reached ¾-inch of axial displacement. Terracon recorded deflections after the load was brought back to zero from the target load.

Axial Tension Test Results

Tension Load Test Result for PLT-1A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

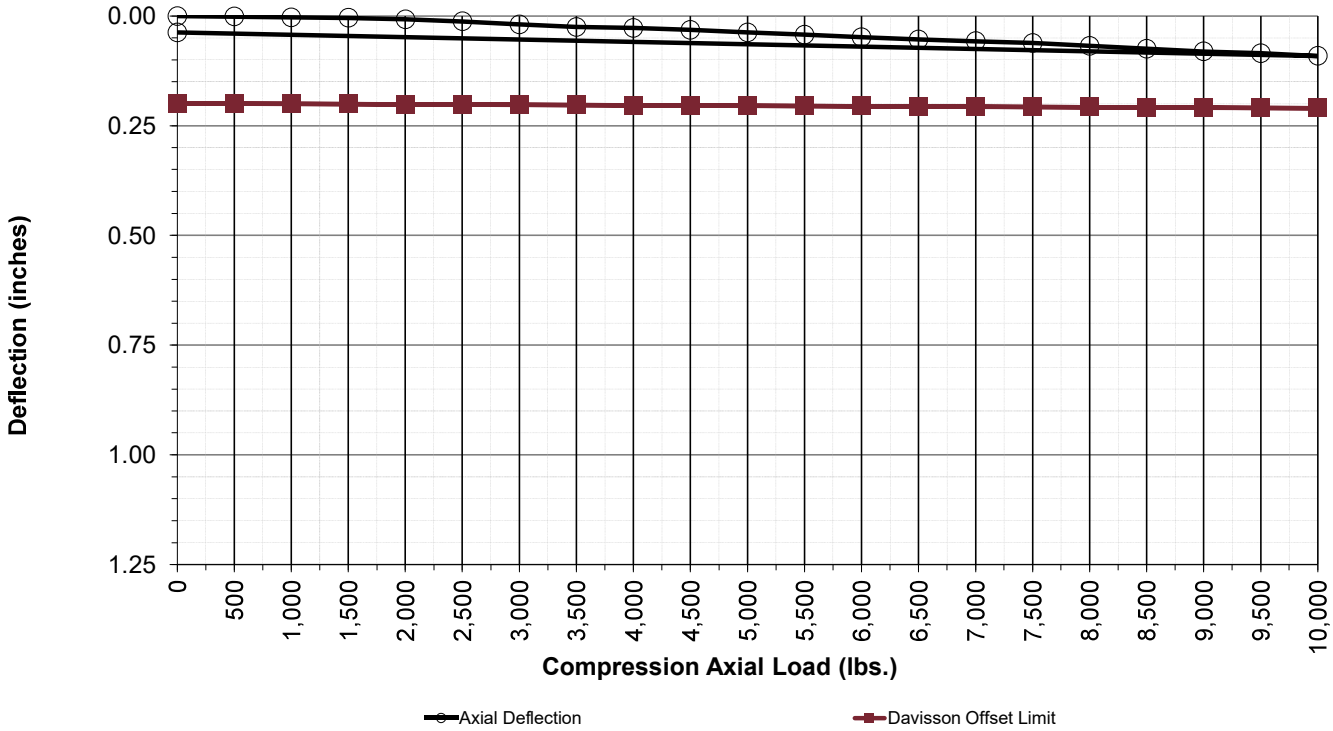
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-1A
Group Latitude [deg.]:	33.6672
Group Longitude[deg.]:	-114.6995
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	32

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.004	0.001	0.200	
15%	1500	0.005	0.002	0.201	
20%	2000	0.008	0.002	0.201	
25%	2500	0.012	0.003	0.202	
30%	3000	0.020	0.003	0.202	
35%	3500	0.025	0.004	0.203	
40%	4000	0.027	0.004	0.203	
45%	4500	0.032	0.005	0.204	
50%	5000	0.038	0.005	0.205	
55%	5500	0.042	0.006	0.205	
60%	6000	0.049	0.006	0.206	
65%	6500	0.054	0.007	0.206	
70%	7000	0.058	0.008	0.207	
75%	7500	0.062	0.008	0.207	
80%	8000	0.068	0.009	0.208	
85%	8500	0.075	0.009	0.208	
90%	9000	0.081	0.010	0.209	
95%	9500	0.085	0.010	0.209	
100%	10000	0.091	0.011	0.210	
0%	0	0.038	0.000	0.199	



Tension Load Test Result for PLT-1B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

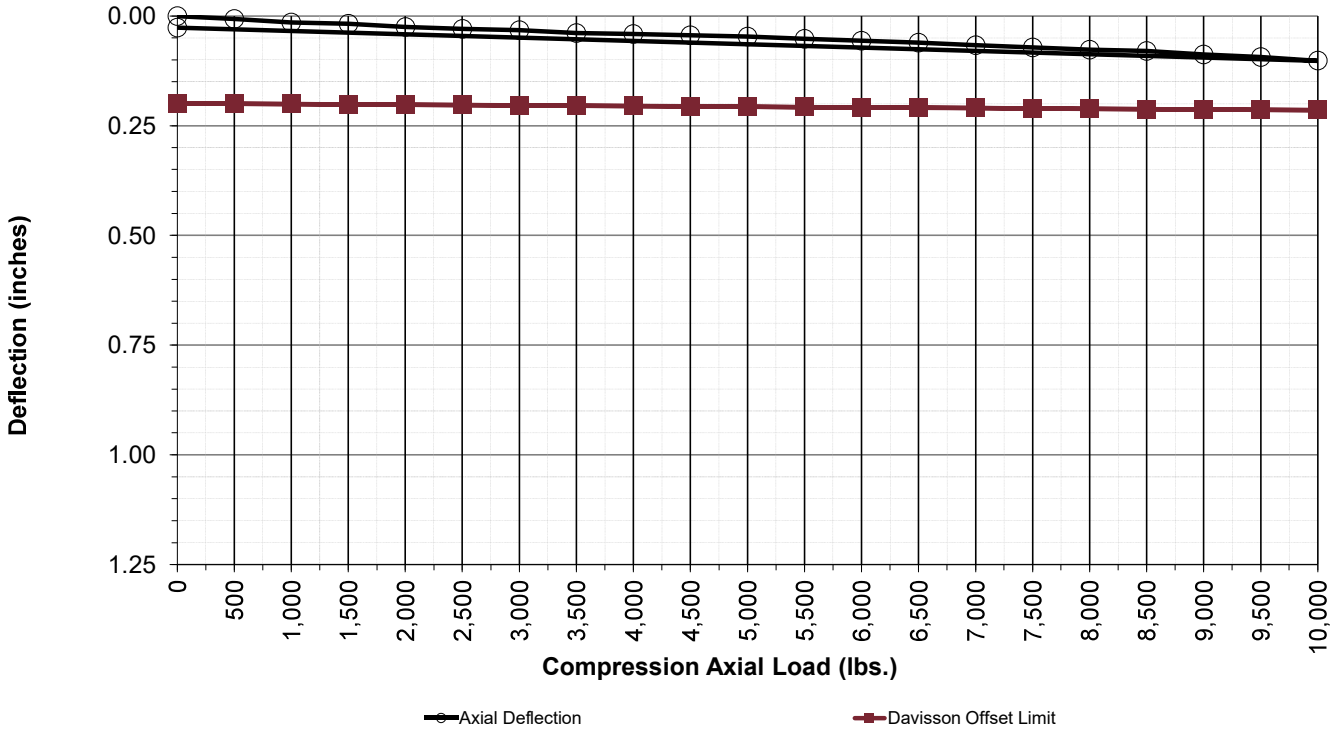
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-1B
Group Latitude [deg.]:	33.6672
Group Longitude[deg.]:	-114.6995
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	52

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.007	0.001	0.200	
10%	1000	0.015	0.002	0.201	
15%	1500	0.018	0.002	0.201	
20%	2000	0.025	0.003	0.202	
25%	2500	0.030	0.004	0.203	
30%	3000	0.033	0.005	0.204	
35%	3500	0.039	0.005	0.205	
40%	4000	0.041	0.006	0.205	
45%	4500	0.044	0.007	0.206	
50%	5000	0.047	0.008	0.207	
55%	5500	0.052	0.008	0.208	
60%	6000	0.056	0.009	0.208	
65%	6500	0.061	0.010	0.209	
70%	7000	0.067	0.011	0.210	
75%	7500	0.072	0.012	0.211	
80%	8000	0.077	0.012	0.212	
85%	8500	0.080	0.013	0.212	
90%	9000	0.088	0.014	0.213	
95%	9500	0.094	0.015	0.214	
100%	10000	0.102	0.015	0.215	
0%	0	0.027	0.000	0.199	



Tension Load Test Result for PLT-2A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

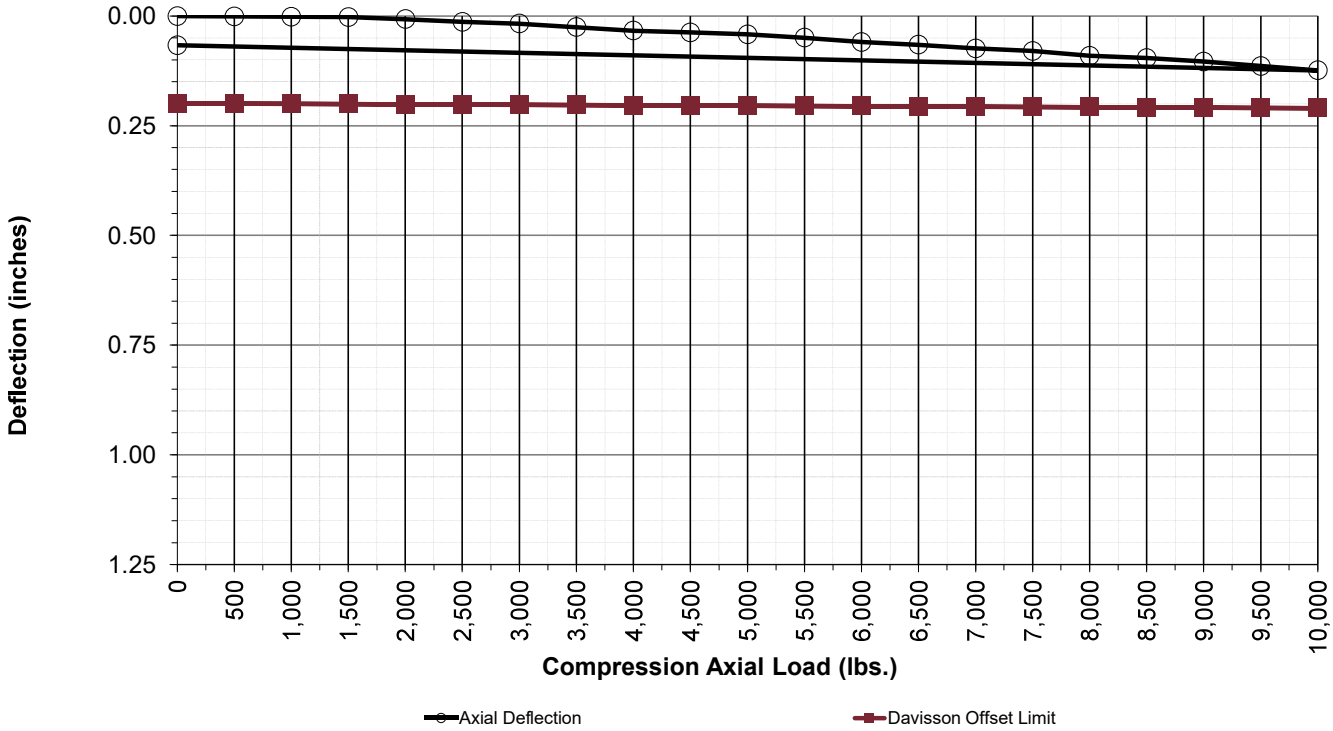
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-2A
Group Latitude [deg.]:	33.6672
Group Longitude[deg.]:	-114.6924
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	24

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.002	0.001	0.200	
15%	1500	0.003	0.002	0.201	
20%	2000	0.008	0.002	0.201	
25%	2500	0.013	0.003	0.202	
30%	3000	0.018	0.003	0.202	
35%	3500	0.026	0.004	0.203	
40%	4000	0.034	0.004	0.203	
45%	4500	0.038	0.005	0.204	
50%	5000	0.042	0.005	0.205	
55%	5500	0.050	0.006	0.205	
60%	6000	0.060	0.006	0.206	
65%	6500	0.066	0.007	0.206	
70%	7000	0.074	0.008	0.207	
75%	7500	0.080	0.008	0.207	
80%	8000	0.091	0.009	0.208	
85%	8500	0.096	0.009	0.208	
90%	9000	0.104	0.010	0.209	
95%	9500	0.114	0.010	0.209	
100%	10000	0.124	0.011	0.210	
0%	0	0.067	0.000	0.199	



Tension Load Test Result for PLT-2B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

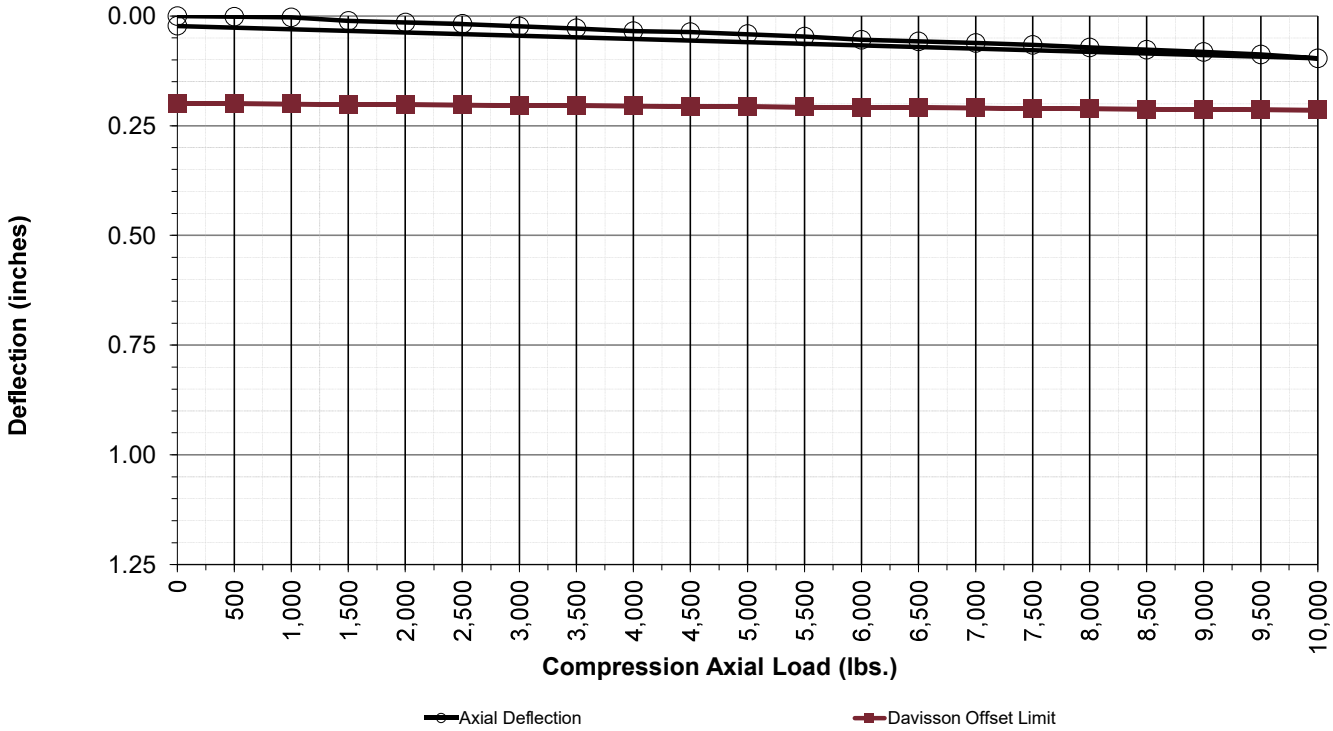
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-2B
Group Latitude [deg.]:	33.6672
Group Longitude[deg.]:	-114.6924
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	42

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.002	0.001	0.200	
10%	1000	0.004	0.002	0.201	
15%	1500	0.011	0.002	0.201	
20%	2000	0.015	0.003	0.202	
25%	2500	0.019	0.004	0.203	
30%	3000	0.024	0.005	0.204	
35%	3500	0.029	0.005	0.205	
40%	4000	0.034	0.006	0.205	
45%	4500	0.037	0.007	0.206	
50%	5000	0.042	0.008	0.207	
55%	5500	0.047	0.008	0.208	
60%	6000	0.054	0.009	0.208	
65%	6500	0.058	0.010	0.209	
70%	7000	0.062	0.011	0.210	
75%	7500	0.066	0.012	0.211	
80%	8000	0.072	0.012	0.212	
85%	8500	0.077	0.013	0.212	
90%	9000	0.082	0.014	0.213	
95%	9500	0.088	0.015	0.214	
100%	10000	0.097	0.015	0.215	
0%	0	0.023	0.000	0.199	



Tension Load Test Result for PLT-3A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

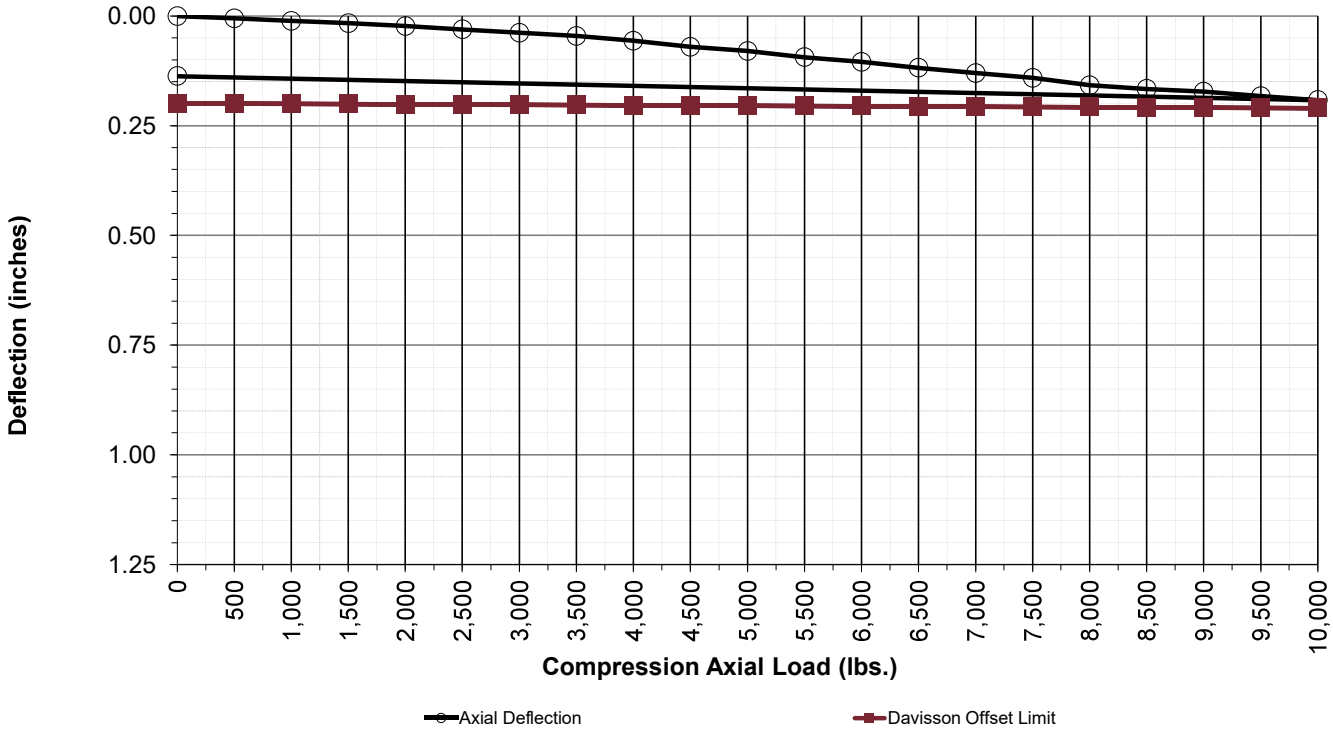
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-3A
Group Latitude [deg.]:	33.6672
Group Longitude[deg.]:	-114.6852
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	17

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.006	0.001	0.200	
10%	1000	0.011	0.001	0.200	
15%	1500	0.016	0.002	0.201	
20%	2000	0.023	0.002	0.201	
25%	2500	0.031	0.003	0.202	
30%	3000	0.038	0.003	0.202	
35%	3500	0.046	0.004	0.203	
40%	4000	0.056	0.004	0.203	
45%	4500	0.071	0.005	0.204	
50%	5000	0.080	0.005	0.205	
55%	5500	0.094	0.006	0.205	
60%	6000	0.105	0.006	0.206	
65%	6500	0.118	0.007	0.206	
70%	7000	0.131	0.008	0.207	
75%	7500	0.141	0.008	0.207	
80%	8000	0.158	0.009	0.208	
85%	8500	0.166	0.009	0.208	
90%	9000	0.173	0.010	0.209	
95%	9500	0.183	0.010	0.209	
100%	10000	0.192	0.011	0.210	
0%	0	0.137	0.000	0.199	



Tension Load Test Result for PLT-3B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

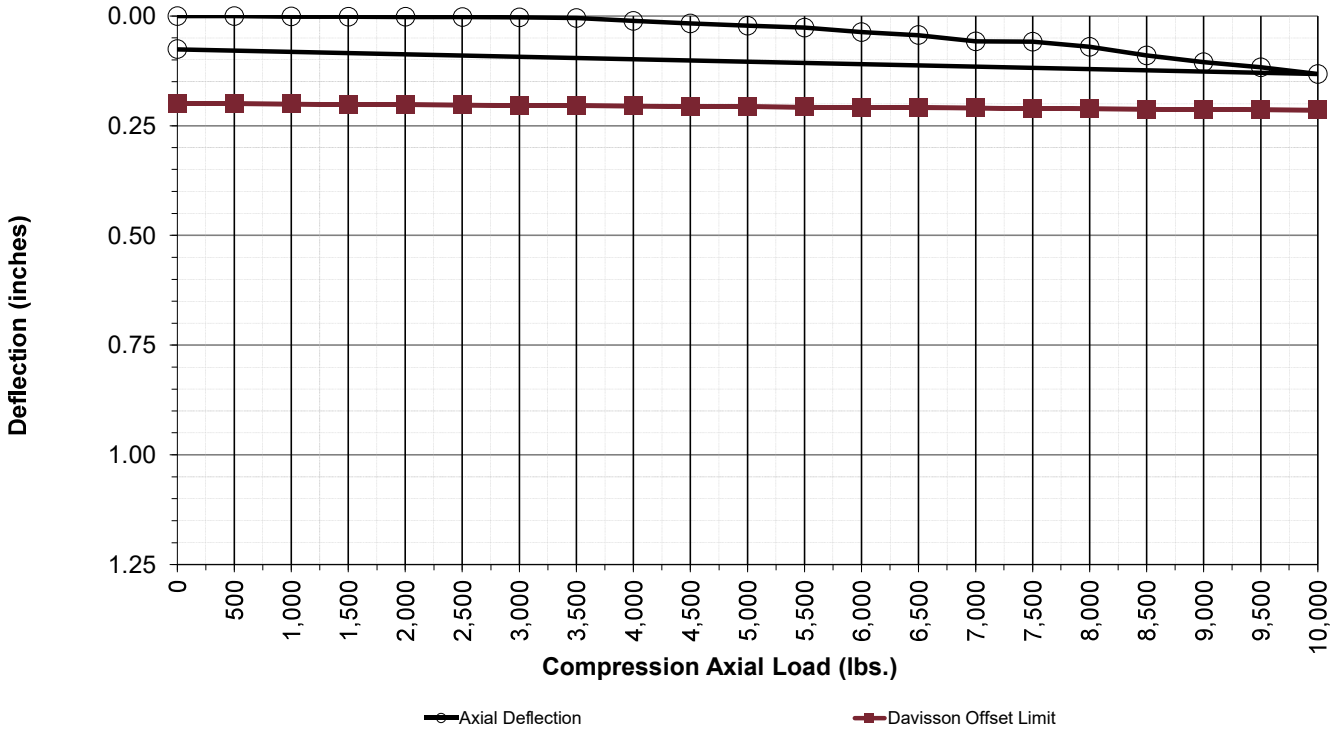
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-3B
Group Latitude [deg.]:	33.6672
Group Longitude[deg.]:	-114.6852
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	44

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.001	0.002	0.201	
15%	1500	0.002	0.002	0.201	
20%	2000	0.002	0.003	0.202	
25%	2500	0.003	0.004	0.203	
30%	3000	0.003	0.005	0.204	
35%	3500	0.005	0.005	0.205	
40%	4000	0.012	0.006	0.205	
45%	4500	0.017	0.007	0.206	
50%	5000	0.022	0.008	0.207	
55%	5500	0.027	0.008	0.208	
60%	6000	0.037	0.009	0.208	
65%	6500	0.044	0.010	0.209	
70%	7000	0.058	0.011	0.210	
75%	7500	0.059	0.012	0.211	
80%	8000	0.070	0.012	0.212	
85%	8500	0.090	0.013	0.212	
90%	9000	0.106	0.014	0.213	
95%	9500	0.117	0.015	0.214	
100%	10000	0.132	0.015	0.215	
0%	0	0.076	0.000	0.199	



Tension Load Test Result for PLT-4A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

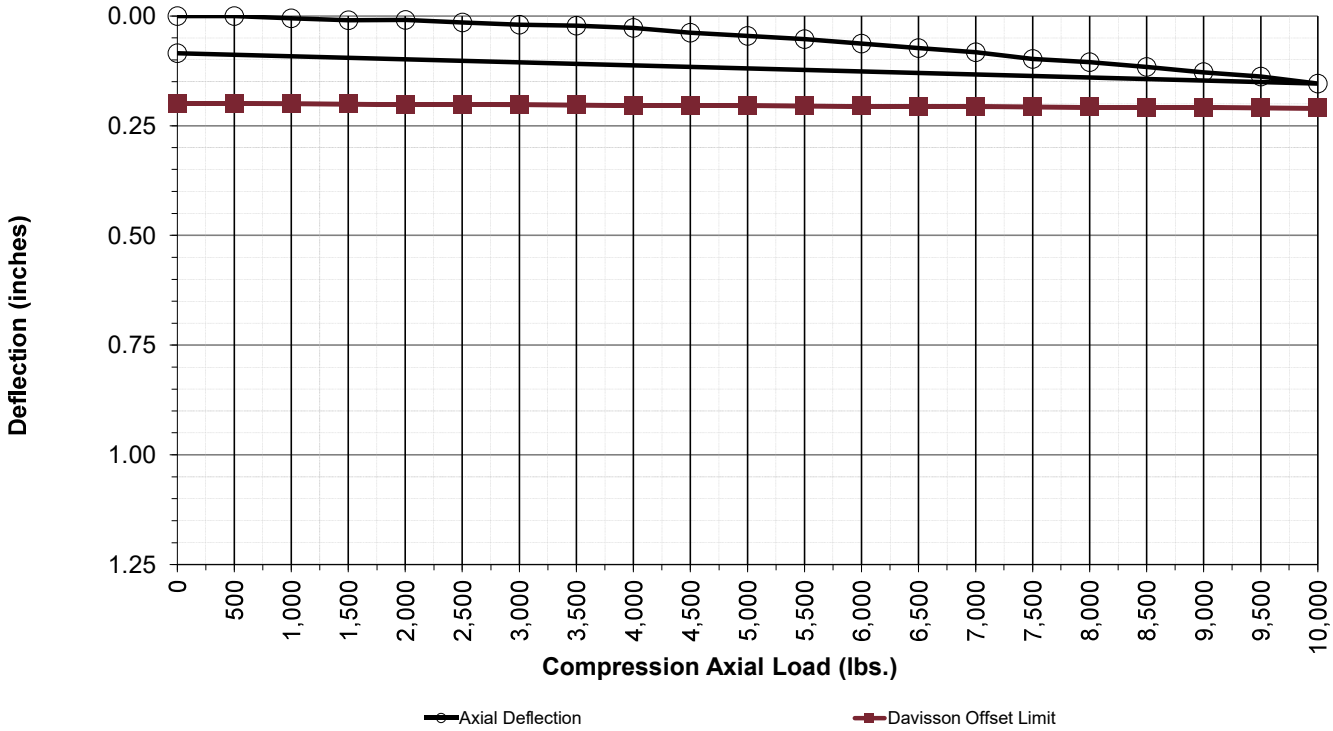
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-4A
Group Latitude [deg.]:	33.6642
Group Longitude[deg.]:	-114.6821
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	20

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.000	0.001	0.200	
10%	1000	0.006	0.001	0.200	
15%	1500	0.010	0.002	0.201	
20%	2000	0.009	0.002	0.201	
25%	2500	0.015	0.003	0.202	
30%	3000	0.020	0.003	0.202	
35%	3500	0.022	0.004	0.203	
40%	4000	0.028	0.004	0.203	
45%	4500	0.038	0.005	0.204	
50%	5000	0.046	0.005	0.205	
55%	5500	0.053	0.006	0.205	
60%	6000	0.063	0.006	0.206	
65%	6500	0.073	0.007	0.206	
70%	7000	0.083	0.008	0.207	
75%	7500	0.098	0.008	0.207	
80%	8000	0.105	0.009	0.208	
85%	8500	0.117	0.009	0.208	
90%	9000	0.128	0.010	0.209	
95%	9500	0.138	0.010	0.209	
100%	10000	0.154	0.011	0.210	
0%	0	0.085	0.000	0.199	



Tension Load Test Result for PLT-4B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

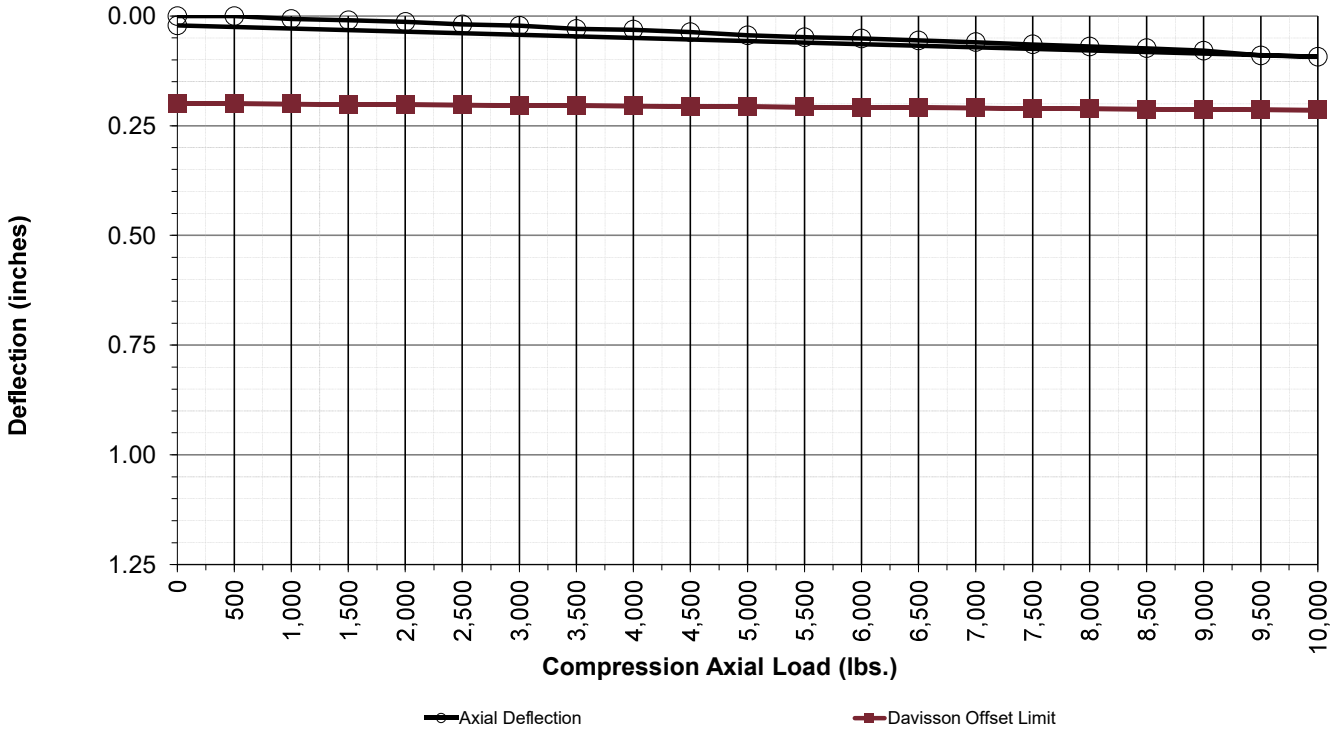
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-4B
Group Latitude [deg.]:	33.6642
Group Longitude[deg.]:	-114.6821
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	46

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.000	0.001	0.200	
10%	1000	0.007	0.002	0.201	
15%	1500	0.010	0.002	0.201	
20%	2000	0.014	0.003	0.202	
25%	2500	0.020	0.004	0.203	
30%	3000	0.022	0.005	0.204	
35%	3500	0.030	0.005	0.205	
40%	4000	0.032	0.006	0.205	
45%	4500	0.037	0.007	0.206	
50%	5000	0.044	0.008	0.207	
55%	5500	0.048	0.008	0.208	
60%	6000	0.051	0.009	0.208	
65%	6500	0.056	0.010	0.209	
70%	7000	0.060	0.011	0.210	
75%	7500	0.065	0.012	0.211	
80%	8000	0.070	0.012	0.212	
85%	8500	0.074	0.013	0.212	
90%	9000	0.079	0.014	0.213	
95%	9500	0.090	0.015	0.214	
100%	10000	0.093	0.015	0.215	
0%	0	0.021	0.000	0.199	



Tension Load Test Result for PLT-5A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

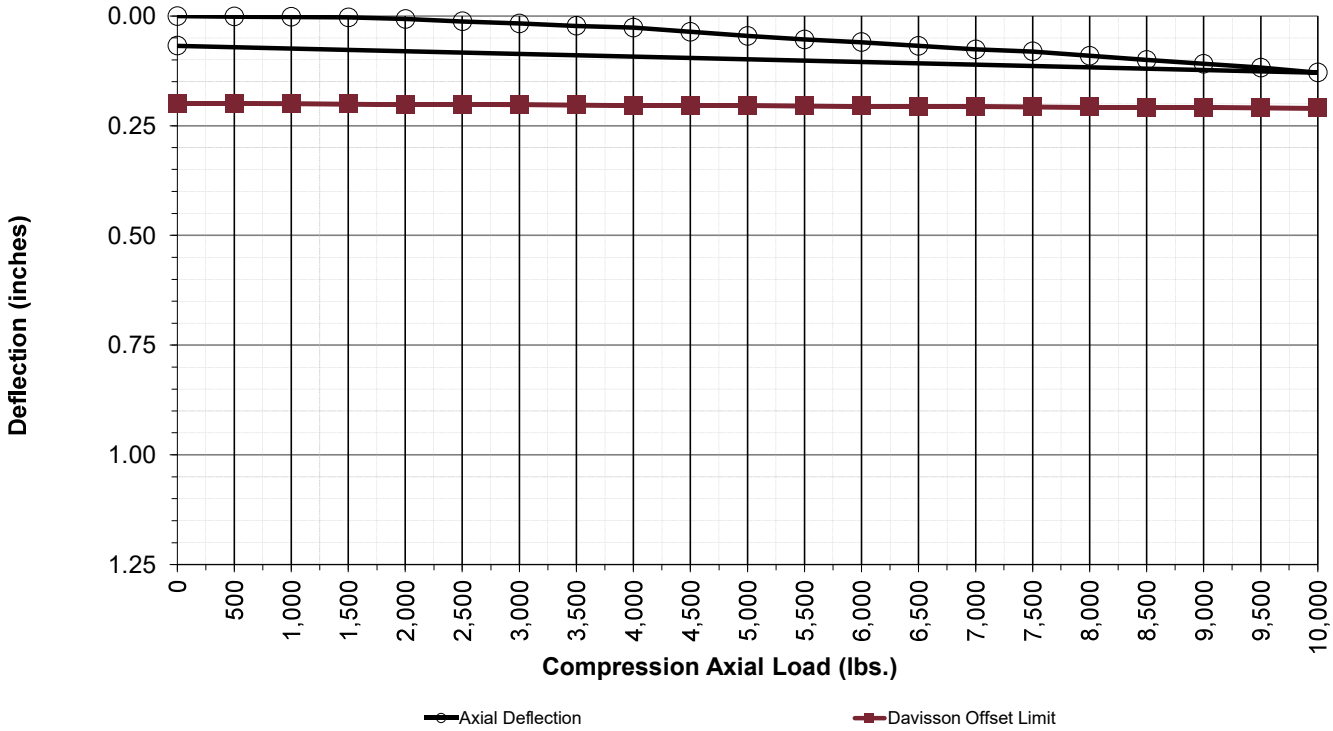
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/27/2023

Pile Information

Pile ID:	PLT-5A
Group Latitude [deg.]:	33.6636
Group Longitude[deg.]:	-114.6960
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	26

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.002	0.001	0.200	
10%	1000	0.003	0.001	0.200	
15%	1500	0.004	0.002	0.201	
20%	2000	0.007	0.002	0.201	
25%	2500	0.013	0.003	0.202	
30%	3000	0.017	0.003	0.202	
35%	3500	0.023	0.004	0.203	
40%	4000	0.026	0.004	0.203	
45%	4500	0.036	0.005	0.204	
50%	5000	0.046	0.005	0.205	
55%	5500	0.054	0.006	0.205	
60%	6000	0.060	0.006	0.206	
65%	6500	0.068	0.007	0.206	
70%	7000	0.076	0.008	0.207	
75%	7500	0.081	0.008	0.207	
80%	8000	0.091	0.009	0.208	
85%	8500	0.100	0.009	0.208	
90%	9000	0.109	0.010	0.209	
95%	9500	0.118	0.010	0.209	
100%	10000	0.129	0.011	0.210	
0%	0	0.068	0.000	0.199	



Tension Load Test Result for PLT-5B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

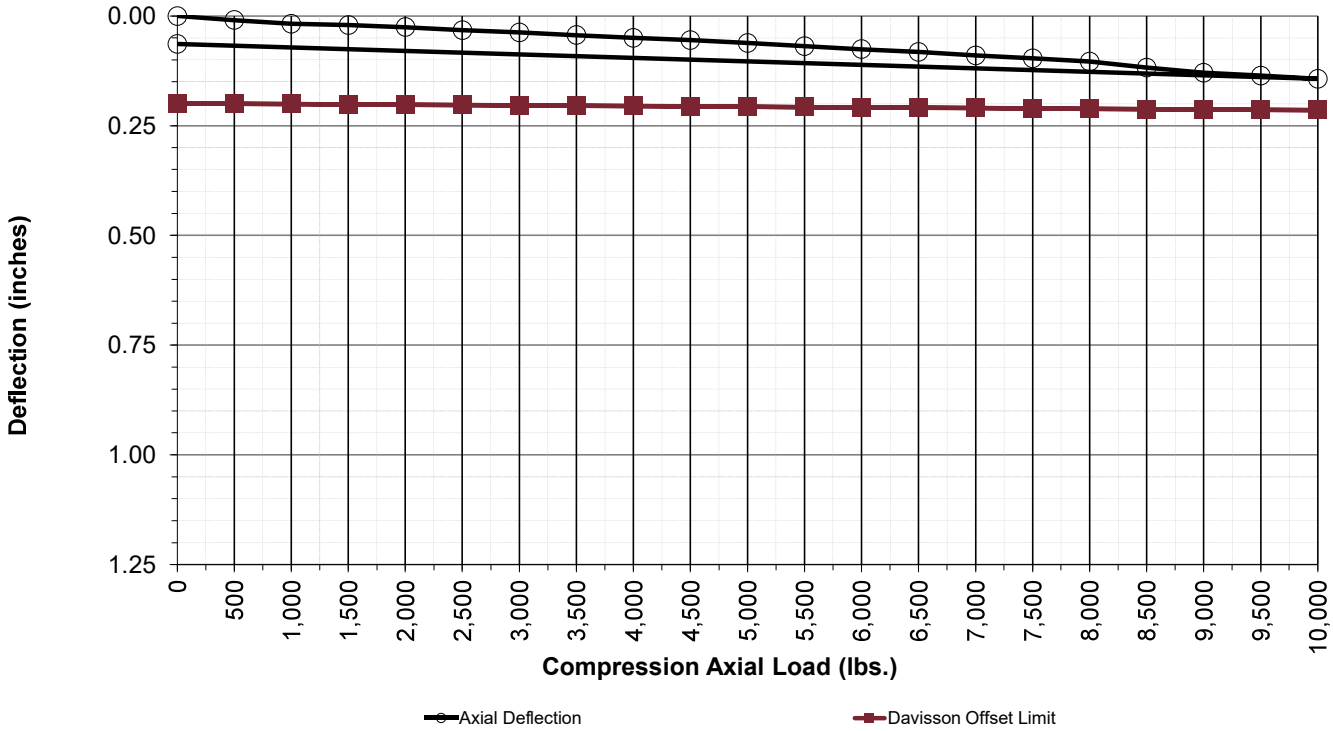
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/27/2023

Pile Information

Pile ID:	PLT-5B
Group Latitude [deg.]:	33.6636
Group Longitude[deg.]:	-114.6960
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	42

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.010	0.001	0.200	
10%	1000	0.018	0.002	0.201	
15%	1500	0.021	0.002	0.201	
20%	2000	0.026	0.003	0.202	
25%	2500	0.032	0.004	0.203	
30%	3000	0.037	0.005	0.204	
35%	3500	0.044	0.005	0.205	
40%	4000	0.050	0.006	0.205	
45%	4500	0.055	0.007	0.206	
50%	5000	0.062	0.008	0.207	
55%	5500	0.069	0.008	0.208	
60%	6000	0.076	0.009	0.208	
65%	6500	0.082	0.010	0.209	
70%	7000	0.090	0.011	0.210	
75%	7500	0.097	0.012	0.211	
80%	8000	0.104	0.012	0.212	
85%	8500	0.118	0.013	0.212	
90%	9000	0.130	0.014	0.213	
95%	9500	0.136	0.015	0.214	
100%	10000	0.143	0.015	0.215	
0%	0	0.064	0.000	0.199	



Tension Load Test Result for PLT-6A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

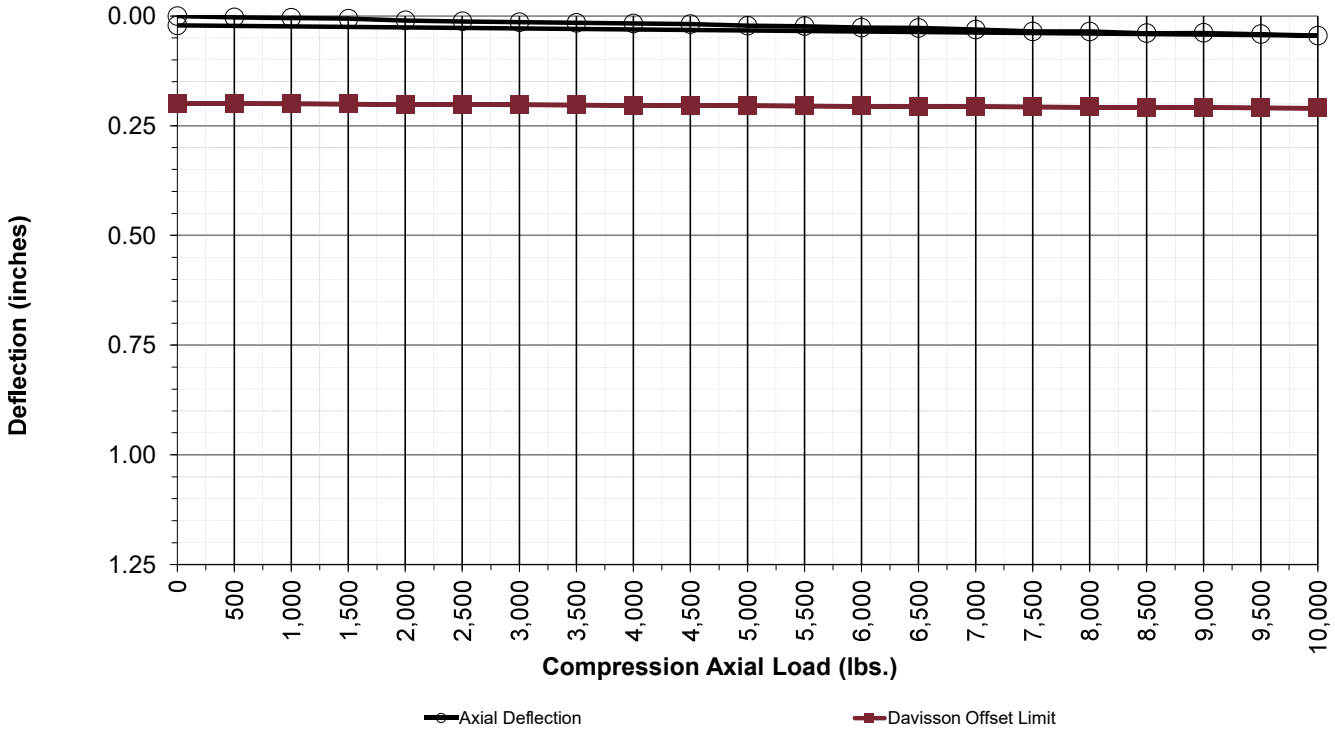
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-6A
Group Latitude [deg.]:	33.6635
Group Longitude[deg.]:	-114.7068
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	20

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.004	0.001	0.200	
10%	1000	0.005	0.001	0.200	
15%	1500	0.006	0.002	0.201	
20%	2000	0.011	0.002	0.201	
25%	2500	0.012	0.003	0.202	
30%	3000	0.014	0.003	0.202	
35%	3500	0.016	0.004	0.203	
40%	4000	0.017	0.004	0.203	
45%	4500	0.019	0.005	0.204	
50%	5000	0.022	0.005	0.205	
55%	5500	0.023	0.006	0.205	
60%	6000	0.027	0.006	0.206	
65%	6500	0.028	0.007	0.206	
70%	7000	0.031	0.008	0.207	
75%	7500	0.035	0.008	0.207	
80%	8000	0.035	0.009	0.208	
85%	8500	0.040	0.009	0.208	
90%	9000	0.039	0.010	0.209	
95%	9500	0.042	0.010	0.209	
100%	10000	0.045	0.011	0.210	
0%	0	0.022	0.000	0.199	



Tension Load Test Result for PLT-6B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

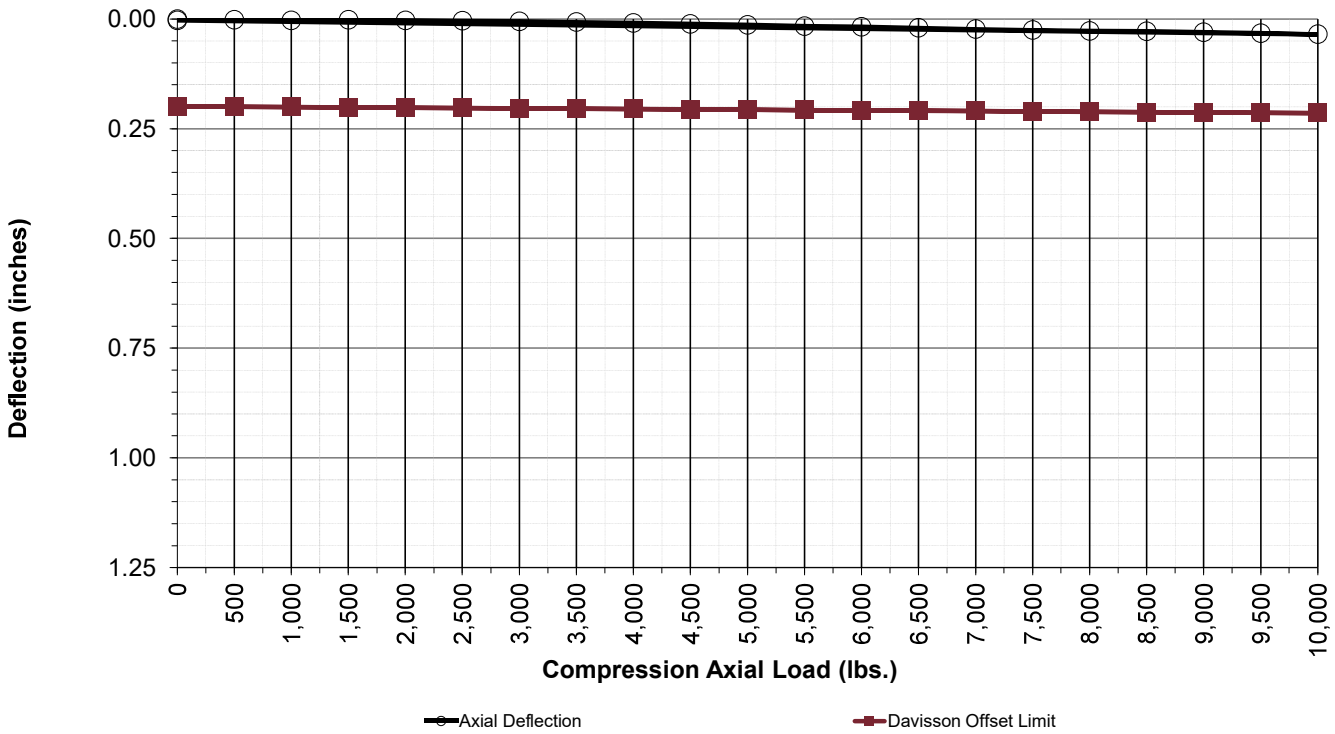
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-6B
Group Latitude [deg.]:	33.6635
Group Longitude[deg.]:	-114.7068
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	74

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.002	0.001	0.200	
10%	1000	0.004	0.002	0.201	
15%	1500	0.002	0.002	0.201	
20%	2000	0.003	0.003	0.202	
25%	2500	0.005	0.004	0.203	
30%	3000	0.006	0.005	0.204	
35%	3500	0.008	0.005	0.205	
40%	4000	0.009	0.006	0.205	
45%	4500	0.012	0.007	0.206	
50%	5000	0.014	0.008	0.207	
55%	5500	0.017	0.008	0.208	
60%	6000	0.018	0.009	0.208	
65%	6500	0.021	0.010	0.209	
70%	7000	0.023	0.011	0.210	
75%	7500	0.026	0.012	0.211	
80%	8000	0.028	0.012	0.212	
85%	8500	0.028	0.013	0.212	
90%	9000	0.030	0.014	0.213	
95%	9500	0.032	0.015	0.214	
100%	10000	0.035	0.015	0.215	
0%	0	0.004	0.000	0.199	



Tension Load Test Result for PLT-7A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

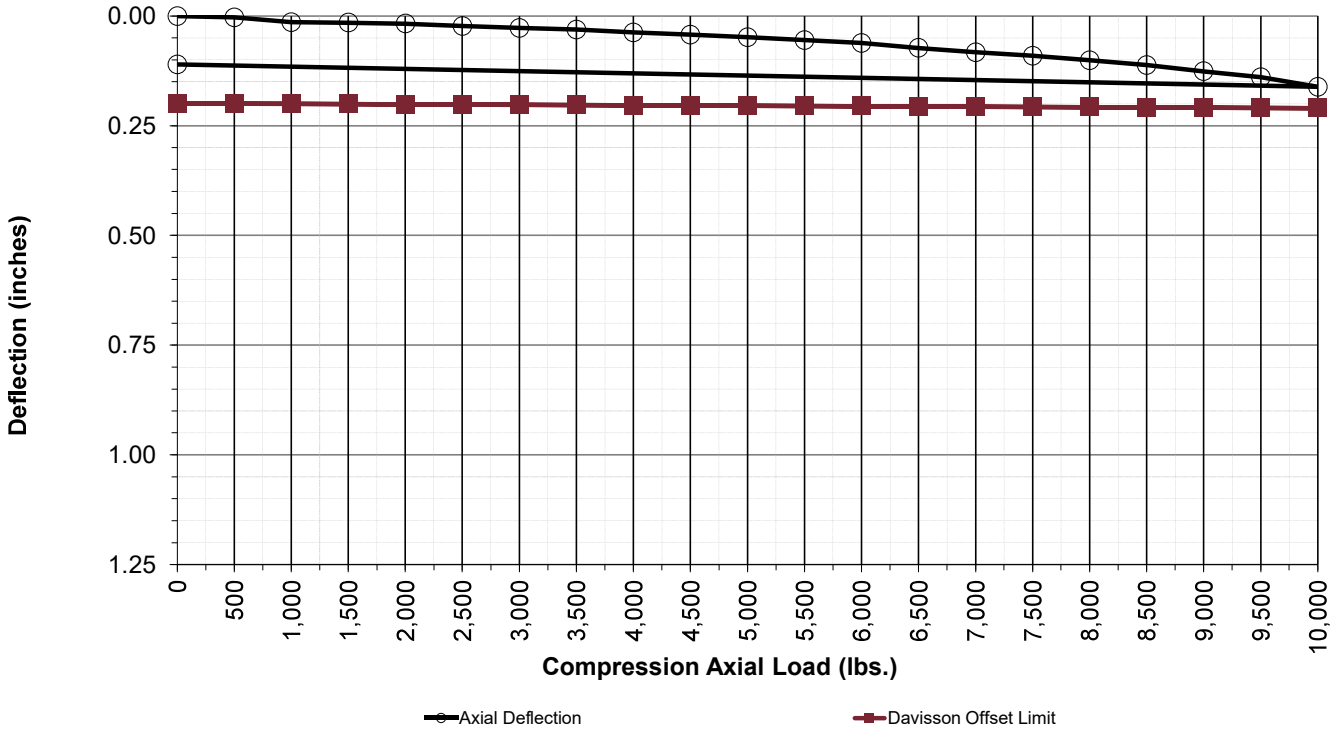
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-7A
Group Latitude [deg.]:	33.6612
Group Longitude[deg.]:	-114.7029
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	21

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.004	0.001	0.200	
10%	1000	0.014	0.001	0.200	
15%	1500	0.016	0.002	0.201	
20%	2000	0.018	0.002	0.201	
25%	2500	0.023	0.003	0.202	
30%	3000	0.027	0.003	0.202	
35%	3500	0.031	0.004	0.203	
40%	4000	0.038	0.004	0.203	
45%	4500	0.043	0.005	0.204	
50%	5000	0.048	0.005	0.205	
55%	5500	0.055	0.006	0.205	
60%	6000	0.062	0.006	0.206	
65%	6500	0.073	0.007	0.206	
70%	7000	0.083	0.008	0.207	
75%	7500	0.091	0.008	0.207	
80%	8000	0.101	0.009	0.208	
85%	8500	0.112	0.009	0.208	
90%	9000	0.126	0.010	0.209	
95%	9500	0.140	0.010	0.209	
100%	10000	0.162	0.011	0.210	
0%	0	0.111	0.000	0.199	



Tension Load Test Result for PLT-7B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

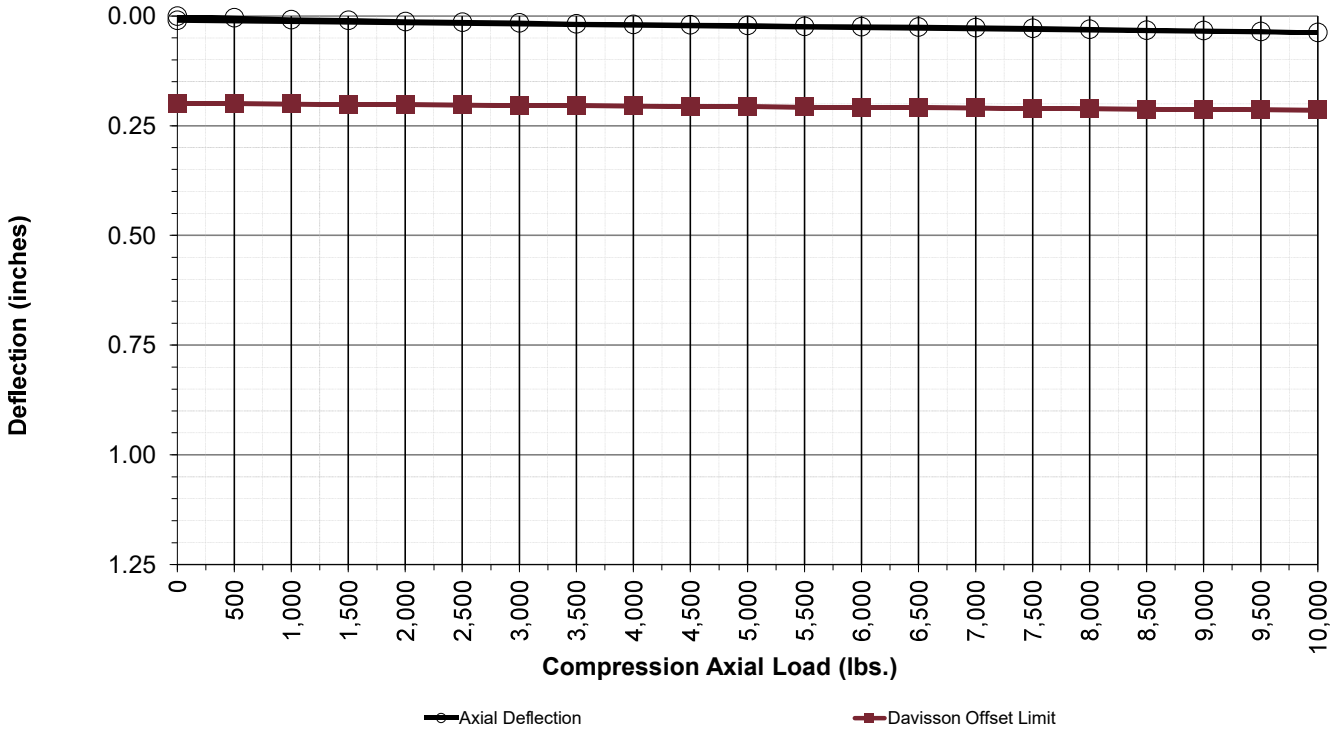
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-7B
Group Latitude [deg.]:	33.6612
Group Longitude[deg.]:	-114.7029
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	45

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.004	0.001	0.200	
10%	1000	0.009	0.002	0.201	
15%	1500	0.010	0.002	0.201	
20%	2000	0.013	0.003	0.202	
25%	2500	0.014	0.004	0.203	
30%	3000	0.016	0.005	0.204	
35%	3500	0.018	0.005	0.205	
40%	4000	0.020	0.006	0.205	
45%	4500	0.021	0.007	0.206	
50%	5000	0.022	0.008	0.207	
55%	5500	0.024	0.008	0.208	
60%	6000	0.024	0.009	0.208	
65%	6500	0.025	0.010	0.209	
70%	7000	0.027	0.011	0.210	
75%	7500	0.028	0.012	0.211	
80%	8000	0.030	0.012	0.212	
85%	8500	0.032	0.013	0.212	
90%	9000	0.034	0.014	0.213	
95%	9500	0.036	0.015	0.214	
100%	10000	0.038	0.015	0.215	
0%	0	0.011	0.000	0.199	



Tension Load Test Result for PLT-8A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

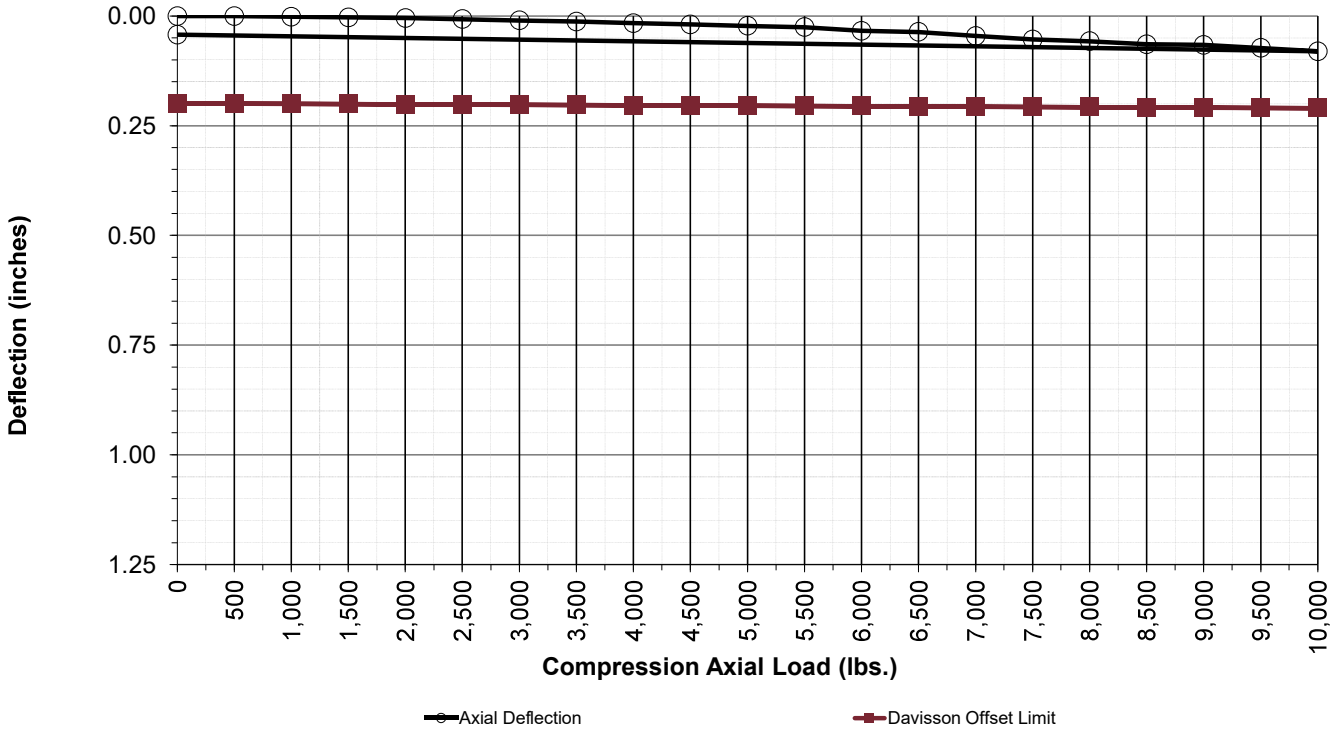
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-8A
Group Latitude [deg.]:	33.6600
Group Longitude[deg.]:	-114.6888
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	30

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.002	0.001	0.200	
15%	1500	0.003	0.002	0.201	
20%	2000	0.005	0.002	0.201	
25%	2500	0.007	0.003	0.202	
30%	3000	0.010	0.003	0.202	
35%	3500	0.013	0.004	0.203	
40%	4000	0.017	0.004	0.203	
45%	4500	0.019	0.005	0.204	
50%	5000	0.023	0.005	0.205	
55%	5500	0.026	0.006	0.205	
60%	6000	0.034	0.006	0.206	
65%	6500	0.037	0.007	0.206	
70%	7000	0.046	0.008	0.207	
75%	7500	0.054	0.008	0.207	
80%	8000	0.058	0.009	0.208	
85%	8500	0.064	0.009	0.208	
90%	9000	0.066	0.010	0.209	
95%	9500	0.073	0.010	0.209	
100%	10000	0.081	0.011	0.210	
0%	0	0.042	0.000	0.199	



Tension Load Test Result for PLT-8B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

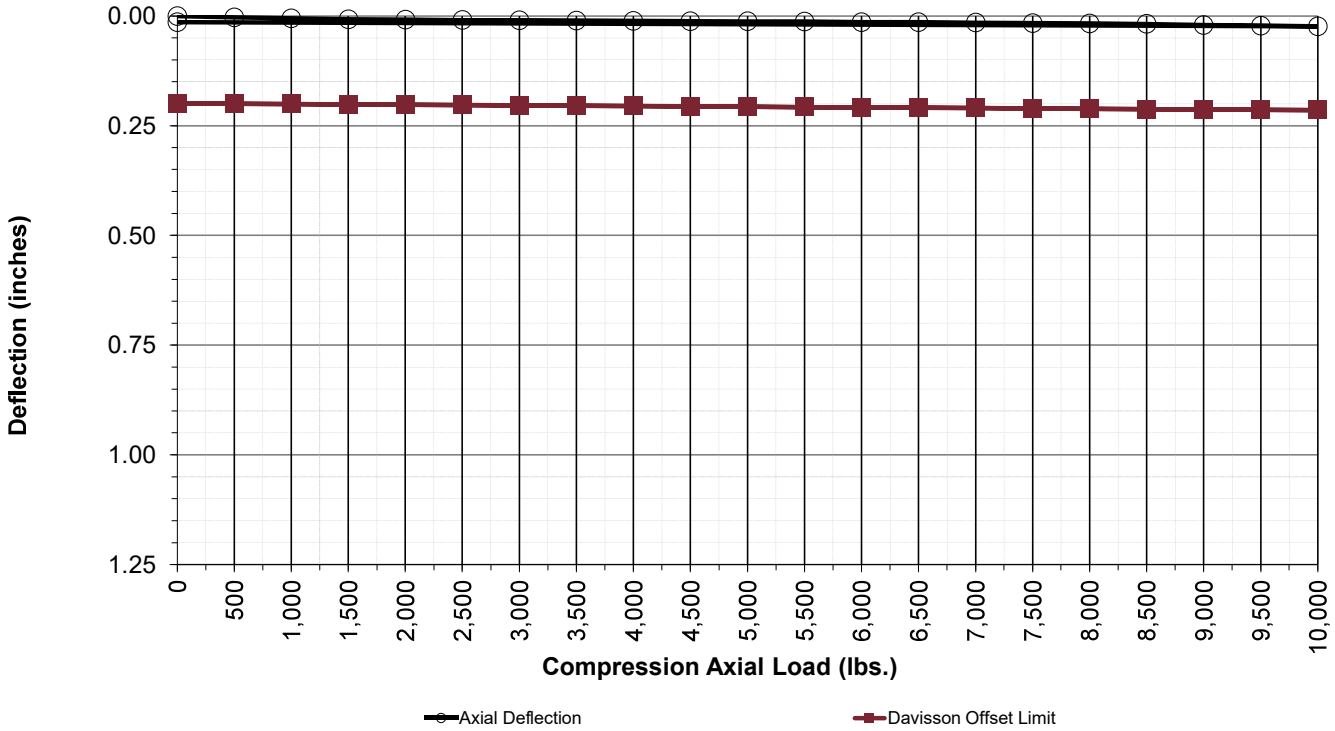
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-8B
Group Latitude [deg.]:	33.6600
Group Longitude[deg.]:	-114.6888
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	45

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.004	0.001	0.200	
10%	1000	0.006	0.002	0.201	
15%	1500	0.008	0.002	0.201	
20%	2000	0.009	0.003	0.202	
25%	2500	0.010	0.004	0.203	
30%	3000	0.010	0.005	0.204	
35%	3500	0.011	0.005	0.205	
40%	4000	0.012	0.006	0.205	
45%	4500	0.012	0.007	0.206	
50%	5000	0.013	0.008	0.207	
55%	5500	0.013	0.008	0.208	
60%	6000	0.014	0.009	0.208	
65%	6500	0.015	0.010	0.209	
70%	7000	0.016	0.011	0.210	
75%	7500	0.016	0.012	0.211	
80%	8000	0.017	0.012	0.212	
85%	8500	0.019	0.013	0.212	
90%	9000	0.021	0.014	0.213	
95%	9500	0.022	0.015	0.214	
100%	10000	0.024	0.015	0.215	
0%	0	0.014	0.000	0.199	



Tension Load Test Result for PLT-9A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

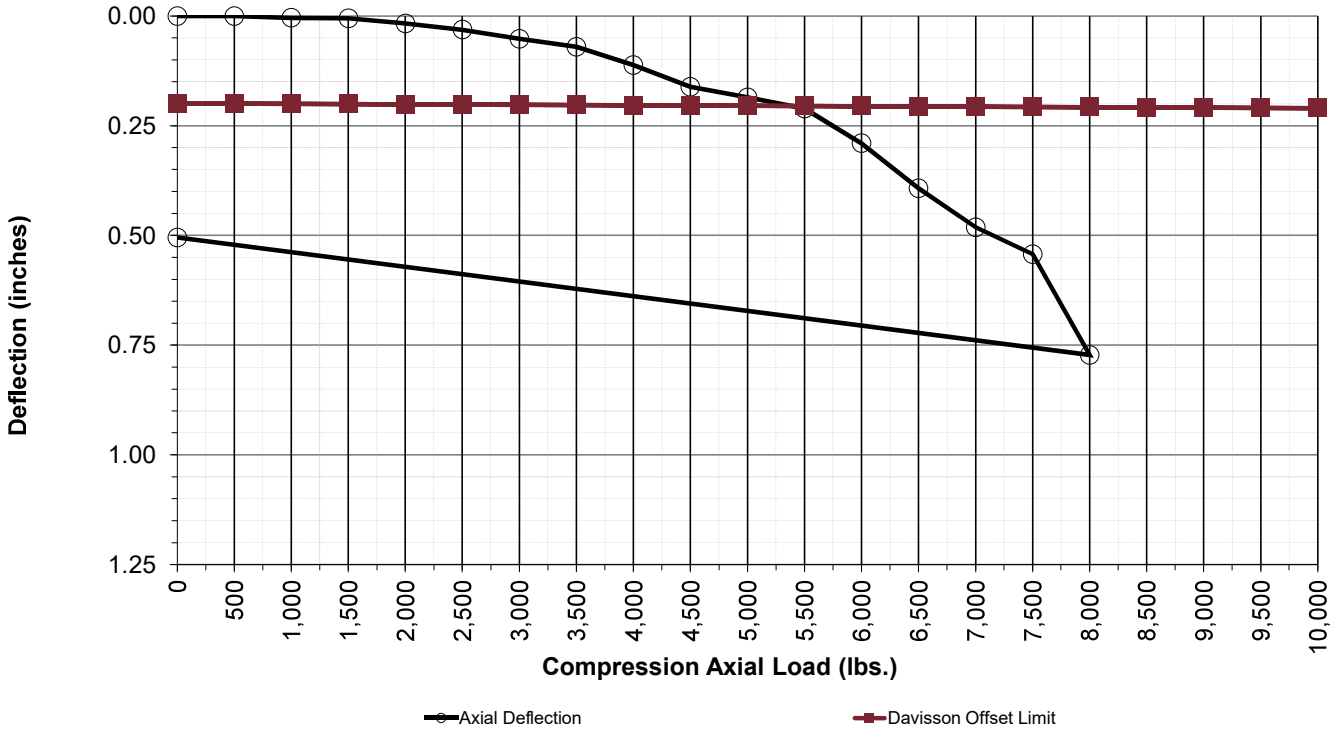
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-9A
Group Latitude [deg.]:	33.6600
Group Longitude[deg.]:	-114.6779
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	13

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.004	0.001	0.200	
15%	1500	0.005	0.002	0.201	
20%	2000	0.017	0.002	0.201	
25%	2500	0.032	0.003	0.202	
30%	3000	0.052	0.003	0.202	
35%	3500	0.071	0.004	0.203	
40%	4000	0.112	0.004	0.203	
45%	4500	0.161	0.005	0.204	
50%	5000	0.186	0.005	0.205	
55%	5500	0.211	0.006	0.205	
60%	6000	0.290	0.006	0.206	
65%	6500	0.393	0.007	0.206	
70%	7000	0.482	0.008	0.207	
75%	7500	0.543	0.008	0.207	
80%	8000	0.773	0.009	0.208	
85%	8500		0.009	0.208	
90%	9000		0.010	0.209	
95%	9500		0.010	0.209	
100%	10000		0.011	0.210	
0%	0	0.505	0.000	0.199	



Tension Load Test Result for PLT-9B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

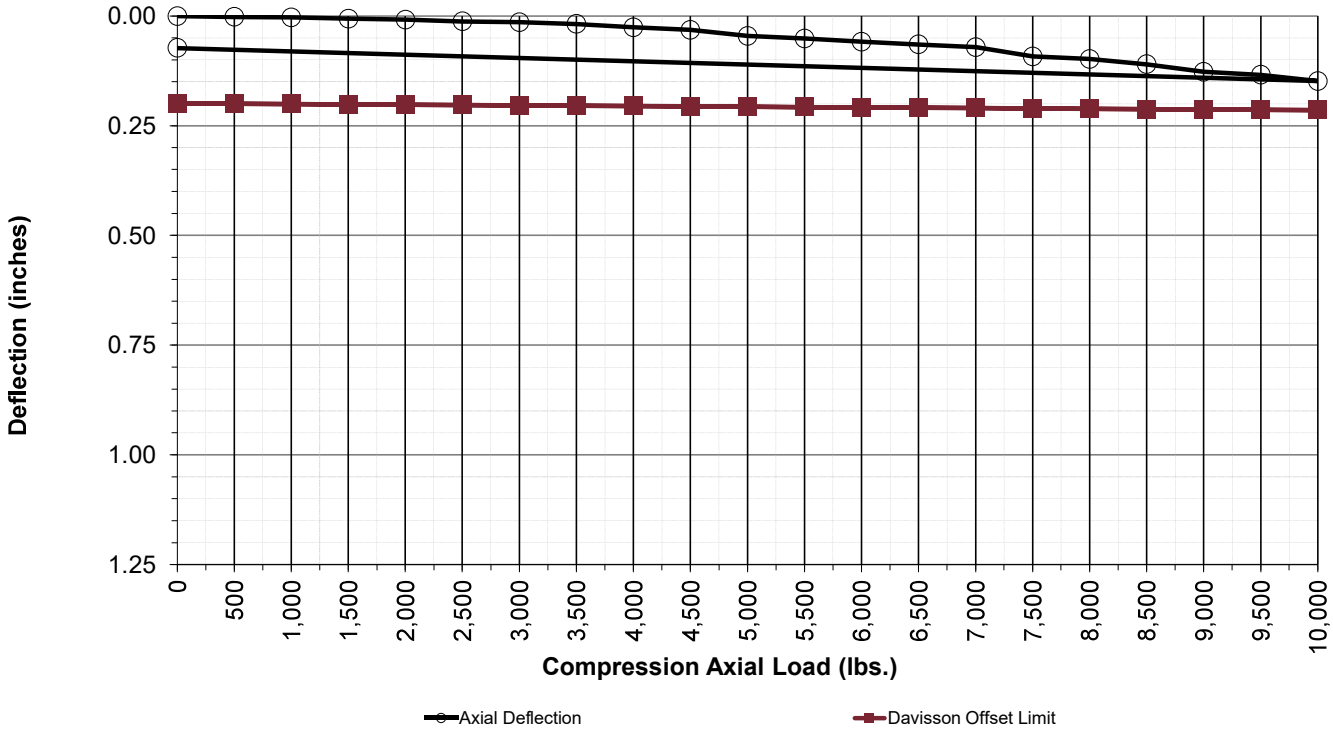
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-9B
Group Latitude [deg.]:	33.6600
Group Longitude[deg.]:	-114.6779
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	32

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.003	0.001	0.200	
10%	1000	0.004	0.002	0.201	
15%	1500	0.006	0.002	0.201	
20%	2000	0.009	0.003	0.202	
25%	2500	0.012	0.004	0.203	
30%	3000	0.015	0.005	0.204	
35%	3500	0.018	0.005	0.205	
40%	4000	0.026	0.006	0.205	
45%	4500	0.032	0.007	0.206	
50%	5000	0.046	0.008	0.207	
55%	5500	0.051	0.008	0.208	
60%	6000	0.059	0.009	0.208	
65%	6500	0.065	0.010	0.209	
70%	7000	0.071	0.011	0.210	
75%	7500	0.093	0.012	0.211	
80%	8000	0.098	0.012	0.212	
85%	8500	0.110	0.013	0.212	
90%	9000	0.127	0.014	0.213	
95%	9500	0.134	0.015	0.214	
100%	10000	0.149	0.015	0.215	
0%	0	0.073	0.000	0.199	



Tension Load Test Result for PLT-10A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

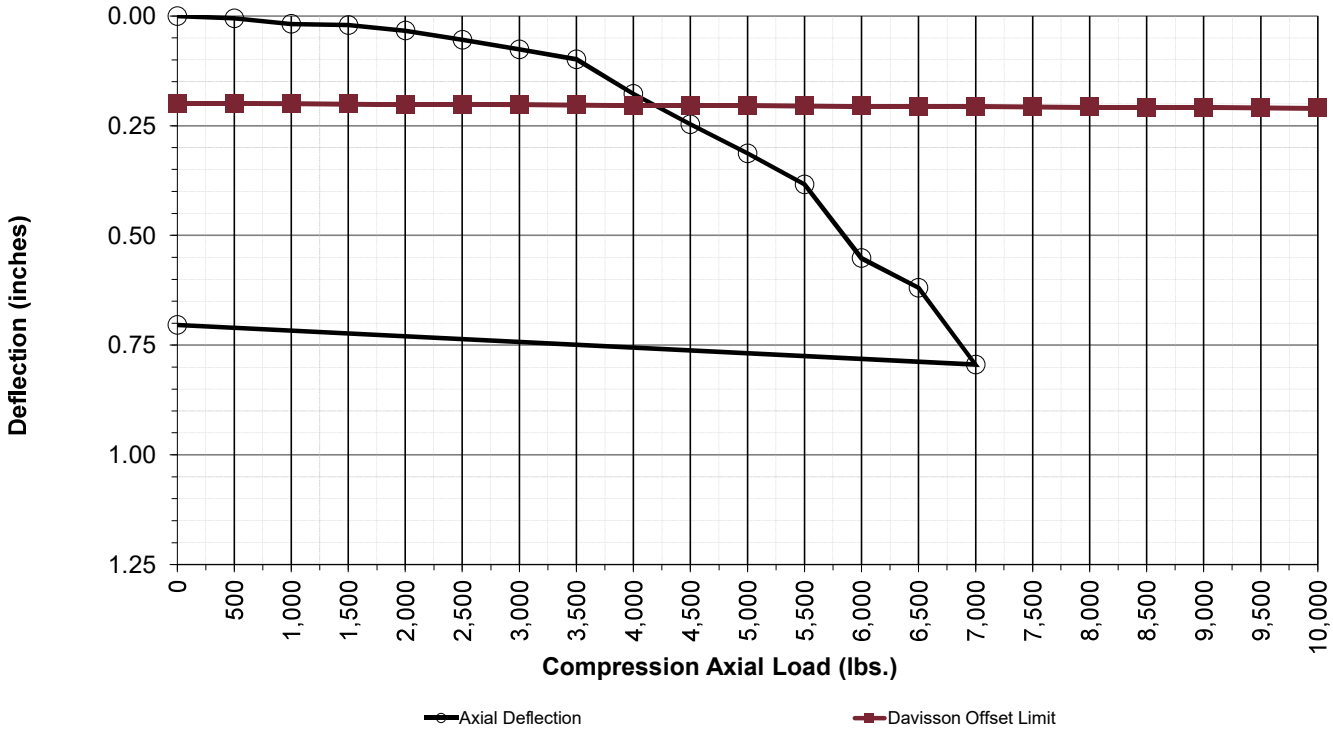
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-10A
Group Latitude [deg.]:	33.6565
Group Longitude[deg.]:	-114.6743
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	16

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.006	0.001	0.200	
10%	1000	0.019	0.001	0.200	
15%	1500	0.021	0.002	0.201	
20%	2000	0.034	0.002	0.201	
25%	2500	0.055	0.003	0.202	
30%	3000	0.076	0.003	0.202	
35%	3500	0.099	0.004	0.203	
40%	4000	0.178	0.004	0.203	
45%	4500	0.247	0.005	0.204	
50%	5000	0.313	0.005	0.205	
55%	5500	0.384	0.006	0.205	
60%	6000	0.552	0.006	0.206	
65%	6500	0.620	0.007	0.206	
70%	7000	0.795	0.008	0.207	
75%	7500		0.008	0.207	
80%	8000		0.009	0.208	
85%	8500		0.009	0.208	
90%	9000		0.010	0.209	
95%	9500		0.010	0.209	
100%	10000		0.011	0.210	
0%	0	0.704	0.000	0.199	



Tension Load Test Result for PLT-10B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

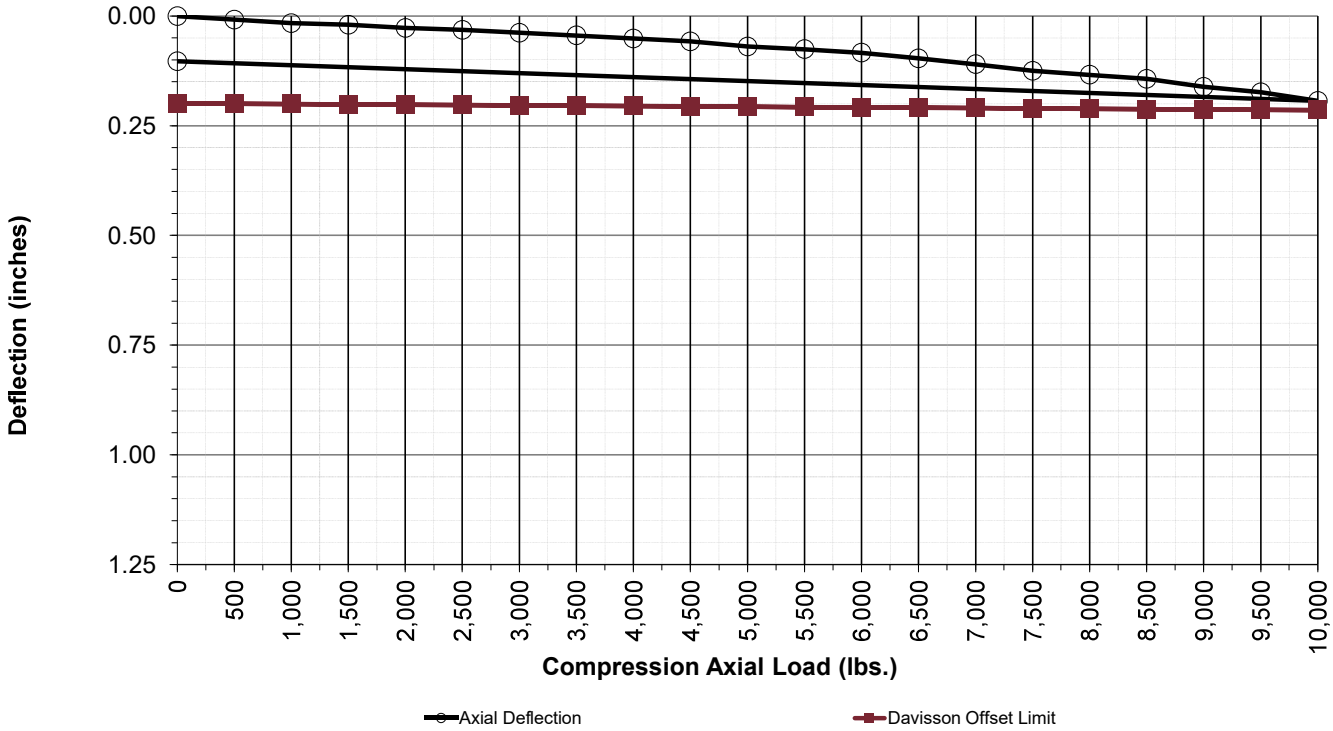
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-10B
Group Latitude [deg.]:	33.6565
Group Longitude[deg.]:	-114.6743
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	34

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.009	0.001	0.200	
10%	1000	0.017	0.002	0.201	
15%	1500	0.020	0.002	0.201	
20%	2000	0.027	0.003	0.202	
25%	2500	0.032	0.004	0.203	
30%	3000	0.038	0.005	0.204	
35%	3500	0.045	0.005	0.205	
40%	4000	0.051	0.006	0.205	
45%	4500	0.058	0.007	0.206	
50%	5000	0.070	0.008	0.207	
55%	5500	0.076	0.008	0.208	
60%	6000	0.084	0.009	0.208	
65%	6500	0.097	0.010	0.209	
70%	7000	0.110	0.011	0.210	
75%	7500	0.125	0.012	0.211	
80%	8000	0.134	0.012	0.212	
85%	8500	0.143	0.013	0.212	
90%	9000	0.162	0.014	0.213	
95%	9500	0.174	0.015	0.214	
100%	10000	0.194	0.015	0.215	
0%	0	0.104	0.000	0.199	



Tension Load Test Result for PLT-11A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

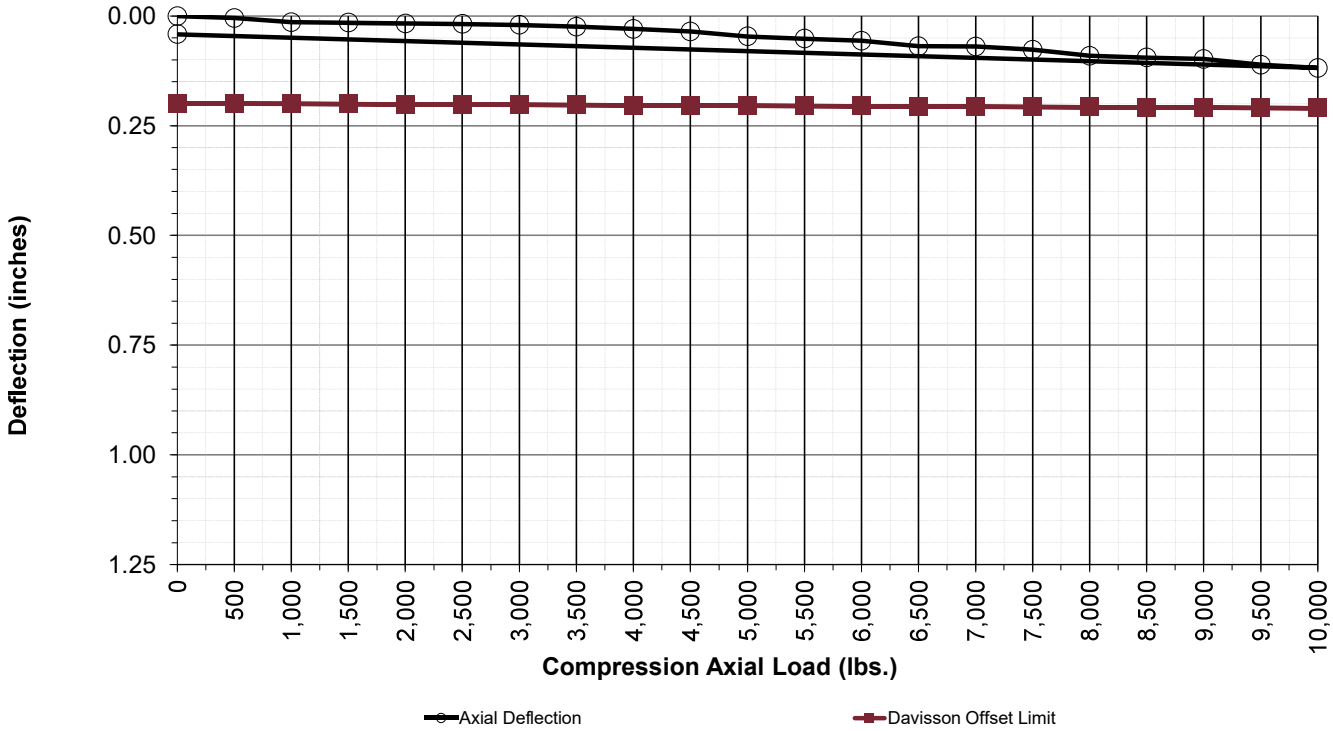
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-11A
Group Latitude [deg.]:	33.6564
Group Longitude[deg.]:	-114.6815
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	23

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.005	0.001	0.200	
10%	1000	0.014	0.001	0.200	
15%	1500	0.016	0.002	0.201	
20%	2000	0.017	0.002	0.201	
25%	2500	0.019	0.003	0.202	
30%	3000	0.021	0.003	0.202	
35%	3500	0.025	0.004	0.203	
40%	4000	0.029	0.004	0.203	
45%	4500	0.035	0.005	0.204	
50%	5000	0.047	0.005	0.205	
55%	5500	0.052	0.006	0.205	
60%	6000	0.057	0.006	0.206	
65%	6500	0.069	0.007	0.206	
70%	7000	0.070	0.008	0.207	
75%	7500	0.077	0.008	0.207	
80%	8000	0.091	0.009	0.208	
85%	8500	0.095	0.009	0.208	
90%	9000	0.098	0.010	0.209	
95%	9500	0.111	0.010	0.209	
100%	10000	0.119	0.011	0.210	
0%	0	0.042	0.000	0.199	



Tension Load Test Result for PLT-11B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

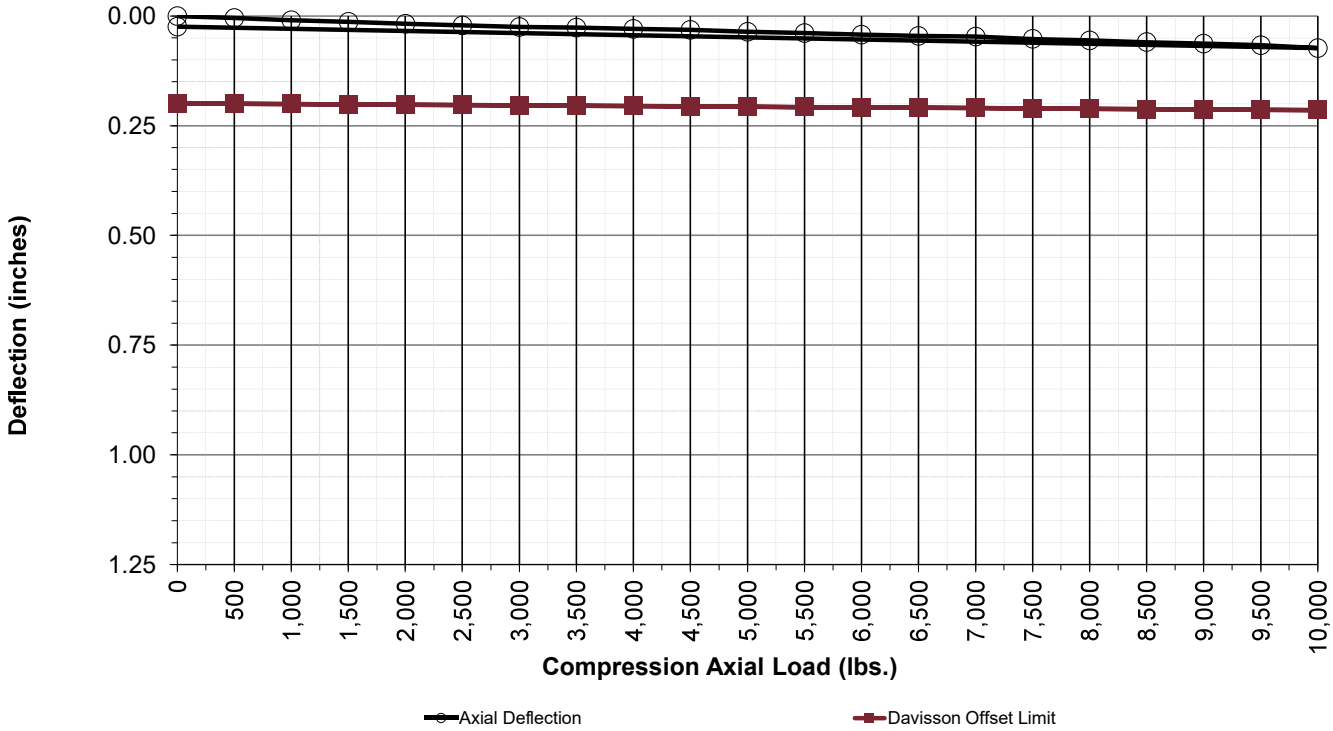
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-11B
Group Latitude [deg.]:	33.6564
Group Longitude[deg.]:	-114.6815
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	46

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.005	0.001	0.200	
10%	1000	0.010	0.002	0.201	
15%	1500	0.014	0.002	0.201	
20%	2000	0.018	0.003	0.202	
25%	2500	0.022	0.004	0.203	
30%	3000	0.025	0.005	0.204	
35%	3500	0.027	0.005	0.205	
40%	4000	0.030	0.006	0.205	
45%	4500	0.032	0.007	0.206	
50%	5000	0.036	0.008	0.207	
55%	5500	0.039	0.008	0.208	
60%	6000	0.042	0.009	0.208	
65%	6500	0.046	0.010	0.209	
70%	7000	0.047	0.011	0.210	
75%	7500	0.053	0.012	0.211	
80%	8000	0.056	0.012	0.212	
85%	8500	0.060	0.013	0.212	
90%	9000	0.063	0.014	0.213	
95%	9500	0.067	0.015	0.214	
100%	10000	0.074	0.015	0.215	
0%	0	0.024	0.000	0.199	



Tension Load Test Result for PLT-12A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

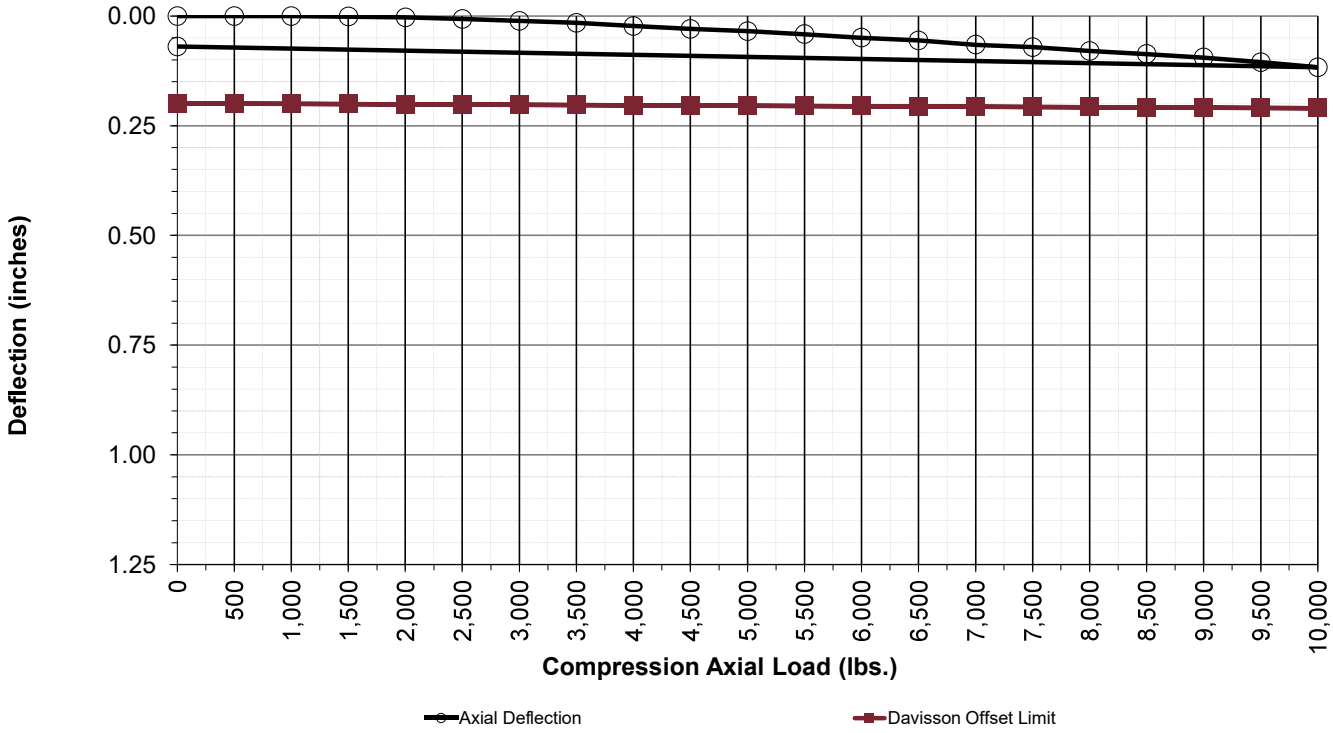
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-12A
Group Latitude [deg.]:	33.6563
Group Longitude[deg.]:	-114.6902
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	19

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.000	0.001	0.200	
10%	1000	0.000	0.001	0.200	
15%	1500	0.002	0.002	0.201	
20%	2000	0.004	0.002	0.201	
25%	2500	0.007	0.003	0.202	
30%	3000	0.011	0.003	0.202	
35%	3500	0.016	0.004	0.203	
40%	4000	0.023	0.004	0.203	
45%	4500	0.030	0.005	0.204	
50%	5000	0.035	0.005	0.205	
55%	5500	0.042	0.006	0.205	
60%	6000	0.050	0.006	0.206	
65%	6500	0.056	0.007	0.206	
70%	7000	0.066	0.008	0.207	
75%	7500	0.071	0.008	0.207	
80%	8000	0.080	0.009	0.208	
85%	8500	0.087	0.009	0.208	
90%	9000	0.095	0.010	0.209	
95%	9500	0.106	0.010	0.209	
100%	10000	0.117	0.011	0.210	
0%	0	0.070	0.000	0.199	



Tension Load Test Result for PLT-12B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

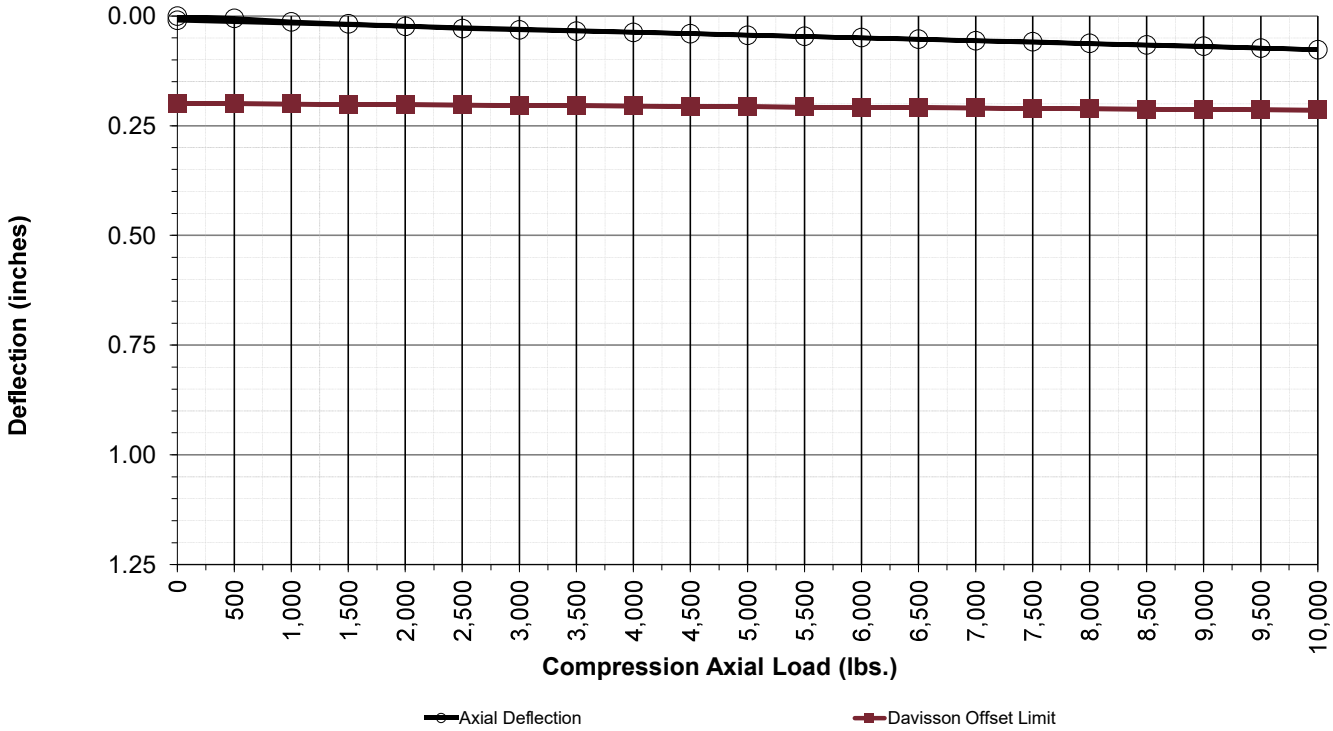
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-12B
Group Latitude [deg.]:	33.6563
Group Longitude[deg.]:	-114.6902
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	44

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.006	0.001	0.200	
10%	1000	0.014	0.002	0.201	
15%	1500	0.018	0.002	0.201	
20%	2000	0.024	0.003	0.202	
25%	2500	0.029	0.004	0.203	
30%	3000	0.032	0.005	0.204	
35%	3500	0.035	0.005	0.205	
40%	4000	0.038	0.006	0.205	
45%	4500	0.040	0.007	0.206	
50%	5000	0.044	0.008	0.207	
55%	5500	0.047	0.008	0.208	
60%	6000	0.050	0.009	0.208	
65%	6500	0.053	0.010	0.209	
70%	7000	0.056	0.011	0.210	
75%	7500	0.059	0.012	0.211	
80%	8000	0.063	0.012	0.212	
85%	8500	0.066	0.013	0.212	
90%	9000	0.069	0.014	0.213	
95%	9500	0.074	0.015	0.214	
100%	10000	0.077	0.015	0.215	
0%	0	0.010	0.000	0.199	



Tension Load Test Result for PLT-13A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

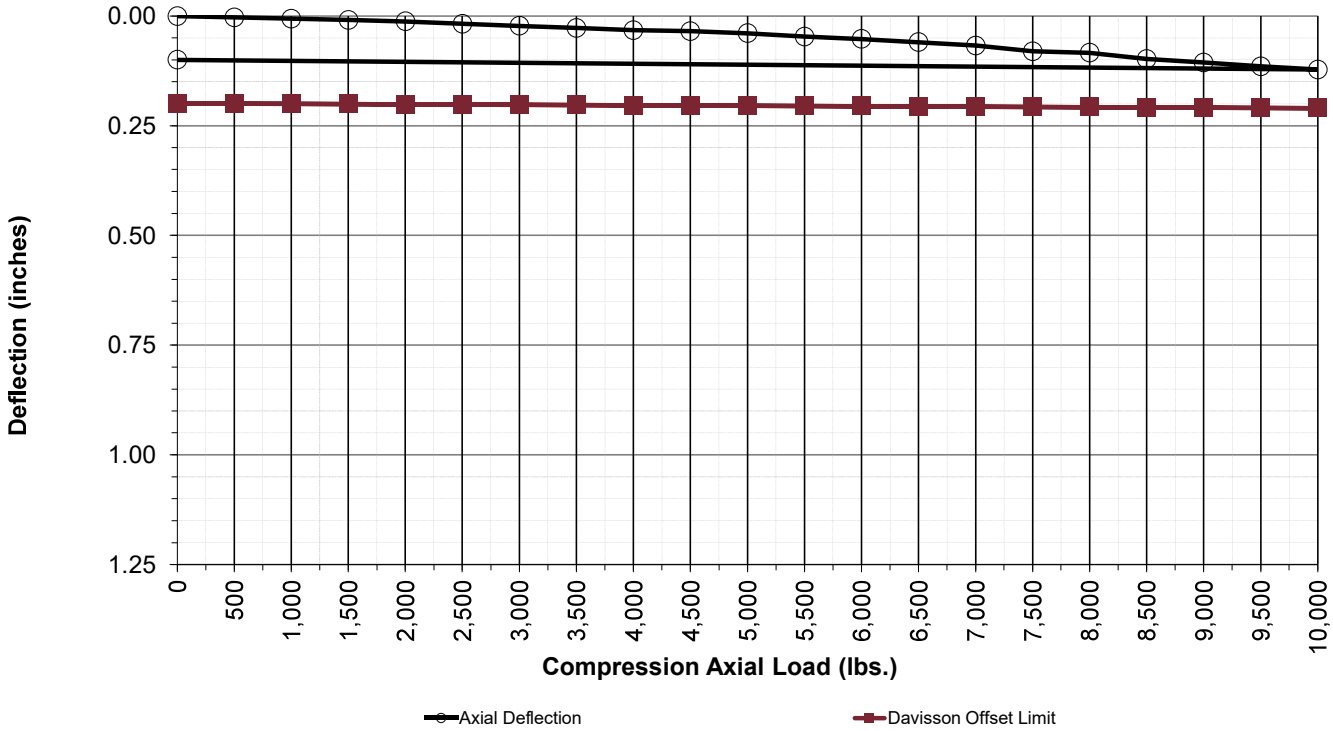
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-13A
Group Latitude [deg.]:	33.6563
Group Longitude[deg.]:	-114.7076
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	19

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.004	0.001	0.200	
10%	1000	0.006	0.001	0.200	
15%	1500	0.009	0.002	0.201	
20%	2000	0.013	0.002	0.201	
25%	2500	0.018	0.003	0.202	
30%	3000	0.023	0.003	0.202	
35%	3500	0.028	0.004	0.203	
40%	4000	0.033	0.004	0.203	
45%	4500	0.035	0.005	0.204	
50%	5000	0.040	0.005	0.205	
55%	5500	0.047	0.006	0.205	
60%	6000	0.053	0.006	0.206	
65%	6500	0.060	0.007	0.206	
70%	7000	0.067	0.008	0.207	
75%	7500	0.081	0.008	0.207	
80%	8000	0.084	0.009	0.208	
85%	8500	0.098	0.009	0.208	
90%	9000	0.106	0.010	0.209	
95%	9500	0.115	0.010	0.209	
100%	10000	0.122	0.011	0.210	
0%	0	0.100	0.000	0.199	



Tension Load Test Result for PLT-13B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

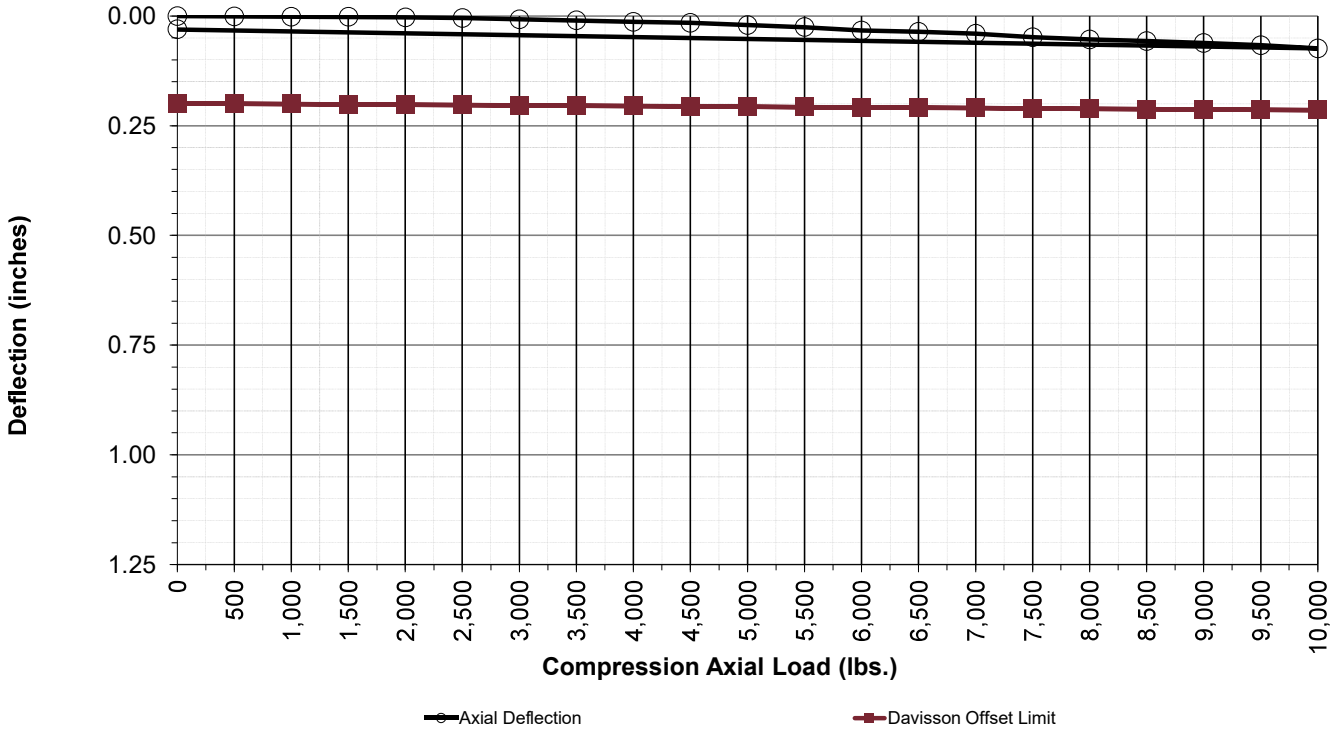
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-13B
Group Latitude [deg.]:	33.6563
Group Longitude[deg.]:	-114.7076
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	36

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.002	0.002	0.201	
15%	1500	0.003	0.002	0.201	
20%	2000	0.003	0.003	0.202	
25%	2500	0.005	0.004	0.203	
30%	3000	0.008	0.005	0.204	
35%	3500	0.011	0.005	0.205	
40%	4000	0.013	0.006	0.205	
45%	4500	0.016	0.007	0.206	
50%	5000	0.021	0.008	0.207	
55%	5500	0.026	0.008	0.208	
60%	6000	0.033	0.009	0.208	
65%	6500	0.036	0.010	0.209	
70%	7000	0.040	0.011	0.210	
75%	7500	0.049	0.012	0.211	
80%	8000	0.054	0.012	0.212	
85%	8500	0.057	0.013	0.212	
90%	9000	0.062	0.014	0.213	
95%	9500	0.067	0.015	0.214	
100%	10000	0.074	0.015	0.215	
0%	0	0.031	0.000	0.199	



Tension Load Test Result for PLT-14A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

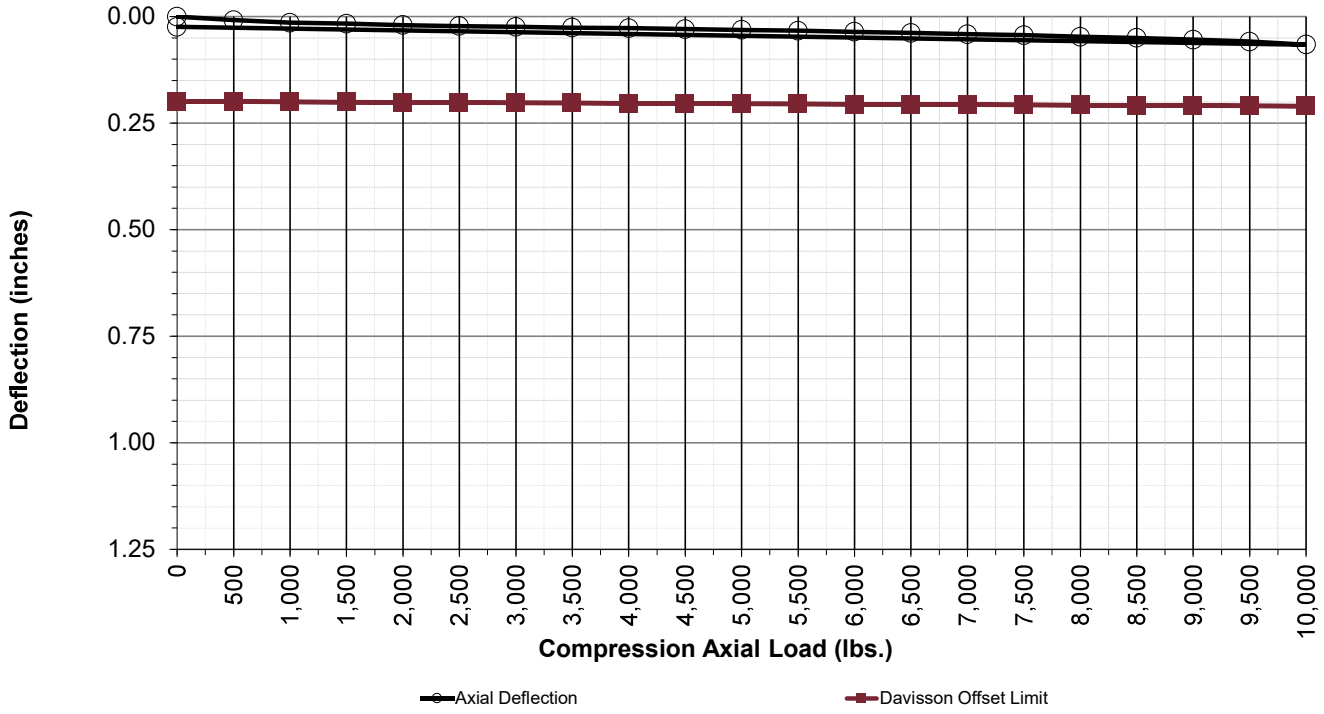
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-14A
Group Latitude [deg.]:	33.6527
Group Longitude[deg.]:	-114.7150
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	16

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.008	0.001	0.200	
10%	1000	0.015	0.001	0.200	
15%	1500	0.017	0.002	0.201	
20%	2000	0.020	0.002	0.201	
25%	2500	0.022	0.003	0.202	
30%	3000	0.024	0.003	0.202	
35%	3500	0.026	0.004	0.203	
40%	4000	0.027	0.004	0.203	
45%	4500	0.029	0.005	0.204	
50%	5000	0.031	0.005	0.205	
55%	5500	0.033	0.006	0.205	
60%	6000	0.036	0.006	0.206	
65%	6500	0.039	0.007	0.206	
70%	7000	0.041	0.008	0.207	
75%	7500	0.044	0.008	0.207	
80%	8000	0.047	0.009	0.208	
85%	8500	0.050	0.009	0.208	
90%	9000	0.054	0.010	0.209	
95%	9500	0.059	0.010	0.209	
100%	10000	0.066	0.011	0.210	
0%	0	0.024	0.000	0.199	



Tension Load Test Result for PLT-14B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

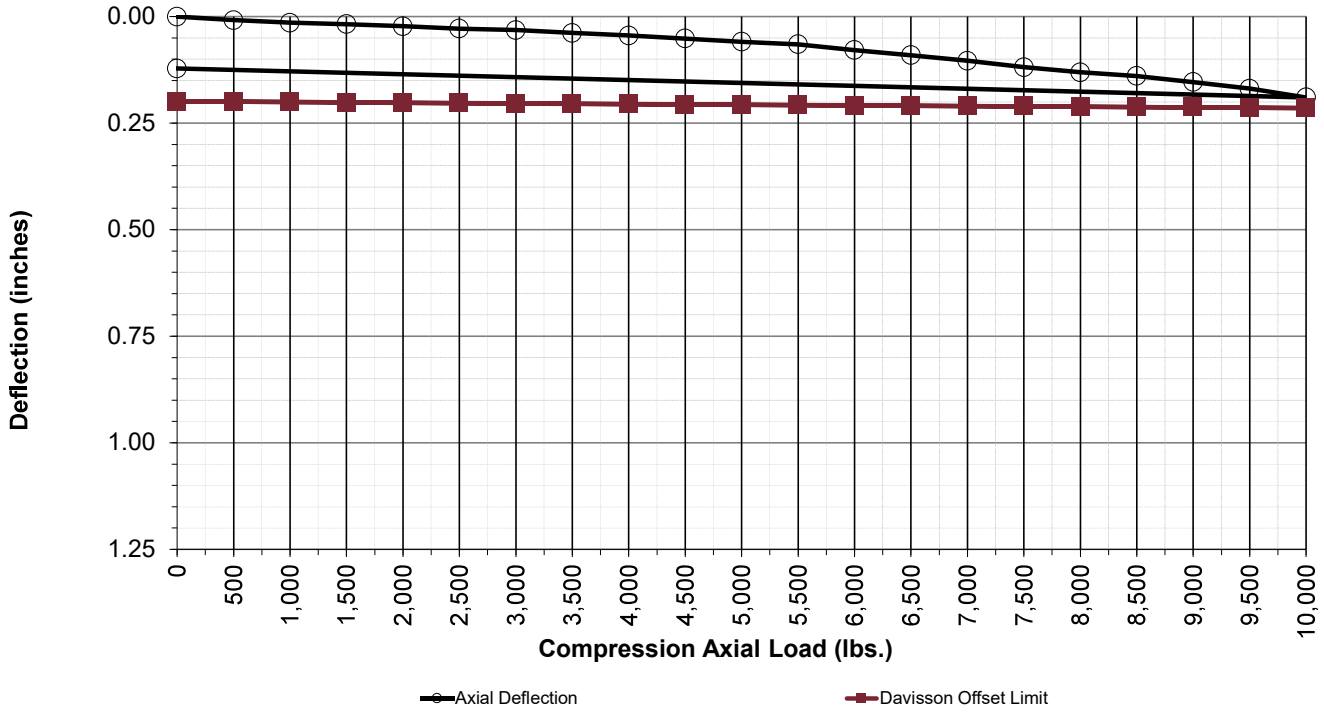
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-14B
Group Latitude [deg.]:	33.6527
Group Longitude[deg.]:	-114.7150
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	40

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.008	0.001	0.200	
10%	1000	0.014	0.002	0.201	
15%	1500	0.018	0.002	0.201	
20%	2000	0.023	0.003	0.202	
25%	2500	0.028	0.004	0.203	
30%	3000	0.032	0.005	0.204	
35%	3500	0.038	0.005	0.205	
40%	4000	0.044	0.006	0.205	
45%	4500	0.052	0.007	0.206	
50%	5000	0.059	0.008	0.207	
55%	5500	0.065	0.008	0.208	
60%	6000	0.079	0.009	0.208	
65%	6500	0.091	0.010	0.209	
70%	7000	0.104	0.011	0.210	
75%	7500	0.119	0.012	0.211	
80%	8000	0.131	0.012	0.212	
85%	8500	0.140	0.013	0.212	
90%	9000	0.154	0.014	0.213	
95%	9500	0.169	0.015	0.214	
100%	10000	0.190	0.015	0.215	
0%	0	0.122	0.000	0.199	



Tension Load Test Result for PLT-15A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

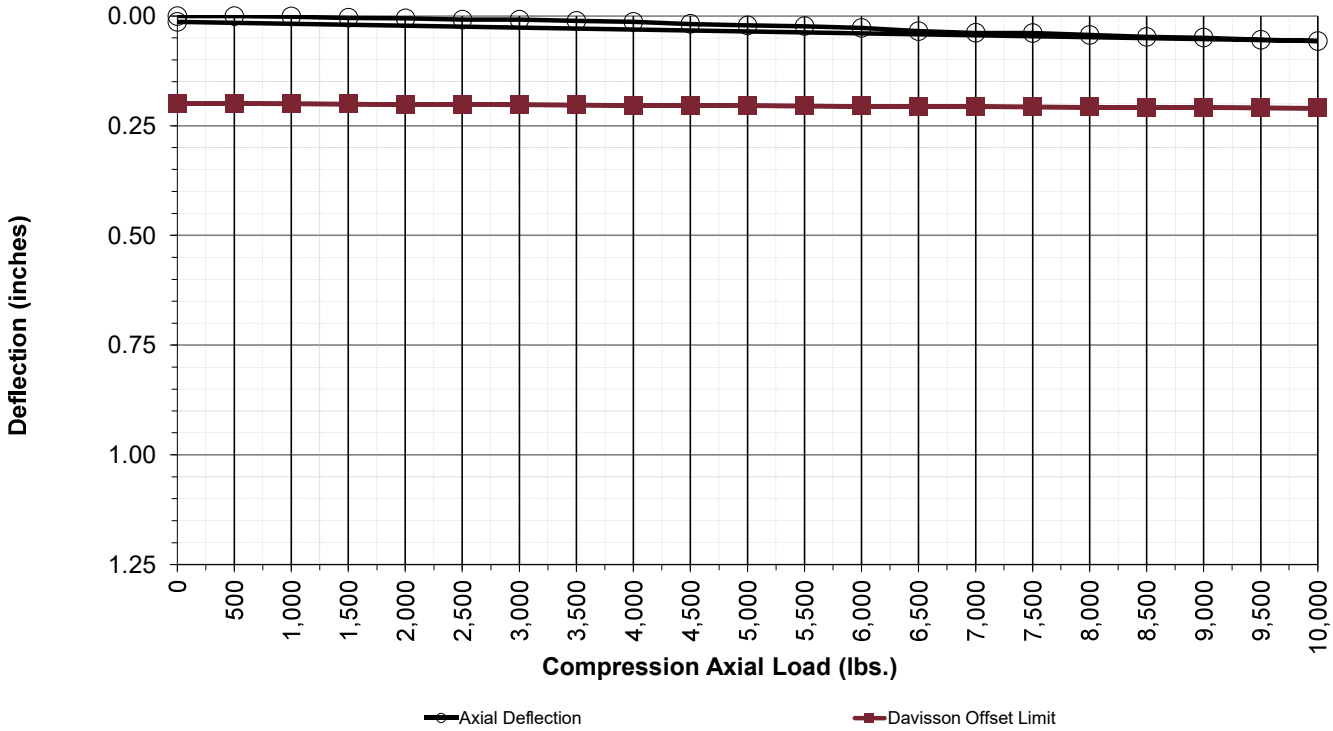
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-15A
Group Latitude [deg.]:	33.6527
Group Longitude[deg.]:	-114.7050
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	26

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.002	0.001	0.200	
15%	1500	0.005	0.002	0.201	
20%	2000	0.006	0.002	0.201	
25%	2500	0.008	0.003	0.202	
30%	3000	0.009	0.003	0.202	
35%	3500	0.011	0.004	0.203	
40%	4000	0.014	0.004	0.203	
45%	4500	0.019	0.005	0.204	
50%	5000	0.022	0.005	0.205	
55%	5500	0.023	0.006	0.205	
60%	6000	0.028	0.006	0.206	
65%	6500	0.035	0.007	0.206	
70%	7000	0.039	0.008	0.207	
75%	7500	0.039	0.008	0.207	
80%	8000	0.044	0.009	0.208	
85%	8500	0.048	0.009	0.208	
90%	9000	0.050	0.010	0.209	
95%	9500	0.054	0.010	0.209	
100%	10000	0.058	0.011	0.210	
0%	0	0.014	0.000	0.199	



Tension Load Test Result for PLT-15B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

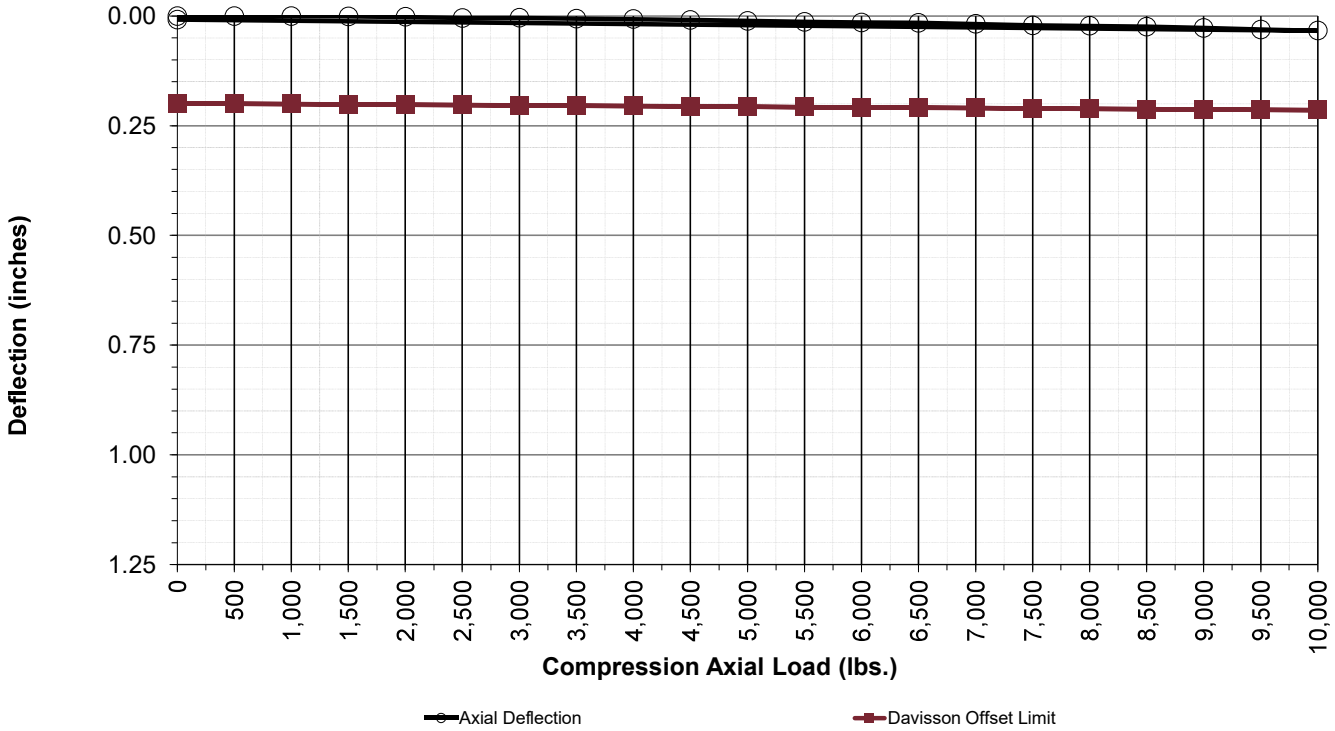
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-15B
Group Latitude [deg.]:	33.6527
Group Longitude[deg.]:	-114.7050
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	40

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.000	0.001	0.200	
10%	1000	0.001	0.002	0.201	
15%	1500	0.001	0.002	0.201	
20%	2000	0.003	0.003	0.202	
25%	2500	0.005	0.004	0.203	
30%	3000	0.004	0.005	0.204	
35%	3500	0.006	0.005	0.205	
40%	4000	0.008	0.006	0.205	
45%	4500	0.009	0.007	0.206	
50%	5000	0.011	0.008	0.207	
55%	5500	0.013	0.008	0.208	
60%	6000	0.015	0.009	0.208	
65%	6500	0.016	0.010	0.209	
70%	7000	0.019	0.011	0.210	
75%	7500	0.021	0.012	0.211	
80%	8000	0.023	0.012	0.212	
85%	8500	0.025	0.013	0.212	
90%	9000	0.028	0.014	0.213	
95%	9500	0.031	0.015	0.214	
100%	10000	0.034	0.015	0.215	
0%	0	0.009	0.000	0.199	



Tension Load Test Result for PLT-16A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

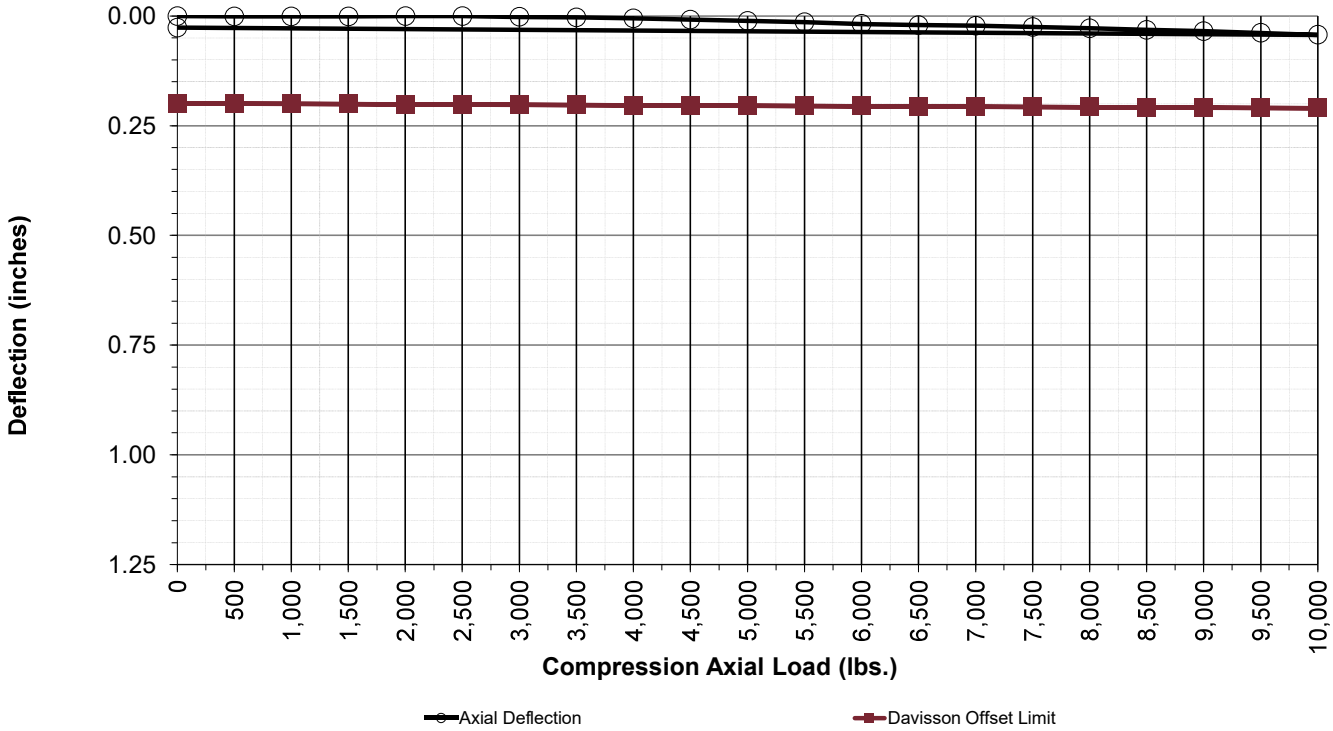
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-16A
Group Latitude [deg.]:	33.6496
Group Longitude[deg.]:	-114.7092
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	11

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.001	0.001	0.200	
15%	1500	0.001	0.002	0.201	
20%	2000	0.001	0.002	0.201	
25%	2500	0.001	0.003	0.202	
30%	3000	0.003	0.003	0.202	
35%	3500	0.004	0.004	0.203	
40%	4000	0.006	0.004	0.203	
45%	4500	0.009	0.005	0.204	
50%	5000	0.011	0.005	0.205	
55%	5500	0.015	0.006	0.205	
60%	6000	0.018	0.006	0.206	
65%	6500	0.021	0.007	0.206	
70%	7000	0.022	0.008	0.207	
75%	7500	0.025	0.008	0.207	
80%	8000	0.028	0.009	0.208	
85%	8500	0.032	0.009	0.208	
90%	9000	0.035	0.010	0.209	
95%	9500	0.039	0.010	0.209	
100%	10000	0.043	0.011	0.210	
0%	0	0.027	0.000	0.199	



Tension Load Test Result for PLT-16B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

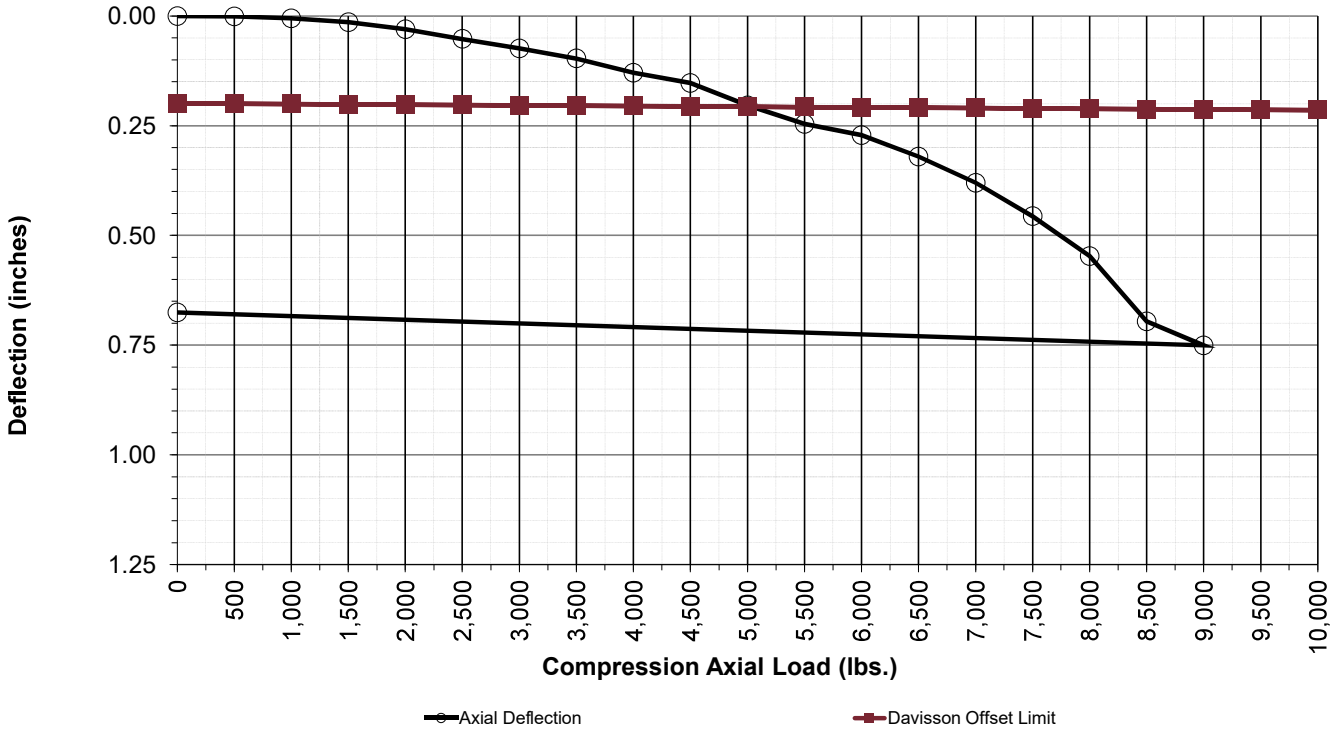
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-16B
Group Latitude [deg.]:	33.6496
Group Longitude[deg.]:	-114.7092
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	32

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.006	0.002	0.201	
15%	1500	0.014	0.002	0.201	
20%	2000	0.030	0.003	0.202	
25%	2500	0.053	0.004	0.203	
30%	3000	0.074	0.005	0.204	
35%	3500	0.097	0.005	0.205	
40%	4000	0.130	0.006	0.205	
45%	4500	0.153	0.007	0.206	
50%	5000	0.204	0.008	0.207	
55%	5500	0.246	0.008	0.208	
60%	6000	0.272	0.009	0.208	
65%	6500	0.320	0.010	0.209	
70%	7000	0.380	0.011	0.210	
75%	7500	0.457	0.012	0.211	
80%	8000	0.547	0.012	0.212	
85%	8500	0.696	0.013	0.212	
90%	9000	0.751	0.014	0.213	
95%	9500		0.015	0.214	
100%	10000		0.015	0.215	
0%	0	0.676	0.000	0.199	



Tension Load Test Result for PLT-17A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

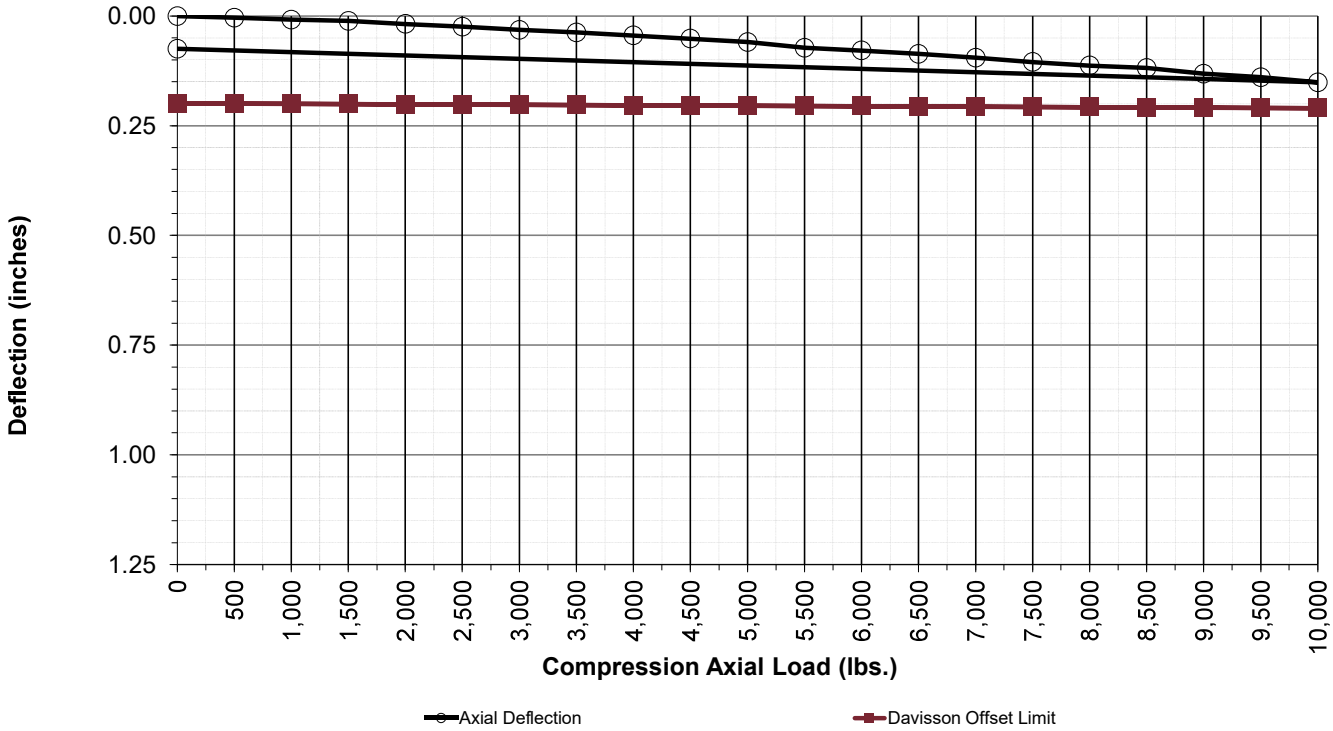
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-17A
Group Latitude [deg.]:	33.6516
Group Longitude[deg.]:	-114.6847
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	25

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.004	0.001	0.200	
10%	1000	0.008	0.001	0.200	
15%	1500	0.012	0.002	0.201	
20%	2000	0.018	0.002	0.201	
25%	2500	0.024	0.003	0.202	
30%	3000	0.032	0.003	0.202	
35%	3500	0.038	0.004	0.203	
40%	4000	0.045	0.004	0.203	
45%	4500	0.052	0.005	0.204	
50%	5000	0.059	0.005	0.205	
55%	5500	0.073	0.006	0.205	
60%	6000	0.079	0.006	0.206	
65%	6500	0.087	0.007	0.206	
70%	7000	0.095	0.008	0.207	
75%	7500	0.105	0.008	0.207	
80%	8000	0.113	0.009	0.208	
85%	8500	0.119	0.009	0.208	
90%	9000	0.132	0.010	0.209	
95%	9500	0.140	0.010	0.209	
100%	10000	0.151	0.011	0.210	
0%	0	0.075	0.000	0.199	



Tension Load Test Result for PLT-17B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

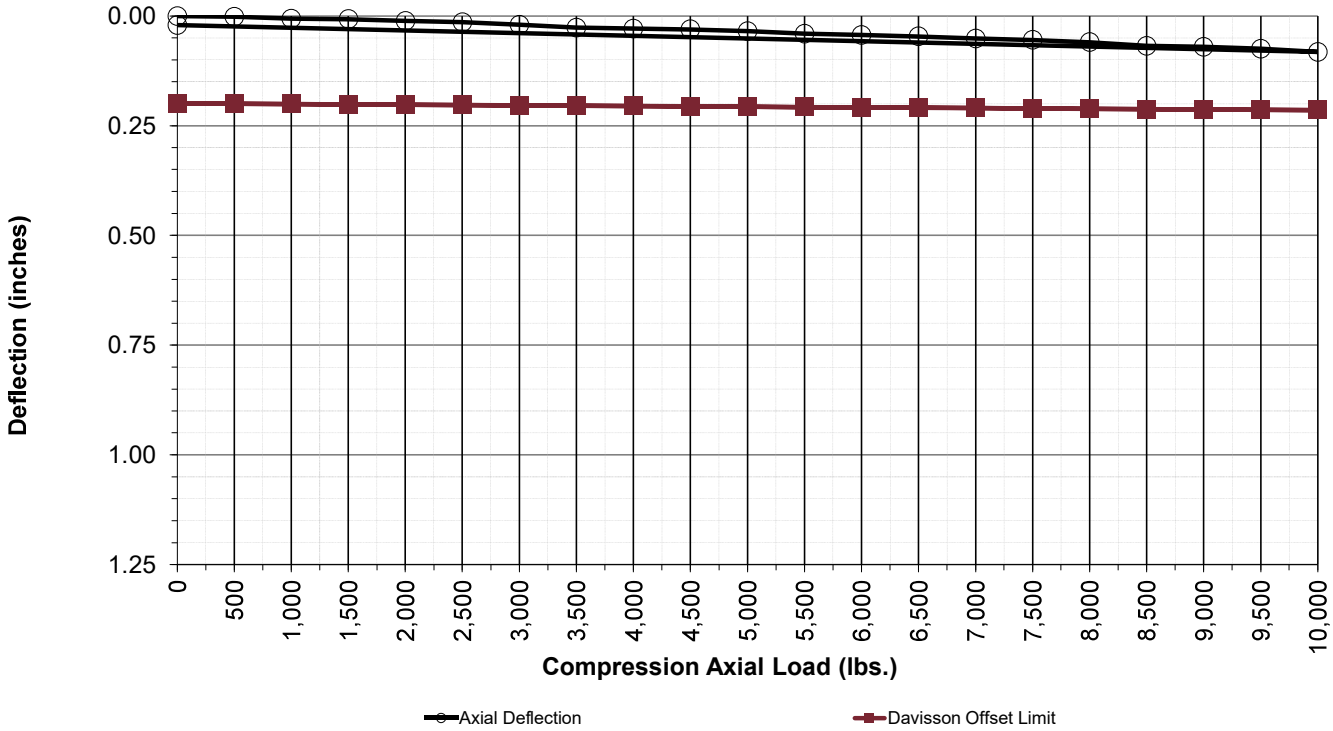
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-17B
Group Latitude [deg.]:	33.6516
Group Longitude[deg.]:	-114.6847
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	34

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.002	0.001	0.200	
10%	1000	0.006	0.002	0.201	
15%	1500	0.008	0.002	0.201	
20%	2000	0.011	0.003	0.202	
25%	2500	0.015	0.004	0.203	
30%	3000	0.020	0.005	0.204	
35%	3500	0.027	0.005	0.205	
40%	4000	0.029	0.006	0.205	
45%	4500	0.031	0.007	0.206	
50%	5000	0.035	0.008	0.207	
55%	5500	0.040	0.008	0.208	
60%	6000	0.043	0.009	0.208	
65%	6500	0.047	0.010	0.209	
70%	7000	0.051	0.011	0.210	
75%	7500	0.055	0.012	0.211	
80%	8000	0.060	0.012	0.212	
85%	8500	0.068	0.013	0.212	
90%	9000	0.071	0.014	0.213	
95%	9500	0.075	0.015	0.214	
100%	10000	0.082	0.015	0.215	
0%	0	0.021	0.000	0.199	



Tension Load Test Result for PLT-18A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

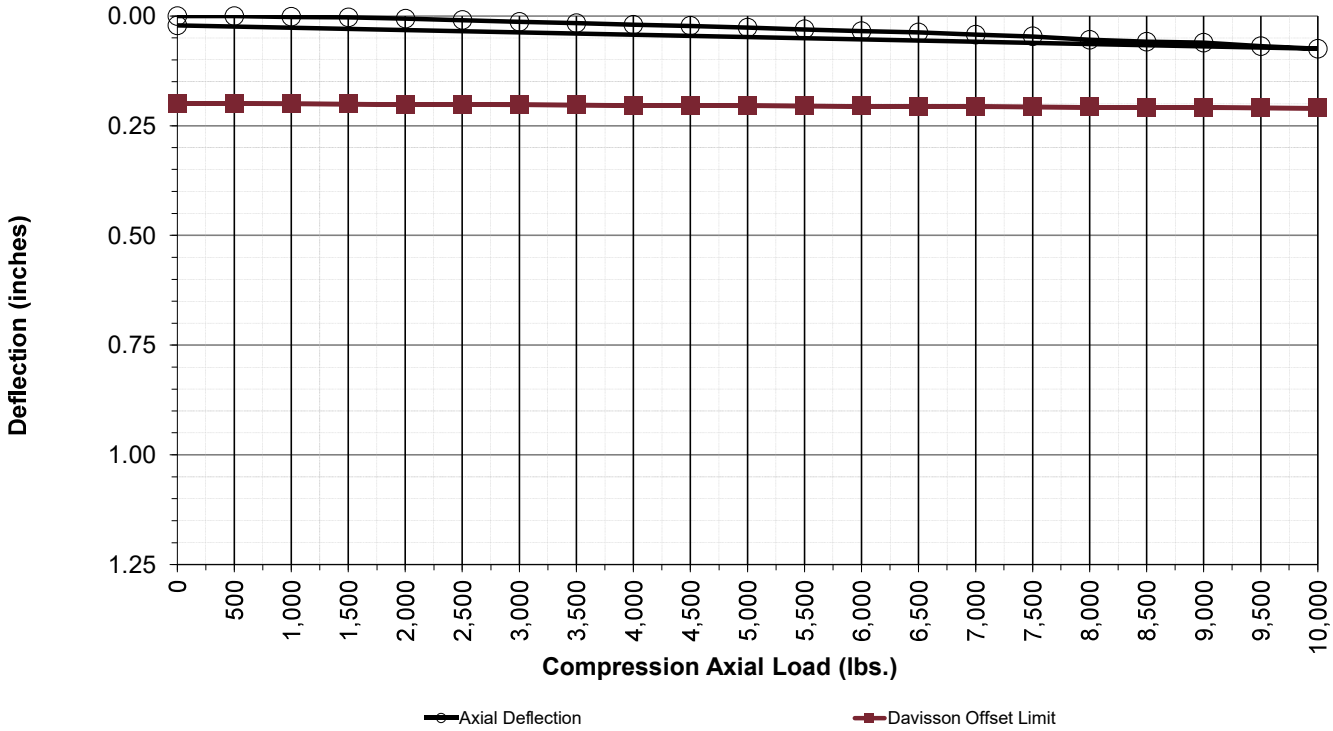
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-18A
Group Latitude [deg.]:	33.6528
Group Longitude[deg.]:	-114.6700
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	24

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.000	0.001	0.200	
10%	1000	0.002	0.001	0.200	
15%	1500	0.004	0.002	0.201	
20%	2000	0.007	0.002	0.201	
25%	2500	0.010	0.003	0.202	
30%	3000	0.014	0.003	0.202	
35%	3500	0.017	0.004	0.203	
40%	4000	0.020	0.004	0.203	
45%	4500	0.023	0.005	0.204	
50%	5000	0.027	0.005	0.205	
55%	5500	0.031	0.006	0.205	
60%	6000	0.035	0.006	0.206	
65%	6500	0.037	0.007	0.206	
70%	7000	0.043	0.008	0.207	
75%	7500	0.047	0.008	0.207	
80%	8000	0.055	0.009	0.208	
85%	8500	0.059	0.009	0.208	
90%	9000	0.061	0.010	0.209	
95%	9500	0.069	0.010	0.209	
100%	10000	0.075	0.011	0.210	
0%	0	0.022	0.000	0.199	



Tension Load Test Result for PLT-18B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

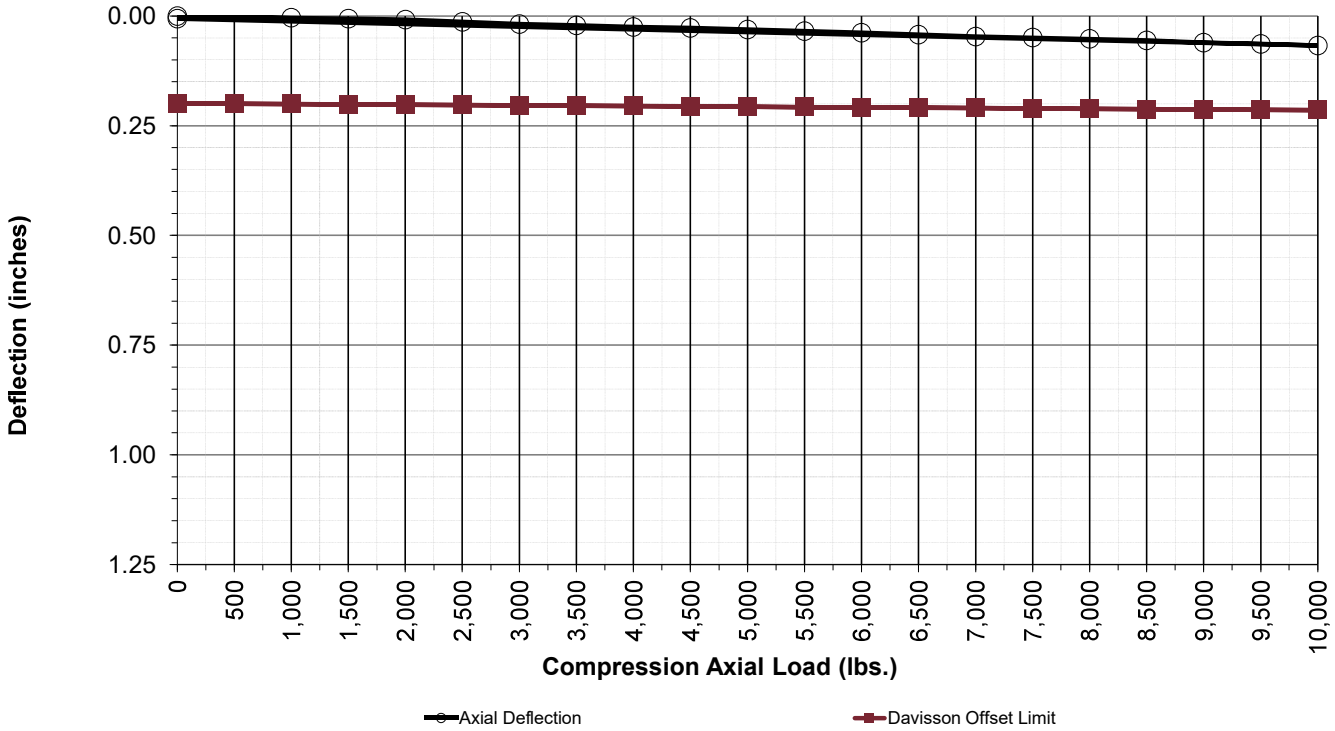
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-18B
Group Latitude [deg.]:	33.6528
Group Longitude[deg.]:	-114.6700
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	51

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	-0.001	0.001	0.200	
10%	1000	0.004	0.002	0.201	
15%	1500	0.006	0.002	0.201	
20%	2000	0.009	0.003	0.202	
25%	2500	0.013	0.004	0.203	
30%	3000	0.019	0.005	0.204	
35%	3500	0.021	0.005	0.205	
40%	4000	0.025	0.006	0.205	
45%	4500	0.028	0.007	0.206	
50%	5000	0.031	0.008	0.207	
55%	5500	0.034	0.008	0.208	
60%	6000	0.038	0.009	0.208	
65%	6500	0.042	0.010	0.209	
70%	7000	0.047	0.011	0.210	
75%	7500	0.050	0.012	0.211	
80%	8000	0.053	0.012	0.212	
85%	8500	0.056	0.013	0.212	
90%	9000	0.061	0.014	0.213	
95%	9500	0.064	0.015	0.214	
100%	10000	0.067	0.015	0.215	
0%	0	0.006	0.000	0.199	



Tension Load Test Result for PLT-19A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

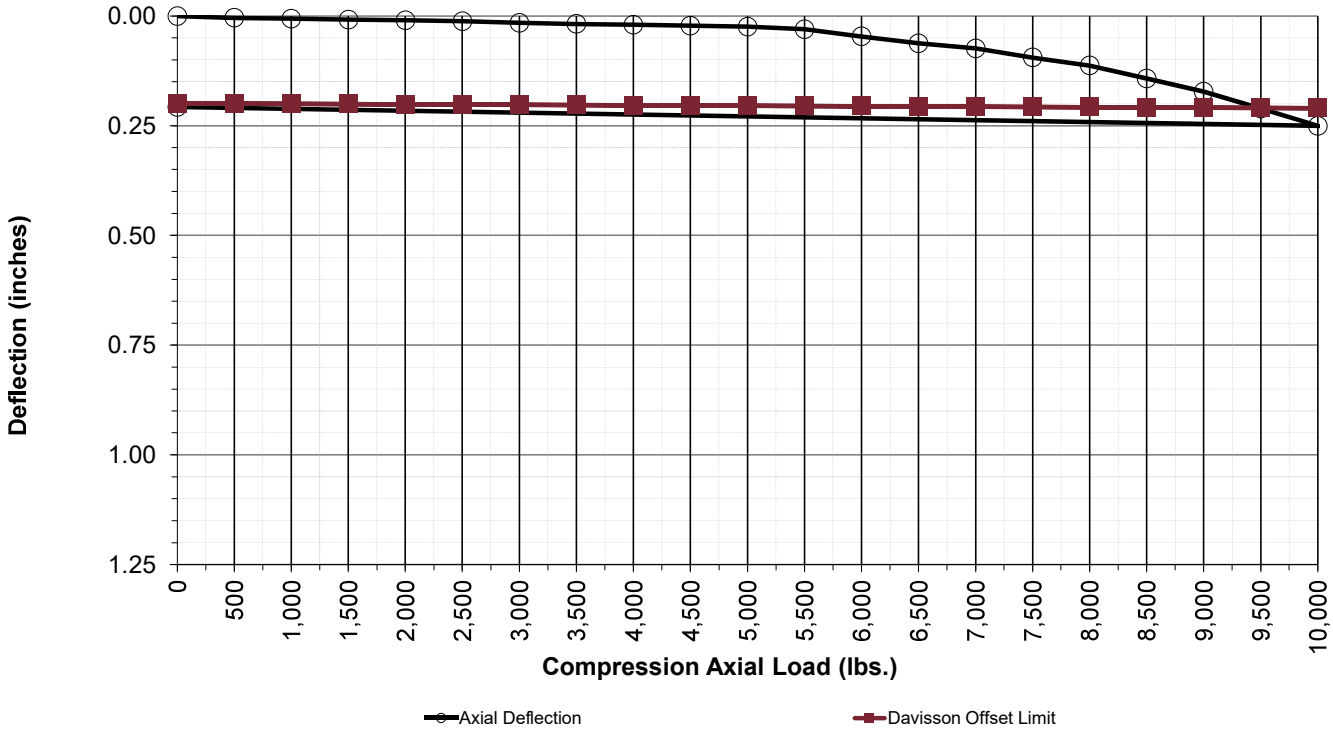
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-19A
Group Latitude [deg.]:	33.6487
Group Longitude[deg.]:	-114.6659
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	33

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.005	0.001	0.200	
10%	1000	0.007	0.001	0.200	
15%	1500	0.009	0.002	0.201	
20%	2000	0.010	0.002	0.201	
25%	2500	0.012	0.003	0.202	
30%	3000	0.016	0.003	0.202	
35%	3500	0.019	0.004	0.203	
40%	4000	0.020	0.004	0.203	
45%	4500	0.022	0.005	0.204	
50%	5000	0.024	0.005	0.205	
55%	5500	0.030	0.006	0.205	
60%	6000	0.047	0.006	0.206	
65%	6500	0.063	0.007	0.206	
70%	7000	0.074	0.008	0.207	
75%	7500	0.095	0.008	0.207	
80%	8000	0.113	0.009	0.208	
85%	8500	0.143	0.009	0.208	
90%	9000	0.172	0.010	0.209	
95%	9500	0.211	0.010	0.209	
100%	10000	0.251	0.011	0.210	
0%	0	0.207	0.000	0.199	



Tension Load Test Result for PLT-19B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

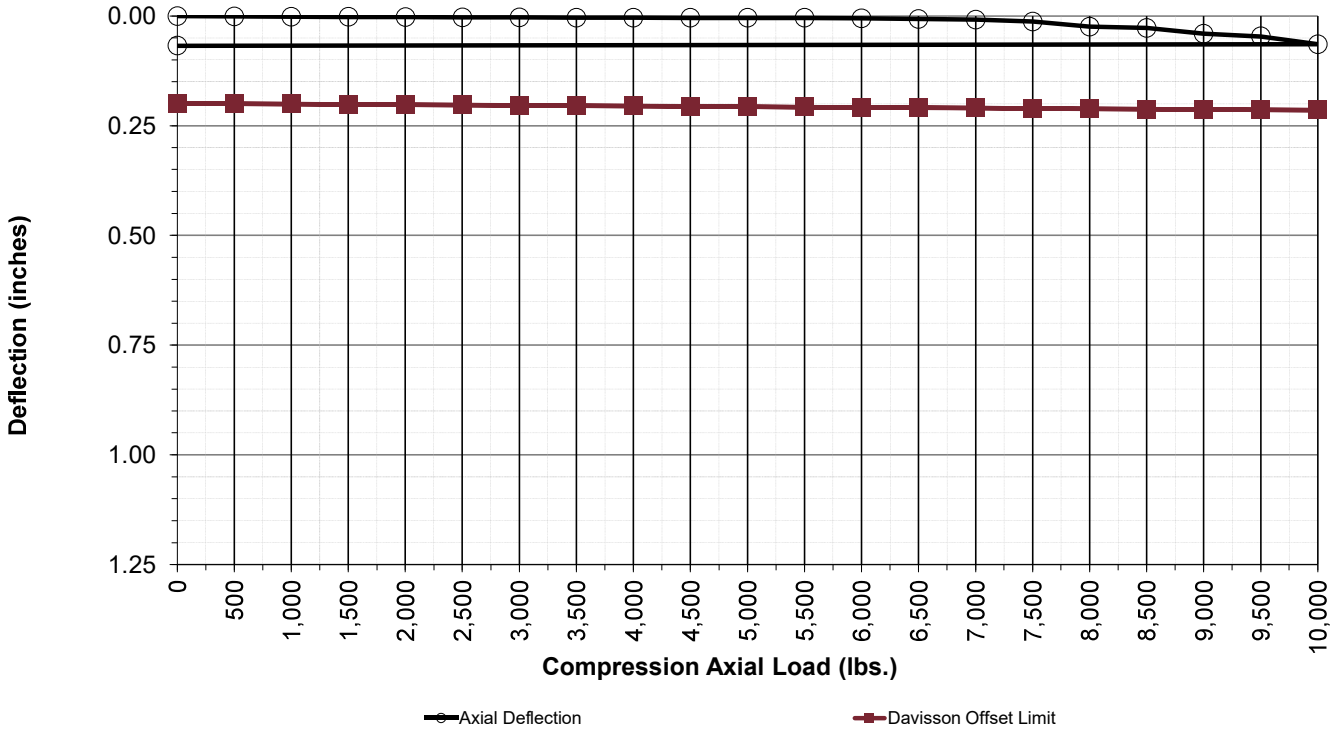
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-19B
Group Latitude [deg.]:	33.6487
Group Longitude[deg.]:	-114.6659
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	66

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.002	0.002	0.201	
15%	1500	0.003	0.002	0.201	
20%	2000	0.003	0.003	0.202	
25%	2500	0.003	0.004	0.203	
30%	3000	0.004	0.005	0.204	
35%	3500	0.004	0.005	0.205	
40%	4000	0.004	0.006	0.205	
45%	4500	0.005	0.007	0.206	
50%	5000	0.005	0.008	0.207	
55%	5500	0.005	0.008	0.208	
60%	6000	0.005	0.009	0.208	
65%	6500	0.007	0.010	0.209	
70%	7000	0.009	0.011	0.210	
75%	7500	0.013	0.012	0.211	
80%	8000	0.025	0.012	0.212	
85%	8500	0.028	0.013	0.212	
90%	9000	0.040	0.014	0.213	
95%	9500	0.047	0.015	0.214	
100%	10000	0.064	0.015	0.215	
0%	0	0.068	0.000	0.199	



Tension Load Test Result for PLT-20A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

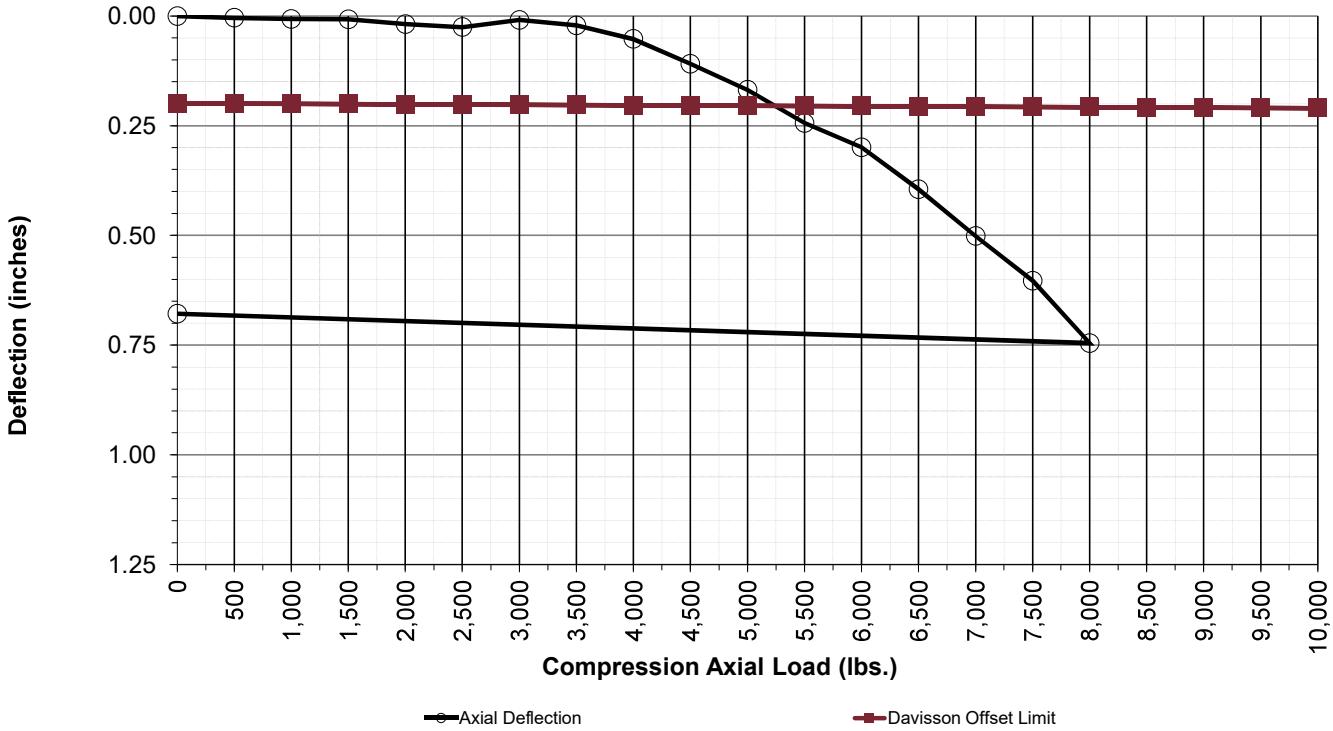
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-20A
Group Latitude [deg.]:	33.6492
Group Longitude[deg.]:	-114.6801
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	29

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.004	0.001	0.200	
10%	1000	0.007	0.001	0.200	
15%	1500	0.008	0.002	0.201	
20%	2000	0.019	0.002	0.201	
25%	2500	0.026	0.003	0.202	
30%	3000	0.009	0.003	0.202	
35%	3500	0.022	0.004	0.203	
40%	4000	0.053	0.004	0.203	
45%	4500	0.109	0.005	0.204	
50%	5000	0.169	0.005	0.205	
55%	5500	0.244	0.006	0.205	
60%	6000	0.299	0.006	0.206	
65%	6500	0.395	0.007	0.206	
70%	7000	0.502	0.008	0.207	
75%	7500	0.603	0.008	0.207	
80%	8000	0.745	0.009	0.208	
85%	8500		0.009	0.208	
90%	9000		0.010	0.209	
95%	9500		0.010	0.209	
100%	10000		0.011	0.210	
0%	0	0.678	0.000	0.199	



Tension Load Test Result for PLT-20B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

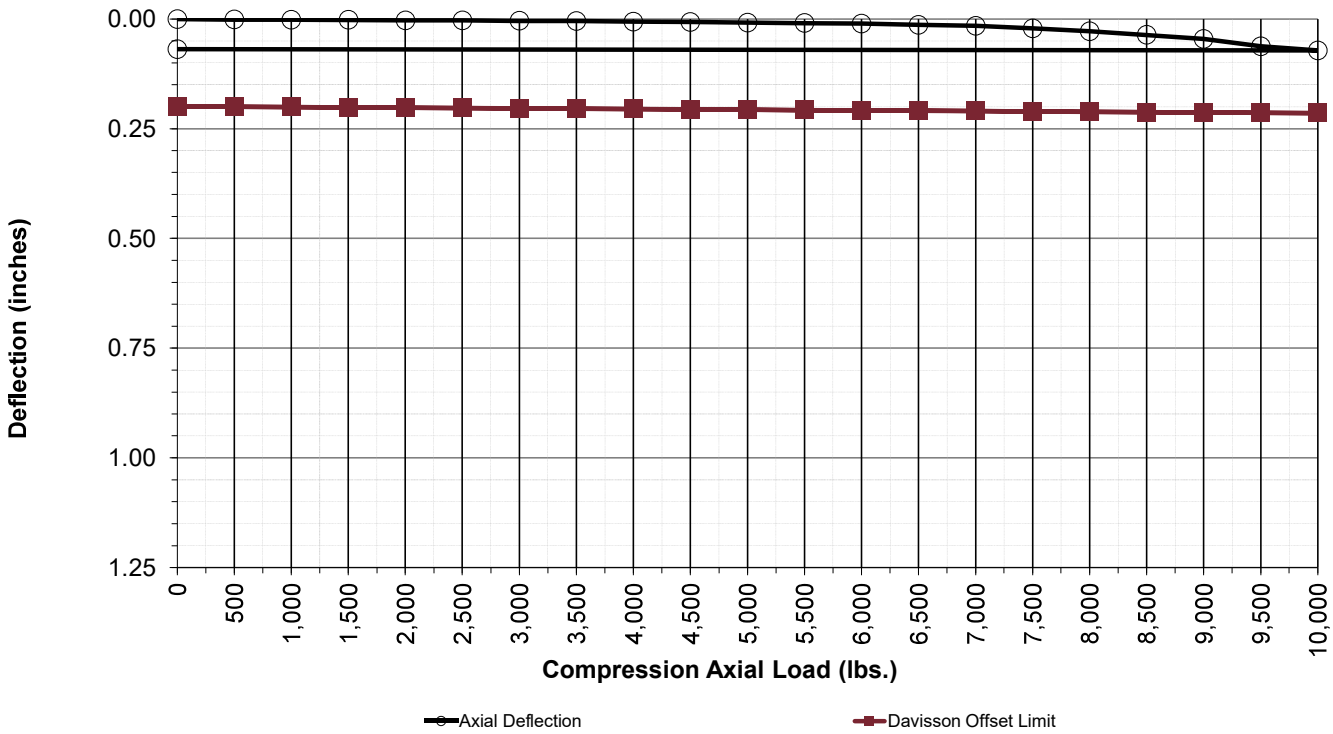
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-20B
Group Latitude [deg.]:	33.6492
Group Longitude[deg.]:	-114.6801
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	50

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.002	0.002	0.201	
15%	1500	0.002	0.002	0.201	
20%	2000	0.003	0.003	0.202	
25%	2500	0.004	0.004	0.203	
30%	3000	0.004	0.005	0.204	
35%	3500	0.005	0.005	0.205	
40%	4000	0.006	0.006	0.205	
45%	4500	0.007	0.007	0.206	
50%	5000	0.008	0.008	0.207	
55%	5500	0.010	0.008	0.208	
60%	6000	0.011	0.009	0.208	
65%	6500	0.014	0.010	0.209	
70%	7000	0.016	0.011	0.210	
75%	7500	0.022	0.012	0.211	
80%	8000	0.028	0.012	0.212	
85%	8500	0.037	0.013	0.212	
90%	9000	0.046	0.014	0.213	
95%	9500	0.063	0.015	0.214	
100%	10000	0.072	0.015	0.215	
0%	0	0.069	0.000	0.199	



Tension Load Test Result for PLT-21A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

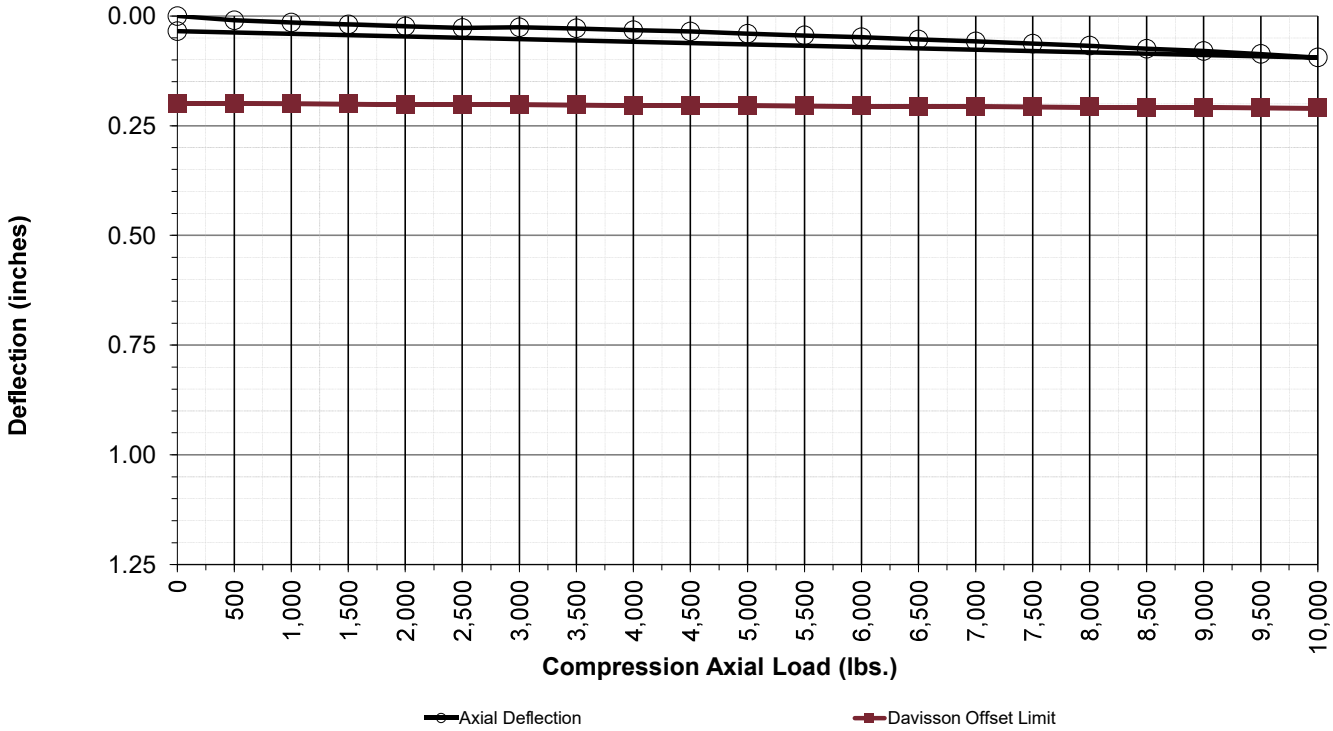
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-21A
Group Latitude [deg.]:	33.6492
Group Longitude[deg.]:	-114.6901
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	23

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.010	0.001	0.200	
10%	1000	0.015	0.001	0.200	
15%	1500	0.019	0.002	0.201	
20%	2000	0.024	0.002	0.201	
25%	2500	0.027	0.003	0.202	
30%	3000	0.026	0.003	0.202	
35%	3500	0.029	0.004	0.203	
40%	4000	0.033	0.004	0.203	
45%	4500	0.035	0.005	0.204	
50%	5000	0.041	0.005	0.205	
55%	5500	0.045	0.006	0.205	
60%	6000	0.048	0.006	0.206	
65%	6500	0.054	0.007	0.206	
70%	7000	0.058	0.008	0.207	
75%	7500	0.063	0.008	0.207	
80%	8000	0.068	0.009	0.208	
85%	8500	0.075	0.009	0.208	
90%	9000	0.080	0.010	0.209	
95%	9500	0.087	0.010	0.209	
100%	10000	0.095	0.011	0.210	
0%	0	0.035	0.000	0.199	



Tension Load Test Result for PLT-21B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

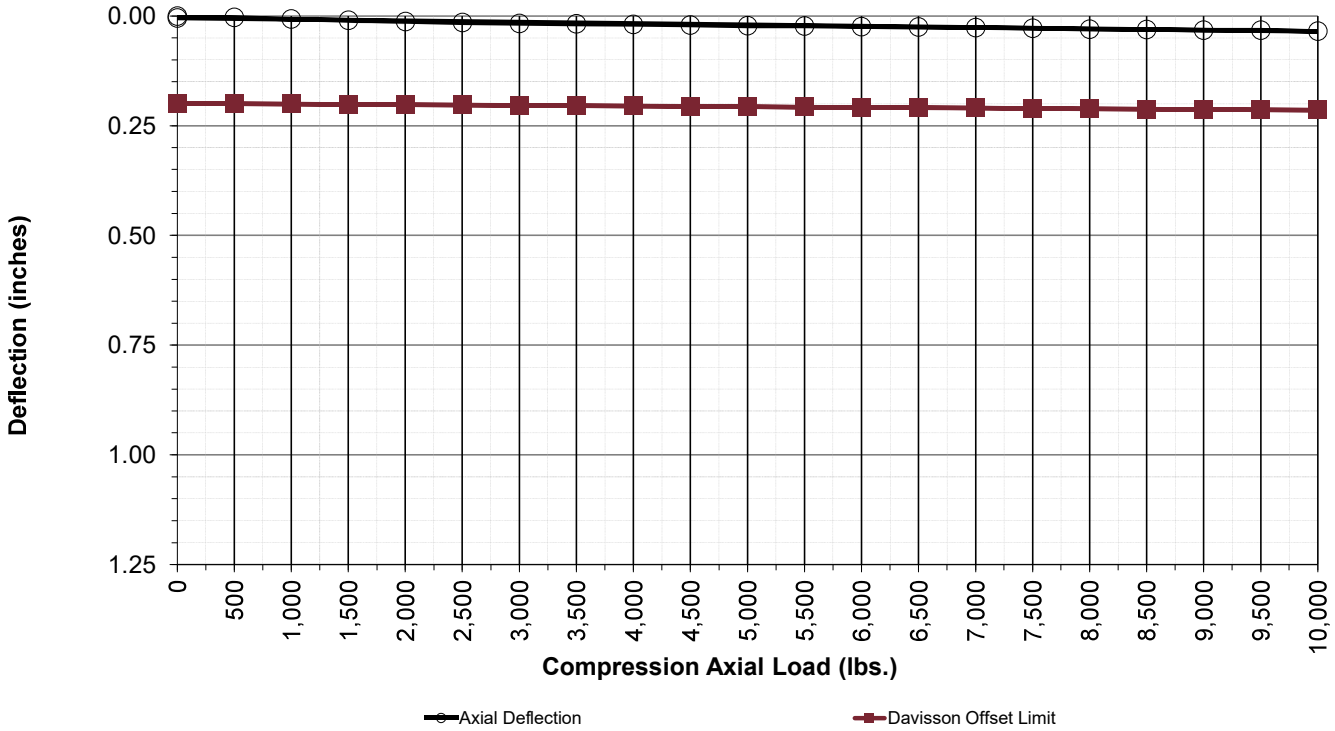
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-21B
Group Latitude [deg.]:	33.6492
Group Longitude[deg.]:	-114.6901
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	52

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.003	0.001	0.200	
10%	1000	0.007	0.002	0.201	
15%	1500	0.010	0.002	0.201	
20%	2000	0.013	0.003	0.202	
25%	2500	0.015	0.004	0.203	
30%	3000	0.017	0.005	0.204	
35%	3500	0.018	0.005	0.205	
40%	4000	0.020	0.006	0.205	
45%	4500	0.021	0.007	0.206	
50%	5000	0.023	0.008	0.207	
55%	5500	0.023	0.008	0.208	
60%	6000	0.024	0.009	0.208	
65%	6500	0.026	0.010	0.209	
70%	7000	0.027	0.011	0.210	
75%	7500	0.028	0.012	0.211	
80%	8000	0.030	0.012	0.212	
85%	8500	0.031	0.013	0.212	
90%	9000	0.032	0.014	0.213	
95%	9500	0.033	0.015	0.214	
100%	10000	0.035	0.015	0.215	
0%	0	0.005	0.000	0.199	



Tension Load Test Result for PLT-22A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

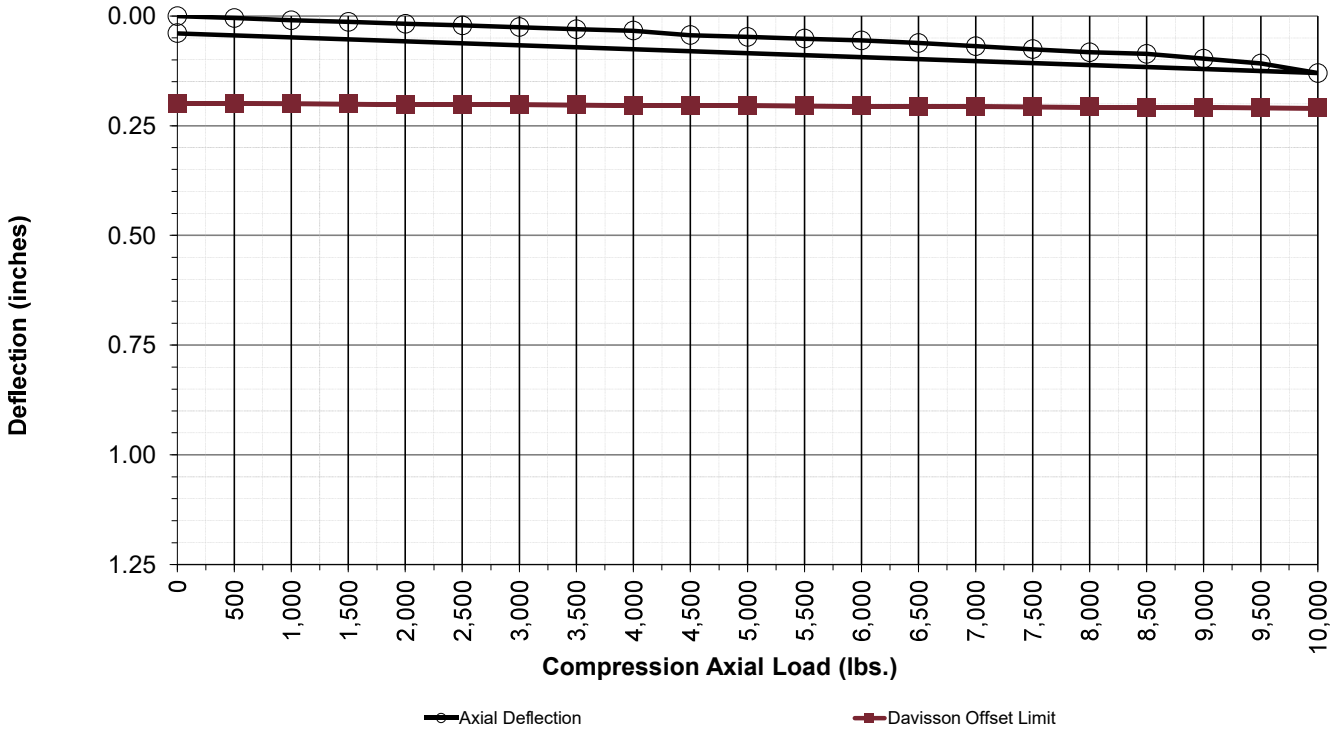
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-22A
Group Latitude [deg.]:	33.6455
Group Longitude[deg.]:	-114.7150
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	20

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.005	0.001	0.200	
10%	1000	0.010	0.001	0.200	
15%	1500	0.013	0.002	0.201	
20%	2000	0.018	0.002	0.201	
25%	2500	0.022	0.003	0.202	
30%	3000	0.026	0.003	0.202	
35%	3500	0.030	0.004	0.203	
40%	4000	0.034	0.004	0.203	
45%	4500	0.044	0.005	0.204	
50%	5000	0.048	0.005	0.205	
55%	5500	0.052	0.006	0.205	
60%	6000	0.056	0.006	0.206	
65%	6500	0.062	0.007	0.206	
70%	7000	0.069	0.008	0.207	
75%	7500	0.076	0.008	0.207	
80%	8000	0.083	0.009	0.208	
85%	8500	0.087	0.009	0.208	
90%	9000	0.098	0.010	0.209	
95%	9500	0.108	0.010	0.209	
100%	10000	0.130	0.011	0.210	
0%	0	0.040	0.000	0.199	



Tension Load Test Result for PLT-22B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

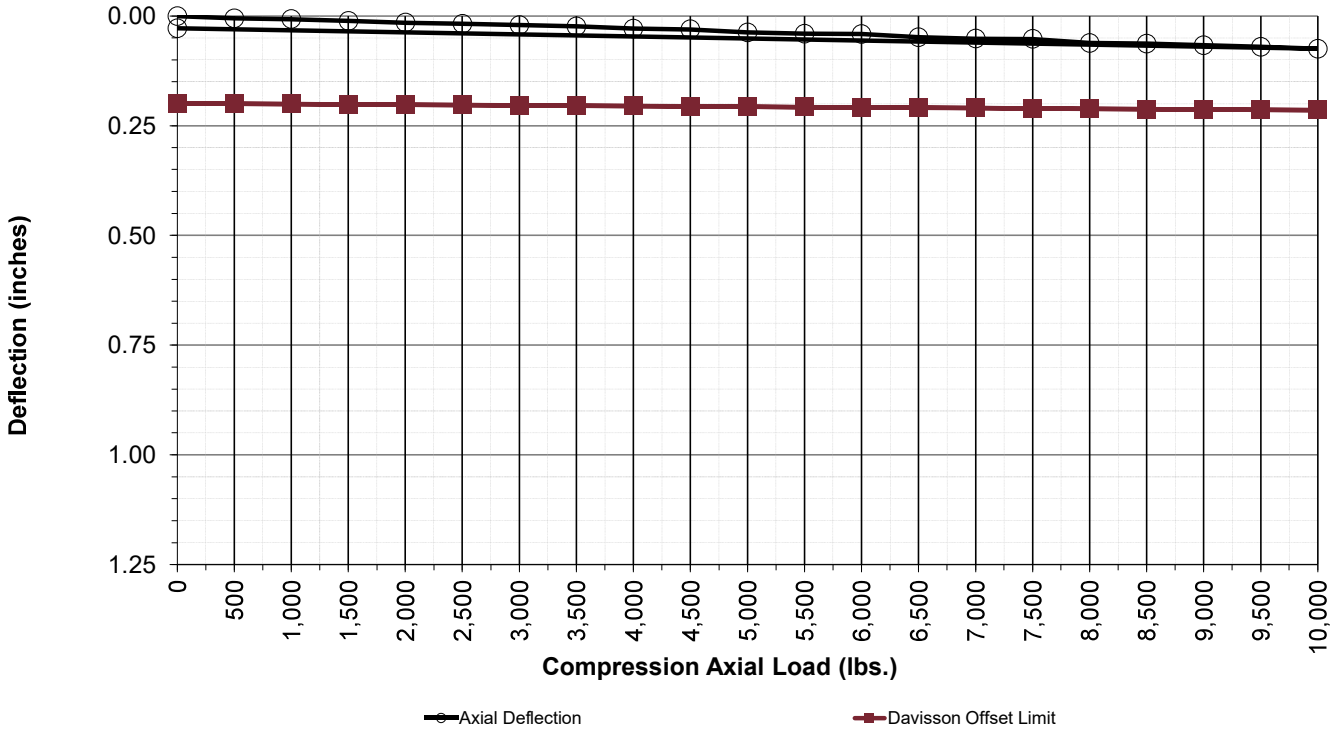
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-22B
Group Latitude [deg.]:	33.6455
Group Longitude[deg.]:	-114.7150
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	40

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.005	0.001	0.200	
10%	1000	0.008	0.002	0.201	
15%	1500	0.012	0.002	0.201	
20%	2000	0.016	0.003	0.202	
25%	2500	0.018	0.004	0.203	
30%	3000	0.021	0.005	0.204	
35%	3500	0.024	0.005	0.205	
40%	4000	0.029	0.006	0.205	
45%	4500	0.031	0.007	0.206	
50%	5000	0.038	0.008	0.207	
55%	5500	0.041	0.008	0.208	
60%	6000	0.041	0.009	0.208	
65%	6500	0.049	0.010	0.209	
70%	7000	0.052	0.011	0.210	
75%	7500	0.053	0.012	0.211	
80%	8000	0.062	0.012	0.212	
85%	8500	0.063	0.013	0.212	
90%	9000	0.067	0.014	0.213	
95%	9500	0.071	0.015	0.214	
100%	10000	0.075	0.015	0.215	
0%	0	0.028	0.000	0.199	



Tension Load Test Result for PLT-23A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

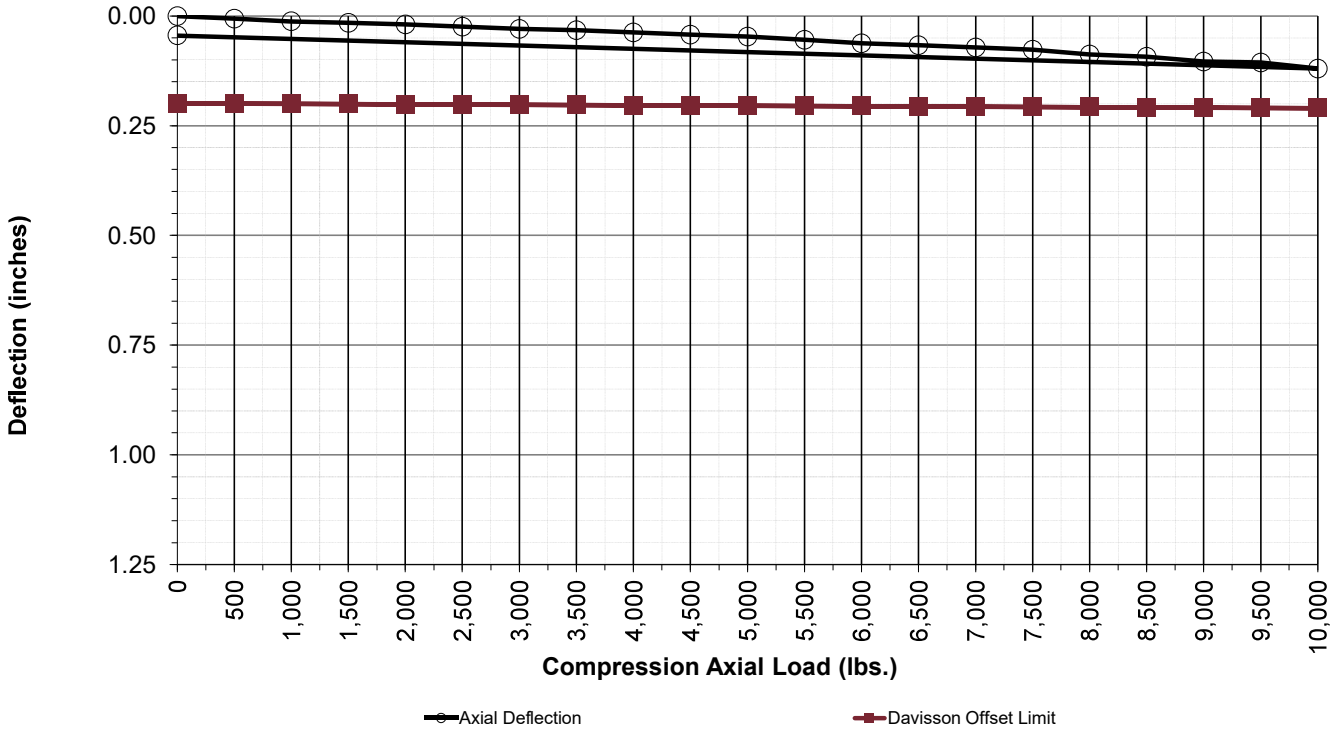
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/3/2023

Pile Information

Pile ID:	PLT-23A
Group Latitude [deg.]:	33.6383
Group Longitude[deg.]:	-114.6951
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	22

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.006	0.001	0.200	
10%	1000	0.012	0.001	0.200	
15%	1500	0.016	0.002	0.201	
20%	2000	0.020	0.002	0.201	
25%	2500	0.024	0.003	0.202	
30%	3000	0.029	0.003	0.202	
35%	3500	0.033	0.004	0.203	
40%	4000	0.038	0.004	0.203	
45%	4500	0.042	0.005	0.204	
50%	5000	0.047	0.005	0.205	
55%	5500	0.054	0.006	0.205	
60%	6000	0.062	0.006	0.206	
65%	6500	0.067	0.007	0.206	
70%	7000	0.072	0.008	0.207	
75%	7500	0.077	0.008	0.207	
80%	8000	0.088	0.009	0.208	
85%	8500	0.093	0.009	0.208	
90%	9000	0.104	0.010	0.209	
95%	9500	0.106	0.010	0.209	
100%	10000	0.120	0.011	0.210	
0%	0	0.045	0.000	0.199	



Tension Load Test Result for PLT-23B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

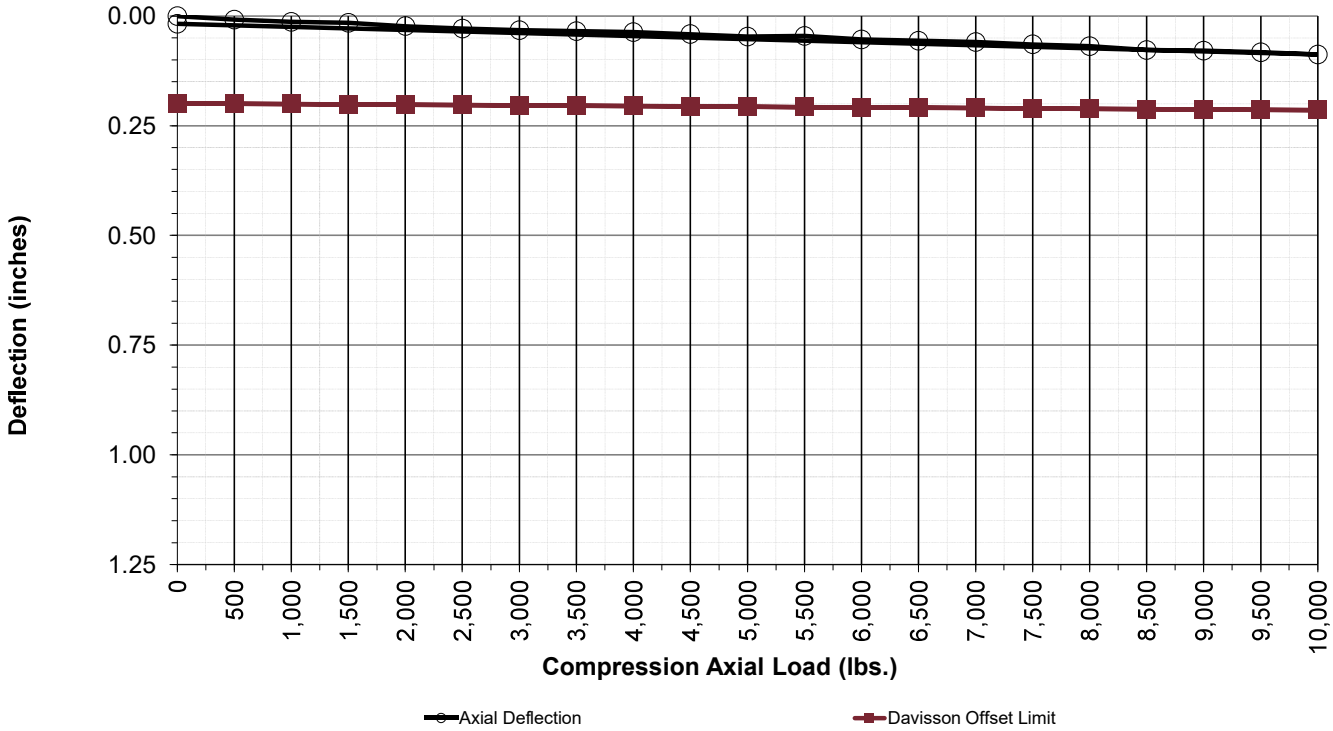
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/3/2023

Pile Information

Pile ID:	PLT-23B
Group Latitude [deg.]:	33.6383
Group Longitude[deg.]:	-114.6951
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	66

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.009	0.001	0.200	
10%	1000	0.013	0.002	0.201	
15%	1500	0.016	0.002	0.201	
20%	2000	0.024	0.003	0.202	
25%	2500	0.029	0.004	0.203	
30%	3000	0.033	0.005	0.204	
35%	3500	0.035	0.005	0.205	
40%	4000	0.037	0.006	0.205	
45%	4500	0.042	0.007	0.206	
50%	5000	0.047	0.008	0.207	
55%	5500	0.046	0.008	0.208	
60%	6000	0.053	0.009	0.208	
65%	6500	0.057	0.010	0.209	
70%	7000	0.059	0.011	0.210	
75%	7500	0.065	0.012	0.211	
80%	8000	0.069	0.012	0.212	
85%	8500	0.078	0.013	0.212	
90%	9000	0.079	0.014	0.213	
95%	9500	0.083	0.015	0.214	
100%	10000	0.088	0.015	0.215	
0%	0	0.018	0.000	0.199	



Tension Load Test Result for PLT-24A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

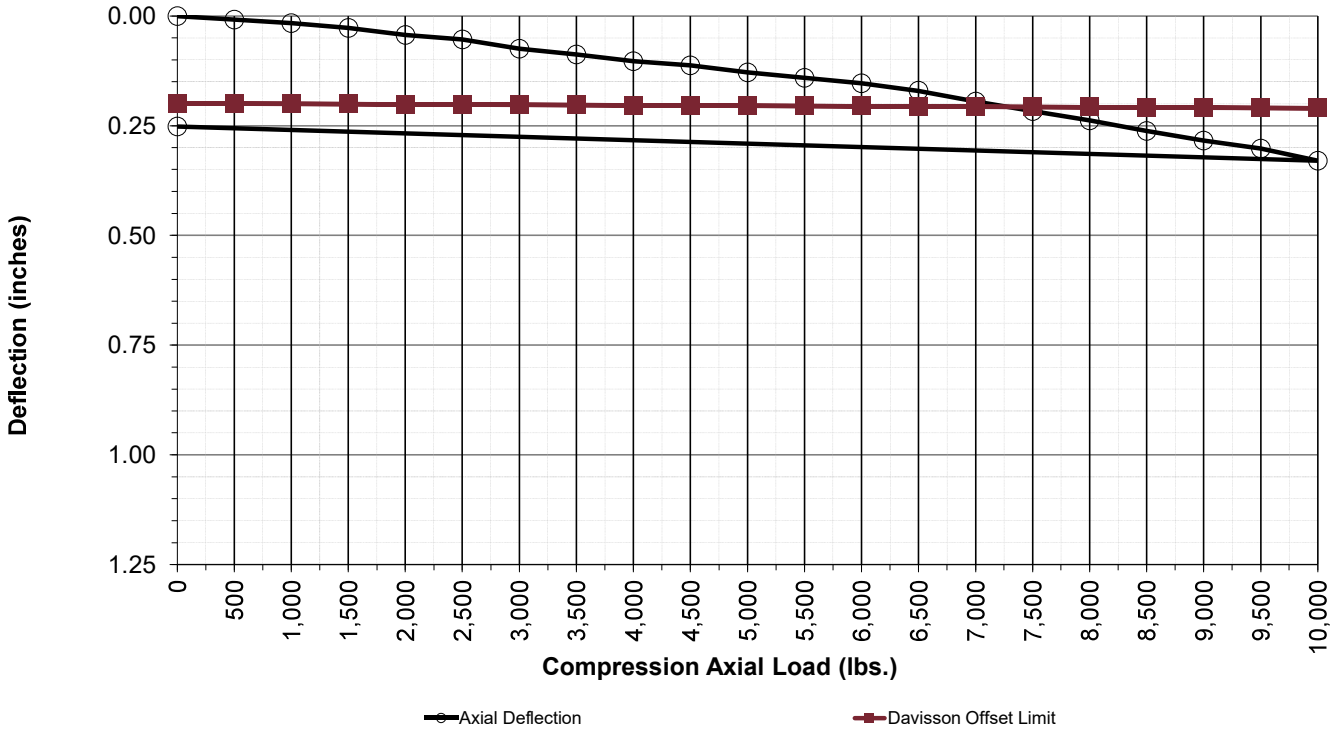
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-24A
Group Latitude [deg.]:	33.6328
Group Longitude[deg.]:	-114.6866
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	12

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.009	0.001	0.200	
10%	1000	0.017	0.001	0.200	
15%	1500	0.028	0.002	0.201	
20%	2000	0.044	0.002	0.201	
25%	2500	0.054	0.003	0.202	
30%	3000	0.075	0.003	0.202	
35%	3500	0.088	0.004	0.203	
40%	4000	0.104	0.004	0.203	
45%	4500	0.113	0.005	0.204	
50%	5000	0.129	0.005	0.205	
55%	5500	0.142	0.006	0.205	
60%	6000	0.154	0.006	0.206	
65%	6500	0.171	0.007	0.206	
70%	7000	0.195	0.008	0.207	
75%	7500	0.217	0.008	0.207	
80%	8000	0.238	0.009	0.208	
85%	8500	0.263	0.009	0.208	
90%	9000	0.284	0.010	0.209	
95%	9500	0.302	0.010	0.209	
100%	10000	0.330	0.011	0.210	
0%	0	0.252	0.000	0.199	



Tension Load Test Result for PLT-24B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

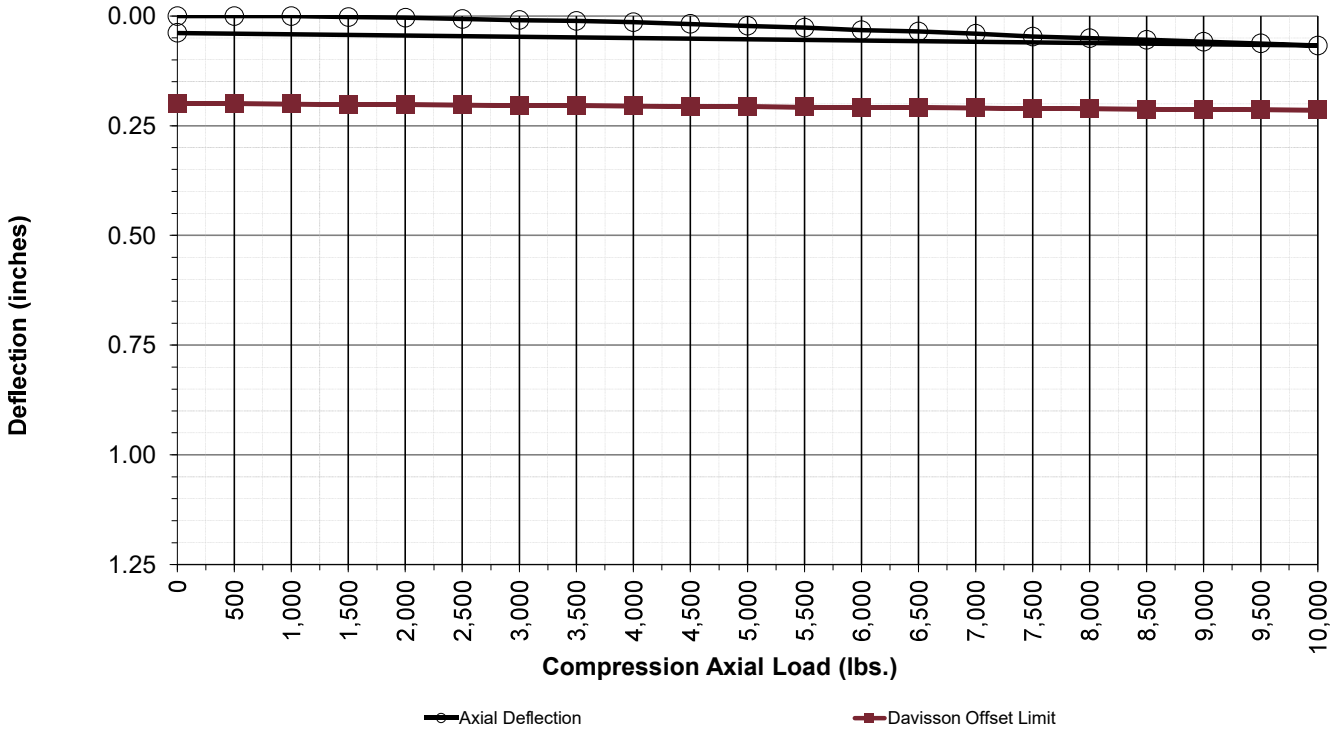
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-24B
Group Latitude [deg.]:	33.6328
Group Longitude[deg.]:	-114.6866
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	16

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.000	0.001	0.200	
10%	1000	0.001	0.002	0.201	
15%	1500	0.003	0.002	0.201	
20%	2000	0.004	0.003	0.202	
25%	2500	0.007	0.004	0.203	
30%	3000	0.010	0.005	0.204	
35%	3500	0.012	0.005	0.205	
40%	4000	0.014	0.006	0.205	
45%	4500	0.019	0.007	0.206	
50%	5000	0.023	0.008	0.207	
55%	5500	0.027	0.008	0.208	
60%	6000	0.033	0.009	0.208	
65%	6500	0.036	0.010	0.209	
70%	7000	0.041	0.011	0.210	
75%	7500	0.047	0.012	0.211	
80%	8000	0.051	0.012	0.212	
85%	8500	0.055	0.013	0.212	
90%	9000	0.059	0.014	0.213	
95%	9500	0.063	0.015	0.214	
100%	10000	0.068	0.015	0.215	
0%	0	0.039	0.000	0.199	



Tension Load Test Result for PLT-25A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

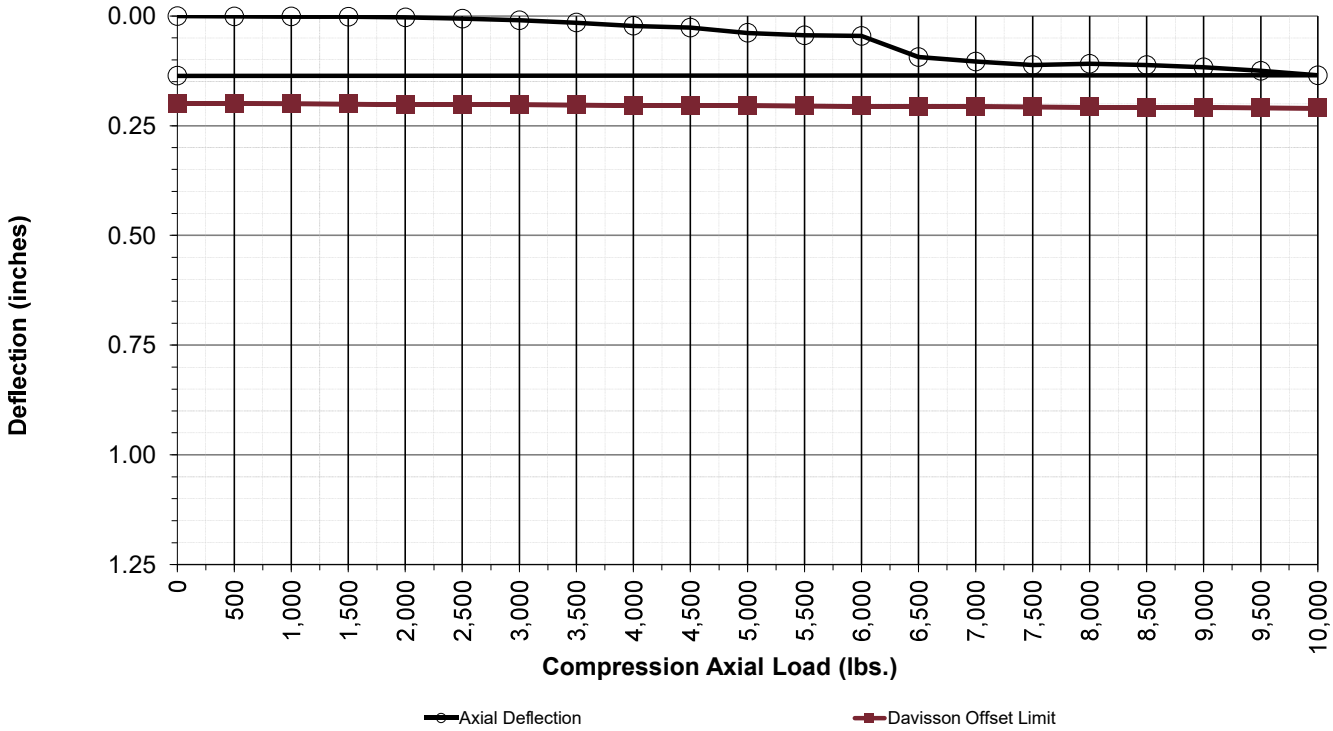
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-25A
Group Latitude [deg.]:	33.6371
Group Longitude[deg.]:	-114.6871
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	19

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.002	0.001	0.200	
15%	1500	0.002	0.002	0.201	
20%	2000	0.004	0.002	0.201	
25%	2500	0.006	0.003	0.202	
30%	3000	0.010	0.003	0.202	
35%	3500	0.015	0.004	0.203	
40%	4000	0.023	0.004	0.203	
45%	4500	0.027	0.005	0.204	
50%	5000	0.039	0.005	0.205	
55%	5500	0.044	0.006	0.205	
60%	6000	0.046	0.006	0.206	
65%	6500	0.094	0.007	0.206	
70%	7000	0.104	0.008	0.207	
75%	7500	0.112	0.008	0.207	
80%	8000	0.109	0.009	0.208	
85%	8500	0.112	0.009	0.208	
90%	9000	0.117	0.010	0.209	
95%	9500	0.125	0.010	0.209	
100%	10000	0.136	0.011	0.210	
0%	0	0.137	0.000	0.199	



Tension Load Test Result for PLT-25B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

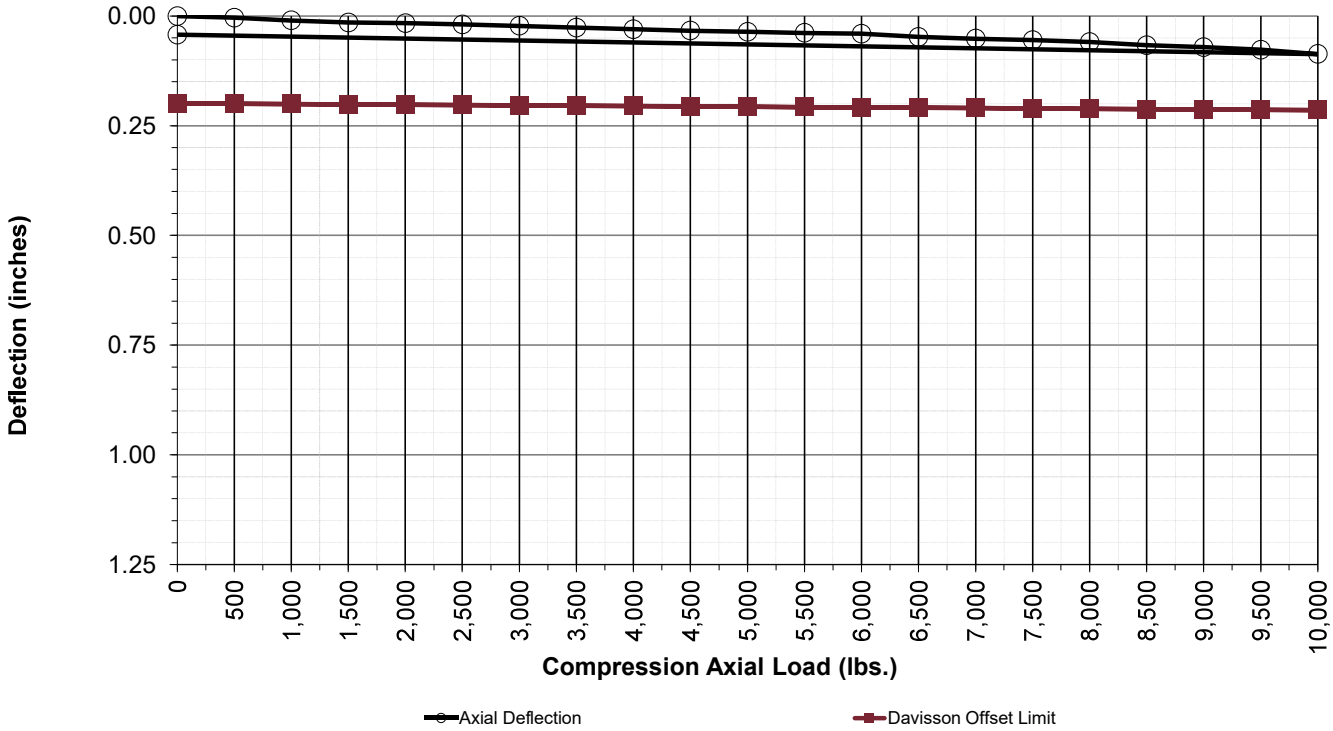
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-25B
Group Latitude [deg.]:	33.6371
Group Longitude[deg.]:	-114.6871
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	44

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.004	0.001	0.200	
10%	1000	0.010	0.002	0.201	
15%	1500	0.015	0.002	0.201	
20%	2000	0.017	0.003	0.202	
25%	2500	0.019	0.004	0.203	
30%	3000	0.023	0.005	0.204	
35%	3500	0.026	0.005	0.205	
40%	4000	0.030	0.006	0.205	
45%	4500	0.034	0.007	0.206	
50%	5000	0.036	0.008	0.207	
55%	5500	0.039	0.008	0.208	
60%	6000	0.041	0.009	0.208	
65%	6500	0.048	0.010	0.209	
70%	7000	0.052	0.011	0.210	
75%	7500	0.055	0.012	0.211	
80%	8000	0.059	0.012	0.212	
85%	8500	0.067	0.013	0.212	
90%	9000	0.071	0.014	0.213	
95%	9500	0.077	0.015	0.214	
100%	10000	0.087	0.015	0.215	
0%	0	0.042	0.000	0.199	



Tension Load Test Result for PLT-26A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

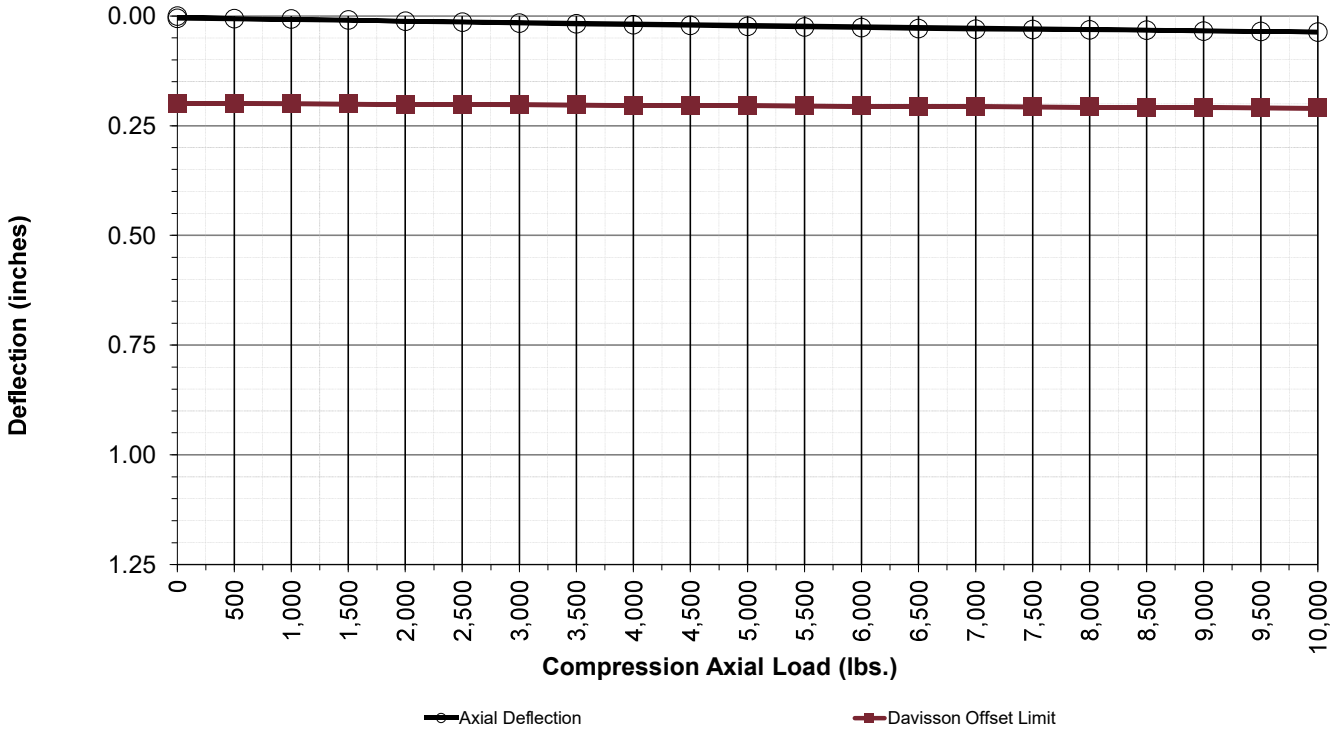
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-26A
Group Latitude [deg.]:	33.6346
Group Longitude[deg.]:	-114.6763
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	26

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.006	0.001	0.200	
10%	1000	0.007	0.001	0.200	
15%	1500	0.009	0.002	0.201	
20%	2000	0.012	0.002	0.201	
25%	2500	0.015	0.003	0.202	
30%	3000	0.016	0.003	0.202	
35%	3500	0.019	0.004	0.203	
40%	4000	0.020	0.004	0.203	
45%	4500	0.022	0.005	0.204	
50%	5000	0.024	0.005	0.205	
55%	5500	0.025	0.006	0.205	
60%	6000	0.027	0.006	0.206	
65%	6500	0.029	0.007	0.206	
70%	7000	0.030	0.008	0.207	
75%	7500	0.031	0.008	0.207	
80%	8000	0.032	0.009	0.208	
85%	8500	0.032	0.009	0.208	
90%	9000	0.035	0.010	0.209	
95%	9500	0.036	0.010	0.209	
100%	10000	0.037	0.011	0.210	
0%	0	0.006	0.000	0.199	



Tension Load Test Result for PLT-26B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

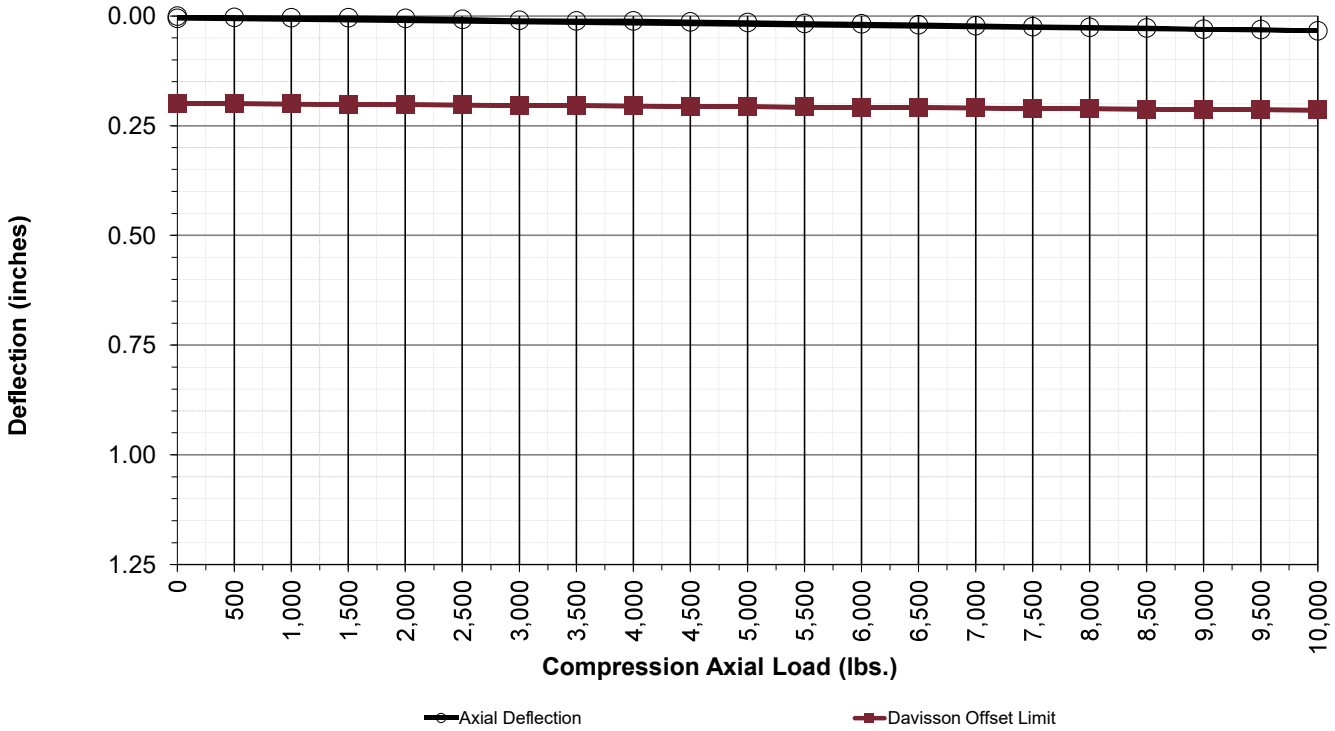
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-26B
Group Latitude [deg.]:	33.6346
Group Longitude[deg.]:	-114.6763
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	61

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.003	0.001	0.200	
10%	1000	0.004	0.002	0.201	
15%	1500	0.004	0.002	0.201	
20%	2000	0.006	0.003	0.202	
25%	2500	0.008	0.004	0.203	
30%	3000	0.010	0.005	0.204	
35%	3500	0.011	0.005	0.205	
40%	4000	0.012	0.006	0.205	
45%	4500	0.013	0.007	0.206	
50%	5000	0.015	0.008	0.207	
55%	5500	0.017	0.008	0.208	
60%	6000	0.018	0.009	0.208	
65%	6500	0.020	0.010	0.209	
70%	7000	0.022	0.011	0.210	
75%	7500	0.024	0.012	0.211	
80%	8000	0.026	0.012	0.212	
85%	8500	0.028	0.013	0.212	
90%	9000	0.030	0.014	0.213	
95%	9500	0.031	0.015	0.214	
100%	10000	0.034	0.015	0.215	
0%	0	0.005	0.000	0.199	



Tension Load Test Result for PLT-27A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

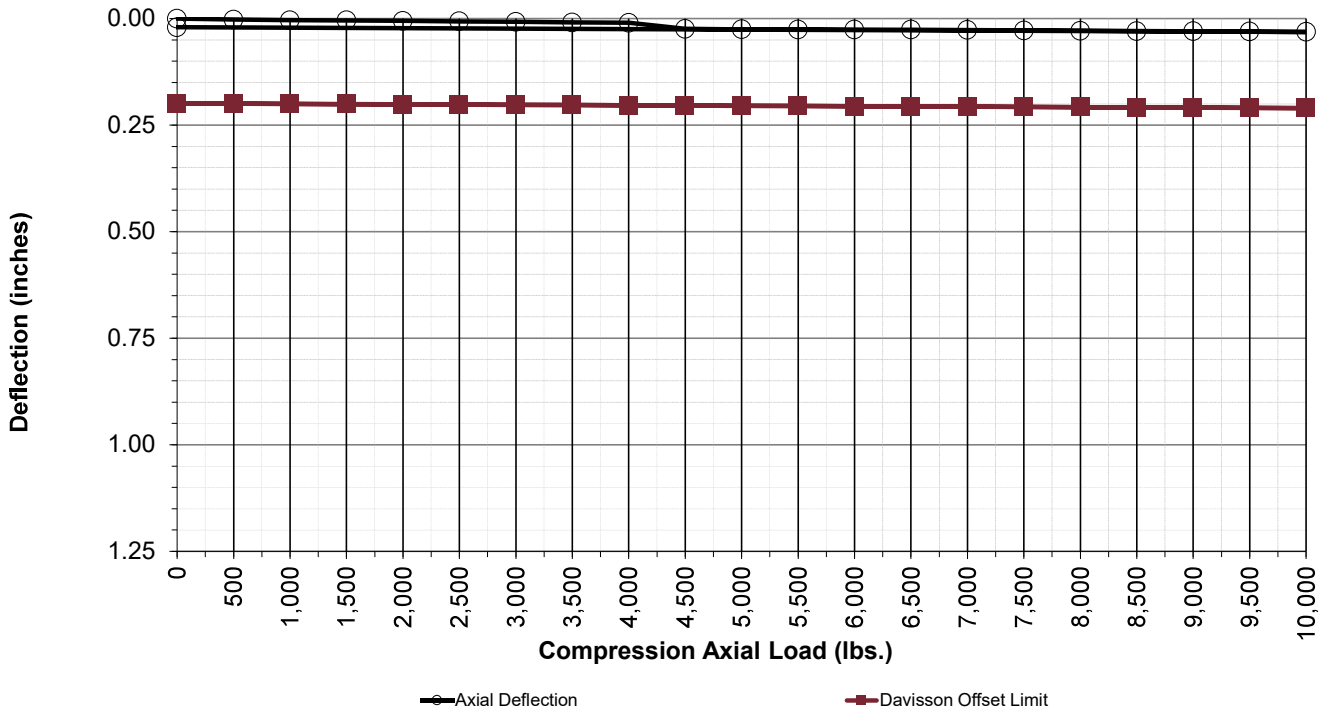
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-27A
Group Latitude [deg.]:	33.6398
Group Longitude[deg.]:	-114.6813
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	58

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.003	0.001	0.200	
10%	1000	0.004	0.001	0.200	
15%	1500	0.004	0.002	0.201	
20%	2000	0.005	0.002	0.201	
25%	2500	0.007	0.003	0.202	
30%	3000	0.008	0.003	0.202	
35%	3500	0.009	0.004	0.203	
40%	4000	0.010	0.004	0.203	
45%	4500	0.024	0.005	0.204	
50%	5000	0.025	0.005	0.205	
55%	5500	0.026	0.006	0.205	
60%	6000	0.026	0.006	0.206	
65%	6500	0.026	0.007	0.206	
70%	7000	0.027	0.008	0.207	
75%	7500	0.028	0.008	0.207	
80%	8000	0.029	0.009	0.208	
85%	8500	0.029	0.009	0.208	
90%	9000	0.030	0.010	0.209	
95%	9500	0.031	0.010	0.209	
100%	10000	0.031	0.011	0.210	
0%	0	0.020	0.000	0.199	



Tension Load Test Result for PLT-27B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

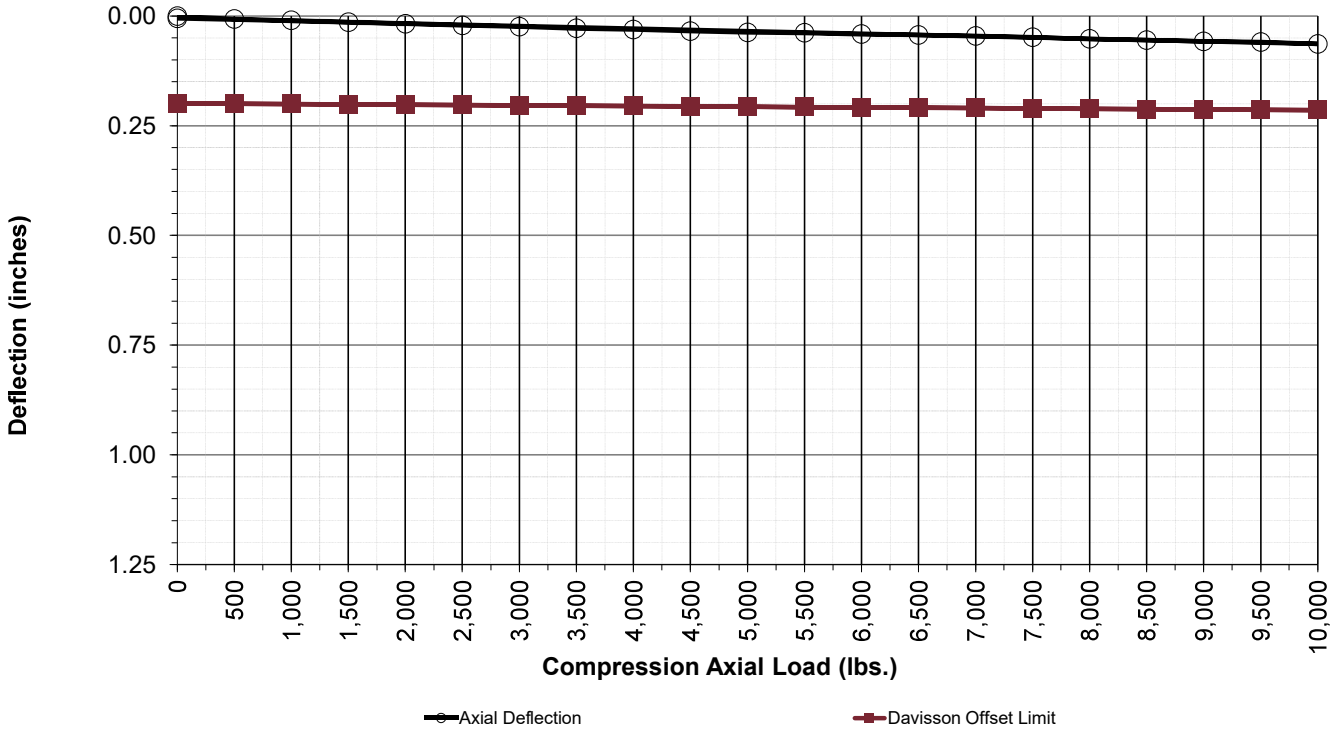
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-27B
Group Latitude [deg.]:	33.6398
Group Longitude[deg.]:	-114.6813
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	79

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.007	0.001	0.200	
10%	1000	0.011	0.002	0.201	
15%	1500	0.015	0.002	0.201	
20%	2000	0.018	0.003	0.202	
25%	2500	0.022	0.004	0.203	
30%	3000	0.025	0.005	0.204	
35%	3500	0.029	0.005	0.205	
40%	4000	0.031	0.006	0.205	
45%	4500	0.035	0.007	0.206	
50%	5000	0.038	0.008	0.207	
55%	5500	0.039	0.008	0.208	
60%	6000	0.042	0.009	0.208	
65%	6500	0.044	0.010	0.209	
70%	7000	0.046	0.011	0.210	
75%	7500	0.049	0.012	0.211	
80%	8000	0.053	0.012	0.212	
85%	8500	0.055	0.013	0.212	
90%	9000	0.058	0.014	0.213	
95%	9500	0.060	0.015	0.214	
100%	10000	0.064	0.015	0.215	
0%	0	0.006	0.000	0.199	



Tension Load Test Result for PLT-28A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

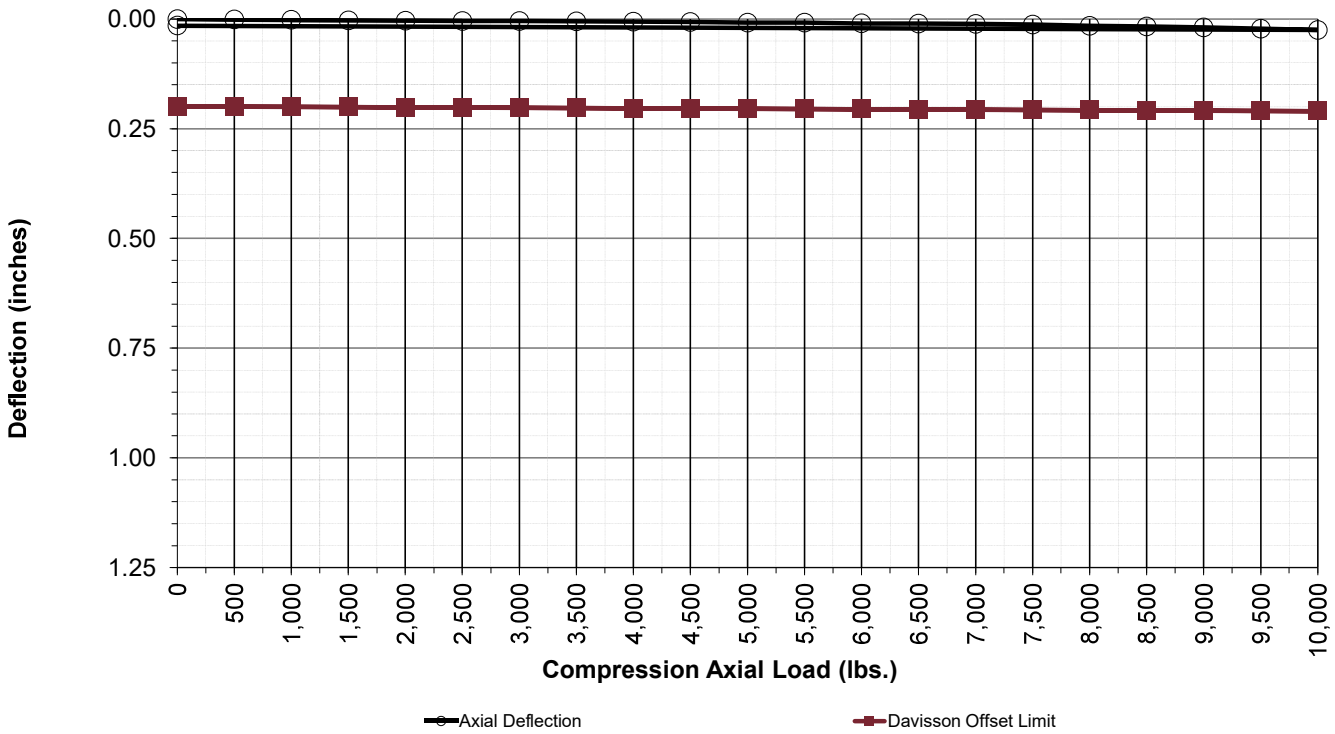
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-28A
Group Latitude [deg.]:	33.6455
Group Longitude[deg.]:	-114.6798
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	11

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.003	0.001	0.200	
15%	1500	0.003	0.002	0.201	
20%	2000	0.004	0.002	0.201	
25%	2500	0.005	0.003	0.202	
30%	3000	0.005	0.003	0.202	
35%	3500	0.006	0.004	0.203	
40%	4000	0.006	0.004	0.203	
45%	4500	0.007	0.005	0.204	
50%	5000	0.008	0.005	0.205	
55%	5500	0.009	0.006	0.205	
60%	6000	0.010	0.006	0.206	
65%	6500	0.011	0.007	0.206	
70%	7000	0.012	0.008	0.207	
75%	7500	0.013	0.008	0.207	
80%	8000	0.016	0.009	0.208	
85%	8500	0.017	0.009	0.208	
90%	9000	0.020	0.010	0.209	
95%	9500	0.022	0.010	0.209	
100%	10000	0.025	0.011	0.210	
0%	0	0.016	0.000	0.199	



Tension Load Test Result for PLT-28B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

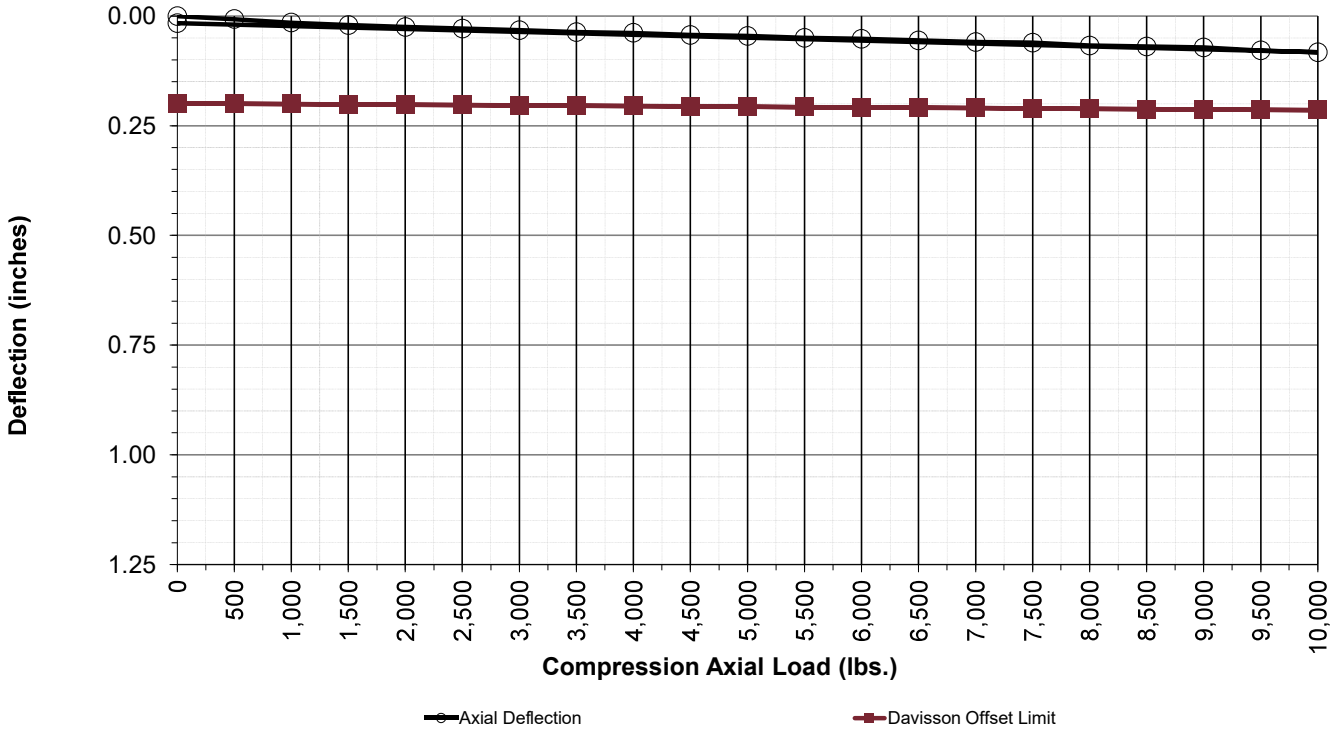
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-28B
Group Latitude [deg.]:	33.6455
Group Longitude[deg.]:	-114.6798
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	38

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.008	0.001	0.200	
10%	1000	0.016	0.002	0.201	
15%	1500	0.021	0.002	0.201	
20%	2000	0.025	0.003	0.202	
25%	2500	0.029	0.004	0.203	
30%	3000	0.033	0.005	0.204	
35%	3500	0.037	0.005	0.205	
40%	4000	0.039	0.006	0.205	
45%	4500	0.043	0.007	0.206	
50%	5000	0.046	0.008	0.207	
55%	5500	0.050	0.008	0.208	
60%	6000	0.052	0.009	0.208	
65%	6500	0.056	0.010	0.209	
70%	7000	0.059	0.011	0.210	
75%	7500	0.061	0.012	0.211	
80%	8000	0.068	0.012	0.212	
85%	8500	0.070	0.013	0.212	
90%	9000	0.072	0.014	0.213	
95%	9500	0.079	0.015	0.214	
100%	10000	0.083	0.015	0.215	
0%	0	0.016	0.000	0.199	



Tension Load Test Result for PLT-29A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

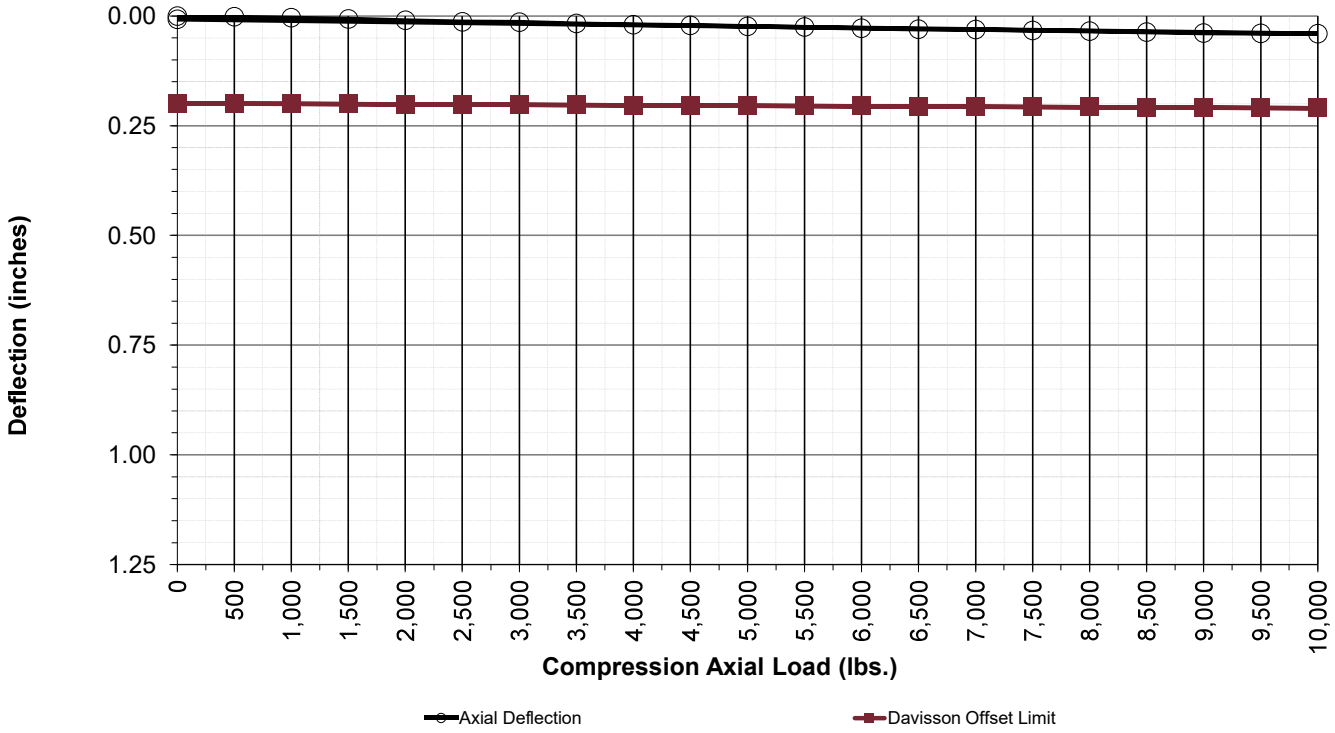
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-29A
Group Latitude [deg.]:	33.6455
Group Longitude[deg.]:	-114.6699
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	16

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.002	0.001	0.200	
10%	1000	0.005	0.001	0.200	
15%	1500	0.007	0.002	0.201	
20%	2000	0.010	0.002	0.201	
25%	2500	0.013	0.003	0.202	
30%	3000	0.015	0.003	0.202	
35%	3500	0.018	0.004	0.203	
40%	4000	0.020	0.004	0.203	
45%	4500	0.022	0.005	0.204	
50%	5000	0.024	0.005	0.205	
55%	5500	0.026	0.006	0.205	
60%	6000	0.028	0.006	0.206	
65%	6500	0.030	0.007	0.206	
70%	7000	0.031	0.008	0.207	
75%	7500	0.033	0.008	0.207	
80%	8000	0.035	0.009	0.208	
85%	8500	0.037	0.009	0.208	
90%	9000	0.039	0.010	0.209	
95%	9500	0.040	0.010	0.209	
100%	10000	0.041	0.011	0.210	
0%	0	0.008	0.000	0.199	



Tension Load Test Result for PLT-29B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

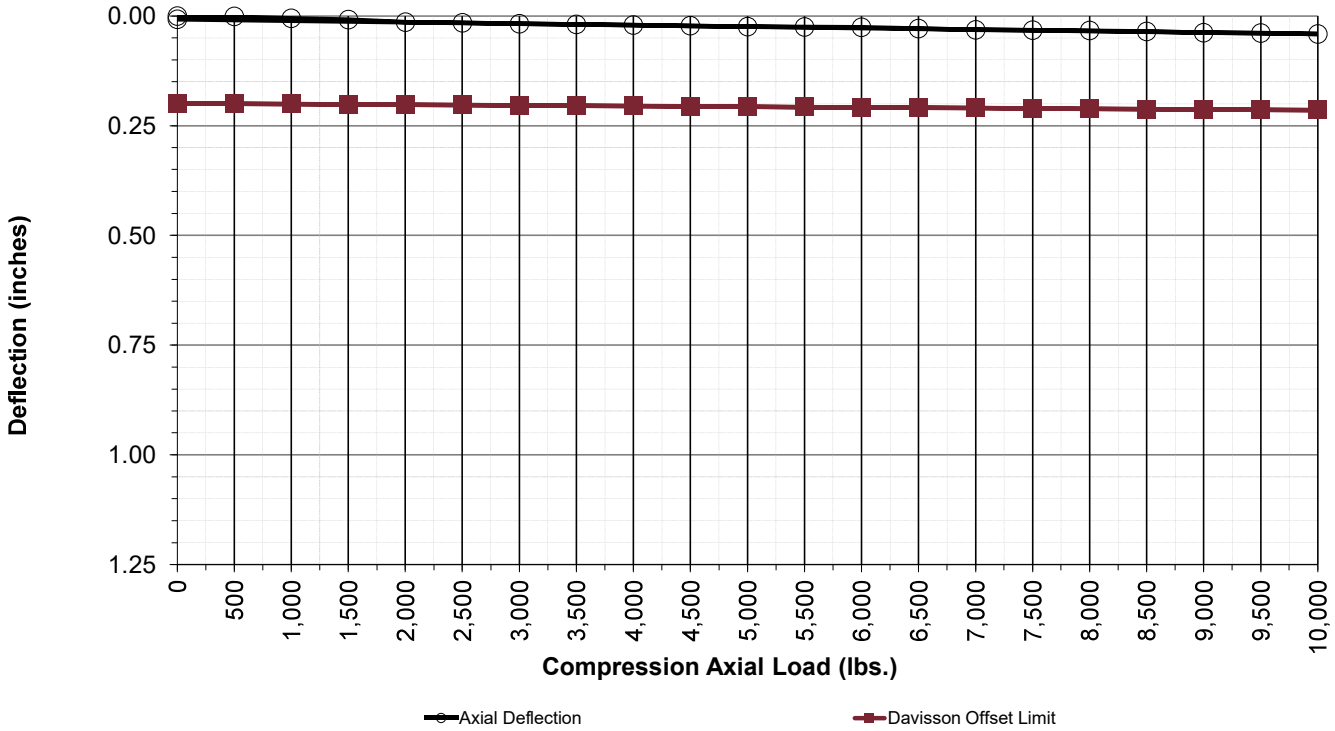
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-29B
Group Latitude [deg.]:	33.6455
Group Longitude[deg.]:	-114.6699
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	58

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.002	0.001	0.200	
10%	1000	0.006	0.002	0.201	
15%	1500	0.008	0.002	0.201	
20%	2000	0.014	0.003	0.202	
25%	2500	0.016	0.004	0.203	
30%	3000	0.018	0.005	0.204	
35%	3500	0.020	0.005	0.205	
40%	4000	0.021	0.006	0.205	
45%	4500	0.023	0.007	0.206	
50%	5000	0.025	0.008	0.207	
55%	5500	0.025	0.008	0.208	
60%	6000	0.026	0.009	0.208	
65%	6500	0.029	0.010	0.209	
70%	7000	0.032	0.011	0.210	
75%	7500	0.032	0.012	0.211	
80%	8000	0.034	0.012	0.212	
85%	8500	0.036	0.013	0.212	
90%	9000	0.038	0.014	0.213	
95%	9500	0.039	0.015	0.214	
100%	10000	0.042	0.015	0.215	
0%	0	0.008	0.000	0.199	



Tension Load Test Result for PLT-30A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

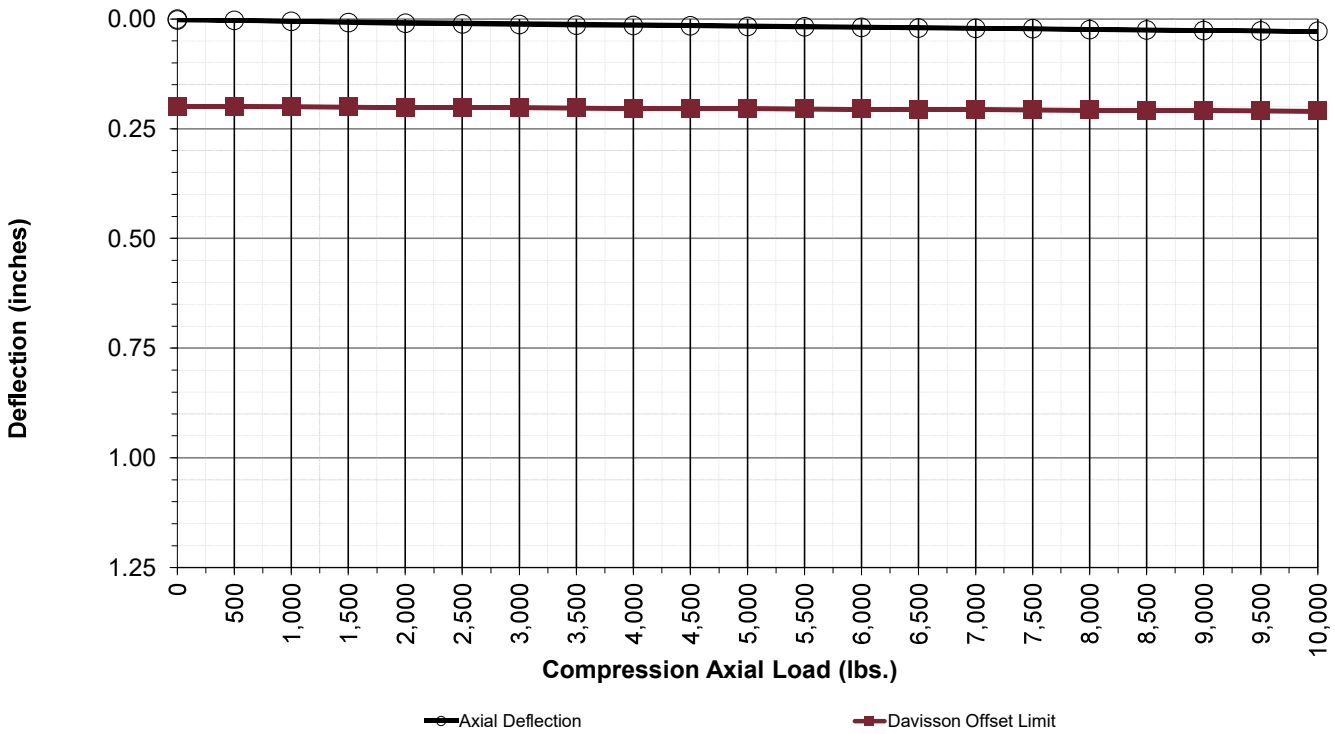
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-30A
Group Latitude [deg.]:	33.6435
Group Longitude[deg.]:	-114.6650
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	20

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.003	0.001	0.200	
10%	1000	0.006	0.001	0.200	
15%	1500	0.008	0.002	0.201	
20%	2000	0.010	0.002	0.201	
25%	2500	0.012	0.003	0.202	
30%	3000	0.013	0.003	0.202	
35%	3500	0.014	0.004	0.203	
40%	4000	0.015	0.004	0.203	
45%	4500	0.016	0.005	0.204	
50%	5000	0.017	0.005	0.205	
55%	5500	0.018	0.006	0.205	
60%	6000	0.020	0.006	0.206	
65%	6500	0.021	0.007	0.206	
70%	7000	0.022	0.008	0.207	
75%	7500	0.023	0.008	0.207	
80%	8000	0.025	0.009	0.208	
85%	8500	0.026	0.009	0.208	
90%	9000	0.027	0.010	0.209	
95%	9500	0.028	0.010	0.209	
100%	10000	0.029	0.011	0.210	
0%	0	0.003	0.000	0.199	



Tension Load Test Result for PLT-30B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

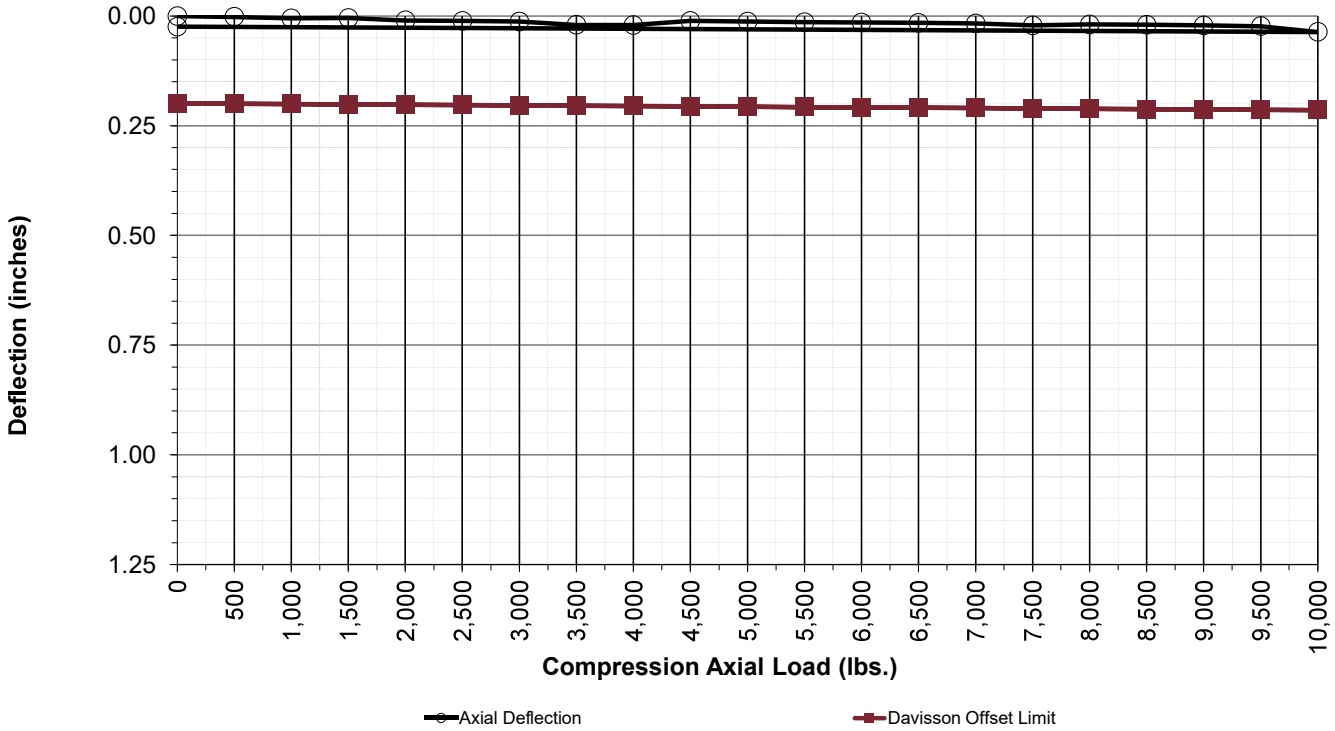
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-30B
Group Latitude [deg.]:	33.6435
Group Longitude[deg.]:	-114.6650
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	60

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.003	0.001	0.200	
10%	1000	0.006	0.002	0.201	
15%	1500	0.005	0.002	0.201	
20%	2000	0.010	0.003	0.202	
25%	2500	0.011	0.004	0.203	
30%	3000	0.013	0.005	0.204	
35%	3500	0.021	0.005	0.205	
40%	4000	0.021	0.006	0.205	
45%	4500	0.012	0.007	0.206	
50%	5000	0.013	0.008	0.207	
55%	5500	0.014	0.008	0.208	
60%	6000	0.015	0.009	0.208	
65%	6500	0.016	0.010	0.209	
70%	7000	0.017	0.011	0.210	
75%	7500	0.021	0.012	0.211	
80%	8000	0.019	0.012	0.212	
85%	8500	0.020	0.013	0.212	
90%	9000	0.021	0.014	0.213	
95%	9500	0.023	0.015	0.214	
100%	10000	0.037	0.015	0.215	
0%	0	0.024	0.000	0.199	



Tension Load Test Result for PLT-31A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

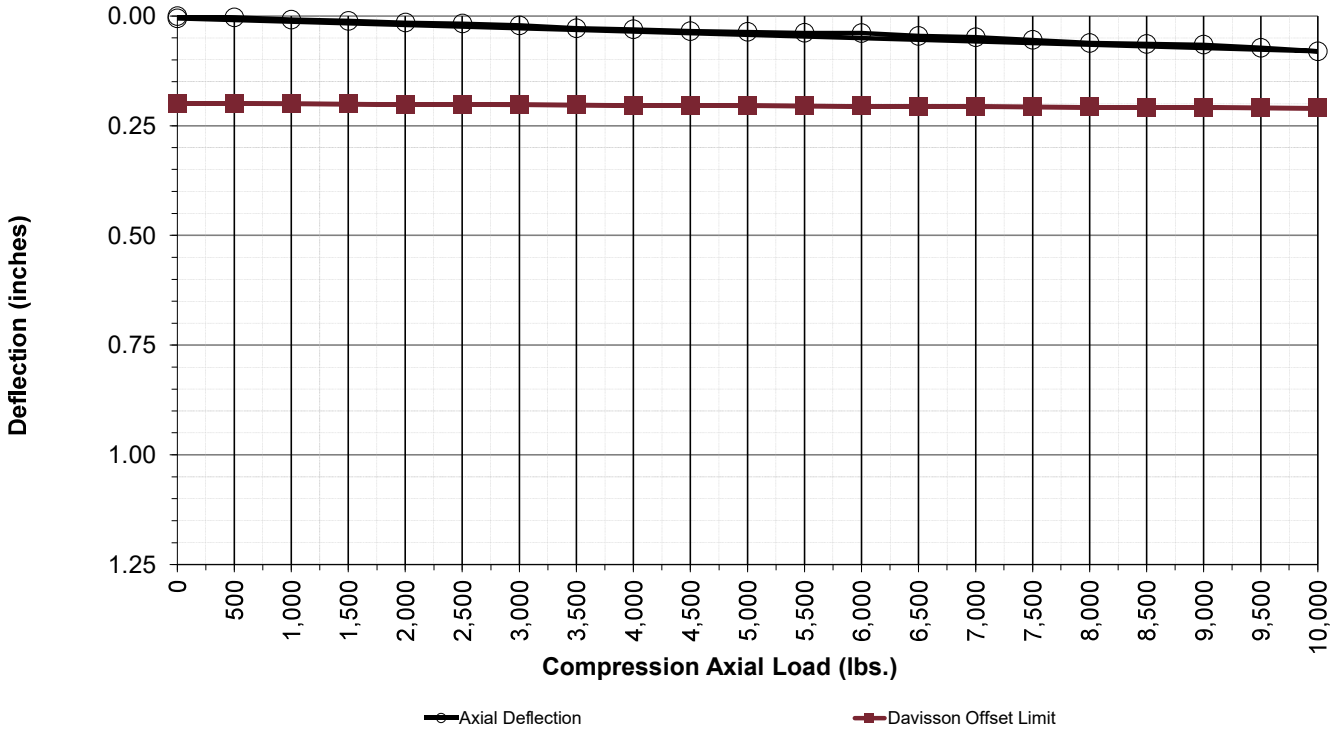
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-31A
Group Latitude [deg.]:	33.6465
Group Longitude[deg.]:	-114.6614
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	36

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.003	0.001	0.200	
10%	1000	0.008	0.001	0.200	
15%	1500	0.012	0.002	0.201	
20%	2000	0.016	0.002	0.201	
25%	2500	0.018	0.003	0.202	
30%	3000	0.021	0.003	0.202	
35%	3500	0.028	0.004	0.203	
40%	4000	0.031	0.004	0.203	
45%	4500	0.035	0.005	0.204	
50%	5000	0.037	0.005	0.205	
55%	5500	0.039	0.006	0.205	
60%	6000	0.039	0.006	0.206	
65%	6500	0.045	0.007	0.206	
70%	7000	0.049	0.008	0.207	
75%	7500	0.055	0.008	0.207	
80%	8000	0.062	0.009	0.208	
85%	8500	0.064	0.009	0.208	
90%	9000	0.066	0.010	0.209	
95%	9500	0.073	0.010	0.209	
100%	10000	0.081	0.011	0.210	
0%	0	0.005	0.000	0.199	



Tension Load Test Result for PLT-31B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

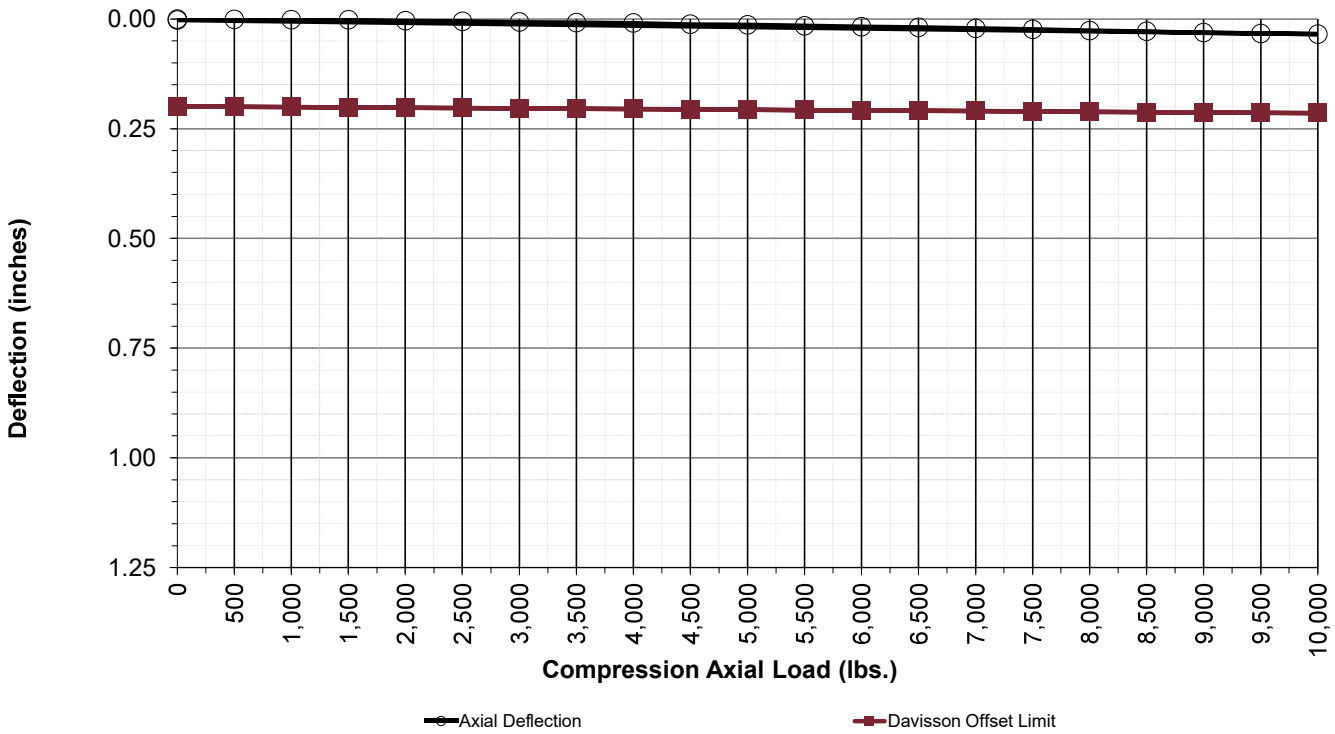
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-31B
Group Latitude [deg.]:	33.6465
Group Longitude[deg.]:	-114.6614
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	70

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.002	0.002	0.201	
15%	1500	0.003	0.002	0.201	
20%	2000	0.004	0.003	0.202	
25%	2500	0.006	0.004	0.203	
30%	3000	0.007	0.005	0.204	
35%	3500	0.008	0.005	0.205	
40%	4000	0.010	0.006	0.205	
45%	4500	0.012	0.007	0.206	
50%	5000	0.014	0.008	0.207	
55%	5500	0.016	0.008	0.208	
60%	6000	0.018	0.009	0.208	
65%	6500	0.020	0.010	0.209	
70%	7000	0.022	0.011	0.210	
75%	7500	0.024	0.012	0.211	
80%	8000	0.027	0.012	0.212	
85%	8500	0.029	0.013	0.212	
90%	9000	0.032	0.014	0.213	
95%	9500	0.033	0.015	0.214	
100%	10000	0.034	0.015	0.215	
0%	0	0.003	0.000	0.199	



Tension Load Test Result for PLT-32A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

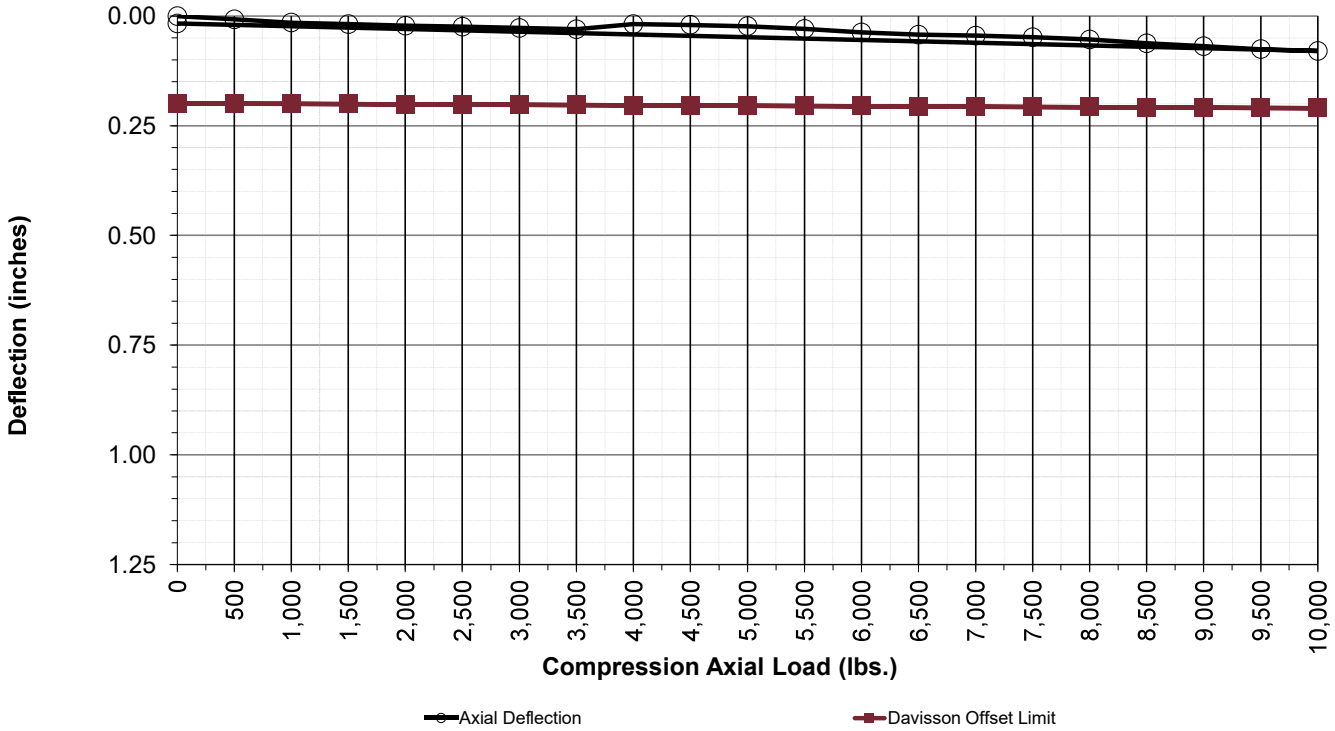
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-32A
Group Latitude [deg.]:	33.6329
Group Longitude[deg.]:	-114.6950
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	32

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.008	0.001	0.200	
10%	1000	0.016	0.001	0.200	
15%	1500	0.019	0.002	0.201	
20%	2000	0.022	0.002	0.201	
25%	2500	0.025	0.003	0.202	
30%	3000	0.028	0.003	0.202	
35%	3500	0.030	0.004	0.203	
40%	4000	0.019	0.004	0.203	
45%	4500	0.021	0.005	0.204	
50%	5000	0.024	0.005	0.205	
55%	5500	0.030	0.006	0.205	
60%	6000	0.038	0.006	0.206	
65%	6500	0.043	0.007	0.206	
70%	7000	0.045	0.008	0.207	
75%	7500	0.049	0.008	0.207	
80%	8000	0.054	0.009	0.208	
85%	8500	0.062	0.009	0.208	
90%	9000	0.069	0.010	0.209	
95%	9500	0.076	0.010	0.209	
100%	10000	0.080	0.011	0.210	
0%	0	0.017	0.000	0.199	



Tension Load Test Result for PLT-32B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

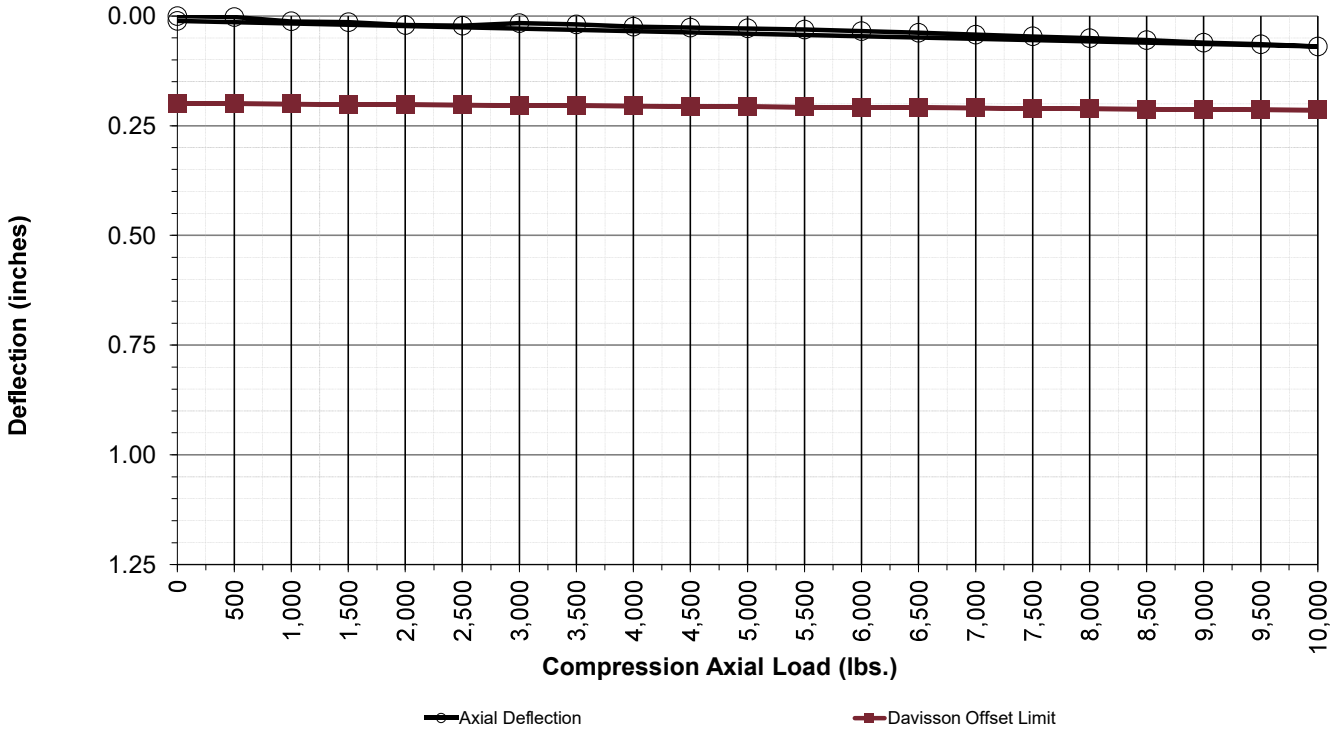
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-32B
Group Latitude [deg.]:	33.6329
Group Longitude[deg.]:	-114.6950
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	41

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.003	0.001	0.200	
10%	1000	0.013	0.002	0.201	
15%	1500	0.014	0.002	0.201	
20%	2000	0.021	0.003	0.202	
25%	2500	0.022	0.004	0.203	
30%	3000	0.017	0.005	0.204	
35%	3500	0.020	0.005	0.205	
40%	4000	0.024	0.006	0.205	
45%	4500	0.027	0.007	0.206	
50%	5000	0.029	0.008	0.207	
55%	5500	0.031	0.008	0.208	
60%	6000	0.035	0.009	0.208	
65%	6500	0.038	0.010	0.209	
70%	7000	0.043	0.011	0.210	
75%	7500	0.047	0.012	0.211	
80%	8000	0.051	0.012	0.212	
85%	8500	0.055	0.013	0.212	
90%	9000	0.061	0.014	0.213	
95%	9500	0.064	0.015	0.214	
100%	10000	0.070	0.015	0.215	
0%	0	0.012	0.000	0.199	



Tension Load Test Result for PLT-33A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

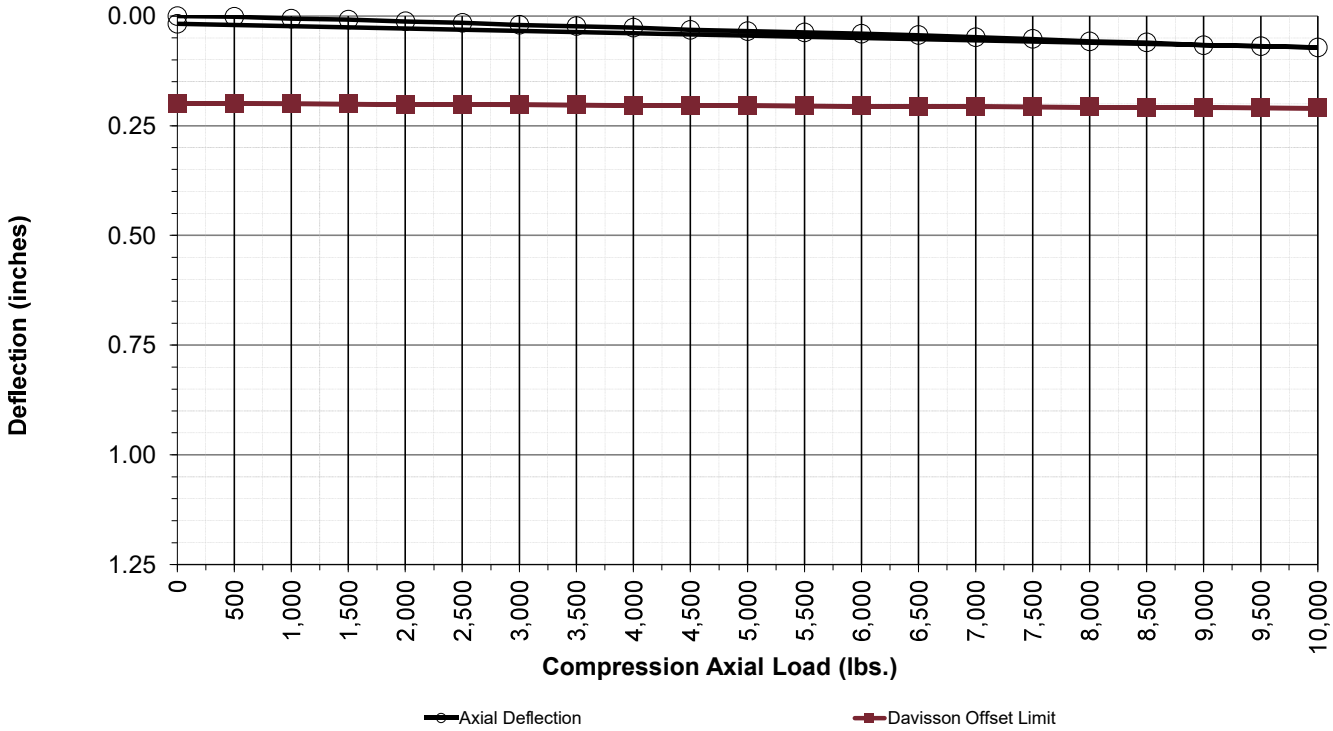
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-33A
Group Latitude [deg.]:	33.6491
Group Longitude[deg.]:	-114.7191
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	22

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.002	0.001	0.200	
10%	1000	0.006	0.001	0.200	
15%	1500	0.009	0.002	0.201	
20%	2000	0.012	0.002	0.201	
25%	2500	0.016	0.003	0.202	
30%	3000	0.021	0.003	0.202	
35%	3500	0.024	0.004	0.203	
40%	4000	0.027	0.004	0.203	
45%	4500	0.032	0.005	0.204	
50%	5000	0.034	0.005	0.205	
55%	5500	0.038	0.006	0.205	
60%	6000	0.041	0.006	0.206	
65%	6500	0.044	0.007	0.206	
70%	7000	0.049	0.008	0.207	
75%	7500	0.053	0.008	0.207	
80%	8000	0.058	0.009	0.208	
85%	8500	0.061	0.009	0.208	
90%	9000	0.067	0.010	0.209	
95%	9500	0.069	0.010	0.209	
100%	10000	0.072	0.011	0.210	
0%	0	0.018	0.000	0.199	



Tension Load Test Result for PLT-33B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

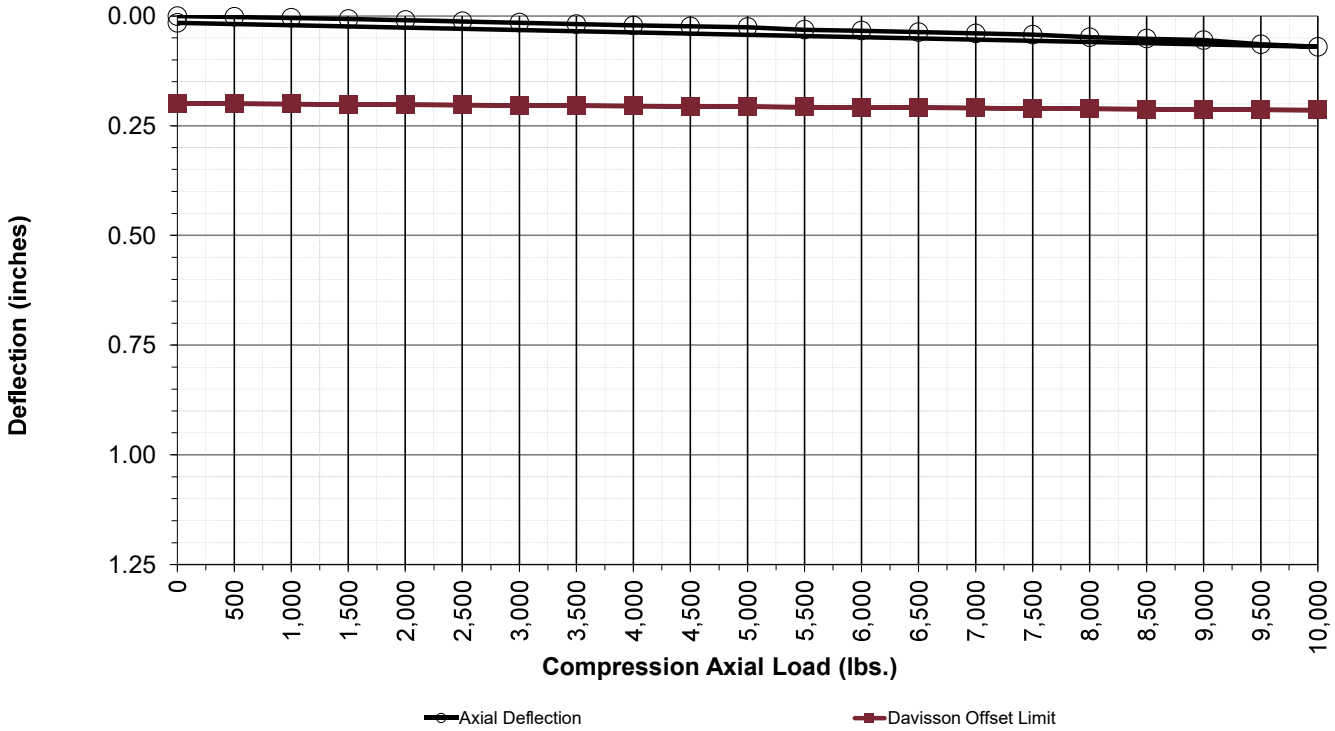
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-33B
Group Latitude [deg.]:	33.6491
Group Longitude[deg.]:	-114.7191
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	41

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.003	0.001	0.200	
10%	1000	0.004	0.002	0.201	
15%	1500	0.007	0.002	0.201	
20%	2000	0.010	0.003	0.202	
25%	2500	0.012	0.004	0.203	
30%	3000	0.015	0.005	0.204	
35%	3500	0.019	0.005	0.205	
40%	4000	0.022	0.006	0.205	
45%	4500	0.024	0.007	0.206	
50%	5000	0.026	0.008	0.207	
55%	5500	0.032	0.008	0.208	
60%	6000	0.034	0.009	0.208	
65%	6500	0.037	0.010	0.209	
70%	7000	0.040	0.011	0.210	
75%	7500	0.043	0.012	0.211	
80%	8000	0.048	0.012	0.212	
85%	8500	0.052	0.013	0.212	
90%	9000	0.055	0.014	0.213	
95%	9500	0.065	0.015	0.214	
100%	10000	0.070	0.015	0.215	
0%	0	0.016	0.000	0.199	



Tension Load Test Result for PLT-34A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

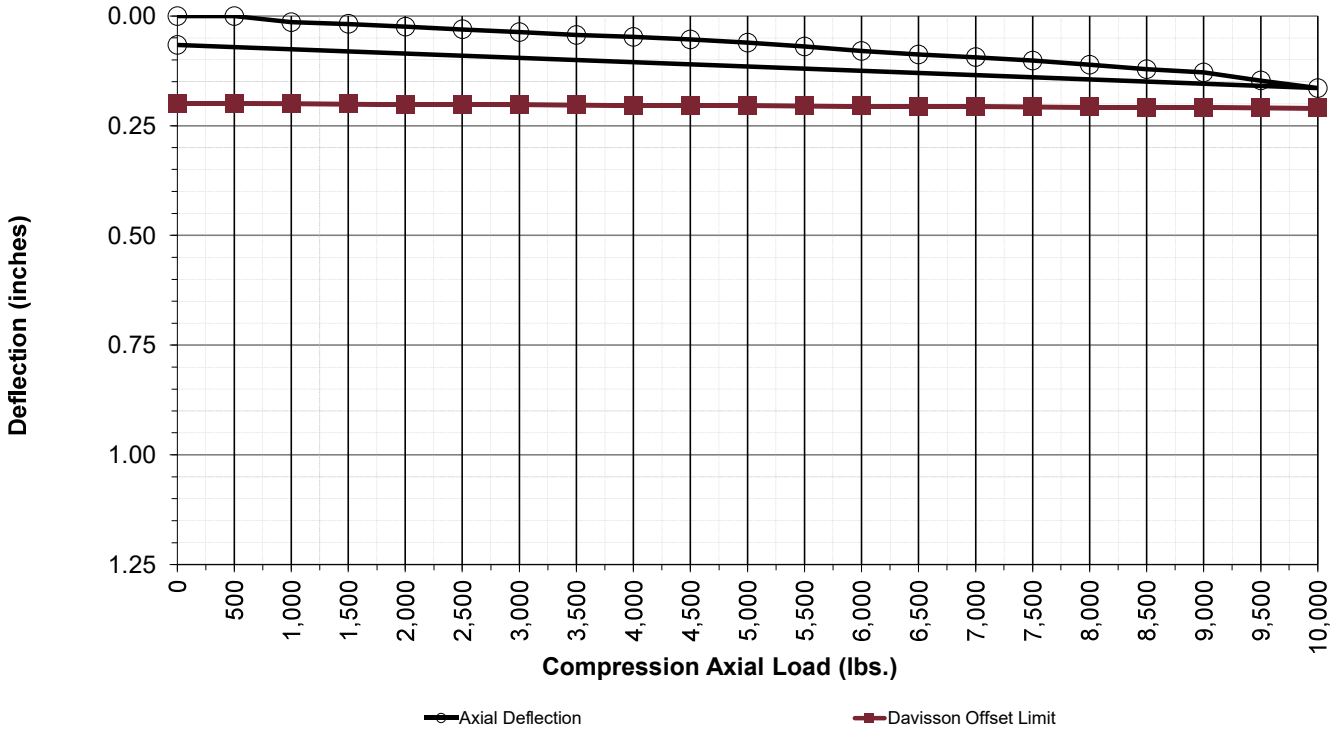
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-34A
Group Latitude [deg.]:	33.6411
Group Longitude[deg.]:	-114.7110
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	22

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.001	0.001	0.200	
10%	1000	0.015	0.001	0.200	
15%	1500	0.019	0.002	0.201	
20%	2000	0.024	0.002	0.201	
25%	2500	0.031	0.003	0.202	
30%	3000	0.037	0.003	0.202	
35%	3500	0.043	0.004	0.203	
40%	4000	0.048	0.004	0.203	
45%	4500	0.053	0.005	0.204	
50%	5000	0.061	0.005	0.205	
55%	5500	0.070	0.006	0.205	
60%	6000	0.080	0.006	0.206	
65%	6500	0.088	0.007	0.206	
70%	7000	0.094	0.008	0.207	
75%	7500	0.102	0.008	0.207	
80%	8000	0.112	0.009	0.208	
85%	8500	0.122	0.009	0.208	
90%	9000	0.129	0.010	0.209	
95%	9500	0.147	0.010	0.209	
100%	10000	0.165	0.011	0.210	
0%	0	0.066	0.000	0.199	



Tension Load Test Result for PLT-34B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

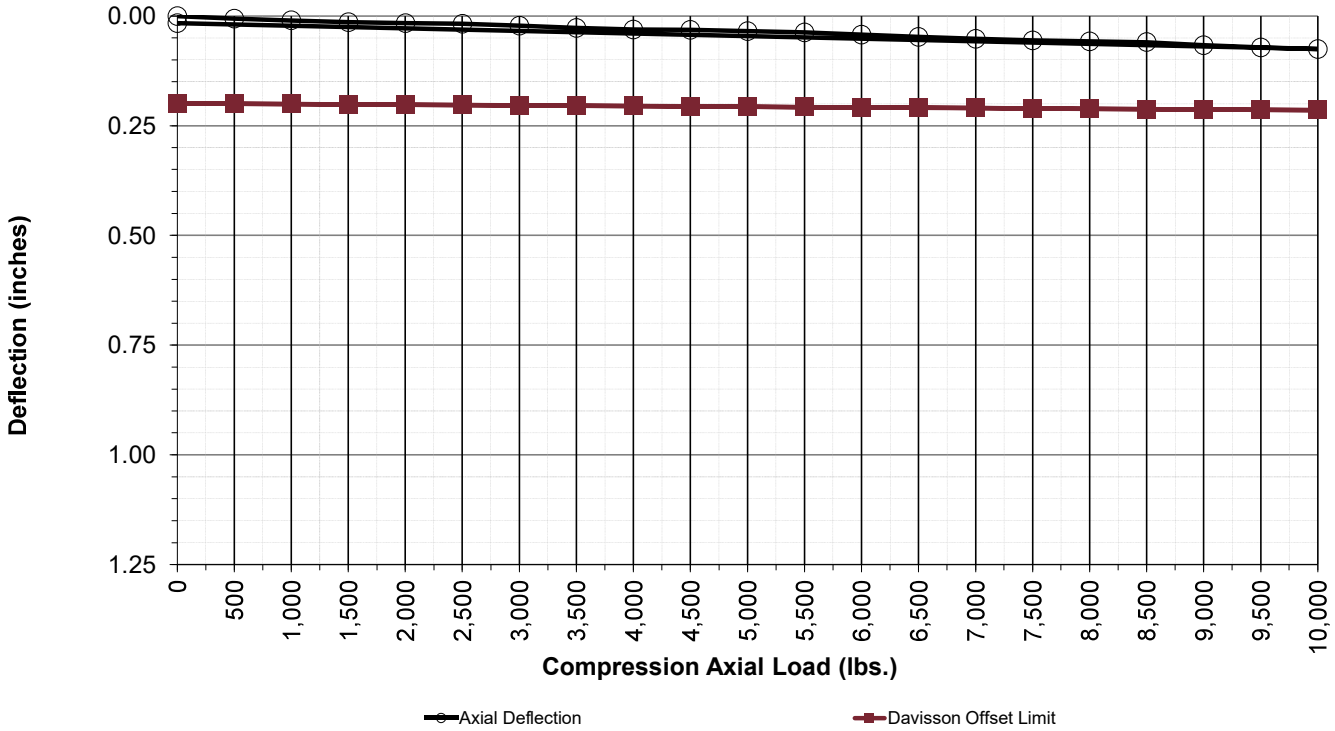
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-34B
Group Latitude [deg.]:	33.6411
Group Longitude[deg.]:	-114.7110
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	48

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.007	0.001	0.200	
10%	1000	0.010	0.002	0.201	
15%	1500	0.014	0.002	0.201	
20%	2000	0.017	0.003	0.202	
25%	2500	0.018	0.004	0.203	
30%	3000	0.022	0.005	0.204	
35%	3500	0.028	0.005	0.205	
40%	4000	0.031	0.006	0.205	
45%	4500	0.032	0.007	0.206	
50%	5000	0.035	0.008	0.207	
55%	5500	0.038	0.008	0.208	
60%	6000	0.043	0.009	0.208	
65%	6500	0.048	0.010	0.209	
70%	7000	0.052	0.011	0.210	
75%	7500	0.056	0.012	0.211	
80%	8000	0.058	0.012	0.212	
85%	8500	0.060	0.013	0.212	
90%	9000	0.067	0.014	0.213	
95%	9500	0.072	0.015	0.214	
100%	10000	0.075	0.015	0.215	
0%	0	0.016	0.000	0.199	



Tension Load Test Result for PLT-35A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

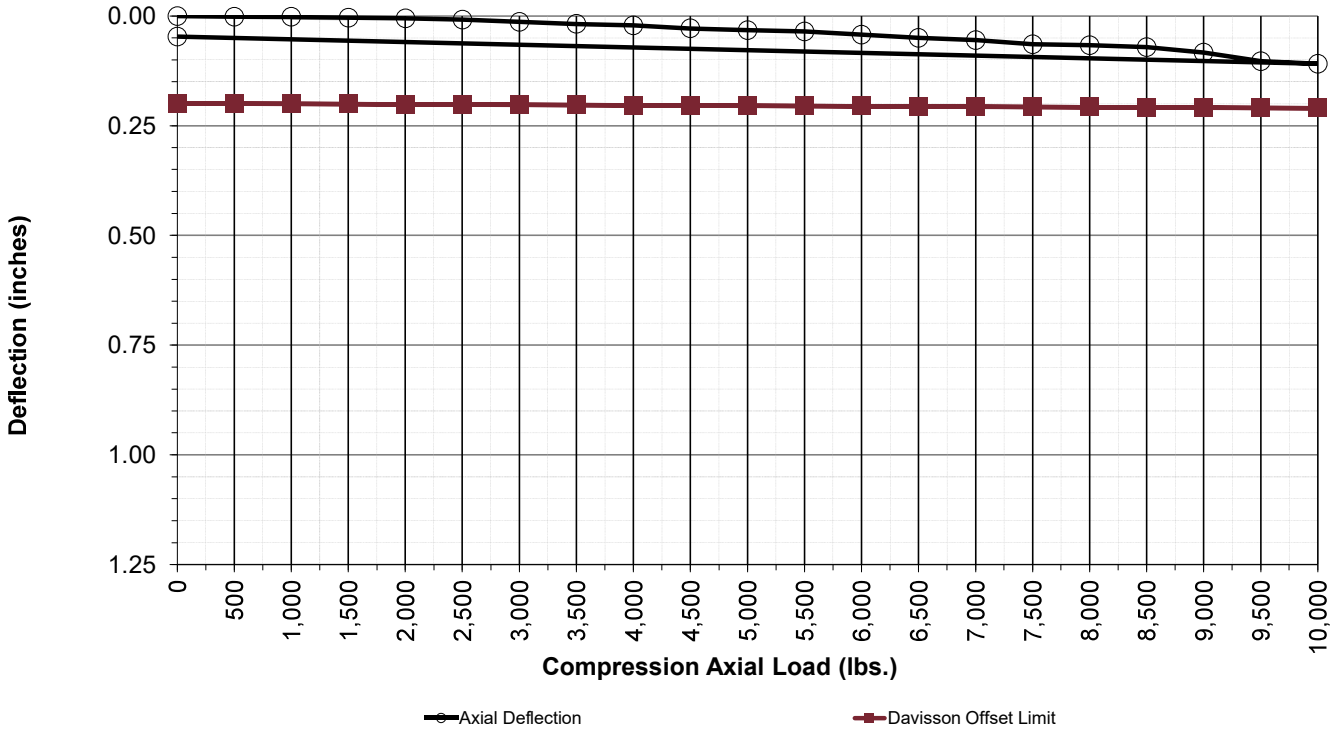
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-35A
Group Latitude [deg.]:	33.6661
Group Longitude[deg.]:	-114.7094
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	22

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.002	0.001	0.200	
10%	1000	0.003	0.001	0.200	
15%	1500	0.004	0.002	0.201	
20%	2000	0.006	0.002	0.201	
25%	2500	0.009	0.003	0.202	
30%	3000	0.013	0.003	0.202	
35%	3500	0.018	0.004	0.203	
40%	4000	0.022	0.004	0.203	
45%	4500	0.029	0.005	0.204	
50%	5000	0.032	0.005	0.205	
55%	5500	0.036	0.006	0.205	
60%	6000	0.043	0.006	0.206	
65%	6500	0.050	0.007	0.206	
70%	7000	0.055	0.008	0.207	
75%	7500	0.065	0.008	0.207	
80%	8000	0.067	0.009	0.208	
85%	8500	0.071	0.009	0.208	
90%	9000	0.084	0.010	0.209	
95%	9500	0.104	0.010	0.209	
100%	10000	0.109	0.011	0.210	
0%	0	0.047	0.000	0.199	



Tension Load Test Result for PLT-35B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Rice Lake 25k

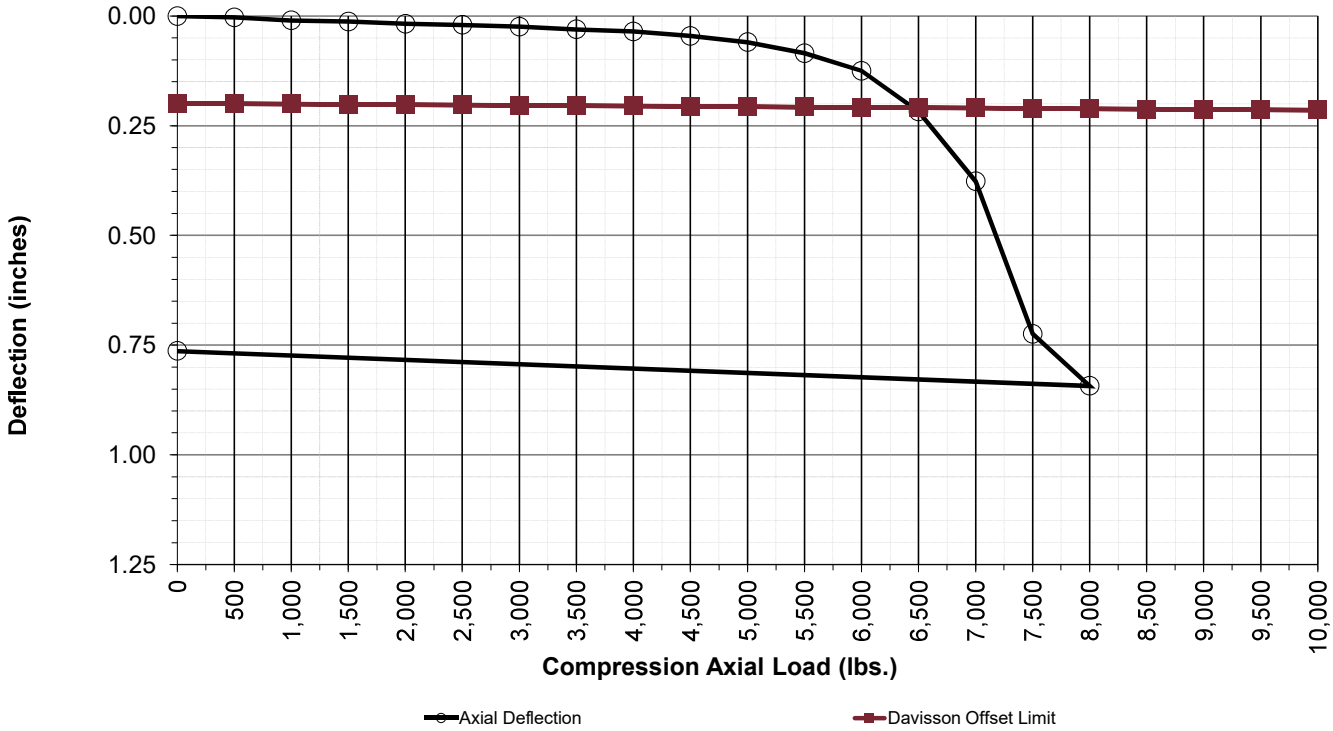
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-35B
Group Latitude [deg.]:	33.6661
Group Longitude[deg.]:	-114.7094
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Target Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	60

Tension Test Results				Davisson Offset Limit Lines	
% of Target Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offset Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.000	0.000	0.199	
5%	500	0.004	0.001	0.200	
10%	1000	0.010	0.002	0.201	
15%	1500	0.013	0.002	0.201	
20%	2000	0.018	0.003	0.202	
25%	2500	0.021	0.004	0.203	
30%	3000	0.025	0.005	0.204	
35%	3500	0.031	0.005	0.205	
40%	4000	0.036	0.006	0.205	
45%	4500	0.045	0.007	0.206	
50%	5000	0.060	0.008	0.207	
55%	5500	0.085	0.008	0.208	
60%	6000	0.125	0.009	0.208	
65%	6500	0.218	0.010	0.209	
70%	7000	0.377	0.011	0.210	
75%	7500	0.724	0.012	0.211	
80%	8000	0.843	0.012	0.212	
85%	8500		0.013	0.212	
90%	9000		0.014	0.213	
95%	9500		0.015	0.214	
100%	10000		0.015	0.215	
0%	0	0.763	0.000	0.199	



Lateral Test Results

Lateral Load Test Result for PLT-1A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

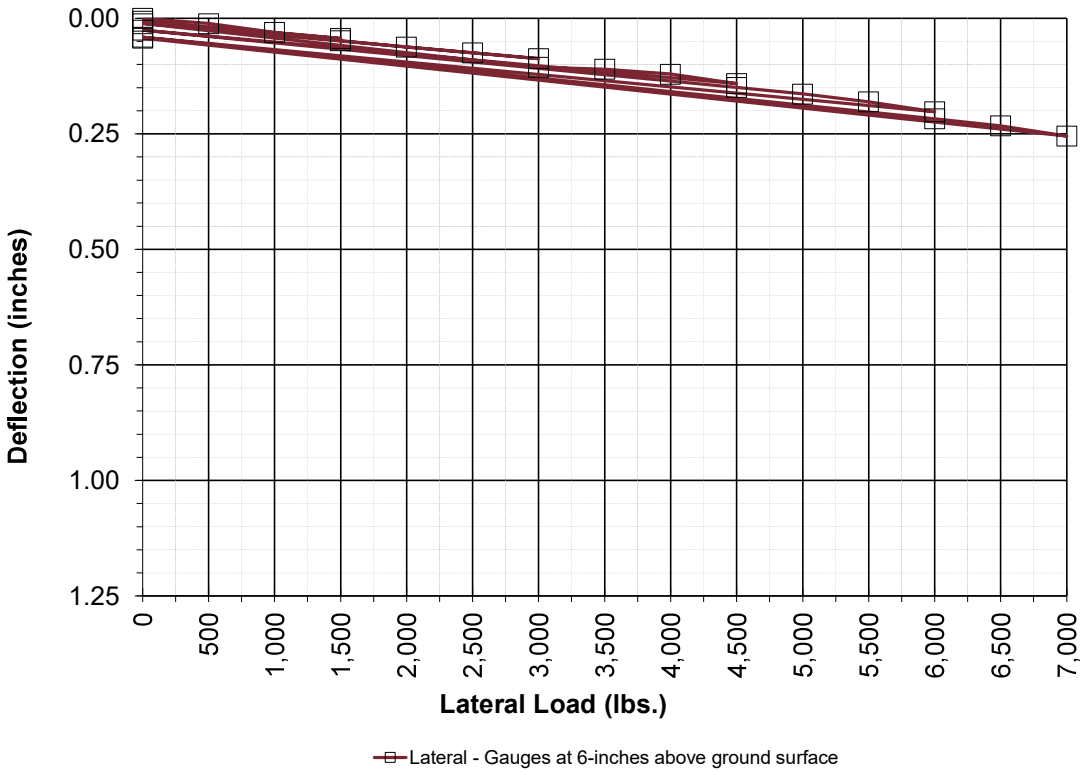
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-1A
Group Latitude [deg.]:	33.6672
Group Longitude [deg.]:	-114.6995
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	32

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.012	
14%	1,000	0.030	
21%	1,500	0.044	
0%	0	0.007	
21%	1,500	0.049	
29%	2,000	0.063	
36%	2,500	0.075	
43%	3,000	0.088	
0%	0	0.012	
43%	3,000	0.106	
50%	3,500	0.110	
57%	4,000	0.121	
64%	4,500	0.142	
0%	0	0.026	
64%	4,500	0.150	
71%	5,000	0.164	
79%	5,500	0.181	
86%	6,000	0.203	
0%	0	0.041	
86%	6,000	0.218	
93%	6,500	0.233	
100%	7,000	0.255	
0%	0	0.043	



Lateral Load Test Result for PLT-1B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

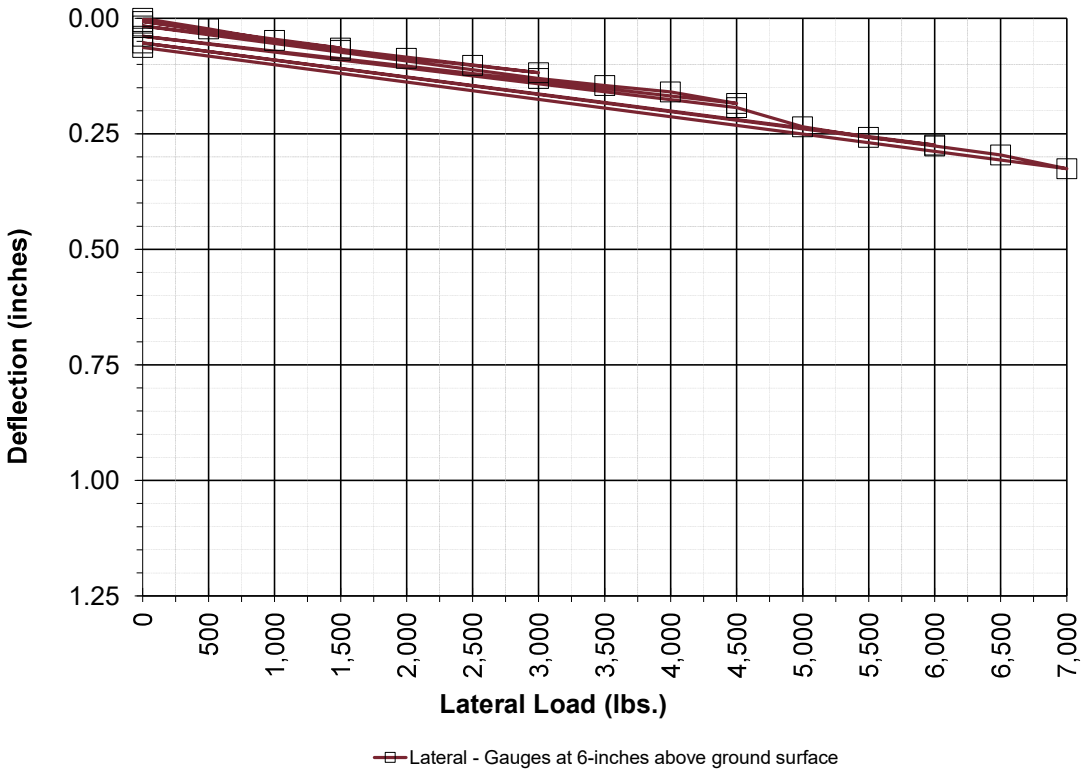
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-1B
Group Latitude [deg.]:	33.6672
Group Longitude [deg.]:	-114.6995
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	52

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.023	
14%	1,000	0.048	
21%	1,500	0.065	
0%	0	0.007	
21%	1,500	0.069	
29%	2,000	0.087	
36%	2,500	0.102	
43%	3,000	0.118	
0%	0	0.017	
43%	3,000	0.131	
50%	3,500	0.145	
57%	4,000	0.159	
64%	4,500	0.184	
0%	0	0.039	
64%	4,500	0.194	
71%	5,000	0.235	
79%	5,500	0.257	
86%	6,000	0.274	
0%	0	0.053	
86%	6,000	0.276	
93%	6,500	0.296	
100%	7,000	0.325	
0%	0	0.063	



Lateral Load Test Result for PLT-2A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

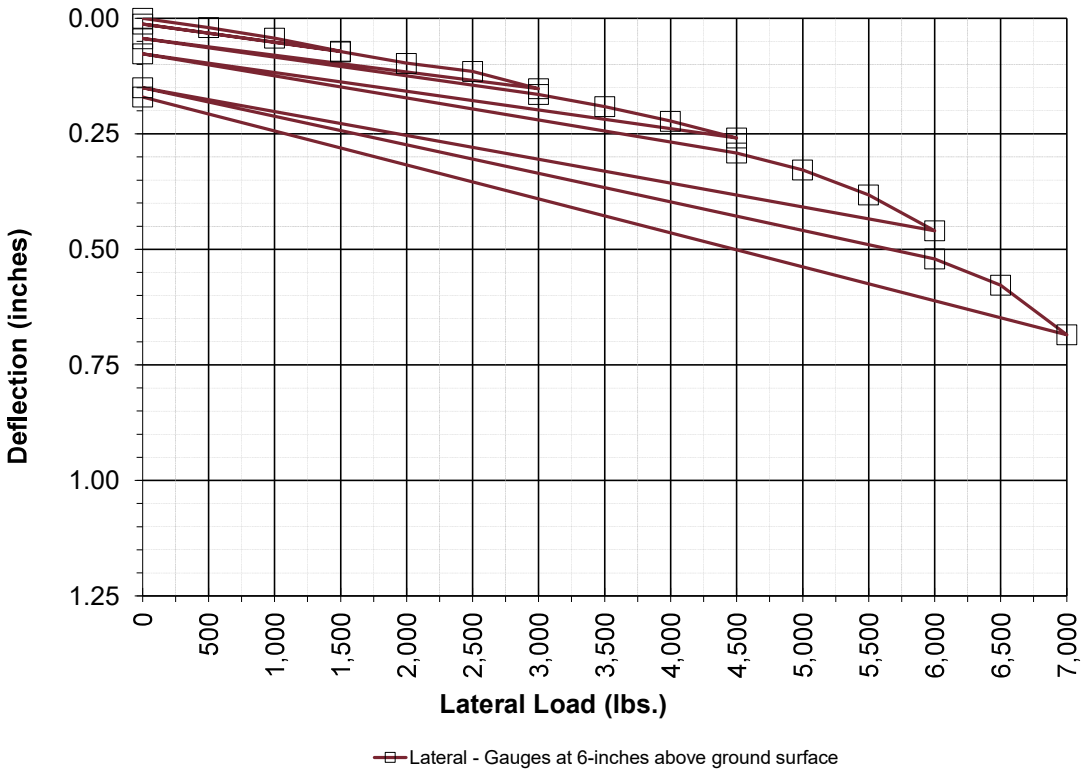
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-2A
Group Latitude [deg.]:	33.6672
Group Longitude [deg.]:	-114.6924
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	24

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.020	
14%	1,000	0.043	
21%	1,500	0.072	
0%	0	0.013	
21%	1,500	0.072	
29%	2,000	0.097	
36%	2,500	0.115	
43%	3,000	0.152	
0%	0	0.044	
43%	3,000	0.165	
50%	3,500	0.191	
57%	4,000	0.223	
64%	4,500	0.259	
0%	0	0.077	
64%	4,500	0.291	
71%	5,000	0.329	
79%	5,500	0.383	
86%	6,000	0.460	
0%	0	0.150	
86%	6,000	0.521	
93%	6,500	0.577	
100%	7,000	0.685	
0%	0	0.171	



Lateral Load Test Result for PLT-2B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

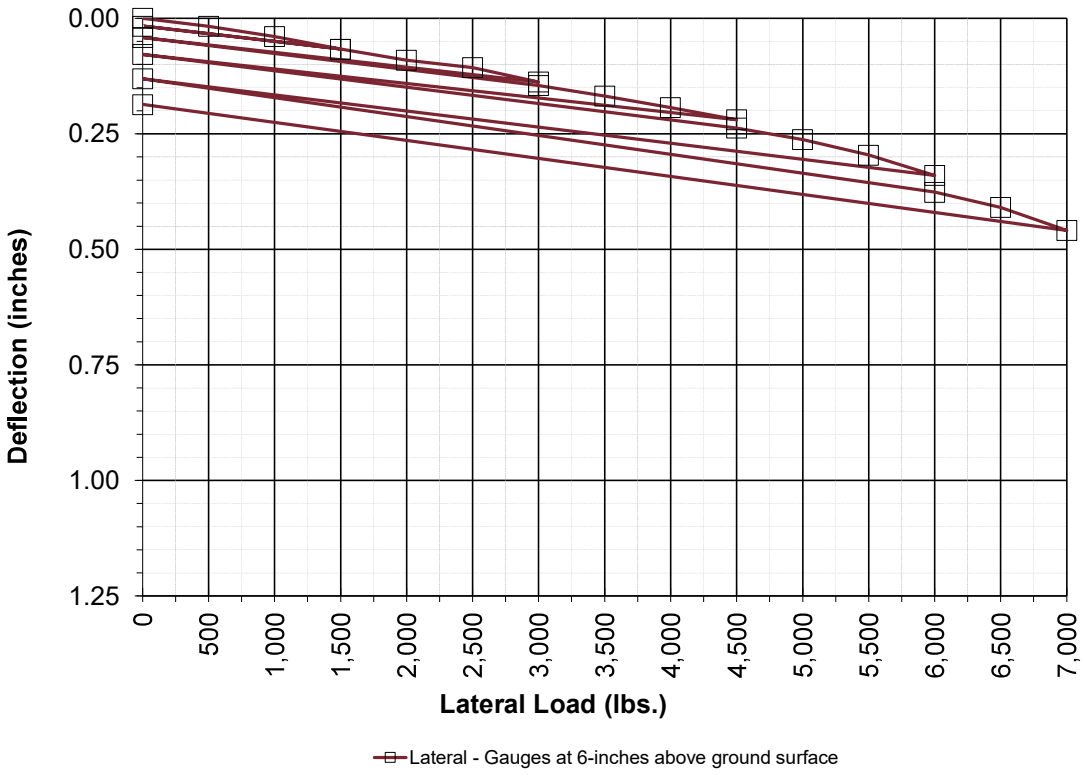
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-2B
Group Latitude [deg.]:	33.6672
Group Longitude [deg.]:	-114.6924
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	42

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.018	
14%	1,000	0.040	
21%	1,500	0.067	
0%	0	0.017	
21%	1,500	0.067	
29%	2,000	0.091	
36%	2,500	0.107	
43%	3,000	0.138	
0%	0	0.042	
43%	3,000	0.146	
50%	3,500	0.168	
57%	4,000	0.193	
64%	4,500	0.220	
0%	0	0.079	
64%	4,500	0.238	
71%	5,000	0.263	
79%	5,500	0.296	
86%	6,000	0.341	
0%	0	0.131	
86%	6,000	0.377	
93%	6,500	0.410	
100%	7,000	0.459	
0%	0	0.187	



Lateral Load Test Result for PLT-3A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

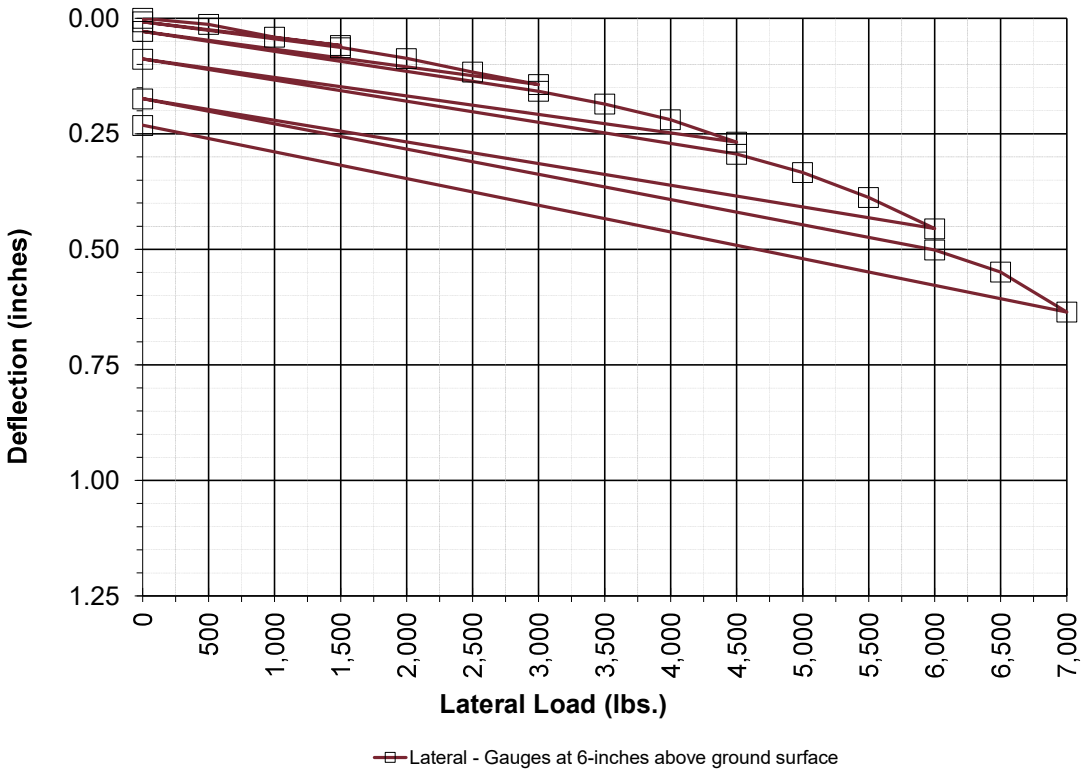
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-3A
Group Latitude [deg.]:	33.6672
Group Longitude [deg.]:	-114.6852
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	17

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.013	
14%	1,000	0.041	
21%	1,500	0.059	
0%	0	0.008	
21%	1,500	0.063	
29%	2,000	0.087	
36%	2,500	0.117	
43%	3,000	0.143	
0%	0	0.029	
43%	3,000	0.158	
50%	3,500	0.186	
57%	4,000	0.220	
64%	4,500	0.268	
0%	0	0.089	
64%	4,500	0.294	
71%	5,000	0.334	
79%	5,500	0.388	
86%	6,000	0.455	
0%	0	0.174	
86%	6,000	0.501	
93%	6,500	0.550	
100%	7,000	0.636	
0%	0	0.232	



Lateral Load Test Result for PLT-3B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

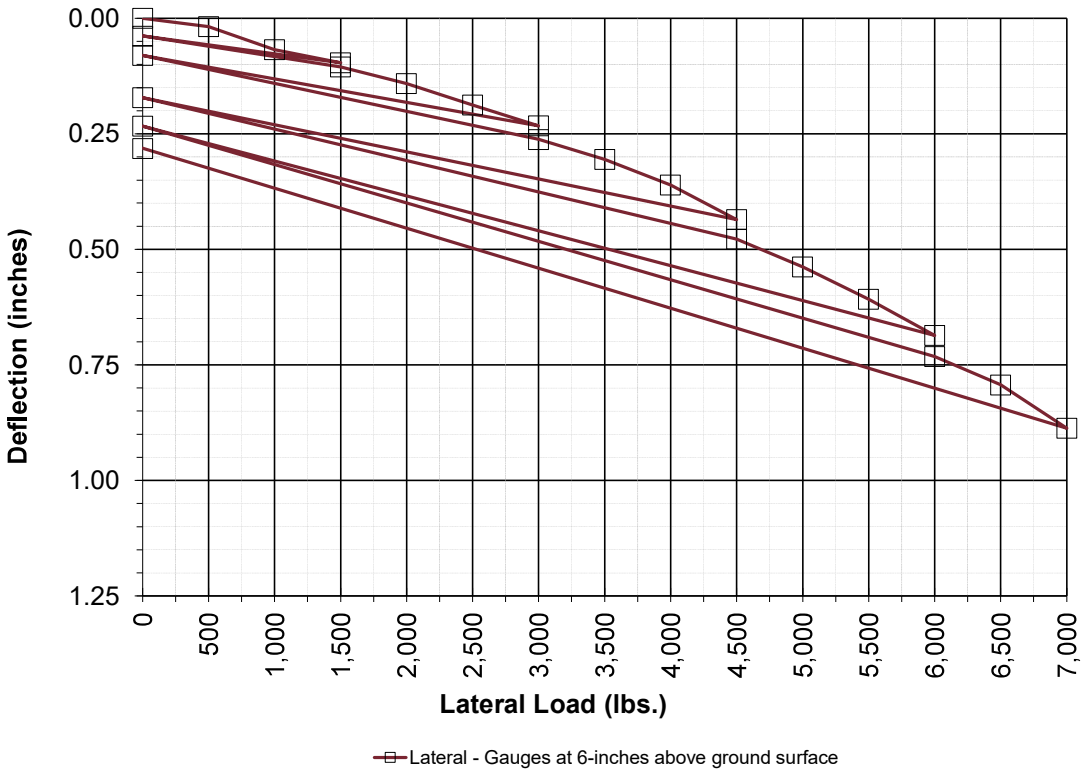
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-3B
Group Latitude [deg.]:	33.6672
Group Longitude [deg.]:	-114.6852
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	44

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.018	
14%	1,000	0.068	
21%	1,500	0.096	
0%	0	0.039	
21%	1,500	0.106	
29%	2,000	0.141	
36%	2,500	0.188	
43%	3,000	0.233	
0%	0	0.081	
43%	3,000	0.262	
50%	3,500	0.306	
57%	4,000	0.361	
64%	4,500	0.436	
0%	0	0.172	
64%	4,500	0.478	
71%	5,000	0.538	
79%	5,500	0.608	
86%	6,000	0.687	
0%	0	0.234	
86%	6,000	0.732	
93%	6,500	0.793	
100%	7,000	0.887	
0%	0	0.282	



Lateral Load Test Result for PLT-4A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

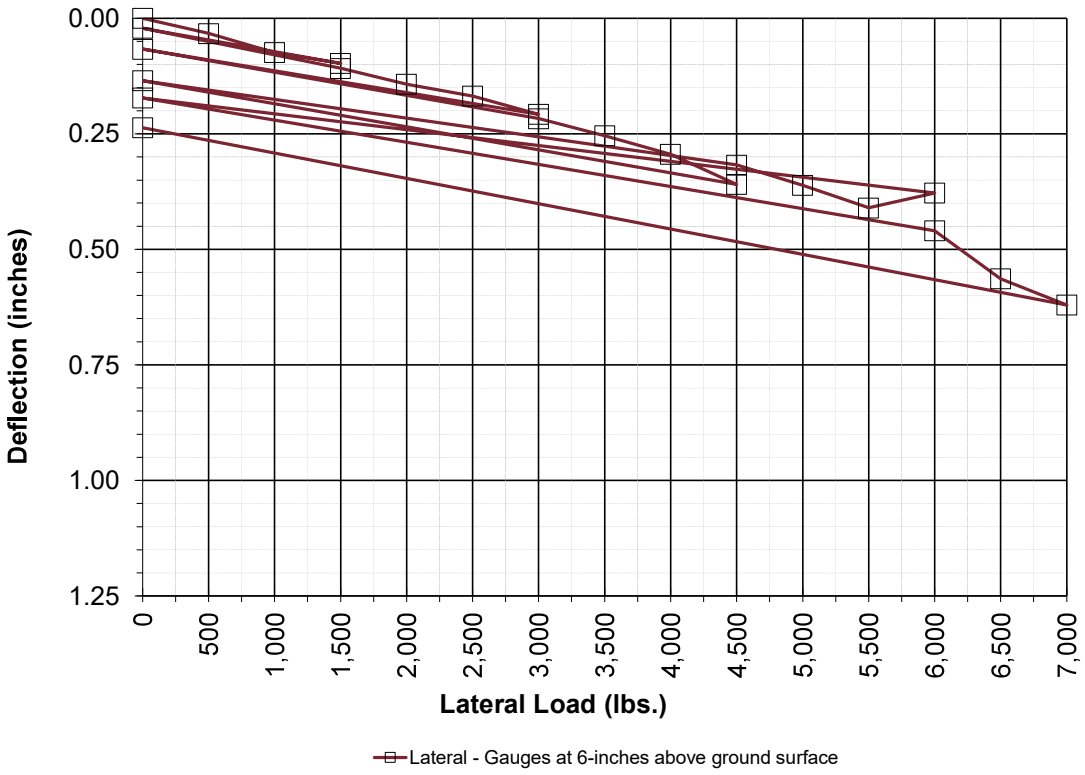
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-4A
Group Latitude [deg.]:	33.6642
Group Longitude [deg.]:	-114.6821
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	20

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.033	
14%	1,000	0.074	
21%	1,500	0.098	
0%	0	0.022	
21%	1,500	0.109	
29%	2,000	0.143	
36%	2,500	0.169	
43%	3,000	0.208	
0%	0	0.067	
43%	3,000	0.217	
50%	3,500	0.254	
57%	4,000	0.295	
64%	4,500	0.359	
0%	0	0.135	
64%	4,500	0.318	
71%	5,000	0.362	
79%	5,500	0.410	
86%	6,000	0.378	
0%	0	0.173	
86%	6,000	0.460	
93%	6,500	0.564	
100%	7,000	0.620	
0%	0	0.237	



Lateral Load Test Result for PLT-4B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

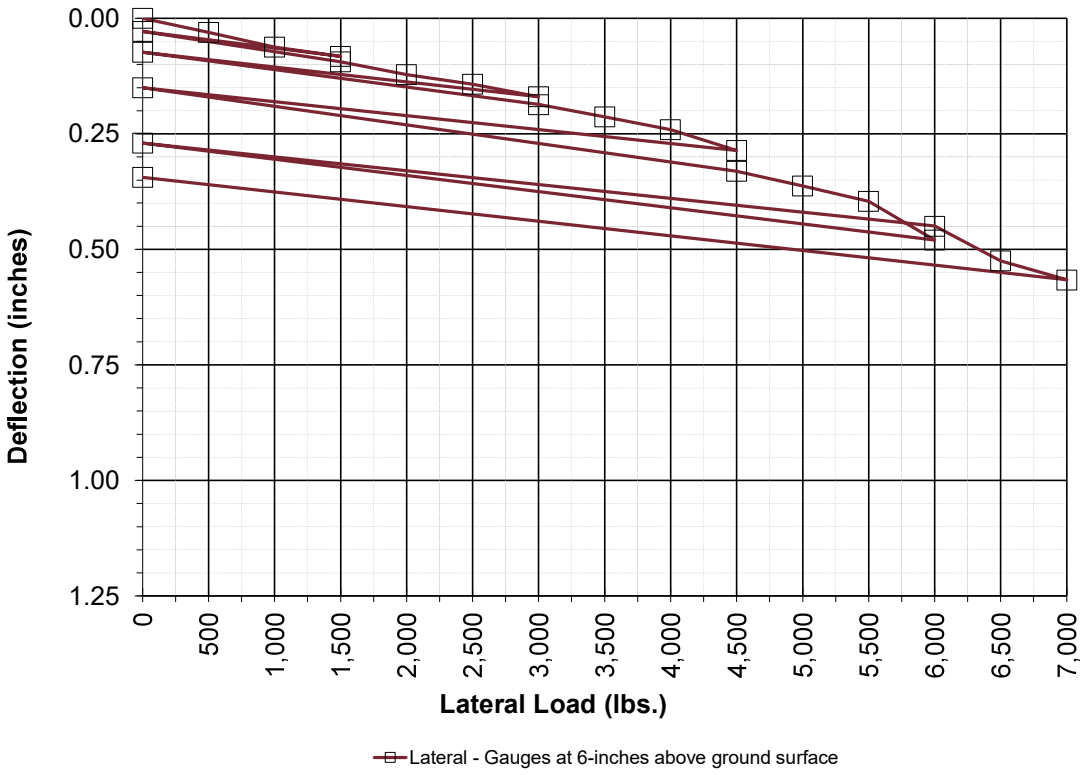
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-4B
Group Latitude [deg.]:	33.6642
Group Longitude [deg.]:	-114.6821
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	46

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.031	
14%	1,000	0.062	
21%	1,500	0.083	
0%	0	0.028	
21%	1,500	0.095	
29%	2,000	0.122	
36%	2,500	0.143	
43%	3,000	0.170	
0%	0	0.073	
43%	3,000	0.187	
50%	3,500	0.213	
57%	4,000	0.241	
64%	4,500	0.286	
0%	0	0.150	
64%	4,500	0.332	
71%	5,000	0.363	
79%	5,500	0.396	
86%	6,000	0.480	
0%	0	0.270	
86%	6,000	0.450	
93%	6,500	0.525	
100%	7,000	0.566	
0%	0	0.344	



Lateral Load Test Result for PLT-5A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

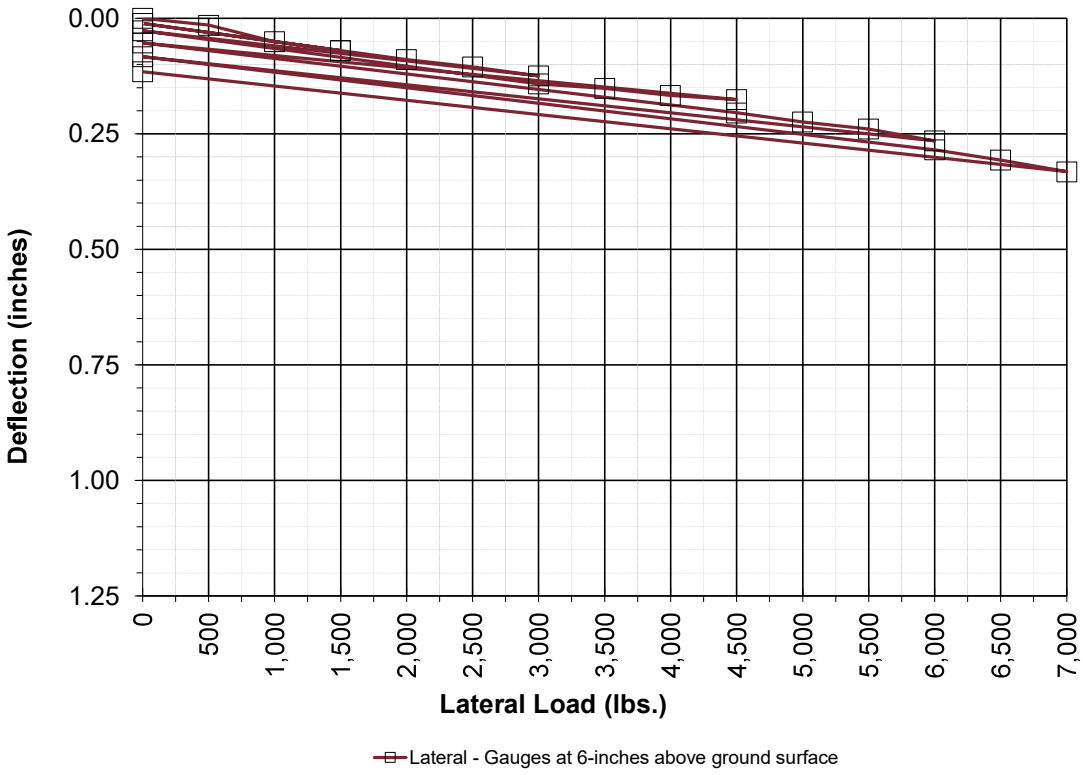
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/27/2023

Pile Information

Pile ID:	PLT-5A
Group Latitude [deg.]:	33.6636
Group Longitude [deg.]:	-114.6960
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	26

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.015	
14%	1,000	0.051	
21%	1,500	0.071	
0%	0	0.011	
21%	1,500	0.071	
29%	2,000	0.090	
36%	2,500	0.105	
43%	3,000	0.125	
0%	0	0.028	
43%	3,000	0.142	
50%	3,500	0.152	
57%	4,000	0.167	
64%	4,500	0.176	
0%	0	0.054	
64%	4,500	0.205	
71%	5,000	0.225	
79%	5,500	0.240	
86%	6,000	0.265	
0%	0	0.083	
86%	6,000	0.285	
93%	6,500	0.307	
100%	7,000	0.332	
0%	0	0.116	



Lateral Load Test Result for PLT-5B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

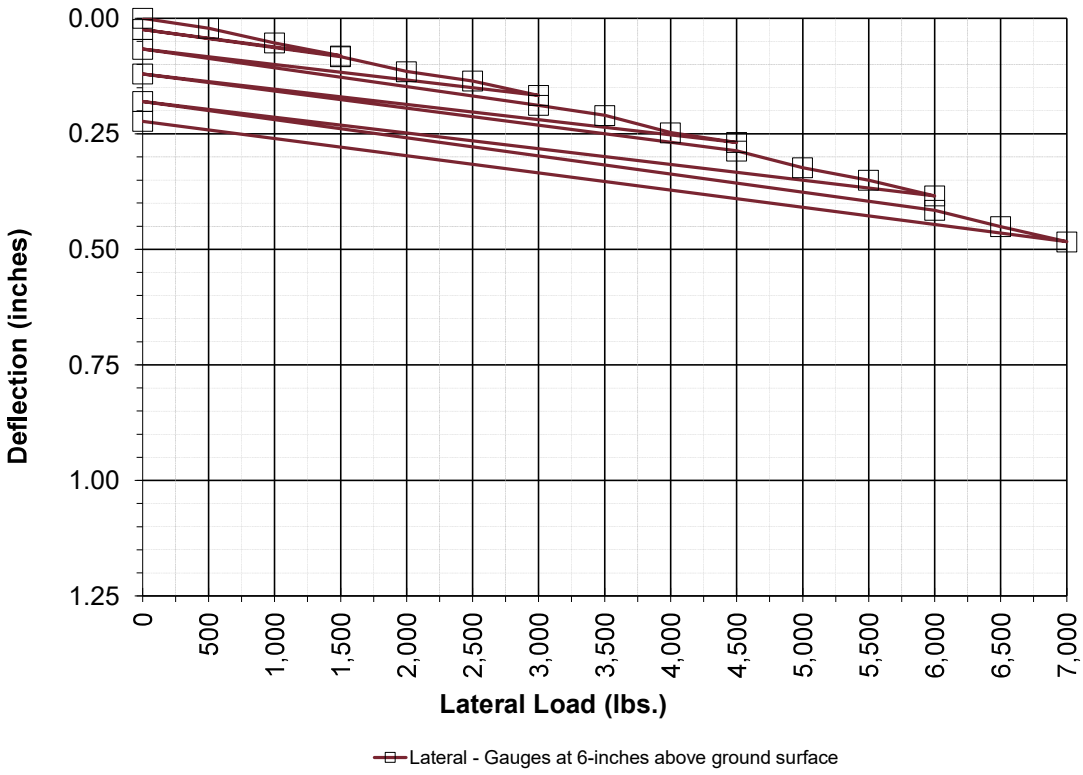
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/27/2023

Pile Information

Pile ID:	PLT-5B
Group Latitude [deg.]:	33.6636
Group Longitude [deg.]:	-114.6960
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	42

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.022	
14%	1,000	0.054	
21%	1,500	0.082	
0%	0	0.025	
21%	1,500	0.083	
29%	2,000	0.115	
36%	2,500	0.136	
43%	3,000	0.167	
0%	0	0.067	
43%	3,000	0.188	
50%	3,500	0.210	
57%	4,000	0.248	
64%	4,500	0.269	
0%	0	0.121	
64%	4,500	0.287	
71%	5,000	0.324	
79%	5,500	0.351	
86%	6,000	0.385	
0%	0	0.180	
86%	6,000	0.416	
93%	6,500	0.451	
100%	7,000	0.484	
0%	0	0.223	



Lateral Load Test Result for PLT-6A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

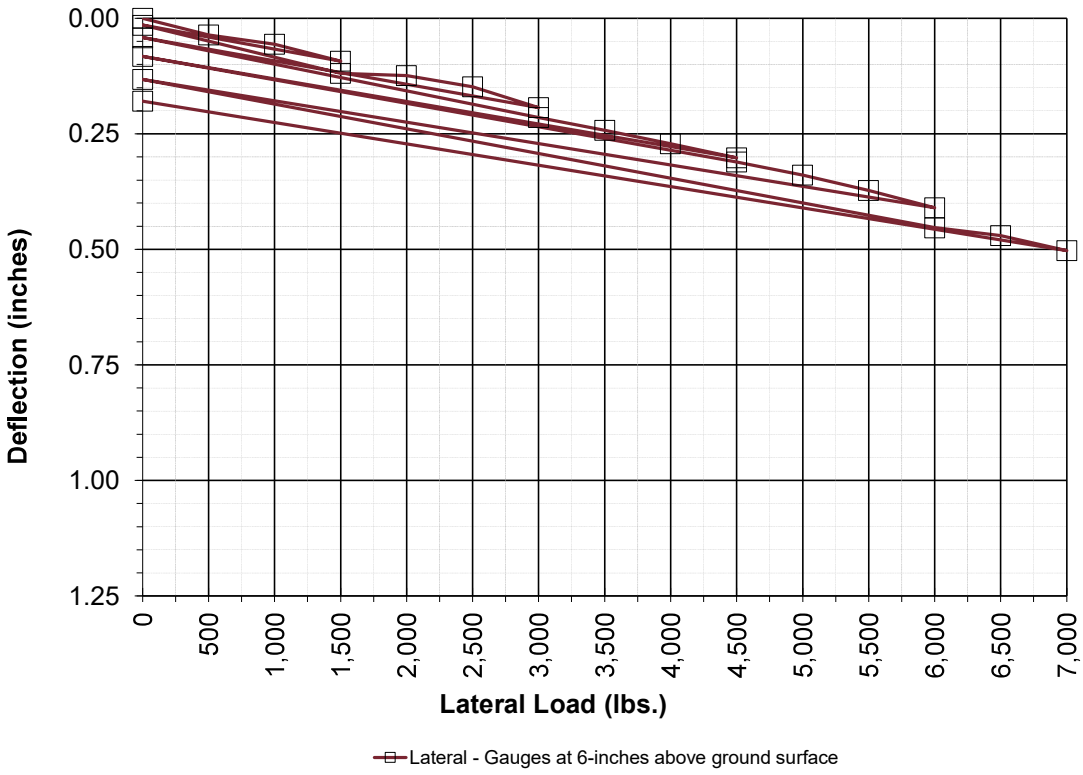
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-6A
Group Latitude [deg.]:	33.6635
Group Longitude [deg.]:	-114.7068
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	20

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.036	
14%	1,000	0.056	
21%	1,500	0.092	
0%	0	0.015	
21%	1,500	0.119	
29%	2,000	0.124	
36%	2,500	0.148	
43%	3,000	0.193	
0%	0	0.042	
43%	3,000	0.215	
50%	3,500	0.242	
57%	4,000	0.272	
64%	4,500	0.302	
0%	0	0.083	
64%	4,500	0.311	
71%	5,000	0.340	
79%	5,500	0.373	
86%	6,000	0.410	
0%	0	0.132	
86%	6,000	0.453	
93%	6,500	0.470	
100%	7,000	0.503	
0%	0	0.179	



Lateral Load Test Result for PLT-6B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

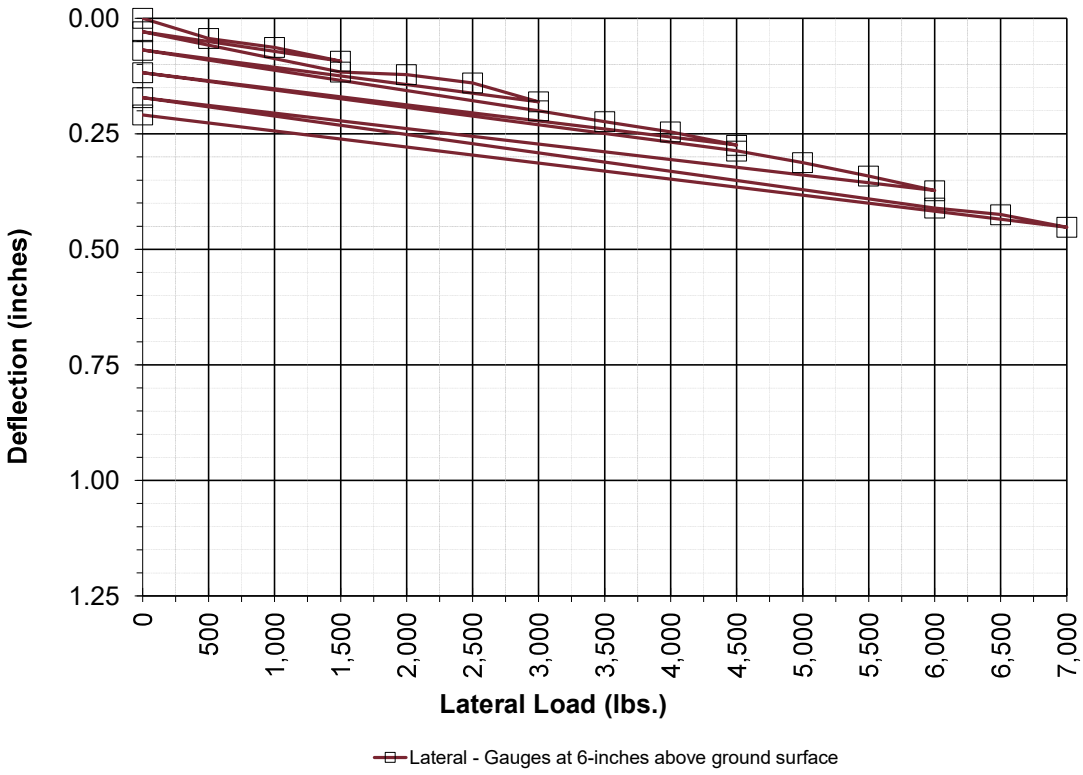
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-6B
Group Latitude [deg.]:	33.6635
Group Longitude [deg.]:	-114.7068
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	74

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.044	
14%	1,000	0.063	
21%	1,500	0.093	
0%	0	0.029	
21%	1,500	0.117	
29%	2,000	0.122	
36%	2,500	0.140	
43%	3,000	0.181	
0%	0	0.069	
43%	3,000	0.200	
50%	3,500	0.224	
57%	4,000	0.246	
64%	4,500	0.275	
0%	0	0.118	
64%	4,500	0.287	
71%	5,000	0.312	
79%	5,500	0.341	
86%	6,000	0.373	
0%	0	0.172	
86%	6,000	0.411	
93%	6,500	0.425	
100%	7,000	0.452	
0%	0	0.209	



Lateral Load Test Result for PLT-7A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

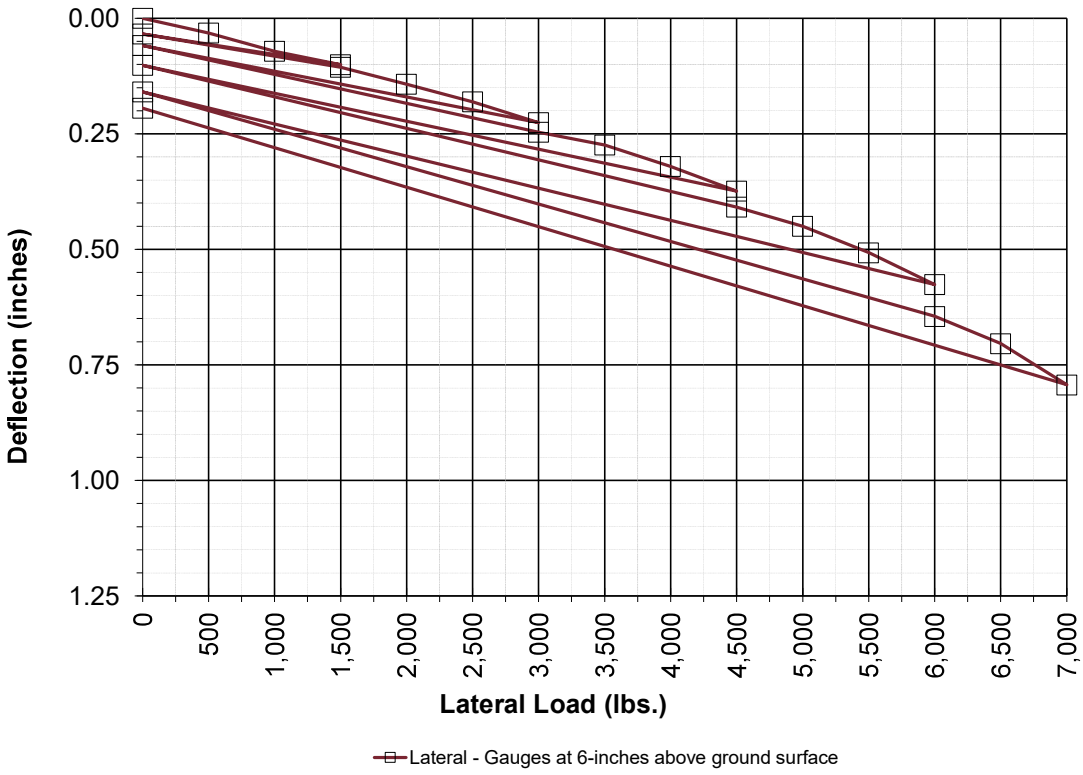
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-7A
Group Latitude [deg.]:	33.6612
Group Longitude [deg.]:	-114.7029
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	21

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.032	
14%	1,000	0.071	
21%	1,500	0.101	
0%	0	0.034	
21%	1,500	0.106	
29%	2,000	0.143	
36%	2,500	0.181	
43%	3,000	0.226	
0%	0	0.059	
43%	3,000	0.247	
50%	3,500	0.274	
57%	4,000	0.321	
64%	4,500	0.374	
0%	0	0.102	
64%	4,500	0.409	
71%	5,000	0.450	
79%	5,500	0.508	
86%	6,000	0.577	
0%	0	0.160	
86%	6,000	0.645	
93%	6,500	0.704	
100%	7,000	0.793	
0%	0	0.195	



Lateral Load Test Result for PLT-7B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

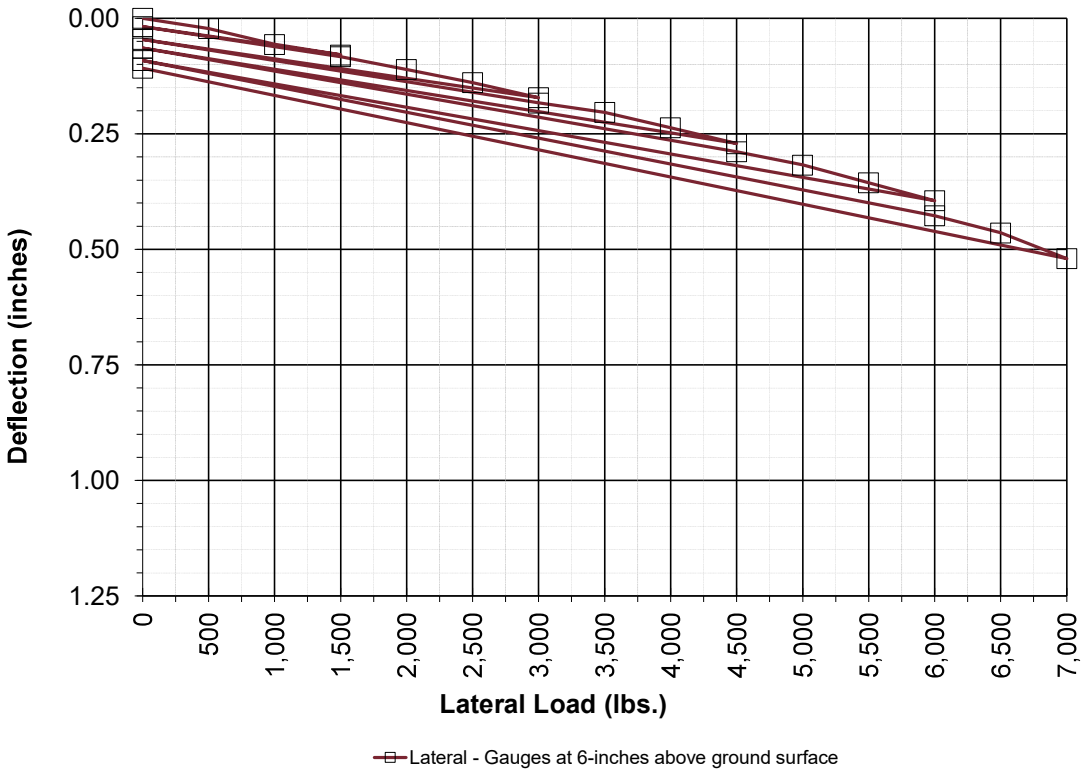
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-7B
Group Latitude [deg.]:	33.6612
Group Longitude [deg.]:	-114.7029
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	45

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.022	
14%	1,000	0.057	
21%	1,500	0.079	
0%	0	0.019	
21%	1,500	0.083	
29%	2,000	0.111	
36%	2,500	0.139	
43%	3,000	0.172	
0%	0	0.046	
43%	3,000	0.183	
50%	3,500	0.203	
57%	4,000	0.237	
64%	4,500	0.271	
0%	0	0.065	
64%	4,500	0.289	
71%	5,000	0.318	
79%	5,500	0.356	
86%	6,000	0.395	
0%	0	0.092	
86%	6,000	0.428	
93%	6,500	0.465	
100%	7,000	0.520	
0%	0	0.108	



Lateral Load Test Result for PLT-8A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

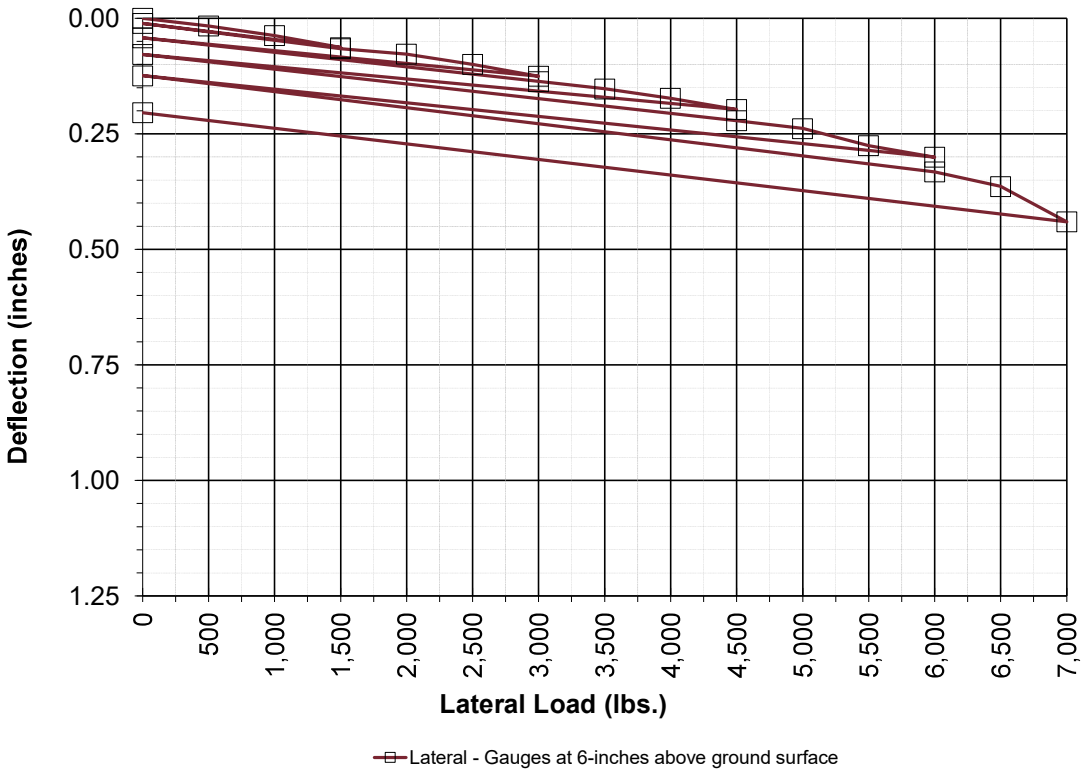
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-8A
Group Latitude [deg.]:	33.6600
Group Longitude [deg.]:	-114.6888
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	30

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.017	
14%	1,000	0.038	
21%	1,500	0.064	
0%	0	0.011	
21%	1,500	0.066	
29%	2,000	0.078	
36%	2,500	0.100	
43%	3,000	0.126	
0%	0	0.043	
43%	3,000	0.137	
50%	3,500	0.153	
57%	4,000	0.174	
64%	4,500	0.197	
0%	0	0.079	
64%	4,500	0.222	
71%	5,000	0.239	
79%	5,500	0.276	
86%	6,000	0.301	
0%	0	0.125	
86%	6,000	0.333	
93%	6,500	0.364	
100%	7,000	0.441	
0%	0	0.204	



Lateral Load Test Result for PLT-8B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

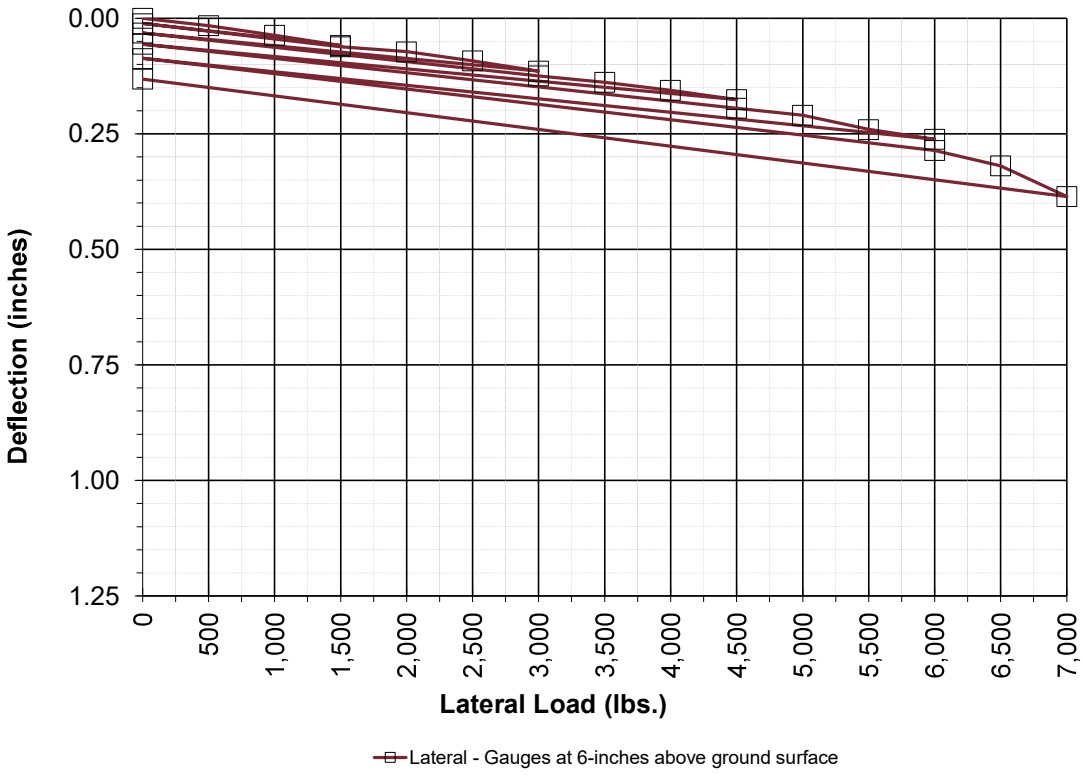
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-8B
Group Latitude [deg.]:	33.6600
Group Longitude [deg.]:	-114.6888
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	45

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.016	
14%	1,000	0.037	
21%	1,500	0.059	
0%	0	0.012	
21%	1,500	0.062	
29%	2,000	0.072	
36%	2,500	0.092	
43%	3,000	0.115	
0%	0	0.032	
43%	3,000	0.125	
50%	3,500	0.139	
57%	4,000	0.156	
64%	4,500	0.176	
0%	0	0.056	
64%	4,500	0.195	
71%	5,000	0.210	
79%	5,500	0.241	
86%	6,000	0.262	
0%	0	0.087	
86%	6,000	0.286	
93%	6,500	0.319	
100%	7,000	0.386	
0%	0	0.132	



Lateral Load Test Result for PLT-9A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

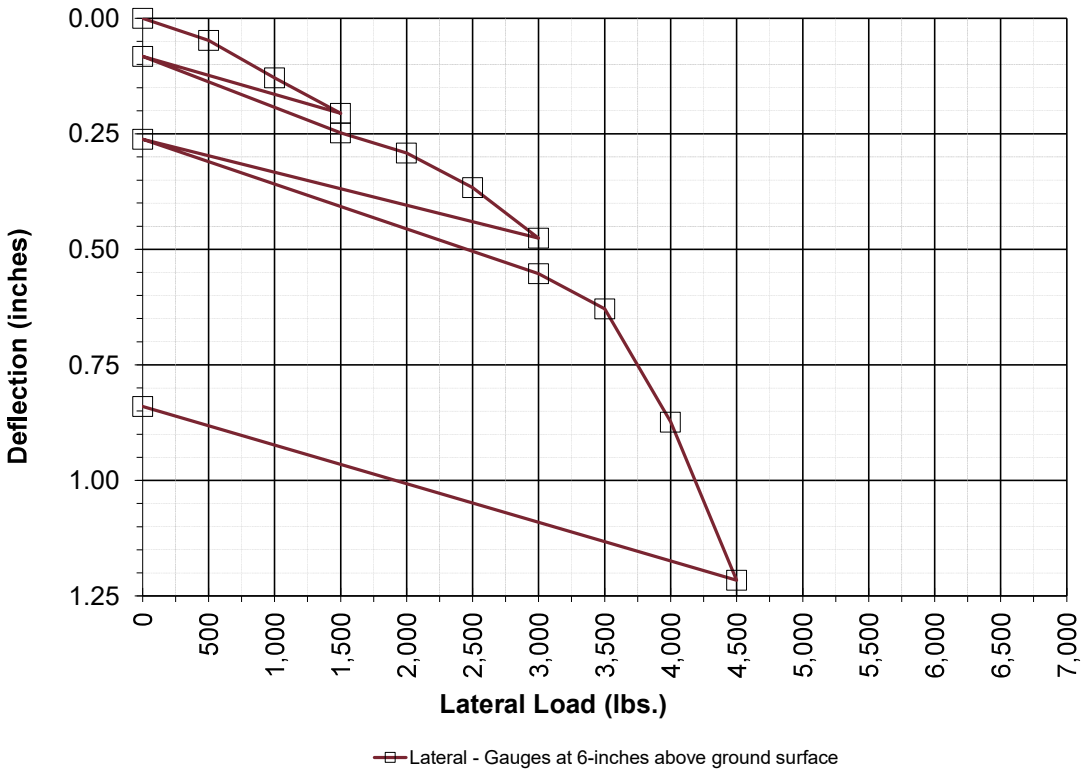
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-9A
Group Latitude [deg.]:	33.6600
Group Longitude [deg.]:	-114.6779
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	13

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.048	
14%	1,000	0.129	
21%	1,500	0.206	
0%	0	0.083	
21%	1,500	0.248	
29%	2,000	0.292	
36%	2,500	0.367	
43%	3,000	0.476	
0%	0	0.262	
43%	3,000	0.552	
50%	3,500	0.629	
57%	4,000	0.873	
64%	4,500	1.216	
0%	0		
64%	4,500		
71%	5,000		
79%	5,500		
86%	6,000		
0%	0		
86%	6,000		
93%	6,500		
100%	7,000		
0%	0	0.840	



Lateral Load Test Result for PLT-9B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

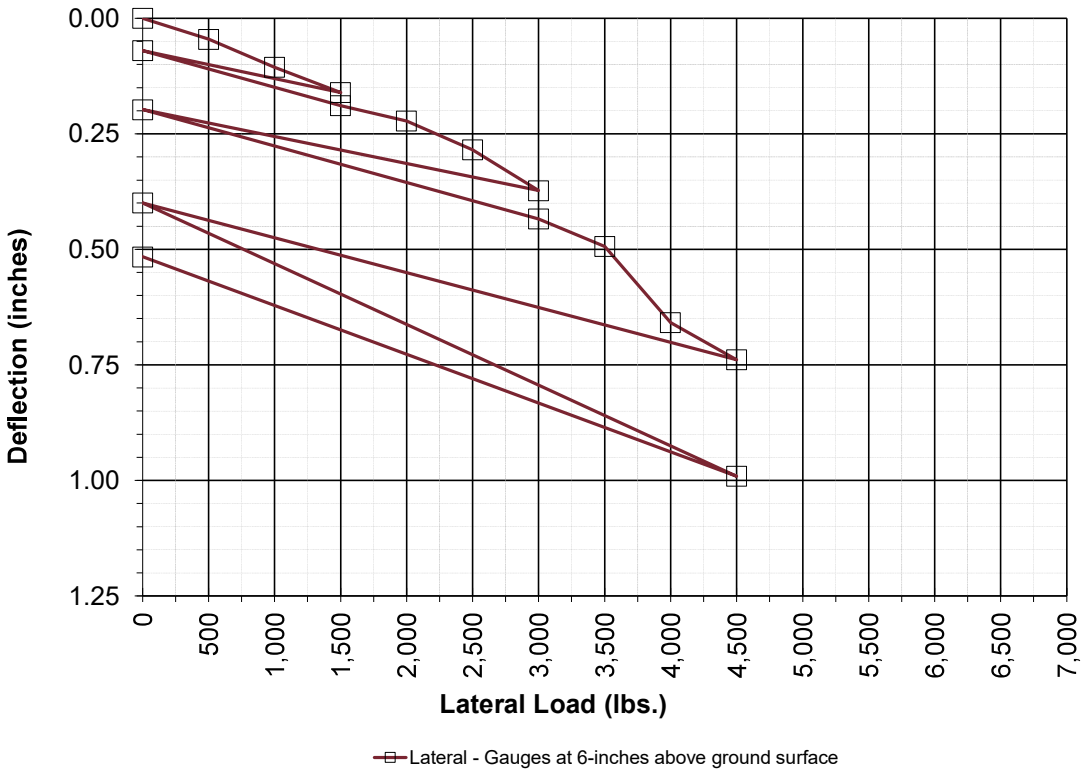
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-9B
Group Latitude [deg.]:	33.6600
Group Longitude [deg.]:	-114.6779
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	32

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.045	
14%	1,000	0.106	
21%	1,500	0.161	
0%	0	0.070	
21%	1,500	0.189	
29%	2,000	0.222	
36%	2,500	0.285	
43%	3,000	0.373	
0%	0	0.198	
43%	3,000	0.434	
50%	3,500	0.494	
57%	4,000	0.659	
64%	4,500	0.739	
0%	0	0.400	
64%	4,500	0.991	
71%	5,000		
79%	5,500		
86%	6,000		
0%	0		
86%	6,000		
93%	6,500		
100%	7,000		
0%	0	0.517	



Lateral Load Test Result for PLT-10A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

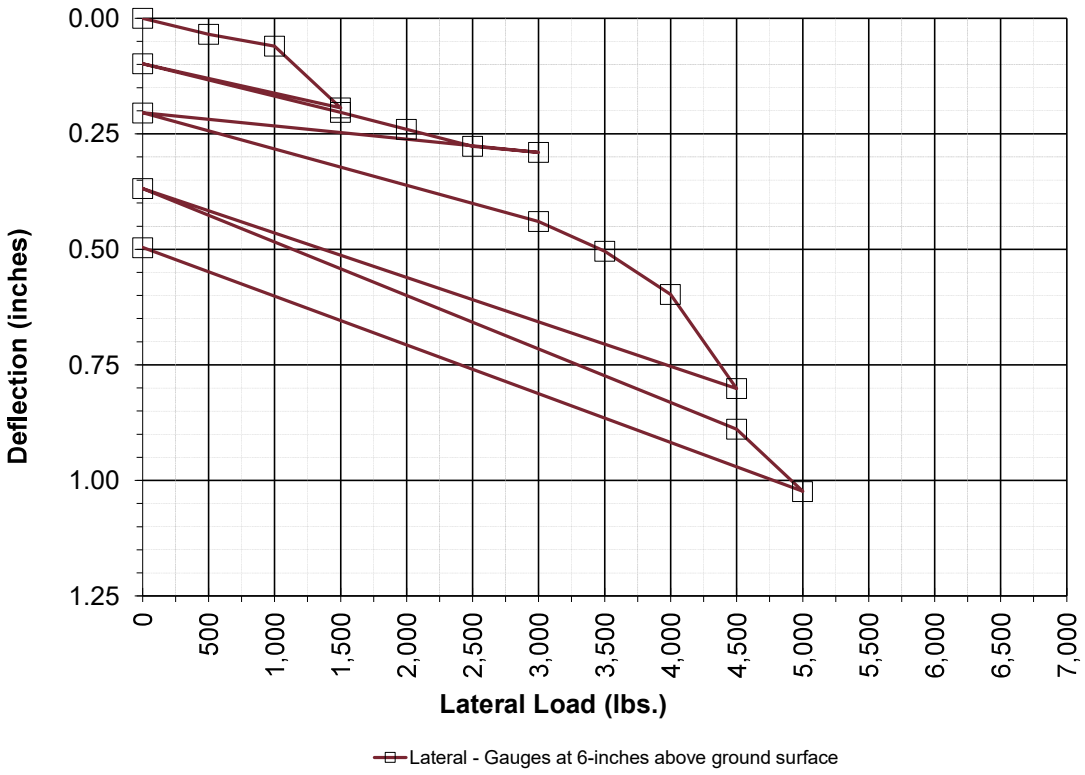
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-10A
Group Latitude [deg.]:	33.6565
Group Longitude [deg.]:	-114.6743
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	16

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.035	
14%	1,000	0.060	
21%	1,500	0.194	
0%	0	0.099	
21%	1,500	0.204	
29%	2,000	0.240	
36%	2,500	0.277	
43%	3,000	0.290	
0%	0	0.205	
43%	3,000	0.440	
50%	3,500	0.504	
57%	4,000	0.597	
64%	4,500	0.801	
0%	0	0.368	
64%	4,500	0.889	
71%	5,000	1.024	
79%	5,500		
86%	6,000		
0%	0		
86%	6,000		
93%	6,500		
100%	7,000		
0%	0	0.496	



Lateral Load Test Result for PLT-10B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

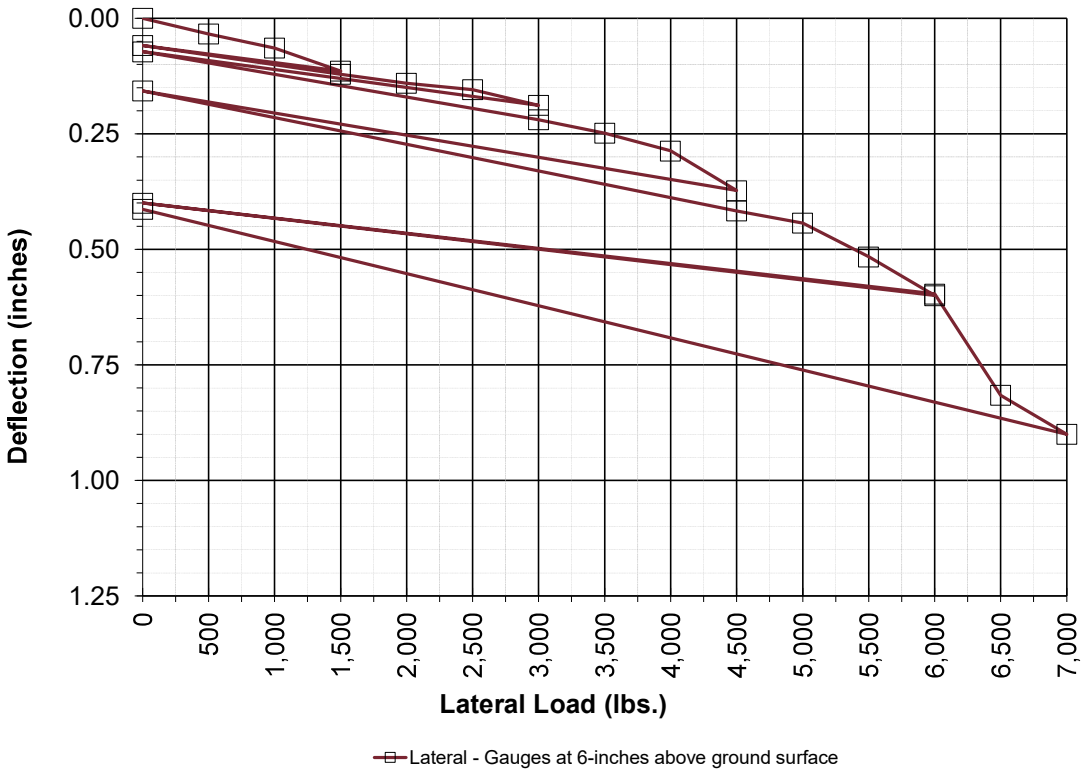
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-10B
Group Latitude [deg.]:	33.6565
Group Longitude [deg.]:	-114.6743
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	34

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.034	
14%	1,000	0.065	
21%	1,500	0.114	
0%	0	0.059	
21%	1,500	0.121	
29%	2,000	0.141	
36%	2,500	0.154	
43%	3,000	0.188	
0%	0	0.072	
43%	3,000	0.220	
50%	3,500	0.248	
57%	4,000	0.287	
64%	4,500	0.373	
0%	0	0.157	
64%	4,500	0.417	
71%	5,000	0.444	
79%	5,500	0.517	
86%	6,000	0.600	
0%	0	0.400	
86%	6,000	0.597	
93%	6,500	0.816	
100%	7,000	0.900	
0%	0	0.414	



Lateral Load Test Result for PLT-11A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

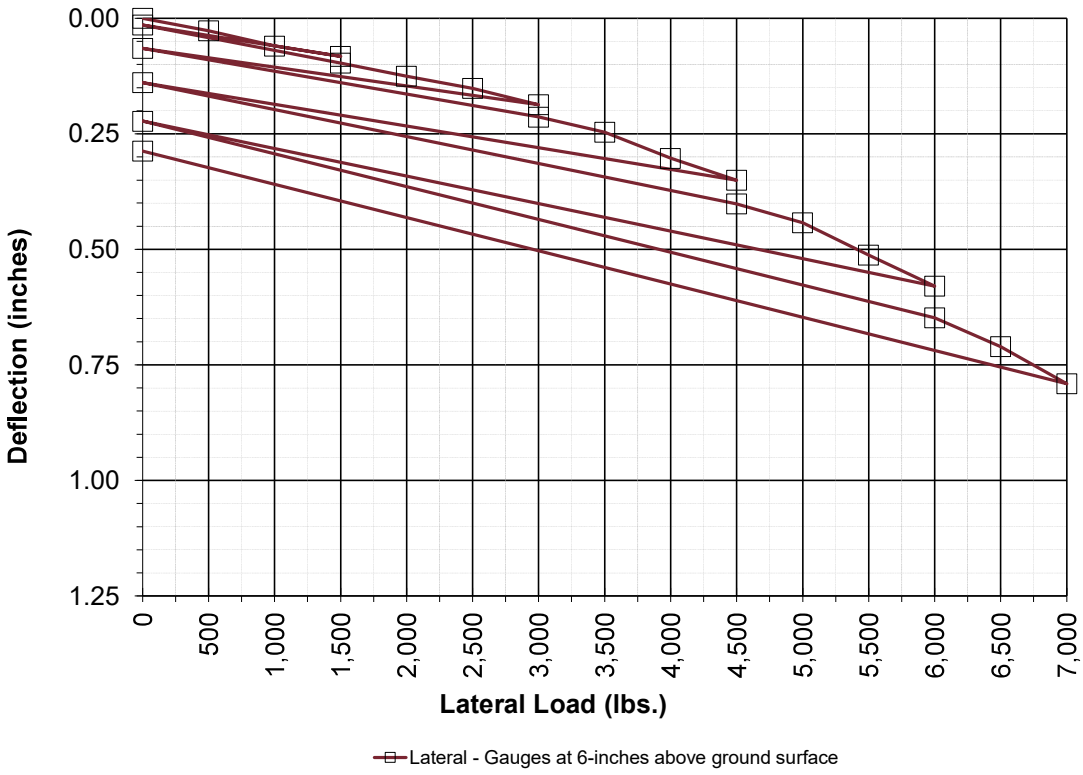
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-11A
Group Latitude [deg.]:	33.6564
Group Longitude [deg.]:	-114.6815
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	23

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.028	
14%	1,000	0.060	
21%	1,500	0.083	
0%	0	0.015	
21%	1,500	0.097	
29%	2,000	0.126	
36%	2,500	0.152	
43%	3,000	0.187	
0%	0	0.065	
43%	3,000	0.214	
50%	3,500	0.246	
57%	4,000	0.303	
64%	4,500	0.350	
0%	0	0.140	
64%	4,500	0.402	
71%	5,000	0.443	
79%	5,500	0.512	
86%	6,000	0.580	
0%	0	0.223	
86%	6,000	0.648	
93%	6,500	0.710	
100%	7,000	0.791	
0%	0	0.288	



Lateral Load Test Result for PLT-11B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

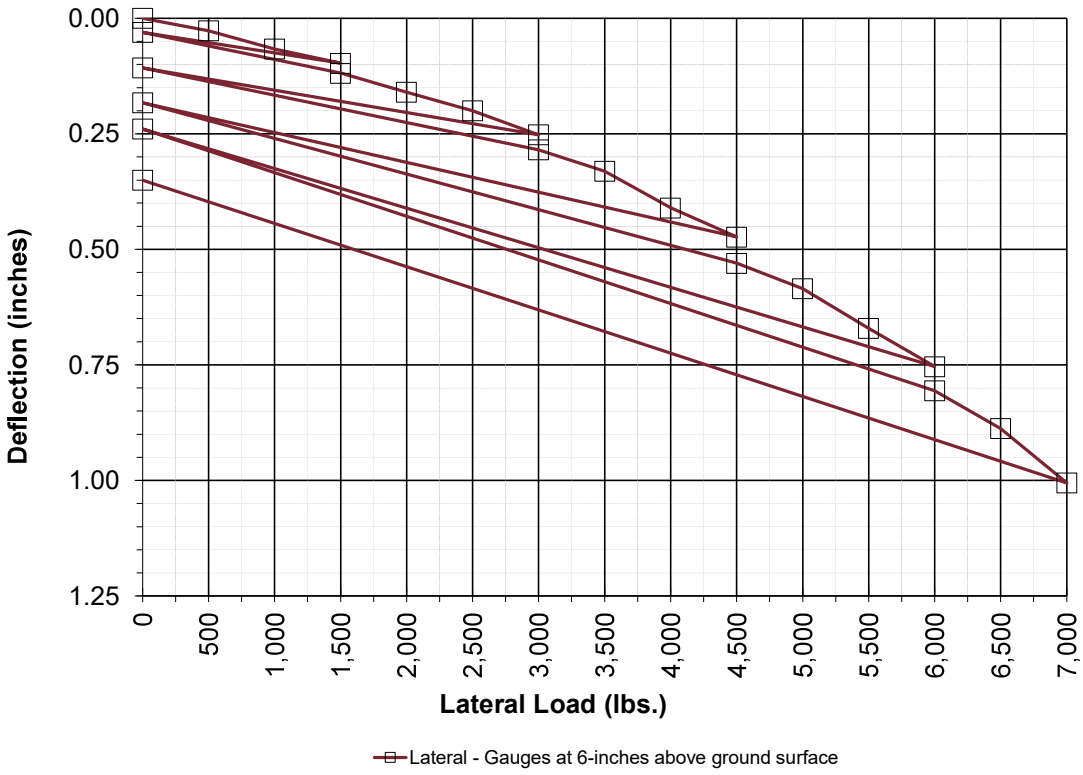
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-11B
Group Latitude [deg.]:	33.6564
Group Longitude [deg.]:	-114.6815
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	46

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.027	
14%	1,000	0.066	
21%	1,500	0.097	
0%	0	0.031	
21%	1,500	0.119	
29%	2,000	0.160	
36%	2,500	0.200	
43%	3,000	0.252	
0%	0	0.107	
43%	3,000	0.285	
50%	3,500	0.331	
57%	4,000	0.410	
64%	4,500	0.474	
0%	0	0.183	
64%	4,500	0.530	
71%	5,000	0.586	
79%	5,500	0.671	
86%	6,000	0.754	
0%	0	0.240	
86%	6,000	0.806	
93%	6,500	0.888	
100%	7,000	1.006	
0%	0	0.350	



Lateral Load Test Result for PLT-12A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

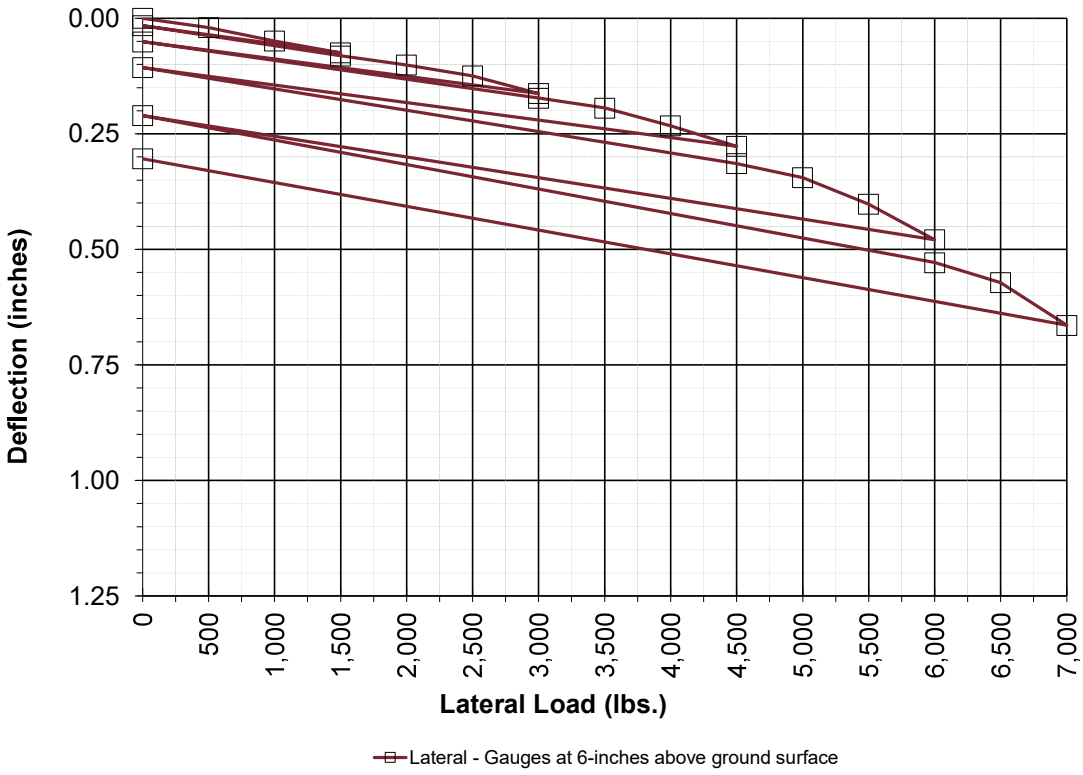
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-12A
Group Latitude [deg.]:	33.6563
Group Longitude [deg.]:	-114.6902
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	19

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.020	
14%	1,000	0.049	
21%	1,500	0.075	
0%	0	0.016	
21%	1,500	0.082	
29%	2,000	0.101	
36%	2,500	0.125	
43%	3,000	0.163	
0%	0	0.051	
43%	3,000	0.173	
50%	3,500	0.194	
57%	4,000	0.233	
64%	4,500	0.277	
0%	0	0.107	
64%	4,500	0.315	
71%	5,000	0.345	
79%	5,500	0.402	
86%	6,000	0.479	
0%	0	0.211	
86%	6,000	0.529	
93%	6,500	0.572	
100%	7,000	0.664	
0%	0	0.304	



Lateral Load Test Result for PLT-12B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

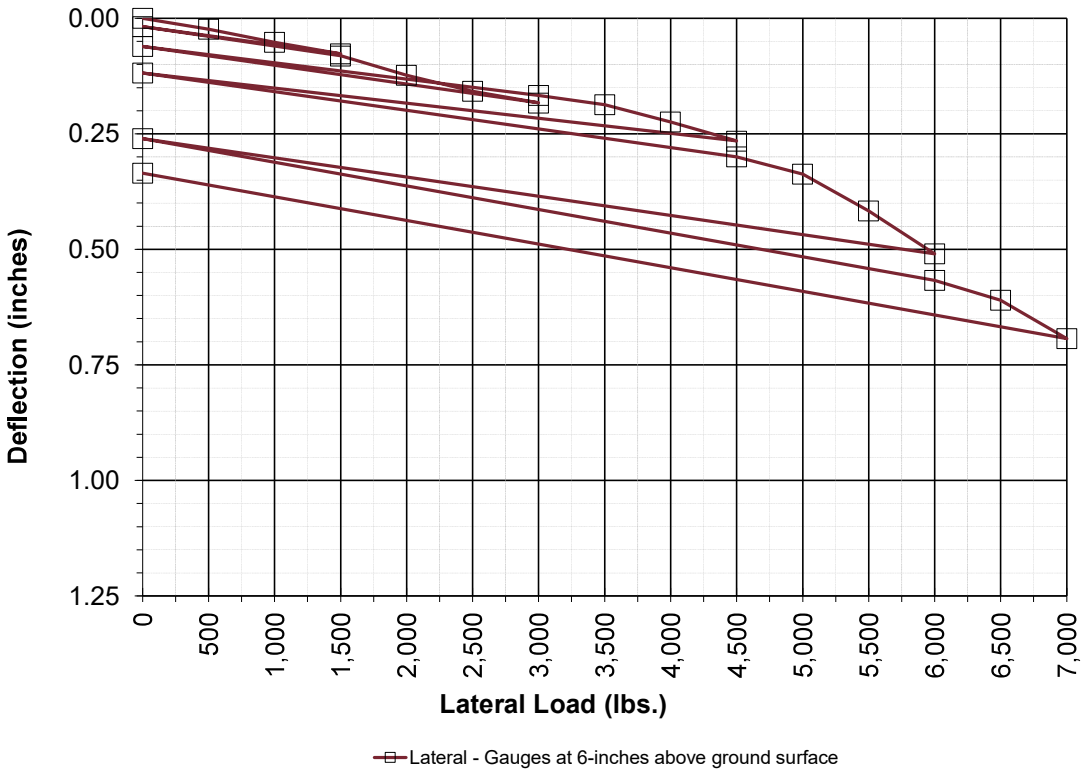
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-12B
Group Latitude [deg.]:	33.6563
Group Longitude [deg.]:	-114.6902
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	44

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.024	
14%	1,000	0.052	
21%	1,500	0.077	
0%	0	0.019	
21%	1,500	0.081	
29%	2,000	0.123	
36%	2,500	0.158	
43%	3,000	0.183	
0%	0	0.061	
43%	3,000	0.167	
50%	3,500	0.187	
57%	4,000	0.225	
64%	4,500	0.266	
0%	0	0.119	
64%	4,500	0.300	
71%	5,000	0.337	
79%	5,500	0.417	
86%	6,000	0.509	
0%	0	0.261	
86%	6,000	0.567	
93%	6,500	0.610	
100%	7,000	0.693	
0%	0	0.335	



Lateral Load Test Result for PLT-13A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

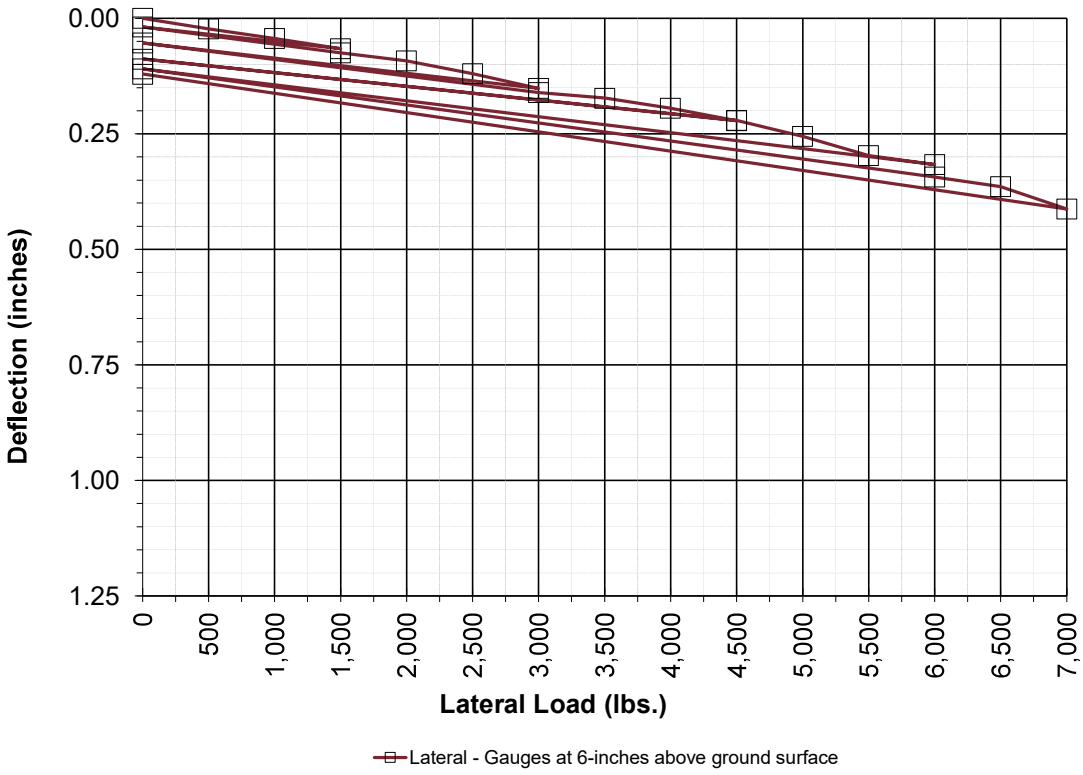
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-13A
Group Latitude [deg.]:	33.6563
Group Longitude [deg.]:	-114.7076
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	19

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.023	
14%	1,000	0.044	
21%	1,500	0.066	
0%	0	0.019	
21%	1,500	0.075	
29%	2,000	0.092	
36%	2,500	0.120	
43%	3,000	0.152	
0%	0	0.054	
43%	3,000	0.161	
50%	3,500	0.173	
57%	4,000	0.195	
64%	4,500	0.222	
0%	0	0.089	
64%	4,500	0.221	
71%	5,000	0.256	
79%	5,500	0.297	
86%	6,000	0.317	
0%	0	0.109	
86%	6,000	0.343	
93%	6,500	0.365	
100%	7,000	0.413	
0%	0	0.121	



Lateral Load Test Result for PLT-13B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

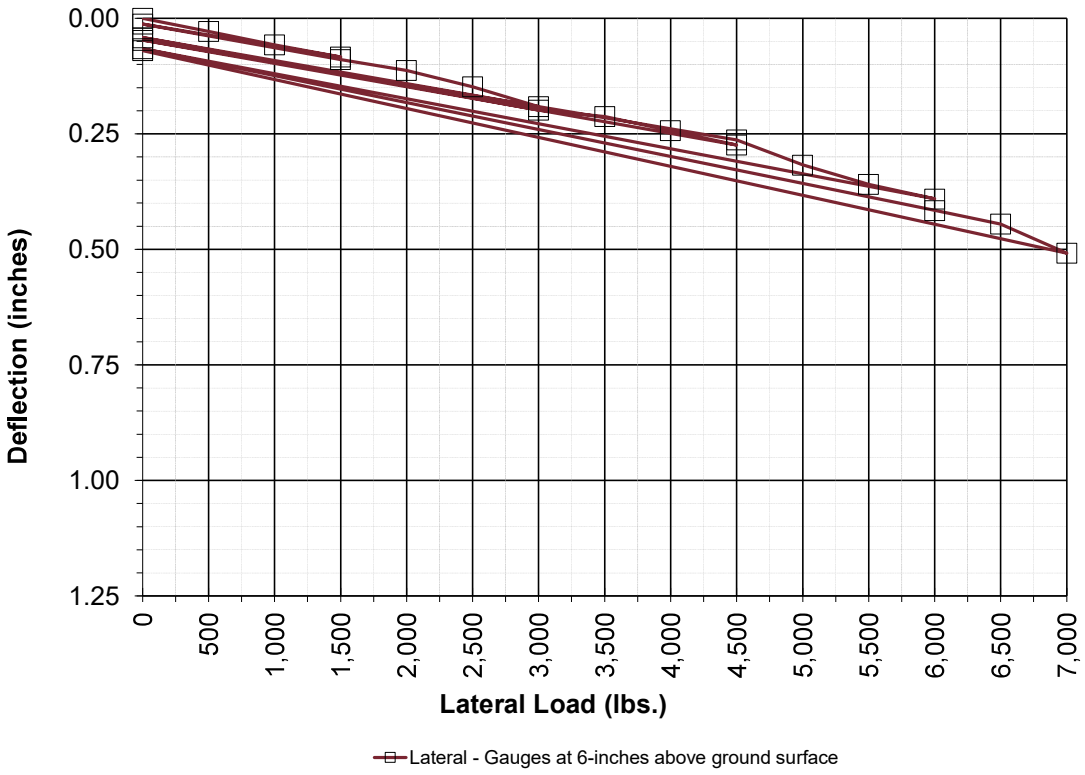
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-13B
Group Latitude [deg.]:	33.6563
Group Longitude [deg.]:	-114.7076
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	36

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.028	
14%	1,000	0.058	
21%	1,500	0.084	
0%	0	0.013	
21%	1,500	0.089	
29%	2,000	0.113	
36%	2,500	0.149	
43%	3,000	0.191	
0%	0	0.042	
43%	3,000	0.199	
50%	3,500	0.212	
57%	4,000	0.243	
64%	4,500	0.274	
0%	0	0.048	
64%	4,500	0.264	
71%	5,000	0.318	
79%	5,500	0.359	
86%	6,000	0.391	
0%	0	0.066	
86%	6,000	0.416	
93%	6,500	0.445	
100%	7,000	0.508	
0%	0	0.070	



Lateral Load Test Result for PLT-14A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

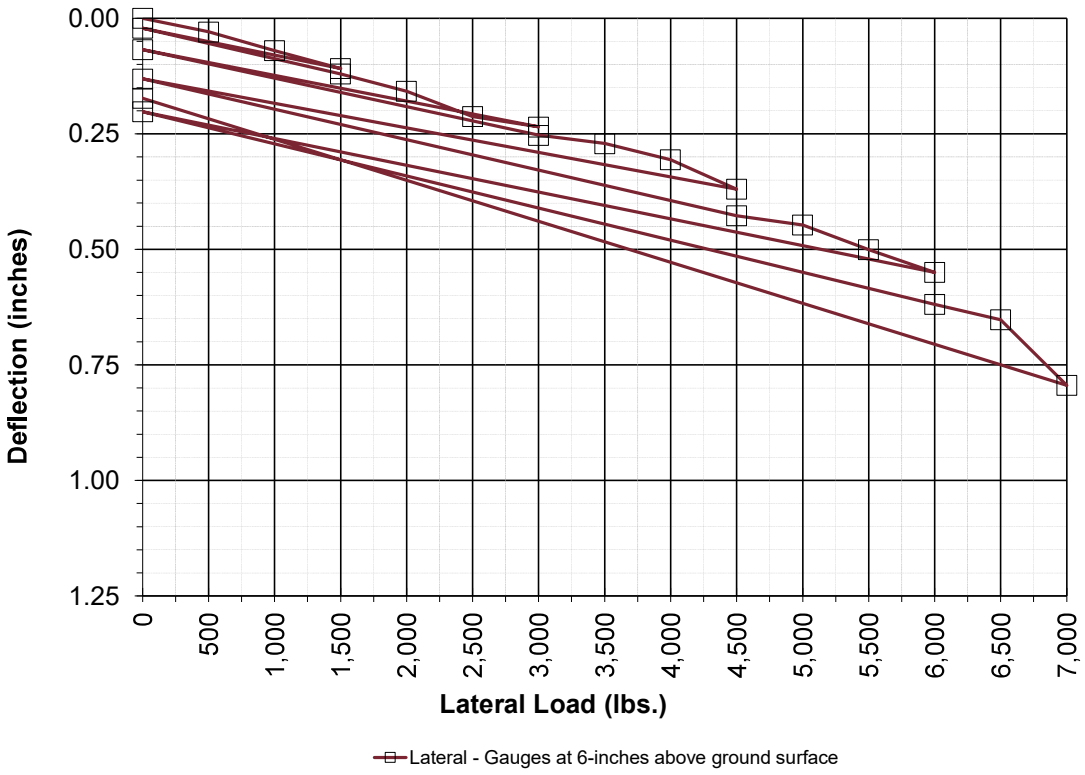
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-14A
Group Latitude [deg.]:	33.6527
Group Longitude [deg.]:	-114.7150
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	16

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.030	
14%	1,000	0.071	
21%	1,500	0.109	
0%	0	0.022	
21%	1,500	0.121	
29%	2,000	0.158	
36%	2,500	0.213	
43%	3,000	0.235	
0%	0	0.068	
43%	3,000	0.253	
50%	3,500	0.271	
57%	4,000	0.306	
64%	4,500	0.370	
0%	0	0.131	
64%	4,500	0.428	
71%	5,000	0.447	
79%	5,500	0.501	
86%	6,000	0.550	
0%	0	0.202	
86%	6,000	0.619	
93%	6,500	0.652	
100%	7,000	0.794	
0%	0	0.174	



Lateral Load Test Result for PLT-14B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

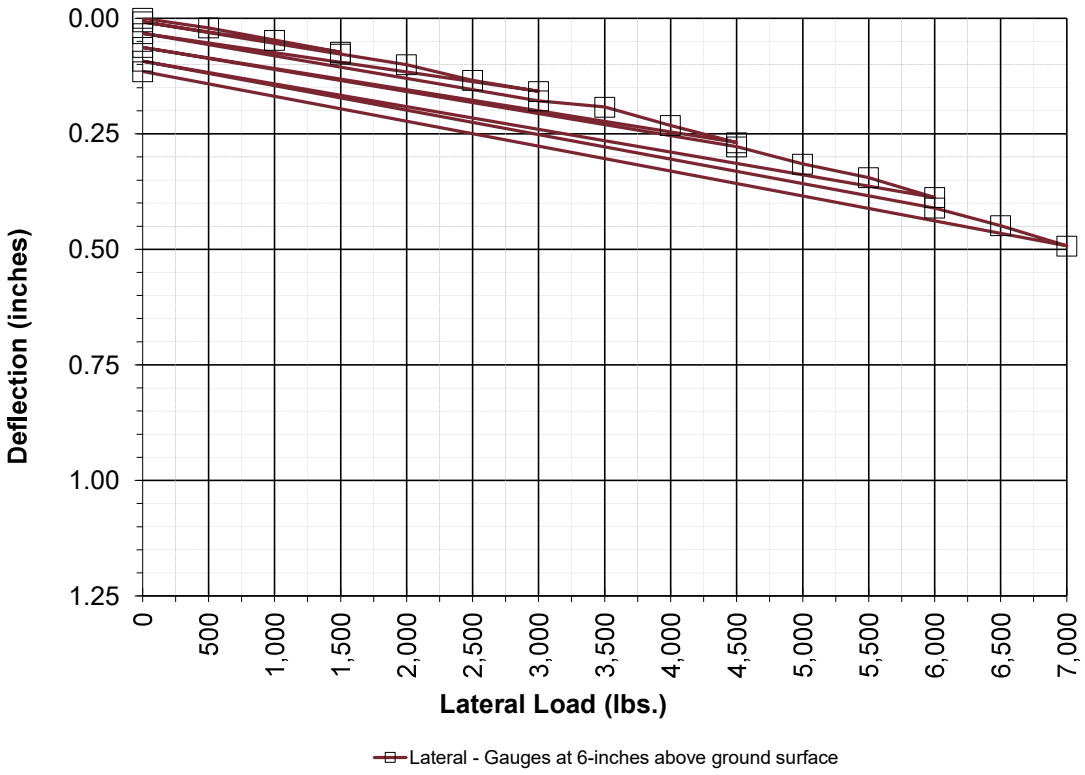
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-14B
Group Latitude [deg.]:	33.6527
Group Longitude [deg.]:	-114.7150
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	40

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.021	
14%	1,000	0.048	
21%	1,500	0.074	
0%	0	0.008	
21%	1,500	0.078	
29%	2,000	0.100	
36%	2,500	0.134	
43%	3,000	0.159	
0%	0	0.033	
43%	3,000	0.179	
50%	3,500	0.192	
57%	4,000	0.232	
64%	4,500	0.269	
0%	0	0.063	
64%	4,500	0.278	
71%	5,000	0.316	
79%	5,500	0.345	
86%	6,000	0.388	
0%	0	0.093	
86%	6,000	0.411	
93%	6,500	0.449	
100%	7,000	0.493	
0%	0	0.116	



Lateral Load Test Result for PLT-15A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

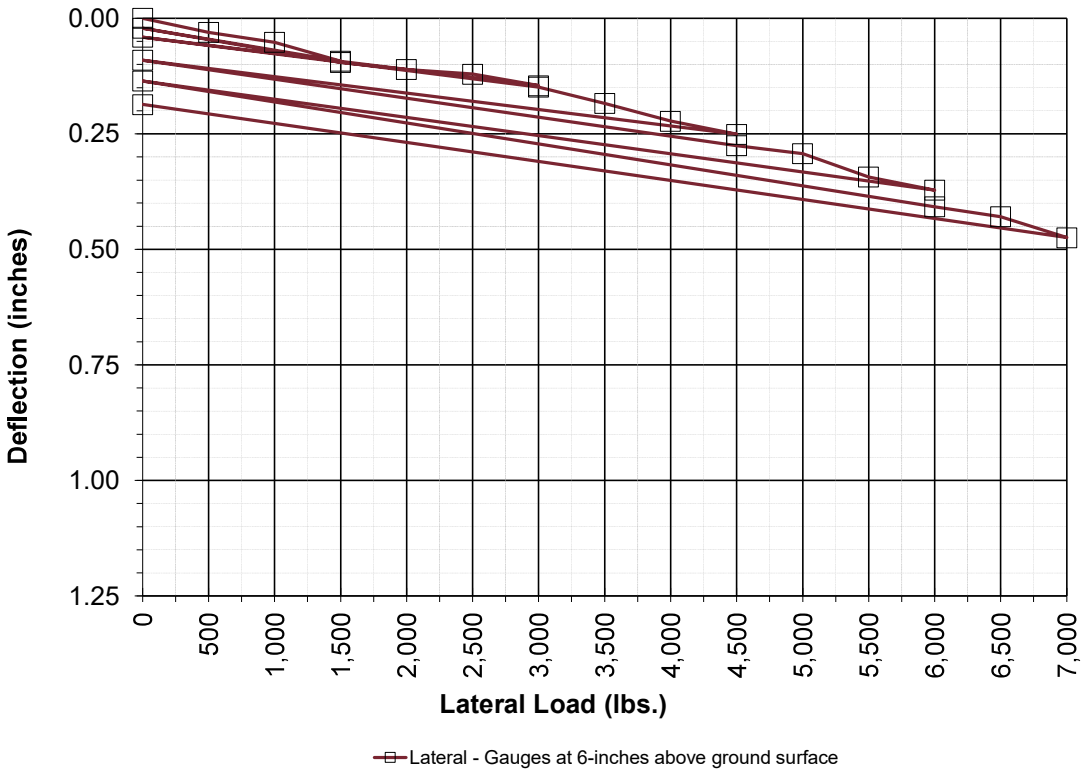
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-15A
Group Latitude [deg.]:	33.6527
Group Longitude [deg.]:	-114.7050
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	26

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.031	
14%	1,000	0.052	
21%	1,500	0.093	
0%	0	0.022	
21%	1,500	0.096	
29%	2,000	0.111	
36%	2,500	0.121	
43%	3,000	0.146	
0%	0	0.041	
43%	3,000	0.149	
50%	3,500	0.184	
57%	4,000	0.223	
64%	4,500	0.251	
0%	0	0.091	
64%	4,500	0.276	
71%	5,000	0.293	
79%	5,500	0.343	
86%	6,000	0.372	
0%	0	0.135	
86%	6,000	0.408	
93%	6,500	0.430	
100%	7,000	0.475	
0%	0	0.187	



Lateral Load Test Result for PLT-15B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

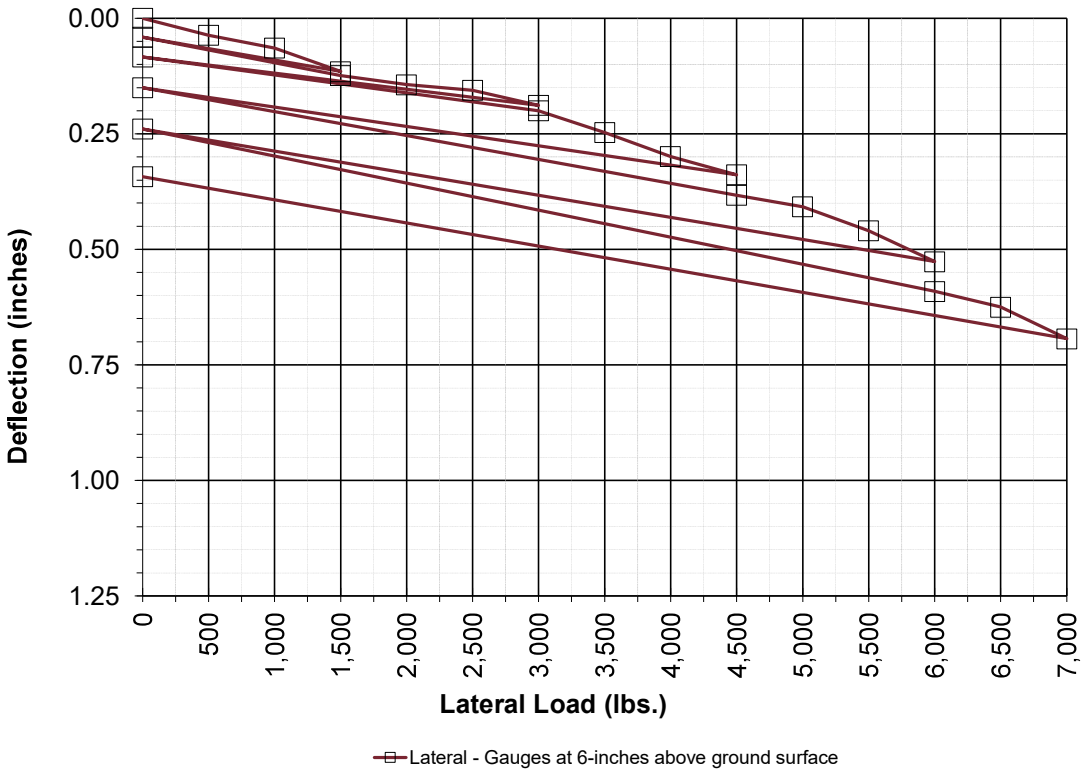
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-15B
Group Latitude [deg.]:	33.6527
Group Longitude [deg.]:	-114.7050
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	40

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.037	
14%	1,000	0.065	
21%	1,500	0.115	
0%	0	0.041	
21%	1,500	0.124	
29%	2,000	0.144	
36%	2,500	0.156	
43%	3,000	0.189	
0%	0	0.084	
43%	3,000	0.200	
50%	3,500	0.247	
57%	4,000	0.299	
64%	4,500	0.339	
0%	0	0.151	
64%	4,500	0.383	
71%	5,000	0.408	
79%	5,500	0.460	
86%	6,000	0.527	
0%	0	0.240	
86%	6,000	0.591	
93%	6,500	0.625	
100%	7,000	0.694	
0%	0	0.343	



Lateral Load Test Result for PLT-16A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

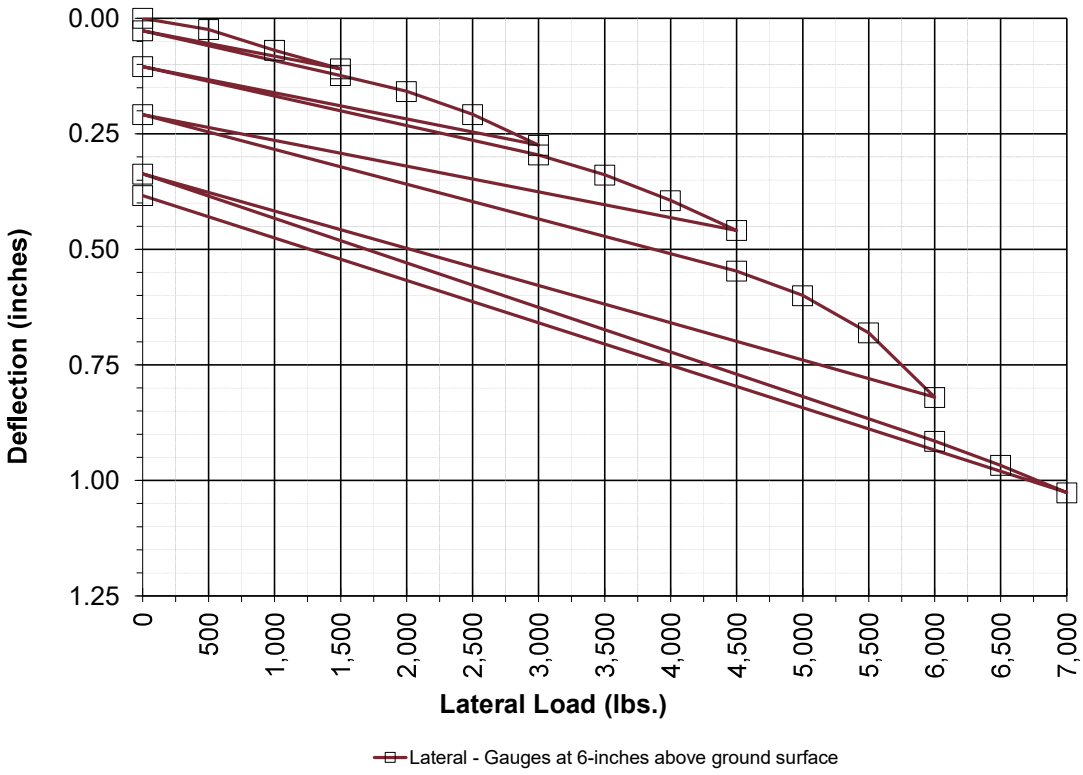
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-16A
Group Latitude [deg.]:	33.6496
Group Longitude [deg.]:	-114.7092
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	11

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.024	
14%	1,000	0.070	
21%	1,500	0.110	
0%	0	0.027	
21%	1,500	0.124	
29%	2,000	0.159	
36%	2,500	0.208	
43%	3,000	0.275	
0%	0	0.105	
43%	3,000	0.296	
50%	3,500	0.339	
57%	4,000	0.395	
64%	4,500	0.459	
0%	0	0.209	
64%	4,500	0.547	
71%	5,000	0.600	
79%	5,500	0.681	
86%	6,000	0.820	
0%	0	0.337	
86%	6,000	0.915	
93%	6,500	0.967	
100%	7,000	1.027	
0%	0	0.384	



Lateral Load Test Result for PLT-16B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

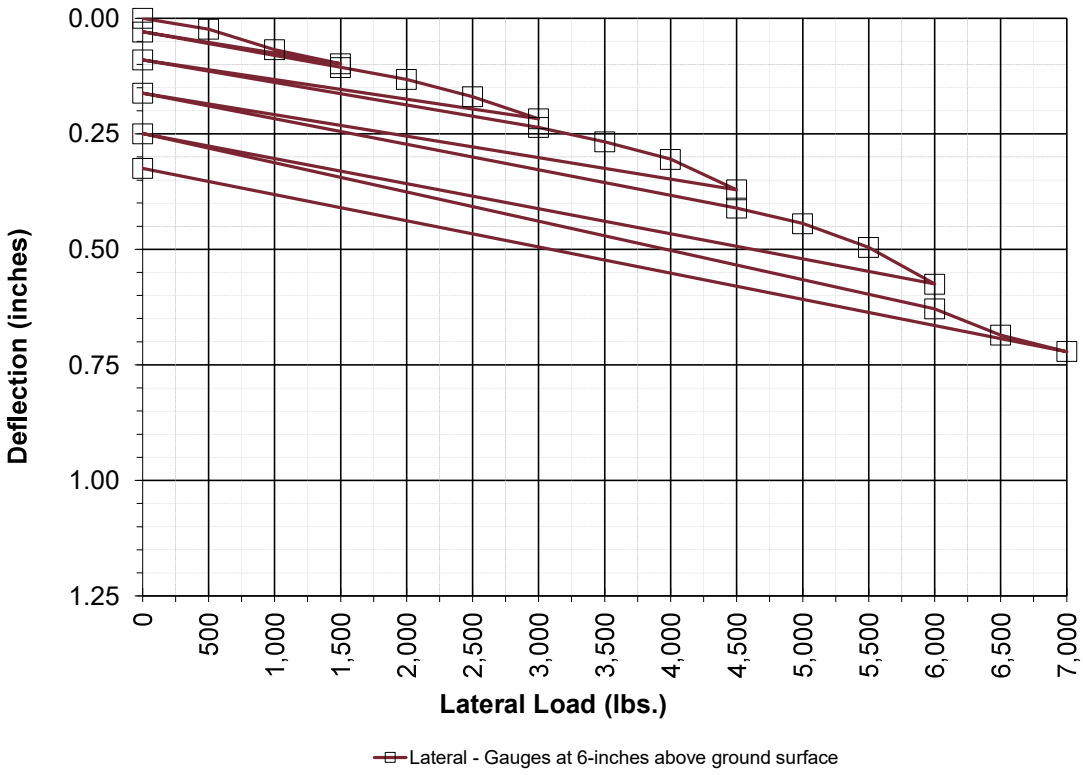
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-16B
Group Latitude [deg.]:	33.6496
Group Longitude [deg.]:	-114.7092
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	32

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.024	
14%	1,000	0.068	
21%	1,500	0.098	
0%	0	0.030	
21%	1,500	0.107	
29%	2,000	0.133	
36%	2,500	0.170	
43%	3,000	0.218	
0%	0	0.090	
43%	3,000	0.237	
50%	3,500	0.268	
57%	4,000	0.305	
64%	4,500	0.371	
0%	0	0.162	
64%	4,500	0.411	
71%	5,000	0.444	
79%	5,500	0.496	
86%	6,000	0.575	
0%	0	0.250	
86%	6,000	0.629	
93%	6,500	0.686	
100%	7,000	0.721	
0%	0	0.325	



Lateral Load Test Result for PLT-17A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

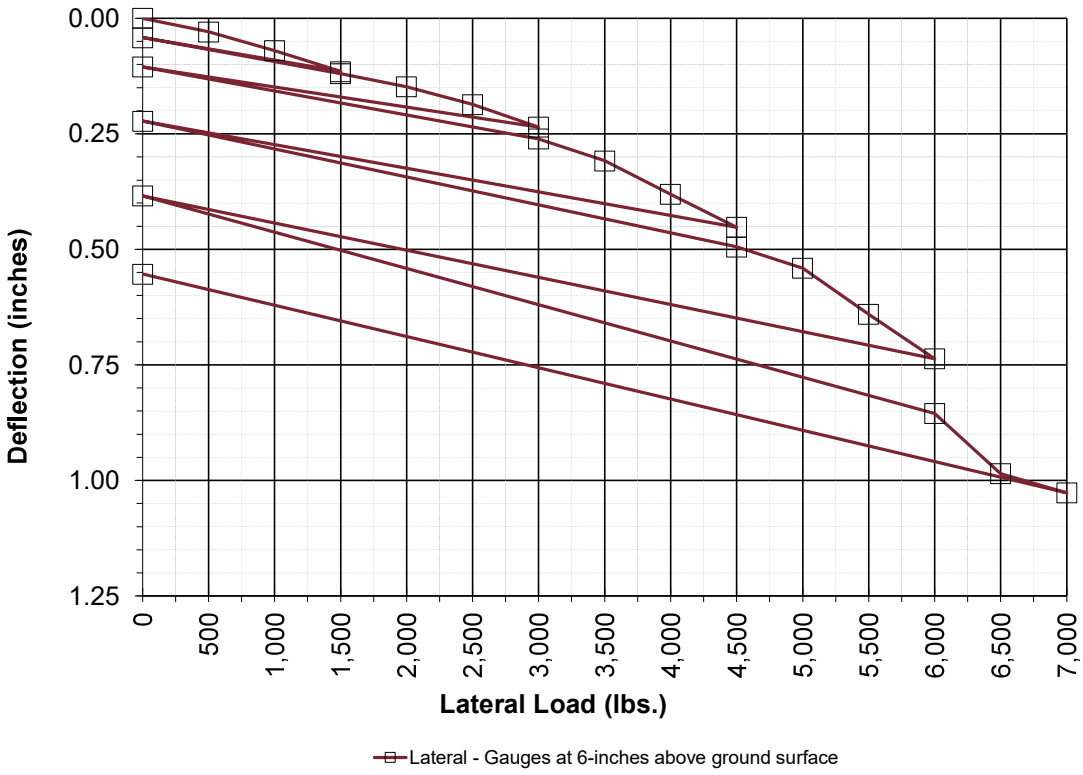
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-17A
Group Latitude [deg.]:	33.6516
Group Longitude [deg.]:	-114.6847
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	25

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.030	
14%	1,000	0.071	
21%	1,500	0.116	
0%	0	0.042	
21%	1,500	0.120	
29%	2,000	0.149	
36%	2,500	0.187	
43%	3,000	0.236	
0%	0	0.105	
43%	3,000	0.261	
50%	3,500	0.309	
57%	4,000	0.381	
64%	4,500	0.452	
0%	0	0.223	
64%	4,500	0.495	
71%	5,000	0.541	
79%	5,500	0.640	
86%	6,000	0.737	
0%	0	0.384	
86%	6,000	0.855	
93%	6,500	0.985	
100%	7,000	1.027	
0%	0	0.554	



Lateral Load Test Result for PLT-17B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

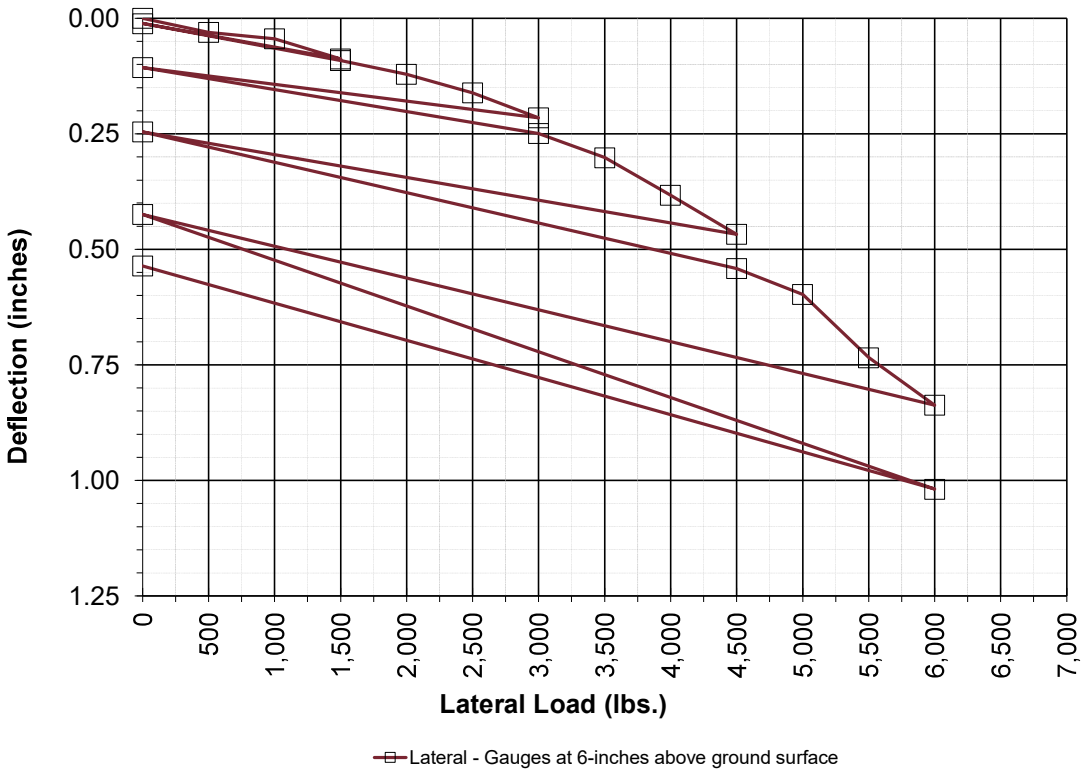
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-17B
Group Latitude [deg.]:	33.6516
Group Longitude [deg.]:	-114.6847
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	34

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.030	
14%	1,000	0.045	
21%	1,500	0.088	
0%	0	0.012	
21%	1,500	0.092	
29%	2,000	0.122	
36%	2,500	0.162	
43%	3,000	0.215	
0%	0	0.107	
43%	3,000	0.250	
50%	3,500	0.302	
57%	4,000	0.383	
64%	4,500	0.468	
0%	0	0.246	
64%	4,500	0.542	
71%	5,000	0.598	
79%	5,500	0.735	
86%	6,000	0.837	
0%	0	0.425	
86%	6,000	1.019	
93%	6,500		
100%	7,000		
0%	0	0.536	



Lateral Load Test Result for PLT-18A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

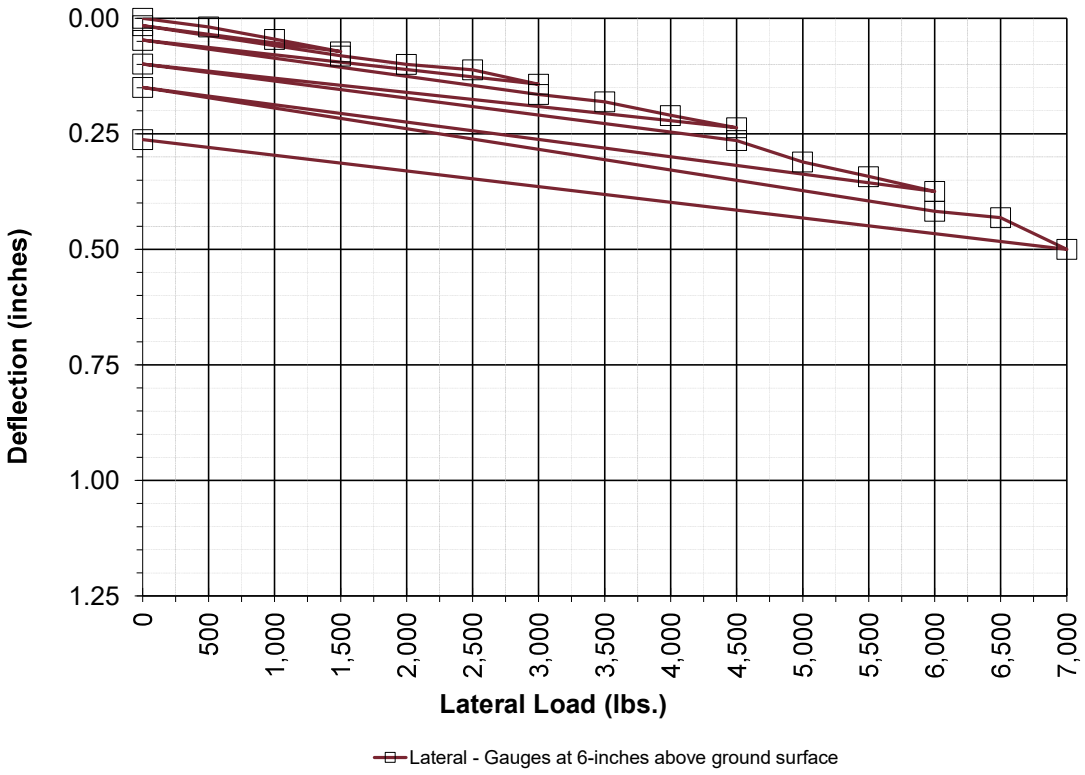
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-18A
Group Latitude [deg.]:	33.6528
Group Longitude [deg.]:	-114.6700
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	24

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.019	
14%	1,000	0.046	
21%	1,500	0.073	
0%	0	0.016	
21%	1,500	0.082	
29%	2,000	0.100	
36%	2,500	0.112	
43%	3,000	0.143	
0%	0	0.048	
43%	3,000	0.165	
50%	3,500	0.181	
57%	4,000	0.210	
64%	4,500	0.237	
0%	0	0.099	
64%	4,500	0.265	
71%	5,000	0.311	
79%	5,500	0.342	
86%	6,000	0.375	
0%	0	0.150	
86%	6,000	0.418	
93%	6,500	0.432	
100%	7,000	0.500	
0%	0	0.263	



Lateral Load Test Result for PLT-18B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

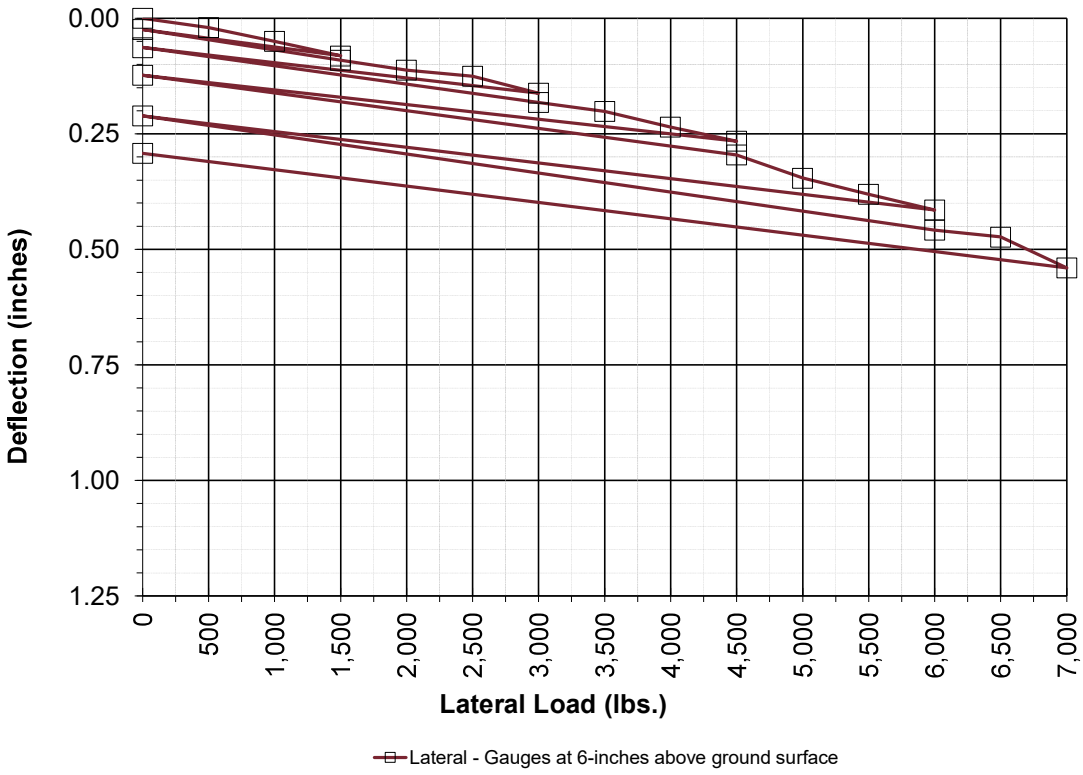
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-18B
Group Latitude [deg.]:	33.6528
Group Longitude [deg.]:	-114.6700
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	51

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.020	
14%	1,000	0.050	
21%	1,500	0.081	
0%	0	0.025	
21%	1,500	0.091	
29%	2,000	0.112	
36%	2,500	0.126	
43%	3,000	0.162	
0%	0	0.063	
43%	3,000	0.182	
50%	3,500	0.202	
57%	4,000	0.236	
64%	4,500	0.266	
0%	0	0.124	
64%	4,500	0.296	
71%	5,000	0.346	
79%	5,500	0.381	
86%	6,000	0.415	
0%	0	0.212	
86%	6,000	0.459	
93%	6,500	0.474	
100%	7,000	0.541	
0%	0	0.292	



Lateral Load Test Result for PLT-19A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

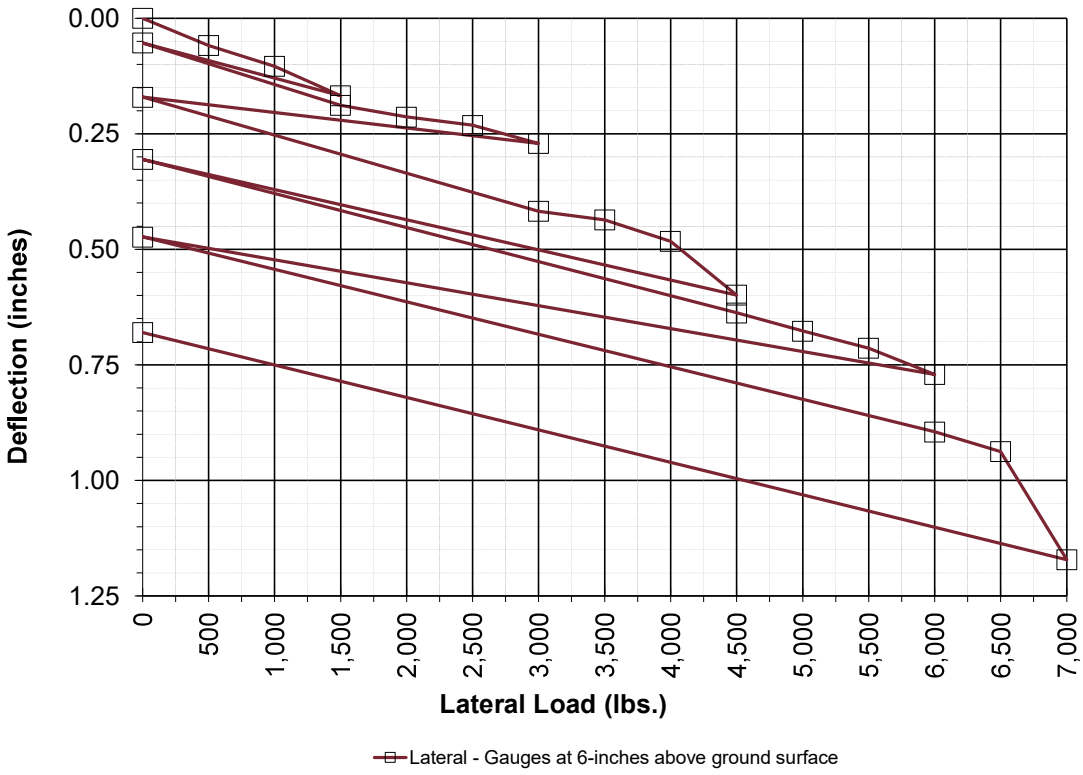
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-19A
Group Latitude [deg.]:	33.6487
Group Longitude [deg.]:	-114.6659
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	33

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.059	
14%	1,000	0.104	
21%	1,500	0.168	
0%	0	0.054	
21%	1,500	0.188	
29%	2,000	0.214	
36%	2,500	0.232	
43%	3,000	0.271	
0%	0	0.171	
43%	3,000	0.418	
50%	3,500	0.436	
57%	4,000	0.483	
64%	4,500	0.599	
0%	0	0.306	
64%	4,500	0.637	
71%	5,000	0.677	
79%	5,500	0.714	
86%	6,000	0.771	
0%	0	0.474	
86%	6,000	0.895	
93%	6,500	0.938	
100%	7,000	1.172	
0%	0	0.680	



Lateral Load Test Result for PLT-19B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

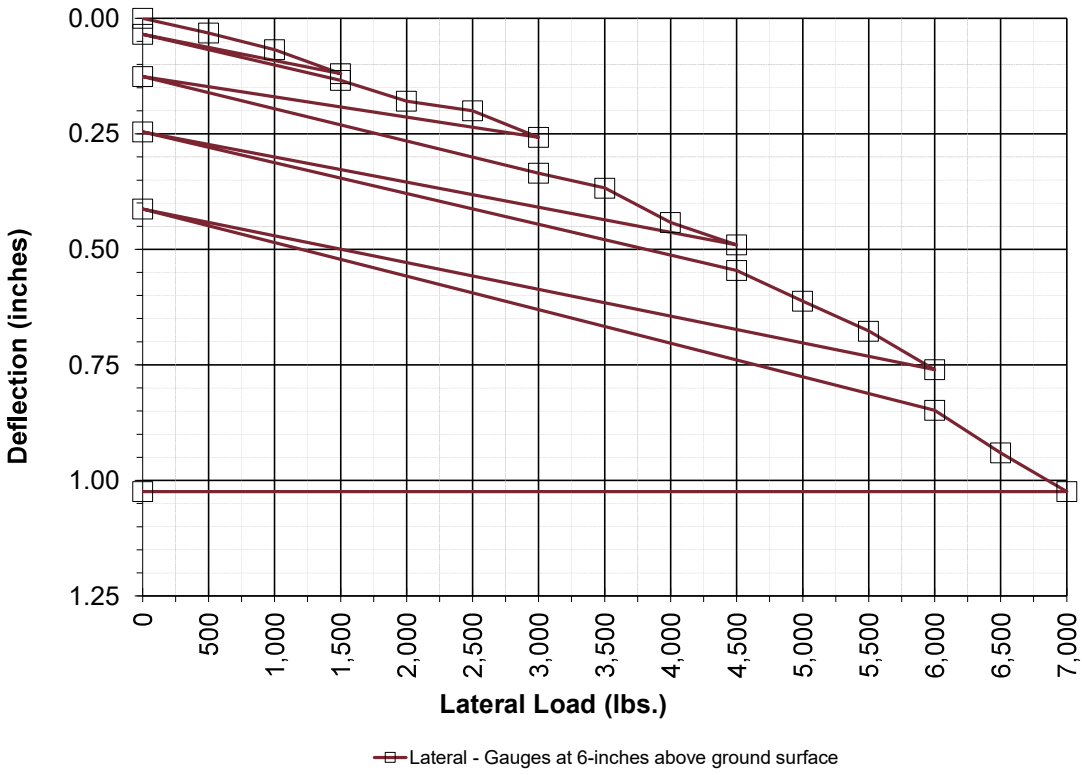
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-19B
Group Latitude [deg.]:	33.6487
Group Longitude [deg.]:	-114.6659
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	66

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.032	
14%	1,000	0.068	
21%	1,500	0.120	
0%	0	0.035	
21%	1,500	0.134	
29%	2,000	0.180	
36%	2,500	0.200	
43%	3,000	0.257	
0%	0	0.126	
43%	3,000	0.335	
50%	3,500	0.367	
57%	4,000	0.442	
64%	4,500	0.491	
0%	0	0.246	
64%	4,500	0.546	
71%	5,000	0.612	
79%	5,500	0.677	
86%	6,000	0.760	
0%	0	0.413	
86%	6,000	0.848	
93%	6,500	0.941	
100%	7,000	1.024	
0%	0	1.024	



Lateral Load Test Result for PLT-20A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

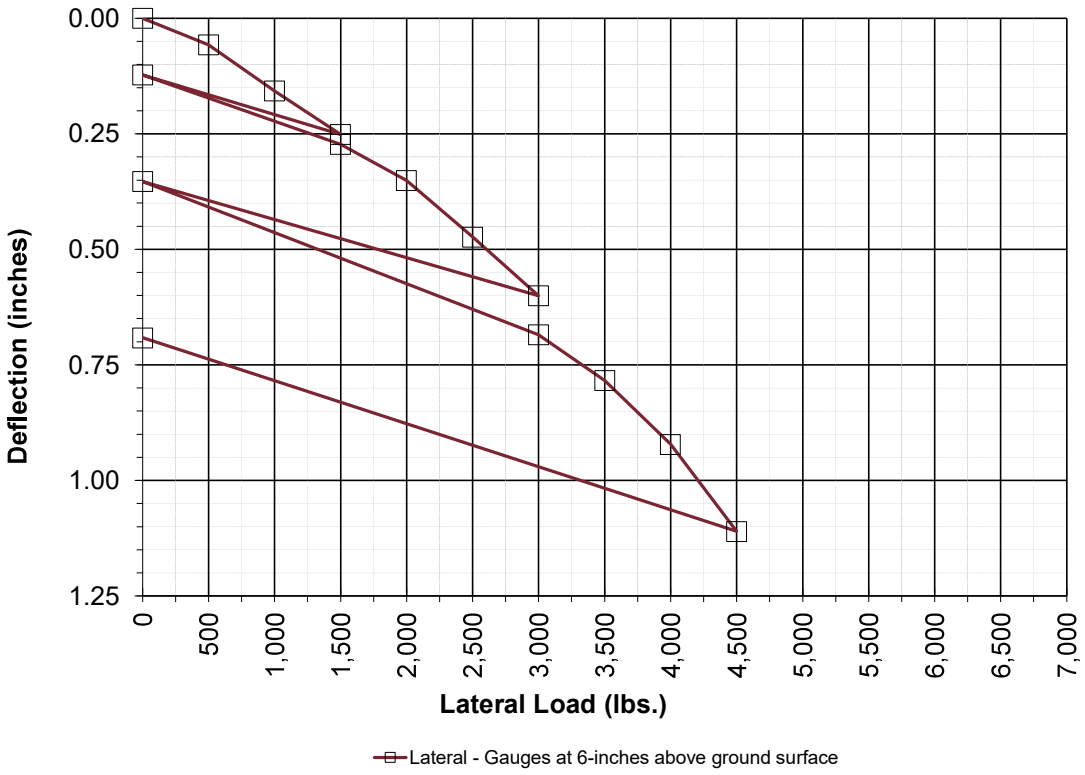
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-20A
Group Latitude [deg.]:	33.6492
Group Longitude [deg.]:	-114.6801
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	29

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.057	
14%	1,000	0.157	
21%	1,500	0.251	
0%	0	0.123	
21%	1,500	0.273	
29%	2,000	0.351	
36%	2,500	0.473	
43%	3,000	0.600	
0%	0	0.353	
43%	3,000	0.685	
50%	3,500	0.784	
57%	4,000	0.922	
64%	4,500	1.110	
0%	0		
64%	4,500		
71%	5,000		
79%	5,500		
86%	6,000		
0%	0		
86%	6,000		
93%	6,500		
100%	7,000		
0%	0	0.692	



Lateral Load Test Result for PLT-20B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

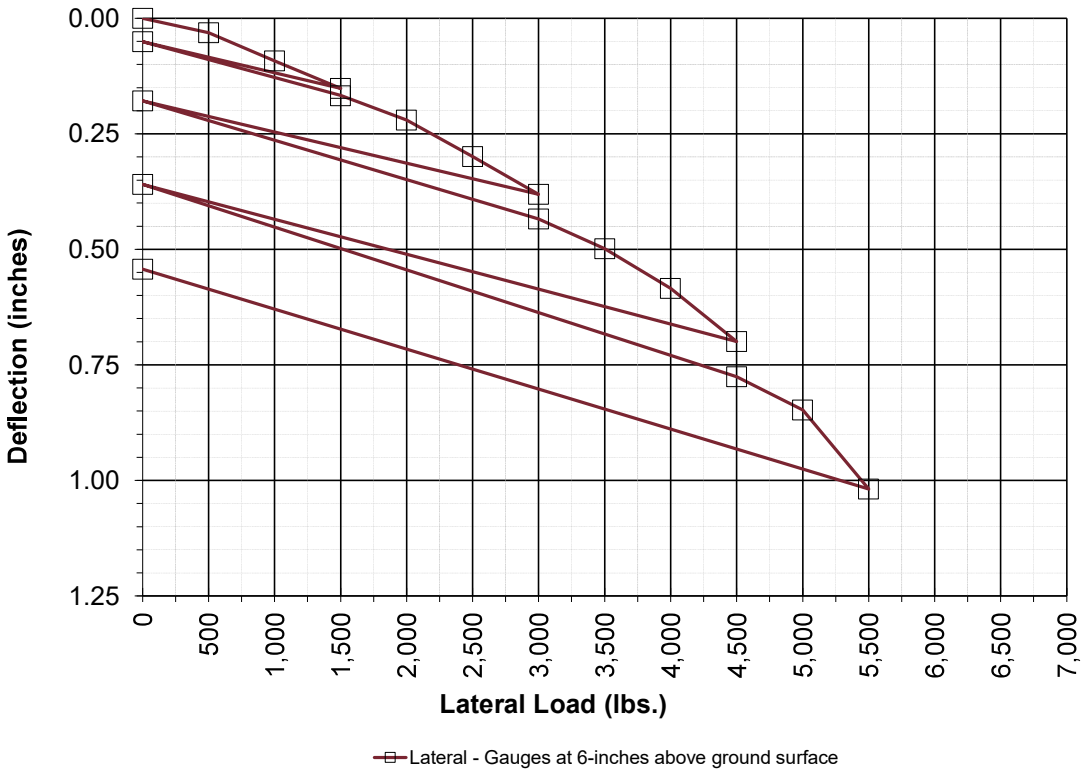
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-20B
Group Latitude [deg.]:	33.6492
Group Longitude [deg.]:	-114.6801
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	50

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.031	
14%	1,000	0.092	
21%	1,500	0.152	
0%	0	0.051	
21%	1,500	0.167	
29%	2,000	0.220	
36%	2,500	0.299	
43%	3,000	0.381	
0%	0	0.179	
43%	3,000	0.435	
50%	3,500	0.499	
57%	4,000	0.585	
64%	4,500	0.700	
0%	0	0.360	
64%	4,500	0.776	
71%	5,000	0.847	
79%	5,500	1.019	
86%	6,000		
0%	0		
86%	6,000		
93%	6,500		
100%	7,000		
0%	0	0.543	



Lateral Load Test Result for PLT-21A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

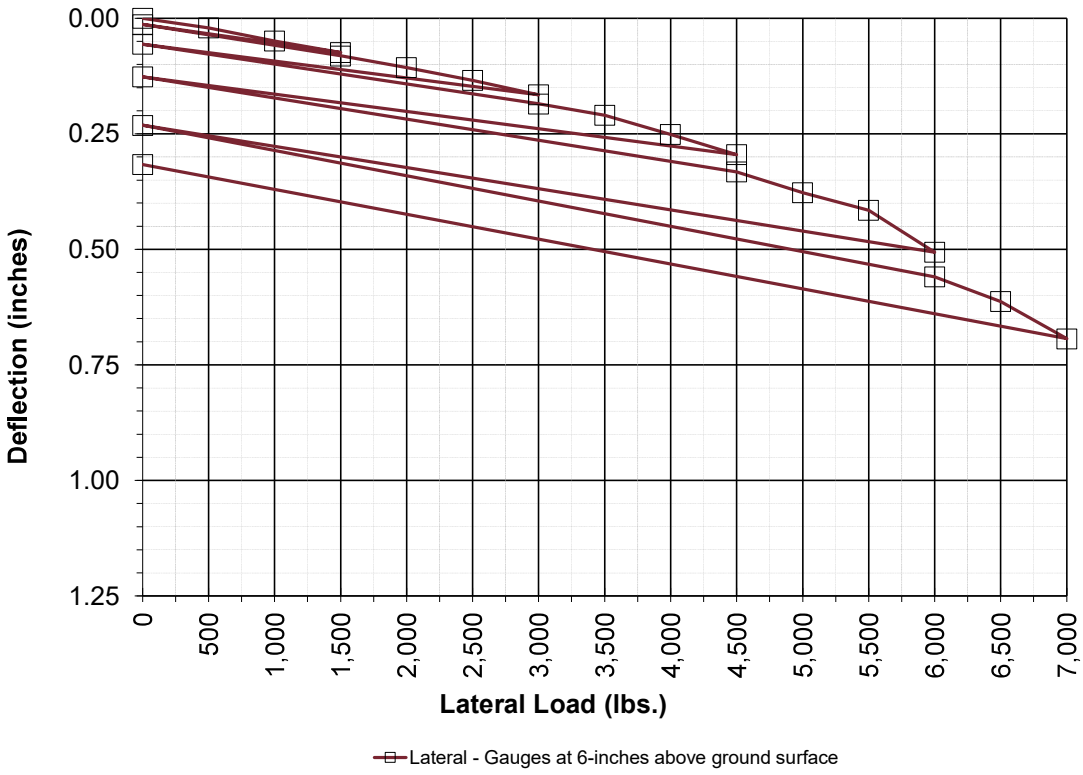
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-21A
Group Latitude [deg.]:	33.6492
Group Longitude [deg.]:	-114.6901
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	23

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.021	
14%	1,000	0.049	
21%	1,500	0.074	
0%	0	0.014	
21%	1,500	0.081	
29%	2,000	0.107	
36%	2,500	0.135	
43%	3,000	0.166	
0%	0	0.056	
43%	3,000	0.186	
50%	3,500	0.210	
57%	4,000	0.252	
64%	4,500	0.295	
0%	0	0.127	
64%	4,500	0.332	
71%	5,000	0.378	
79%	5,500	0.416	
86%	6,000	0.506	
0%	0	0.232	
86%	6,000	0.560	
93%	6,500	0.613	
100%	7,000	0.694	
0%	0	0.317	



Lateral Load Test Result for PLT-21B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

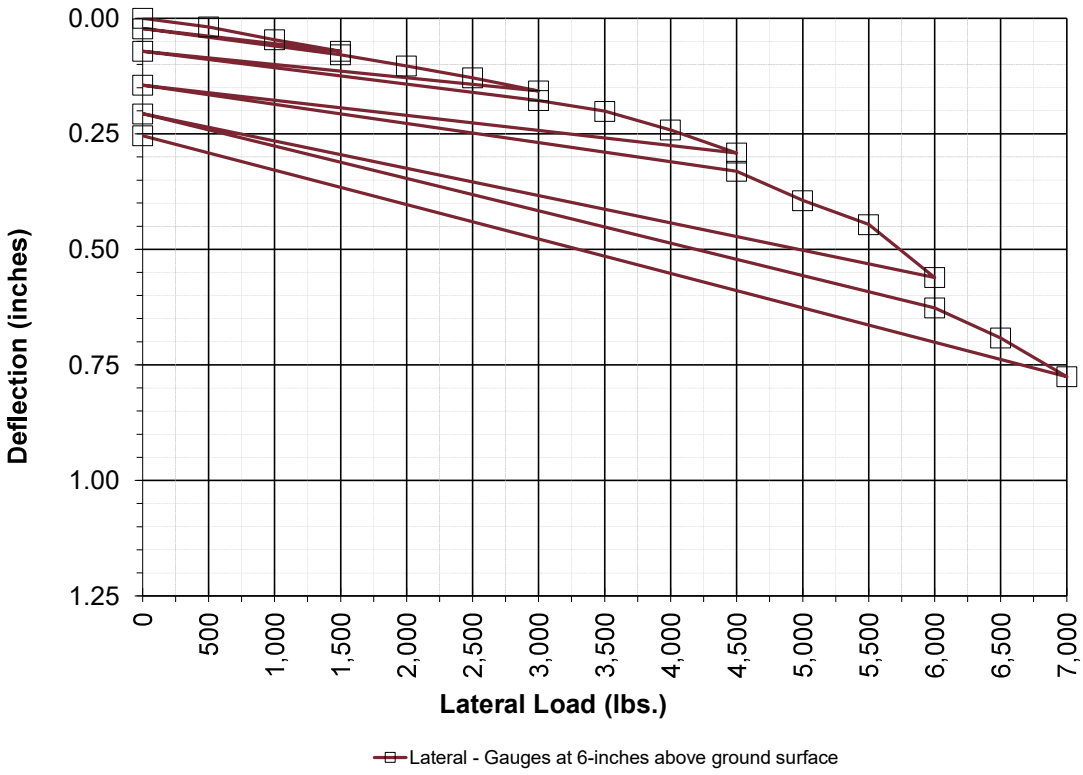
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-21B
Group Latitude [deg.]:	33.6492
Group Longitude [deg.]:	-114.6901
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	52

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.019	
14%	1,000	0.046	
21%	1,500	0.071	
0%	0	0.023	
21%	1,500	0.079	
29%	2,000	0.103	
36%	2,500	0.129	
43%	3,000	0.157	
0%	0	0.072	
43%	3,000	0.178	
50%	3,500	0.201	
57%	4,000	0.242	
64%	4,500	0.292	
0%	0	0.145	
64%	4,500	0.331	
71%	5,000	0.395	
79%	5,500	0.446	
86%	6,000	0.561	
0%	0	0.207	
86%	6,000	0.627	
93%	6,500	0.692	
100%	7,000	0.776	
0%	0	0.254	



Lateral Load Test Result for PLT-22A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

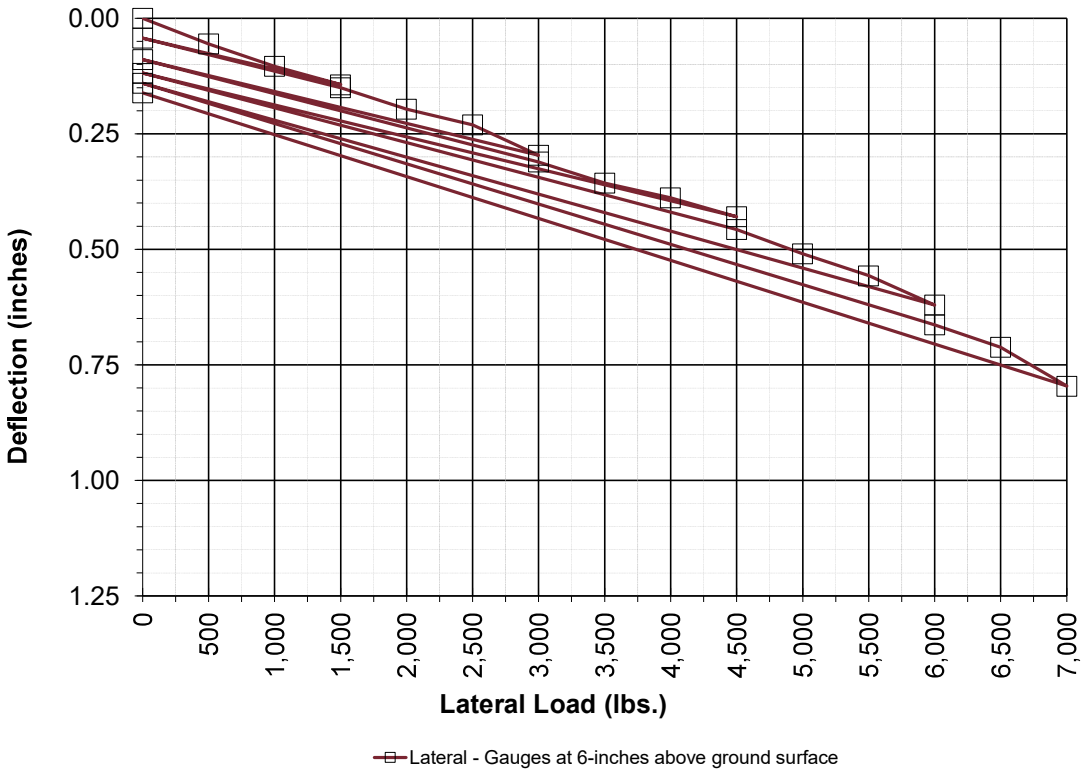
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-22A
Group Latitude [deg.]:	33.6455
Group Longitude [deg.]:	-114.7150
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	20

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.055	
14%	1,000	0.104	
21%	1,500	0.144	
0%	0	0.043	
21%	1,500	0.151	
29%	2,000	0.197	
36%	2,500	0.231	
43%	3,000	0.297	
0%	0	0.090	
43%	3,000	0.311	
50%	3,500	0.357	
57%	4,000	0.388	
64%	4,500	0.430	
0%	0	0.119	
64%	4,500	0.457	
71%	5,000	0.510	
79%	5,500	0.557	
86%	6,000	0.621	
0%	0	0.141	
86%	6,000	0.664	
93%	6,500	0.712	
100%	7,000	0.796	
0%	0	0.161	



Lateral Load Test Result for PLT-22B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

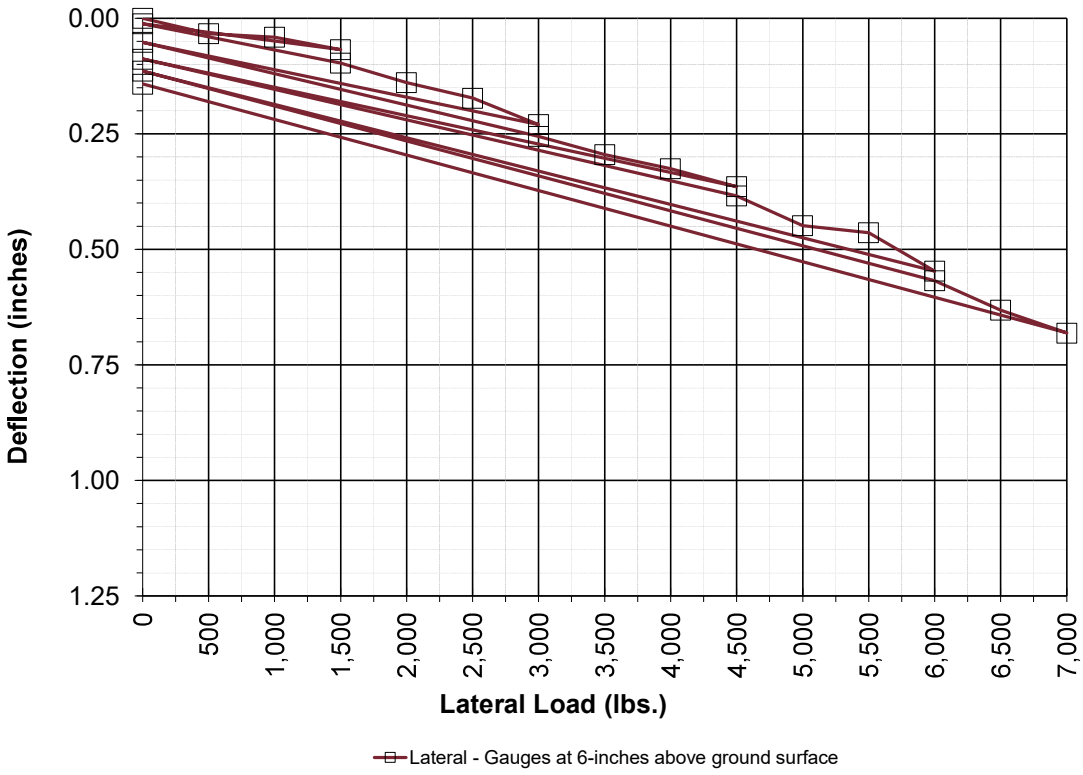
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-22B
Group Latitude [deg.]:	33.6455
Group Longitude [deg.]:	-114.7150
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	40

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.034	
14%	1,000	0.041	
21%	1,500	0.068	
0%	0	0.012	
21%	1,500	0.097	
29%	2,000	0.140	
36%	2,500	0.173	
43%	3,000	0.230	
0%	0	0.052	
43%	3,000	0.256	
50%	3,500	0.295	
57%	4,000	0.325	
64%	4,500	0.365	
0%	0	0.088	
64%	4,500	0.384	
71%	5,000	0.449	
79%	5,500	0.464	
86%	6,000	0.547	
0%	0	0.114	
86%	6,000	0.568	
93%	6,500	0.632	
100%	7,000	0.681	
0%	0	0.142	



Lateral Load Test Result for PLT-23A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

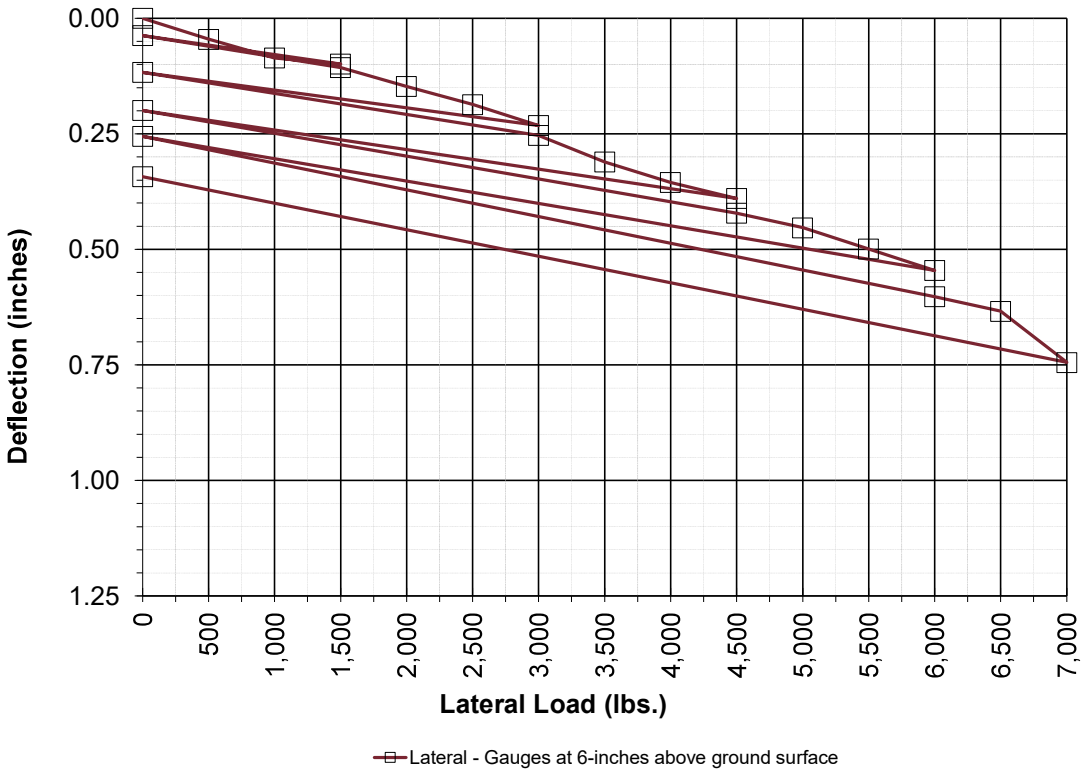
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/3/2023

Pile Information

Pile ID:	PLT-23A
Group Latitude [deg.]:	33.6383
Group Longitude [deg.]:	-114.6951
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	22

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.045	
14%	1,000	0.086	
21%	1,500	0.099	
0%	0	0.038	
21%	1,500	0.107	
29%	2,000	0.148	
36%	2,500	0.187	
43%	3,000	0.232	
0%	0	0.117	
43%	3,000	0.254	
50%	3,500	0.311	
57%	4,000	0.356	
64%	4,500	0.390	
0%	0	0.200	
64%	4,500	0.422	
71%	5,000	0.453	
79%	5,500	0.499	
86%	6,000	0.546	
0%	0	0.255	
86%	6,000	0.603	
93%	6,500	0.634	
100%	7,000	0.745	
0%	0	0.343	



Lateral Load Test Result for PLT-23B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

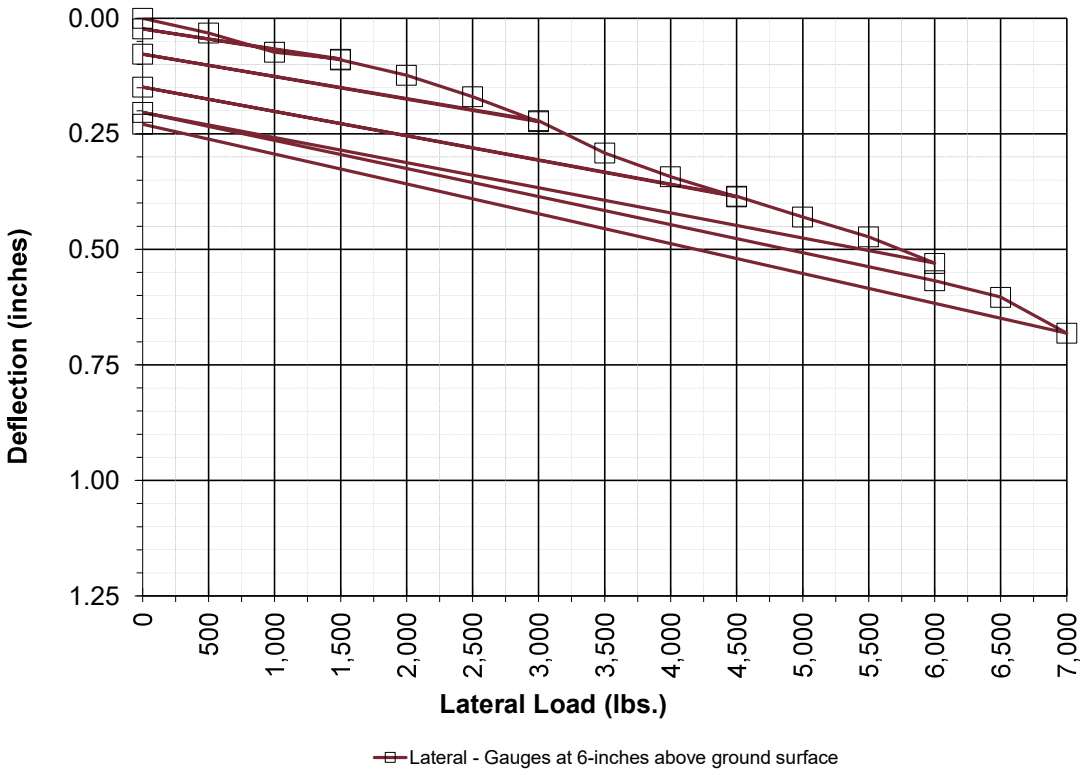
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/3/2023

Pile Information

Pile ID:	PLT-23B
Group Latitude [deg.]:	33.6383
Group Longitude [deg.]:	-114.6951
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	66

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.032	
14%	1,000	0.074	
21%	1,500	0.089	
0%	0	0.023	
21%	1,500	0.090	
29%	2,000	0.124	
36%	2,500	0.170	
43%	3,000	0.224	
0%	0	0.078	
43%	3,000	0.221	
50%	3,500	0.292	
57%	4,000	0.343	
64%	4,500	0.387	
0%	0	0.149	
64%	4,500	0.385	
71%	5,000	0.430	
79%	5,500	0.473	
86%	6,000	0.530	
0%	0	0.204	
86%	6,000	0.568	
93%	6,500	0.603	
100%	7,000	0.682	
0%	0	0.229	



Lateral Load Test Result for PLT-24A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

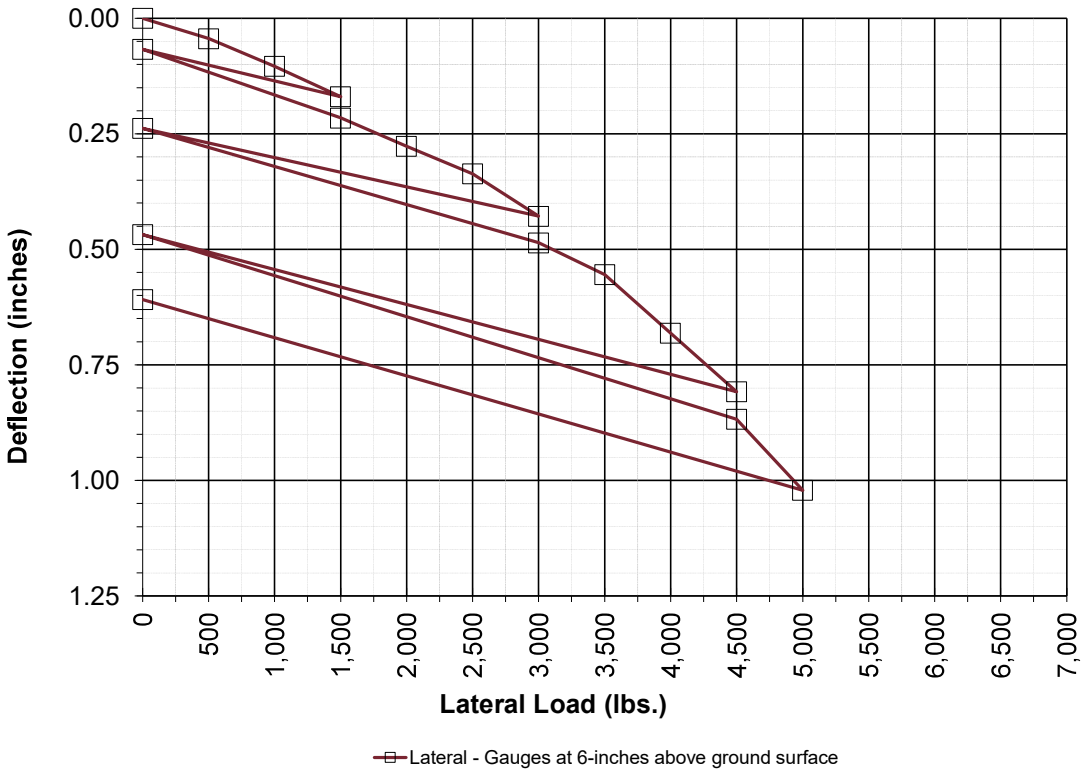
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-24A
Group Latitude [deg.]:	33.6328
Group Longitude [deg.]:	-114.6866
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	12

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.044	
14%	1,000	0.104	
21%	1,500	0.170	
0%	0	0.067	
21%	1,500	0.216	
29%	2,000	0.277	
36%	2,500	0.337	
43%	3,000	0.428	
0%	0	0.238	
43%	3,000	0.486	
50%	3,500	0.555	
57%	4,000	0.682	
64%	4,500	0.808	
0%	0	0.468	
64%	4,500	0.868	
71%	5,000	1.022	
79%	5,500		
86%	6,000		
0%	0		
86%	6,000		
93%	6,500		
100%	7,000		
0%	0	0.609	



Lateral Load Test Result for PLT-24B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

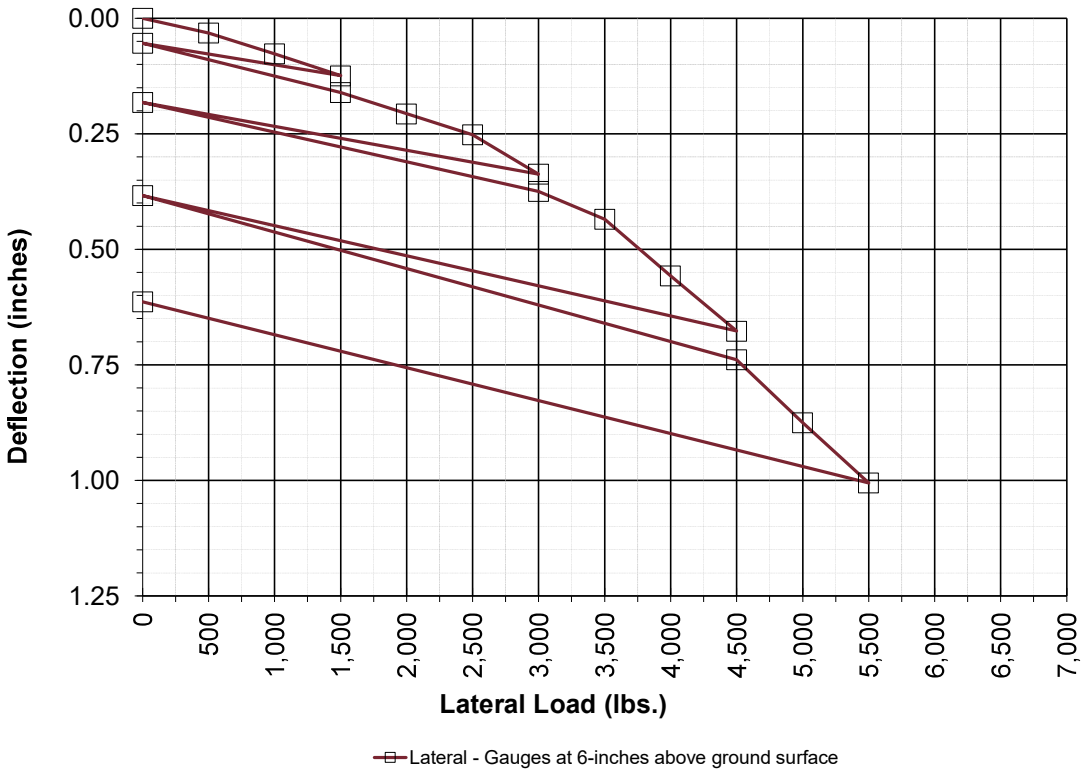
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-24B
Group Latitude [deg.]:	33.6328
Group Longitude [deg.]:	-114.6866
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	16

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.032	
14%	1,000	0.077	
21%	1,500	0.125	
0%	0	0.054	
21%	1,500	0.161	
29%	2,000	0.207	
36%	2,500	0.252	
43%	3,000	0.337	
0%	0	0.183	
43%	3,000	0.375	
50%	3,500	0.435	
57%	4,000	0.558	
64%	4,500	0.677	
0%	0	0.384	
64%	4,500	0.739	
71%	5,000	0.875	
79%	5,500	1.006	
86%	6,000		
0%	0		
86%	6,000		
93%	6,500		
100%	7,000		
0%	0	0.613	



Lateral Load Test Result for PLT-25A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

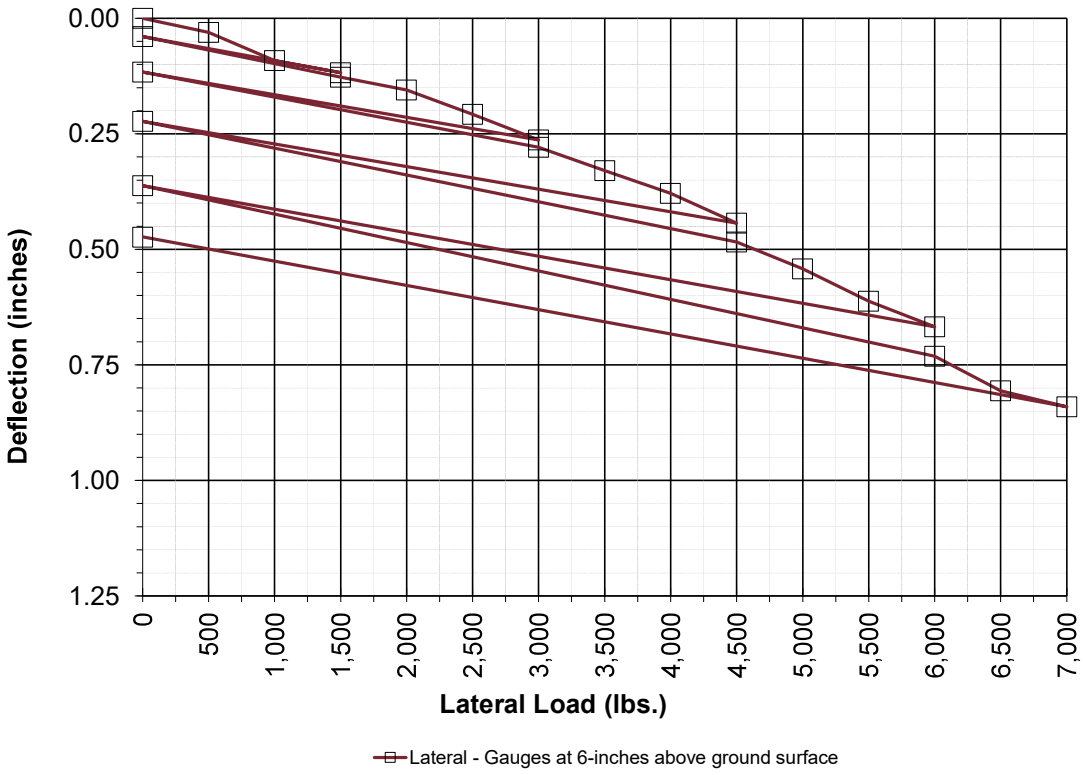
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-25A
Group Latitude [deg.]:	33.6371
Group Longitude [deg.]:	-114.6871
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	19

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.031	
14%	1,000	0.091	
21%	1,500	0.118	
0%	0	0.040	
21%	1,500	0.127	
29%	2,000	0.155	
36%	2,500	0.208	
43%	3,000	0.263	
0%	0	0.117	
43%	3,000	0.279	
50%	3,500	0.330	
57%	4,000	0.379	
64%	4,500	0.443	
0%	0	0.223	
64%	4,500	0.484	
71%	5,000	0.542	
79%	5,500	0.612	
86%	6,000	0.668	
0%	0	0.362	
86%	6,000	0.732	
93%	6,500	0.806	
100%	7,000	0.841	
0%	0	0.474	



Lateral Load Test Result for PLT-25B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

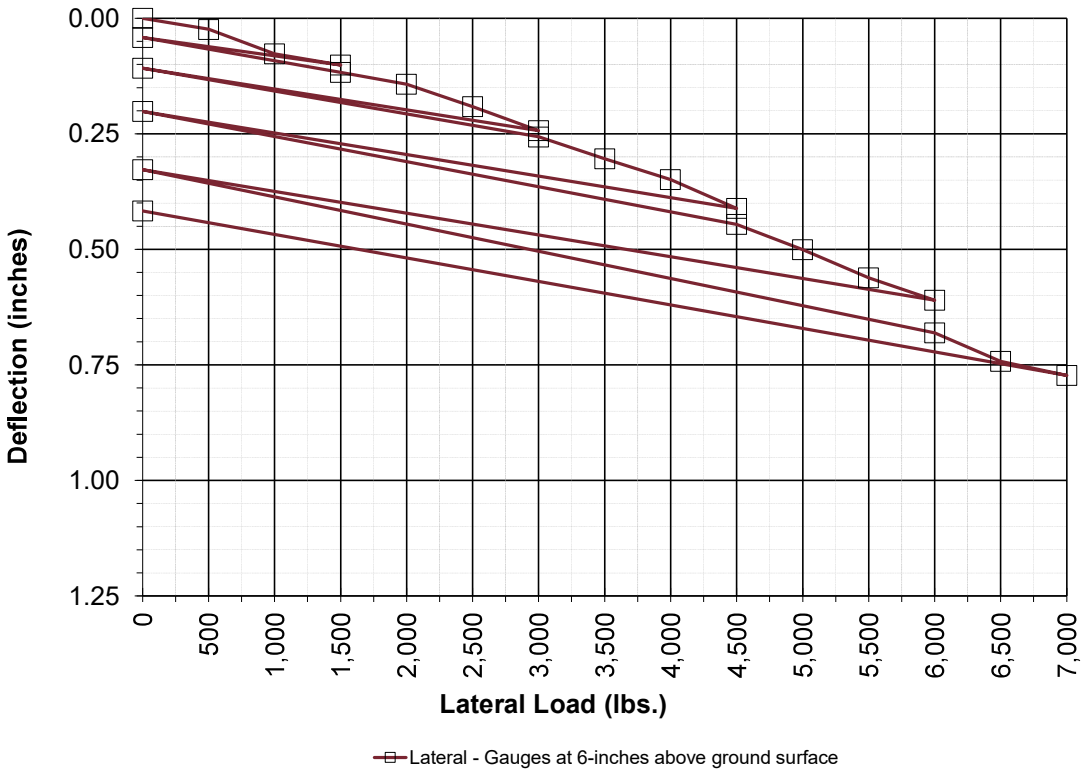
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-25B
Group Latitude [deg.]:	33.6371
Group Longitude [deg.]:	-114.6871
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	44

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.024	
14%	1,000	0.077	
21%	1,500	0.101	
0%	0	0.042	
21%	1,500	0.117	
29%	2,000	0.143	
36%	2,500	0.191	
43%	3,000	0.243	
0%	0	0.108	
43%	3,000	0.257	
50%	3,500	0.304	
57%	4,000	0.349	
64%	4,500	0.411	
0%	0	0.202	
64%	4,500	0.446	
71%	5,000	0.501	
79%	5,500	0.562	
86%	6,000	0.611	
0%	0	0.328	
86%	6,000	0.681	
93%	6,500	0.742	
100%	7,000	0.773	
0%	0	0.417	



Lateral Load Test Result for PLT-26A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

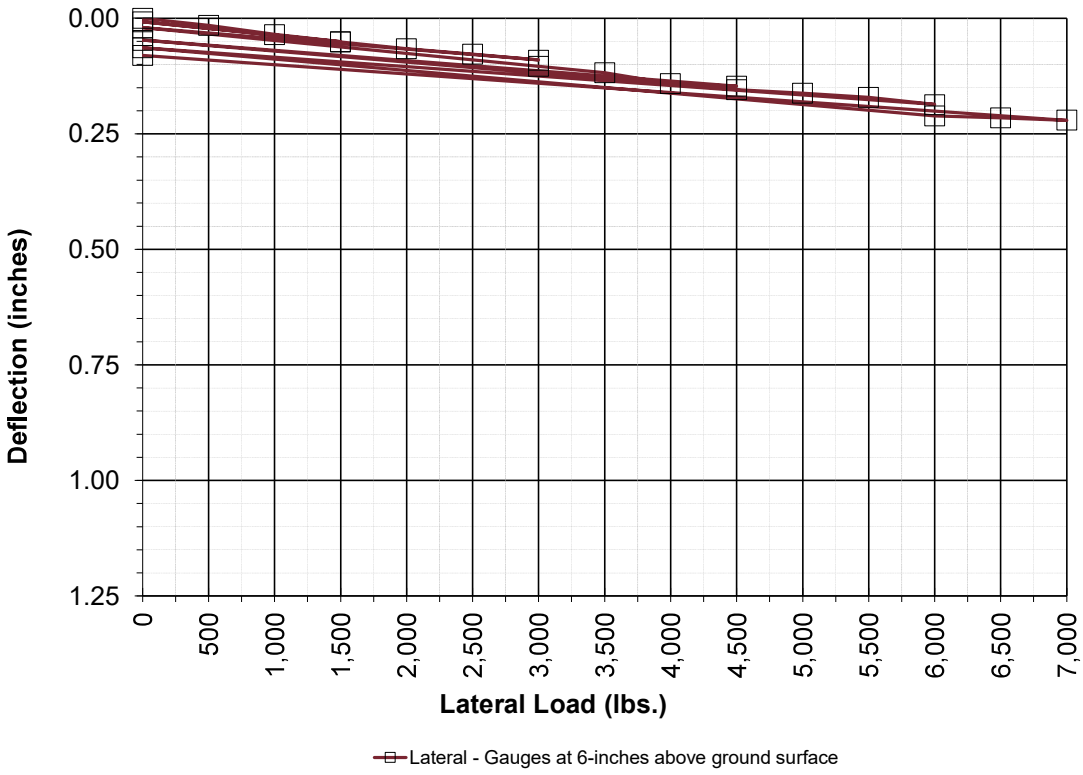
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-26A
Group Latitude [deg.]:	33.6346
Group Longitude [deg.]:	-114.6763
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	26

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.015	
14%	1,000	0.035	
21%	1,500	0.051	
0%	0	0.008	
21%	1,500	0.052	
29%	2,000	0.066	
36%	2,500	0.078	
43%	3,000	0.091	
0%	0	0.021	
43%	3,000	0.104	
50%	3,500	0.118	
57%	4,000	0.141	
64%	4,500	0.147	
0%	0	0.048	
64%	4,500	0.155	
71%	5,000	0.162	
79%	5,500	0.171	
86%	6,000	0.187	
0%	0	0.064	
86%	6,000	0.211	
93%	6,500	0.215	
100%	7,000	0.221	
0%	0	0.081	



Lateral Load Test Result for PLT-26B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

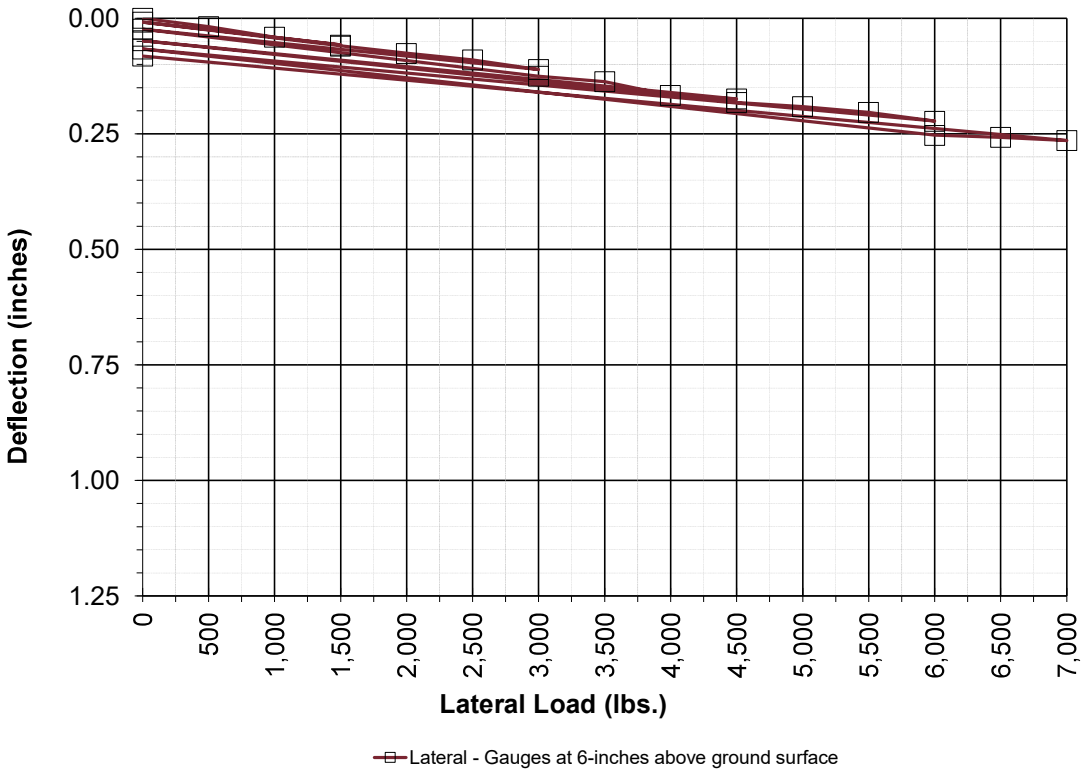
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-26B
Group Latitude [deg.]:	33.6346
Group Longitude [deg.]:	-114.6763
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	61

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.019	
14%	1,000	0.041	
21%	1,500	0.058	
0%	0	0.009	
21%	1,500	0.060	
29%	2,000	0.076	
36%	2,500	0.090	
43%	3,000	0.111	
0%	0	0.024	
43%	3,000	0.126	
50%	3,500	0.137	
57%	4,000	0.167	
64%	4,500	0.175	
0%	0	0.049	
64%	4,500	0.182	
71%	5,000	0.192	
79%	5,500	0.204	
86%	6,000	0.223	
0%	0	0.067	
86%	6,000	0.253	
93%	6,500	0.258	
100%	7,000	0.265	
0%	0	0.082	



Lateral Load Test Result for PLT-27A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

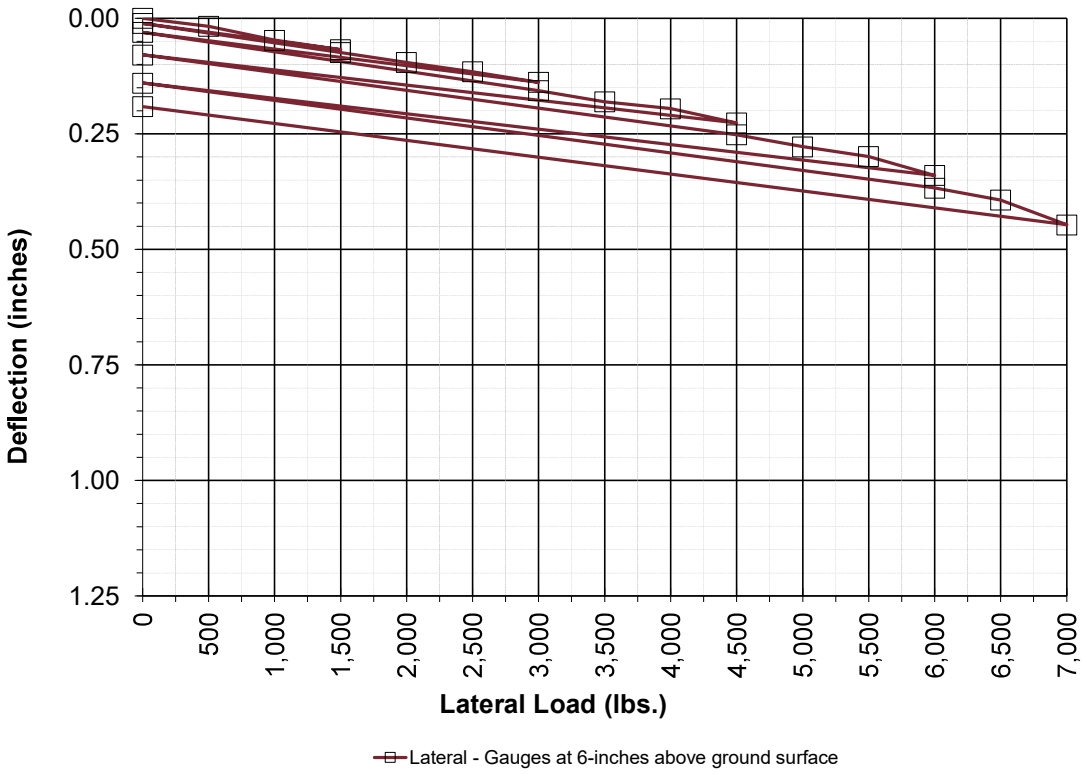
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-27A
Group Latitude [deg.]:	33.6398
Group Longitude [deg.]:	-114.6813
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	58

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.018	
14%	1,000	0.048	
21%	1,500	0.068	
0%	0	0.011	
21%	1,500	0.074	
29%	2,000	0.096	
36%	2,500	0.116	
43%	3,000	0.139	
0%	0	0.031	
43%	3,000	0.157	
50%	3,500	0.181	
57%	4,000	0.196	
64%	4,500	0.227	
0%	0	0.080	
64%	4,500	0.253	
71%	5,000	0.278	
79%	5,500	0.299	
86%	6,000	0.340	
0%	0	0.140	
86%	6,000	0.368	
93%	6,500	0.394	
100%	7,000	0.447	
0%	0	0.191	



Lateral Load Test Result for PLT-27B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

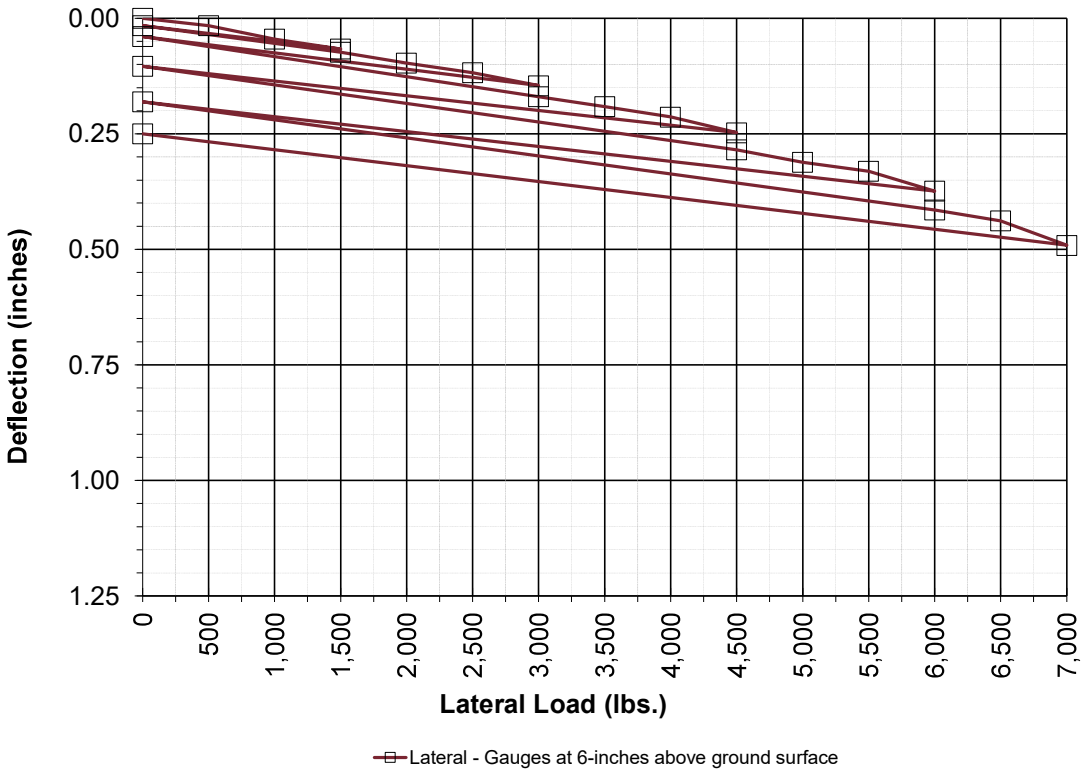
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-27B
Group Latitude [deg.]:	33.6398
Group Longitude [deg.]:	-114.6813
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	79

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.016	
14%	1,000	0.045	
21%	1,500	0.067	
0%	0	0.016	
21%	1,500	0.073	
29%	2,000	0.097	
36%	2,500	0.118	
43%	3,000	0.146	
0%	0	0.039	
43%	3,000	0.170	
50%	3,500	0.191	
57%	4,000	0.214	
64%	4,500	0.248	
0%	0	0.104	
64%	4,500	0.285	
71%	5,000	0.312	
79%	5,500	0.331	
86%	6,000	0.374	
0%	0	0.181	
86%	6,000	0.415	
93%	6,500	0.438	
100%	7,000	0.491	
0%	0	0.250	



Lateral Load Test Result for PLT-28A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

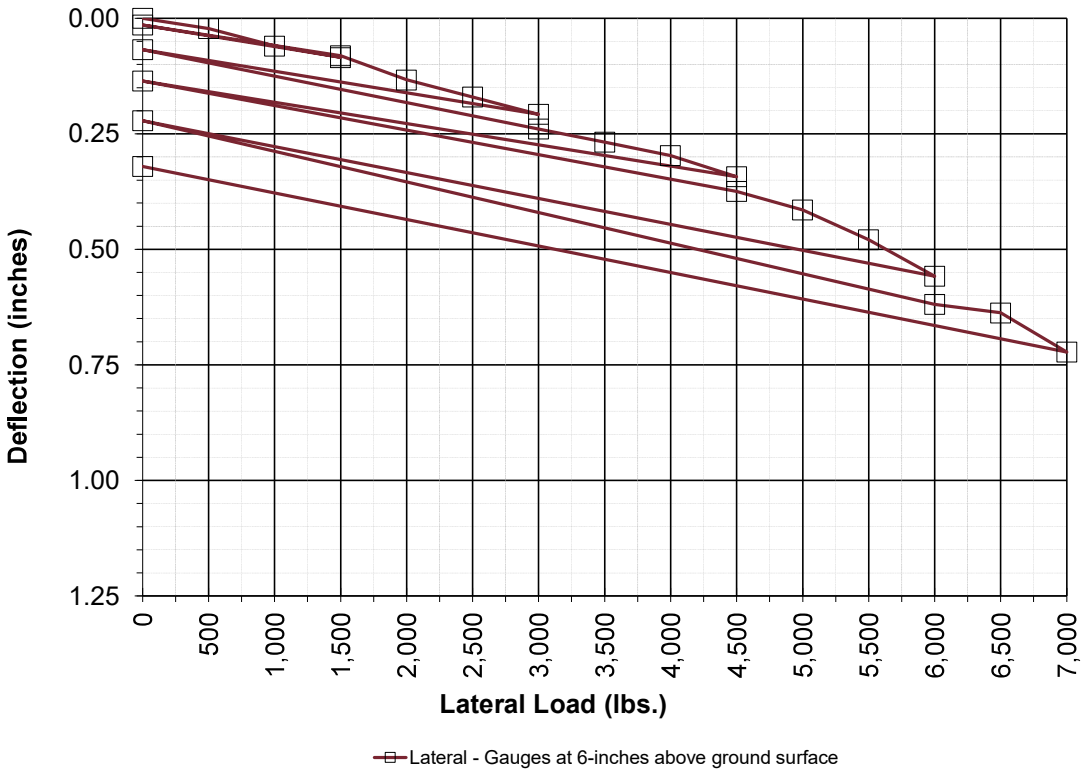
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-28A
Group Latitude [deg.]:	33.6455
Group Longitude [deg.]:	-114.6798
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	11

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.022	
14%	1,000	0.060	
21%	1,500	0.085	
0%	0	0.015	
21%	1,500	0.081	
29%	2,000	0.133	
36%	2,500	0.170	
43%	3,000	0.208	
0%	0	0.068	
43%	3,000	0.240	
50%	3,500	0.268	
57%	4,000	0.298	
64%	4,500	0.343	
0%	0	0.136	
64%	4,500	0.375	
71%	5,000	0.415	
79%	5,500	0.479	
86%	6,000	0.558	
0%	0	0.221	
86%	6,000	0.620	
93%	6,500	0.638	
100%	7,000	0.722	
0%	0	0.321	



Lateral Load Test Result for PLT-28B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

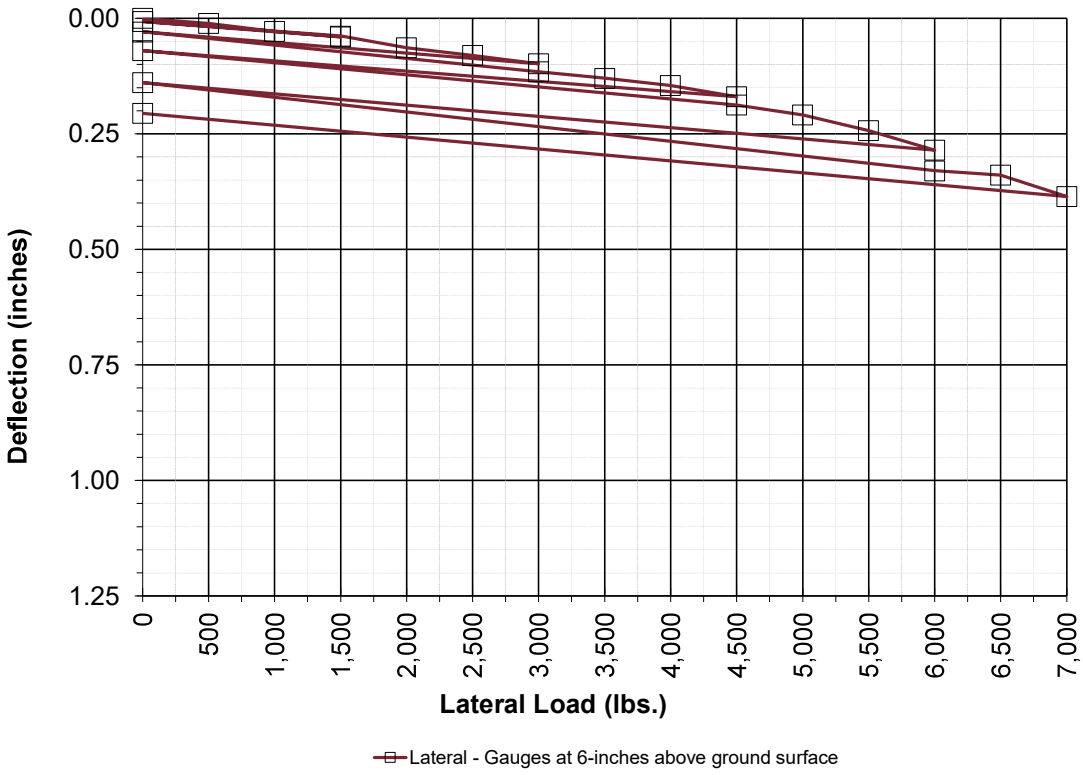
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-28B
Group Latitude [deg.]:	33.6455
Group Longitude [deg.]:	-114.6798
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	38

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.011	
14%	1,000	0.029	
21%	1,500	0.040	
0%	0	0.007	
21%	1,500	0.039	
29%	2,000	0.064	
36%	2,500	0.081	
43%	3,000	0.098	
0%	0	0.030	
43%	3,000	0.116	
50%	3,500	0.130	
57%	4,000	0.145	
64%	4,500	0.169	
0%	0	0.070	
64%	4,500	0.188	
71%	5,000	0.209	
79%	5,500	0.243	
86%	6,000	0.285	
0%	0	0.140	
86%	6,000	0.330	
93%	6,500	0.340	
100%	7,000	0.386	
0%	0	0.206	



Lateral Load Test Result for PLT-29A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

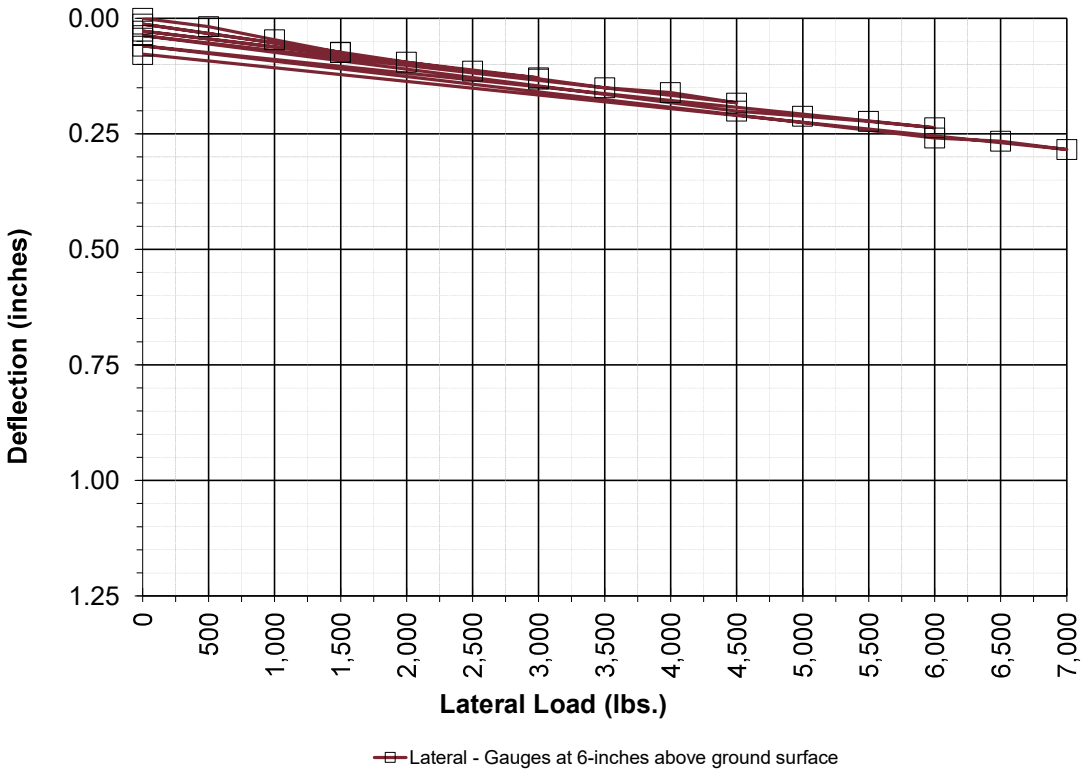
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-29A
Group Latitude [deg.]:	33.6455
Group Longitude [deg.]:	-114.6699
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	16

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.018	
14%	1,000	0.046	
21%	1,500	0.074	
0%	0	0.012	
21%	1,500	0.074	
29%	2,000	0.095	
36%	2,500	0.115	
43%	3,000	0.129	
0%	0	0.028	
43%	3,000	0.131	
50%	3,500	0.150	
57%	4,000	0.161	
64%	4,500	0.183	
0%	0	0.037	
64%	4,500	0.201	
71%	5,000	0.212	
79%	5,500	0.223	
86%	6,000	0.237	
0%	0	0.060	
86%	6,000	0.259	
93%	6,500	0.266	
100%	7,000	0.284	
0%	0	0.078	



Lateral Load Test Result for PLT-29B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

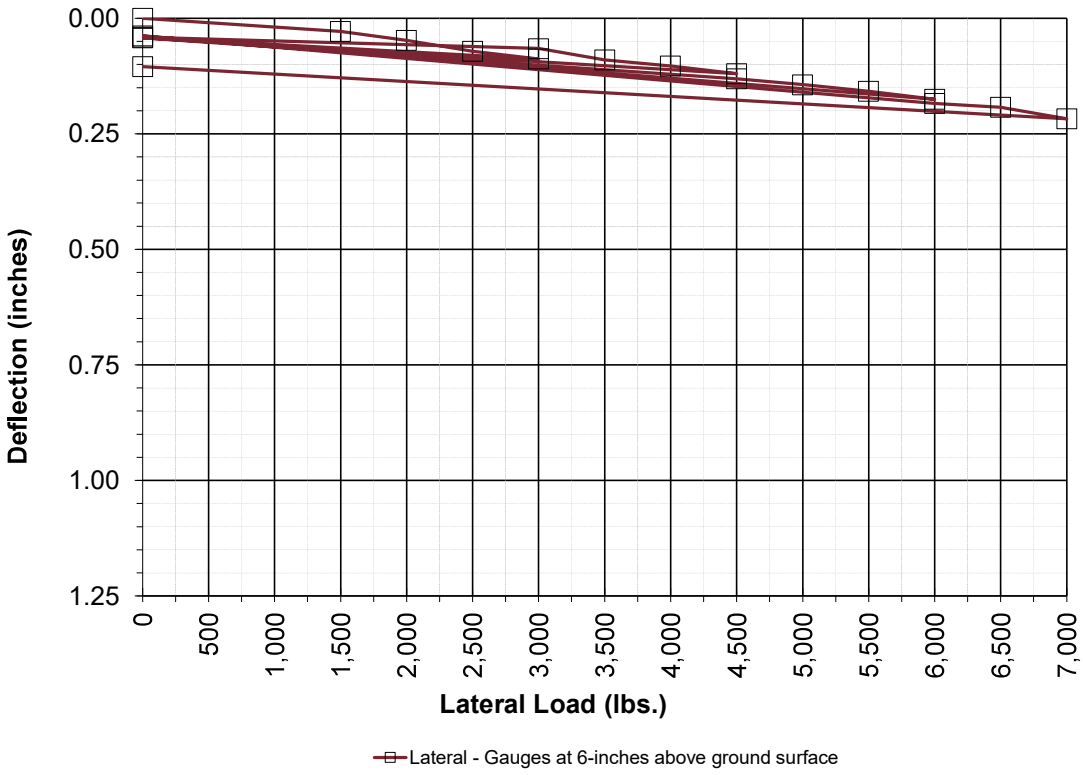
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-29B
Group Latitude [deg.]:	33.6455
Group Longitude [deg.]:	-114.6699
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	58

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500		
14%	1,000		
21%	1,500		
0%	0		
21%	1,500	0.029	
29%	2,000	0.048	
36%	2,500	0.072	
43%	3,000	0.088	
0%	0	0.041	
43%	3,000	0.066	
50%	3,500	0.090	
57%	4,000	0.104	
64%	4,500	0.120	
0%	0	0.043	
64%	4,500	0.131	
71%	5,000	0.144	
79%	5,500	0.158	
86%	6,000	0.175	
0%	0	0.038	
86%	6,000	0.184	
93%	6,500	0.193	
100%	7,000	0.218	
0%	0	0.105	



Lateral Load Test Result for PLT-30A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

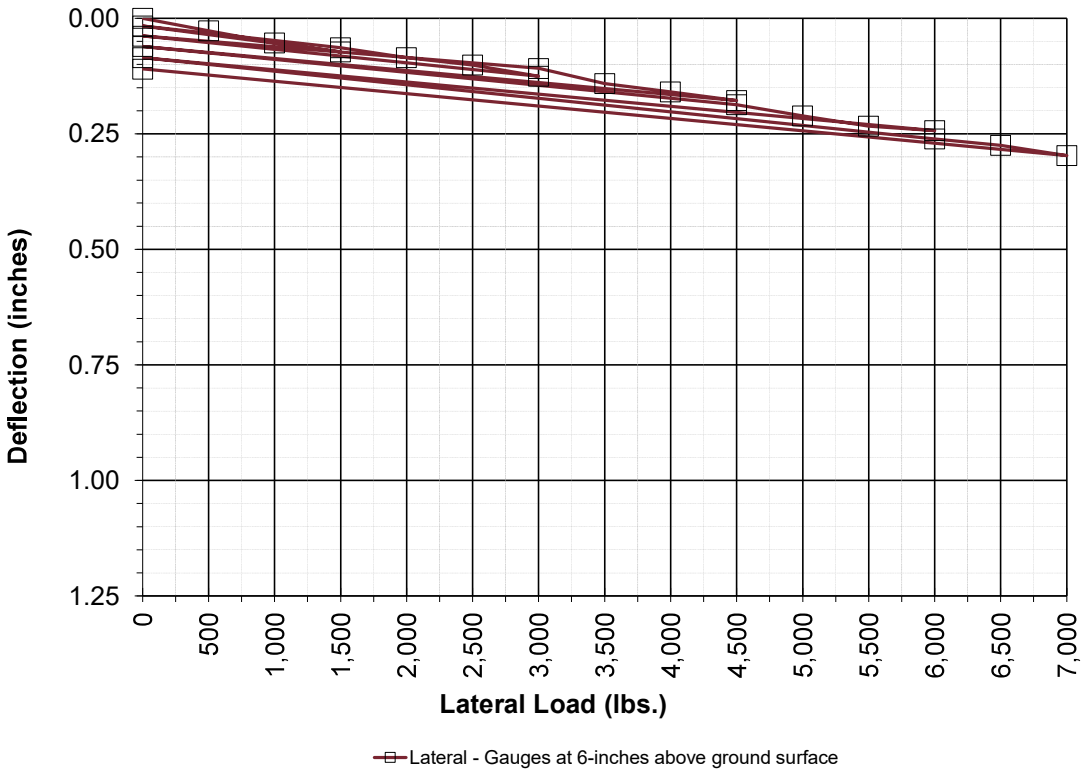
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-30A
Group Latitude [deg.]:	33.6435
Group Longitude [deg.]:	-114.6650
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	20

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.028	
14%	1,000	0.054	
21%	1,500	0.073	
0%	0	0.017	
21%	1,500	0.064	
29%	2,000	0.085	
36%	2,500	0.101	
43%	3,000	0.125	
0%	0	0.039	
43%	3,000	0.109	
50%	3,500	0.141	
57%	4,000	0.159	
64%	4,500	0.178	
0%	0	0.061	
64%	4,500	0.187	
71%	5,000	0.212	
79%	5,500	0.233	
86%	6,000	0.244	
0%	0	0.086	
86%	6,000	0.261	
93%	6,500	0.275	
100%	7,000	0.298	
0%	0	0.110	



Lateral Load Test Result for PLT-30B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

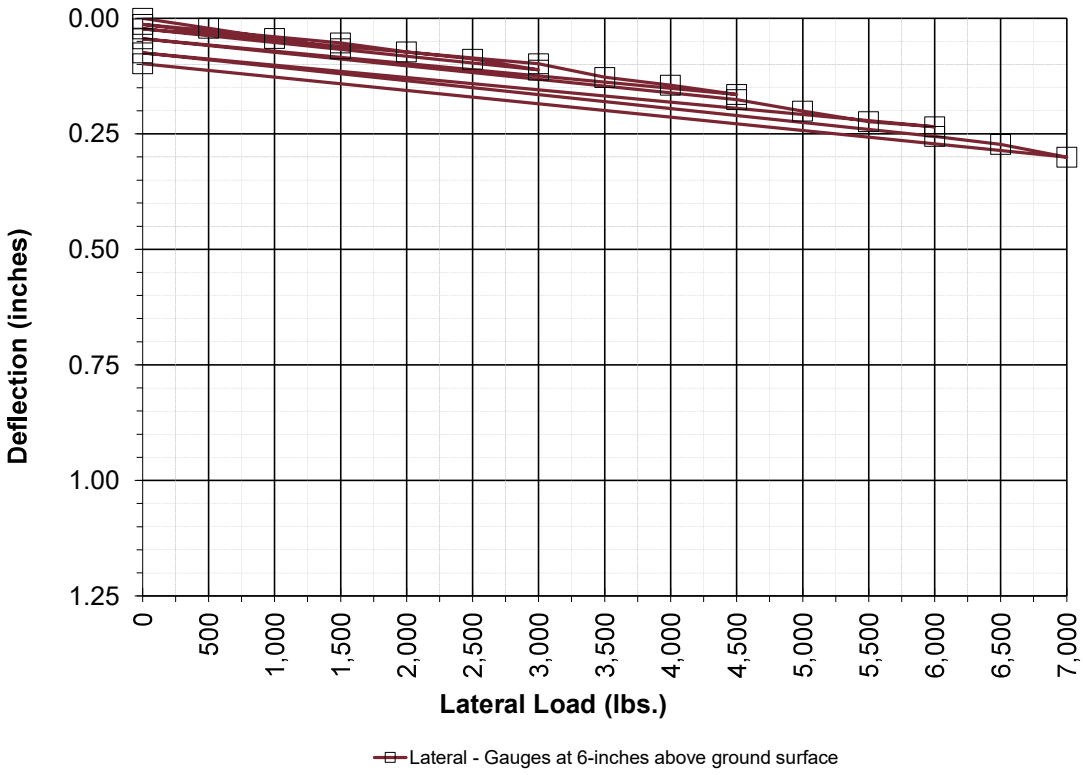
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-30B
Group Latitude [deg.]:	33.6435
Group Longitude [deg.]:	-114.6650
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	60

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.022	
14%	1,000	0.044	
21%	1,500	0.066	
0%	0	0.013	
21%	1,500	0.053	
29%	2,000	0.073	
36%	2,500	0.088	
43%	3,000	0.112	
0%	0	0.023	
43%	3,000	0.098	
50%	3,500	0.128	
57%	4,000	0.145	
64%	4,500	0.165	
0%	0	0.045	
64%	4,500	0.176	
71%	5,000	0.201	
79%	5,500	0.224	
86%	6,000	0.235	
0%	0	0.075	
86%	6,000	0.256	
93%	6,500	0.273	
100%	7,000	0.301	
0%	0	0.098	



Lateral Load Test Result for PLT-31A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

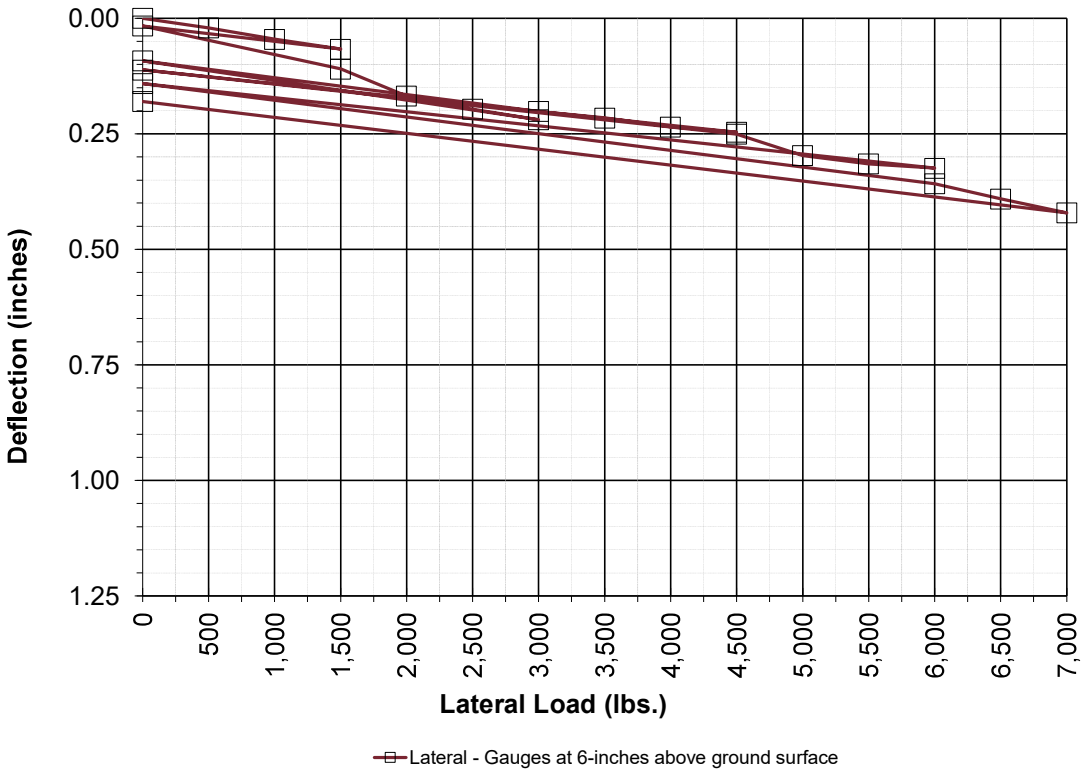
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-31A
Group Latitude [deg.]:	33.6465
Group Longitude [deg.]:	-114.6614
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	36

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.021	
14%	1,000	0.046	
21%	1,500	0.067	
0%	0	0.017	
21%	1,500	0.110	
29%	2,000	0.168	
36%	2,500	0.197	
43%	3,000	0.220	
0%	0	0.092	
43%	3,000	0.202	
50%	3,500	0.216	
57%	4,000	0.235	
64%	4,500	0.246	
0%	0	0.112	
64%	4,500	0.251	
71%	5,000	0.297	
79%	5,500	0.315	
86%	6,000	0.325	
0%	0	0.142	
86%	6,000	0.358	
93%	6,500	0.391	
100%	7,000	0.421	
0%	0	0.180	



Lateral Load Test Result for PLT-31B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

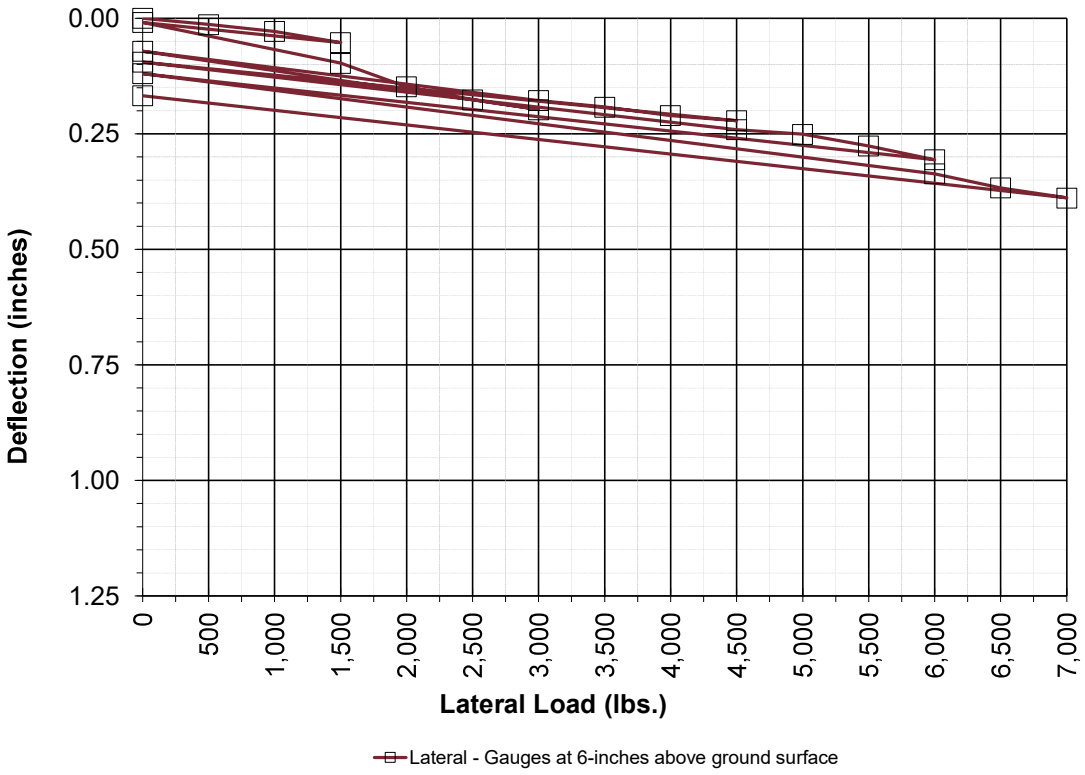
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-31B
Group Latitude [deg.]:	33.6465
Group Longitude [deg.]:	-114.6614
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	70

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.014	
14%	1,000	0.029	
21%	1,500	0.053	
0%	0	0.009	
21%	1,500	0.098	
29%	2,000	0.148	
36%	2,500	0.176	
43%	3,000	0.198	
0%	0	0.071	
43%	3,000	0.178	
50%	3,500	0.192	
57%	4,000	0.211	
64%	4,500	0.222	
0%	0	0.095	
64%	4,500	0.241	
71%	5,000	0.251	
79%	5,500	0.277	
86%	6,000	0.306	
0%	0	0.120	
86%	6,000	0.337	
93%	6,500	0.367	
100%	7,000	0.389	
0%	0	0.168	



Lateral Load Test Result for PLT-32A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

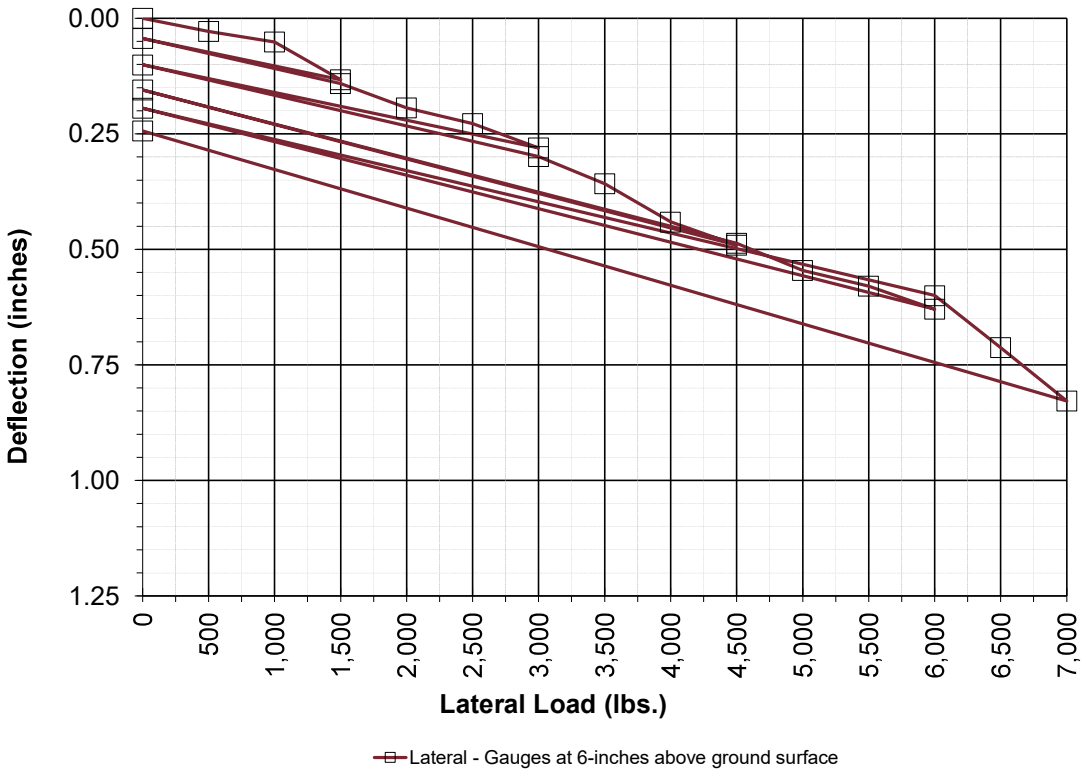
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-32A
Group Latitude [deg.]:	33.6329
Group Longitude [deg.]:	-114.6950
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	32

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.029	
14%	1,000	0.052	
21%	1,500	0.133	
0%	0	0.044	
21%	1,500	0.142	
29%	2,000	0.194	
36%	2,500	0.228	
43%	3,000	0.281	
0%	0	0.101	
43%	3,000	0.299	
50%	3,500	0.358	
57%	4,000	0.441	
64%	4,500	0.491	
0%	0	0.156	
64%	4,500	0.487	
71%	5,000	0.546	
79%	5,500	0.580	
86%	6,000	0.630	
0%	0	0.195	
86%	6,000	0.600	
93%	6,500	0.713	
100%	7,000	0.829	
0%	0	0.244	



Lateral Load Test Result for PLT-32B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

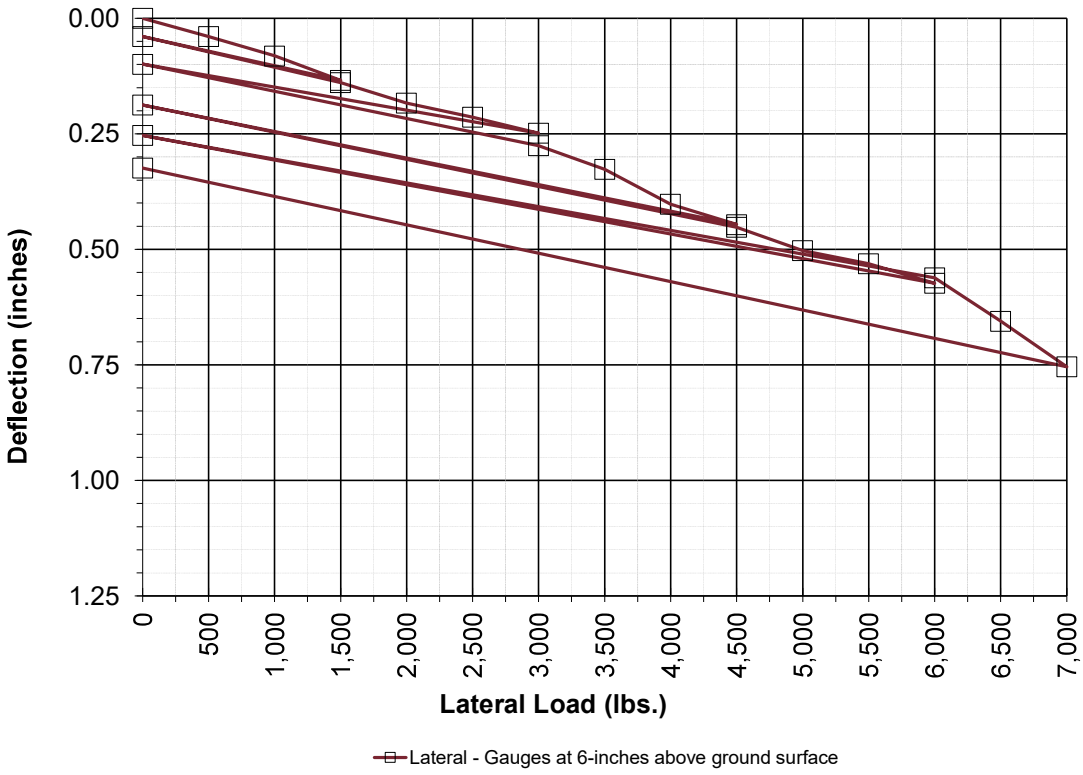
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-32B
Group Latitude [deg.]:	33.6329
Group Longitude [deg.]:	-114.6950
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	41

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.039	
14%	1,000	0.082	
21%	1,500	0.134	
0%	0	0.040	
21%	1,500	0.139	
29%	2,000	0.184	
36%	2,500	0.214	
43%	3,000	0.249	
0%	0	0.100	
43%	3,000	0.276	
50%	3,500	0.327	
57%	4,000	0.402	
64%	4,500	0.447	
0%	0	0.188	
64%	4,500	0.453	
71%	5,000	0.503	
79%	5,500	0.531	
86%	6,000	0.574	
0%	0	0.254	
86%	6,000	0.561	
93%	6,500	0.656	
100%	7,000	0.754	
0%	0	0.324	



Lateral Load Test Result for PLT-33A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

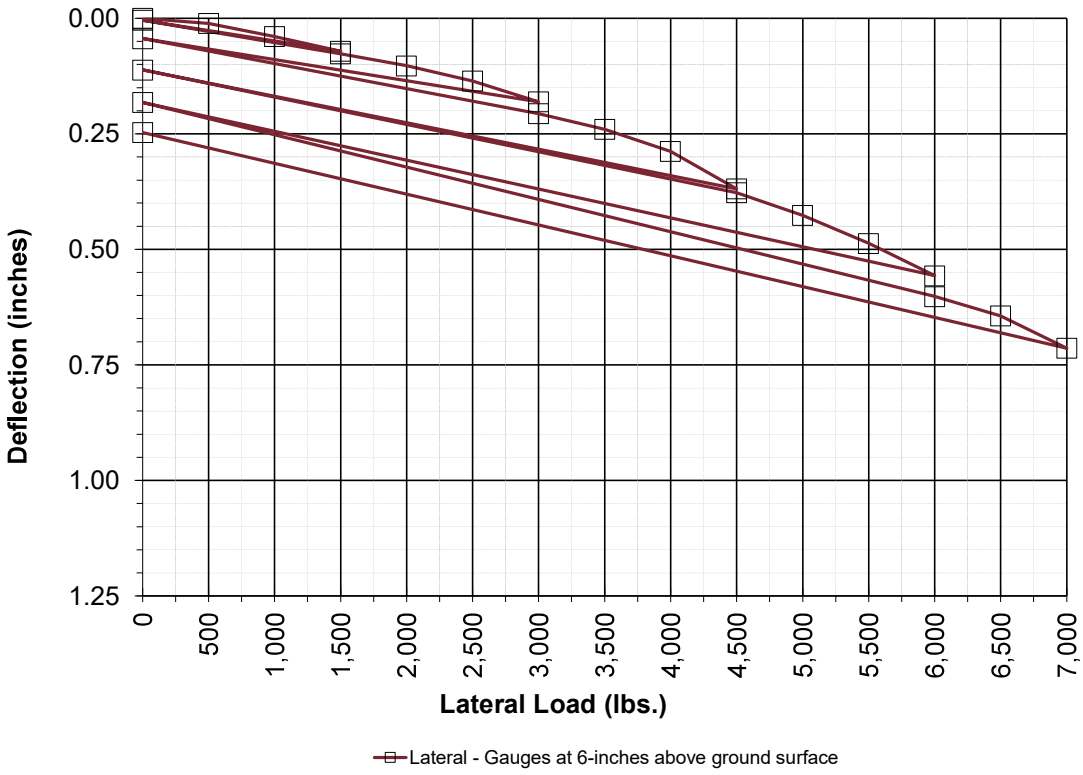
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-33A
Group Latitude [deg.]:	33.6491
Group Longitude [deg.]:	-114.7191
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	22

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.011	
14%	1,000	0.040	
21%	1,500	0.072	
0%	0	0.004	
21%	1,500	0.077	
29%	2,000	0.103	
36%	2,500	0.136	
43%	3,000	0.181	
0%	0	0.044	
43%	3,000	0.206	
50%	3,500	0.241	
57%	4,000	0.288	
64%	4,500	0.369	
0%	0	0.112	
64%	4,500	0.378	
71%	5,000	0.427	
79%	5,500	0.487	
86%	6,000	0.557	
0%	0	0.183	
86%	6,000	0.602	
93%	6,500	0.644	
100%	7,000	0.714	
0%	0	0.247	



Lateral Load Test Result for PLT-33B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

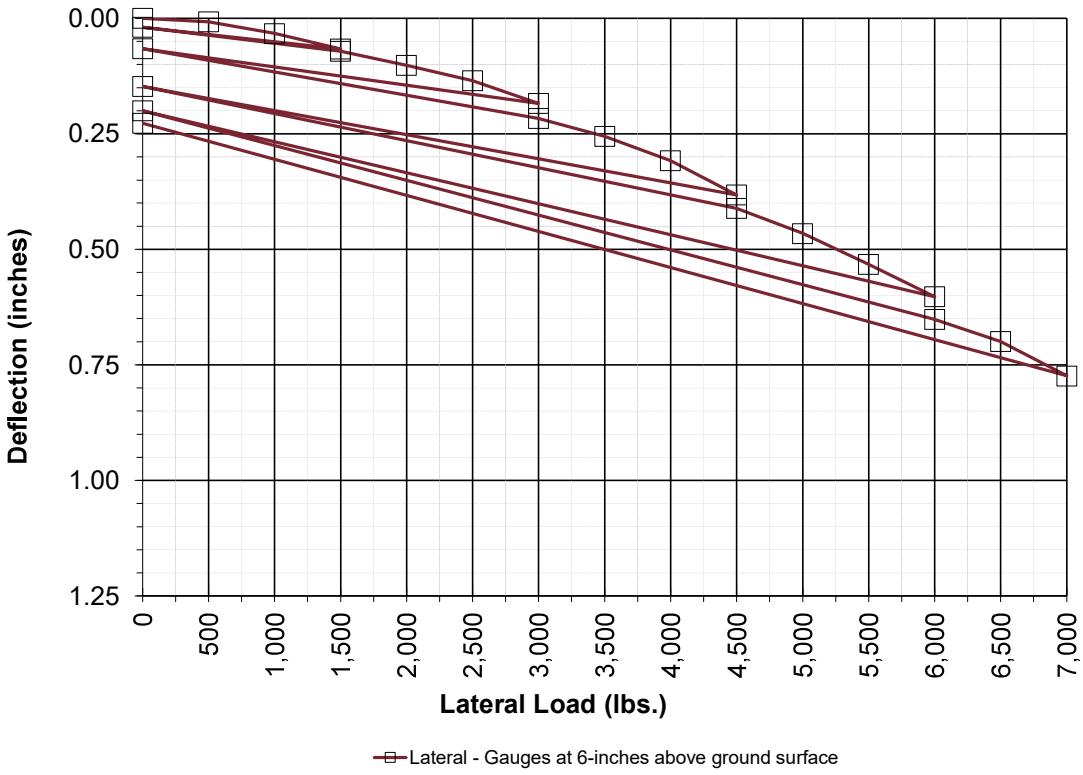
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-33B
Group Latitude [deg.]:	33.6491
Group Longitude [deg.]:	-114.7191
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	41

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.008	
14%	1,000	0.033	
21%	1,500	0.066	
0%	0	0.020	
21%	1,500	0.071	
29%	2,000	0.102	
36%	2,500	0.135	
43%	3,000	0.185	
0%	0	0.066	
43%	3,000	0.217	
50%	3,500	0.256	
57%	4,000	0.308	
64%	4,500	0.383	
0%	0	0.148	
64%	4,500	0.412	
71%	5,000	0.466	
79%	5,500	0.533	
86%	6,000	0.603	
0%	0	0.201	
86%	6,000	0.652	
93%	6,500	0.700	
100%	7,000	0.774	
0%	0	0.228	



Lateral Load Test Result for PLT-34A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

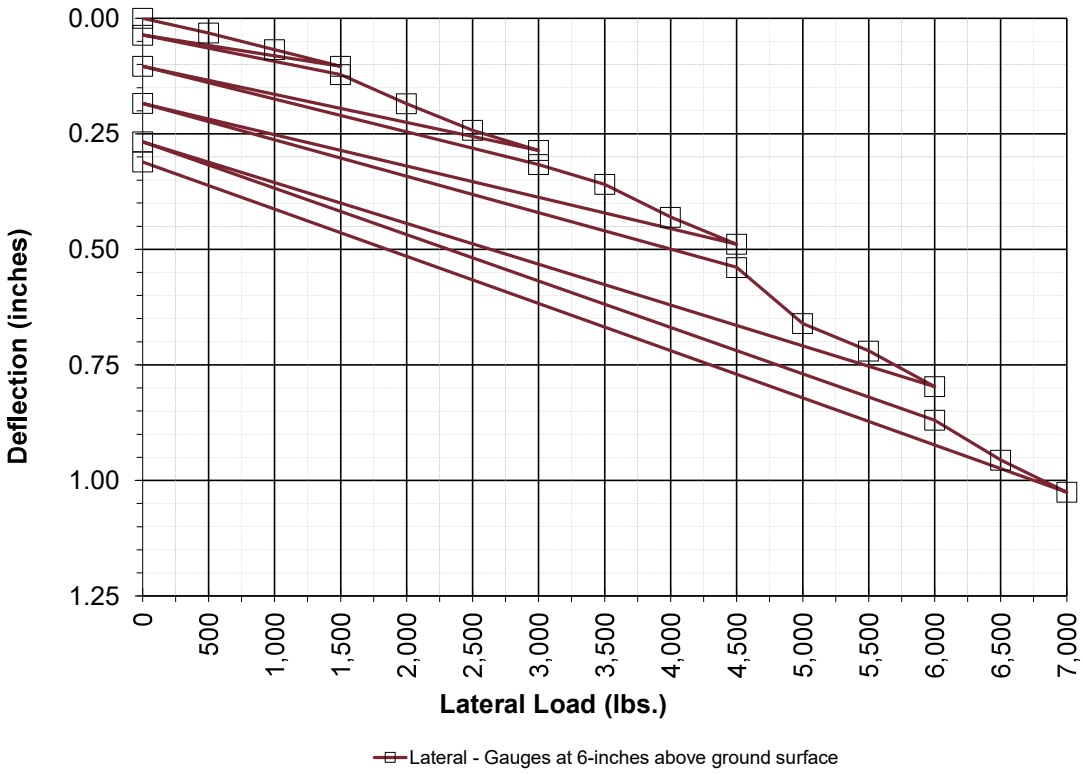
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-34A
Group Latitude [deg.]:	33.6411
Group Longitude [deg.]:	-114.7110
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	22

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.032	
14%	1,000	0.068	
21%	1,500	0.104	
0%	0	0.036	
21%	1,500	0.122	
29%	2,000	0.185	
36%	2,500	0.242	
43%	3,000	0.286	
0%	0	0.104	
43%	3,000	0.316	
50%	3,500	0.360	
57%	4,000	0.430	
64%	4,500	0.489	
0%	0	0.185	
64%	4,500	0.539	
71%	5,000	0.661	
79%	5,500	0.720	
86%	6,000	0.797	
0%	0	0.268	
86%	6,000	0.870	
93%	6,500	0.956	
100%	7,000	1.025	Failed at 6,700 LBS
0%	0	0.311	



Lateral Load Test Result for PLT-34B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

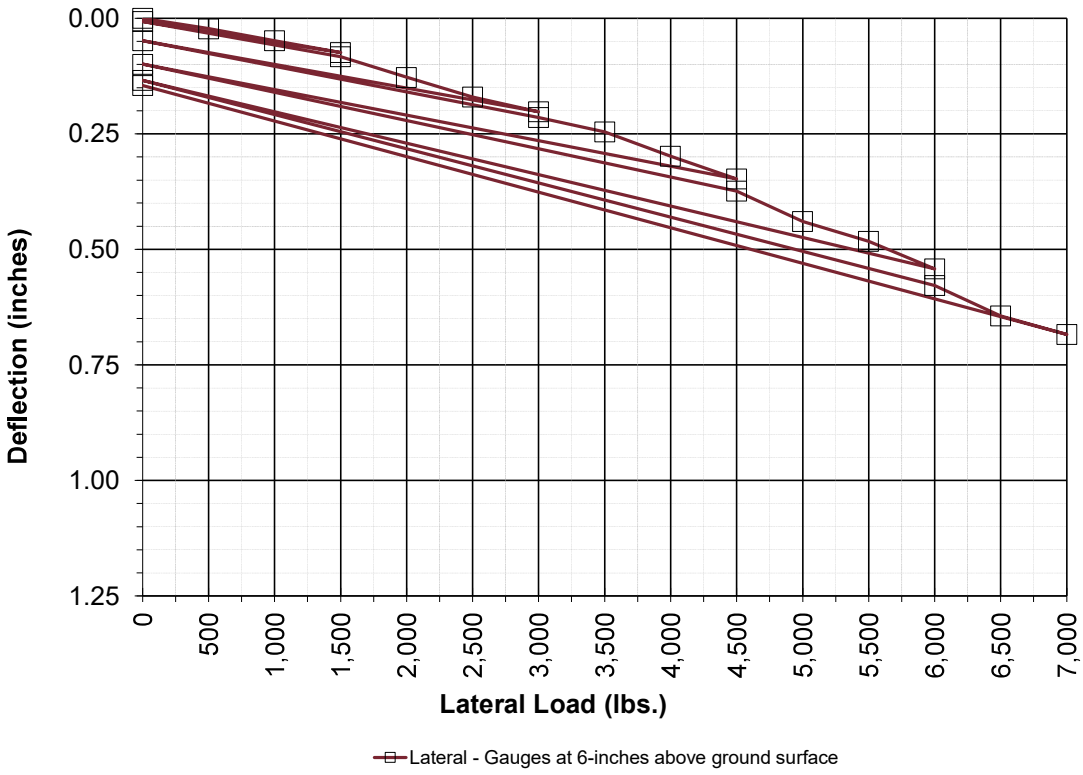
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-34B
Group Latitude [deg.]:	33.6411
Group Longitude [deg.]:	-114.7110
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	48

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.023	
14%	1,000	0.049	
21%	1,500	0.074	
0%	0	0.007	
21%	1,500	0.083	
29%	2,000	0.128	
36%	2,500	0.171	
43%	3,000	0.202	
0%	0	0.048	
43%	3,000	0.215	
50%	3,500	0.246	
57%	4,000	0.298	
64%	4,500	0.348	
0%	0	0.100	
64%	4,500	0.374	
71%	5,000	0.440	
79%	5,500	0.482	
86%	6,000	0.542	
0%	0	0.135	
86%	6,000	0.578	
93%	6,500	0.644	
100%	7,000	0.684	
0%	0	0.146	



Lateral Load Test Result for PLT-35A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

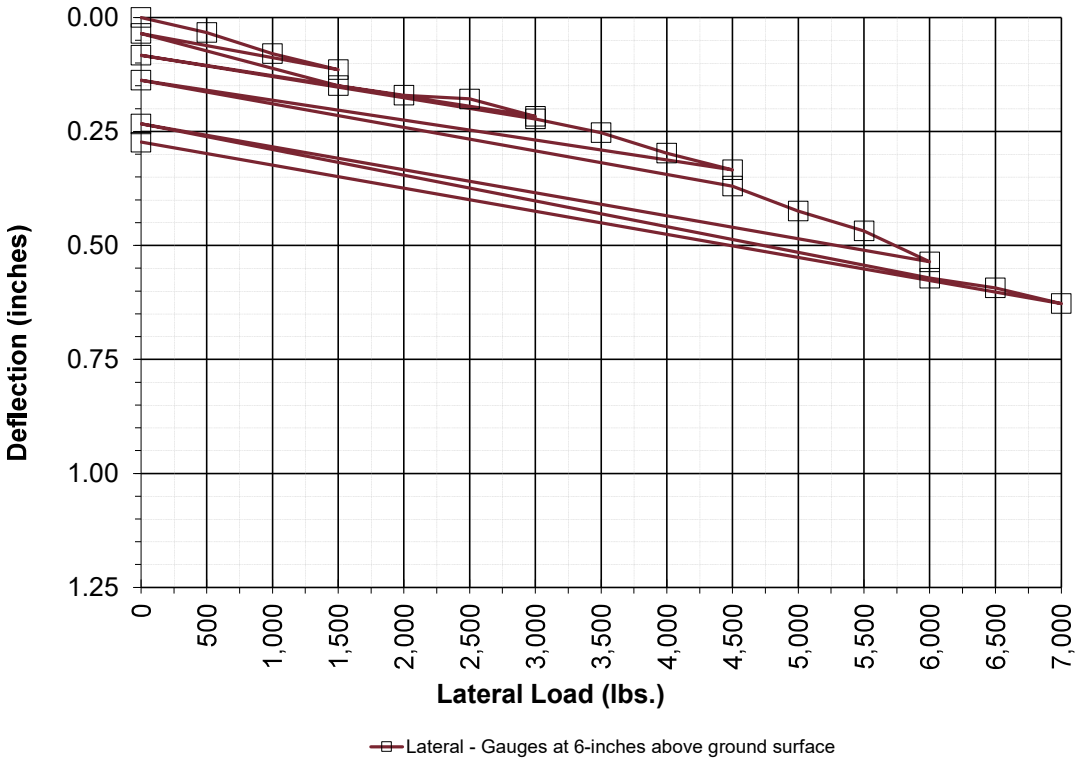
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-35A
Group Latitude [deg.]:	33.6661
Group Longitude [deg.]:	-114.7094
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	22

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.033	
14%	1,000	0.079	
21%	1,500	0.115	
0%	0	0.035	
21%	1,500	0.150	
29%	2,000	0.171	
36%	2,500	0.179	
43%	3,000	0.217	
0%	0	0.083	
43%	3,000	0.223	
50%	3,500	0.253	
57%	4,000	0.298	
64%	4,500	0.334	
0%	0	0.138	
64%	4,500	0.370	
71%	5,000	0.425	
79%	5,500	0.468	
86%	6,000	0.536	
0%	0	0.234	
86%	6,000	0.572	
93%	6,500	0.593	
100%	7,000	0.627	
0%	0	0.274	



Lateral Load Test Result for PLT-35B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Lateral Load Test Set Up

Number of Top Gauges:	0
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	6
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	24
Load Cell:	Dillon ED

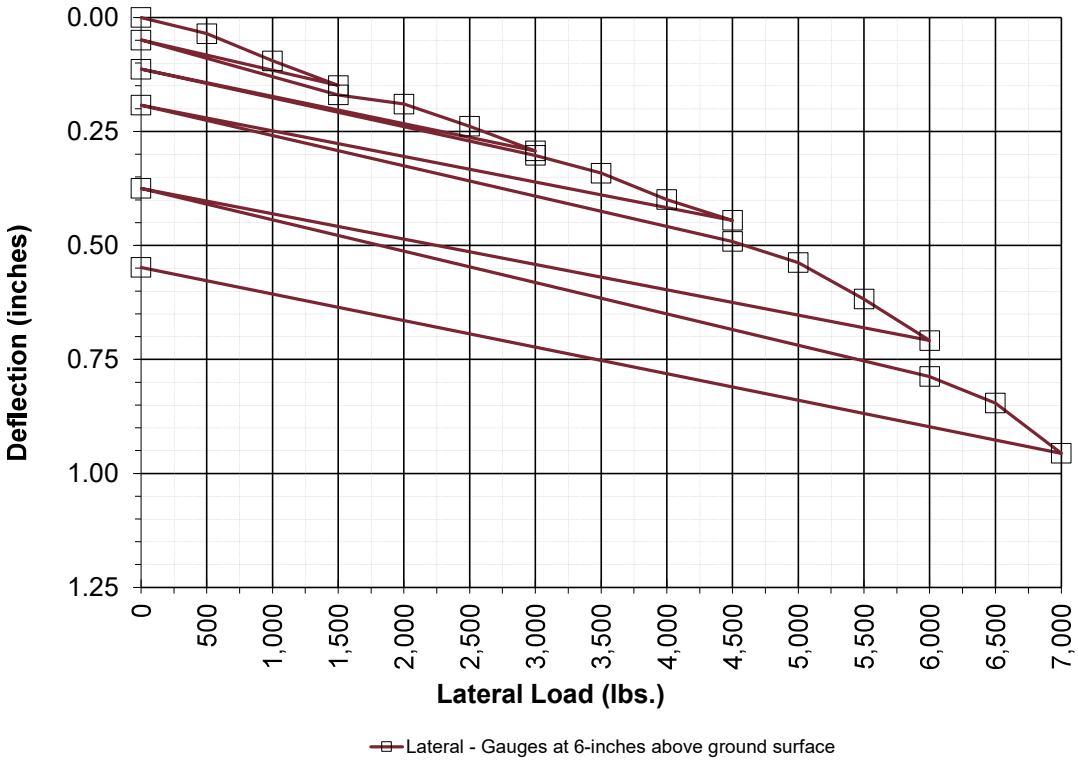
Test Date and Representative

Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-35B
Group Latitude [deg.]:	33.6661
Group Longitude [deg.]:	-114.7094
Pile Type:	W6x9
Pile Embedment Depth [in.]:	120
Pile Stick-Up [in.]:	48
Lateral Target Load [lbs.]:	7,000
Drive Time [sec.]:	60

% of Target Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.036	
14%	1,000	0.095	
21%	1,500	0.149	
0%	0	0.050	
21%	1,500	0.170	
29%	2,000	0.190	
36%	2,500	0.239	
43%	3,000	0.293	
0%	0	0.114	
43%	3,000	0.303	
50%	3,500	0.342	
57%	4,000	0.399	
64%	4,500	0.445	
0%	0	0.193	
64%	4,500	0.491	
71%	5,000	0.538	
79%	5,500	0.618	
86%	6,000	0.709	
0%	0	0.375	
86%	6,000	0.788	
93%	6,500	0.846	
100%	7,000	0.956	
0%	0	0.548	



Axial Compression Test Results

Compression Load Test Result for PLT-1C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

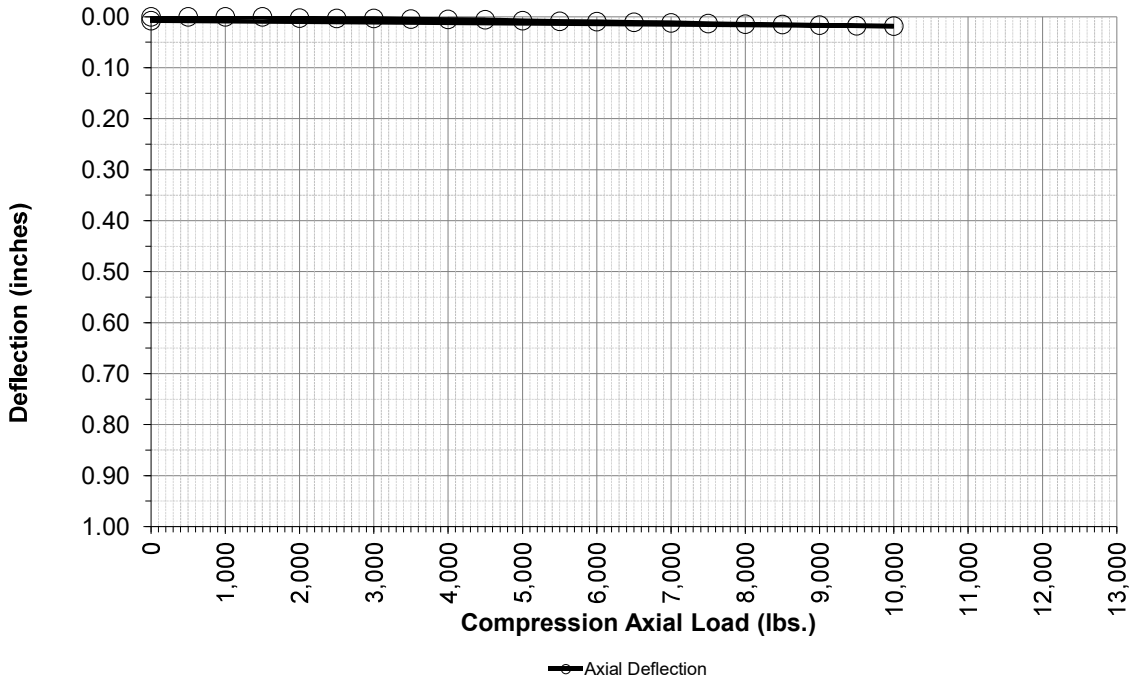
Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-1C
Latitude [deg.]:	33.66719
Longitude [deg.]:	-114.69954
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	34

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.000	
20%	2,000	0.003	
25%	2,500	0.003	
30%	3,000	0.003	
35%	3,500	0.004	
40%	4,000	0.005	
45%	4,500	0.006	
50%	5,000	0.008	
55%	5,500	0.009	
60%	6,000	0.010	
65%	6,500	0.011	
70%	7,000	0.012	
75%	7,500	0.013	
80%	8,000	0.014	
85%	8,500	0.015	
90%	9,000	0.016	
95%	9,500	0.018	
100%	10,000	0.018	
0%	0	0.008	



Compression Load Test Result for PLT-2C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

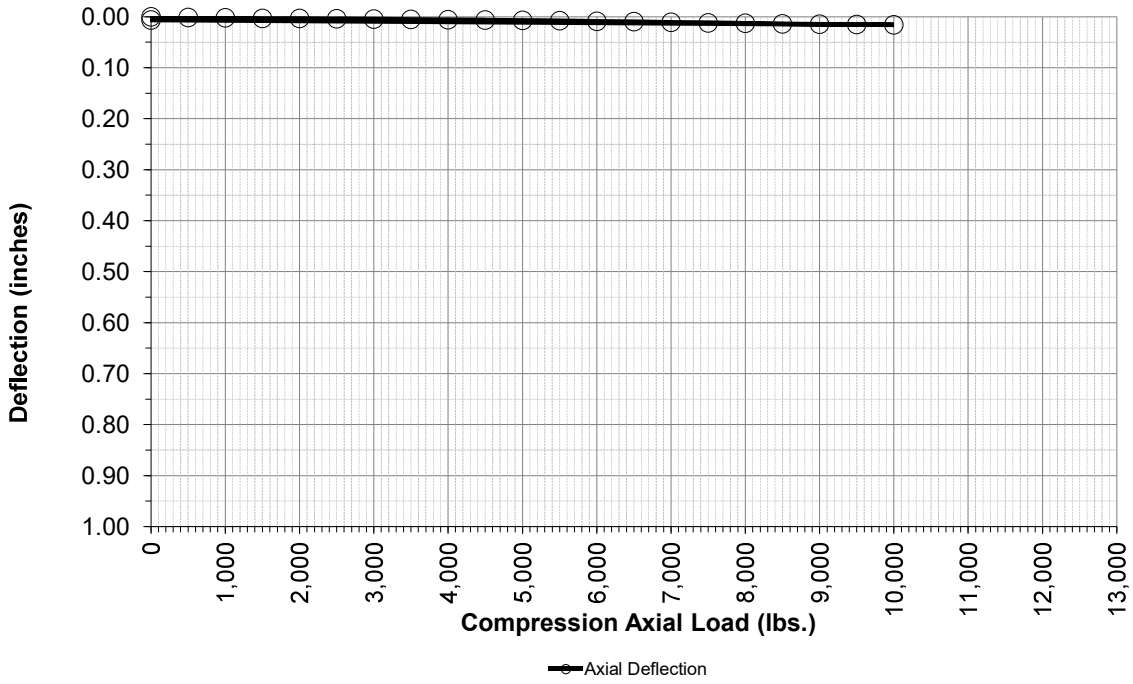
Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-2C
Latitude [deg.]:	33.66718
Longitude [deg.]:	-114.69236
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	16

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.001	
10%	1,000	0.002	
15%	1,500	0.003	
20%	2,000	0.003	
25%	2,500	0.004	
30%	3,000	0.005	
35%	3,500	0.005	
40%	4,000	0.006	
45%	4,500	0.006	
50%	5,000	0.007	
55%	5,500	0.008	
60%	6,000	0.009	
65%	6,500	0.010	
70%	7,000	0.011	
75%	7,500	0.012	
80%	8,000	0.012	
85%	8,500	0.014	
90%	9,000	0.015	
95%	9,500	0.015	
100%	10,000	0.016	
0%	0	0.006	



Compression Load Test Result for PLT-3C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

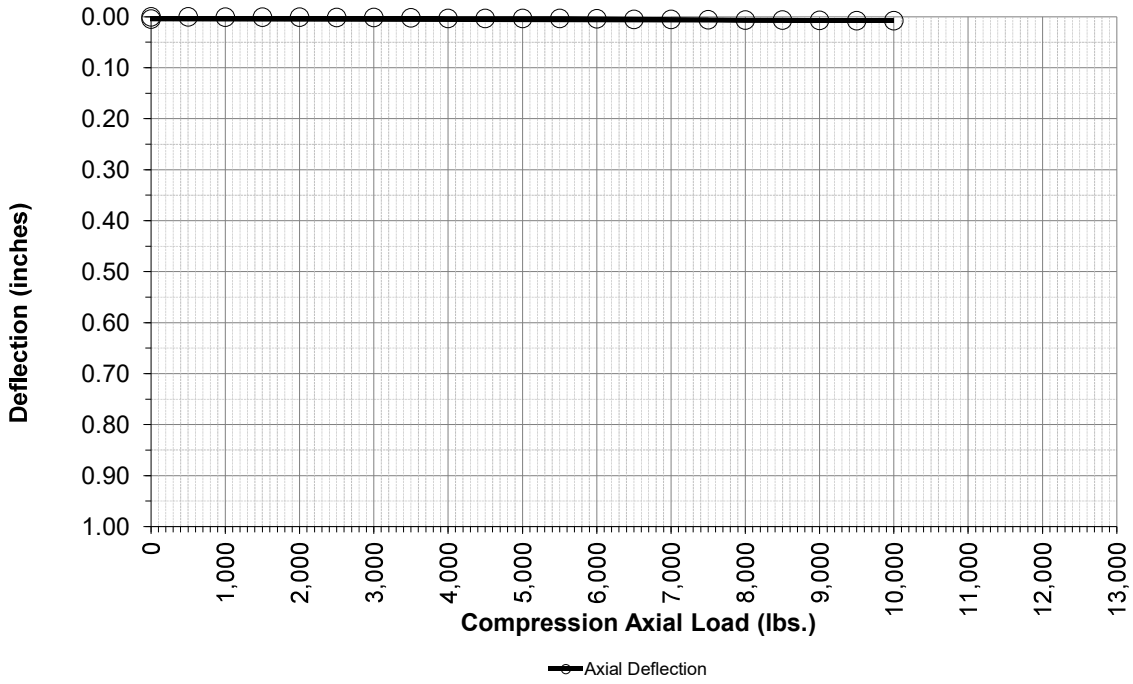
Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-3C
Latitude [deg.]:	33.66721
Longitude [deg.]:	-114.68522
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	26

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.001	
20%	2,000	0.001	
25%	2,500	0.001	
30%	3,000	0.002	
35%	3,500	0.002	
40%	4,000	0.003	
45%	4,500	0.003	
50%	5,000	0.003	
55%	5,500	0.003	
60%	6,000	0.004	
65%	6,500	0.005	
70%	7,000	0.005	
75%	7,500	0.006	
80%	8,000	0.006	
85%	8,500	0.006	
90%	9,000	0.007	
95%	9,500	0.007	
100%	10,000	0.008	
0%	0	0.004	



Compression Load Test Result for PLT-4C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

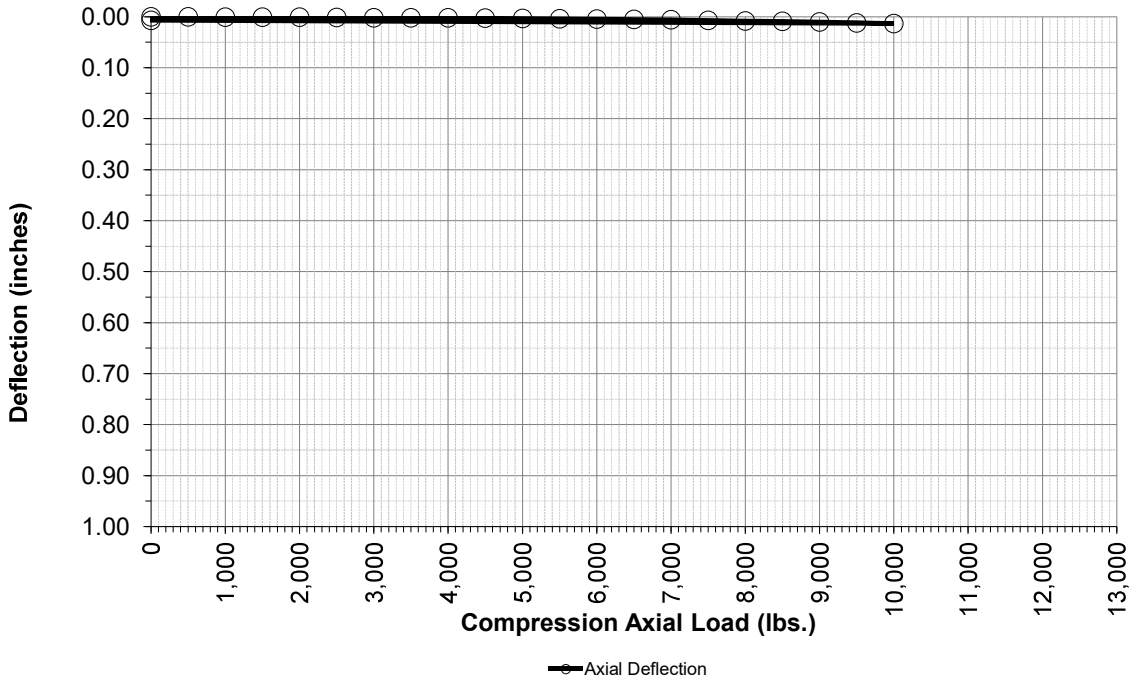
Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-4C
Latitude [deg.]:	33.66419
Longitude [deg.]:	-114.68210
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	21

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.000	
20%	2,000	0.001	
25%	2,500	0.001	
30%	3,000	0.002	
35%	3,500	0.002	
40%	4,000	0.002	
45%	4,500	0.003	
50%	5,000	0.003	
55%	5,500	0.004	
60%	6,000	0.005	
65%	6,500	0.005	
70%	7,000	0.006	
75%	7,500	0.007	
80%	8,000	0.008	
85%	8,500	0.009	
90%	9,000	0.010	
95%	9,500	0.012	
100%	10,000	0.013	
0%	0	0.007	



Compression Load Test Result for PLT-5C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

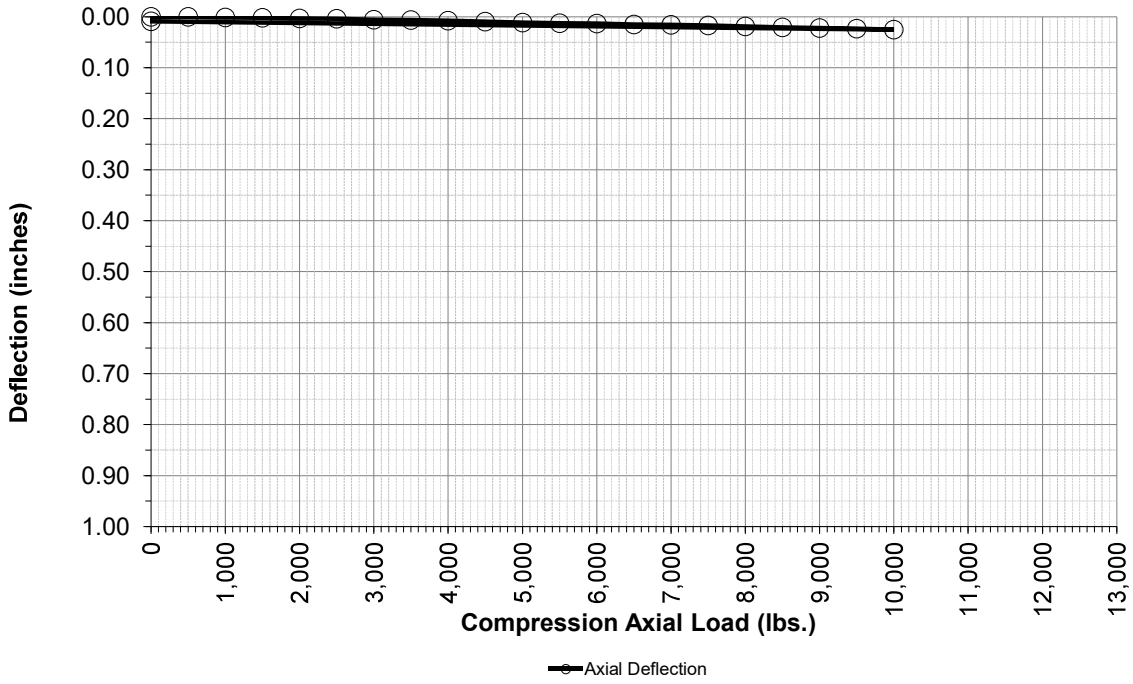
Tested By Terracon Rep:	AS/JB
Date Tested:	10/27/2023

Pile Information

Pile ID:	PLT-5C
Latitude [deg.]:	33.66358
Longitude [deg.]:	-114.69600
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	19

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.002	
15%	1,500	0.002	
20%	2,000	0.003	
25%	2,500	0.004	
30%	3,000	0.006	
35%	3,500	0.007	
40%	4,000	0.008	
45%	4,500	0.010	
50%	5,000	0.011	
55%	5,500	0.012	
60%	6,000	0.013	
65%	6,500	0.015	
70%	7,000	0.016	
75%	7,500	0.017	
80%	8,000	0.019	
85%	8,500	0.021	
90%	9,000	0.022	
95%	9,500	0.023	
100%	10,000	0.025	
0%	0	0.009	



Compression Load Test Result for PLT-6C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

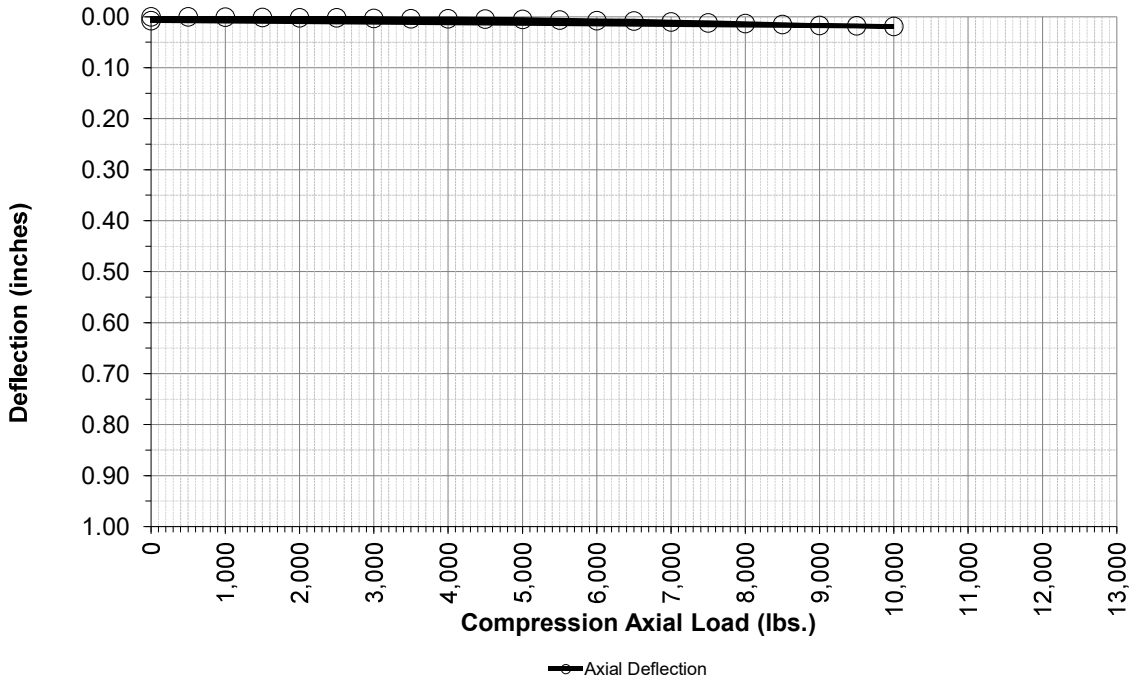
Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-6C
Latitude [deg.]:	33.66353
Longitude [deg.]:	-114.70680
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	35

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.001	
20%	2,000	0.002	
25%	2,500	0.002	
30%	3,000	0.003	
35%	3,500	0.004	
40%	4,000	0.004	
45%	4,500	0.005	
50%	5,000	0.005	
55%	5,500	0.007	
60%	6,000	0.008	
65%	6,500	0.009	
70%	7,000	0.010	
75%	7,500	0.012	
80%	8,000	0.013	
85%	8,500	0.015	
90%	9,000	0.017	
95%	9,500	0.018	
100%	10,000	0.019	
0%	0	0.007	



Compression Load Test Result for PLT-7C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

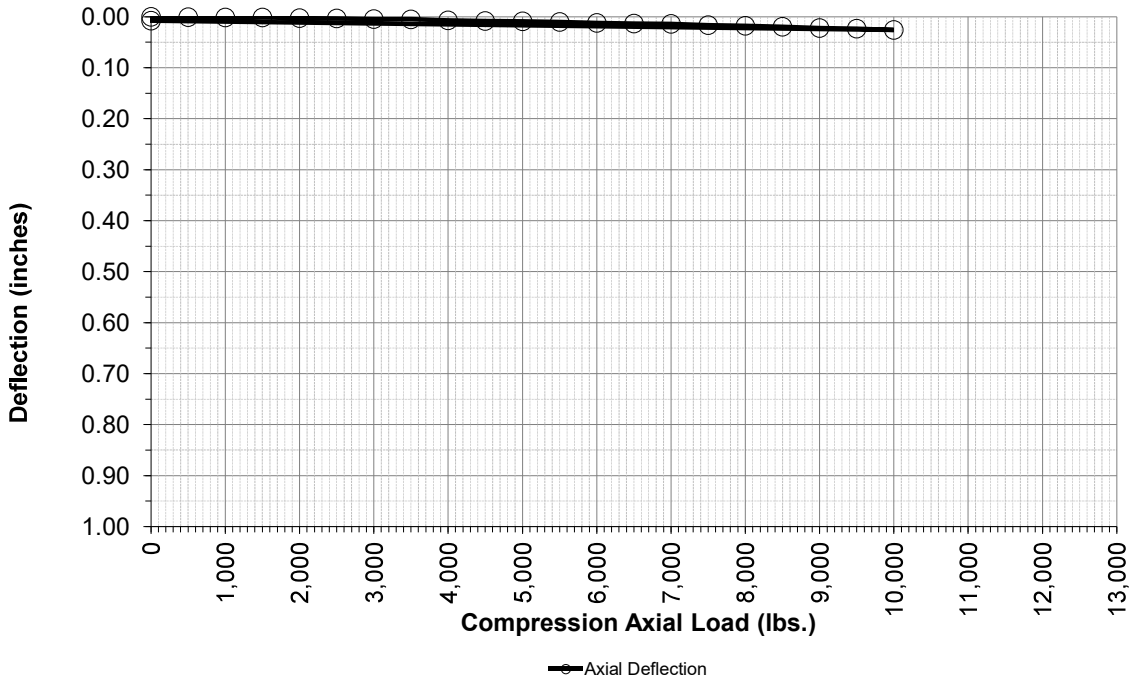
Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-7C
Latitude [deg.]:	33.66120
Longitude [deg.]:	-114.70289
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	19

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.001	
15%	1,500	0.001	
20%	2,000	0.003	
25%	2,500	0.003	
30%	3,000	0.004	
35%	3,500	0.005	
40%	4,000	0.007	
45%	4,500	0.008	
50%	5,000	0.009	
55%	5,500	0.010	
60%	6,000	0.012	
65%	6,500	0.013	
70%	7,000	0.014	
75%	7,500	0.016	
80%	8,000	0.018	
85%	8,500	0.020	
90%	9,000	0.022	
95%	9,500	0.023	
100%	10,000	0.026	
0%	0	0.008	



Compression Load Test Result for PLT-8C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

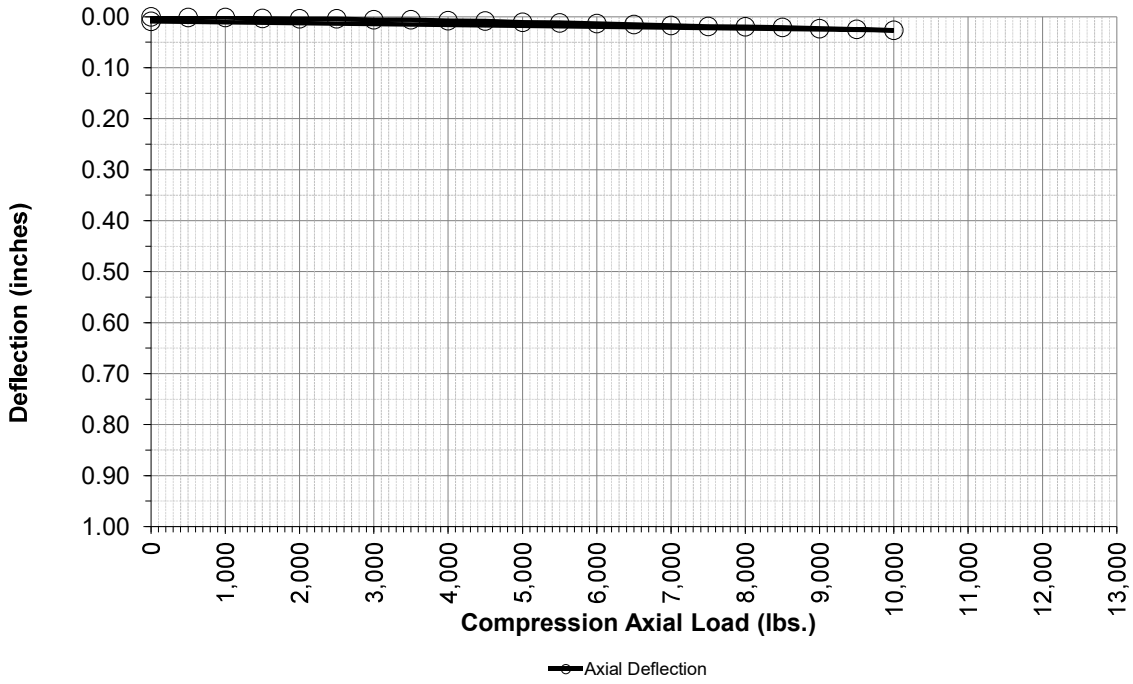
Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-8C
Latitude [deg.]:	33.66002
Longitude [deg.]:	-114.68882
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	20

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.001	
10%	1,000	0.001	
15%	1,500	0.003	
20%	2,000	0.004	
25%	2,500	0.004	
30%	3,000	0.006	
35%	3,500	0.006	
40%	4,000	0.008	
45%	4,500	0.008	
50%	5,000	0.011	
55%	5,500	0.012	
60%	6,000	0.014	
65%	6,500	0.015	
70%	7,000	0.017	
75%	7,500	0.019	
80%	8,000	0.019	
85%	8,500	0.021	
90%	9,000	0.023	
95%	9,500	0.025	
100%	10,000	0.027	
0%	0	0.009	



Compression Load Test Result for PLT-9C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

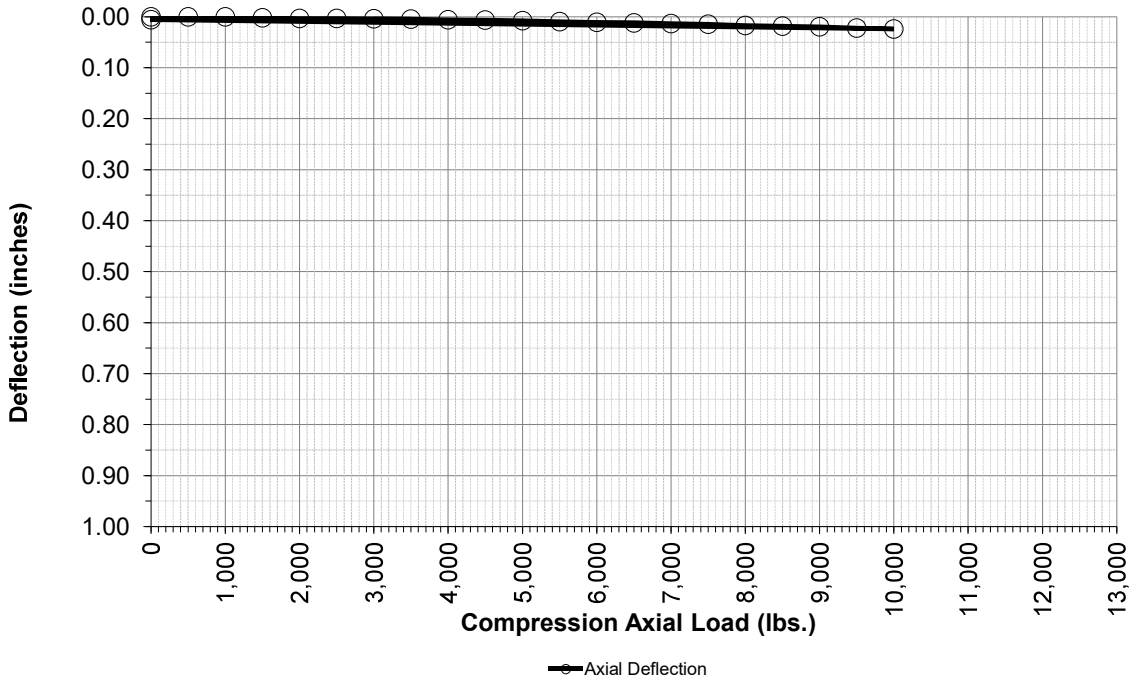
Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-9C
Latitude [deg.]:	33.65999
Longitude [deg.]:	-114.67792
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	19

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.002	
20%	2,000	0.003	
25%	2,500	0.003	
30%	3,000	0.004	
35%	3,500	0.004	
40%	4,000	0.006	
45%	4,500	0.007	
50%	5,000	0.008	
55%	5,500	0.010	
60%	6,000	0.011	
65%	6,500	0.012	
70%	7,000	0.013	
75%	7,500	0.015	
80%	8,000	0.017	
85%	8,500	0.018	
90%	9,000	0.020	
95%	9,500	0.022	
100%	10,000	0.024	
0%	0	0.006	



Compression Load Test Result for PLT-10C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

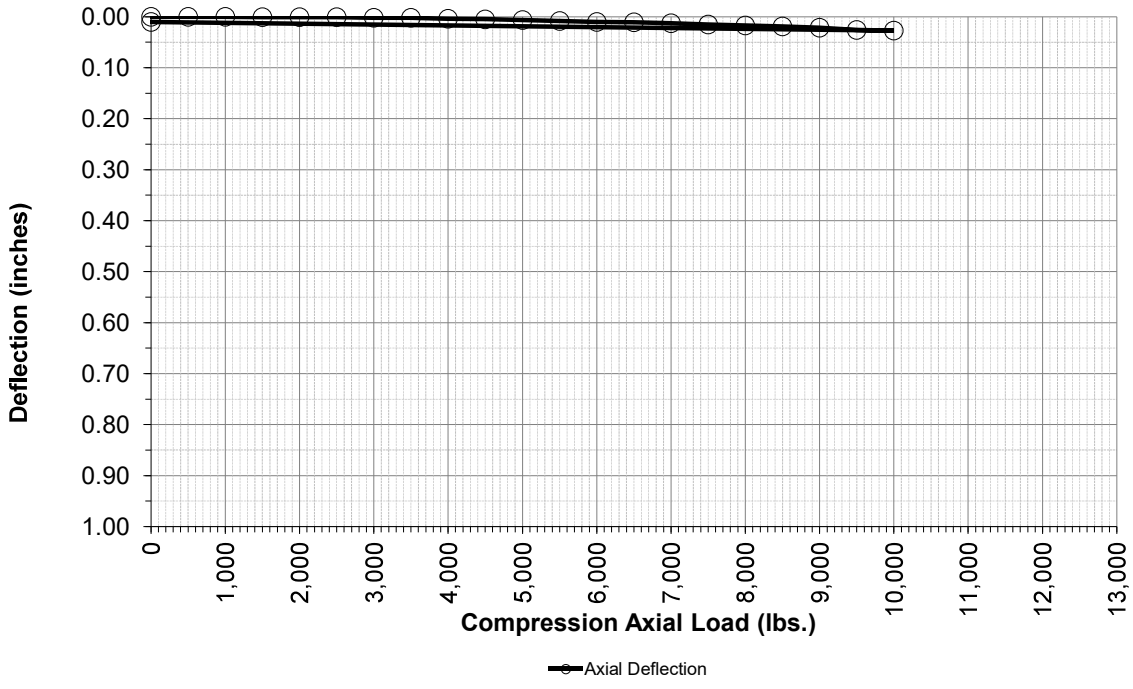
Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-10C
Latitude [deg.]:	33.65646
Longitude [deg.]:	-114.67431
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	13

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.001	
20%	2,000	0.001	
25%	2,500	0.001	
30%	3,000	0.002	
35%	3,500	0.002	
40%	4,000	0.004	
45%	4,500	0.005	
50%	5,000	0.007	
55%	5,500	0.008	
60%	6,000	0.010	
65%	6,500	0.011	
70%	7,000	0.013	
75%	7,500	0.015	
80%	8,000	0.017	
85%	8,500	0.019	
90%	9,000	0.021	
95%	9,500	0.026	
100%	10,000	0.027	
0%	0	0.010	



Compression Load Test Result for PLT-11C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

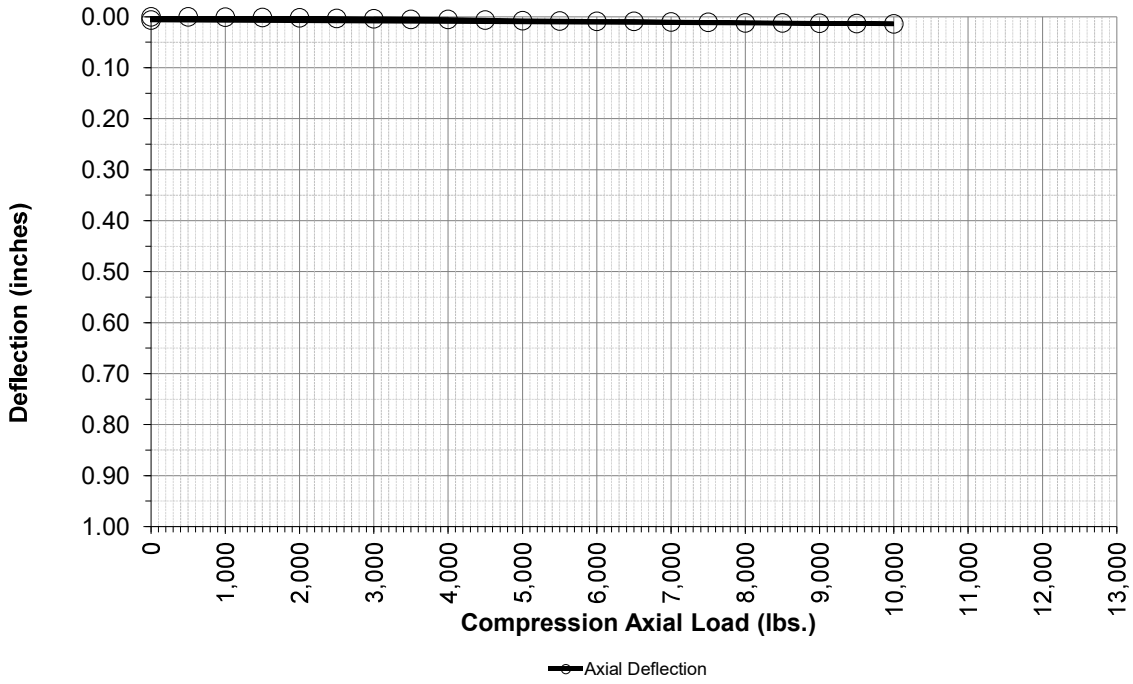
Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-11C
Latitude [deg.]:	33.65637
Longitude [deg.]:	-114.68145
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	19

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.001	
15%	1,500	0.002	
20%	2,000	0.002	
25%	2,500	0.003	
30%	3,000	0.004	
35%	3,500	0.005	
40%	4,000	0.005	
45%	4,500	0.007	
50%	5,000	0.008	
55%	5,500	0.008	
60%	6,000	0.009	
65%	6,500	0.009	
70%	7,000	0.010	
75%	7,500	0.011	
80%	8,000	0.012	
85%	8,500	0.012	
90%	9,000	0.013	
95%	9,500	0.013	
100%	10,000	0.014	
0%	0	0.007	



Compression Load Test Result for PLT-12C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

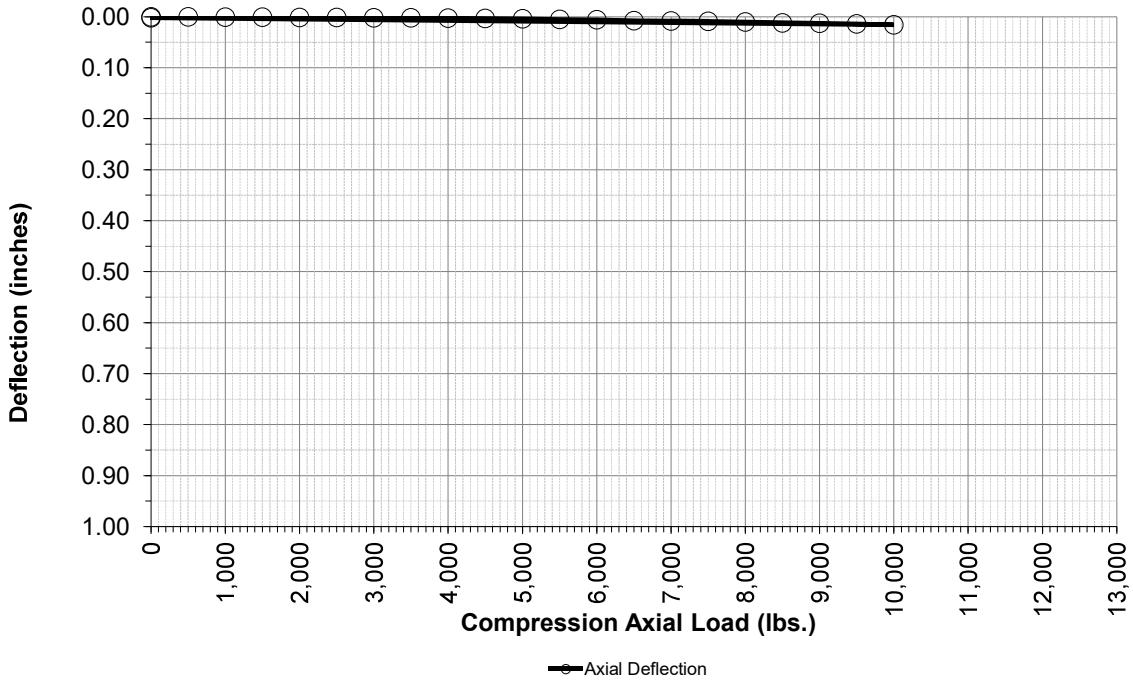
Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-12C
Latitude [deg.]:	33.65629
Longitude [deg.]:	-114.69016
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	27

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.001	
20%	2,000	0.001	
25%	2,500	0.002	
30%	3,000	0.002	
35%	3,500	0.002	
40%	4,000	0.003	
45%	4,500	0.003	
50%	5,000	0.004	
55%	5,500	0.005	
60%	6,000	0.006	
65%	6,500	0.007	
70%	7,000	0.008	
75%	7,500	0.009	
80%	8,000	0.010	
85%	8,500	0.012	
90%	9,000	0.013	
95%	9,500	0.014	
100%	10,000	0.016	
0%	0	0.002	



Compression Load Test Result for PLT-13C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

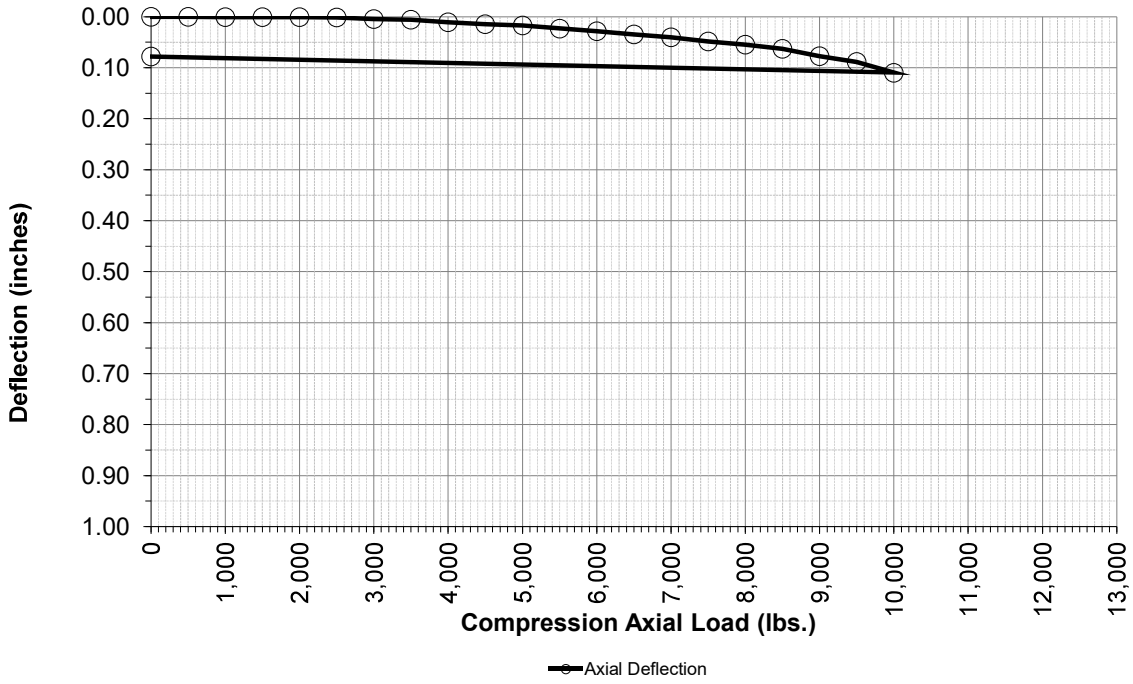
Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-13C
Latitude [deg.]:	33.65634
Longitude [deg.]:	-114.70755
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	14

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.001	
20%	2,000	0.001	
25%	2,500	0.001	
30%	3,000	0.004	
35%	3,500	0.006	
40%	4,000	0.011	
45%	4,500	0.015	
50%	5,000	0.017	
55%	5,500	0.023	
60%	6,000	0.029	
65%	6,500	0.035	
70%	7,000	0.040	
75%	7,500	0.048	
80%	8,000	0.055	
85%	8,500	0.063	
90%	9,000	0.077	
95%	9,500	0.089	
100%	10,000	0.110	
0%	0	0.078	



Compression Load Test Result for PLT-14C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

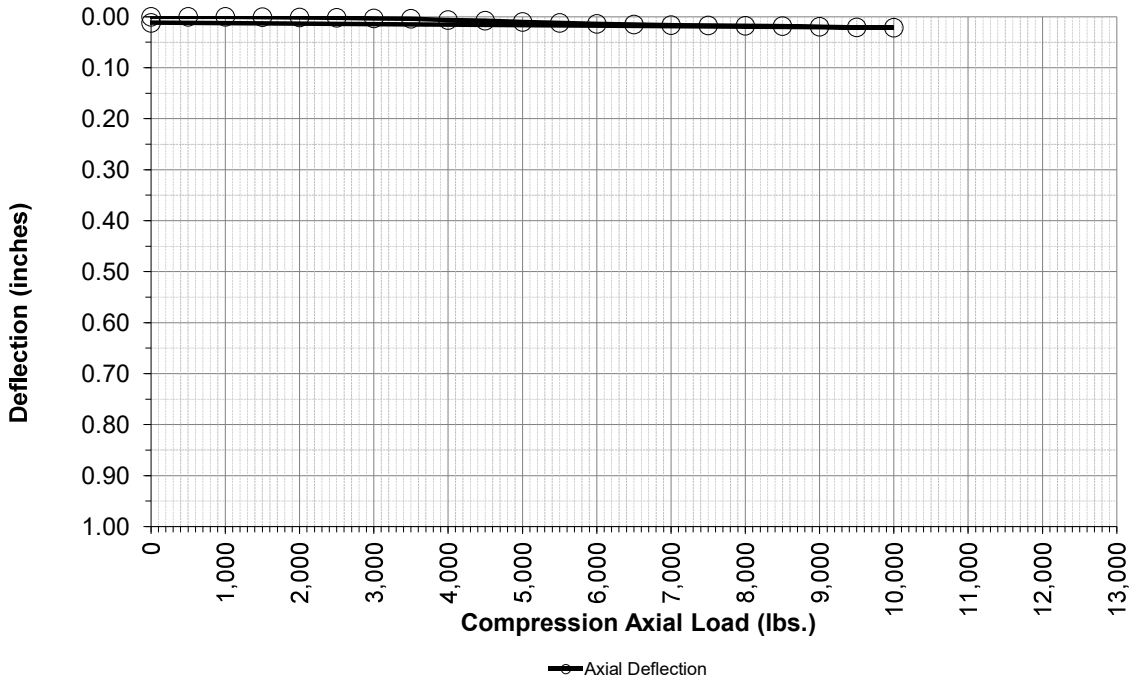
Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-14C
Latitude [deg.]:	33.65271
Longitude [deg.]:	-114.71502
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	16

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.001	
20%	2,000	0.001	
25%	2,500	0.002	
30%	3,000	0.003	
35%	3,500	0.004	
40%	4,000	0.006	
45%	4,500	0.008	
50%	5,000	0.010	
55%	5,500	0.012	
60%	6,000	0.014	
65%	6,500	0.015	
70%	7,000	0.016	
75%	7,500	0.017	
80%	8,000	0.018	
85%	8,500	0.018	
90%	9,000	0.020	
95%	9,500	0.021	
100%	10,000	0.021	
0%	0	0.012	



Compression Load Test Result for PLT-15C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

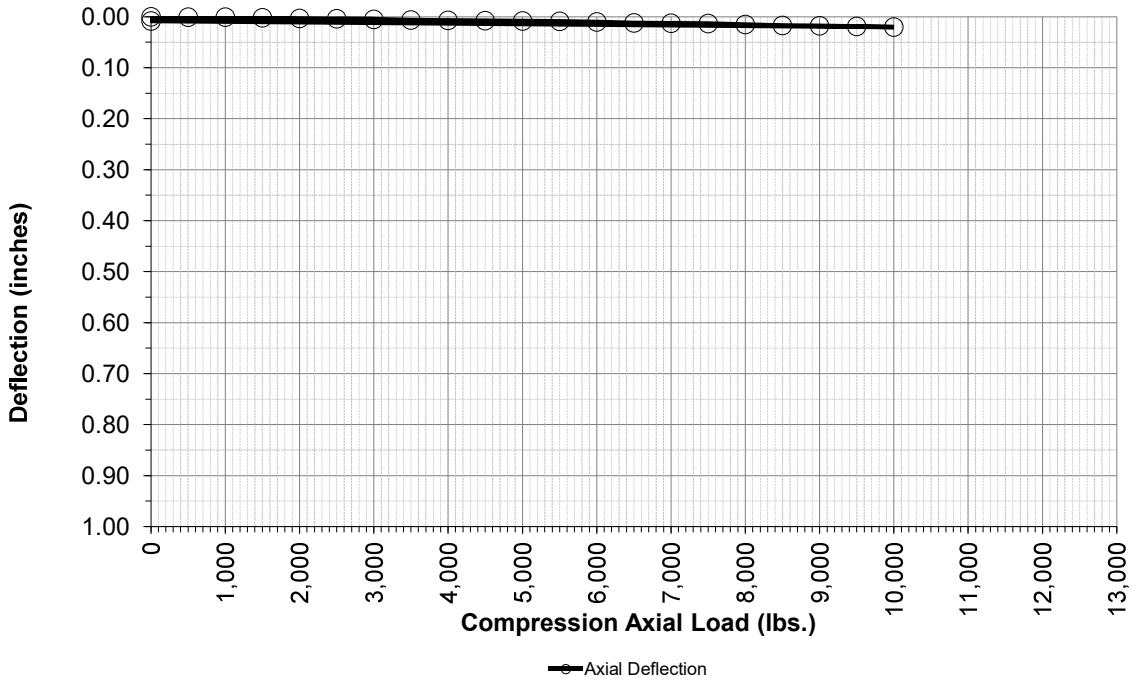
Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-15C
Latitude [deg.]:	33.65271
Longitude [deg.]:	-114.70499
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	28

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.001	
15%	1,500	0.002	
20%	2,000	0.003	
25%	2,500	0.004	
30%	3,000	0.005	
35%	3,500	0.007	
40%	4,000	0.007	
45%	4,500	0.008	
50%	5,000	0.008	
55%	5,500	0.009	
60%	6,000	0.010	
65%	6,500	0.012	
70%	7,000	0.013	
75%	7,500	0.014	
80%	8,000	0.015	
85%	8,500	0.017	
90%	9,000	0.018	
95%	9,500	0.019	
100%	10,000	0.020	
0%	0	0.009	



Compression Load Test Result for PLT-16C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

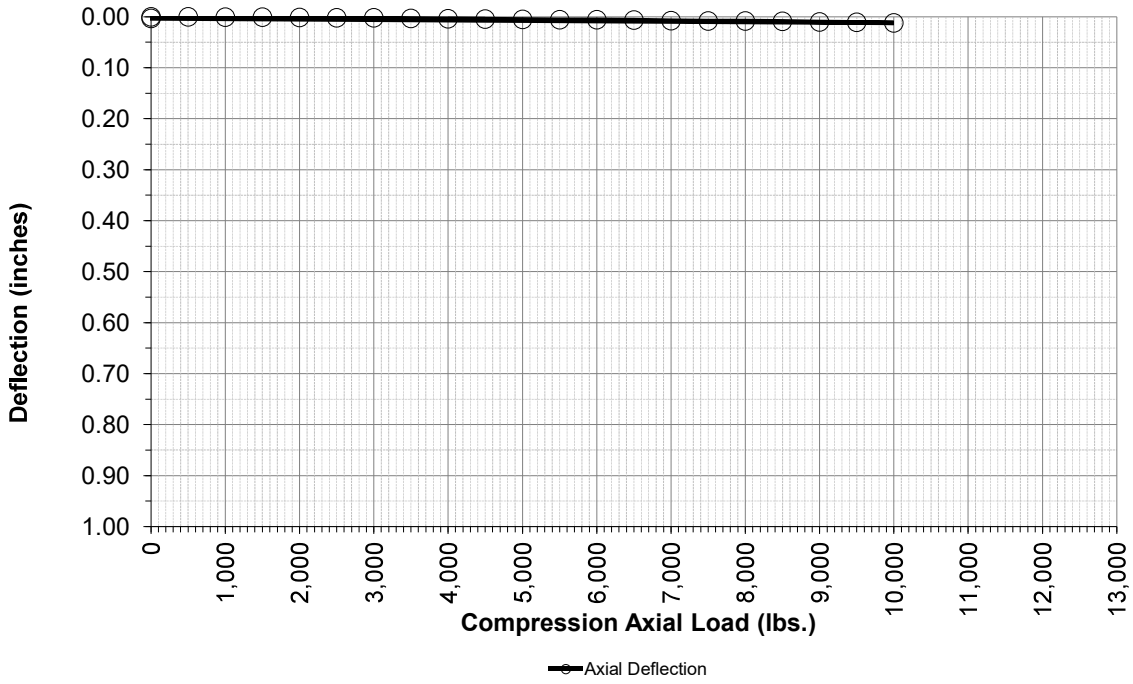
Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-16C
Latitude [deg.]:	33.64961
Longitude [deg.]:	-114.70922
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	17

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.001	
20%	2,000	0.001	
25%	2,500	0.002	
30%	3,000	0.002	
35%	3,500	0.003	
40%	4,000	0.004	
45%	4,500	0.004	
50%	5,000	0.005	
55%	5,500	0.006	
60%	6,000	0.006	
65%	6,500	0.007	
70%	7,000	0.008	
75%	7,500	0.008	
80%	8,000	0.009	
85%	8,500	0.009	
90%	9,000	0.010	
95%	9,500	0.011	
100%	10,000	0.012	
0%	0	0.003	



Compression Load Test Result for PLT-17C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

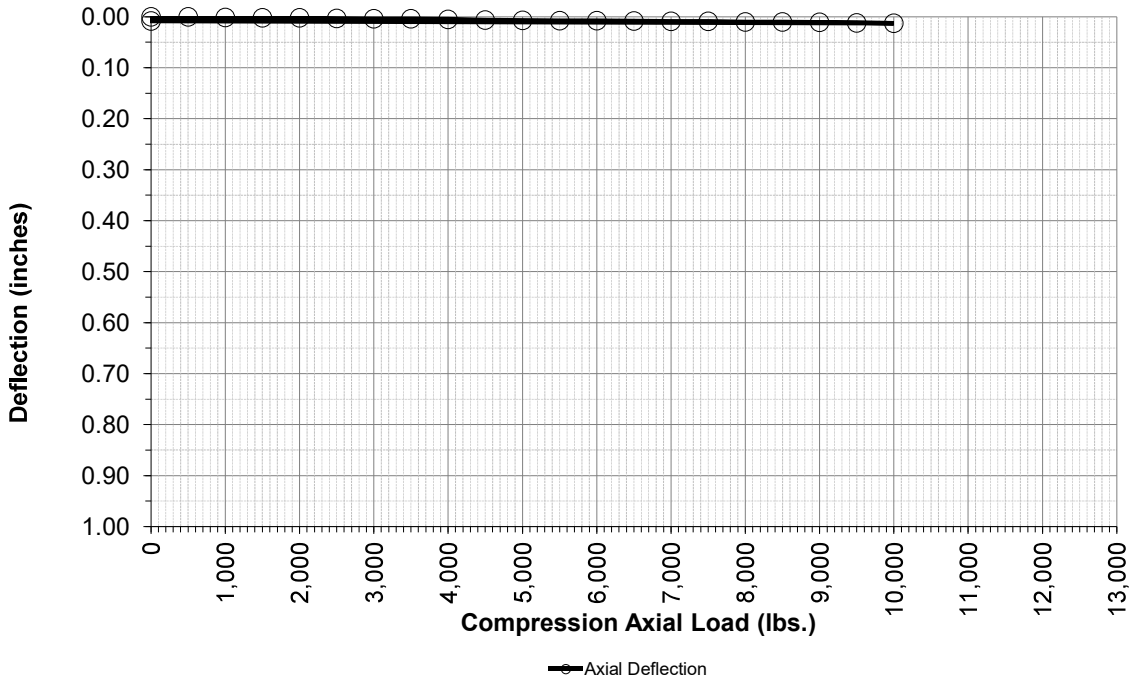
Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-17C
Latitude [deg.]:	33.65156
Longitude [deg.]:	-114.68472
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	14

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.001	
15%	1,500	0.002	
20%	2,000	0.002	
25%	2,500	0.003	
30%	3,000	0.004	
35%	3,500	0.004	
40%	4,000	0.005	
45%	4,500	0.007	
50%	5,000	0.007	
55%	5,500	0.007	
60%	6,000	0.008	
65%	6,500	0.008	
70%	7,000	0.009	
75%	7,500	0.009	
80%	8,000	0.010	
85%	8,500	0.010	
90%	9,000	0.011	
95%	9,500	0.012	
100%	10,000	0.013	
0%	0	0.009	



Compression Load Test Result for PLT-18C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

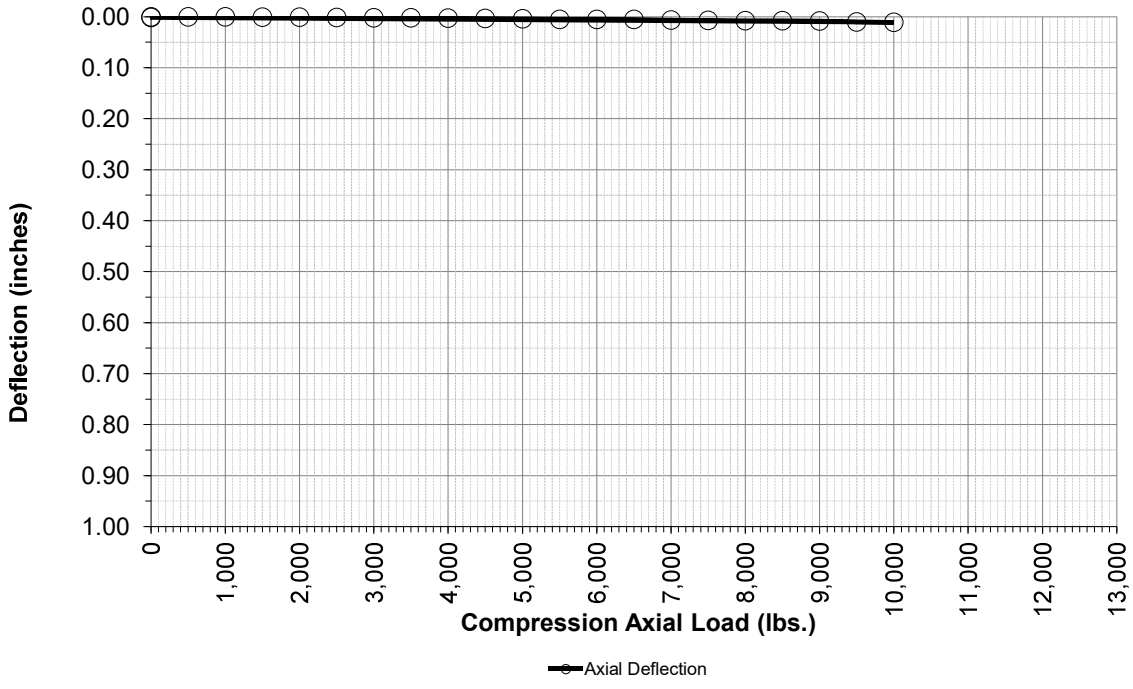
Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-18C
Latitude [deg.]:	33.65278
Longitude [deg.]:	-114.66998
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	31

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.001	
20%	2,000	0.001	
25%	2,500	0.002	
30%	3,000	0.002	
35%	3,500	0.002	
40%	4,000	0.003	
45%	4,500	0.003	
50%	5,000	0.004	
55%	5,500	0.005	
60%	6,000	0.005	
65%	6,500	0.005	
70%	7,000	0.006	
75%	7,500	0.007	
80%	8,000	0.008	
85%	8,500	0.008	
90%	9,000	0.009	
95%	9,500	0.010	
100%	10,000	0.011	
0%	0	0.001	



Compression Load Test Result for PLT-19C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

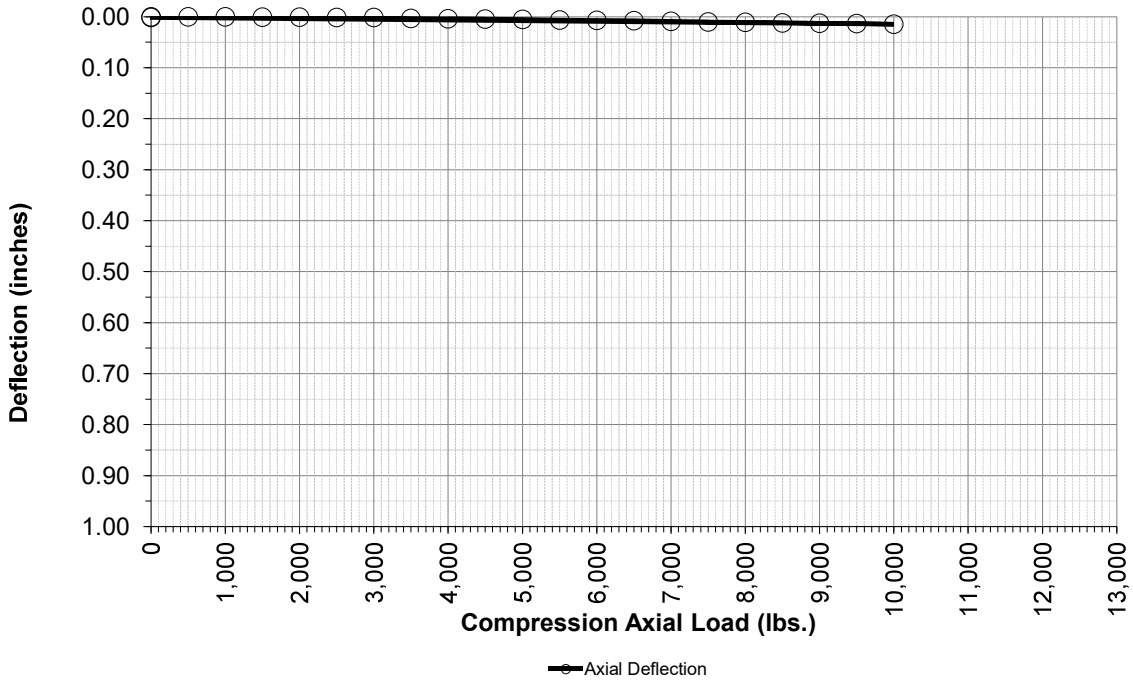
Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-19C
Latitude [deg.]:	33.64872
Longitude [deg.]:	-114.66589
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	29

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.001	
20%	2,000	0.001	
25%	2,500	0.001	
30%	3,000	0.002	
35%	3,500	0.003	
40%	4,000	0.004	
45%	4,500	0.004	
50%	5,000	0.005	
55%	5,500	0.006	
60%	6,000	0.007	
65%	6,500	0.008	
70%	7,000	0.009	
75%	7,500	0.010	
80%	8,000	0.011	
85%	8,500	0.012	
90%	9,000	0.013	
95%	9,500	0.014	
100%	10,000	0.015	
0%	0	0.002	



Compression Load Test Result for PLT-20C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

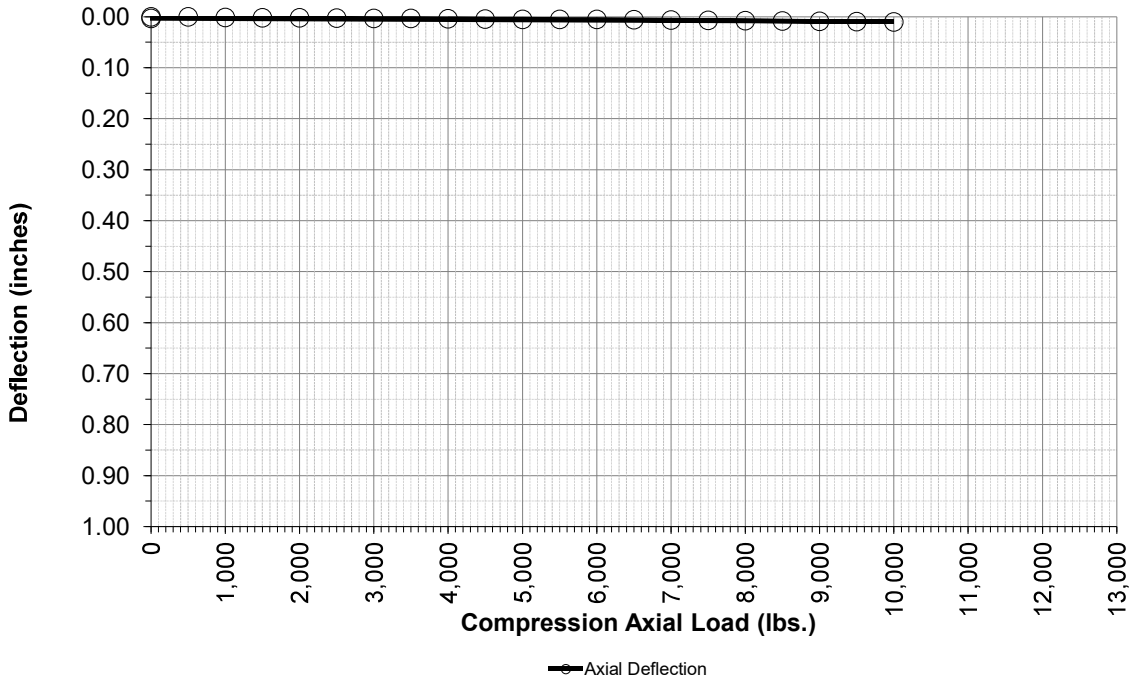
Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-20C
Latitude [deg.]:	33.64924
Longitude [deg.]:	-114.68011
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	25

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.002	
15%	1,500	0.002	
20%	2,000	0.002	
25%	2,500	0.003	
30%	3,000	0.003	
35%	3,500	0.003	
40%	4,000	0.004	
45%	4,500	0.004	
50%	5,000	0.005	
55%	5,500	0.005	
60%	6,000	0.005	
65%	6,500	0.006	
70%	7,000	0.006	
75%	7,500	0.007	
80%	8,000	0.008	
85%	8,500	0.008	
90%	9,000	0.009	
95%	9,500	0.010	
100%	10,000	0.010	
0%	0	0.003	



Compression Load Test Result for PLT-21C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

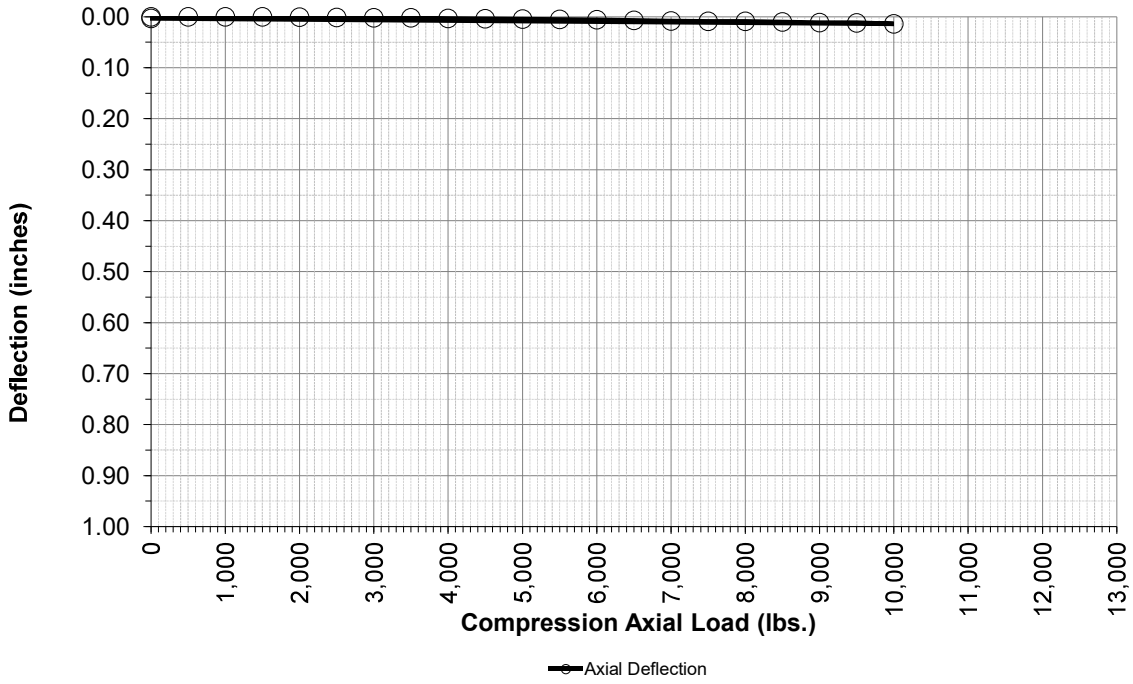
Tested By Terracon Rep:	AS/JB
Date Tested:	10/25/2023

Pile Information

Pile ID:	PLT-21C
Latitude [deg.]:	33.64923
Longitude [deg.]:	-114.69007
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	15

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.000	
20%	2,000	0.001	
25%	2,500	0.001	
30%	3,000	0.002	
35%	3,500	0.002	
40%	4,000	0.003	
45%	4,500	0.004	
50%	5,000	0.005	
55%	5,500	0.005	
60%	6,000	0.006	
65%	6,500	0.007	
70%	7,000	0.008	
75%	7,500	0.009	
80%	8,000	0.009	
85%	8,500	0.010	
90%	9,000	0.011	
95%	9,500	0.012	
100%	10,000	0.014	
0%	0	0.003	



Compression Load Test Result for PLT-22C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

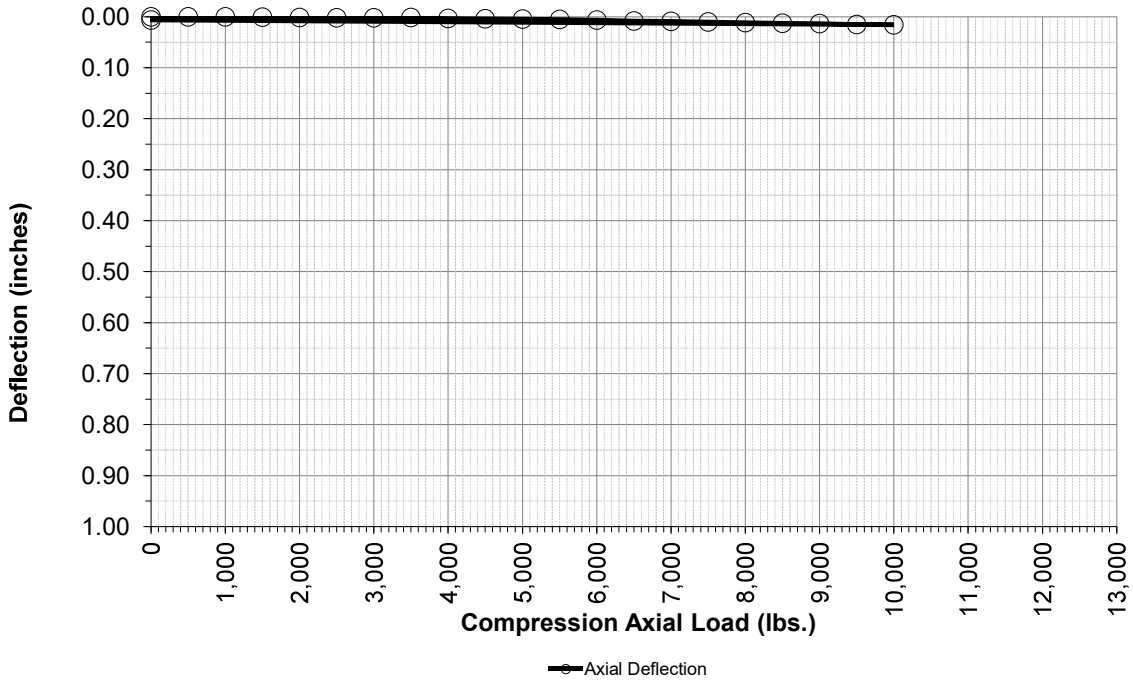
Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-22C
Latitude [deg.]:	33.64553
Longitude [deg.]:	-114.71498
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	22

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.001	
20%	2,000	0.001	
25%	2,500	0.002	
30%	3,000	0.002	
35%	3,500	0.002	
40%	4,000	0.003	
45%	4,500	0.004	
50%	5,000	0.005	
55%	5,500	0.005	
60%	6,000	0.007	
65%	6,500	0.008	
70%	7,000	0.009	
75%	7,500	0.010	
80%	8,000	0.011	
85%	8,500	0.012	
90%	9,000	0.014	
95%	9,500	0.015	
100%	10,000	0.016	
0%	0	0.006	



Compression Load Test Result for PLT-23C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

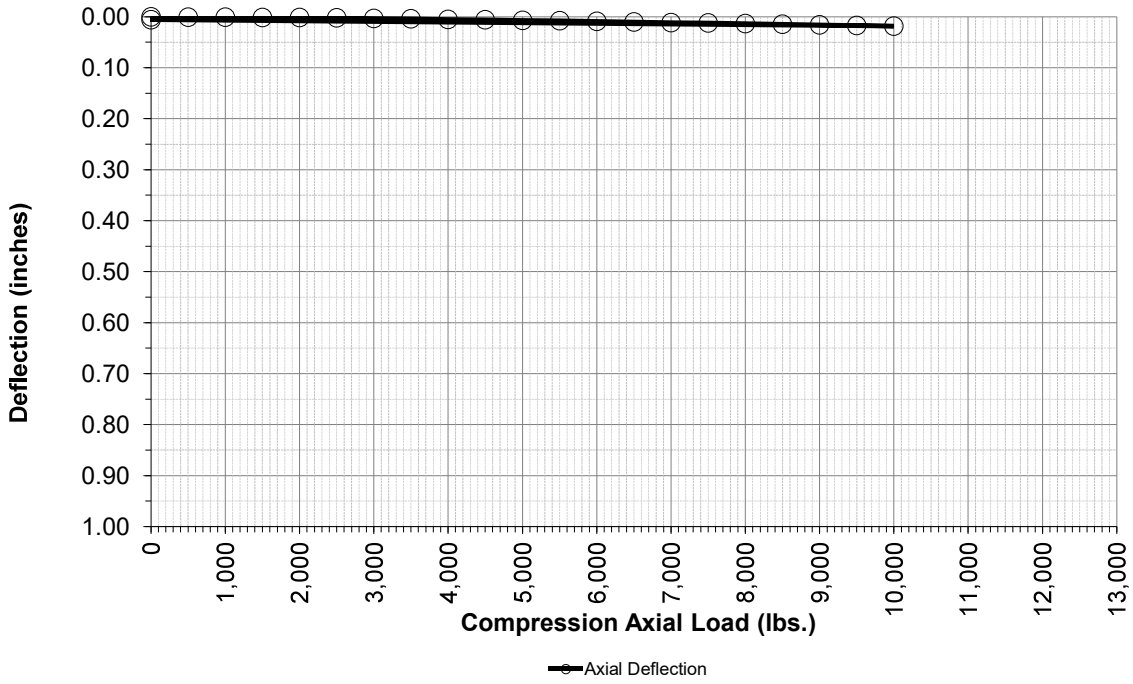
Tested By Terracon Rep:	AS/JB
Date Tested:	11/3/2023

Pile Information

Pile ID:	PLT-23C
Latitude [deg.]:	33.63834
Longitude [deg.]:	-114.69507
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	31

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.001	
10%	1,000	0.001	
15%	1,500	0.001	
20%	2,000	0.002	
25%	2,500	0.002	
30%	3,000	0.003	
35%	3,500	0.004	
40%	4,000	0.005	
45%	4,500	0.006	
50%	5,000	0.007	
55%	5,500	0.008	
60%	6,000	0.009	
65%	6,500	0.010	
70%	7,000	0.011	
75%	7,500	0.012	
80%	8,000	0.013	
85%	8,500	0.015	
90%	9,000	0.016	
95%	9,500	0.017	
100%	10,000	0.018	
0%	0	0.006	



Compression Load Test Result for PLT-24C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

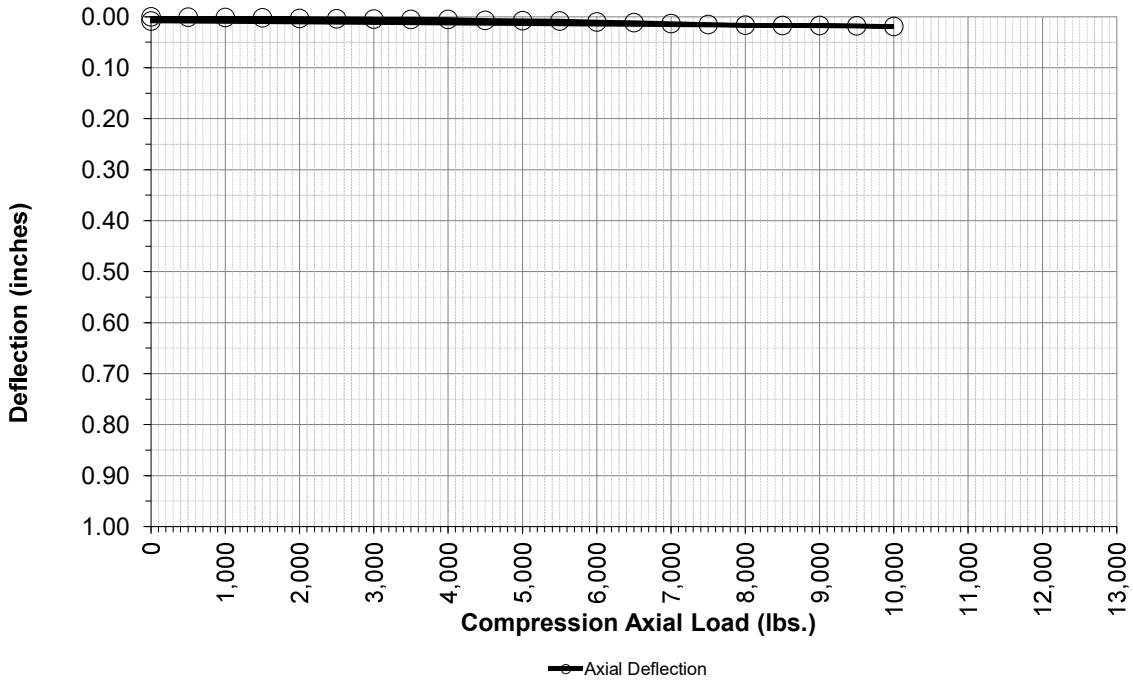
Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-24C
Latitude [deg.]:	33.63275
Longitude [deg.]:	-114.68662
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	11

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.001	
10%	1,000	0.001	
15%	1,500	0.002	
20%	2,000	0.003	
25%	2,500	0.004	
30%	3,000	0.005	
35%	3,500	0.005	
40%	4,000	0.005	
45%	4,500	0.007	
50%	5,000	0.008	
55%	5,500	0.009	
60%	6,000	0.010	
65%	6,500	0.011	
70%	7,000	0.013	
75%	7,500	0.015	
80%	8,000	0.016	
85%	8,500	0.017	
90%	9,000	0.017	
95%	9,500	0.018	
100%	10,000	0.019	
0%	0	0.008	



Compression Load Test Result for PLT-25C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

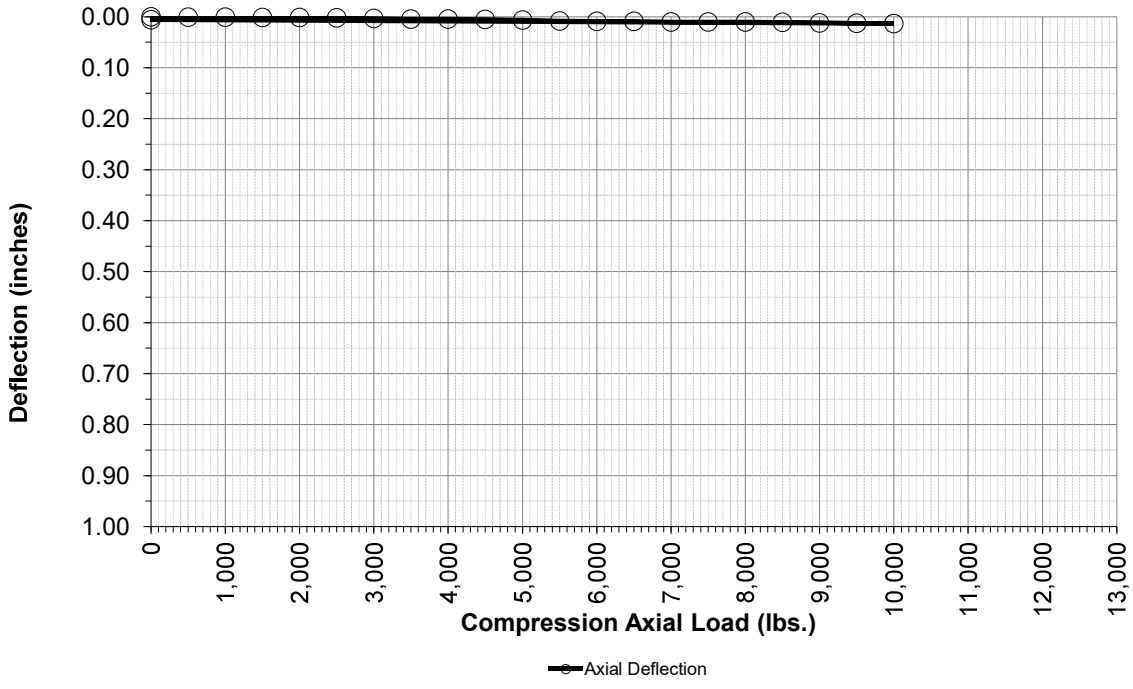
Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-25C
Latitude [deg.]:	33.63707
Longitude [deg.]:	-114.68710
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	16

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.001	
10%	1,000	0.001	
15%	1,500	0.001	
20%	2,000	0.001	
25%	2,500	0.002	
30%	3,000	0.003	
35%	3,500	0.005	
40%	4,000	0.005	
45%	4,500	0.005	
50%	5,000	0.007	
55%	5,500	0.008	
60%	6,000	0.009	
65%	6,500	0.009	
70%	7,000	0.010	
75%	7,500	0.010	
80%	8,000	0.010	
85%	8,500	0.011	
90%	9,000	0.012	
95%	9,500	0.012	
100%	10,000	0.014	
0%	0	0.006	



Compression Load Test Result for PLT-26C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

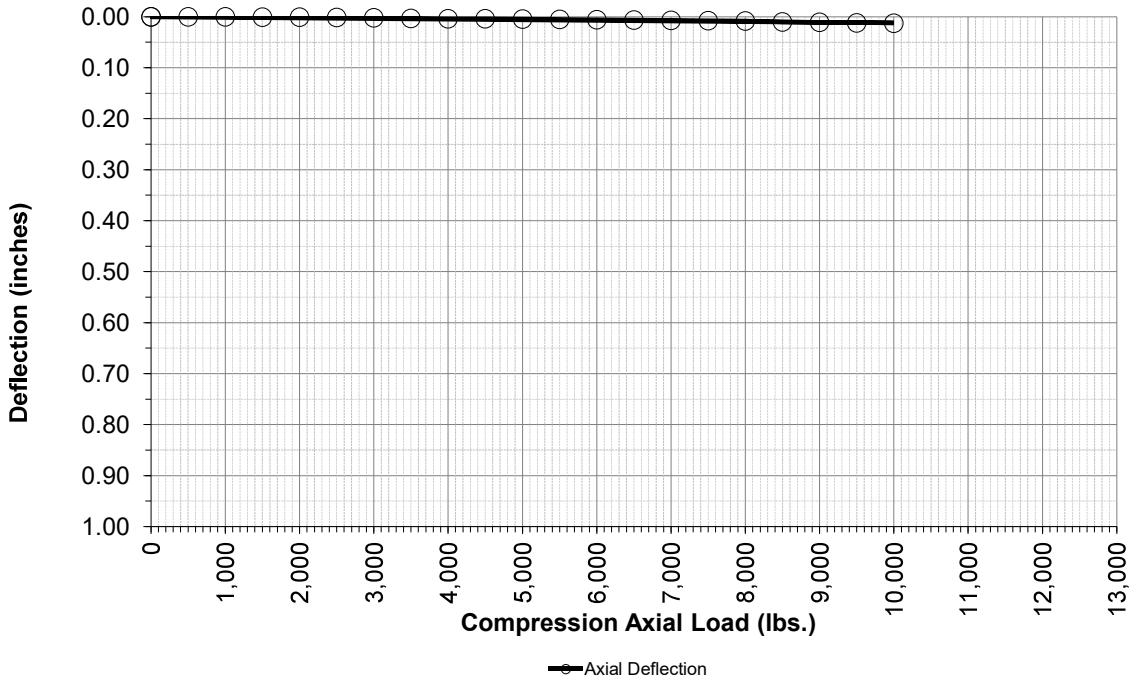
Tested By Terracon Rep:	AS/JB
Date Tested:	10/23/2023

Pile Information

Pile ID:	PLT-26C
Latitude [deg.]:	33.63463
Longitude [deg.]:	-114.67628
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	27

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.000	
20%	2,000	0.001	
25%	2,500	0.002	
30%	3,000	0.002	
35%	3,500	0.003	
40%	4,000	0.004	
45%	4,500	0.004	
50%	5,000	0.005	
55%	5,500	0.005	
60%	6,000	0.006	
65%	6,500	0.007	
70%	7,000	0.007	
75%	7,500	0.007	
80%	8,000	0.008	
85%	8,500	0.010	
90%	9,000	0.011	
95%	9,500	0.012	
100%	10,000	0.012	
0%	0	0.000	



Compression Load Test Result for PLT-27C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

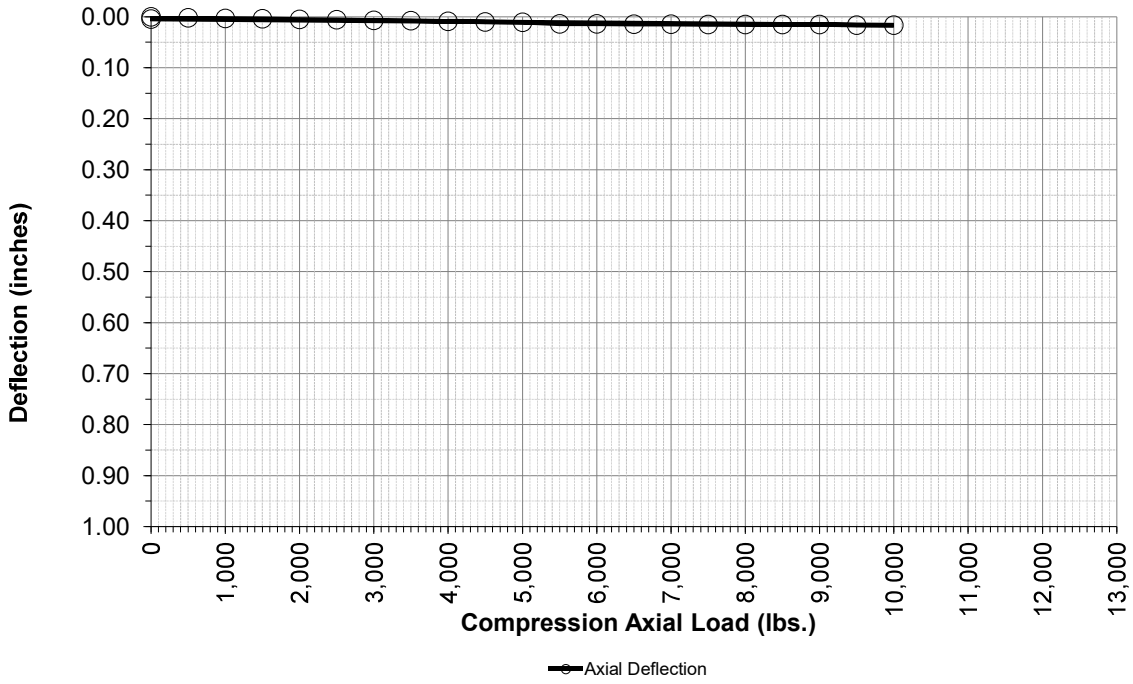
Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-27C
Latitude [deg.]:	33.63981
Longitude [deg.]:	-114.68128
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	18

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.002	
10%	1,000	0.003	
15%	1,500	0.004	
20%	2,000	0.005	
25%	2,500	0.006	
30%	3,000	0.007	
35%	3,500	0.008	
40%	4,000	0.009	
45%	4,500	0.010	
50%	5,000	0.011	
55%	5,500	0.014	
60%	6,000	0.014	
65%	6,500	0.014	
70%	7,000	0.015	
75%	7,500	0.015	
80%	8,000	0.015	
85%	8,500	0.015	
90%	9,000	0.015	
95%	9,500	0.016	
100%	10,000	0.017	
0%	0	0.005	



Compression Load Test Result for PLT-28C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

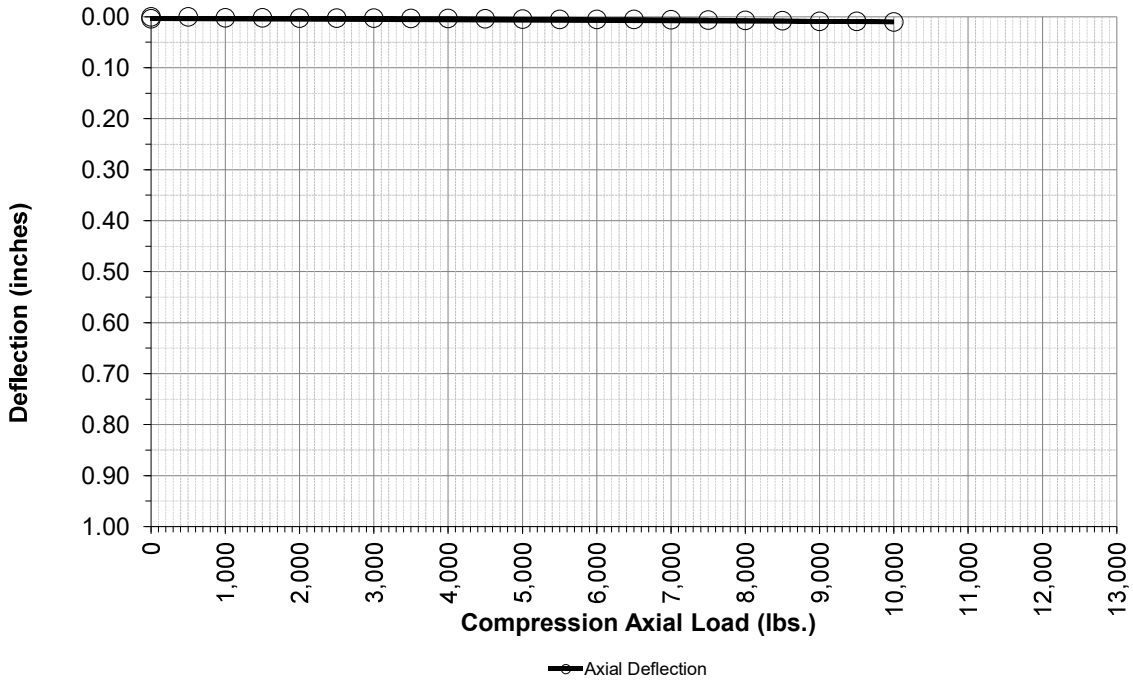
Tested By Terracon Rep:	AS/JB
Date Tested:	10/24/2023

Pile Information

Pile ID:	PLT-28C
Latitude [deg.]:	33.64552
Longitude [deg.]:	-114.67984
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	16

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.002	
15%	1,500	0.002	
20%	2,000	0.003	
25%	2,500	0.003	
30%	3,000	0.003	
35%	3,500	0.003	
40%	4,000	0.003	
45%	4,500	0.004	
50%	5,000	0.004	
55%	5,500	0.005	
60%	6,000	0.005	
65%	6,500	0.005	
70%	7,000	0.006	
75%	7,500	0.006	
80%	8,000	0.007	
85%	8,500	0.008	
90%	9,000	0.009	
95%	9,500	0.009	
100%	10,000	0.010	
0%	0	0.004	



Compression Load Test Result for PLT-29C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

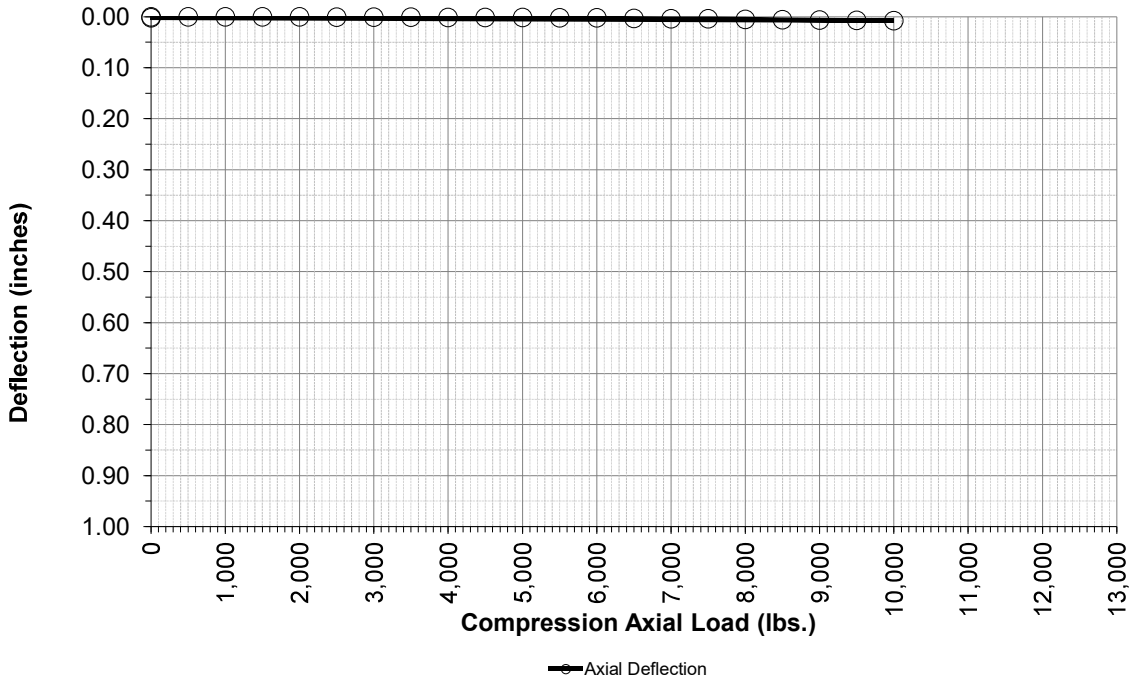
Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-29C
Latitude [deg.]:	33.64547
Longitude [deg.]:	-114.66992
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	38

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.000	
20%	2,000	0.000	
25%	2,500	0.000	
30%	3,000	0.001	
35%	3,500	0.001	
40%	4,000	0.001	
45%	4,500	0.001	
50%	5,000	0.001	
55%	5,500	0.002	
60%	6,000	0.002	
65%	6,500	0.003	
70%	7,000	0.004	
75%	7,500	0.004	
80%	8,000	0.005	
85%	8,500	0.006	
90%	9,000	0.006	
95%	9,500	0.007	
100%	10,000	0.008	
0%	0	0.002	



Compression Load Test Result for PLT-30C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

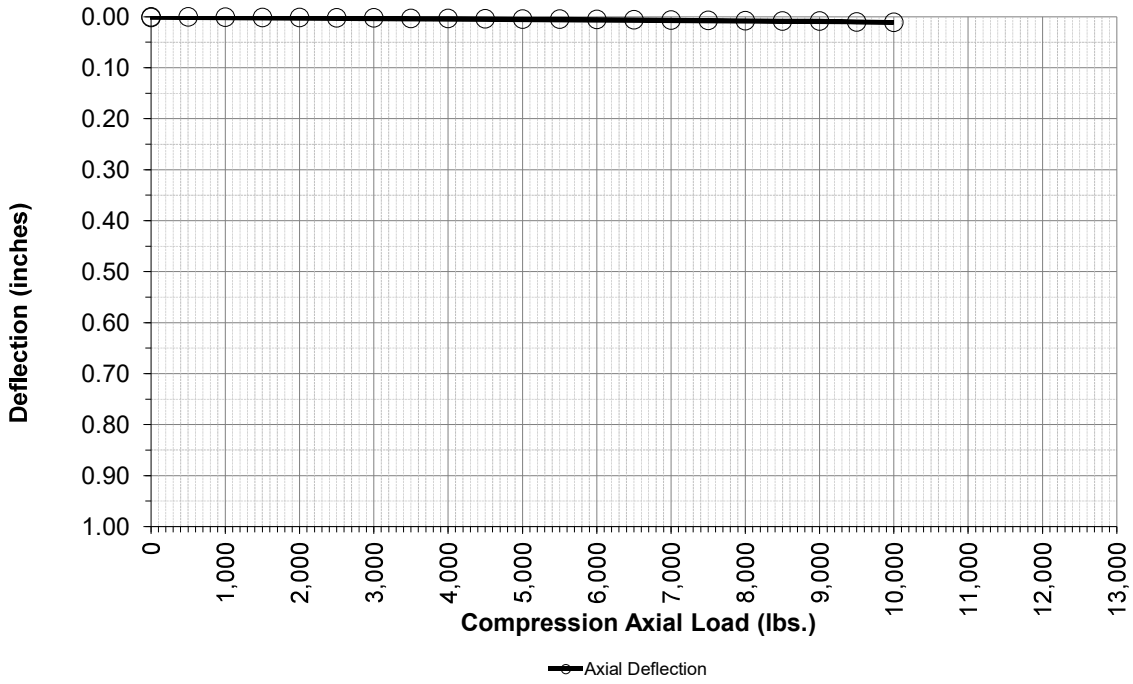
Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-30C
Latitude [deg.]:	33.64348
Longitude [deg.]:	-114.66501
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	25

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.001	
15%	1,500	0.001	
20%	2,000	0.002	
25%	2,500	0.002	
30%	3,000	0.002	
35%	3,500	0.003	
40%	4,000	0.003	
45%	4,500	0.004	
50%	5,000	0.004	
55%	5,500	0.004	
60%	6,000	0.005	
65%	6,500	0.006	
70%	7,000	0.007	
75%	7,500	0.007	
80%	8,000	0.008	
85%	8,500	0.008	
90%	9,000	0.009	
95%	9,500	0.010	
100%	10,000	0.011	
0%	0	0.001	



Compression Load Test Result for PLT-31C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

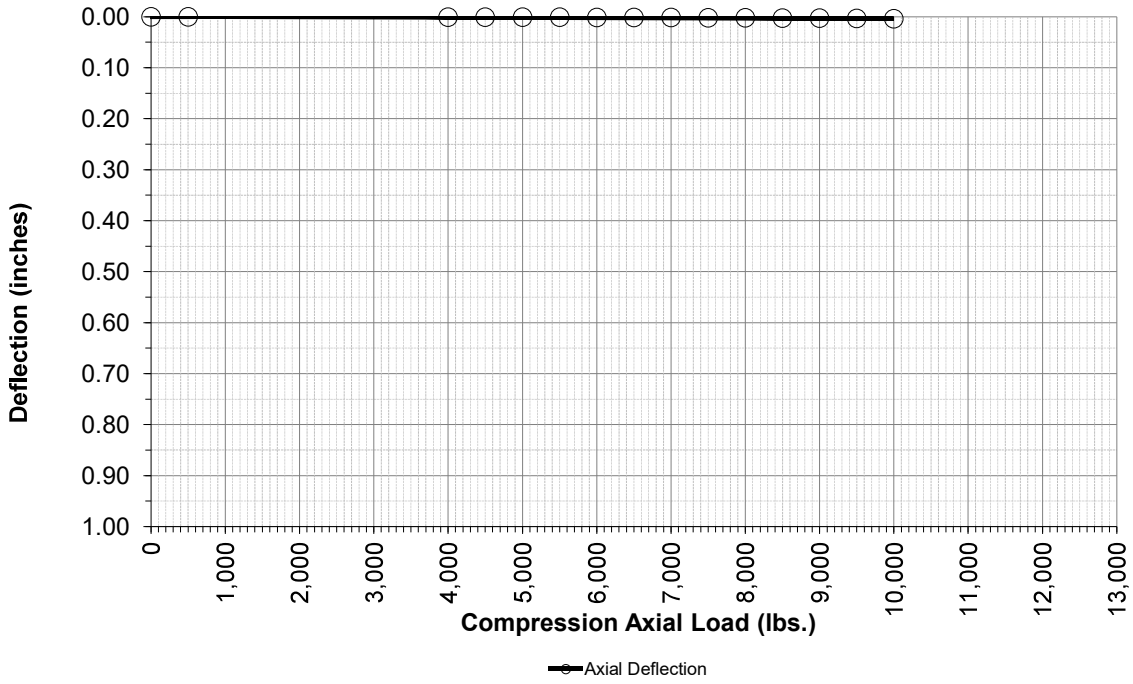
Tested By Terracon Rep:	AS/JB
Date Tested:	10/31/2023

Pile Information

Pile ID:	PLT-31C
Latitude [deg.]:	33.64645
Longitude [deg.]:	-114.66137
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	38

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	-0.001	
20%	2,000	-0.001	
25%	2,500	-0.001	
30%	3,000	0.000	
35%	3,500	0.000	
40%	4,000	0.000	
45%	4,500	0.000	
50%	5,000	0.001	
55%	5,500	0.001	
60%	6,000	0.001	
65%	6,500	0.001	
70%	7,000	0.002	
75%	7,500	0.002	
80%	8,000	0.002	
85%	8,500	0.003	
90%	9,000	0.003	
95%	9,500	0.003	
100%	10,000	0.004	
0%	0	-0.001	



Compression Load Test Result for PLT-32C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

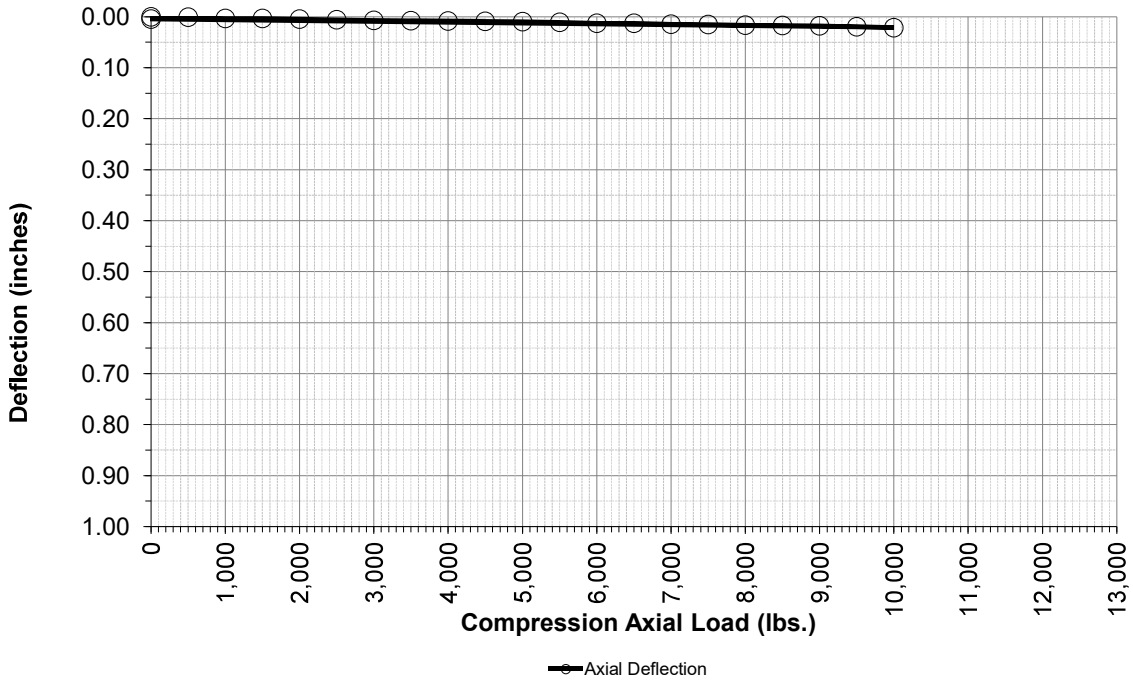
Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-32C
Latitude [deg.]:	33.63285
Longitude [deg.]:	-114.69500
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	28

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.001	
10%	1,000	0.003	
15%	1,500	0.003	
20%	2,000	0.005	
25%	2,500	0.006	
30%	3,000	0.007	
35%	3,500	0.008	
40%	4,000	0.009	
45%	4,500	0.009	
50%	5,000	0.010	
55%	5,500	0.011	
60%	6,000	0.012	
65%	6,500	0.013	
70%	7,000	0.014	
75%	7,500	0.015	
80%	8,000	0.016	
85%	8,500	0.017	
90%	9,000	0.018	
95%	9,500	0.020	
100%	10,000	0.021	
0%	0	0.005	



Compression Load Test Result for PLT-33C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

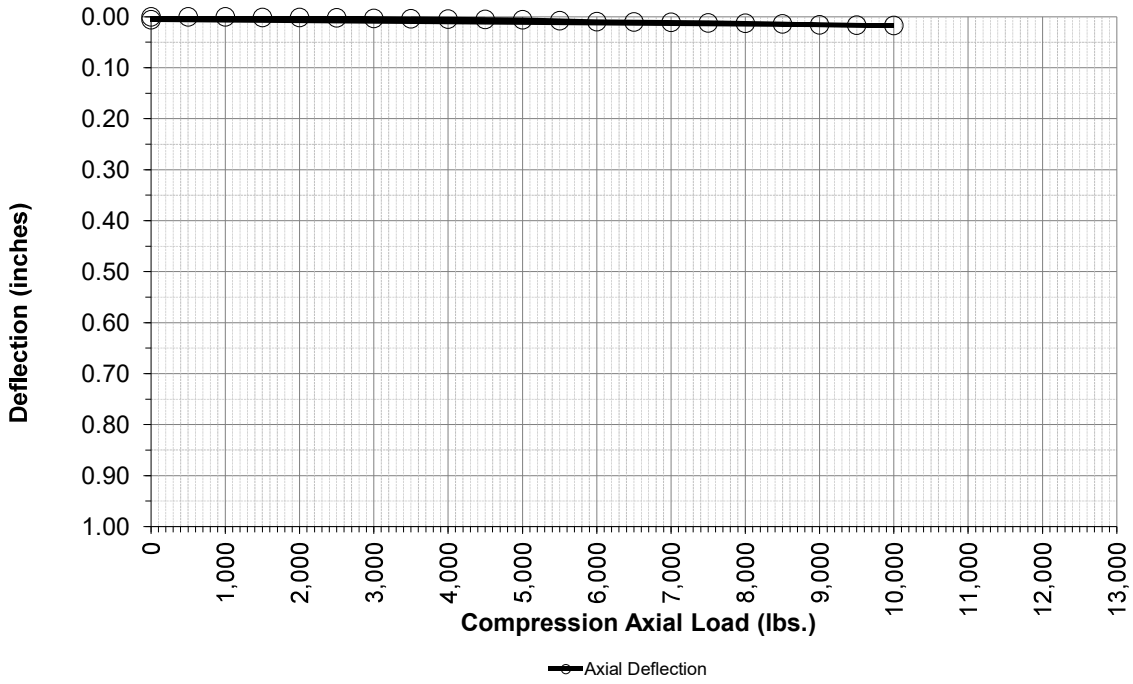
Tested By Terracon Rep:	AS/JB
Date Tested:	10/26/2023

Pile Information

Pile ID:	PLT-33C
Latitude [deg.]:	33.64912
Longitude [deg.]:	-114.71907
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	20

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.001	
20%	2,000	0.002	
25%	2,500	0.002	
30%	3,000	0.003	
35%	3,500	0.004	
40%	4,000	0.005	
45%	4,500	0.005	
50%	5,000	0.006	
55%	5,500	0.008	
60%	6,000	0.010	
65%	6,500	0.010	
70%	7,000	0.011	
75%	7,500	0.012	
80%	8,000	0.013	
85%	8,500	0.014	
90%	9,000	0.016	
95%	9,500	0.016	
100%	10,000	0.017	
0%	0	0.006	



Compression Load Test Result for PLT-34C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

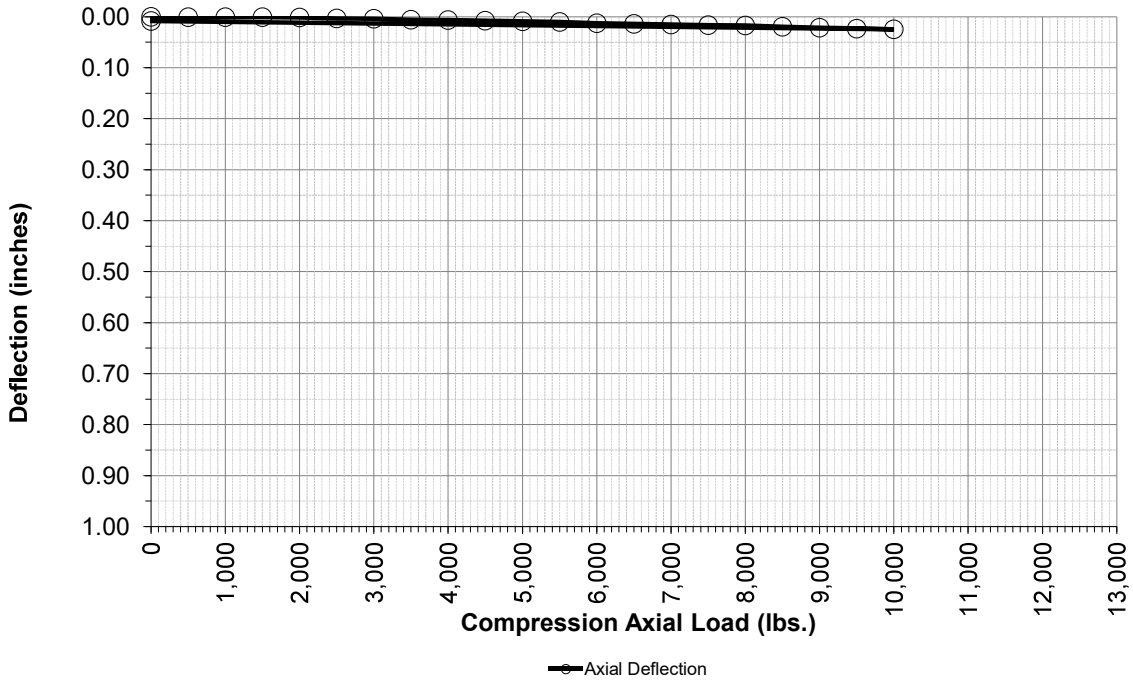
Tested By Terracon Rep:	AS/JB
Date Tested:	11/2/2023

Pile Information

Pile ID:	PLT-34C
Latitude [deg.]:	33.64113
Longitude [deg.]:	-114.71104
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	27

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.001	
10%	1,000	0.001	
15%	1,500	0.001	
20%	2,000	0.002	
25%	2,500	0.003	
30%	3,000	0.004	
35%	3,500	0.006	
40%	4,000	0.007	
45%	4,500	0.008	
50%	5,000	0.009	
55%	5,500	0.010	
60%	6,000	0.012	
65%	6,500	0.014	
70%	7,000	0.015	
75%	7,500	0.017	
80%	8,000	0.017	
85%	8,500	0.019	
90%	9,000	0.022	
95%	9,500	0.023	
100%	10,000	0.024	
0%	0	0.008	



Compression Load Test Result for PLT-35C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, LA
Project Number:	60225164

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Dillon ED

Test Date and Representative

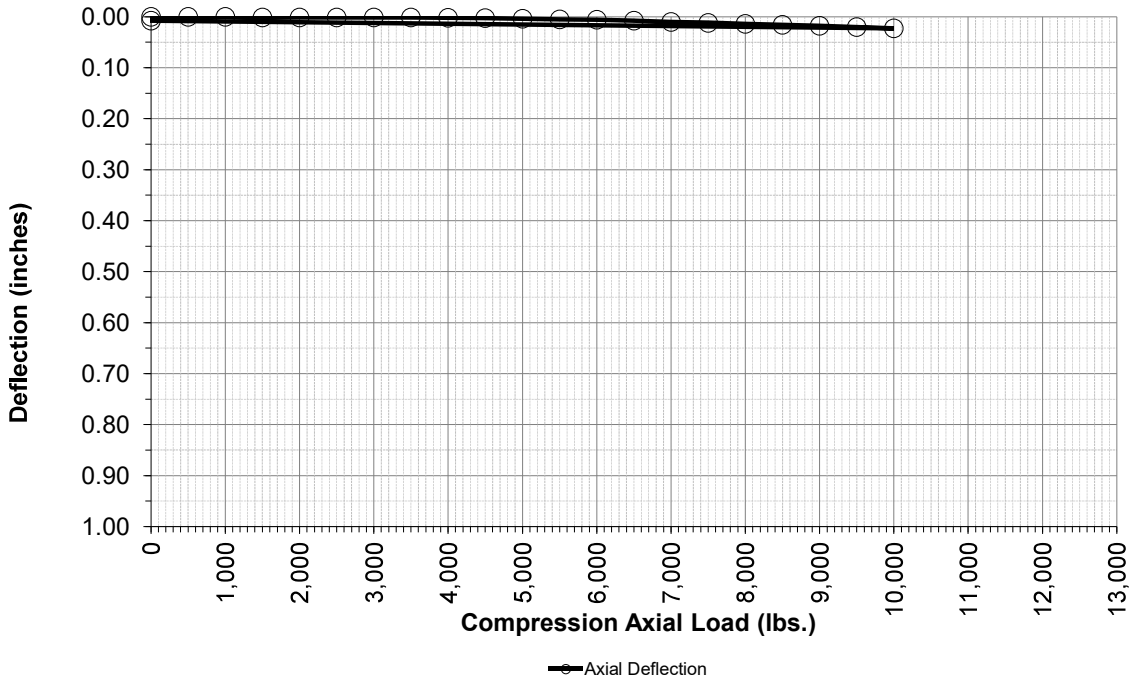
Tested By Terracon Rep:	AS/JB
Date Tested:	11/1/2023

Pile Information

Pile ID:	PLT-35C
Latitude [deg.]:	33.66613
Longitude [deg.]:	-114.70942
Pile Type:	W6x9
Pile Embedment Depth [in.]:	84
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	48
Axial Design Load [lbs.]:	10,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	19

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
5%	500	0.000	
10%	1,000	0.000	
15%	1,500	0.001	
20%	2,000	0.001	
25%	2,500	0.001	
30%	3,000	0.001	
35%	3,500	0.001	
40%	4,000	0.002	
45%	4,500	0.003	
50%	5,000	0.004	
55%	5,500	0.005	
60%	6,000	0.006	
65%	6,500	0.008	
70%	7,000	0.010	
75%	7,500	0.012	
80%	8,000	0.014	
85%	8,500	0.016	
90%	9,000	0.018	
95%	9,500	0.021	
100%	10,000	0.023	
0%	0	0.008	



Supporting Information

Contents:

Gamechange Solar – Cupertino – Grace – Solar Project: Pile Load Testing Report, Blythe, CA prepared by ANS Geo dated November 7, 2025 (377 pages)

November 7, 2025

Austin Pruitt, PE
Director of Geotechnical Engineering
Gamechange Solar
230 East Avenue Norwalk, CT 06855
Via e-mail: austin.pruitt@gamechangesolar.com

Gamechange Solar – Cupertino – Grace - Solar Project: Pile Load Testing Report
Blythe, CA

Dear Austin:

ANS was retained by Gamechange Solar (Gamechange) to conduct a pile load testing program in support of the Cupertino – Grace Solar project in Blythe, CA. The purpose of this Pile Load Testing Report (Report) is to provide data collected from the program to Gamechange for evaluation of their system. This Report summarizes the information gathered during our pile load testing program conducted in October of 2025.

A Location Plan identifying our testing locations is provided as **Attachment A**. The results of the pile testing program have been recorded using the template provided by Gamechange and presented as **Attachment B**.

ANS followed the *Pull Test Procedure* (Procedure) dated September 23rd, 2025 provided by the Gamechange team to complete the pile load testing program at the accessible locations. As part of this work, the following activities were completed:

- Pile installation was carried out in which 180 piles were installed across the site in accordance with Gamechange's provided *Procedure*.
- The full scope included 76 load test locations:
    - All pile locations included two (2) test piles ("A" and "B") with 28 select locations including a third (3) test pile ("C" or "D").
        - "A" Piles were tested for uplift and lateral loadings
        - "B" Piles were tested for uplift and lateral loadings.
        - "C" Piles were installed at 12 select locations and tested for compression loading only.
        - "D" Piles were installed at 16 select locations and tested for uplift and lateral loadings.
- Each "A", "B", and "C" pile was W6x9 in shape and embedded between five (5) to eight (8) feet below grade per Gamechange's *Procedure.*
- Each "D" pile was W6x12 in shape and embedded to five (5) feet below grade per Gamechange's *Procedure.*
- Piles were each installed, then tested by ANS following at least a 72-hour wait period.
- At the conclusion of the program, all 180 piles were extracted and removed from site.

1 Methodology

1.1 Test Pile Installations

ANS conducted pile load testing of W6x9 and W6x12 galvanized steel sections across the proposed solar array areas in October of 2025. A total of 76 test locations were considered. At all testing locations, two (2) test piles ("A" and "B") were installed. At 28 select locations, a third (3) test pile was installed ("C" or "D"). A total of 12 test locations included "C" piles, and 16 test locations included "D" piles

All "A" and "B" piles were W6x9 in shape and were tested for both uplift and lateral testing. All "C" piles were W6x9 in shape and were tested for compression loading only. All "D" piles were W6x12 in shape and were tested for both uplift and lateral testing. Each test location was installed to depths ranging between five (5) and eight (8) feet below grade per Gamechange's *Procedure*. Of the 180 total piles installed, early refusal was not encountered at any location during installation.

Once installed, the piles completed a 72-hour "setup" period prior to being tested for uplift, lateral, and compression loading. The load testing program was overseen and logged by our representative under the direction of a Professional Engineer licensed in the State of California. A photo log showing the general testing setup and conditions is provided as **Attachment C.**

1.2 Uplift Load Testing

Once installed to the targeted embedment depth, an uplift load test was performed on each test pile in accordance with the ASTM D3689 (uplift) test method. The tension load was applied using an excavator or telehandler which applied the load concentrically to the pile using a chain as well as a "pacman" clamp attachment to secure the assembly to the pile.

Uplift tests were performed on each "A", "B" and "D" pile. Tension loads were applied in two-minute, targeted load increments specified by Gamechange. Loads were applied until the maximum target load was reached or until a maximum vertical displacement of approximately 0.9 inch (pile failure criteria) was achieved, and the final movement was recorded. Following maximum load or pile failure, the load was released, and the residual deflection was recorded. A photograph generally depicting the uplift test arrangement is provided as **Figure 1**.

Figure 1 – Typical Uplift Test

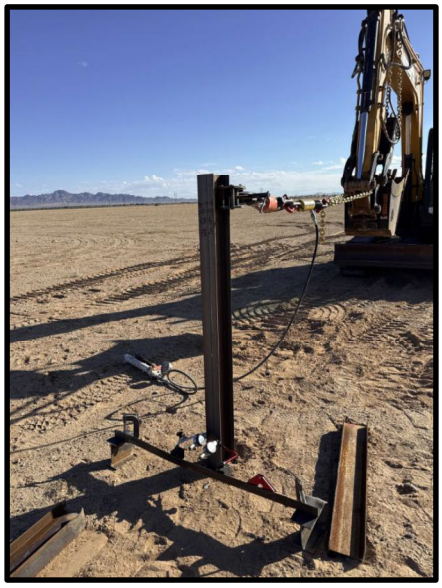


1.3 Lateral Load Testing

A lateral load test was performed on each “A”, “B”, and “D” pile following the uplift load test in accordance with ASTM D3966 (lateral) test method. Each test load was applied in two-minute, targeted load increments specified by Gamechange. Per the Gamechange *Procedure*, lateral loads were applied at 60 inches above grade.

Loads were applied until a maximum lateral deflection of approximately 1.0-inch (pile failure criteria) was achieved. If the test did not achieve 1.0-inch of lateral deflection with the maximum target load specified, the load was increased until the failure criteria was reached with the subsequent load recorded. Equipment maximum capacity, pile bending, and/or pile twisting were the only reasons for which 1.0-inch deflection was not achieved. After achieving the maximum lateral deflection, the load was released, and the residual deflection was recorded. A photograph generally depicting the lateral test arrangement is provided as **Figure 2**.

Figure 2 – Typical Lateral Test



1.4 Compression Load Testing

A compression load test was performed on each “C” pile in accordance with ASTM D1143 (compression) test method. Each test load was applied in two-minute, targeted load increments specified by Gamechange. Loads were applied until the maximum specified target load was reached or until a maximum compression deflection of approximately 0.9 inch (pile failure criteria) was achieved, after which, the load was released, and the residual deflection was recorded. A photograph generally depicting the compression test arrangement is provided as **Figure 3**.

Figure 3 – Typical Compression Test



At the conclusion of the program, all 180 piles were extracted.

2 Results

The results from the pile load testing program are provided as **Attachment B**; however, a summary of the recorded data for the uplift, lateral, and compression testing is provided in **Table 1.**

Table 1 – Summarized Table of Maximum Loads and Corresponding Movements

PT-ID	Embedment Depth (ft.)	Max Uplift (lbs.)	Uplift Displacement (in.)	Max Compression (lbs.)	Compression Displacement (in.)	Max Lateral (lbs.)	Lateral Displacement (in.)
PT-1A	5.0	6,445	1.01	N/A	N/A	6,997	1.13
PT-1B	7.0	7,979	0.02	N/A	N/A	6,979	1.06
PT-2A	6.0	8,037	0.14	N/A	N/A	7,237	1.14
PT-2B	8.0	8,001	0.05	N/A	N/A	7,473	1.00
PT-2D	5.0	8,025	0.08	N/A	N/A	6,400	1.09
PT-3A	5.0	7,995	0.29	N/A	N/A	6,679	1.05
PT-3B	7.0	7,956	0.17	N/A	N/A	6,957	1.10
PT-4A	6.0	8,020	0.28	N/A	N/A	6,485	1.07
PT-4B	8.0	8,017	0.04	N/A	N/A	6,405	1.11
PT-5A	5.0	8,076	0.26	N/A	N/A	7,245	1.01
PT-5B	7.0	7,982	0.31	N/A	N/A	7,040	1.11
PT-5C	5.0	N/A	N/A	15000	0.06	N/A	N/A
PT-6A	6.0	8,017	0.16	N/A	N/A	8,205	1.39
PT-6B	8.0	8,046	0.13	N/A	N/A	7,295	1.05
PT-6D	5.0	8,010	0.33	N/A	N/A	6,700	1.12
PT-7A	5.0	8,031	0.27	N/A	N/A	6,227	1.21
PT-7B	7.0	8,032	0.02	N/A	N/A	6,918	1.12
PT-7C	5.0	N/A	N/A	9500	0.92	N/A	N/A
PT-8A	6.0	8,020	0.07	N/A	N/A	7,643	1.06
PT-8B	8.0	7,964	0.01	N/A	N/A	8,609	1.09
PT-9A	5.0	7,987	0.47	N/A	N/A	6,943	1.09
PT-9B	7.0	8,009	0.27	N/A	N/A	6,147	1.10
PT-10A	6.0	8,070	0.10	N/A	N/A	7,219	1.05
PT-10B	8.0	8,037	0.02	N/A	N/A	7,187	1.12
PT-11A	5.0	7,975	0.32	N/A	N/A	6,857	1.03
PT-11B	7.0	8,027	0.07	N/A	N/A	6,976	1.07
PT-12A	6.0	8,035	0.10	N/A	N/A	6,987	1.32
PT-12B	8.0	8,004	0.06	N/A	N/A	7,240	1.04
PT-12D	5.0	7,995	0.17	N/A	N/A	6,200	1.08
PT-13A	5.0	8,100	0.07	N/A	N/A	5,850	1.10
PT-13B	7.0	8,543	0.17	N/A	N/A	7,280	1.05
PT-14A	6.0	7,980	0.18	N/A	N/A	5,900	1.14
PT-14B	8.0	7,975	0.08	N/A	N/A	6,500	0.98
PT-15A	5.0	8,010	0.36	N/A	N/A	5,590	1.04
PT-15B	7.0	8,025	0.18	N/A	N/A	6,170	0.92
PT-15C	5.0	N/A	N/A	15150	0.15	N/A	N/A

PT-ID	Embedment Depth (ft.)	Max Uplift (lbs.)	Uplift Displacement (in.)	Max Compression (lbs.)	Compression Displacement (in.)	Max Lateral (Lbs.)	Lateral Displacement (in.)
PT-16A	6.0	5,910	0.92	N/A	N/A	5,300	1.07
PT-16B	8.0	8,040	0.18	N/A	N/A	4,930	1.06
PT-16D	5.0	6,460	1.06	N/A	N/A	5,440	1.02
PT-17A	5.0	4,790	0.96	N/A	N/A	7,260	1.07
PT-17B	7.0	8,020	0.23	N/A	N/A	7,480	1.12
PT-17C	5.0	N/A	N/A	15000	0.03	N/A	N/A
PT-18A	6.0	8,020	0.27	N/A	N/A	7,180	1.04
PT-18B	8.0	8,030	0.12	N/A	N/A	7,430	1.05
PT-19A	5.0	8,100	0.09	N/A	N/A	6,020	1.20
PT-19B	7.0	8,010	0.26	N/A	N/A	5,880	1.05
PT-20A	6.0	8,030	0.06	N/A	N/A	6,780	1.06
PT-20B	8.0	8,040	0.10	N/A	N/A	6,210	1.07
PT-21A	5.0	8,030	0.64	N/A	N/A	4,890	1.06
PT-21B	7.0	8,020	0.24	N/A	N/A	5,570	1.01
PT-21C	5.0	N/A	N/A	15060	0.16	N/A	N/A
PT-22A	6.0	8,031	0.03	N/A	N/A	4,598	0.43
PT-22B	8.0	8,000	0.02	N/A	N/A	5,350	0.92
PT-23A	5.0	7,880	0.04	N/A	N/A	5,830	1.06
PT-23B	7.0	8,013	0.01	N/A	N/A	5,840	1.09
PT-24A	6.0	7,980	0.37	N/A	N/A	6,507	1.09
PT-24B	8.0	7,970	0.04	N/A	N/A	7,263	1.02
PT-25A	5.0	8,020	0.21	N/A	N/A	6,800	1.06
PT-25B	7.0	8,000	0.05	N/A	N/A	7,180	1.12
PT-26A	6.0	8,030	0.10	N/A	N/A	7,280	1.08
PT-26B	8.0	8,020	0.04	N/A	N/A	7,240	1.11
PT-26D	5.0	8,030	0.25	N/A	N/A	6,630	1.21
PT-27A	5.0	8,040	0.09	N/A	N/A	5,940	1.08
PT-27B	7.0	8,020	0.09	N/A	N/A	6,210	0.98
PT-28A	6.0	8,060	0.12	N/A	N/A	6,040	1.04
PT-28B	8.0	8,040	0.05	N/A	N/A	6,020	1.11
PT-29A	5.0	7,990	0.08	N/A	N/A	5,490	1.09
PT-29B	7.0	8,020	0.05	N/A	N/A	5,380	1.04
PT-30A	6.0	8,025	0.28	N/A	N/A	6,070	1.06
PT-30B	8.0	8,040	0.17	N/A	N/A	6,000	0.96
PT-30D	5.0	8,035	0.31	N/A	N/A	6,220	1.03
PT-31A	5.0	8,025	0.32	N/A	N/A	6,440	1.06
PT-31B	7.0	7,990	0.09	N/A	N/A	5,960	1.02
PT-31C	5.0	N/A	N/A	14900	0.06	N/A	N/A
PT-32A	6.0	8,100	0.11	N/A	N/A	5,200	1.06
PT-32B	8.0	8,100	0.03	N/A	N/A	6,310	0.96
PT-33A	5.0	8,060	0.43	N/A	N/A	5,640	1.08

PT-ID	Embedment Depth (ft.)	Max Uplift (lbs.)	Uplift Displacement (in.)	Max Compression (lbs.)	Compression Displacement (in.)	Max Lateral (Lbs.)	Lateral Displacement (in.)
PT-33B	7.0	8,090	0.19	N/A	N/A	6,480	1.08
PT-34A	6.0	8,080	0.12	N/A	N/A	5,960	1.08
PT-34B	8.0	8,020	0.06	N/A	N/A	6,530	1.22
PT-34D	5.0	8,040	0.16	N/A	N/A	6,655	1.07
PT-35A	5.0	8,000	0.10	N/A	N/A	6,300	1.12
PT-35B	7.0	8,040	0.08	N/A	N/A	5,780	1.07
PT-36A	6.0	8,020	0.07	N/A	N/A	6,380	1.37
PT-36B	8.0	8,000	0.05	N/A	N/A	7,020	1.11
PT-37A	5.0	8,100	0.05	N/A	N/A	5,690	1.08
PT-37B	7.0	8,090	0.10	N/A	N/A	5,180	1.10
PT-38A	6.0	7,995	0.21	N/A	N/A	6,040	1.05
PT-38B	8.0	7,990	0.02	N/A	N/A	6,130	1.01
PT-38D	5.0	8,030	0.17	N/A	N/A	5,845	1.00
PT-39A	5.0	6,005	1.22	N/A	N/A	4,490	1.02
PT-39B	7.0	8,035	0.16	N/A	N/A	5,010	0.91
PT-39C	5.0	N/A	N/A	15000	0.04	N/A	N/A
PT-40A	6.0	8,040	0.19	N/A	N/A	6,500	1.05
PT-40B	8.0	7,990	0.04	N/A	N/A	6,510	1.11
PT-41A	5.0	8,040	0.10	N/A	N/A	6,380	1.07
PT-41B	7.0	8,000	0.10	N/A	N/A	6,580	1.06
PT-42A	6.0	8,040	0.13	N/A	N/A	6,530	1.05
PT-42B	8.0	8,060	0.11	N/A	N/A	7,280	1.07
PT-43A	5.0	8,090	0.21	N/A	N/A	7,190	1.04
PT-43B	7.0	8,030	0.06	N/A	N/A	6,140	0.89
PT-44A	6.0	8,040	0.05	N/A	N/A	7,340	1.12
PT-44B	8.0	8,030	0.02	N/A	N/A	6,600	0.94
PT-44D	5.0	8,040	0.03	N/A	N/A	6,540	1.27
PT-45A	5.0	8,080	0.17	N/A	N/A	6,280	1.04
PT-45B	7.0	8,010	0.10	N/A	N/A	5,780	1.04
PT-46A	6.0	8,030	0.37	N/A	N/A	4,570	1.13
PT-46B	8.0	8,040	0.25	N/A	N/A	3,830	1.03
PT-47A	5.0	8,040	0.28	N/A	N/A	5,290	1.00
PT-47B	7.0	7,990	0.14	N/A	N/A	6,270	1.00
PT-48A	6.0	8,005	0.11	N/A	N/A	6,230	1.02
PT-48B	8.0	8,035	0.05	N/A	N/A	6,150	1.05
PT-48D	5.0	8,010	0.17	N/A	N/A	5,300	1.07
PT-49A	5.0	8,000	0.51	N/A	N/A	6,280	1.03
PT-49B	7.0	8,000	0.40	N/A	N/A	6,280	1.15
PT-50A	6.0	8,030	0.25	N/A	N/A	5,540	1.13
PT-50B	8.0	8,030	0.18	N/A	N/A	5,940	1.08

PT-ID	Embedment Depth (ft.)	Max Uplift (lbs.)	Uplift Displacement (in.)	Max Compression (lbs.)	Compression Displacement (in.)	Max Lateral (Lbs.)	Lateral Displacement (in.)
PT-51A	5.0	8,090	0.66	N/A	N/A	5,030	1.03
PT-51B	7.0	8,010	0.19	N/A	N/A	4,960	1.11
PT-51C	5.0	N/A	N/A	15000	0.27	N/A	N/A
PT-52A	6.0	8,000	0.12	N/A	N/A	6,330	1.08
PT-52B	8.0	8,040	0.16	N/A	N/A	6,760	1.05
PT-53A	5.0	8,025	0.63	N/A	N/A	6,230	1.10
PT-53B	7.0	8,035	0.18	N/A	N/A	5,020	1.02
PT-54A	6.0	8,040	0.14	N/A	N/A	6,840	1.06
PT-54B	8.0	8,040	0.05	N/A	N/A	7,520	1.14
PT-54D	5.0	8,010	0.20	N/A	N/A	6,480	1.04
PT-55A	5.0	8,060	0.26	N/A	N/A	6,660	1.04
PT-55B	7.0	8,030	0.10	N/A	N/A	7,190	1.11
PT-55C	5.0	N/A	N/A	15000	0.04	N/A	N/A
PT-56A	6.0	7,980	0.02	N/A	N/A	6,780	1.03
PT-56B	8.0	8,030	0.05	N/A	N/A	6,140	1.08
PT-57A	5.0	7,985	0.24	N/A	N/A	4,980	1.01
PT-57B	7.0	8,030	0.12	N/A	N/A	4,940	1.04
PT-58A	6.0	8,015	0.10	N/A	N/A	5,280	1.00
PT-58B	8.0	7,985	0.02	N/A	N/A	5,130	1.02
PT-59A	5.0	8,000	0.07	N/A	N/A	6,380	1.07
PT-59B	7.0	8,030	0.09	N/A	N/A	6,330	1.04
PT-59C	5.0	N/A	N/A	15000	0.02	N/A	N/A
PT-60A	6.0	8,030	0.03	N/A	N/A	6,230	1.07
PT-60B	8.0	8,030	0.02	N/A	N/A	6,530	1.02
PT-60D	5.0	8,030	0.01	N/A	N/A	6,980	1.10
PT-61A	5.0	8,020	0.73	N/A	N/A	4,450	1.04
PT-61B	7.0	7,975	0.09	N/A	N/A	4,925	1.03
PT-62A	6.0	8,025	0.18	N/A	N/A	5,580	1.02
PT-62B	8.0	8,020	0.08	N/A	N/A	5,700	0.96
PT-62D	5.0	8,020	0.27	N/A	N/A	6,210	1.12
PT-63A	5.0	8,040	0.14	N/A	N/A	4,950	1.12
PT-63B	7.0	8,030	0.02	N/A	N/A	5,980	1.07
PT-64A	6.0	8,000	0.03	N/A	N/A	7,340	1.01
PT-64B	8.0	8,030	0.04	N/A	N/A	7,140	1.07
PT-65A	5.0	8,000	0.12	N/A	N/A	6,220	1.02
PT-65B	7.0	8,020	0.20	N/A	N/A	6,530	1.05
PT-66A	6.0	8,030	0.04	N/A	N/A	6,940	1.11
PT-66B	8.0	8,040	0.02	N/A	N/A	5,580	1.19
PT-67A	5.0	8,030	0.02	N/A	N/A	6,080	1.05
PT-67B	7.0	8,000	0.01	N/A	N/A	6,530	1.06
PT-67C	5.0	N/A	N/A	15000	0.03	N/A	N/A

PT-ID	Embedment Depth (ft.)	Max Uplift (lbs.)	Uplift Displacement (in.)	Max Compression (lbs.)	Compression Displacement (in.)	Max Lateral (Lbs.)	Lateral Displacement (in.)
PT-68A	6.0	8,030	0.03	N/A	N/A	6,480	1.01
PT-68B	8.0	8,020	0.06	N/A	N/A	6,480	1.10
PT-69A	5.0	4,280	0.92	N/A	N/A	3,655	1.07
PT-69B	7.0	8,025	0.03	N/A	N/A	4,600	1.07
PT-69C	5.0	N/A	N/A	15000	0.04	N/A	N/A
PT-70A	6.0	8,030	0.41	N/A	N/A	5,680	1.06
PT-70B	8.0	8,030	0.04	N/A	N/A	5,450	1.04
PT-70D	5.0	8,030	0.18	N/A	N/A	5,870	1.19
PT-71A	5.0	8,000	0.09	N/A	N/A	6,230	1.02
PT-71B	7.0	8,030	0.05	N/A	N/A	5,980	1.08
PT-72A	6.0	8,030	0.03	N/A	N/A	5,480	0.95
PT-72B	8.0	8,020	0.01	N/A	N/A	6,380	0.82
PT-72D	5.0	8,000	0.09	N/A	N/A	6,480	1.16
PT-73A	5.0	8,000	0.22	N/A	N/A	6,060	1.05
PT-73B	7.0	8,000	0.23	N/A	N/A	4,380	1.08
PT-74A	6.0	8,030	0.04	N/A	N/A	5,850	1.04
PT-74B	8.0	8,030	0.01	N/A	N/A	5,820	1.00
PT-75A	5.0	8,040	0.42	N/A	N/A	3,840	1.08
PT-75B	7.0	8,000	0.07	N/A	N/A	4,000	1.07
PT-76A	6.0	8,000	0.03	N/A	N/A	5,770	1.01
PT-76B	8.0	8,030	0.03	N/A	N/A	4,360	1.10
PT-76D	5.0	8,030	0.09	N/A	N/A	6,480	1.11

Purple text denotes W6x12 section “D” piles.

Red text denotes tests which have exceeded the failure criteria (0.9-inch of displacement in uplift/compression tests and 1.0-inch for lateral tests).

Blue text denotes tests which experienced pile twisting and/or bending during testing, resulting in unavailable failure deflection data.

Limitations

It should be noted that the findings within this Pile Load Testing Report are based on our load testing program completed in October of 2025. As indicated in our proposal, we have only provided data collection; engineering services, expressed or implied, have not been provided for this project. This report has been prepared to summarize the information gathered during our field activities, as well as provide project-specific data to assist Gamechange with the design and construction of the planned solar facility.

We sincerely appreciate the opportunity to support this project, and please feel free to contact us should you have any questions regarding the findings of this Report.

Yours Truly,



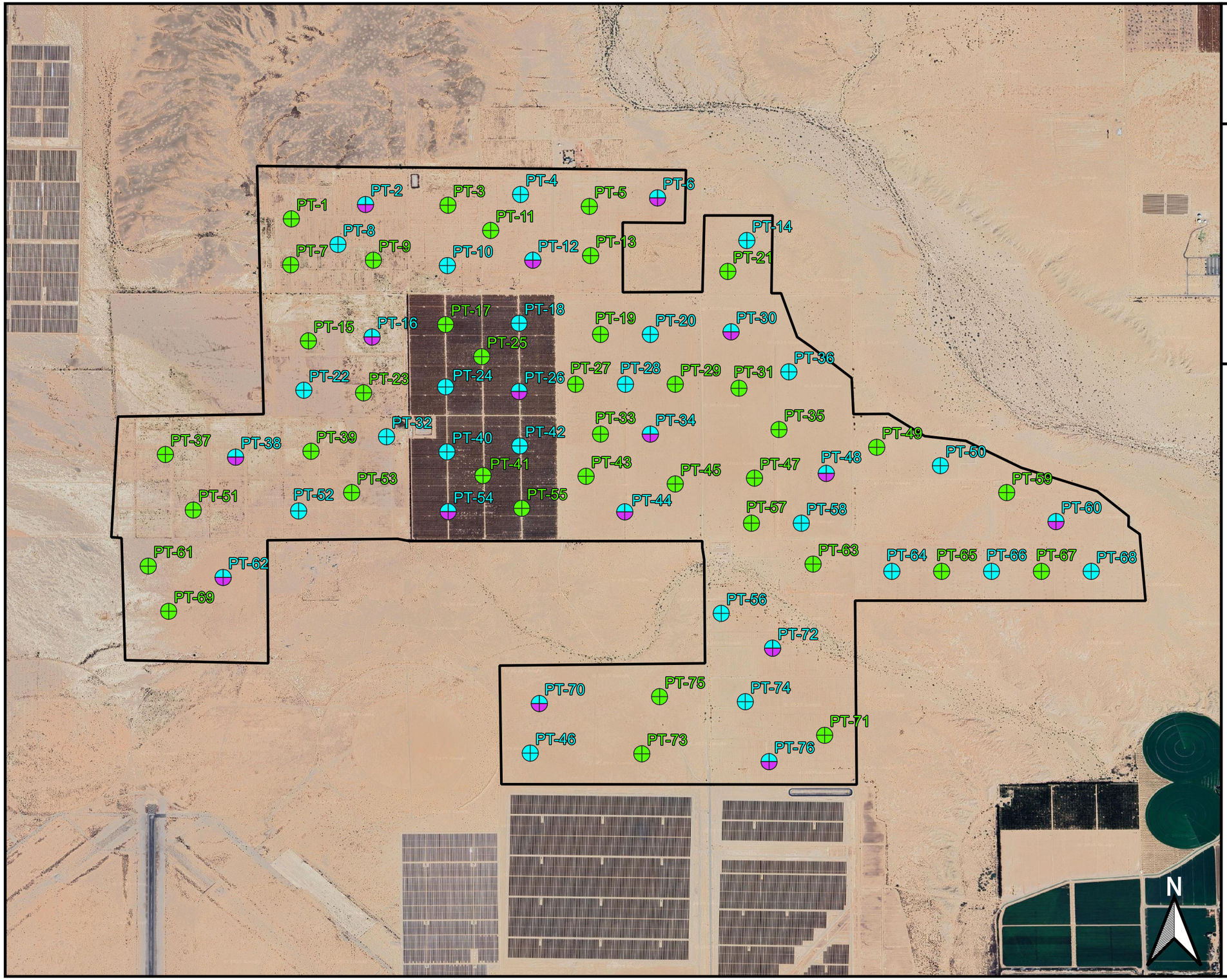
Vatsal A. Shah, PE, Ph.D, D.GE
Principal Engineer
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Vatsal.Shah@ansgeo.com

Attachments

Attachment A – Location Plan
Attachment B – Pile Load Testing Logs
Attachment C – Photo Logs
Attachment D – Calibration Certificates

Attachment A

Location Plan



ANS

Client:

GAMECHANGE SOLAR
REPOWERING THE PLANET

INVESTIGATION LOCATION PLAN

GAMECHANGE SOLAR
CUPERTINO - GRACE PROJECT
BLYTHE, CALIFORNIA

Legend

- Approximate Project Boundary
- PT Location - Category 1
- PT Location - Category 2
- PT Location - W6x12 "D" Pile

0 1,750 3,500 ft

Absolute Scale: 1 inch = 1,750 feet
Scale at 11" x 17" AS SHOWN

Prepared by: Kristen Kakascik
Date: November 6, 2025
Drawing Number: ILP-1 Rev.1

Attachment B

Pile Load Testing Logs

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-1A

Axial Uplift Load Test Results

PTL - **1**
Section: W6x9
Latitude: 33.665216
Longitude: -114.70852
Tested By: ANS
Test Date: 10/19/2025

Designation: **A**

Target Test Load (lb): **8,000** (≈ 67970% Design Load)
Installation Method: Direct Drive (DD)
- -
Target Embedment Depth (ft): 5.00
Actual Embedment Depth (ft): 5

Saturated Soil? No
Drive Time (s): 9.02
Pile Damaged? No

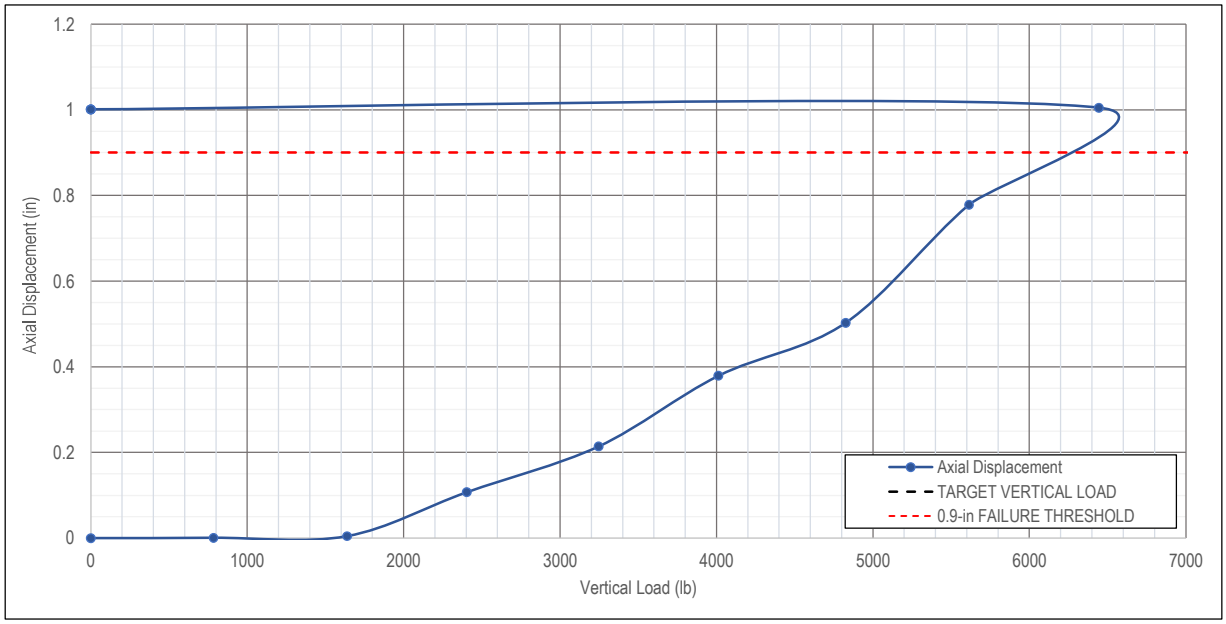
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	785	0.001	0.001	0.00
13594%	20%	1,600	1638	0.0055	0.004	0.00
20391%	30%	2,400	2403	0.108	0.1065	0.11
27188%	40%	3,200	3246	0.215	0.213	0.21
33985%	50%	4,000	4012	0.3805	0.3775	0.38
40782%	60%	4,800	4826	0.5065	0.4995	0.50
47579%	70%	5,600	5614	0.78	0.7765	0.78
54376%	80%	6,400	6445	1.0055	1.0045	1.01
61173%	90%	7,200				
67970%	100%	8,000				
0%	0%	0	0	1.0045	0.997	1.00

Load at 0.9-in axial displacement: **6,060** **lb**

Residual axial displacement: **1.00** **in**



Notes:

Failure reached at 6,445 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-1A

Lateral Load Test Results

PTL- **1** Designation: **A**

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Section: W6x9

Installation Method: Direct Drive (DD)

Saturated Soil? No

Latitude: 33.665216

- -

Drive Time (s): 9.02

Longitude: -114.70852

Target Embedment Depth (ft): 5.00

Pile Damaged? No

Tested By: ANS

Actual Embedment Depth (ft): 5

Gauge 1 Height (in): 6

Test Date: 10/19/2025

Lateral Load Application Height (ft): 5

Gauge 2 Height (in): 6

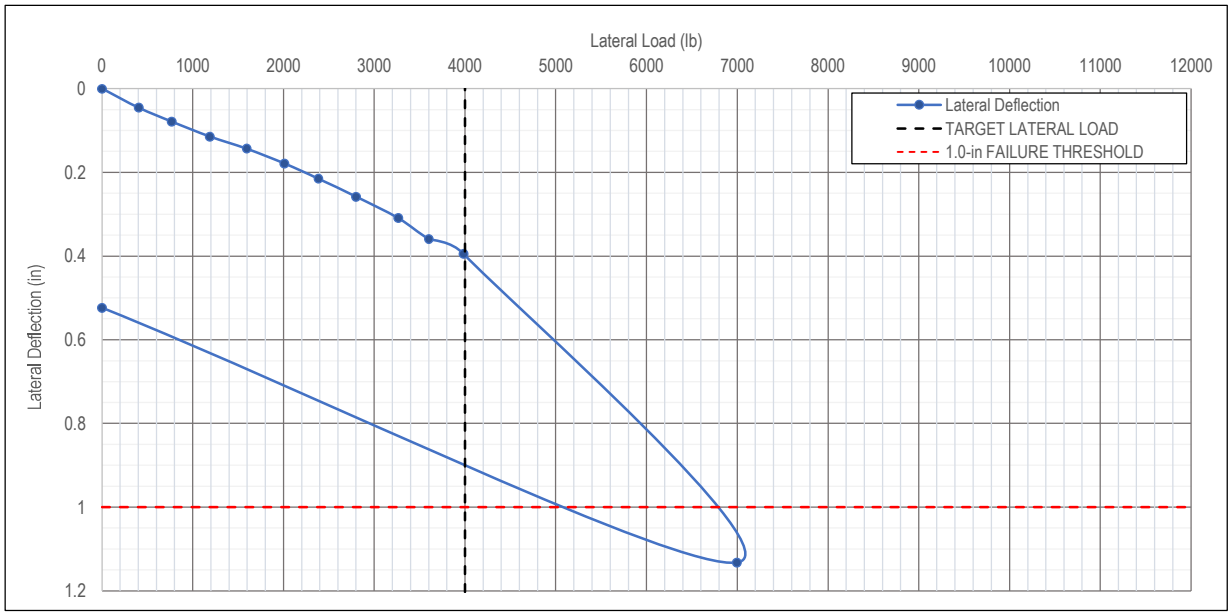
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	406	0.047	0.0435	0.05
62%	20%	800	767	0.0805	0.076	0.08
93%	30%	1,200	1189	0.116	0.113	0.11
124%	40%	1,600	1596	0.1445	0.142	0.14
155%	50%	2,000	2011	0.18	0.178	0.18
186%	60%	2,400	2385	0.216	0.214	0.22
217%	70%	2,800	2800	0.258	0.258	0.26
248%	80%	3,200	3265	0.3007	0.3175	0.31
279%	90%	3,600	3603	0.357	0.361	0.36
310%	100%	4,000	3985	0.392	0.3975	0.39
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6997	1.0525	1.212	1.13
0%	0%	0	0	0.4345	0.612	0.52

Load at 1.0-in lateral deflection: **6,457** **lb**

Residual lateral deformation: **0.52** **in**



Notes:

Failure reached at 6,997 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-1B

Axial Uplift Load Test Results

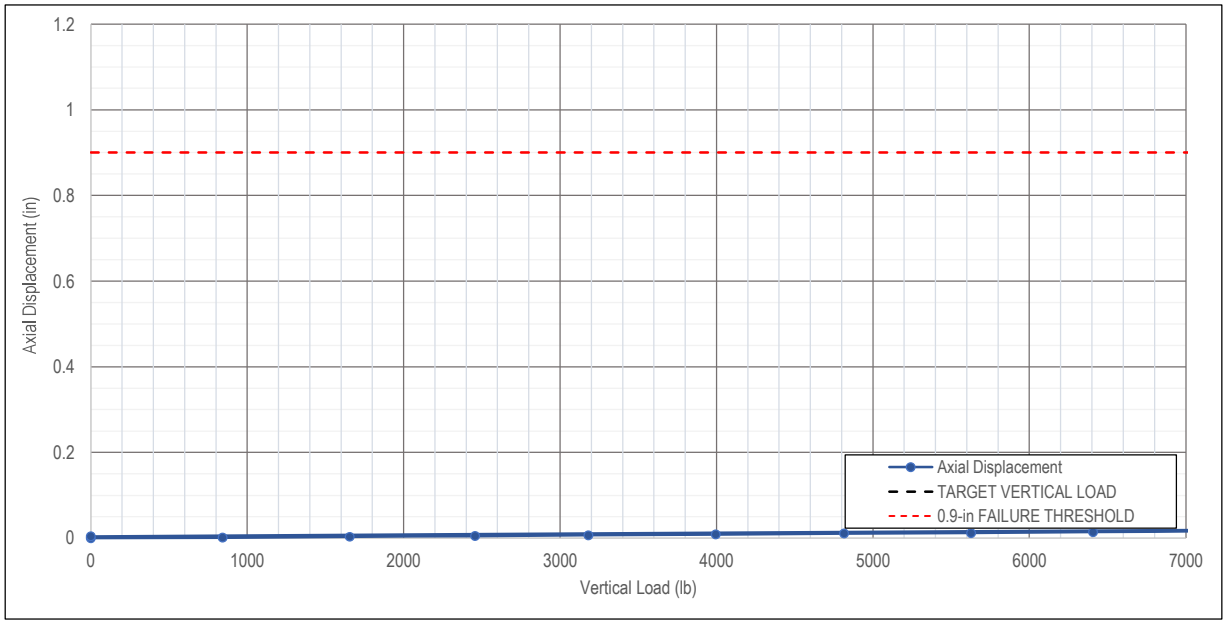
PTL - **1**
Designation: **B**
Target Test Load (lb): **8,000** (≈ 67970% Design Load)
Section: W6x9
Installation Method: Direct Drive (DD)
Latitude: 33.665216
Longitude: -114.70852
Tested By: ANS
Test Date: 10/19/2025
- -
Target Embedment Depth (ft): 7.00
Actual Embedment Depth (ft): 7
Saturated Soil? No
Drive Time (s): 27.1
Pile Damaged? No

Strong Axis Plumb. (deg):
Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	842	0.0015	0.001	0.00
13594%	20%	1,600	1655	0.0035	0.003	0.00
20391%	30%	2,400	2456	0.0055	0.0045	0.01
27188%	40%	3,200	3180	0.007	0.0065	0.01
33985%	50%	4,000	3995	0.009	0.0075	0.01
40782%	60%	4,800	4816	0.0115	0.01	0.01
47579%	70%	5,600	5627	0.013	0.011	0.01
54376%	80%	6,400	6408	0.016	0.0125	0.01
61173%	90%	7,200	7197	0.0175	0.015	0.02
67970%	100%	8,000	7979	0.022	0.019	0.02
0%	0%	0	0	0.004	0.0045	0.00

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.00** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-1B

Lateral Load Test Results

PTL- **1** Designation: **B**

Section: W6x9

Latitude: 33.665216

Longitude: -114.70852

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 27.1

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

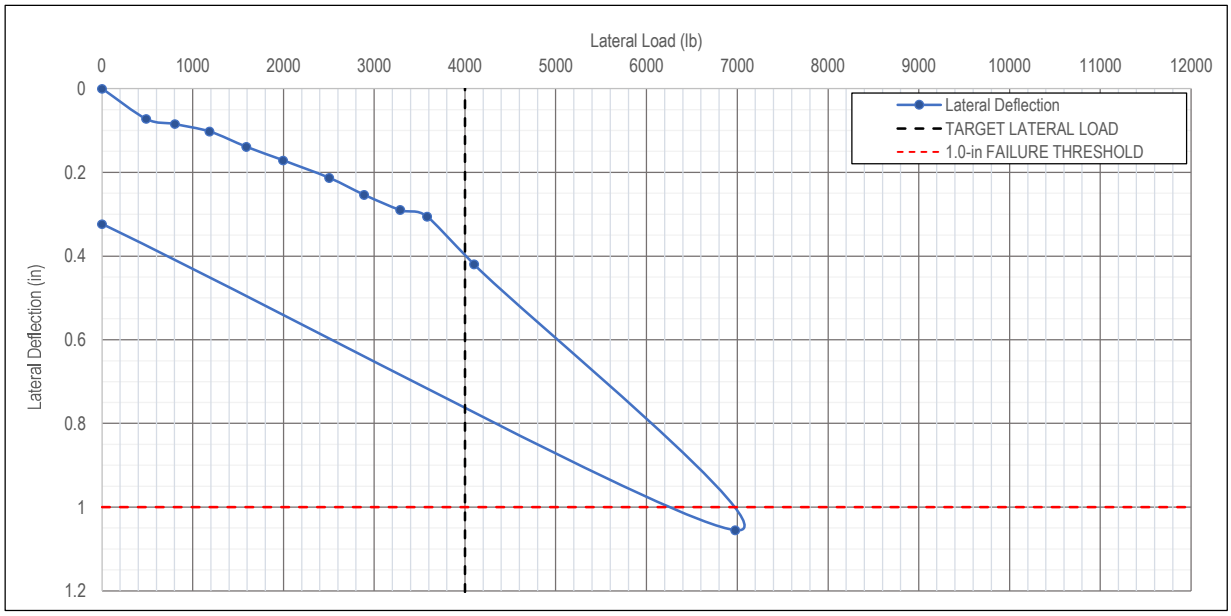
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	487	0.0685	0.076	0.07
62%	20%	800	805	0.0805	0.0885	0.08
93%	30%	1,200	1186	0.0985	0.1065	0.10
124%	40%	1,600	1592	0.129	0.148	0.14
155%	50%	2,000	1997	0.162	0.1805	0.17
186%	60%	2,400	2505	0.205	0.221	0.21
217%	70%	2,800	2890	0.245	0.2625	0.25
248%	80%	3,200	3287	0.2815	0.2985	0.29
279%	90%	3,600	3585	0.2975	0.314	0.31
310%	100%	4,000	4100	0.4175	0.422	0.42
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6979	1.065	1.0455	1.06
0%	0%	0	0	0.3235		0.32

Load at 1.0-in lateral deflection: **6,729** **lb**

Residual lateral deformation: **0.32** **in**



Notes:

Failure reached at 6,979 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-2A

Axial Uplift Load Test Results

PTL - **2** Designation: **A** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.666069 - - Saturated Soil? No

Longitude: -114.70414 Target Embedment Depth (ft): 6.00 Drive Time (s): 13.14

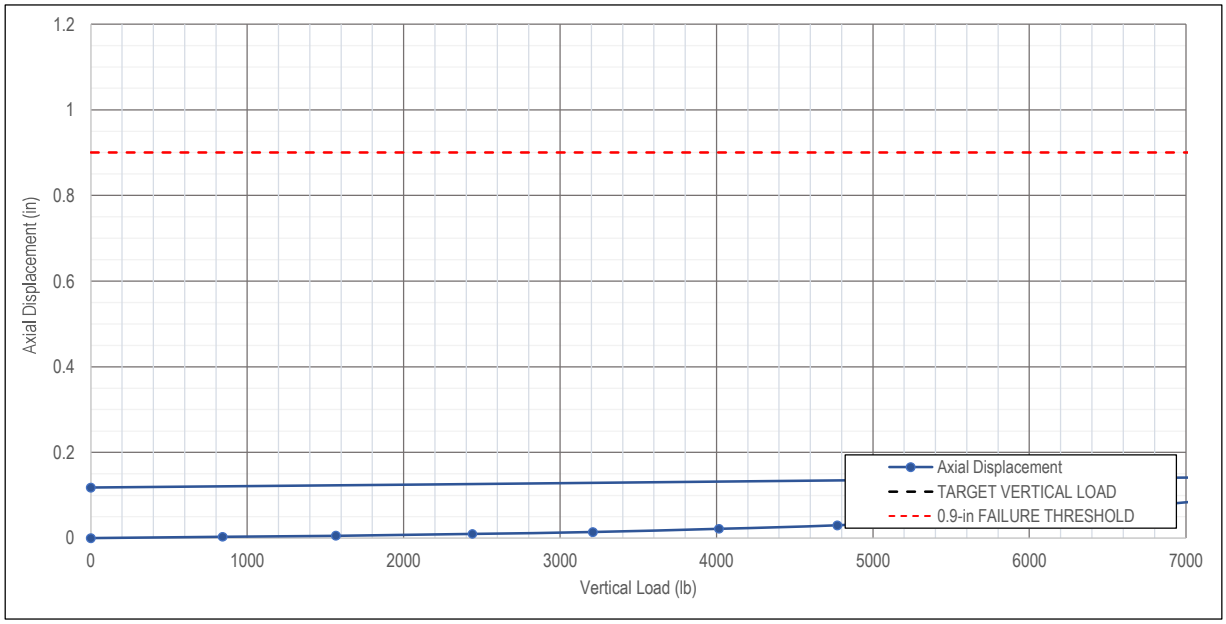
Tested By: ANS Actual Embedment Depth (ft): 6 Pile Damaged? No

Test Date: 10/19/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	842	0.003	0.003	0.00
436%	20%	1,600	1566	0.0055	0.0055	0.01
654%	30%	2,400	2439	0.01	0.0095	0.01
872%	40%	3,200	3210	0.0145	0.014	0.01
1090%	50%	4,000	4016	0.0225	0.021	0.02
1308%	60%	4,800	4772	0.031	0.029	0.03
1526%	70%	5,600	5592	0.0465	0.0445	0.05
1744%	80%	6,400	6460	0.0715	0.0685	0.07
1962%	90%	7,200	7180	0.0925	0.088	0.09
2180%	100%	8,000	8037	0.1435	0.1385	0.14
0%	0%	0	0	0.1175	0.1185	0.12

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.12** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-2A

Lateral Load Test Results

PTL- **2** Designation: **A**

Section: W6x9

Latitude: 33.666069

Longitude: -114.70414

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 13.14

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

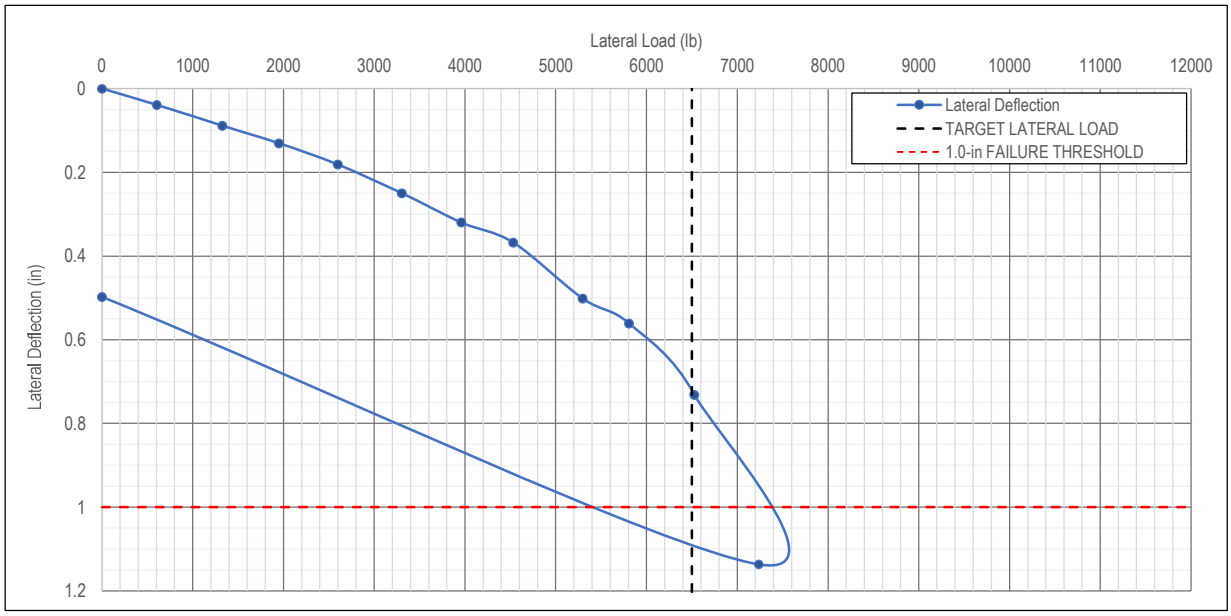
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	605	0.0375	0.04	0.04
62%	20%	1,300	1327	0.0865	0.0905	0.09
93%	30%	1,950	1950	0.1285	0.132	0.13
124%	40%	2,600	2597	0.18	0.1815	0.18
155%	50%	3,250	3305	0.2505	0.249	0.25
186%	60%	3,900	3960	0.3235	0.316	0.32
217%	70%	4,550	4531	0.3715	0.364	0.37
248%	80%	5,200	5297	0.51	0.493	0.50
279%	90%	5,850	5809	0.57	0.552	0.56
310%	100%	6,500	6527	0.751	0.712	0.73
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7237	1.2	1.073	1.14
0%	0%	0	0	0.549	0.4455	0.50

Load at 1.0-in lateral deflection: **6,998** **lb**

Residual lateral deformation: **0.50** **in**



Notes:

Failure reached at 7,237 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-2B

Axial Uplift Load Test Results

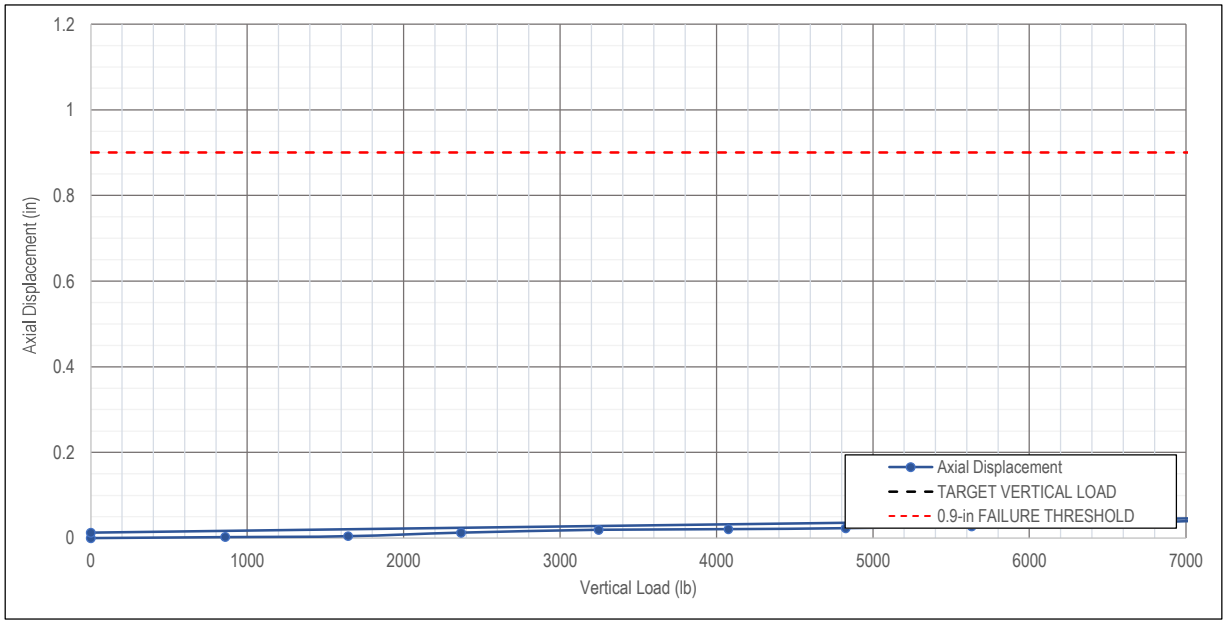
PTL - **2**
Designation: **B**
Target Test Load (lb): **8,000** (≈ 2180% Design Load)
Section: W6x9
Installation Method: Direct Drive (DD)
Latitude: 33.666069
- -
Saturated Soil? No
Longitude: -114.70414
Target Embedment Depth (ft): 8.00
Drive Time (s): 17.34
Tested By: ANS
Actual Embedment Depth (ft): 8
Pile Damaged? No
Test Date: 10/19/2025

Strong Axis Plumb. (deg):
Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	860	0.0025	0.003	0.00
436%	20%	1,600	1645	0.004	0.005	0.00
654%	30%	2,400	2368	0.012	0.014	0.01
872%	40%	3,200	3247	0.0185	0.02	0.02
1090%	50%	4,000	4076	0.0195	0.0225	0.02
1308%	60%	4,800	4826	0.022	0.0245	0.02
1526%	70%	5,600	5632	0.026	0.0295	0.03
1744%	80%	6,400	6399	0.032	0.035	0.03
1962%	90%	7,200	7226	0.0405	0.043	0.04
2180%	100%	8,000	8001	0.049	0.0516	0.05
0%	0%	0	0	0.0125	0.013	0.01

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-2B

Lateral Load Test Results

PTL- **2** Designation: **B**

Section: W6x9

Latitude: 33.666069

Longitude: -114.70414

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 17.34

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

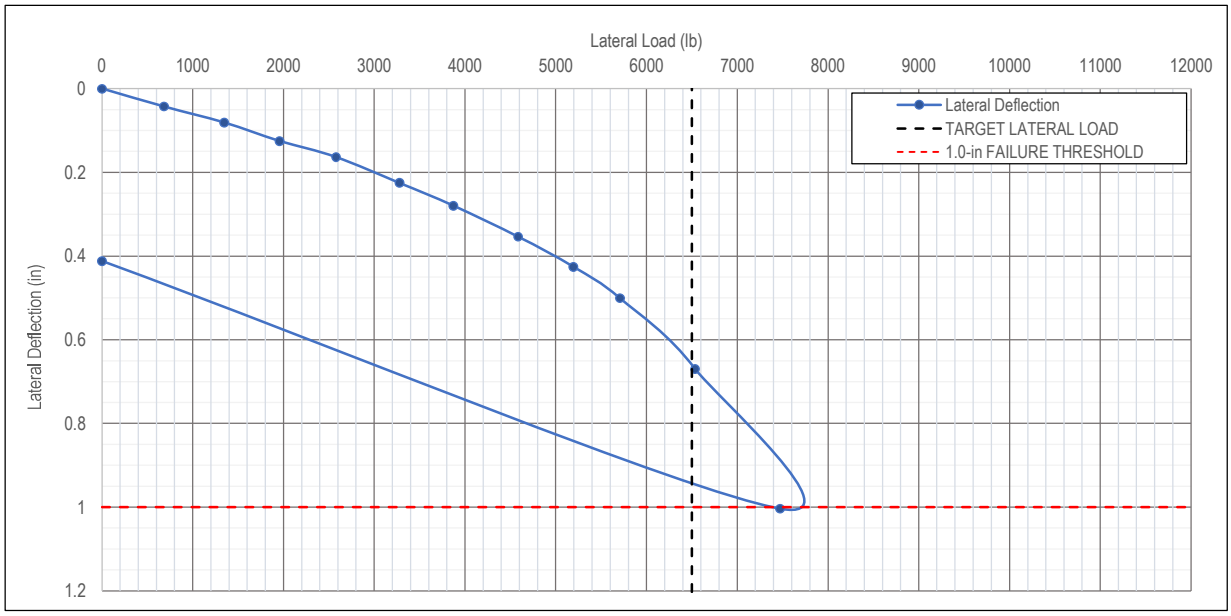
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	685	0.039	0.046	0.04
62%	20%	1,300	1347	0.0765	0.0855	0.08
93%	30%	1,950	1958	0.1205	0.129	0.12
124%	40%	2,600	2579	0.159	0.1675	0.16
155%	50%	3,250	3279	0.2215	0.2285	0.23
186%	60%	3,900	3872	0.279	0.28	0.28
217%	70%	4,550	4586	0.355	0.352	0.35
248%	80%	5,200	5195	0.428	0.423	0.43
279%	90%	5,850	5707	0.505	0.4955	0.50
310%	100%	6,500	6536	0.677	0.662	0.67
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7473	1.0065	1.0005	1.00
0%	0%	0	0	0.436	0.3875	0.41

Load at 1.0-in lateral deflection: **7,463** **lb**

Residual lateral deformation: **0.41** **in**



Notes:

Failure reached at 7,473 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-2D

Axial Uplift Load Test Results

PTL - **2**
Section: **W6x12**
Latitude: 33.666069
Longitude: -114.70414
Tested By: ANS
Test Date: 10/24/2025

Designation: **D**

Target Test Load (lb): **8,000** (≈ 2180% Design Load)
Installation Method: Direct Drive (DD)
- -
Target Embedment Depth (ft): 5.00
Actual Embedment Depth (ft): 5

Saturated Soil? No
Drive Time (s): 23.51
Pile Damaged? No

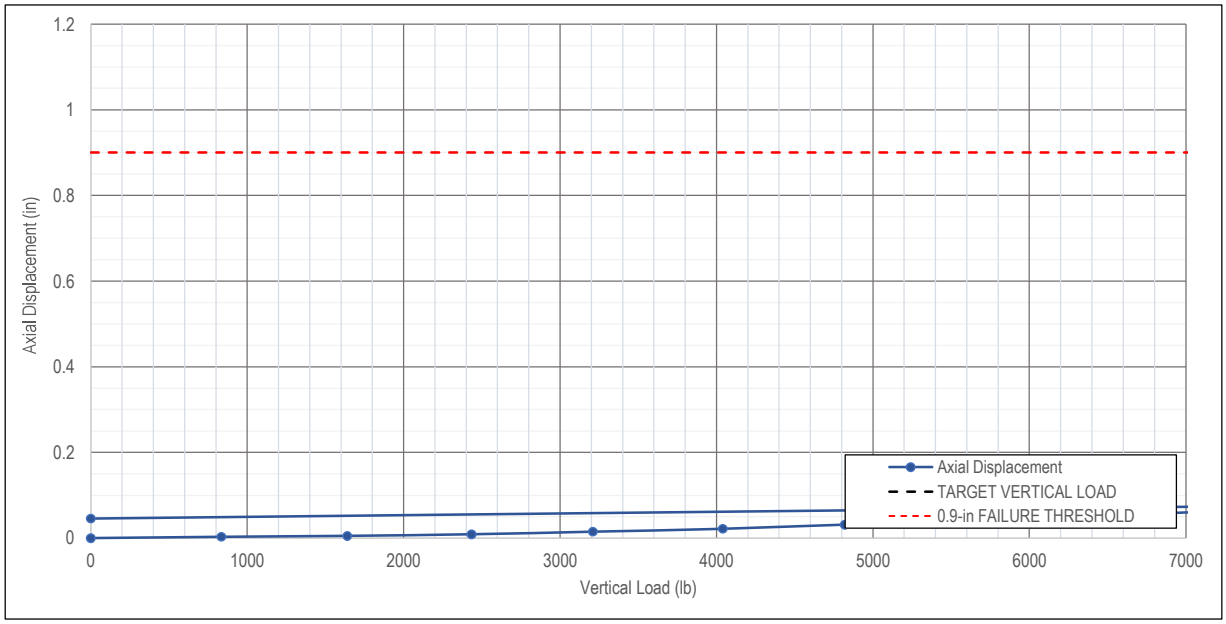
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	835	0.003	0.0035	0.00
436%	20%	1,600	1640	0.005	0.0055	0.01
654%	30%	2,400	2435	0.0095	0.0085	0.01
872%	40%	3,200	3210	0.0155	0.0145	0.02
1090%	50%	4,000	4040	0.0225	0.021	0.02
1308%	60%	4,800	4820	0.0325	0.03	0.03
1526%	70%	5,600	5605	0.0405	0.0375	0.04
1744%	80%	6,400	6435	0.0545	0.0505	0.05
1962%	90%	7,200	7170	0.0645	0.06	0.06
2180%	100%	8,000	8025	0.0795	0.0735	0.08
0%	0%	0	0	0.046	0.0455	0.05

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.05** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-2D

Lateral Load Test Results

PTL- **2** Designation: **D**

Section: **W6x12**

Latitude: 33.666069

Longitude: -114.70414

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 23.51

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

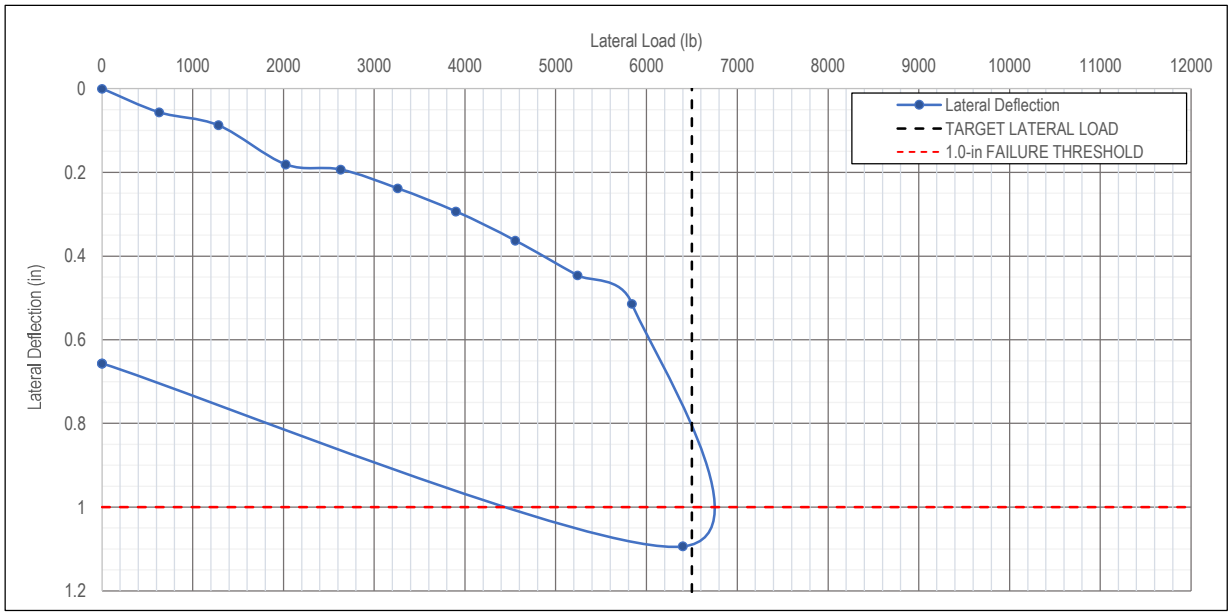
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	630	0.0555	0.0575	0.06
62%	20%	1,300	1285	0.0875	0.0875	0.09
93%	30%	1,950	2025	0.18	0.1825	0.18
124%	40%	2,600	2630	0.1915	0.1945	0.19
155%	50%	3,250	3260	0.236	0.24	0.24
186%	60%	3,900	3900	0.2915	0.295	0.29
217%	70%	4,550	4555	0.361	0.3645	0.36
248%	80%	5,200	5240	0.445	0.447	0.45
279%	90%	5,850	5840	0.5085	0.52	0.51
310%	100%	6,500	6400	1.129	1.0585	1.09
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.6855	0.628	0.66

Load at 1.0-in lateral deflection: **6,309** **lb**

Residual lateral deformation: **0.66** **in**



Notes:

Failure reached at 6,400 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-3A

Axial Uplift Load Test Results

PTL - **3** Designation: **A**

Section: W6x9

Latitude: 33.666027

Longitude: -114.69927

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 7.98

Pile Damaged? No

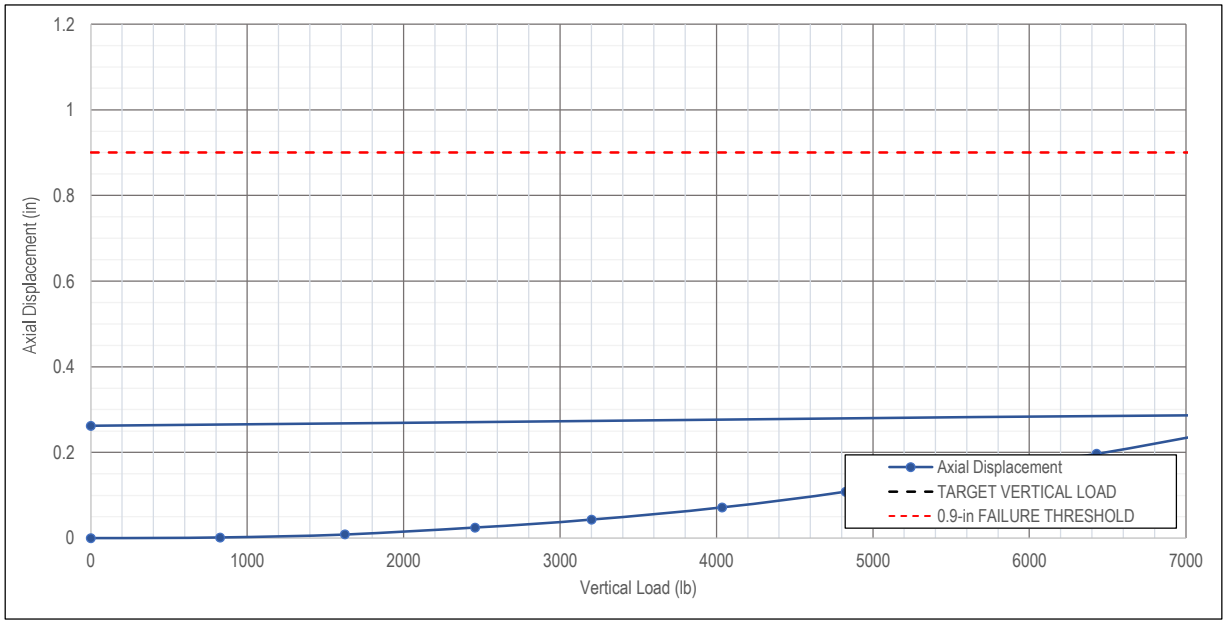
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	827	0.001	0.002	0.00
13594%	20%	1,600	1625	0.009	0.0085	0.01
20391%	30%	2,400	2456	0.025	0.024	0.02
27188%	40%	3,200	3202	0.044	0.0425	0.04
33985%	50%	4,000	4035	0.073	0.0705	0.07
40782%	60%	4,800	4825	0.11	0.107	0.11
47579%	70%	5,600	5620	0.156	0.154	0.16
54376%	80%	6,400	6430	0.199	0.1945	0.20
61173%	90%	7,200	7268	0.253	0.249	0.25
67970%	100%	8,000	7995	0.29	0.2855	0.29
0%	0%	0	0	0.262	0.262	0.26

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.26** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-3A

Lateral Load Test Results

PTL- **3** Designation: **A**

Section: W6x9

Latitude: 33.666027

Longitude: -114.69927

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 7.98

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

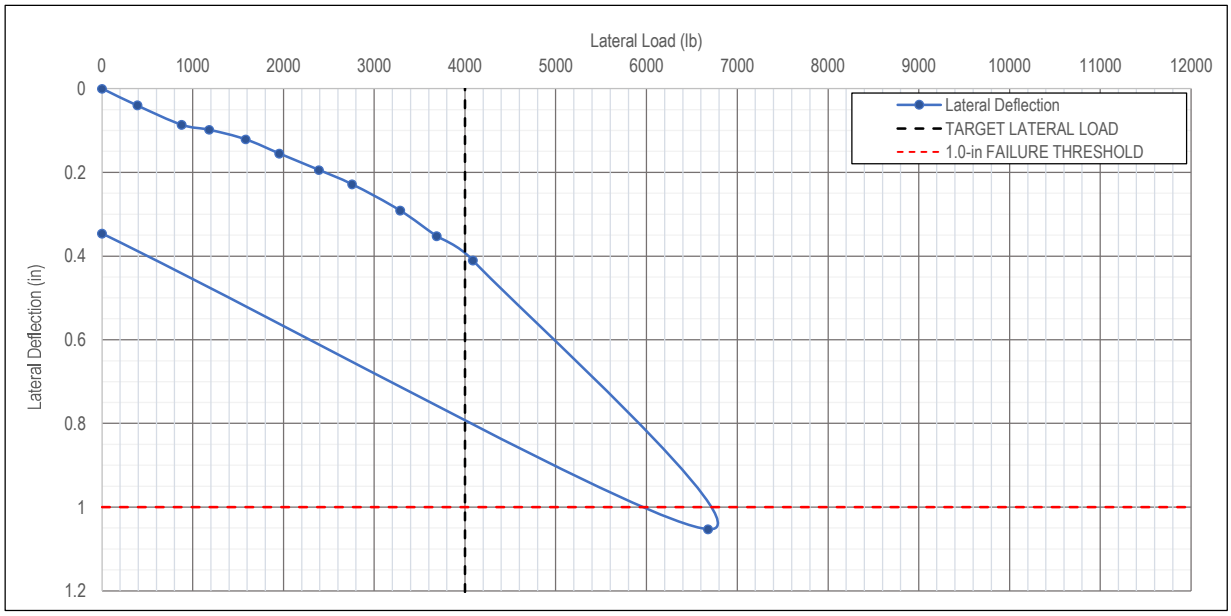
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	390	0.041	0.039	0.04
62%	20%	800	879	0.088	0.0845	0.09
93%	30%	1,200	1183	0.1	0.0965	0.10
124%	40%	1,600	1585	0.1225	0.1195	0.12
155%	50%	2,000	1955	0.1565	0.153	0.15
186%	60%	2,400	2390	0.1955	0.193	0.19
217%	70%	2,800	2757	0.2295	0.227	0.23
248%	80%	3,200	3286	0.292	0.29	0.29
279%	90%	3,600	3687	0.3525	0.352	0.35
310%	100%	4,000	4087	0.4105	0.41	0.41
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6679	1.036	1.07	1.05
0%	0%	0	0	0.3335	0.3585	0.35

Load at 1.0-in lateral deflection: **6,465** **lb**

Residual lateral deformation: **0.35** **in**



Notes:

Failure reached at 6,679 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-3B

Axial Uplift Load Test Results

PTL - **3** Designation: **B** Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.666027 - - Saturated Soil? No

Longitude: -114.69927 Target Embedment Depth (ft): 7.00 Drive Time (s): 12.69

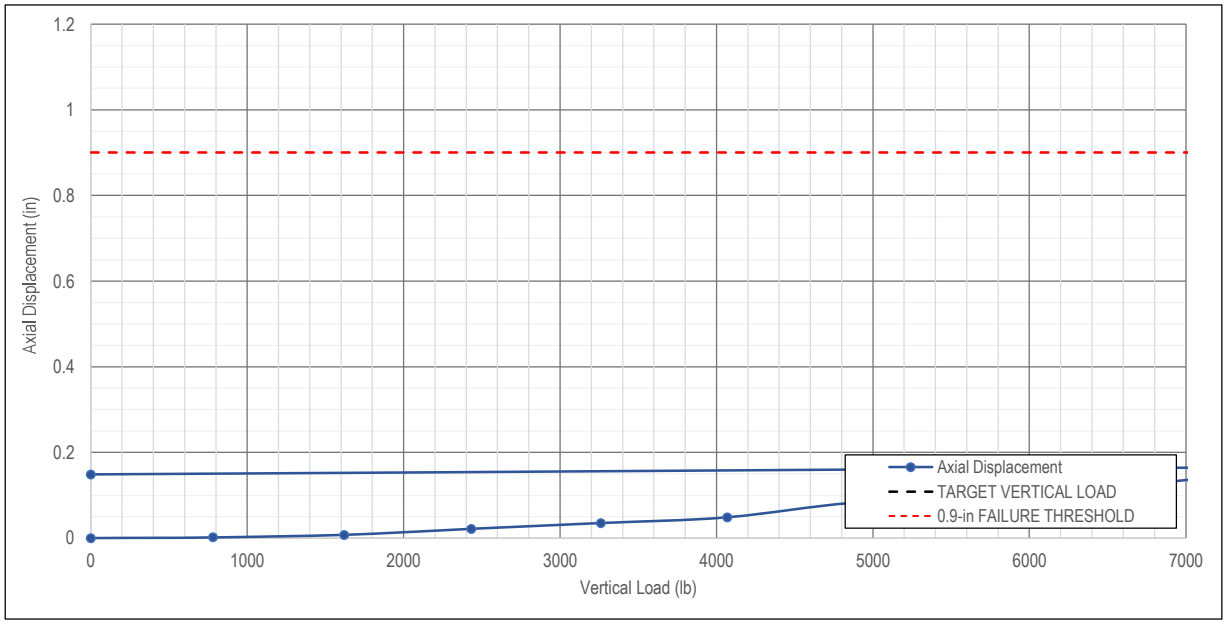
Tested By: ANS Actual Embedment Depth (ft): 7 Pile Damaged? No

Test Date: 10/19/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	781	0.002	0.0015	0.00
13594%	20%	1,600	1620	0.0075	0.008	0.01
20391%	30%	2,400	2433	0.0215	0.0215	0.02
27188%	40%	3,200	3260	0.035	0.0355	0.04
33985%	50%	4,000	4070	0.0485	0.049	0.05
40782%	60%	4,800	4865	0.0825	0.0825	0.08
47579%	70%	5,600	5659	0.0935	0.0935	0.09
54376%	80%	6,400	6425	0.1155	0.115	0.12
61173%	90%	7,200	7260	0.1455	0.144	0.14
67970%	100%	8,000	7956	0.1665	0.1645	0.17
0%	0%	0	0	0.149	0.148	0.15

Load at 0.9-in axial displacement: N/A lb **Residual axial displacement:** 0.15 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-3B

Lateral Load Test Results

PTL- **3** Designation: **B**

Section: W6x9

Latitude: 33.666027

Longitude: -114.69927

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 12.69

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

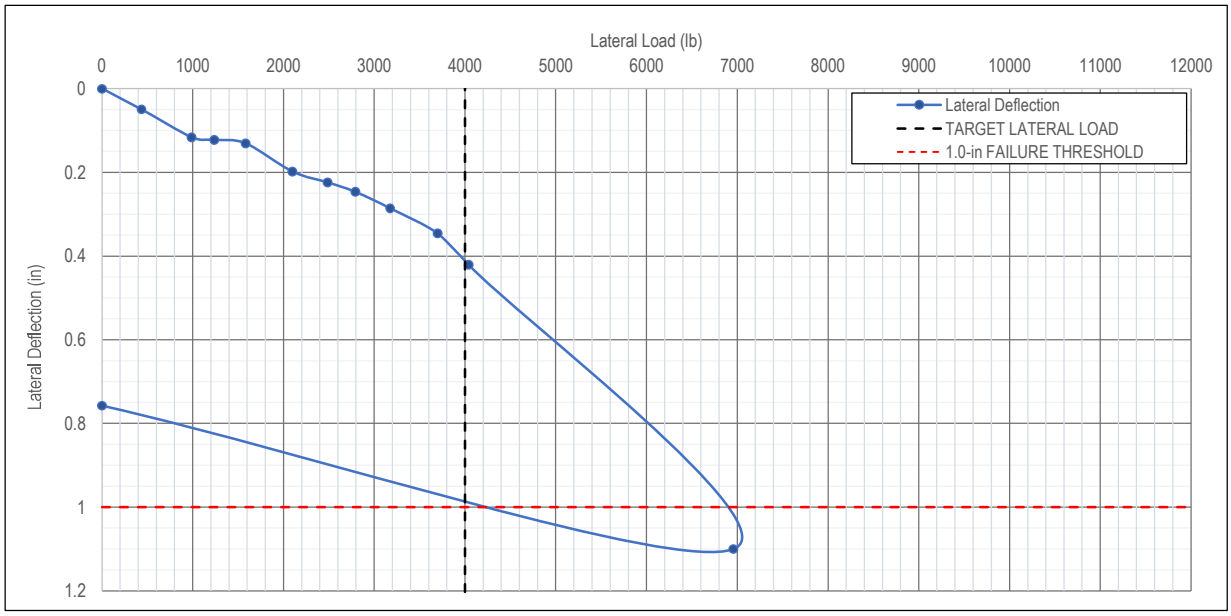
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	435	0.0475	0.051	0.05
62%	20%	800	987	0.1125	0.1195	0.12
93%	30%	1,200	1237	0.119	0.1255	0.12
124%	40%	1,600	1583	0.127	0.134	0.13
155%	50%	2,000	2100	0.194	0.202	0.20
186%	60%	2,400	2487	0.22	0.228	0.22
217%	70%	2,800	2792	0.2425	0.2505	0.25
248%	80%	3,200	3175	0.282	0.2895	0.29
279%	90%	3,600	3700	0.3425	0.349	0.35
310%	100%	4,000	4039	0.418	0.423	0.42
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6957	1.115	1.085	1.10
0%	0%	0	0	0.7135	0.8015	0.76

Load at 1.0-in lateral deflection: **6,528** **lb**

Residual lateral deformation: **0.76** **in**



Notes:

Failure reached at 6,957 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-4A

Axial Uplift Load Test Results

PTL - **4** Designation: **A** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.666659 - - Saturated Soil? No

Longitude: -114.69498 Target Embedment Depth (ft): 6.00 Drive Time (s): 12.09

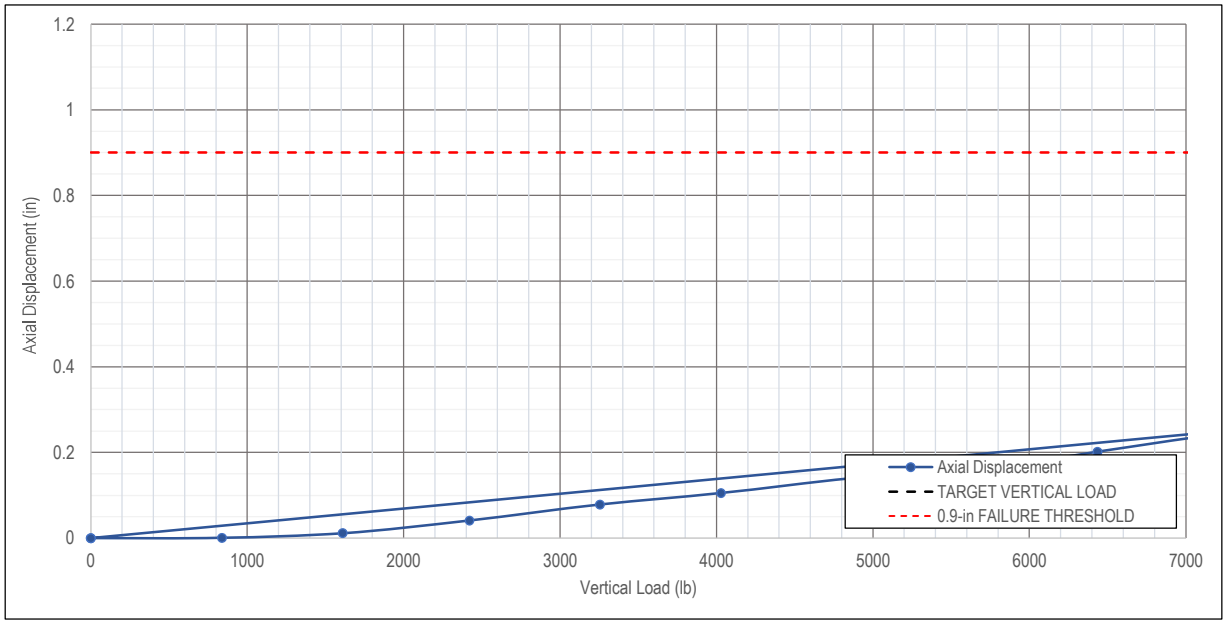
Tested By: ANS Actual Embedment Depth (ft): 6 Pile Damaged? No

Test Date: 10/18/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	839	0.0005	0.001	0.00
436%	20%	1,600	1611	0.011	0.0125	0.01
654%	30%	2,400	2421	0.0405	0.0415	0.04
872%	40%	3,200	3255	0.0775	0.0795	0.08
1090%	50%	4,000	4029	0.104	0.1065	0.11
1308%	60%	4,800	4862	0.139	0.14	0.14
1526%	70%	5,600	5614	0.15	0.1615	0.16
1744%	80%	6,400	6435	0.1995	0.2035	0.20
1962%	90%	7,200	7215	0.24	0.2465	0.24
2180%	100%	8,000	8020	0.2735	0.28	0.28
0%	0%	0				

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **TBD** **in**



Notes:

Residual deflection not recorded.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-4A

Lateral Load Test Results

PTL- **4** Designation: **A**

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Section: W6x9

Installation Method: Direct Drive (DD)

Saturated Soil? No

Latitude: 33.666659

- -

Drive Time (s): 12.09

Longitude: -114.69498

Target Embedment Depth (ft): 6.00

Pile Damaged? No

Tested By: ANS

Actual Embedment Depth (ft): 6

Gauge 1 Height (in): 6

Test Date: 10/18/2025

Lateral Load Application Height (ft): 5

Gauge 2 Height (in): 6

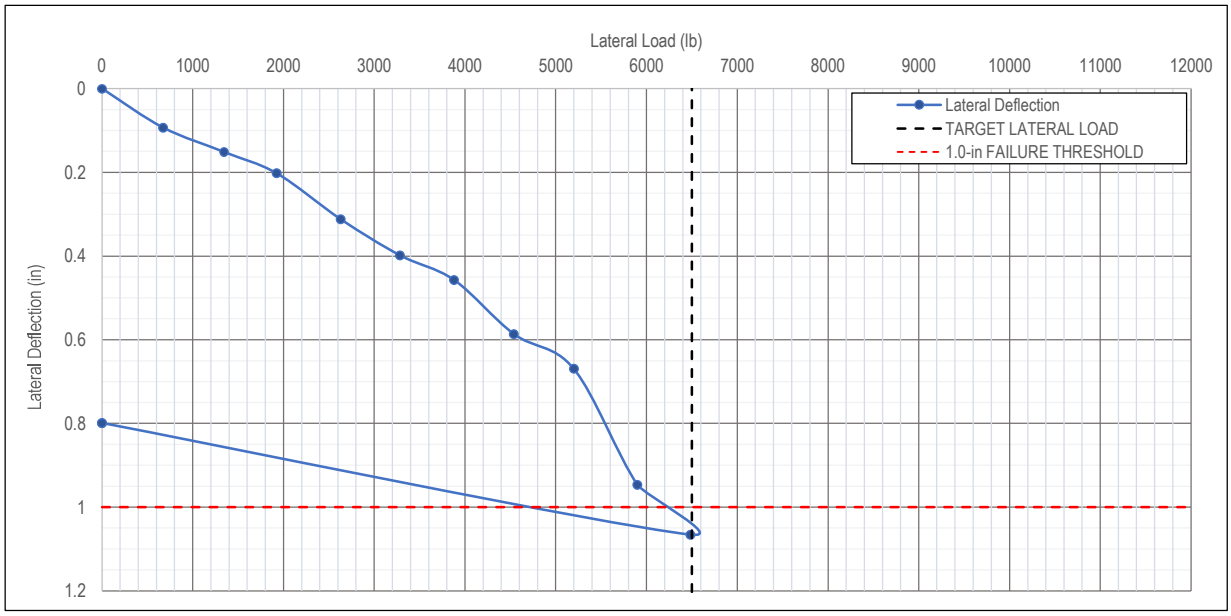
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	675	0.0935	0.093	0.09
62%	20%	1,300	1345	0.1515	0.1505	0.15
93%	30%	1,950	1926	0.203	0.201	0.20
124%	40%	2,600	2630	0.312	0.3115	0.31
155%	50%	3,250	3285	0.3975	0.399	0.40
186%	60%	3,900	3879	0.455	0.4585	0.46
217%	70%	4,550	4539	0.5795	0.5935	0.59
248%	80%	5,200	5200	0.6555	0.683	0.67
279%	90%	5,850	5900	0.8735	1.019	0.95
310%	100%	6,500	6485	1.0085	1.1235	1.07
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0		0.791	0.807	0.80

Load at 1.0-in lateral deflection: **6,163** **lb**

Residual lateral deformation: **0.80** **in**



Notes:

Failure reached at 6,485 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-4B

Axial Uplift Load Test Results

PTL - **4** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.666659 - - Saturated Soil? No

Longitude: -114.69498 Target Embedment Depth (ft): 8.00 Drive Time (s): 17.45

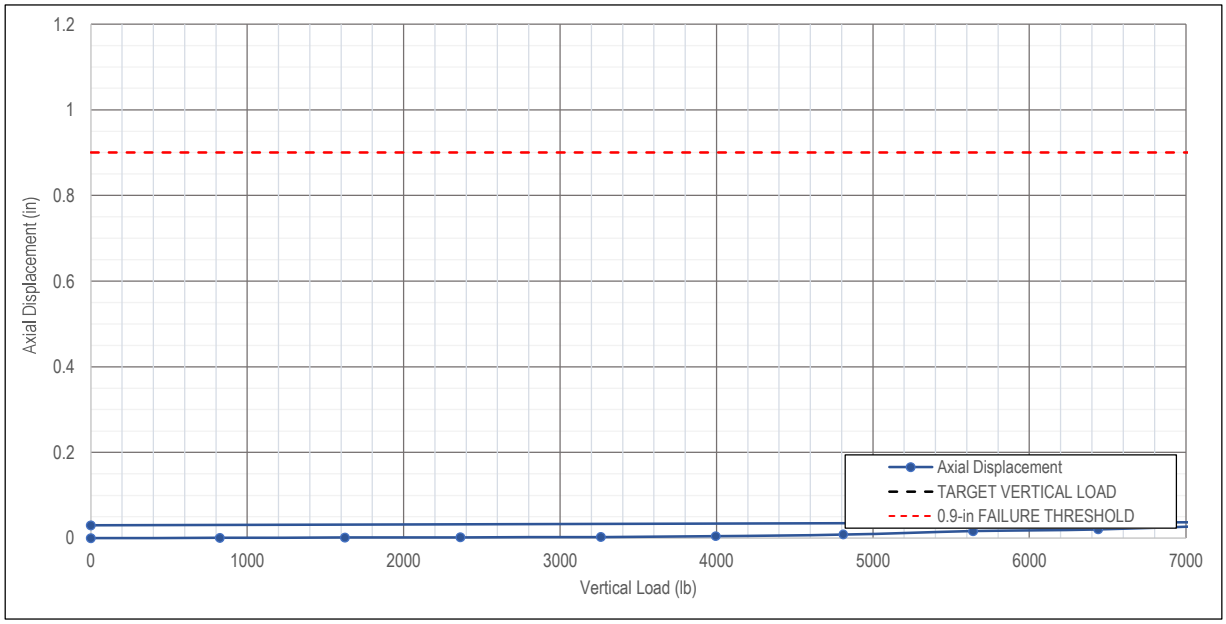
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/18/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	825	0.0005	0.001	0.00
436%	20%	1,600	1625	0.0015	0.0015	0.00
654%	30%	2,400	2362	0.002	0.002	0.00
872%	40%	3,200	3260	0.0025	0.003	0.00
1090%	50%	4,000	3995	0.004	0.0055	0.00
1308%	60%	4,800	4811	0.0075	0.009	0.01
1526%	70%	5,600	5640	0.0155	0.017	0.02
1744%	80%	6,400	6440	0.0195	0.0215	0.02
1962%	90%	7,200	7265	0.029	0.0305	0.03
2180%	100%	8,000	8017	0.0365	0.0385	0.04
0%	0%	0		0.0305	0.0295	0.03

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-4B

Lateral Load Test Results

PTL- **4** Designation: **B**

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Section: W6x9

Installation Method: Direct Drive (DD)

Saturated Soil? No

Latitude: 33.666659

- -

Drive Time (s): 17.45

Longitude: -114.69498

Target Embedment Depth (ft): 8.00

Pile Damaged? No

Tested By: ANS

Actual Embedment Depth (ft): 8

Gauge 1 Height (in): 6

Test Date: 10/18/2025

Lateral Load Application Height (ft): 5

Gauge 2 Height (in): 6

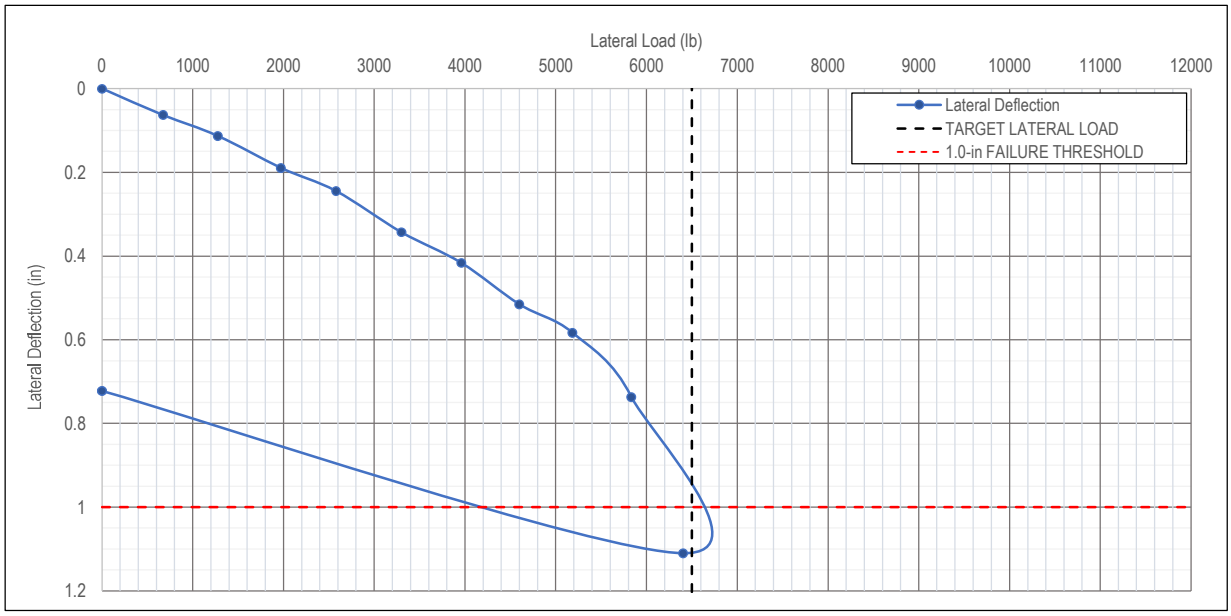
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	675	0.0565	0.0695	0.06
62%	20%	1,300	1275	0.108	0.117	0.11
93%	30%	1,950	1974	0.185	0.194	0.19
124%	40%	2,600	2580	0.24	0.249	0.24
155%	50%	3,250	3300	0.34	0.346	0.34
186%	60%	3,900	3960	0.414	0.4185	0.42
217%	70%	4,550	4600	0.516	0.515	0.52
248%	80%	5,200	5185	0.5735	0.5925	0.58
279%	90%	5,850	5835	0.7565	0.7175	0.74
310%	100%	6,500	6405	1.1285	1.0915	1.11
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.7315	0.7125	0.72

Load at 1.0-in lateral deflection: **6,237** **lb**

Residual lateral deformation: **0.72** **in**



Notes:

Failure reached at 6,405 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-5A

Axial Uplift Load Test Results

PTL - **5** Designation: **A**

Section: W6x9

Latitude: 33.665953

Longitude: -114.69092

Tested By: ANS

Test Date: 10/18/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 16.6

Pile Damaged? No

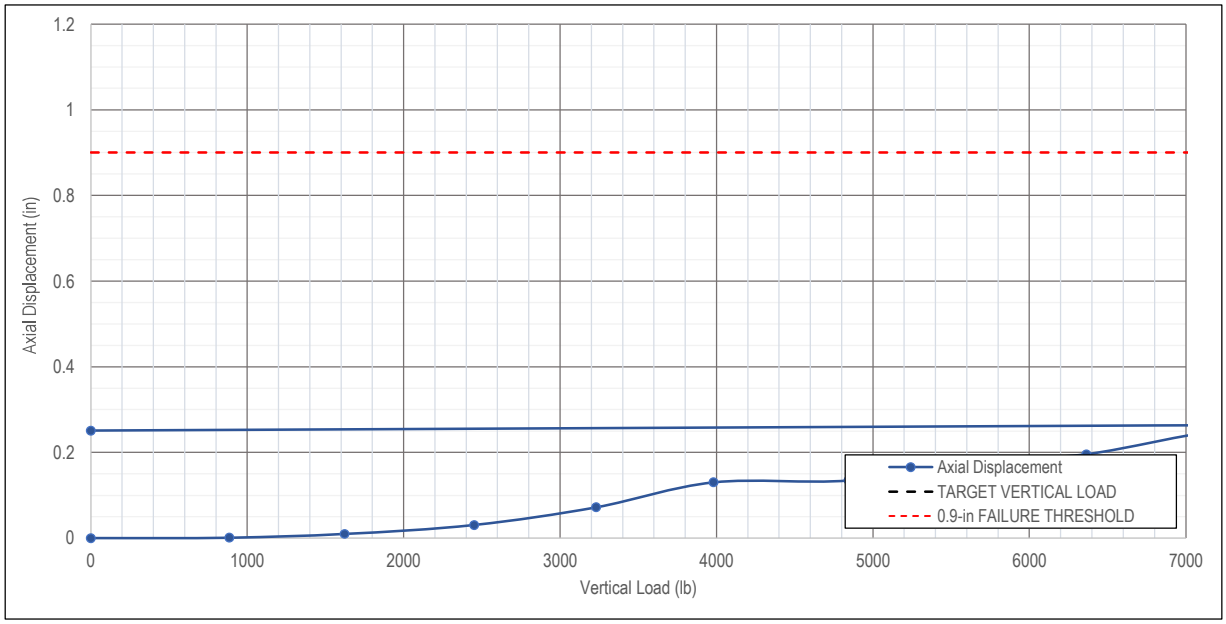
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	886	0.0015	0.001	0.00
13594%	20%	1,600	1623	0.01	0.0095	0.01
20391%	30%	2,400	2452	0.031	0.03	0.03
27188%	40%	3,200	3230	0.073	0.0715	0.07
33985%	50%	4,000	3980	0.131	0.1295	0.13
40782%	60%	4,800	4845	0.136	0.134	0.14
47579%	70%	5,600	5640	0.1805	0.179	0.18
54376%	80%	6,400	6365	0.197	0.195	0.20
61173%	90%	7,200	7260	0.254	0.253	0.25
67970%	100%	8,000	8076	0.2645	0.264	0.26
0%	0%	0	0	0.251	0.251	0.25

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.25** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-5A

Lateral Load Test Results

PTL- **5** Designation: **A**

Section: W6x9

Latitude: 33.665953

Longitude: -114.69092

Tested By: ANS

Test Date: 10/18/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 16.6

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

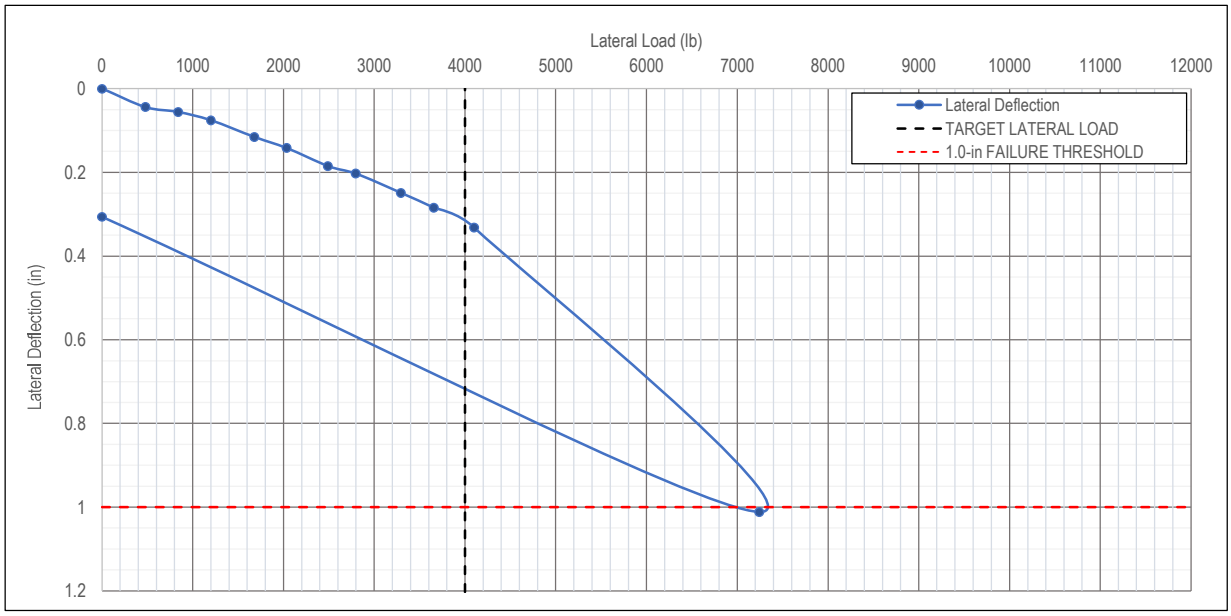
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	480	0.049	0.0385	0.04
62%	20%	800	840	0.061	0.05	0.06
93%	30%	1,200	1200	0.0825	0.069	0.08
124%	40%	1,600	1680	0.124	0.1065	0.12
155%	50%	2,000	2037	0.1505	0.133	0.14
186%	60%	2,400	2490	0.192	0.178	0.19
217%	70%	2,800	2796	0.21	0.1955	0.20
248%	80%	3,200	3295	0.254	0.244	0.25
279%	90%	3,600	3656	0.2985	0.2695	0.28
310%	100%	4,000	4100	0.3345	0.329	0.33
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7245	1.0145	1.009	1.01
0%	0%	0	0	0.2835	0.329	0.31

Load at 1.0-in lateral deflection: **7,191** **lb**

Residual lateral deformation: **0.31** **in**



Notes:

Failure reached at 7,245 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-5B

Axial Uplift Load Test Results

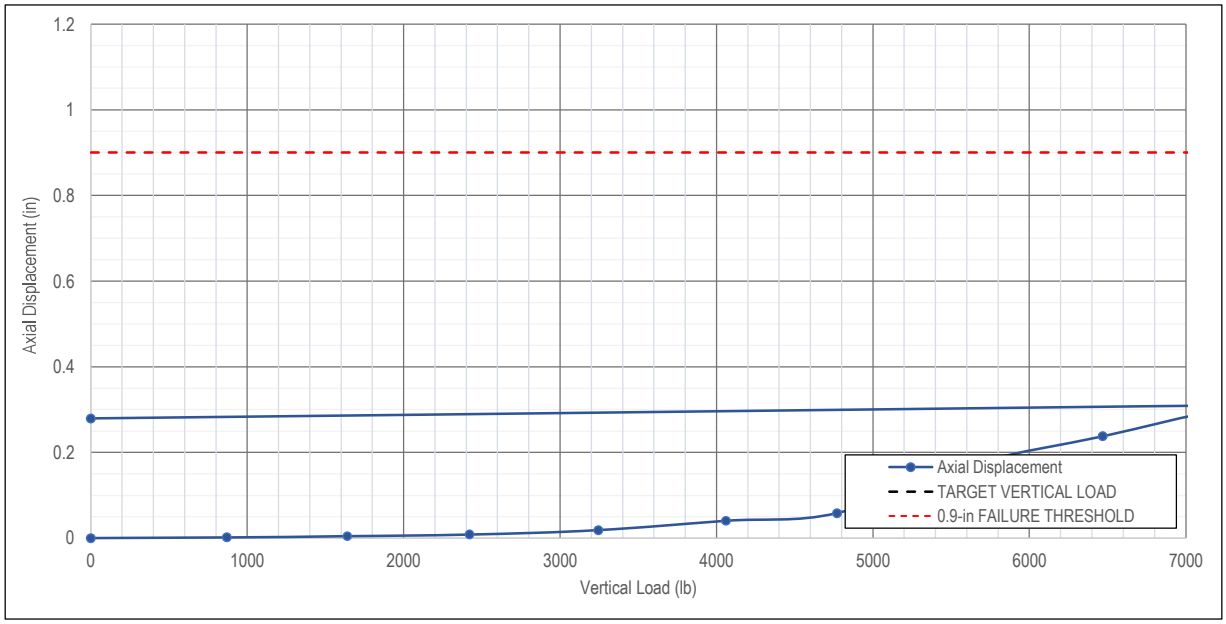
PTL - **5**
Designation: **B**
Target Test Load (lb): **8,000** (≈ 67970% Design Load)
Section: W6x9
Installation Method: Direct Drive (DD)
Latitude: 33.665953
Longitude: -114.69092
- -
Saturated Soil? No
Tested By: ANS
Target Embedment Depth (ft): 7.00
Drive Time (s): 20.75
Test Date: 10/18/2025
Actual Embedment Depth (ft): 7
Pile Damaged? No

Strong Axis Plumb. (deg):
Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	870	0.002	0.002	0.00
13594%	20%	1,600	1640	0.0045	0.0045	0.00
20391%	30%	2,400	2421	0.0085	0.0085	0.01
27188%	40%	3,200	3245	0.019	0.018	0.02
33985%	50%	4,000	4060	0.041	0.04	0.04
40782%	60%	4,800	4770	0.059	0.0575	0.06
47579%	70%	5,600	5665	0.1755	0.175	0.18
54376%	80%	6,400	6470	0.2375	0.238	0.24
61173%	90%	7,200	7259	0.299	0.301	0.30
67970%	100%	8,000	7982	0.312	0.3125	0.31
0%	0%	0	0	0.276	0.2825	0.28

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.28** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-5B

Lateral Load Test Results

PTL- **5** Designation: **B**

Section: W6x9

Latitude: 33.665953

Longitude: -114.69092

Tested By: ANS

Test Date: 10/18/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 20.75

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

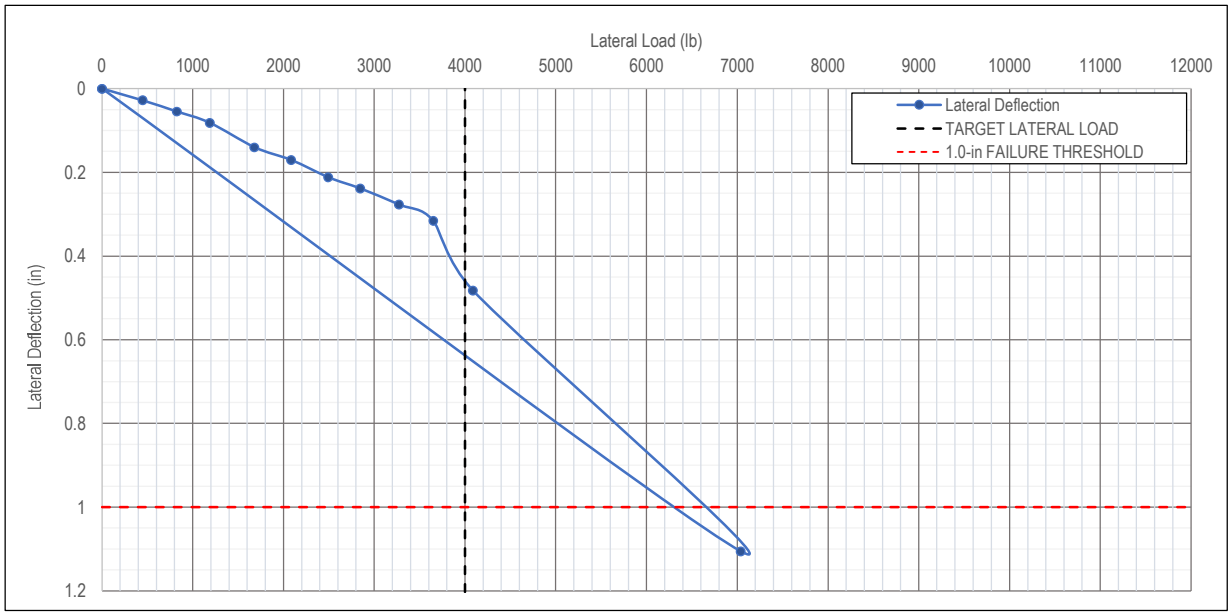
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	450	0.027	0.0285	0.03
62%	20%	800	825	0.0535	0.0555	0.05
93%	30%	1,200	1187	0.08	0.0825	0.08
124%	40%	1,600	1680	0.1385	0.1415	0.14
155%	50%	2,000	2086	0.1695	0.1715	0.17
186%	60%	2,400	2493	0.2105	0.213	0.21
217%	70%	2,800	2846	0.237	0.24	0.24
248%	80%	3,200	3272	0.275	0.2785	0.28
279%	90%	3,600	3651	0.3135	0.317	0.32
310%	100%	4,000	4087	0.4745	0.49	0.48
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7040	1.125	1.0865	1.11
0%	0%	0				

Load at 1.0-in lateral deflection: **6,539** **lb**

Residual lateral deformation: **TBD** **in**



Notes:

Failure reached at 7,040 lbs. Residual deflection not recorded.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-5C

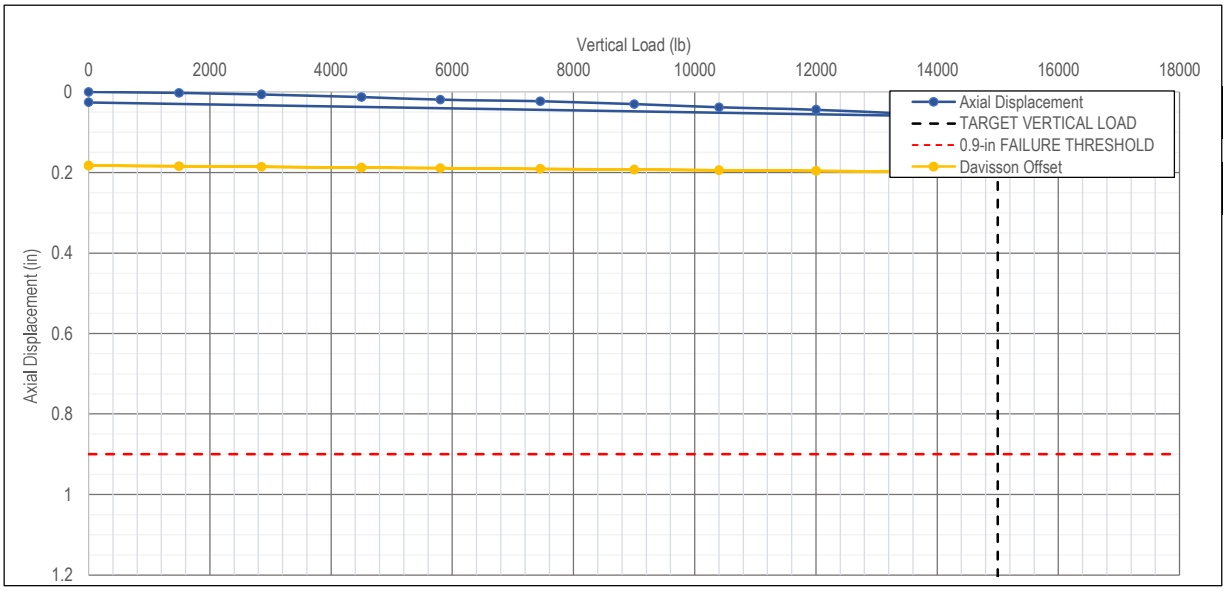
Axial Compression Load Test Results (supplemental pile)

PTL - **5**
Designation: **C**
Target Test Load (lb): **15,000** (≈ 1180% Design Load)
Section: W6x9
Installation Method: Direct Drive (DD)
Latitude: 33.665953
- -
Saturated Soil? No
Longitude: -114.69092
Target Embedment Depth (ft): 5.00
Drive Time (s): 9.3
Tested By: ANS
Actual Embedment Depth (ft): 5
Pile Damaged? No
Test Date: 10/18/2025

Strong Axis Plumb. (deg):
Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL COMPRESSION** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
118%	10%	1,500	1490	0.002	0.003	0.00
236%	20%	3,000	2850	0.0055	0.007	0.01
354%	30%	4,500	4500	0.0115	0.014	0.01
472%	40%	6,000	5800	0.0175	0.021	0.02
590%	50%	7,500	7450	0.0215	0.025	0.02
708%	60%	9,000	9000	0.027	0.0335	0.03
826%	70%	10,500	10400	0.0355	0.041	0.04
944%	80%	12,000	12000	0.042	0.046	0.04
1062%	90%	13,500	13500	0.0525	0.055	0.05
1180%	100%	15,000	15000	0.0605	0.0655	0.06
0%	0%	0	0	0.022	0.0295	0.03

Load at 0.9-in axial displacement: **N/A** **lb**
Residual axial displacement: **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-6A

Axial Uplift Load Test Results

PTL - **6** Designation: **A**

Section: W6x9

Latitude: 33.666457

Longitude: -114.68688

Tested By: ANS

Test Date: 10/18/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 12

Pile Damaged? No

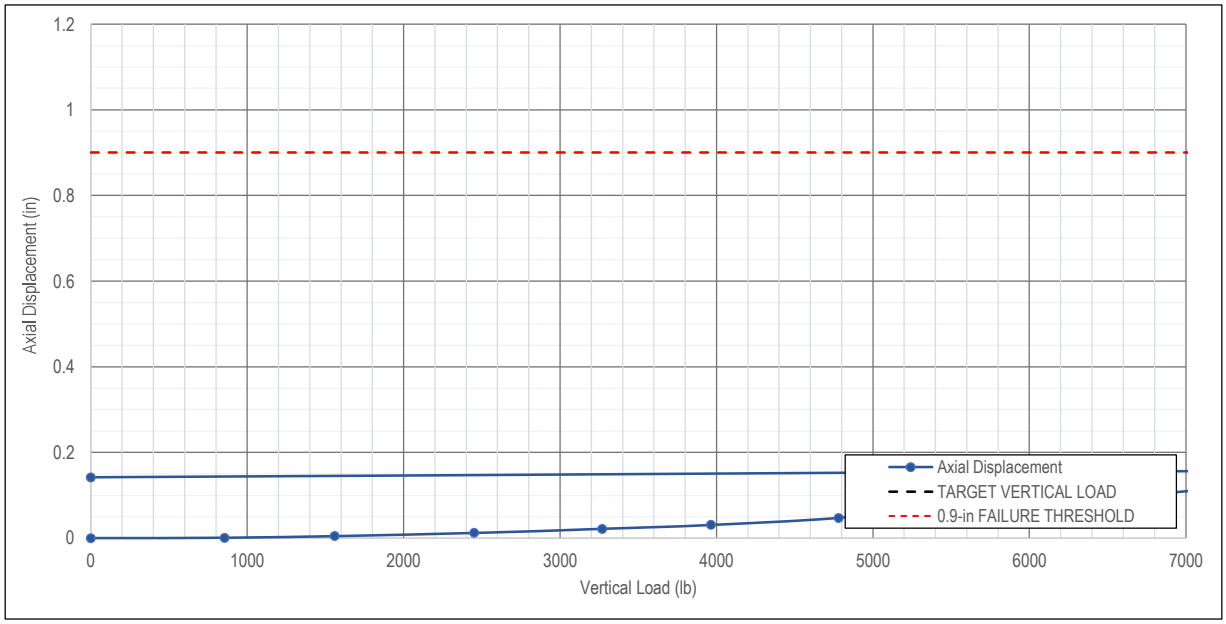
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	856	0.001	0.001	0.00
436%	20%	1,600	1560	0.0055	0.0045	0.01
654%	30%	2,400	2450	0.012	0.0125	0.01
872%	40%	3,200	3270	0.021	0.022	0.02
1090%	50%	4,000	3965	0.03	0.0315	0.03
1308%	60%	4,800	4780	0.046	0.0486	0.05
1526%	70%	5,600	5670	0.0805	0.0835	0.08
1744%	80%	6,400	6430	0.0925	0.0955	0.09
1962%	90%	7,200	7160	0.1135	0.1165	0.12
2180%	100%	8,000	8017	0.1535	0.158	0.16
0%	0%	0	0	0.1435	0.1405	0.14

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.14** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-6A

Lateral Load Test Results

PTL- **6** Designation: **A**

Section: W6x9

Latitude: 33.666457

Longitude: -114.68688

Tested By: ANS

Test Date: 10/18/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 12

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

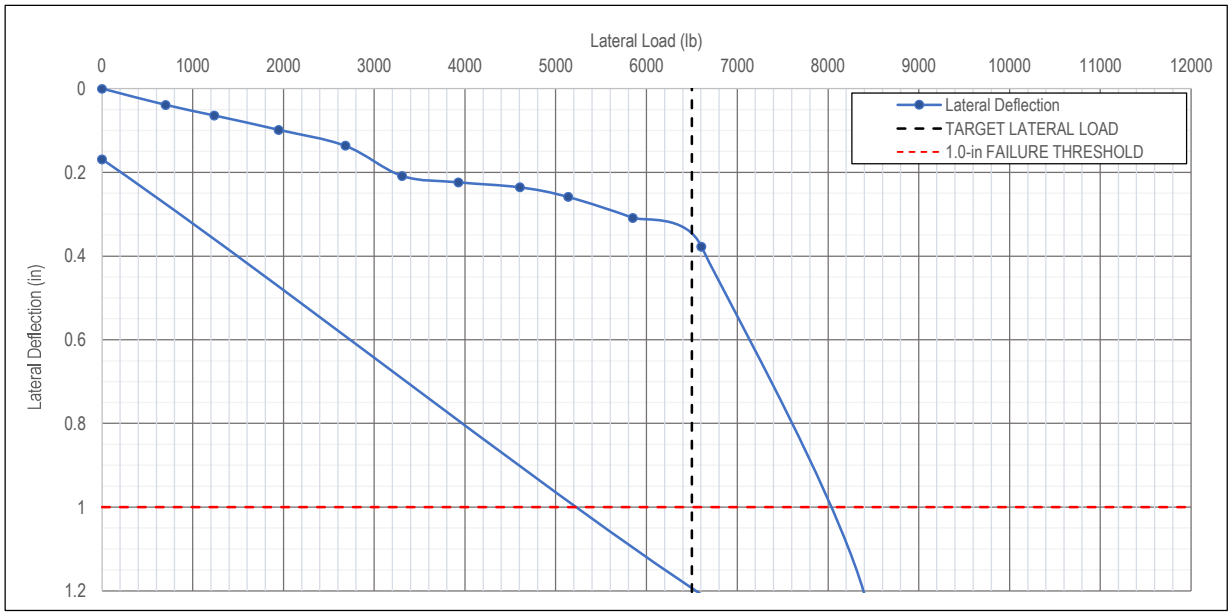
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	703	0.0395	0.038	0.04
62%	20%	1,300	1237	0.0675	0.06	0.06
93%	30%	1,950	1949	0.1035	0.0935	0.10
124%	40%	2,600	2683	0.142	0.1305	0.14
155%	50%	3,250	3307	0.216	0.2015	0.21
186%	60%	3,900	3927	0.229	0.2185	0.22
217%	70%	4,550	4607	0.2435	0.228	0.24
248%	80%	5,200	5139	0.2665	0.251	0.26
279%	90%	5,850	5850	0.3175	0.3005	0.31
310%	100%	6,500	6607	0.3865	0.369	0.38
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			8205	1.4801	1.3015	1.39
0%	0%	0	0	0.1715	0.166	0.17

Load at 1.0-in lateral deflection: **7,589** **lb**

Residual lateral deformation: **0.17** **in**



Notes:

Failure reached at 8,205 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-6B

Axial Uplift Load Test Results

PTL - **6** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.666457 - - Saturated Soil? No

Longitude: -114.68688 Target Embedment Depth (ft): 8.00 Drive Time (s): 15.28

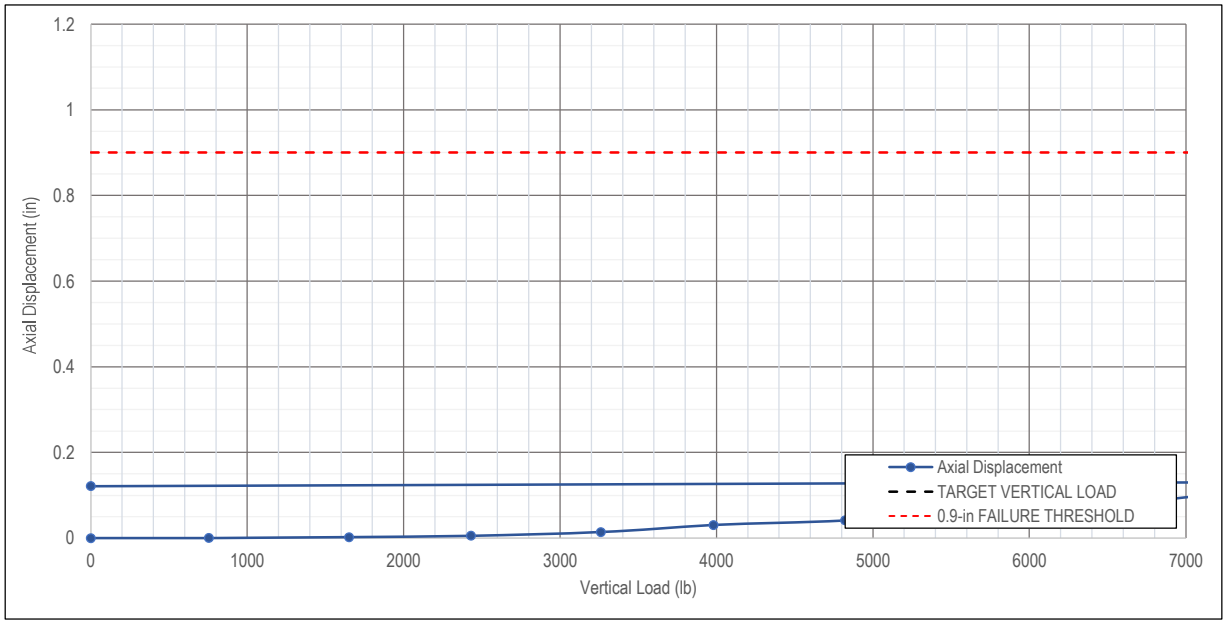
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/18/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	755	0.0005	0.0005	0.00
436%	20%	1,600	1651	0.002	0.003	0.00
654%	30%	2,400	2430	0.0055	0.006	0.01
872%	40%	3,200	3260	0.014	0.014	0.01
1090%	50%	4,000	3980	0.031	0.03	0.03
1308%	60%	4,800	4823	0.0425	0.0405	0.04
1526%	70%	5,600	5618	0.063	0.0615	0.06
1744%	80%	6,400	6434	0.0765	0.073	0.07
1962%	90%	7,200	7259	0.1075	0.103	0.11
2180%	100%	8,000	8046	0.1325	0.1265	0.13
0%	0%	0	0	0.122	0.1205	0.12

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.12** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-6B

Lateral Load Test Results

PTL- **6** Designation: **B**

Section: W6x9

Latitude: 33.666457

Longitude: -114.68688

Tested By: ANS

Test Date: 10/18/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 15.28

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

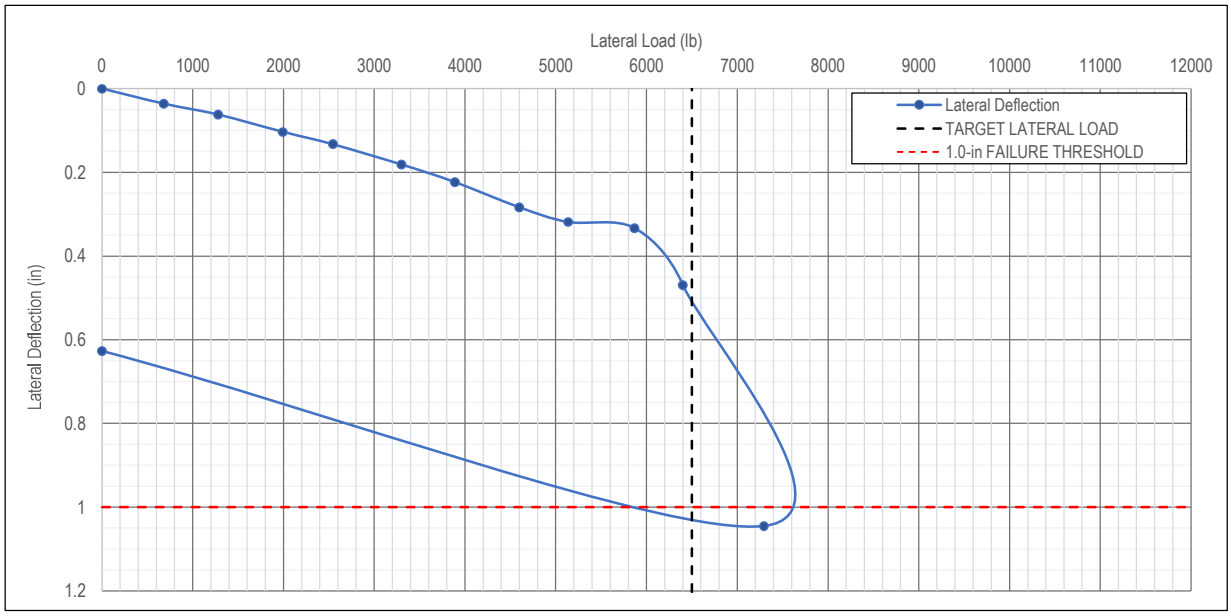
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	683	0.0415	0.0305	0.04
62%	20%	1,300	1280	0.0685	0.0555	0.06
93%	30%	1,950	1993	0.111	0.0955	0.10
124%	40%	2,600	2545	0.141	0.124	0.13
155%	50%	3,250	3300	0.19	0.1725	0.18
186%	60%	3,900	3889	0.2325	0.214	0.22
217%	70%	4,550	4600	0.2935	0.2735	0.28
248%	80%	5,200	5137	0.327	0.3095	0.32
279%	90%	5,850	5869	0.3375	0.329	0.33
310%	100%	6,500	6400	0.467	0.471	0.47
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7295	1.0045	1.0855	1.05
0%	0%	0	0	0.5965	0.657	0.63

Load at 1.0-in lateral deflection: **7,225** **lb**

Residual lateral deformation: **0.63** **in**



Notes:

Failure reached at 7,295 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-6D

Axial Uplift Load Test Results

PTL - **6**
Section: **W6x12**
Latitude: 33.666457
Longitude: -114.68688
Tested By: ANS
Test Date: 10/24/2025

Designation: **D**

Target Test Load (lb): **8,000** (≈ 2180% Design Load)
Installation Method: Direct Drive (DD)
- -
Target Embedment Depth (ft): 5.00
Actual Embedment Depth (ft): 5

Saturated Soil? No
Drive Time (s): 14.04
Pile Damaged? No

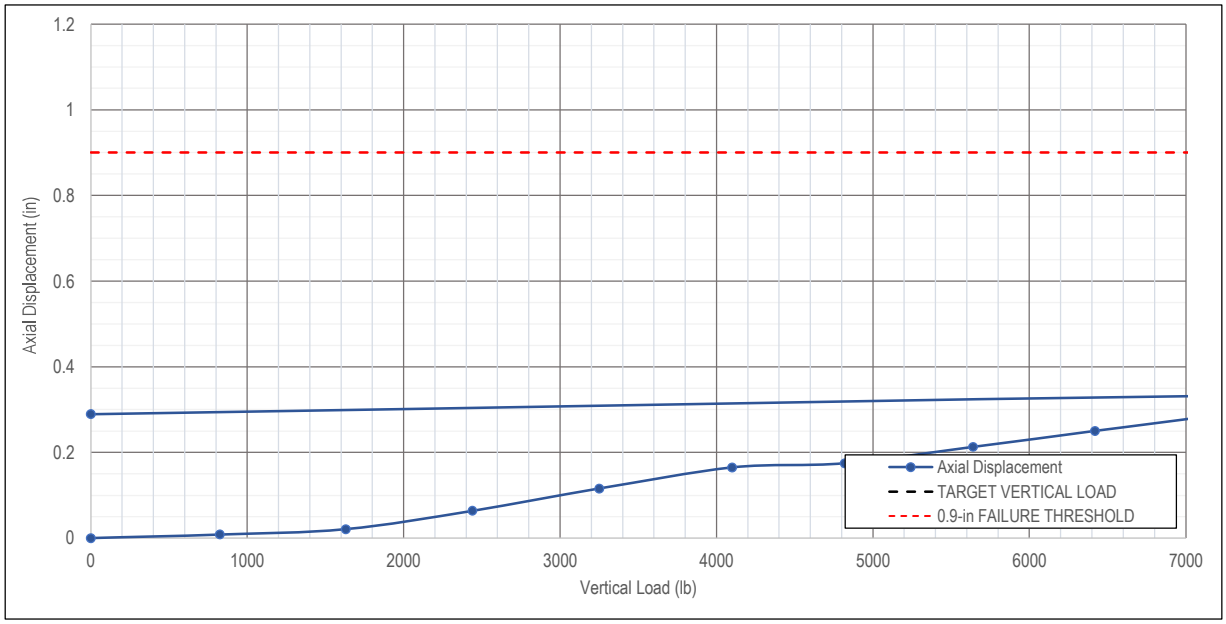
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	825	0.009	0.008	0.01
436%	20%	1,600	1630	0.022	0.02	0.02
654%	30%	2,400	2440	0.064	0.064	0.06
872%	40%	3,200	3250	0.117	0.1145	0.12
1090%	50%	4,000	4100	0.1675	0.1625	0.17
1308%	60%	4,800	4820	0.178	0.172	0.18
1526%	70%	5,600	5640	0.2175	0.209	0.21
1744%	80%	6,400	6420	0.255	0.2455	0.25
1962%	90%	7,200	7180	0.2915	0.282	0.29
2180%	100%	8,000	8010	0.34	0.328	0.33
0%	0%	0	0	0.289	0.29	0.29

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.29** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-6D

Lateral Load Test Results

PTL- **6** Designation: **D**

Section: **W6x12**

Latitude: 33.666457

Longitude: -114.68688

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 14.04

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

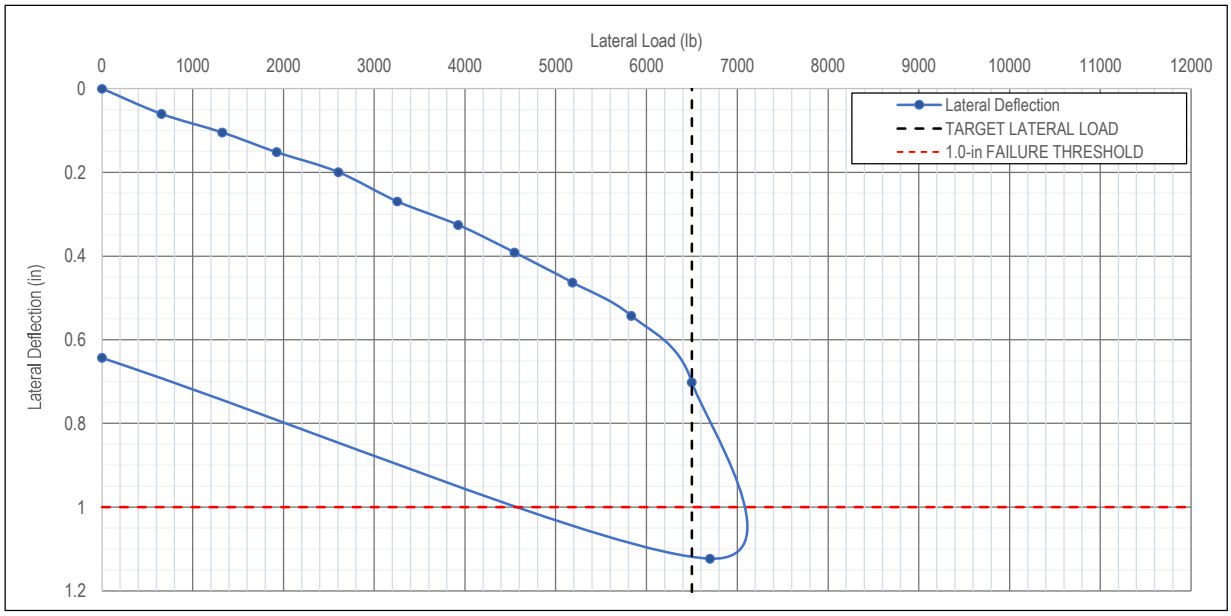
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	655	0.0645	0.056	0.06
62%	20%	1,300	1325	0.1115	0.0975	0.10
93%	30%	1,950	1925	0.1545	0.1485	0.15
124%	40%	2,600	2605	0.204	0.1945	0.20
155%	50%	3,250	3255	0.277	0.262	0.27
186%	60%	3,900	3925	0.3395	0.312	0.33
217%	70%	4,550	4545	0.406	0.376	0.39
248%	80%	5,200	5185	0.4785	0.4475	0.46
279%	90%	5,850	5835	0.563	0.523	0.54
310%	100%	6,500	6500	0.7585	0.6445	0.70
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6700	1.277	0.9685	1.12
0%	0%	0	0	0.7525	0.5345	0.64

Load at 1.0-in lateral deflection: **6,642** **lb**

Residual lateral deformation: **0.64** **in**



Notes:

Failure reached at 6,700 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-7A

Axial Uplift Load Test Results

PTL - **7** Designation: **A**

Section: W6x9

Latitude: 33.662502

Longitude: -114.70856

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 17.69

Pile Damaged? No

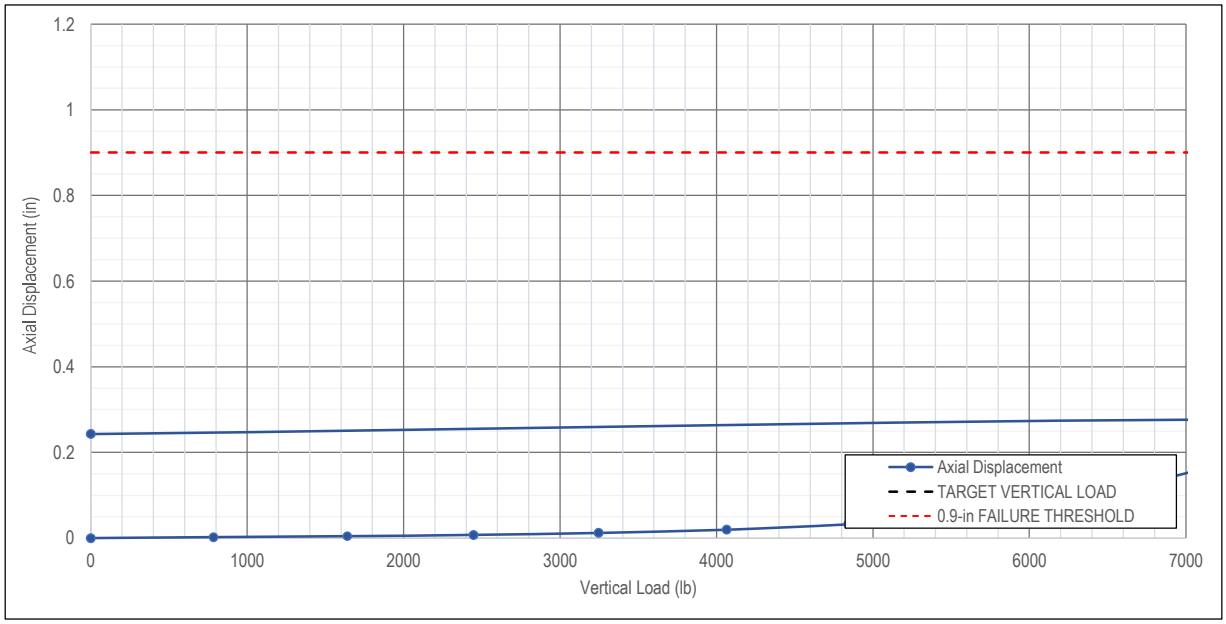
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	785	0.003	0.002	0.00
13594%	20%	1,600	1640	0.0045	0.0045	0.00
20391%	30%	2,400	2448	0.007	0.008	0.01
27188%	40%	3,200	3246	0.012	0.0125	0.01
33985%	50%	4,000	4066	0.0195	0.02	0.02
40782%	60%	4,800	4842	0.032	0.0325	0.03
47579%	70%	5,600	5636	0.0525	0.053	0.05
54376%	80%	6,400	6418	0.103	0.103	0.10
61173%	90%	7,200	7239	0.175	0.175	0.18
67970%	100%	8,000	8031	0.275	0.274	0.27
0%	0%	0	0	0.2435	0.2425	0.24

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.24** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-7A

Lateral Load Test Results

PTL- **7** Designation: **A**

Section: W6x9

Latitude: 33.662502

Longitude: -114.70856

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 17.69

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

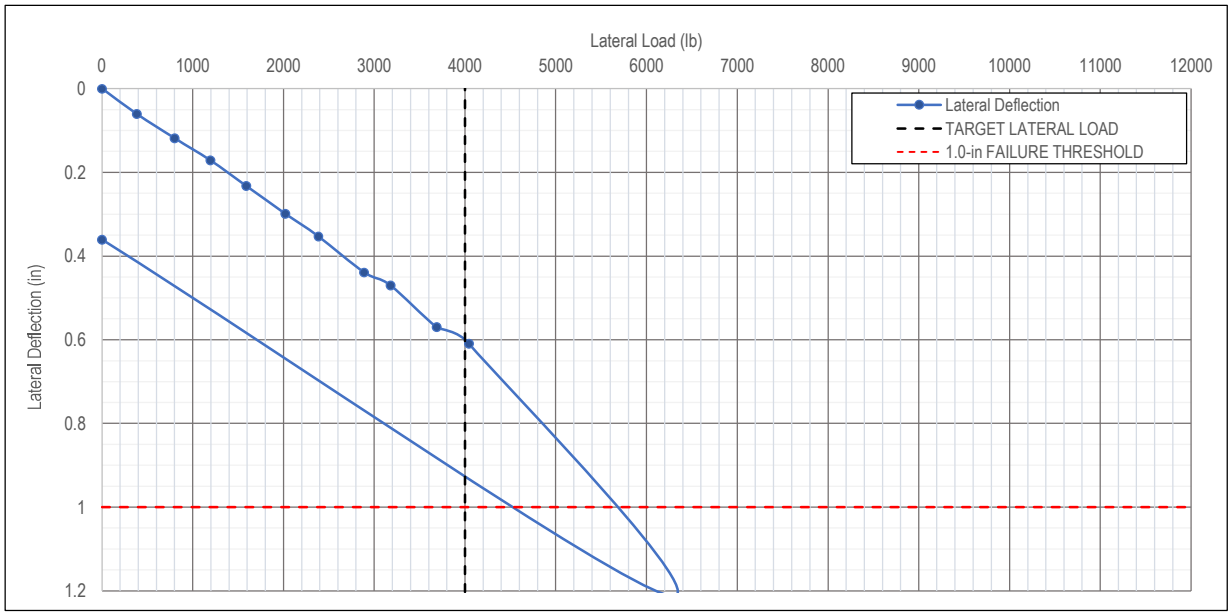
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	384	0.06	0.0605	0.06
62%	20%	800	801	0.1195	0.1175	0.12
93%	30%	1,200	1195	0.172	0.17	0.17
124%	40%	1,600	1590	0.2345	0.23	0.23
155%	50%	2,000	2020	0.302	0.295	0.30
186%	60%	2,400	2386	0.3575	0.3485	0.35
217%	70%	2,800	2889	0.4455	0.4325	0.44
248%	80%	3,200	3180	0.477	0.463	0.47
279%	90%	3,600	3690	0.571	0.569	0.57
310%	100%	4,000	4044	0.619	0.5995	0.61
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6227	1.2214	1.1985	1.21
0%	0%	0	0	0.361	0.3605	0.36

Load at 1.0-in lateral deflection: **5,464** **lb**

Residual lateral deformation: **0.36** **in**



Notes:

Failure reached at 6,227 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-7B

Axial Uplift Load Test Results

PTL - **7** Designation: **B**

Section: W6x9

Latitude: 33.662502

Longitude: -114.70856

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 20.88

Pile Damaged? No

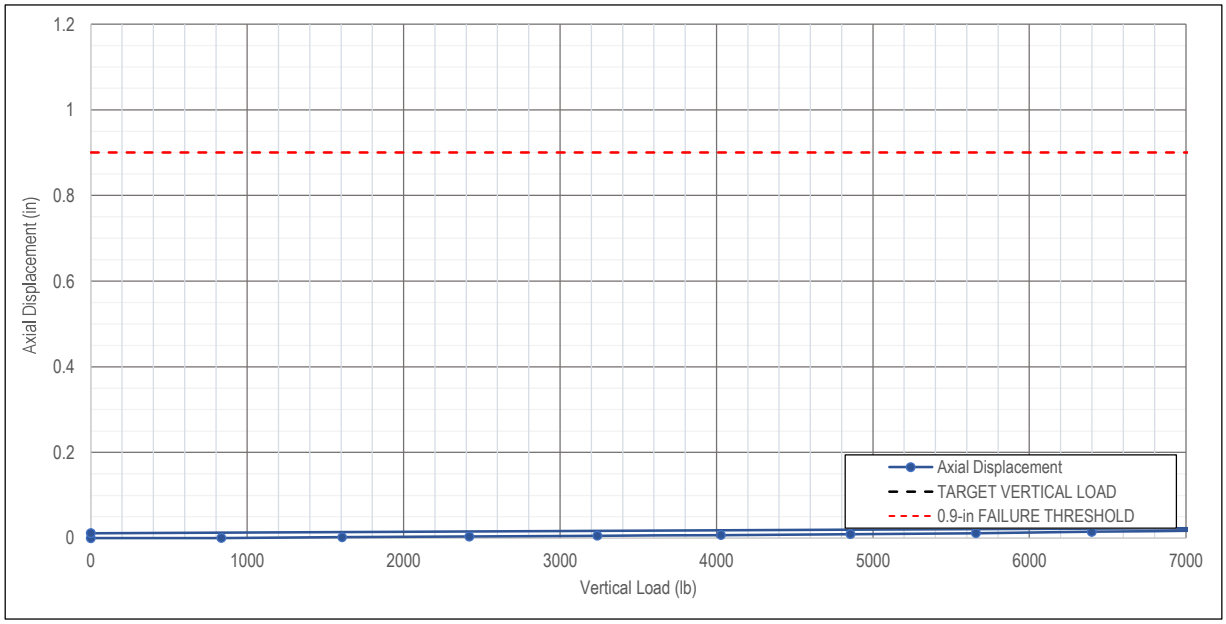
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	835	0.0005	0.0005	0.00
13594%	20%	1,600	1607	0.0015	0.003	0.00
20391%	30%	2,400	2420	0.003	0.0045	0.00
27188%	40%	3,200	3238	0.0045	0.0065	0.01
33985%	50%	4,000	4029	0.0065	0.008	0.01
40782%	60%	4,800	4856	0.009	0.01	0.01
47579%	70%	5,600	5659	0.0115	0.012	0.01
54376%	80%	6,400	6399	0.0145	0.015	0.01
61173%	90%	7,200	7202	0.018	0.018	0.02
67970%	100%	8,000	8032	0.025	0.024	0.02
0%	0%	0	0	0.0115	0.012	0.01

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-7B

Lateral Load Test Results

PTL- **7** Designation: **B**

Section: W6x9

Latitude: 33.662502

Longitude: -114.70856

Tested By: ANS

Test Date: 10/29/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 20.88

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

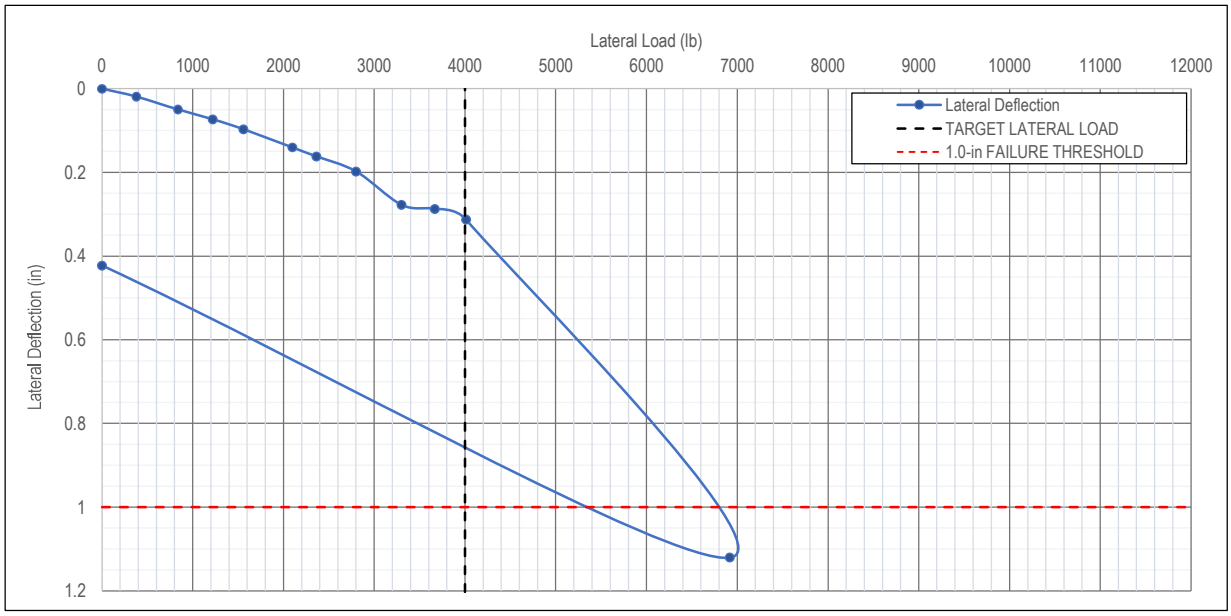
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	379	0.019	0.019	0.02
62%	20%	800	838	0.0485	0.051	0.05
93%	30%	1,200	1222	0.0715	0.0745	0.07
124%	40%	1,600	1560	0.0955	0.0985	0.10
155%	50%	2,000	2099	0.1385	0.142	0.14
186%	60%	2,400	2364	0.1605	0.163	0.16
217%	70%	2,800	2800	0.1965	0.199	0.20
248%	80%	3,200	3300	0.277	0.2775	0.28
279%	90%	3,600	3667	0.288	0.287	0.29
310%	100%	4,000	4013	0.313	0.313	0.31
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6918	1.132	1.1095	1.12
0%	0%	0	0	0.4715	0.3735	0.42

Load at 1.0-in lateral deflection: **6,484** **lb**

Residual lateral deformation: **0.42** **in**



Notes:

Failure reached at 6,918 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-7C

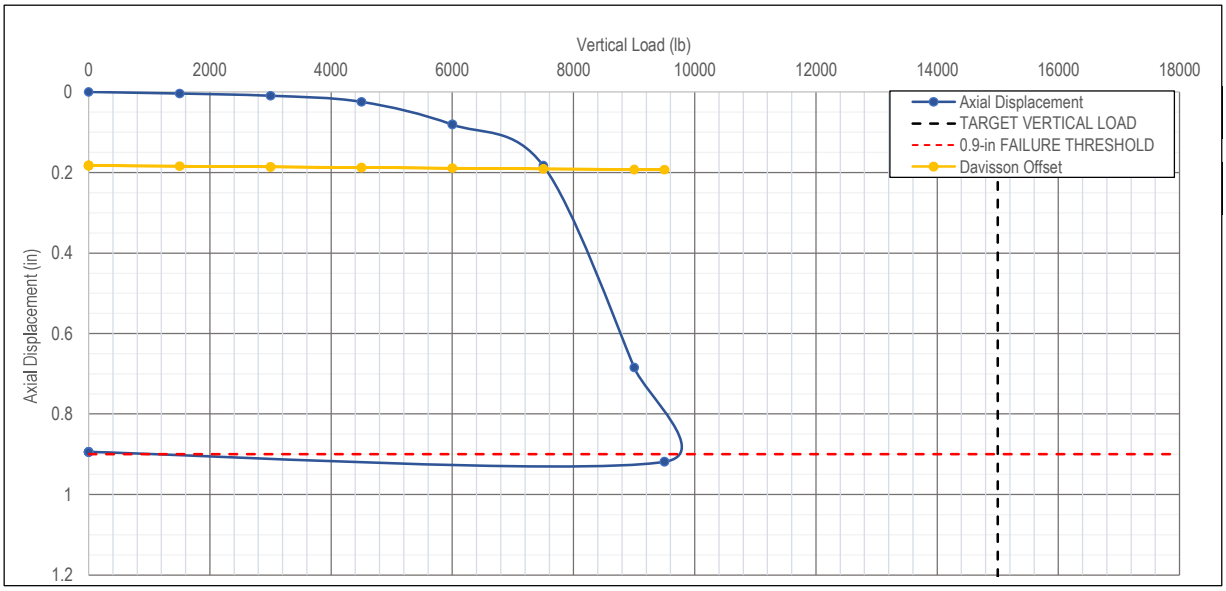
Axial Compression Load Test Results (supplemental pile)

PTL - **7**
Designation: **C**
Target Test Load (lb): **15,000** (≈ 1180% Design Load)
Section: W6x9
Installation Method: Direct Drive (DD)
Latitude: 33.662502
- -
Saturated Soil? No
Longitude: -114.70856
Target Embedment Depth (ft): 5.00
Drive Time (s): 7.18
Tested By: ANS
Actual Embedment Depth (ft): 5
Pile Damaged? No
Test Date: 10/19/2025

Strong Axis Plumb. (deg):
Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL COMPRESSION** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
118%	10%	1,500	1500	0.005	0.003	0.00
236%	20%	3,000	3000	0.011	0.008	0.01
354%	30%	4,500	4500	0.027	0.022	0.02
472%	40%	6,000	6000	0.0835	0.078	0.08
590%	50%	7,500	7500	0.186	0.1805	0.18
708%	60%	9,000	9000	0.688	0.6815	0.68
826%	70%	10,500	9500	0.922	0.915	0.92
944%	80%	12,000				
1062%	90%	13,500				
1180%	100%	15,000				
0%	0%	0	0	0.895	0.894	0.89

Load at 0.9-in axial displacement: **9,460** **lb**
Residual axial displacement: **0.89** **in**



Notes:

Failure reached at 9,500 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-8A

Axial Uplift Load Test Results

PTL - **8** Designation: **A**

Section: W6x9

Latitude: 33.663704

Longitude: -114.70577

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 13.48

Pile Damaged? No

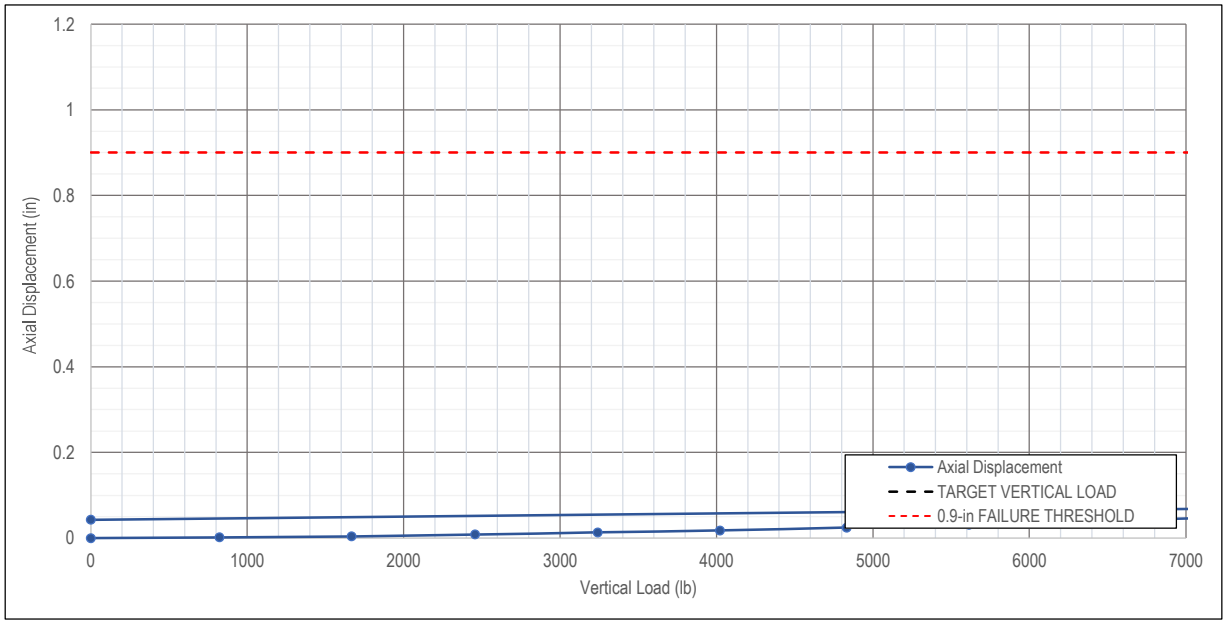
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	823	0.002	0.002	0.00
436%	20%	1,600	1667	0.0045	0.004	0.00
654%	30%	2,400	2456	0.009	0.0075	0.01
872%	40%	3,200	3241	0.014	0.0125	0.01
1090%	50%	4,000	4022	0.02	0.016	0.02
1308%	60%	4,800	4833	0.0265	0.0225	0.02
1526%	70%	5,600	5614	0.034	0.0275	0.03
1744%	80%	6,400	6429	0.043	0.0375	0.04
1962%	90%	7,200	7197	0.051	0.047	0.05
2180%	100%	8,000	8020	0.0735	0.0675	0.07
0%	0%	0	0	0.0425	0.0435	0.04

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.04** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-8A

Lateral Load Test Results

PTL- **8** Designation: **A**

Section: W6x9

Latitude: 33.663704

Longitude: -114.70577

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 13.48

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

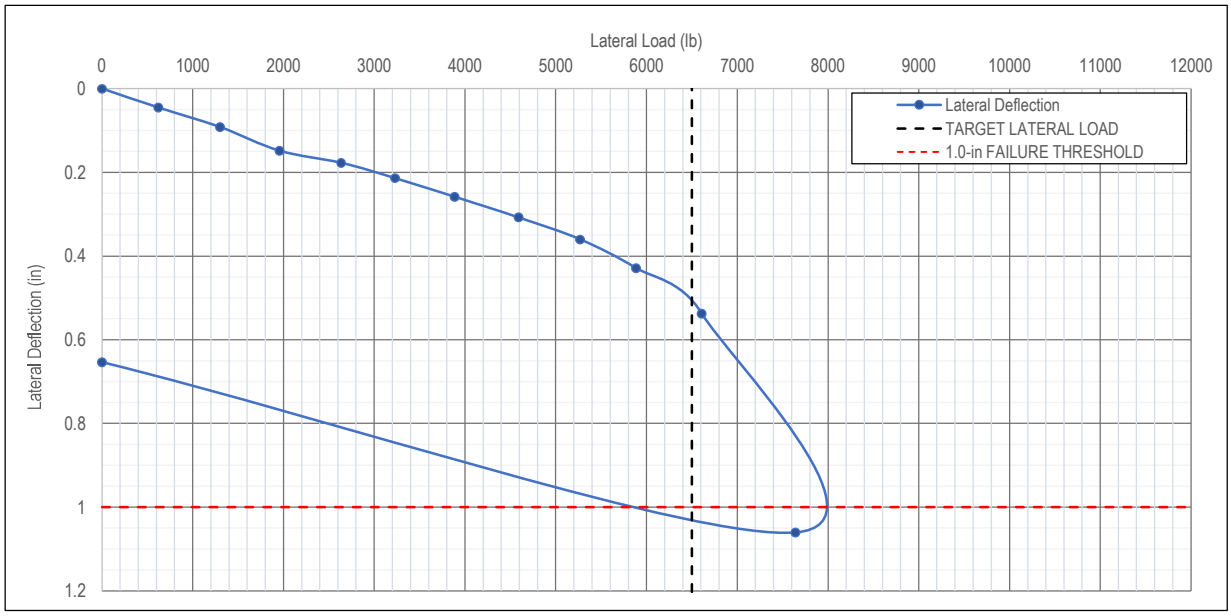
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	621	0.0485	0.041	0.04
62%	20%	1,300	1301	0.0975	0.086	0.09
93%	30%	1,950	1957	0.155	0.1415	0.15
124%	40%	2,600	2637	0.1845	0.17	0.18
155%	50%	3,250	3230	0.221	0.2065	0.21
186%	60%	3,900	3887	0.266	0.251	0.26
217%	70%	4,550	4591	0.315	0.3	0.31
248%	80%	5,200	5270	0.369	0.3515	0.36
279%	90%	5,850	5886	0.436	0.422	0.43
310%	100%	6,500	6609	0.541	0.534	0.54
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7643	1.0485	1.073	1.06
0%	0%	0	0	0.6615	0.6455	0.65

Load at 1.0-in lateral deflection: **7,523** **lb**

Residual lateral deformation: **0.65** **in**



Notes:

Failure reached at 7,643 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-8B

Axial Uplift Load Test Results

PTL - **8** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.663704 - - Saturated Soil? No

Longitude: -114.70577 Target Embedment Depth (ft): 8.00 Drive Time (s): 35.21

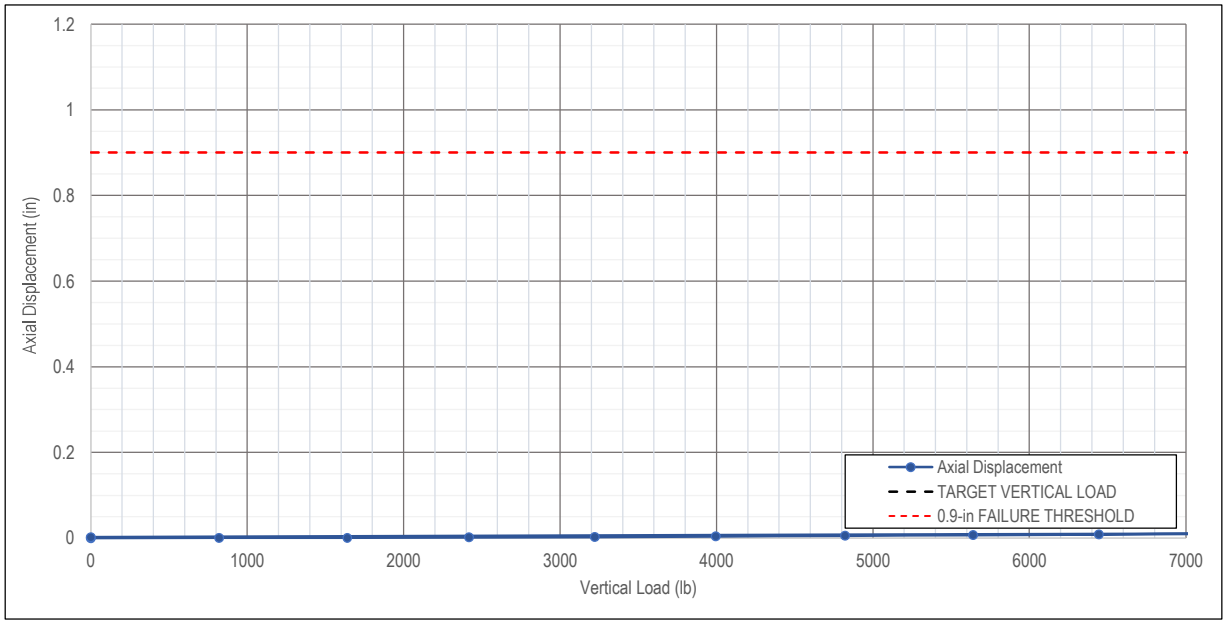
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/19/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	820	0.001	0.001	0.00
436%	20%	1,600	1640	0.001	0.001	0.00
654%	30%	2,400	2418	0.002	0.0015	0.00
872%	40%	3,200	3221	0.003	0.0025	0.00
1090%	50%	4,000	3995	0.0045	0.0035	0.00
1308%	60%	4,800	4822	0.0065	0.005	0.01
1526%	70%	5,600	5640	0.0075	0.0065	0.01
1744%	80%	6,400	6444	0.0095	0.0075	0.01
1962%	90%	7,200	7228	0.011	0.0095	0.01
2180%	100%	8,000	7964	0.013	0.0105	0.01
0%	0%	0	0	0.0025	0.0025	0.00

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.00** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-8B

Lateral Load Test Results

PTL- **8** Designation: **B**

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Section: W6x9

Installation Method: Direct Drive (DD)

Saturated Soil? No

Latitude: 33.663704

- -

Drive Time (s): 35.21

Longitude: -114.70577

Target Embedment Depth (ft): 8.00

Pile Damaged? No

Tested By: ANS

Actual Embedment Depth (ft): 8

Gauge 1 Height (in): 6

Test Date: 10/19/2025

Lateral Load Application Height (ft): 5

Gauge 2 Height (in): 6

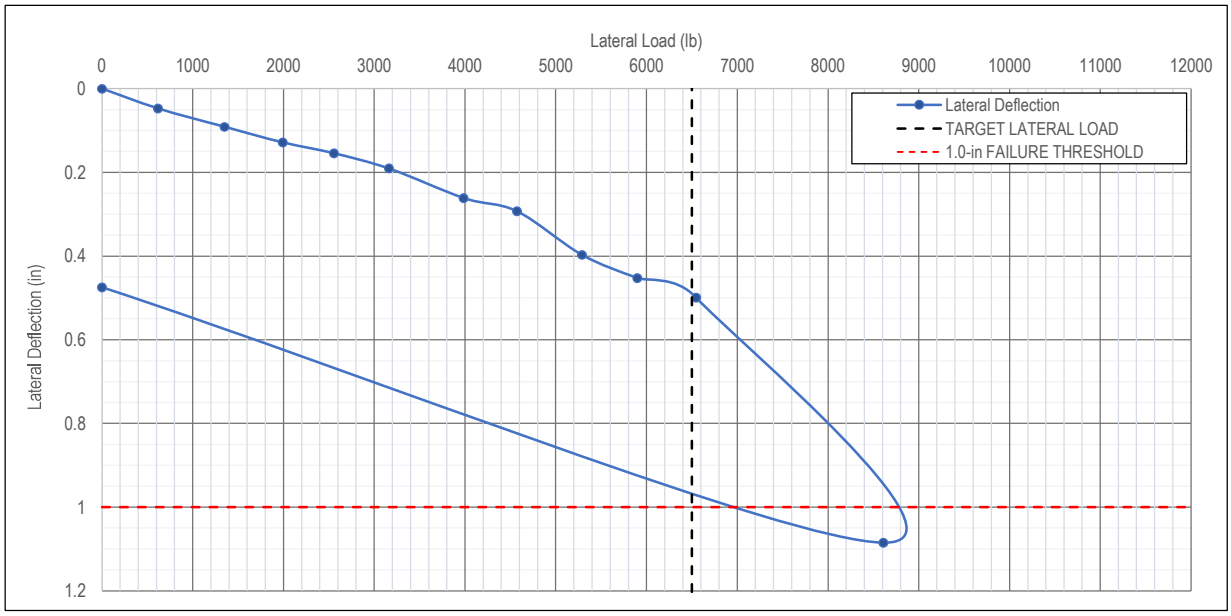
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	617	0.052	0.042	0.05
62%	20%	1,300	1349	0.0975	0.0845	0.09
93%	30%	1,950	1993	0.1345	0.1225	0.13
124%	40%	2,600	2557	0.161	0.148	0.15
155%	50%	3,250	3165	0.197	0.1845	0.19
186%	60%	3,900	3987	0.2685	0.2545	0.26
217%	70%	4,550	4573	0.3	0.2855	0.29
248%	80%	5,200	5290	0.404	0.39	0.40
279%	90%	5,850	5900	0.457	0.448	0.45
310%	100%	6,500	6547	0.503	0.495	0.50
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			8609	1.05	1.1205	1.09
0%	0%	0	0	0.4365	0.512	0.47

Load at 1.0-in lateral deflection: 8,309 lb

Residual lateral deformation: 0.47 in



Notes:

Failure reached at 8,609 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-9A

Axial Uplift Load Test Results

PTL - **9**
Section: W6x9
Latitude: 33.662774
Longitude: -114.70367
Tested By: ANS
Test Date: 10/19/2025

Designation: **A**

Target Test Load (lb): **8,000** (≈ 67970% Design Load)
Installation Method: Direct Drive (DD)
- -
Target Embedment Depth (ft): 5.00
Actual Embedment Depth (ft): 5

Saturated Soil? No
Drive Time (s): 9.51
Pile Damaged? No

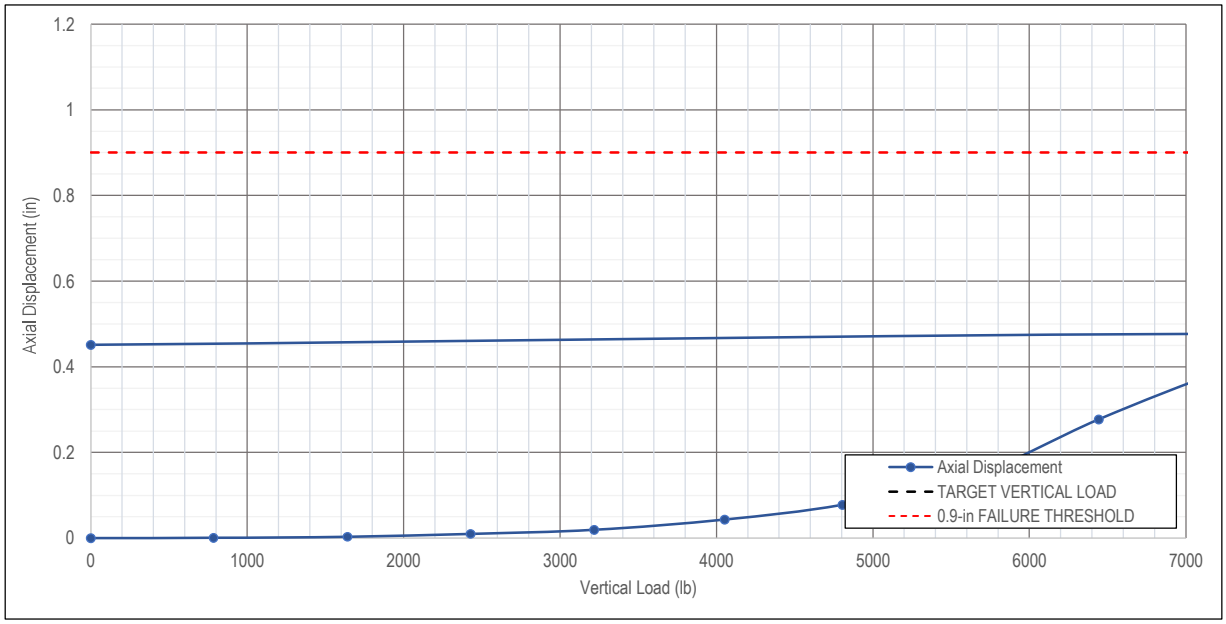
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	785	0.0005	0.001	0.00
13594%	20%	1,600	1641	0.003	0.0035	0.00
20391%	30%	2,400	2428	0.01	0.01	0.01
27188%	40%	3,200	3219	0.019	0.0195	0.02
33985%	50%	4,000	4053	0.0435	0.0435	0.04
40782%	60%	4,800	4805	0.0785	0.077	0.08
47579%	70%	5,600	5607	0.14	0.1385	0.14
54376%	80%	6,400	6443	0.278	0.2765	0.28
61173%	90%	7,200	7260	0.395	0.392	0.39
67970%	100%	8,000	7987	0.4755	0.473	0.47
0%	0%	0	0	0.4515	0.451	0.45

Load at 0.9-in axial displacement: N/A lb

Residual axial displacement: 0.45 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-9A

Lateral Load Test Results

PTL- **9** Designation: **A**

Section: W6x9

Latitude: 33.662774

Longitude: -114.70367

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 9.51

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

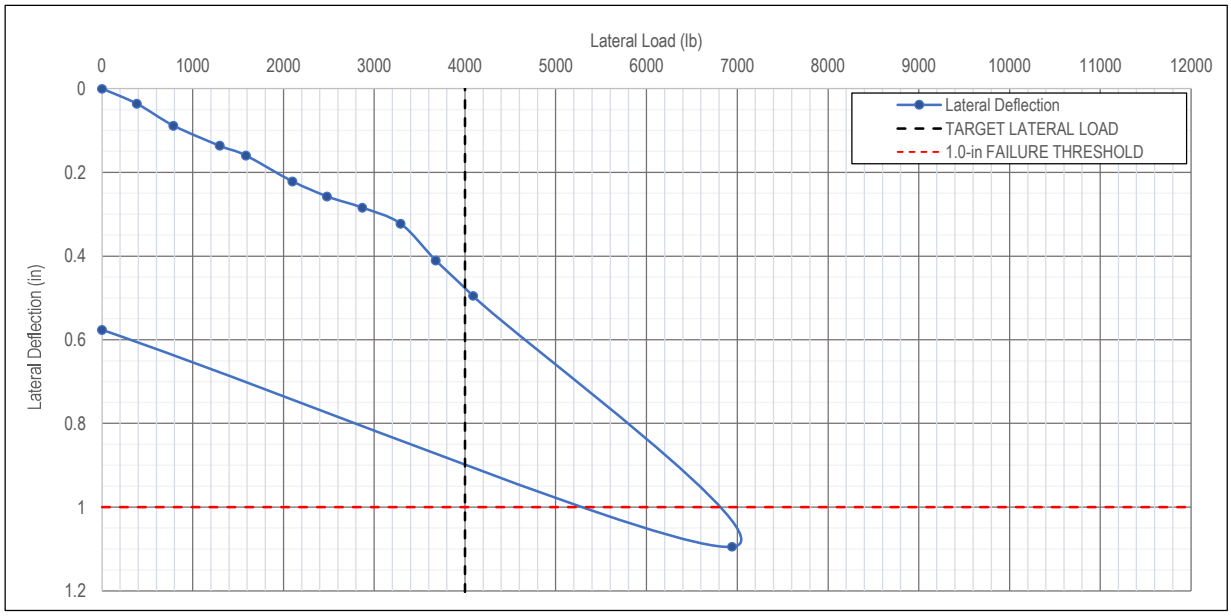
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	383	0.0345	0.037	0.04
62%	20%	800	789	0.087	0.09	0.09
93%	30%	1,200	1297	0.1345	0.138	0.14
124%	40%	1,600	1586	0.1585	0.1605	0.16
155%	50%	2,000	2100	0.2225	0.2205	0.22
186%	60%	2,400	2480	0.259	0.256	0.26
217%	70%	2,800	2869	0.2865	0.282	0.28
248%	80%	3,200	3290	0.325	0.3205	0.32
279%	90%	3,600	3678	0.4119	0.4095	0.41
310%	100%	4,000	4090	0.5	0.49	0.50
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6943	1.1865	1.003	1.09
0%	0%	0	0	0.517	0.6355	0.58

Load at 1.0-in lateral deflection: **6,492** **lb**

Residual lateral deformation: **0.58** **in**



Notes:

Failure reached at 6,943 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-9B

Axial Uplift Load Test Results

PTL - **9** Designation: **B**

Section: W6x9

Latitude: 33.662774

Longitude: -114.70367

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 19.53

Pile Damaged? No

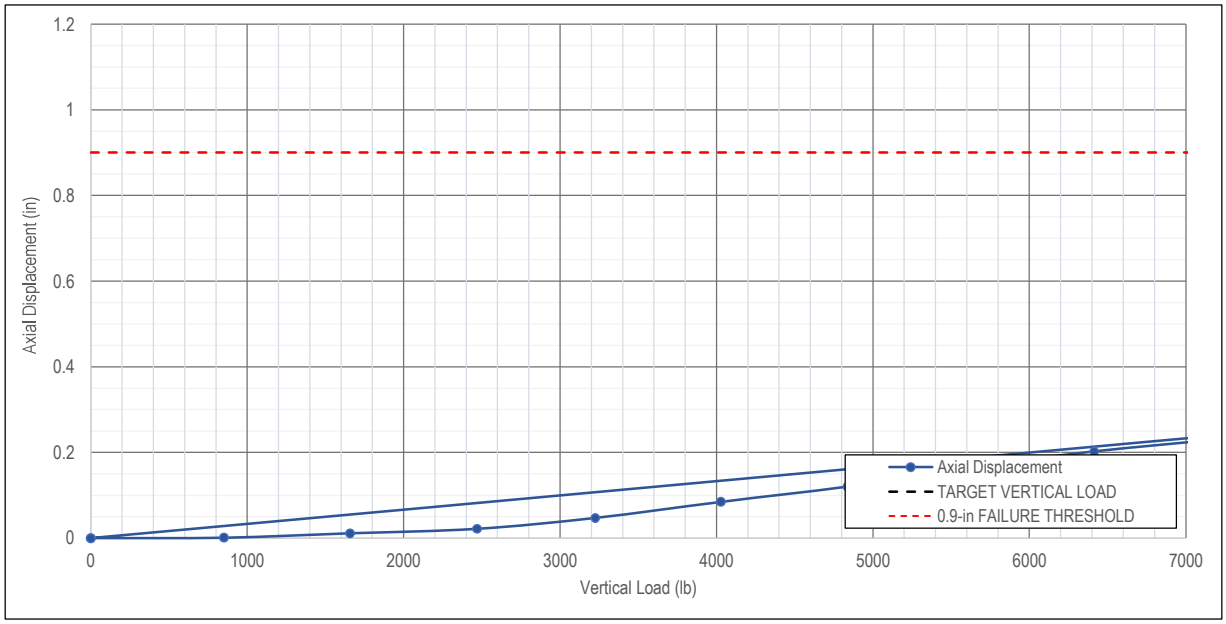
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	851	0.001	0.0015	0.00
13594%	20%	1,600	1658	0.011	0.0115	0.01
20391%	30%	2,400	2470	0.022	0.022	0.02
27188%	40%	3,200	3225	0.047	0.047	0.05
33985%	50%	4,000	4028	0.0855	0.084	0.08
40782%	60%	4,800	4838	0.1215	0.1195	0.12
47579%	70%	5,600	5651	0.1685	0.166	0.17
54376%	80%	6,400	6414	0.2035	0.201	0.20
61173%	90%	7,200	7219	0.2335	0.23	0.23
67970%	100%	8,000	8009	0.2675	0.264	0.27
0%	0%	0	0			

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **TBD** **in**



Notes:

Residual deflection not recorded.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-9B

Lateral Load Test Results

PTL- **9** Designation: **B**

Section: W6x9

Latitude: 33.662774

Longitude: -114.70367

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 19.53

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

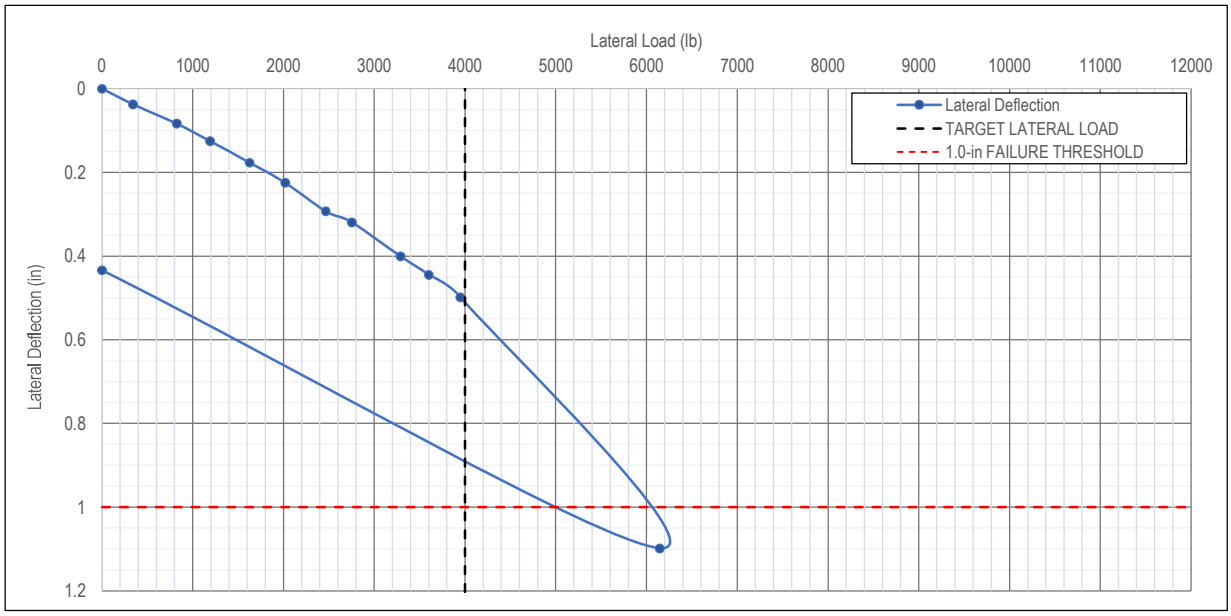
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	342	0.037	0.038	0.04
62%	20%	800	825	0.0825	0.0845	0.08
93%	30%	1,200	1191	0.1235	0.1265	0.13
124%	40%	1,600	1627	0.175	0.1785	0.18
155%	50%	2,000	2020	0.2235	0.2265	0.23
186%	60%	2,400	2468	0.291	0.295	0.29
217%	70%	2,800	2754	0.3175	0.3215	0.32
248%	80%	3,200	3290	0.399	0.4025	0.40
279%	90%	3,600	3603	0.4435	0.4455	0.44
310%	100%	4,000	3950	0.498	0.499	0.50
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6147	1.112	1.0845	1.10
0%	0%	0	0	0.449	0.419	0.43

Load at 1.0-in lateral deflection: **5,787** **lb**

Residual lateral deformation: **0.43** **in**



Notes:

Failure reached at 6,147 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-10A

Axial Uplift Load Test Results

PTL - **10** Designation: **A**

Section: W6x9

Latitude: 33.662461

Longitude: -114.6993

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 14.57

Pile Damaged? No

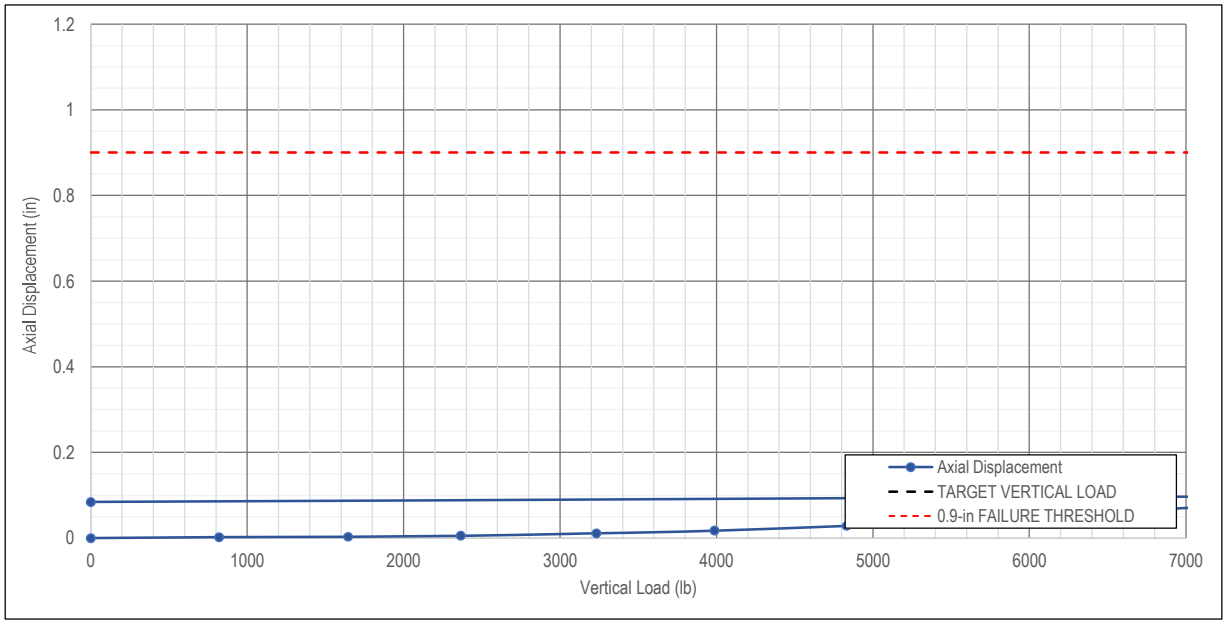
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	820	0.002	0.0025	0.00
436%	20%	1,600	1645	0.003	0.0035	0.00
654%	30%	2,400	2365	0.0055	0.006	0.01
872%	40%	3,200	3233	0.011	0.0115	0.01
1090%	50%	4,000	3987	0.0175	0.0175	0.02
1308%	60%	4,800	4833	0.029	0.0285	0.03
1526%	70%	5,600	5645	0.0435	0.0425	0.04
1744%	80%	6,400	6430	0.06	0.0585	0.06
1962%	90%	7,200	7224	0.0765	0.074	0.08
2180%	100%	8,000	8070	0.099	0.0955	0.10
0%	0%	0	0	0.0845	0.084	0.08

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.08** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-10A

Lateral Load Test Results

PTL- **10** Designation: **A**

Section: W6x9

Latitude: 33.662461

Longitude: -114.6993

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 14.57

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

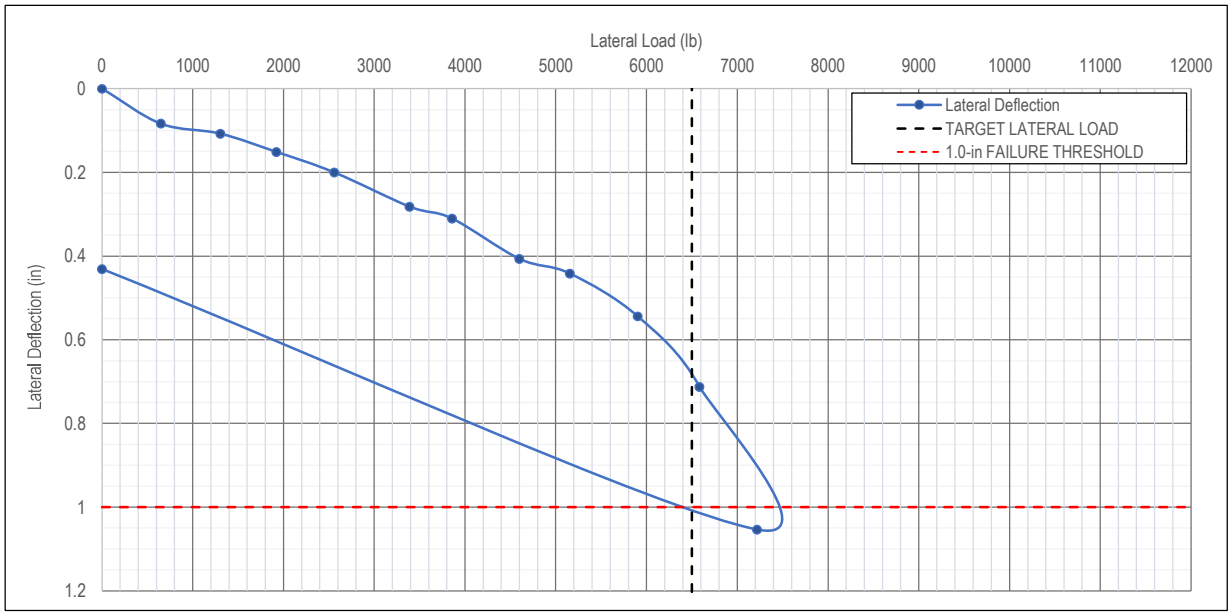
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.0895	0.0775	0.08
62%	20%	1,300	1305	0.112	0.103	0.11
93%	30%	1,950	1923	0.157	0.1455	0.15
124%	40%	2,600	2559	0.2065	0.1935	0.20
155%	50%	3,250	3390	0.289	0.275	0.28
186%	60%	3,900	3859	0.318	0.303	0.31
217%	70%	4,550	4600	0.4155	0.3975	0.41
248%	80%	5,200	5158	0.451	0.4325	0.44
279%	90%	5,850	5905	0.553	0.535	0.54
310%	100%	6,500	6584	0.713	0.712	0.71
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7219	1.0435	1.064	1.05
0%	0%	0	0	0.378	0.4845	0.43

Load at 1.0-in lateral deflection: **7,119** **lb**

Residual lateral deformation: **0.43** **in**



Notes:

Failure reached at 7,219 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-10B

Axial Uplift Load Test Results

PTL - **10** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.662461 - - Saturated Soil? No

Longitude: -114.6993 Target Embedment Depth (ft): 8.00 Drive Time (s): 28.58

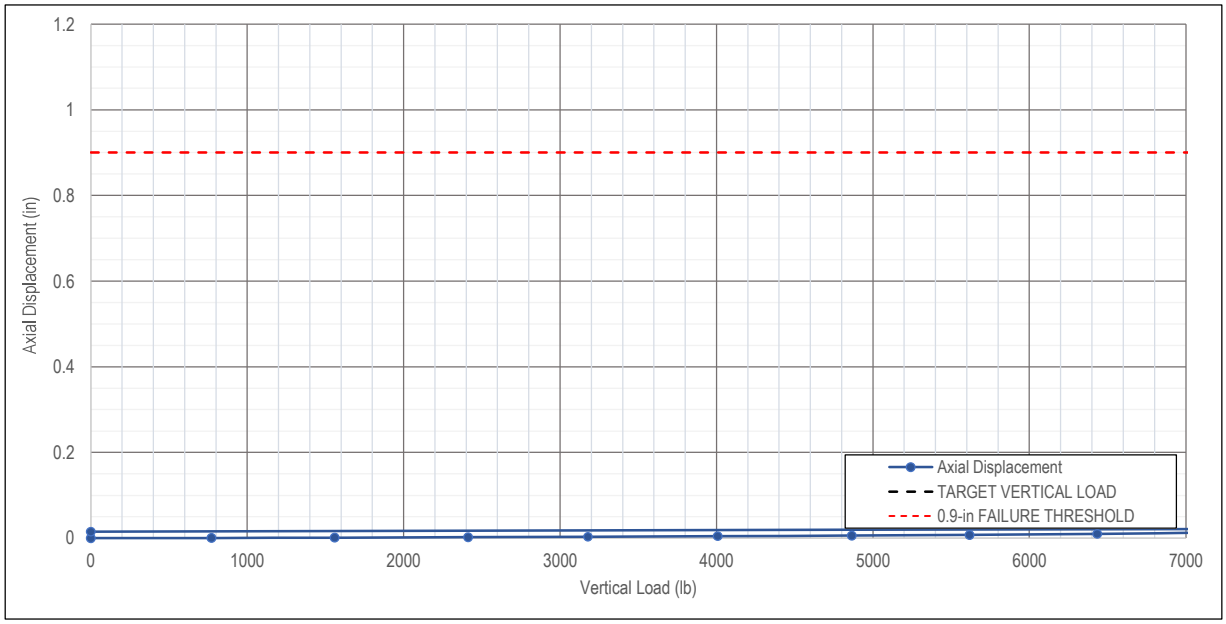
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/19/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	772	0.0005	0.0005	0.00
436%	20%	1,600	1560	0.0015	0.001	0.00
654%	30%	2,400	2412	0.002	0.0025	0.00
872%	40%	3,200	3178	0.003	0.0035	0.00
1090%	50%	4,000	4008	0.004	0.005	0.00
1308%	60%	4,800	4865	0.0055	0.0065	0.01
1526%	70%	5,600	5618	0.007	0.0085	0.01
1744%	80%	6,400	6434	0.0095	0.0105	0.01
1962%	90%	7,200	7219	0.0135	0.014	0.01
2180%	100%	8,000	8037	0.0215	0.0215	0.02
0%	0%	0	0	0.016	0.014	0.02

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.02** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-10B

Lateral Load Test Results

PTL- **10** Designation: **B**

Section: W6x9

Latitude: 33.662461

Longitude: -114.6993

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 28.58

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

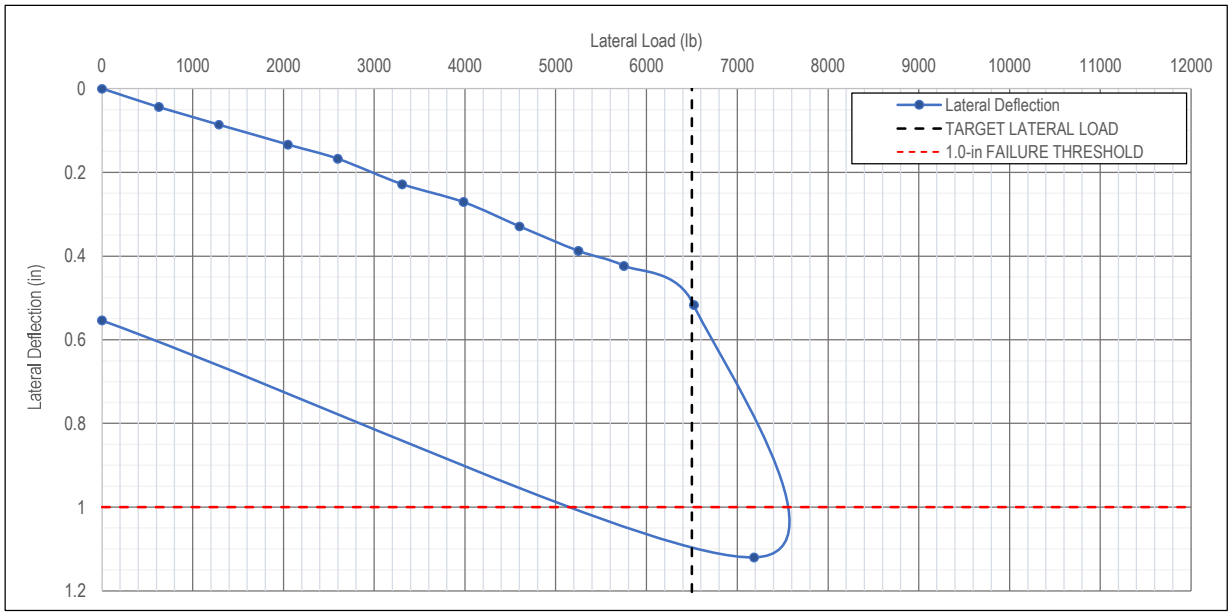
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	627	0.0465	0.041	0.04
62%	20%	1,300	1289	0.091	0.0805	0.09
93%	30%	1,950	2050	0.1405	0.127	0.13
124%	40%	2,600	2597	0.175	0.16	0.17
155%	50%	3,250	3309	0.236	0.2205	0.23
186%	60%	3,900	3987	0.279	0.263	0.27
217%	70%	4,550	4603	0.3365	0.3215	0.33
248%	80%	5,200	5249	0.3935	0.3815	0.39
279%	90%	5,850	5754	0.429	0.418	0.42
310%	100%	6,500	6522	0.5165	0.518	0.52
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7187	1.095	1.145	1.12
0%	0%	0	0	0.4565	0.651	0.55

Load at 1.0-in lateral deflection: **7,055** **lb**

Residual lateral deformation: **0.55** **in**



Notes:

Failure reached at 7,187 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-11A

Axial Uplift Load Test Results

PTL - **11** Designation: **A**

Section: W6x9

Latitude: 33.664536

Longitude: -114.69674

Tested By: ANS

Test Date: 10/18/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 10.34

Pile Damaged? No

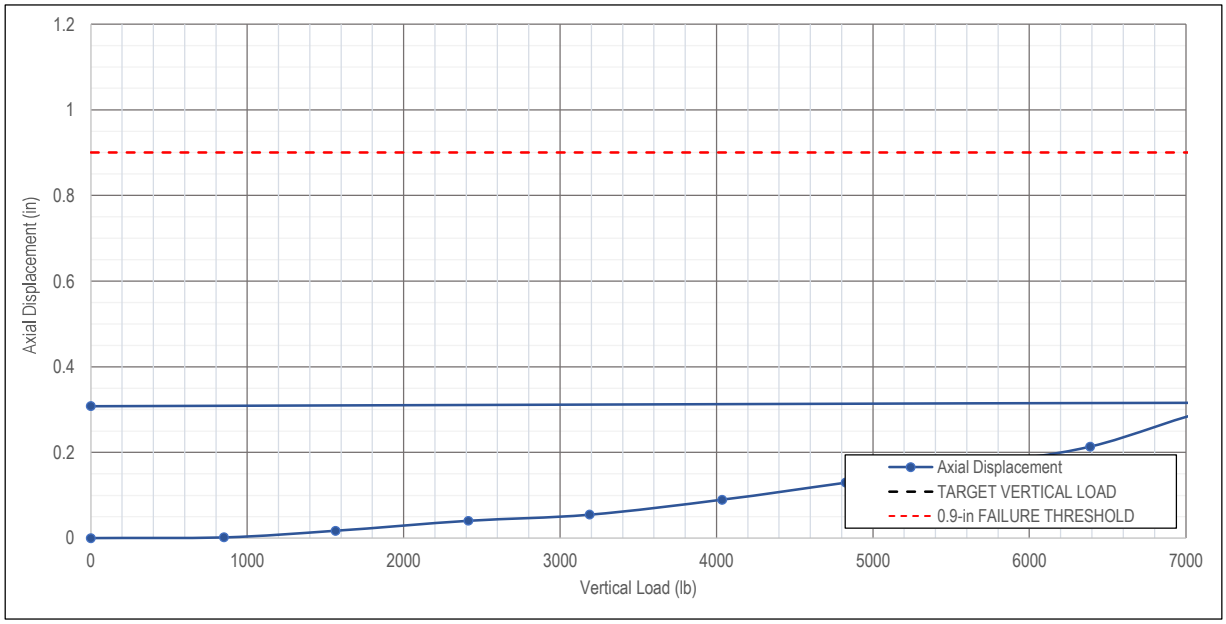
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	851	0.002	0.002	0.00
13594%	20%	1,600	1565	0.0175	0.017	0.02
20391%	30%	2,400	2414	0.0415	0.0395	0.04
27188%	40%	3,200	3188	0.053	0.057	0.06
33985%	50%	4,000	4035	0.0915	0.0875	0.09
40782%	60%	4,800	4825	0.1325	0.127	0.13
47579%	70%	5,600	5670	0.177	0.1695	0.17
54376%	80%	6,400	6390	0.2175	0.21	0.21
61173%	90%	7,200	7245	0.312	0.299	0.31
67970%	100%	8,000	7975	0.32	0.3125	0.32
0%	0%	0	0	0.3105	0.3055	0.31

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.31** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-11A

Lateral Load Test Results

PTL- **11** Designation: **A**

Section: W6x9

Latitude: 33.664536

Longitude: -114.69674

Tested By: ANS

Test Date: 10/18/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 10.34

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

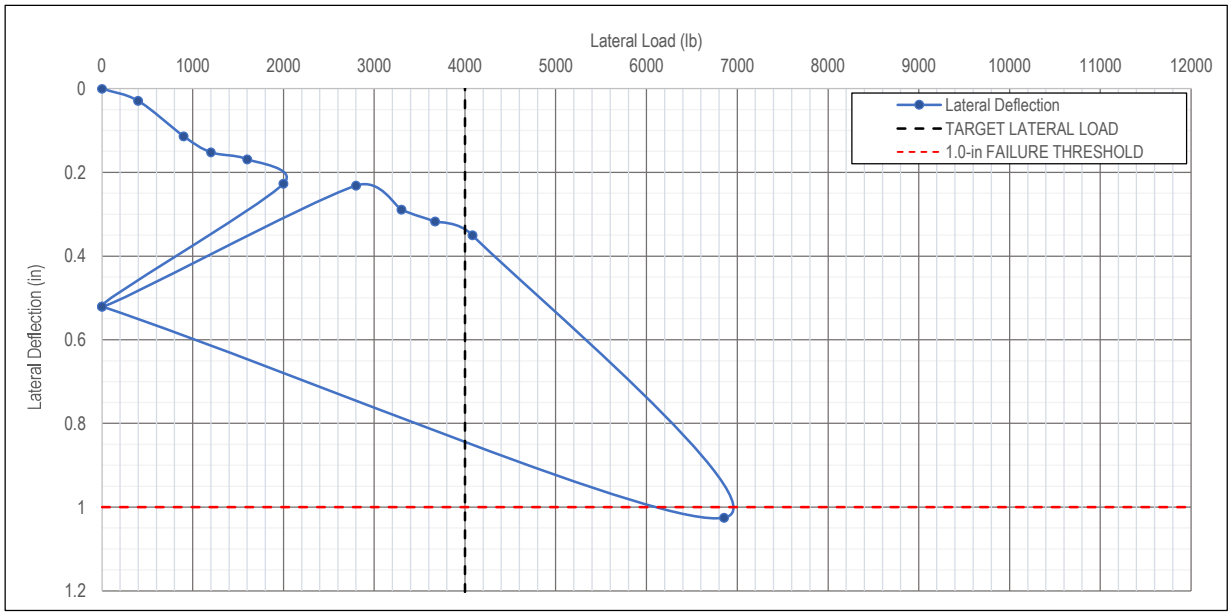
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	397	0.0325	0.0255	0.03
62%	20%	800	900	0.1205	0.1065	0.11
93%	30%	1,200	1200	0.16	0.1435	0.15
124%	40%	1,600	1600	0.1775	0.16	0.17
155%	50%	2,000	2000	0.236	0.2175	0.23
186%	60%	2,400				
217%	70%	2,800	2800	0.2415	0.2215	0.23
248%	80%	3,200	3300	0.299	0.2785	0.29
279%	90%	3,600	3673	0.3275	0.3075	0.32
310%	100%	4,000	4083	0.3605	0.34	0.35
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6857	1.0365	1.0145	1.03
0%	0%	0	0	0.5315	0.51	0.52

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.52** **in**



Notes:

Failure reached at 6,857 lbs. 2,400-lbs load step was skipped.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-11B

Axial Uplift Load Test Results

PTL - **11** Designation: **B**

Section: W6x9

Latitude: 33.664536

Longitude: -114.69674

Tested By: ANS

Test Date: 10/18/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 18.17

Pile Damaged? No

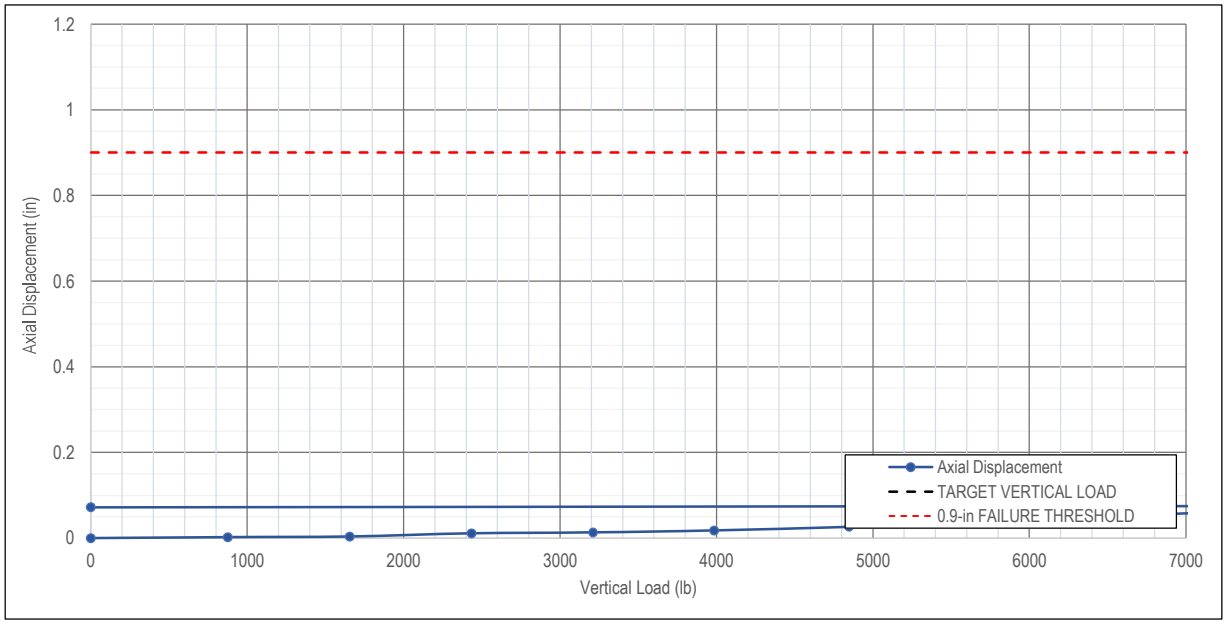
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	876	0.002	0.003	0.00
13594%	20%	1,600	1656	0.0035	0.004	0.00
20391%	30%	2,400	2435	0.01	0.0125	0.01
27188%	40%	3,200	3211	0.012	0.0145	0.01
33985%	50%	4,000	3985	0.0165	0.02	0.02
40782%	60%	4,800	4847	0.0245	0.0285	0.03
47579%	70%	5,600	5570	0.0335	0.037	0.04
54376%	80%	6,400	6470	0.0485	0.052	0.05
61173%	90%	7,200	7260	0.06	0.0635	0.06
67970%	100%	8,000	8027	0.0735	0.075	0.07
0%	0%	0	0	0.0735	0.071	0.07

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.07** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-11B

Lateral Load Test Results

PTL- **11** Designation: **B**

Section: W6x9

Latitude: 33.664536

Longitude: -114.69674

Tested By: ANS

Test Date: 10/18/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 18.17

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

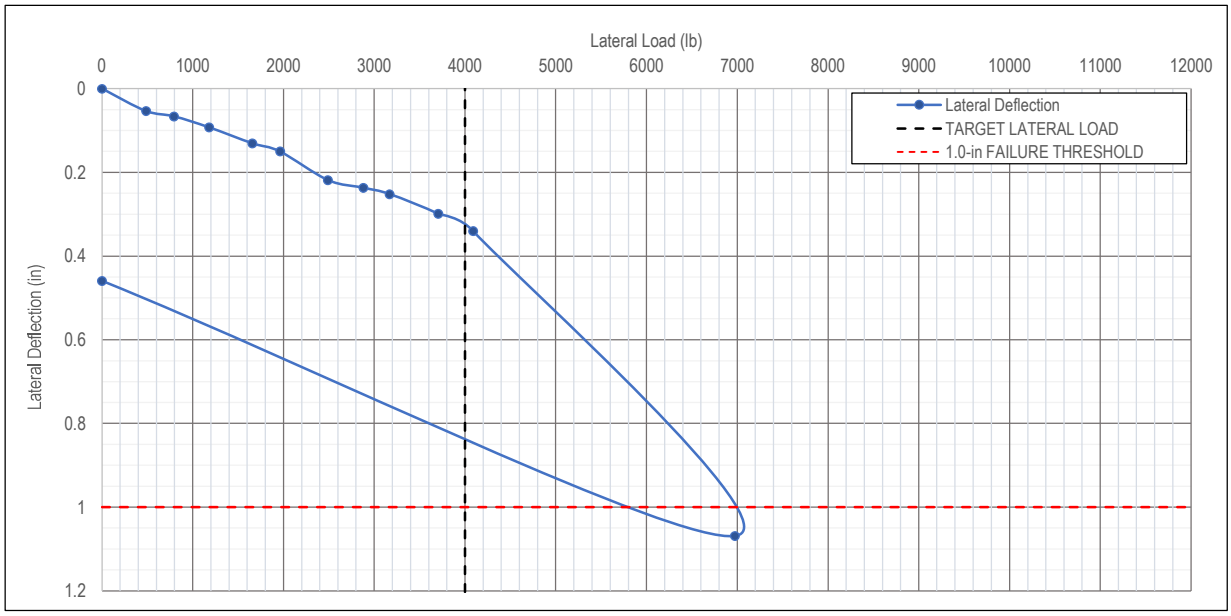
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	485	0.0555	0.051	0.05
62%	20%	800	794	0.069	0.063	0.07
93%	30%	1,200	1180	0.0955	0.0885	0.09
124%	40%	1,600	1658	0.135	0.1265	0.13
155%	50%	2,000	1963	0.1565	0.1435	0.15
186%	60%	2,400	2490	0.224	0.213	0.22
217%	70%	2,800	2880	0.242	0.231	0.24
248%	80%	3,200	3170	0.2575	0.2465	0.25
279%	90%	3,600	3705	0.304	0.293	0.30
310%	100%	4,000	4093	0.3465	0.3345	0.34
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6976	1.0565	1.0805	1.07
0%	0%	0	0	0.3875	0.5315	0.46

Load at 1.0-in lateral deflection: **6,705** **lb**

Residual lateral deformation: **0.46** **in**



Notes:

Failure reached at 6,976 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-12A

Axial Uplift Load Test Results

PTL - **12**
Section: W6x9
Latitude: 33.662776
Longitude: -114.69425
Tested By: ANS
Test Date: 10/18/2025

Designation: **A**

Target Test Load (lb): **8,000** (≈ 2180% Design Load)
Installation Method: Direct Drive (DD)
- -
Target Embedment Depth (ft): 6.00
Actual Embedment Depth (ft): 6

Saturated Soil? No
Drive Time (s): 13.86
Pile Damaged? No

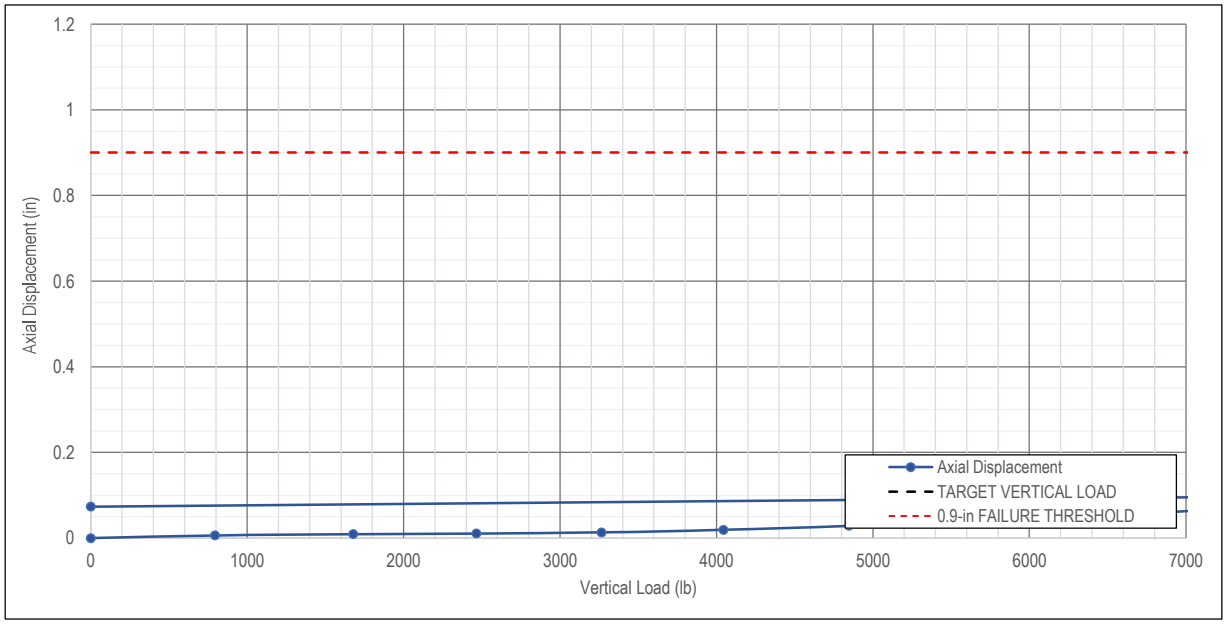
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	794	0.0065	0.0065	0.01
436%	20%	1,600	1678	0.008	0.0105	0.01
654%	30%	2,400	2465	0.01	0.0115	0.01
872%	40%	3,200	3266	0.012	0.0145	0.01
1090%	50%	4,000	4045	0.0175	0.0215	0.02
1308%	60%	4,800	4847	0.027	0.0305	0.03
1526%	70%	5,600	5626	0.0395	0.043	0.04
1744%	80%	6,400	6440	0.051	0.0545	0.05
1962%	90%	7,200	7277	0.0675	0.0715	0.07
2180%	100%	8,000	8035	0.095	0.0985	0.10
0%	0%	0	0	0.074	0.073	0.07

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.07** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-12A

Lateral Load Test Results

PTL- **12** Designation: **A**

Section: W6x9

Latitude: 33.662776

Longitude: -114.69425

Tested By: ANS

Test Date: 10/18/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 13.86

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

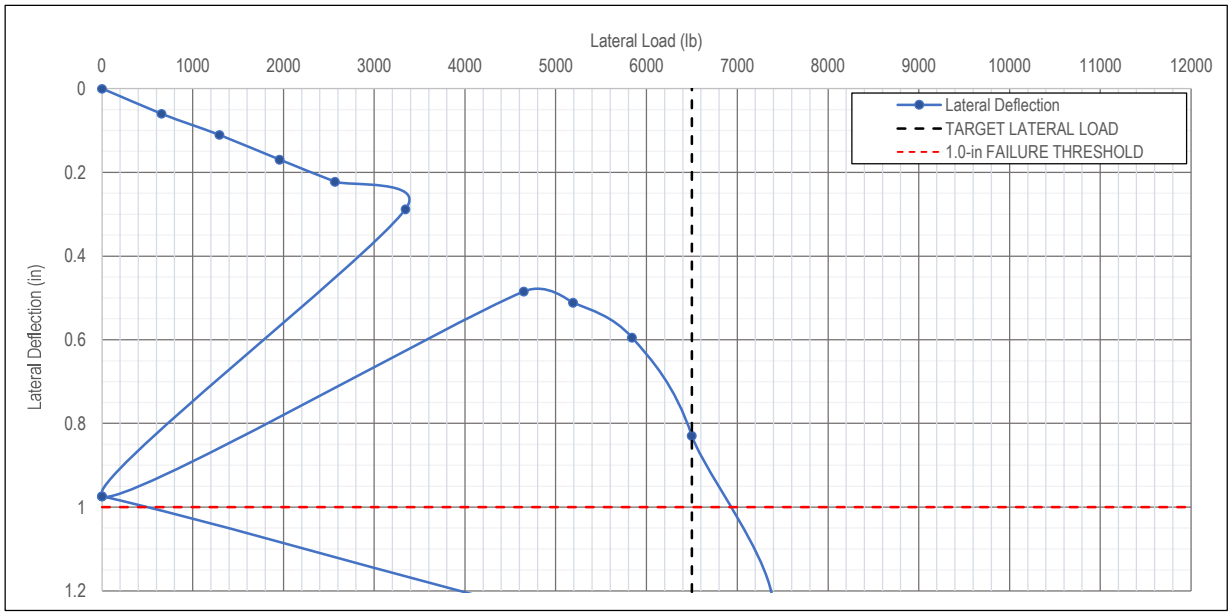
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	657	0.058	0.0615	0.06
62%	20%	1,300	1295	0.1065	0.1145	0.11
93%	30%	1,950	1958	0.163	0.1765	0.17
124%	40%	2,600	2568	0.215	0.2295	0.22
155%	50%	3,250	3345	0.279	0.2965	0.29
186%	60%	3,900				
217%	70%	4,550	4650	0.4735	0.496	0.48
248%	80%	5,200	5192	0.4975	0.525	0.51
279%	90%	5,850	5840	0.572	0.618	0.60
310%	100%	6,500	6500	0.766	0.893	0.83
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6987	1.3385	1.309	1.32
0%	0%	0	0	0.965	0.983	0.97

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.97** **in**



Notes:

Failure reached at 6,987 lbs. 3,900-lbs load step was skipped.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-12B

Axial Uplift Load Test Results

PTL - **12** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.662776 - - Saturated Soil? No

Longitude: -114.69425 Target Embedment Depth (ft): 8.00 Drive Time (s): 22.65

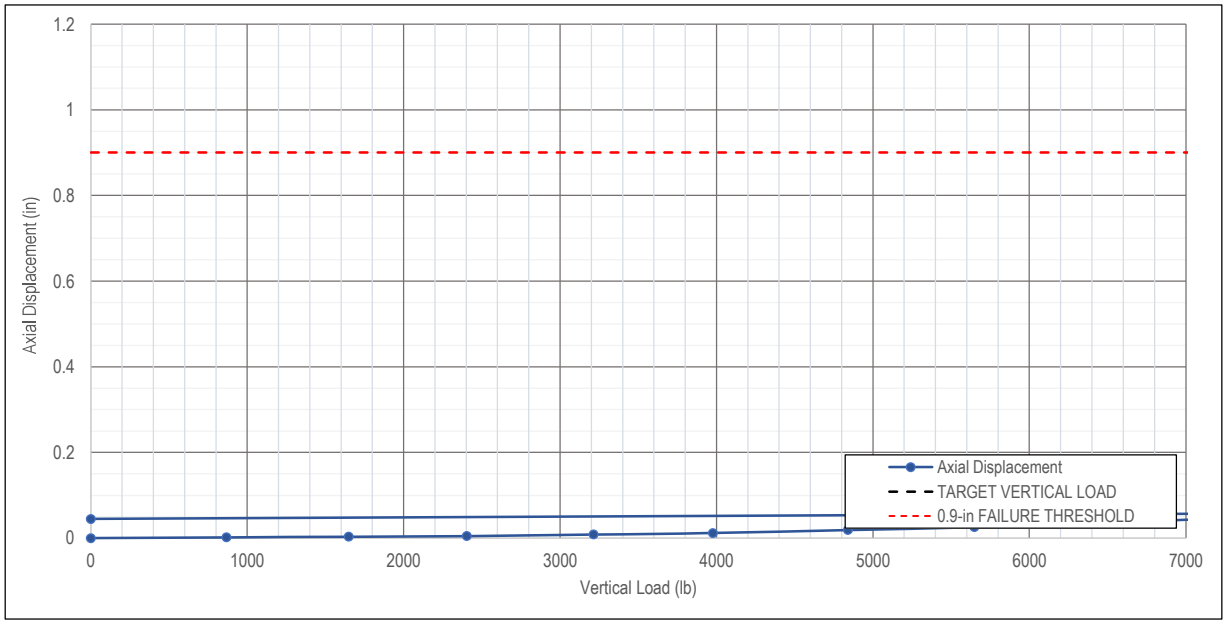
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/18/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	867	0.0015	0.002	0.00
436%	20%	1,600	1649	0.003	0.004	0.00
654%	30%	2,400	2403	0.0045	0.0055	0.01
872%	40%	3,200	3215	0.008	0.0085	0.01
1090%	50%	4,000	3976	0.012	0.012	0.01
1308%	60%	4,800	4839	0.019	0.019	0.02
1526%	70%	5,600	5649	0.0255	0.0255	0.03
1744%	80%	6,400	6472	0.0365	0.036	0.04
1962%	90%	7,200	7215	0.046	0.045	0.05
2180%	100%	8,000	8004	0.0585	0.0565	0.06
0%	0%	0	0	0.0455	0.0445	0.05

Load at 0.9-in axial displacement: N/A lb **Residual axial displacement:** 0.05 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-12B

Lateral Load Test Results

PTL- **12** Designation: **B**

Section: W6x9

Latitude: 33.662776

Longitude: -114.69425

Tested By: ANS

Test Date: 10/18/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 22.65

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

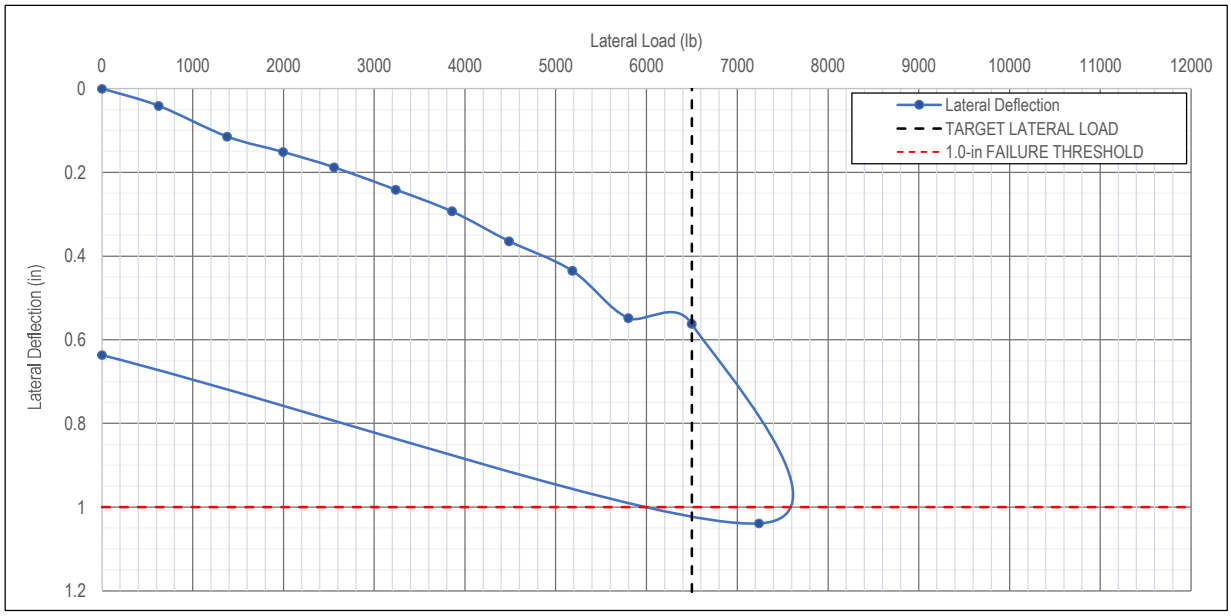
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	625	0.038	0.0435	0.04
62%	20%	1,300	1380	0.11	0.1195	0.11
93%	30%	1,950	1995	0.145	0.157	0.15
124%	40%	2,600	2557	0.181	0.1945	0.19
155%	50%	3,250	3237	0.2325	0.25	0.24
186%	60%	3,900	3857	0.2815	0.3045	0.29
217%	70%	4,550	4487	0.3475	0.381	0.36
248%	80%	5,200	5185	0.413	0.458	0.44
279%	90%	5,850	5800	0.503	0.5935	0.55
310%	100%	6,500	6500	0.514	0.6105	0.56
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7240	1.033	1.0445	1.04
0%	0%	0	0	0.37	0.903	0.64

Load at 1.0-in lateral deflection: **7,180** **lb**

Residual lateral deformation: **0.64** **in**



Notes:

Failure reached at 7,240 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-12D

Axial Uplift Load Test Results

PTL - **12** Designation: **D**

Section: **W6x12**

Latitude: 33.662776

Longitude: -114.69425

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 24.62

Pile Damaged? No

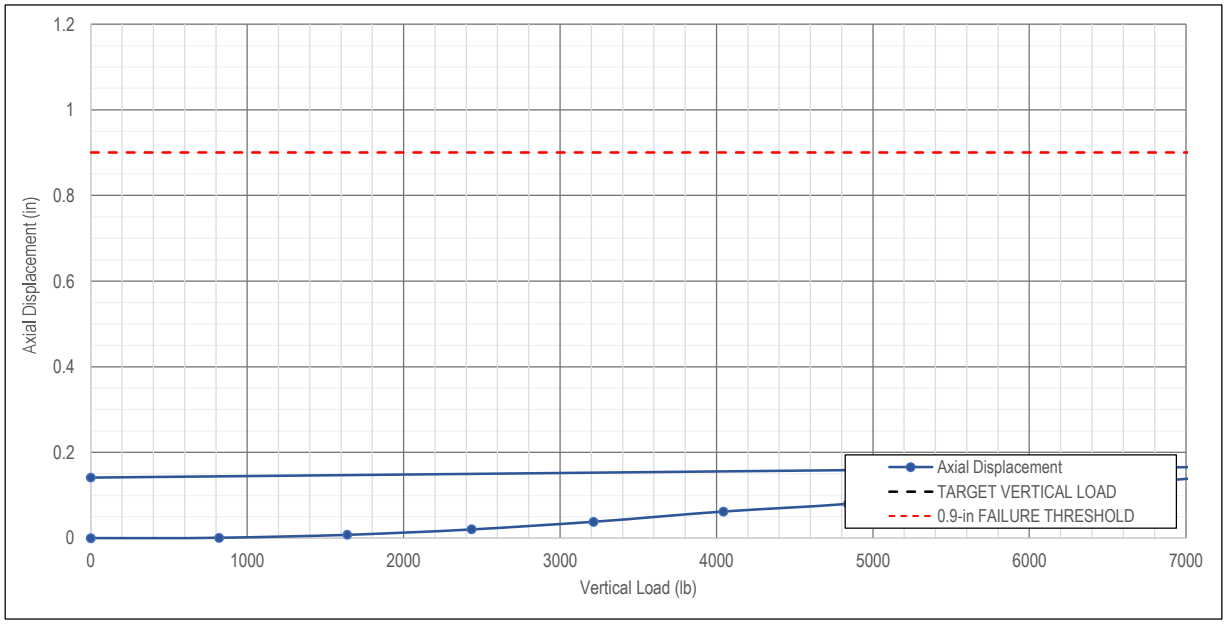
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	820	0.0005	0.001	0.00
436%	20%	1,600	1640	0.0075	0.0085	0.01
654%	30%	2,400	2435	0.02	0.021	0.02
872%	40%	3,200	3215	0.038	0.0385	0.04
1090%	50%	4,000	4045	0.062	0.0625	0.06
1308%	60%	4,800	4840	0.0795	0.08	0.08
1526%	70%	5,600	5615	0.1055	0.106	0.11
1744%	80%	6,400	6391	0.122	0.1225	0.12
1962%	90%	7,200	7175	0.143	0.1435	0.14
2180%	100%	8,000	7995	0.1675	0.1675	0.17
0%	0%	0	0	0.1415	0.1415	0.14

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.14** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-12D

Lateral Load Test Results

PTL- **12** Designation: **D**

Section: **W6x12**

Latitude: 33.662776

Longitude: -114.69425

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 24.62

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

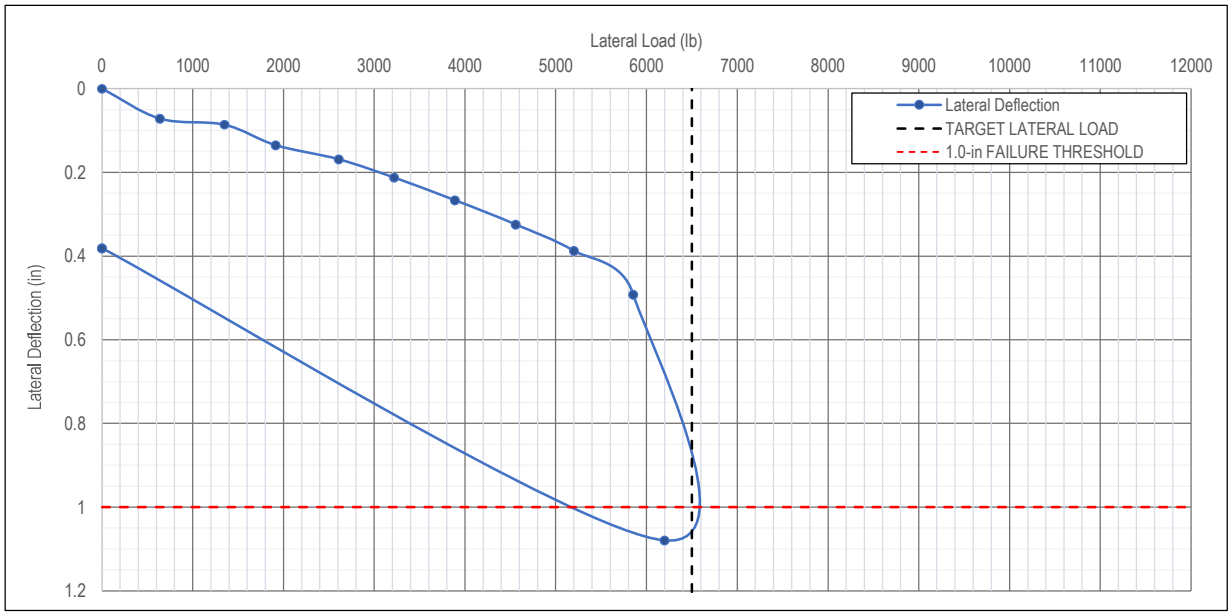
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	640	0.0715	0.0715	0.07
62%	20%	1,300	1350	0.086	0.086	0.09
93%	30%	1,950	1915	0.136	0.1345	0.14
124%	40%	2,600	2610	0.1695	0.1675	0.17
155%	50%	3,250	3220	0.214	0.211	0.21
186%	60%	3,900	3890	0.2685	0.264	0.27
217%	70%	4,550	4560	0.328	0.321	0.32
248%	80%	5,200	5200	0.393	0.3825	0.39
279%	90%	5,850	5855	0.5035	0.4805	0.49
310%	100%	6,500	6200	1.1155	1.0435	1.08
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.7045	0.05875	0.38

Load at 1.0-in lateral deflection: **6,153** **lb**

Residual lateral deformation: **0.38** **in**



Notes:

Failure reached at 6,200 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-13A

Axial Uplift Load Test Results

PTL - **13** Designation: **A**

Section: W6x9

Latitude: 33.663045

Longitude: -114.69084

Tested By: ANS

Test Date: 10/18/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 4.26

Pile Damaged? No

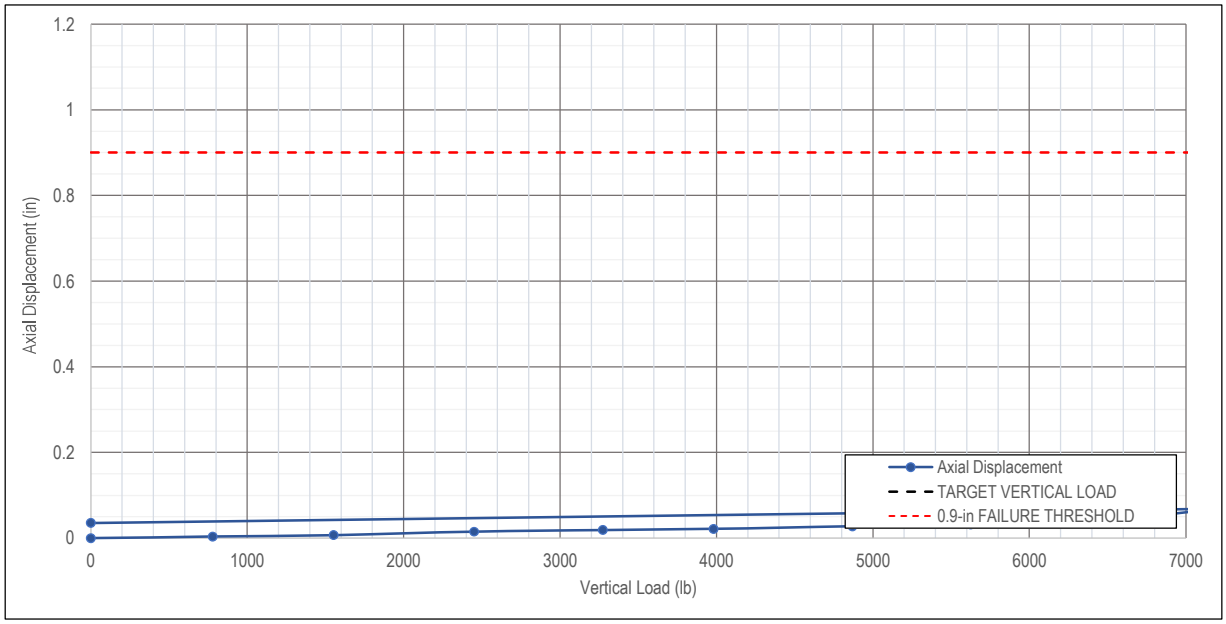
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	780	0.0035	0.004	0.00
13594%	20%	1,600	1552	0.007	0.007	0.01
20391%	30%	2,400	2450	0.0155	0.015	0.02
27188%	40%	3,200	3273	0.0195	0.0185	0.02
33985%	50%	4,000	3982	0.022	0.021	0.02
40782%	60%	4,800	4870	0.0285	0.027	0.03
47579%	70%	5,600	5625	0.033	0.031	0.03
54376%	80%	6,400	6390	0.0375	0.0345	0.04
61173%	90%	7,200	7240	0.07	0.066	0.07
67970%	100%	8,000	8100	0.0745	0.0705	0.07
0%	0%	0	0	0.036	0.035	0.04

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.04** **in**



Notes:

Used "EVEN" target loads.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-13A

Lateral Load Test Results

PTL- **13** Designation: **A**

Section: W6x9

Latitude: 33.663045

Longitude: -114.69084

Tested By: ANS

Test Date: 10/18/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 4.26

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

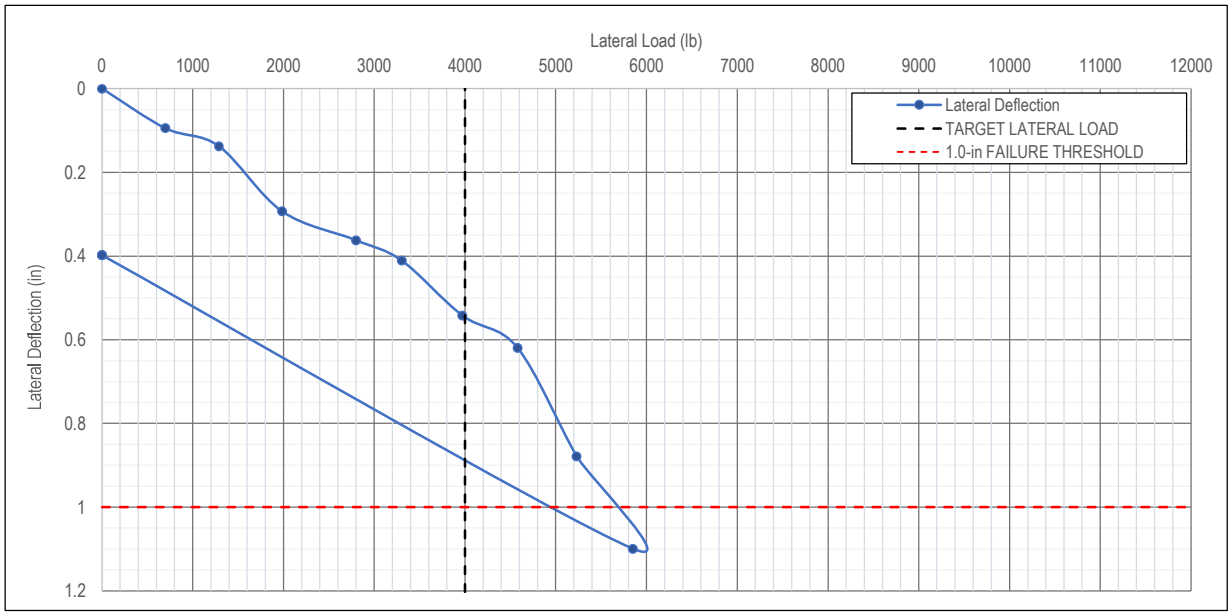
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	700	0.086	0.1025	0.09
62%	20%	800	1288	0.1235	0.1515	0.14
93%	30%	1,200	1985	0.3265	0.2595	0.29
124%	40%	1,600	2800	0.3435	0.381	0.36
155%	50%	2,000	3305	0.3915	0.4305	0.41
186%	60%	2,400	3970	0.5215	0.5625	0.54
217%	70%	2,800	4580	0.5985	0.6405	0.62
248%	80%	3,200	5230	0.851	0.9065	0.88
279%	90%	3,600	5850	1.1655	1.0335	1.10
310%	100%	4,000				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.375	0.4205	0.40

Load at 1.0-in lateral deflection: **5,571** **lb**

Residual lateral deformation: **0.40** **in**



Notes:

Used "EVEN" target loads. Failure reached at 5,850 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-13B

Axial Uplift Load Test Results

PTL - **13** Designation: **B**

Section: W6x9

Latitude: 33.663045

Longitude: -114.69084

Tested By: ANS

Test Date: 10/17/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 12.3

Pile Damaged? No

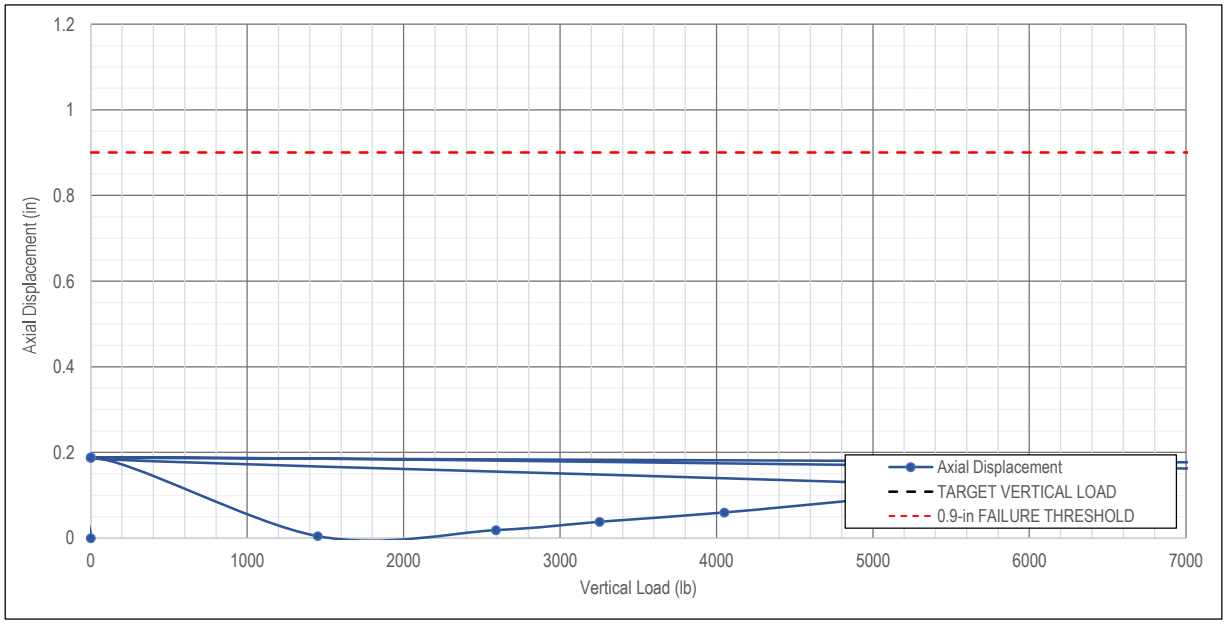
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800				
13594%	20%	1,600	1451	0.0045	0.0045	0.00
20391%	30%	2,400	2590	0.019	0.018	0.02
27188%	40%	3,200	3252	0.0395	0.0365	0.04
33985%	50%	4,000	4050	0.0615	0.058	0.06
40782%	60%	4,800	4859	0.09	0.085	0.09
47579%	70%	5,600	5709	0.122	0.116	0.12
54376%	80%	6,400				
61173%	90%	7,200	7182	0.167	0.158	0.16
67970%	100%	8,000	8543	0.1775	0.1695	0.17
0%	0%	0	0	0.158	0.2175	0.19

Load at 0.9-in axial displacement: **TBD** **lb**

Residual axial displacement: **0.19** **in**



Notes:

800-lbs and 6400-lbs load steps were skipped.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-13B

Lateral Load Test Results

PTL- **13** Designation: **B**

Section: W6x9

Latitude: 33.663045

Longitude: -114.69084

Tested By: ANS

Test Date: 10/17/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 12.3

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

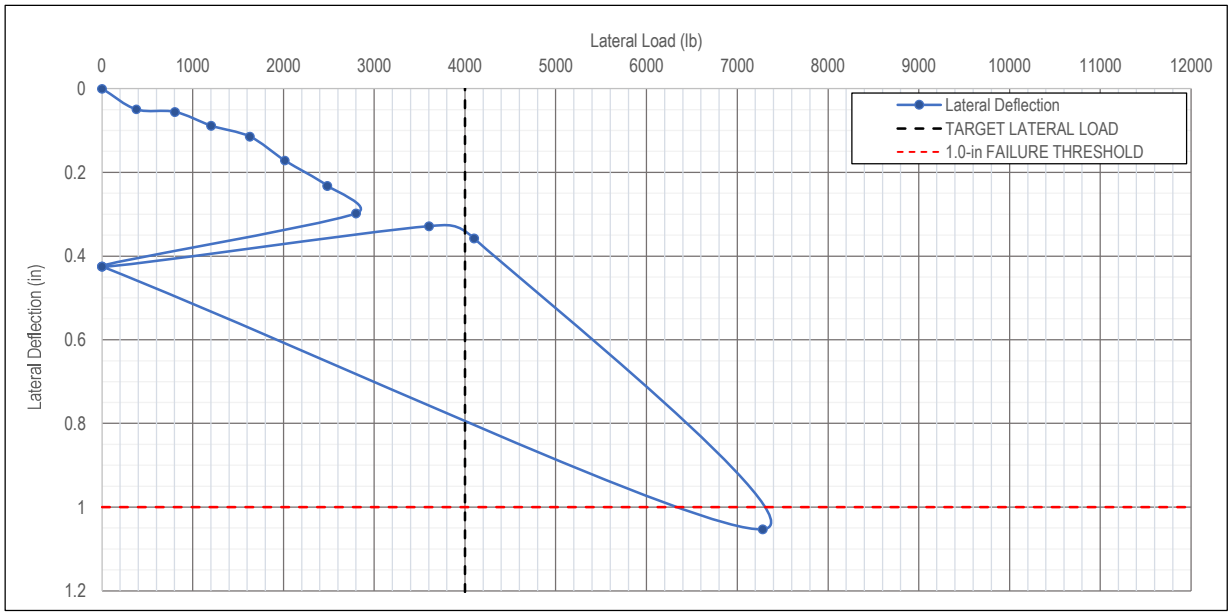
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	380	0.075	0.0235	0.05
62%	20%	800	805	0.049	0.0615	0.06
93%	30%	1,200	1203	0.081	0.0957	0.09
124%	40%	1,600	1630	0.1045	0.124	0.11
155%	50%	2,000	2015	0.158	0.185	0.17
186%	60%	2,400	2483	0.216	0.2485	0.23
217%	70%	2,800	2800	0.2805	0.315	0.30
248%	80%	3,200				
279%	90%	3,600	3605	0.31	0.347	0.33
310%	100%	4,000	4100	0.338	0.3765	0.36
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7280	1.0615	1.0445	1.05
0%	0%	0	0	0.4535	0.397	0.43

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.43** **in**



Notes:

Failure reached at 7,280 lbs. 3,200-lbs load step was skipped.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-14A

Axial Uplift Load Test Results

PTL - **14** Designation: **A** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.663937 - - Saturated Soil? No

Longitude: -114.68161 Target Embedment Depth (ft): 6.00 Drive Time (s): 10.39

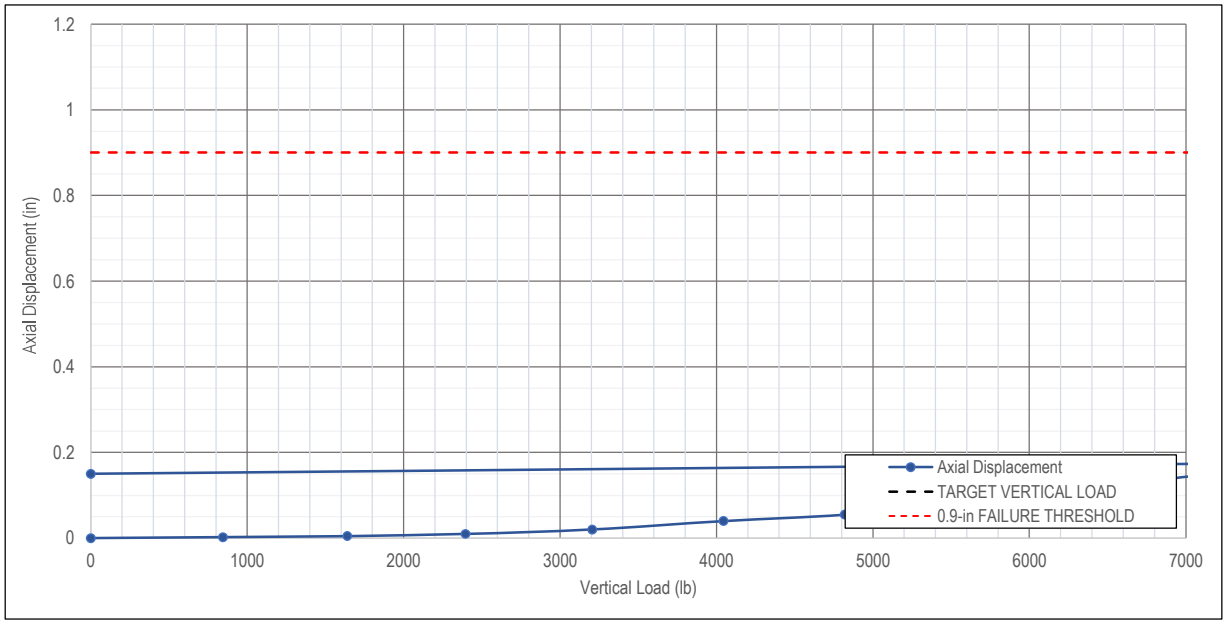
Tested By: ANS Actual Embedment Depth (ft): 6 Pile Damaged? No

Test Date: 10/23/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	845	0	0.005	0.00
436%	20%	1,600	1640	0.005	0.005	0.01
654%	30%	2,400	2395	0.01	0.01	0.01
872%	40%	3,200	3205	0.015	0.025	0.02
1090%	50%	4,000	4045	0.03	0.05	0.04
1308%	60%	4,800	4820	0.06	0.05	0.06
1526%	70%	5,600	5575	0.085	0.08	0.08
1744%	80%	6,400	6365	0.12	0.11	0.12
1962%	90%	7,200	7175	0.155	0.145	0.15
2180%	100%	8,000	7980	0.18	0.17	0.18
0%	0%	0	0	0.15	0.15	0.15

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.15** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-14A

Lateral Load Test Results

PTL- **14** Designation: **A**

Section: W6x9

Latitude: 33.663937

Longitude: -114.68161

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 10.39

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

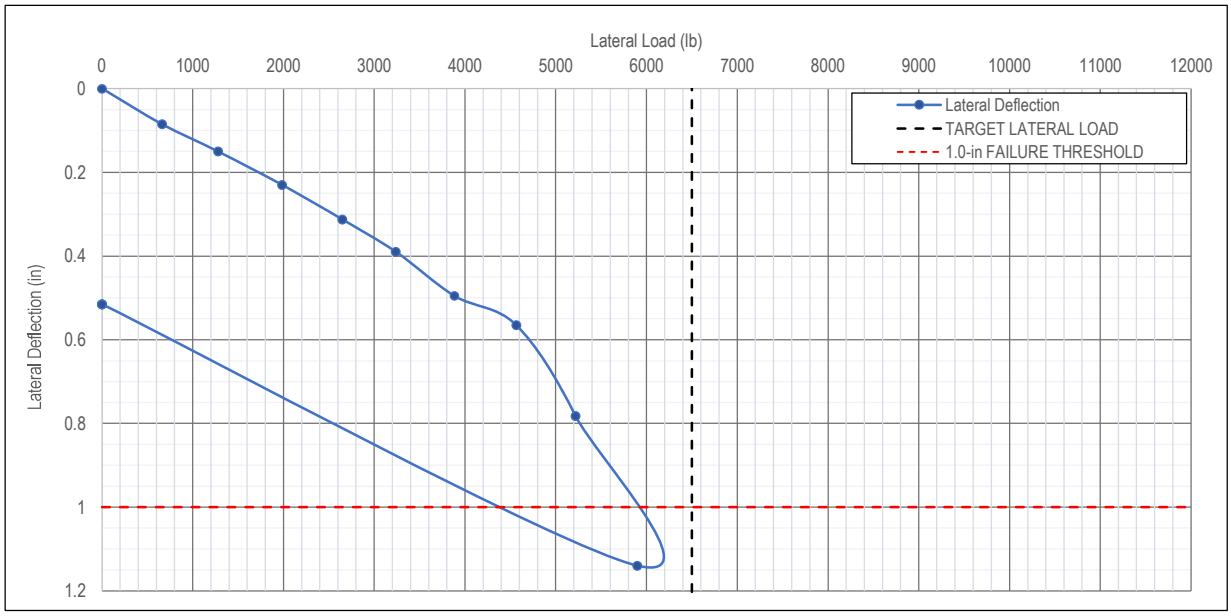
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	665	0.085	0.085	0.09
62%	20%	1,300	1280	0.15	0.15	0.15
93%	30%	1,950	1985	0.23	0.23	0.23
124%	40%	2,600	2650	0.315	0.31	0.31
155%	50%	3,250	3240	0.395	0.385	0.39
186%	60%	3,900	3885	0.5	0.49	0.50
217%	70%	4,550	4565	0.62	0.51	0.57
248%	80%	5,200	5220	0.79	0.775	0.78
279%	90%	5,850	5900	1.2	1.08	1.14
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.555	0.475	0.52

Load at 1.0-in lateral deflection: **5,634** **lb**

Residual lateral deformation: **0.52** **in**



Notes:

Failure reached at 5,900 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-14B

Axial Uplift Load Test Results

PTL - **14** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.663937 - - Saturated Soil? No

Longitude: -114.68161 Target Embedment Depth (ft): 8.00 Drive Time (s): 19.01

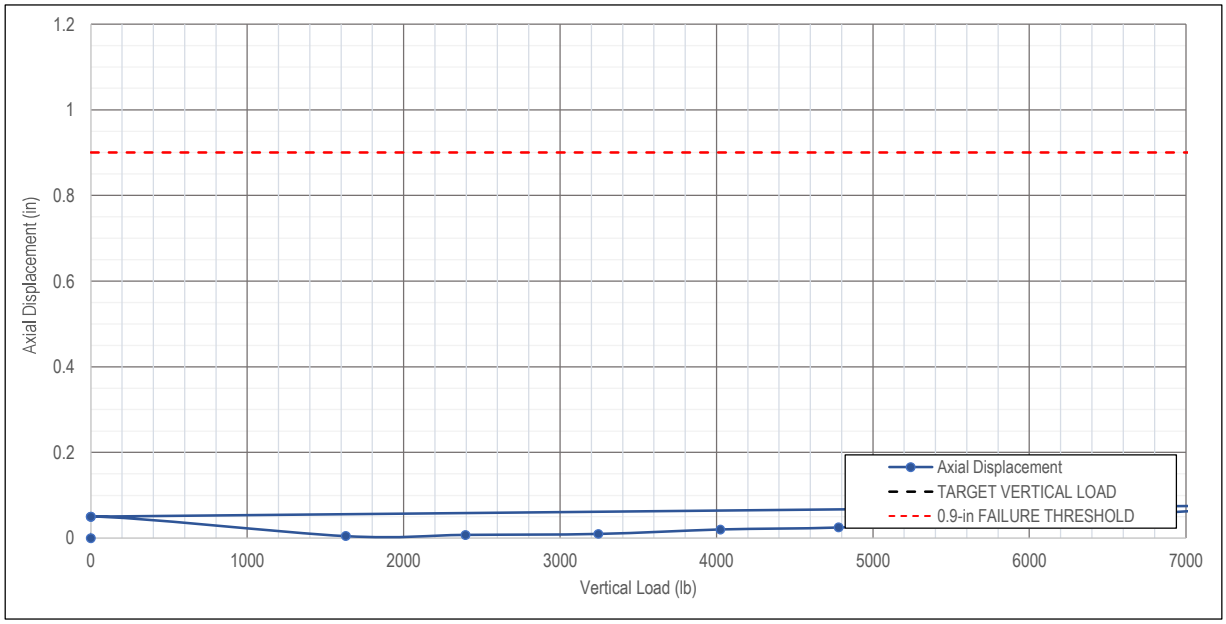
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/23/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800				0.00
436%	20%	1,600	1630	0.005	0.005	0.01
654%	30%	2,400	2395	0.005	0.01	0.01
872%	40%	3,200	3245	0.01	0.01	0.01
1090%	50%	4,000	4025	0.02	0.02	0.02
1308%	60%	4,800	4780	0.025	0.025	0.03
1526%	70%	5,600	5630	0.04	0.035	0.04
1744%	80%	6,400	6395	0.05	0.05	0.05
1962%	90%	7,200	7255	0.07	0.065	0.07
2180%	100%	8,000	7975	0.08	0.075	0.08
0%	0%	0	0	0.05	0.05	0.05

Load at 0.9-in axial displacement: **TBD** **lb** **Residual axial displacement:** **0.05** **in**



Notes:

800-lb load step was skipped.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-14B

Lateral Load Test Results

PTL- **14** Designation: **B**

Section: W6x9

Latitude: 33.663937

Longitude: -114.68161

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 19.01

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

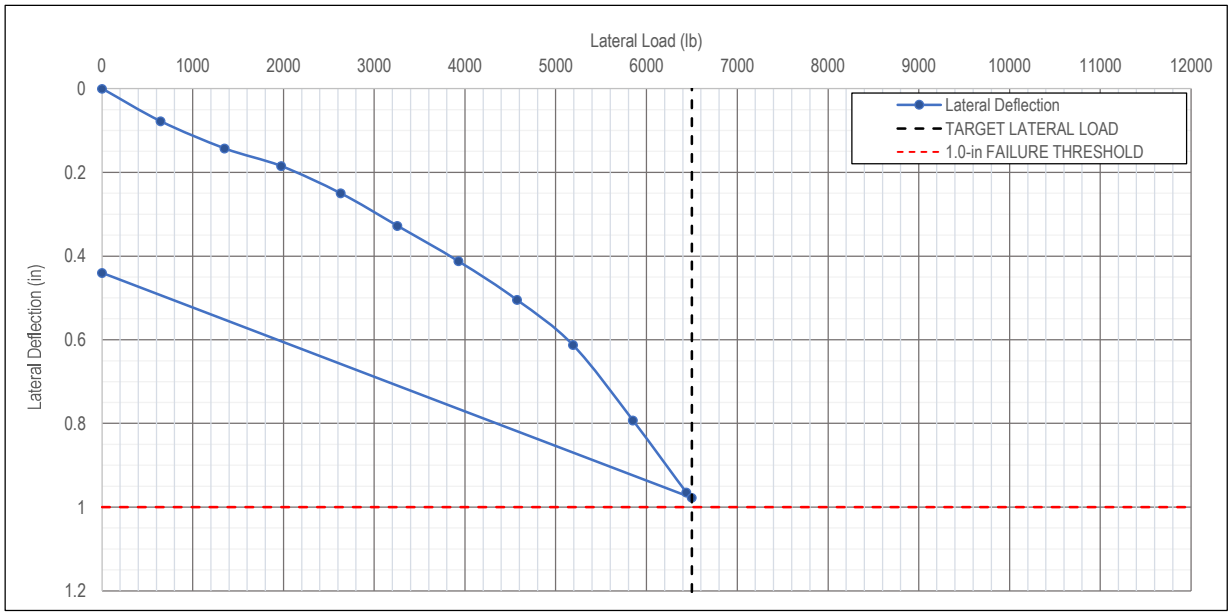
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	645	0.075	0.08	0.08
62%	20%	1,300	1350	0.14	0.145	0.14
93%	30%	1,950	1975	0.185	0.185	0.19
124%	40%	2,600	2630	0.25	0.25	0.25
155%	50%	3,250	3255	0.33	0.325	0.33
186%	60%	3,900	3930	0.415	0.41	0.41
217%	70%	4,550	4575	0.51	0.5	0.51
248%	80%	5,200	5190	0.625	0.6	0.61
279%	90%	5,850	5850	0.83	0.755	0.79
310%	100%	6,500	6440	1.075	0.855	0.97
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6500	1.225	0.73	0.98
0%	0%	0	0	0.72	0.16	0.44

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.44** **in**



Notes:

Pile twisting caused gauge to slip off at 6,500-lb. No reliable test results available after this point.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-15A

Axial Uplift Load Test Results

PTL - **15** Designation: **A**

Section: W6x9

Latitude: 33.658005

Longitude: -114.70751

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 7.01

Pile Damaged? No

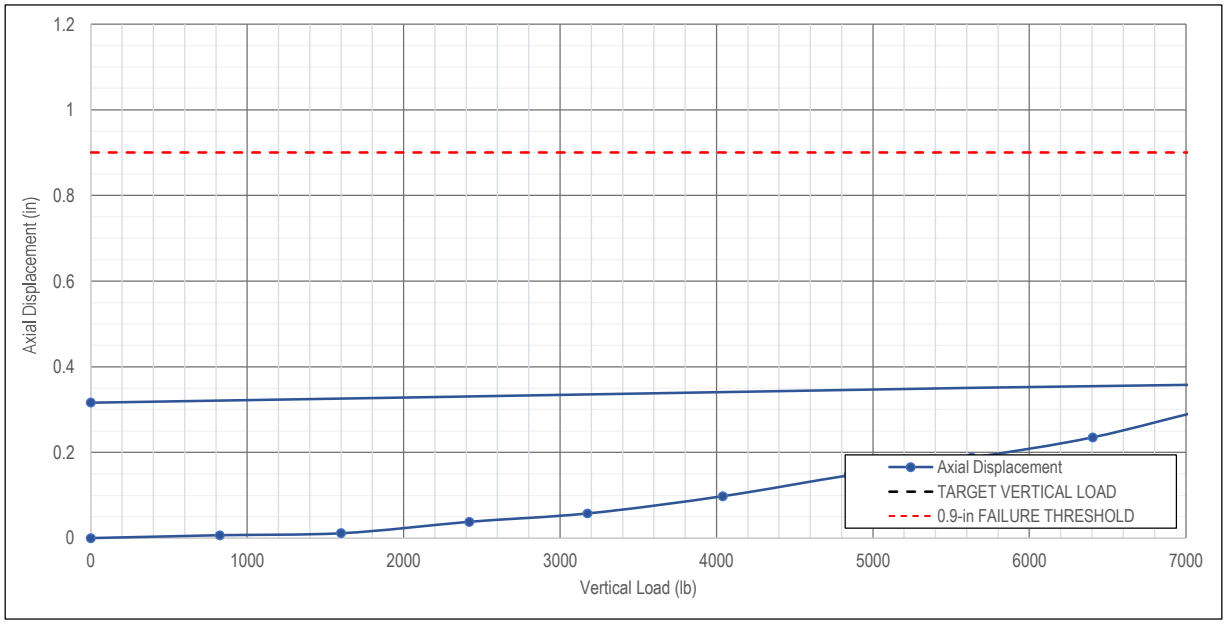
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	825	0.0065	0.007	0.01
13594%	20%	1,600	1600	0.011	0.0125	0.01
20391%	30%	2,400	2420	0.036	0.04	0.04
27188%	40%	3,200	3175	0.055	0.0605	0.06
33985%	50%	4,000	4040	0.0945	0.1015	0.10
40782%	60%	4,800	4845	0.142	0.151	0.15
47579%	70%	5,600	5630	0.183	0.193	0.19
54376%	80%	6,400	6405	0.23	0.2405	0.24
61173%	90%	7,200	7210	0.3	0.313	0.31
67970%	100%	8,000	8010	0.353	0.3675	0.36
0%	0%	0	0	0.3155	0.3175	0.32

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.32** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-15A

Lateral Load Test Results

PTL- **15** Designation: **A**

Section: W6x9

Latitude: 33.658005

Longitude: -114.70751

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 7.01

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

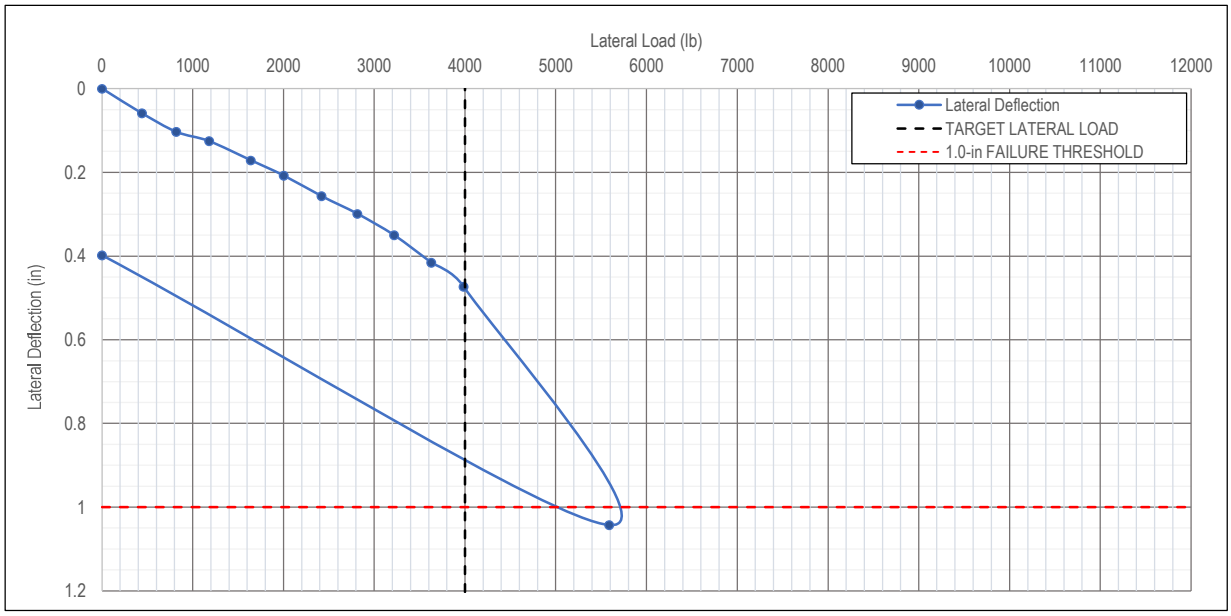
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	440	0.0565	0.06	0.06
62%	20%	800	820	0.101	0.105	0.10
93%	30%	1,200	1180	0.123	0.127	0.13
124%	40%	1,600	1640	0.1695	0.173	0.17
155%	50%	2,000	2005	0.2065	0.209	0.21
186%	60%	2,400	2420	0.255	0.258	0.26
217%	70%	2,800	2815	0.297	0.301	0.30
248%	80%	3,200	3220	0.3485	0.352	0.35
279%	90%	3,600	3630	0.414	0.4175	0.42
310%	100%	4,000	3985	0.472	0.4745	0.47
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5590	1.036	1.0495	1.04
0%	0%	0	0	0.3965	0.4005	0.40

Load at 1.0-in lateral deflection: **5,470** **lb**

Residual lateral deformation: **0.40** **in**



Notes:

Failure reached at 5,590 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-15B

Axial Uplift Load Test Results

PTL - **15** Designation: **B**

Section: W6x9

Latitude: 33.658005

Longitude: -114.70751

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 11.33

Pile Damaged? No

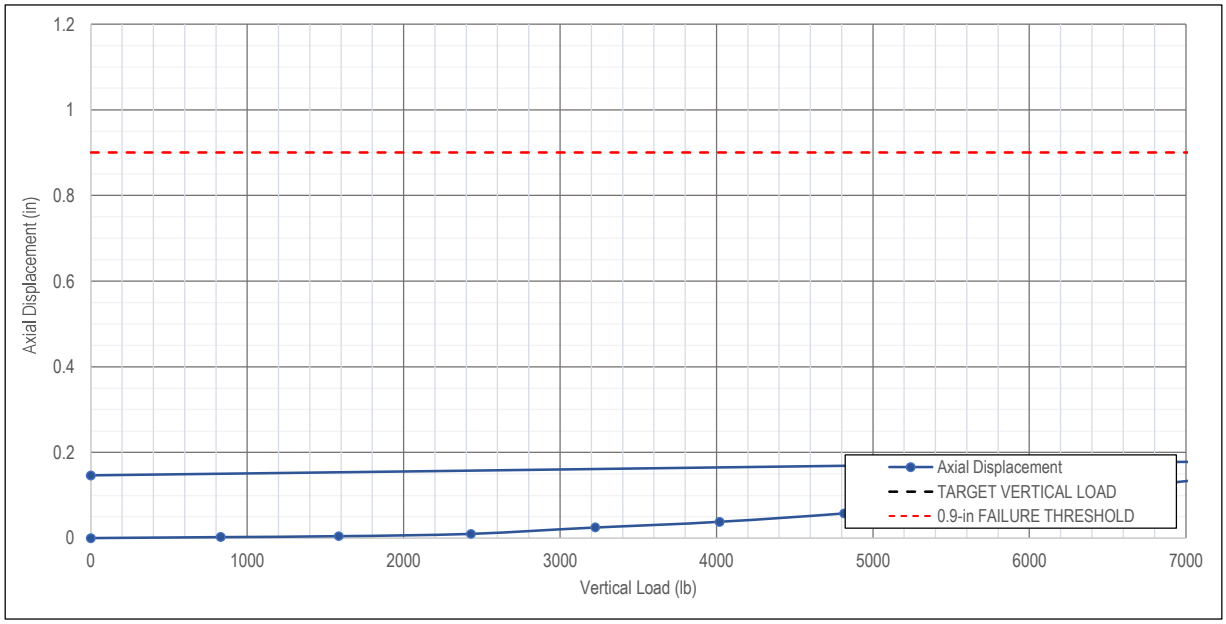
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	830	0.003	0.0025	0.00
13594%	20%	1,600	1585	0.0045	0.0045	0.00
20391%	30%	2,400	2430	0.01	0.01	0.01
27188%	40%	3,200	3225	0.0245	0.0255	0.03
33985%	50%	4,000	4020	0.0375	0.0385	0.04
40782%	60%	4,800	4815	0.057	0.058	0.06
47579%	70%	5,600	5640	0.083	0.0845	0.08
54376%	80%	6,400	6385	0.109	0.111	0.11
61173%	90%	7,200	7215	0.1405	0.1435	0.14
67970%	100%	8,000	8025	0.178	0.182	0.18
0%	0%	0	0	0.147	0.1455	0.15

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.15** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-15B

Lateral Load Test Results

PTL- **15** Designation: **B**

Section: W6x9

Latitude: 33.658005

Longitude: -114.70751

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.33

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

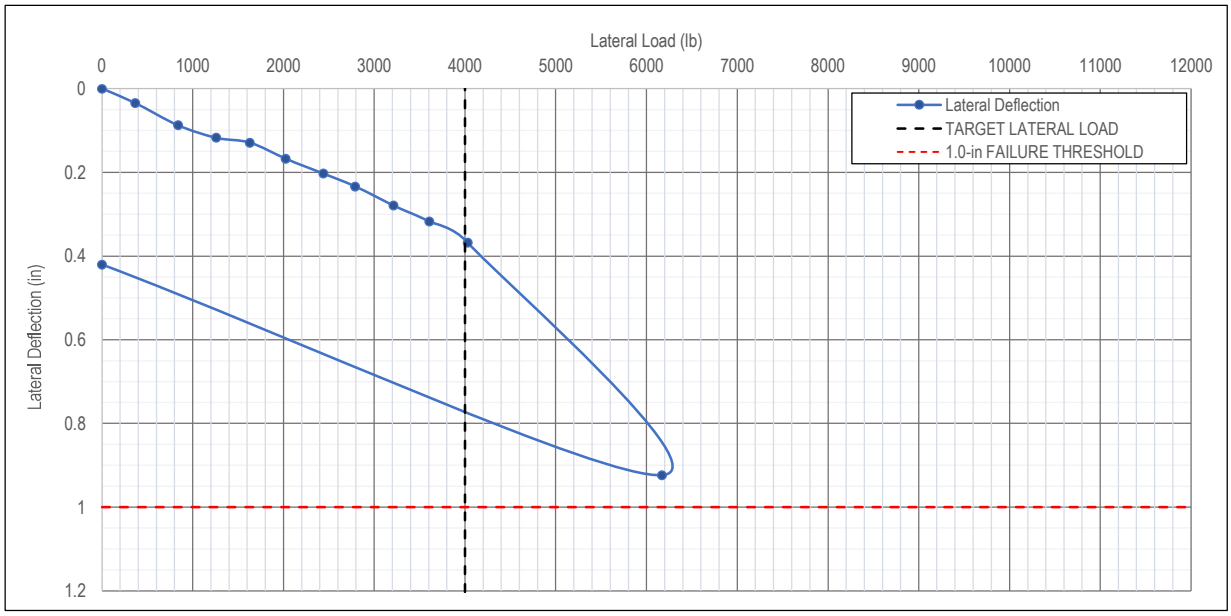
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	365	0.033	0.035	0.03
62%	20%	800	840	0.086	0.089	0.09
93%	30%	1,200	1260	0.112	0.1225	0.12
124%	40%	1,600	1630	0.123	0.1355	0.13
155%	50%	2,000	2025	0.161	0.174	0.17
186%	60%	2,400	2440	0.196	0.21	0.20
217%	70%	2,800	2790	0.2255	0.2415	0.23
248%	80%	3,200	3215	0.2705	0.288	0.28
279%	90%	3,600	3610	0.3075	0.3275	0.32
310%	100%	4,000	4030	0.3565	0.3785	0.37
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6170	0.768	1.0795	0.92
0%	0%	0	0	0.31	0.53	0.42

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.42** **in**



Notes:

Pile twisting caused gauge to slip off at 6,170lbs. No reliable test results available after this point.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-15C

Axial Compression Load Test Results (supplemental pile)

PTL - **15** Designation: **C**

Section: W6x9

Latitude: 33.658005

Longitude: -114.70751

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **15,000** (≈ 1180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 5.98

Pile Damaged? No

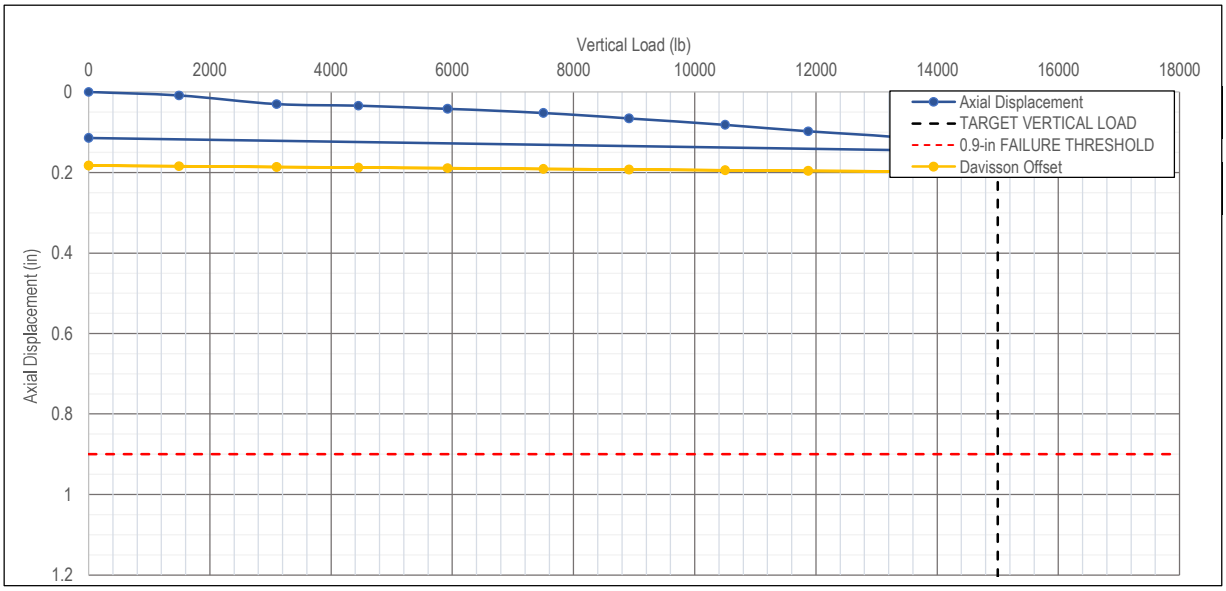
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL COMPRESSION** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
118%	10%	1,500	1490	0.0085	0.009	0.01
236%	20%	3,000	3100	0.0305	0.0295	0.03
354%	30%	4,500	4450	0.0345	0.034	0.03
472%	40%	6,000	5920	0.0445	0.039	0.04
590%	50%	7,500	7500	0.0555	0.049	0.05
708%	60%	9,000	8915	0.07	0.061	0.07
826%	70%	10,500	10500	0.0845	0.079	0.08
944%	80%	12,000	11870	0.101	0.0945	0.10
1062%	90%	13,500	13600	0.1195	0.1125	0.12
1180%	100%	15,000	15150	0.1515	0.146	0.15
0%	0%	0	0	0.1115	0.1165	0.11

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.11** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-16A

Axial Uplift Load Test Results

PTL - **16** Designation: **A** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.658238 - - Saturated Soil? No

Longitude: -114.70375 Target Embedment Depth (ft): 6.00 Drive Time (s): 7.65

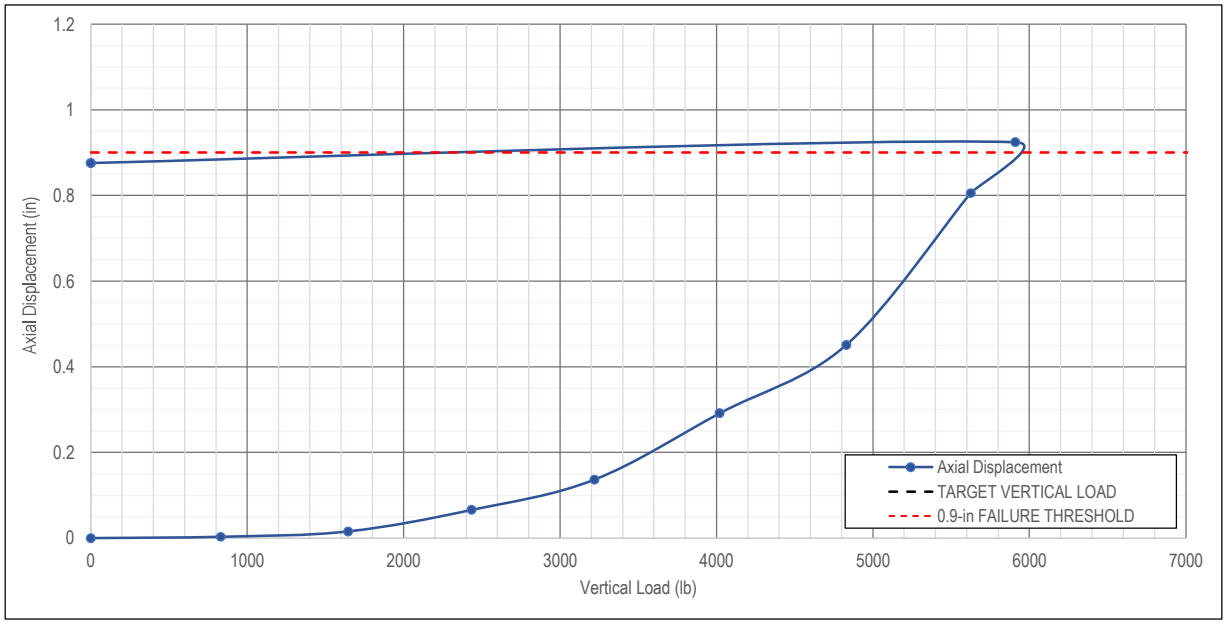
Tested By: ANS Actual Embedment Depth (ft): 6 Pile Damaged? No

Test Date: 10/25/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	830	0.003	0.0035	0.00
436%	20%	1,600	1645	0.0155	0.0155	0.02
654%	30%	2,400	2435	0.0655	0.067	0.07
872%	40%	3,200	3220	0.135	0.138	0.14
1090%	50%	4,000	4020	0.29	0.2935	0.29
1308%	60%	4,800	4830	0.452	0.45	0.45
1526%	70%	5,600	5625	0.809	0.802	0.81
1744%	80%	6,400	5910	0.928	0.921	0.92
1962%	90%	7,200				
2180%	100%	8,000				
0%	0%	0	0	0.874	0.8775	0.88

Load at 0.9-in axial displacement: **5,851** **lb** **Residual axial displacement:** **0.88** **in**



Notes:

Failure reached at 5,910 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-16A

Lateral Load Test Results

PTL- **16** Designation: **A**

Section: W6x9

Latitude: 33.658238

Longitude: -114.70375

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 7.65

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

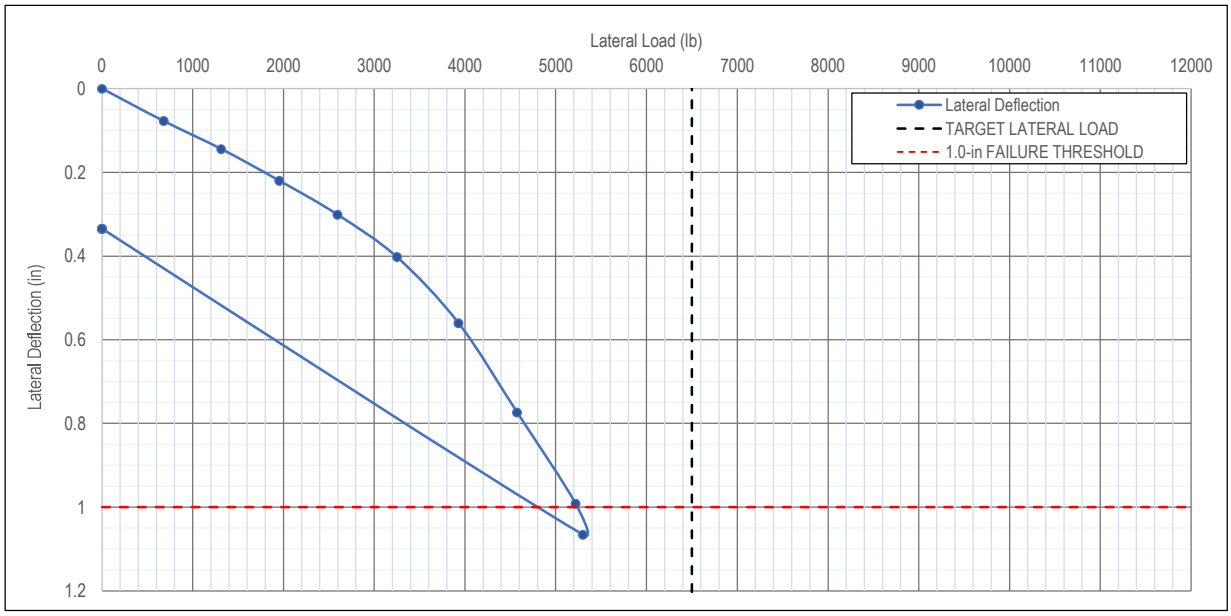
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	680	0.072	0.082	0.08
62%	20%	1,300	1315	0.138	0.151	0.14
93%	30%	1,950	1955	0.2175	0.222	0.22
124%	40%	2,600	2595	0.292	0.31	0.30
155%	50%	3,250	3250	0.3915	0.4125	0.40
186%	60%	3,900	3930	0.547	0.574	0.56
217%	70%	4,550	4575	0.7555	0.792	0.77
248%	80%	5,200	5220	0.9645	1.0195	0.99
279%	90%	5,850	5300	1.036	1.096	1.07
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.326	0.3435	0.33

Load at 1.0-in lateral deflection: **5,229** **lb**

Residual lateral deformation: **0.33** **in**



Notes:

Failure reached at 5,300 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-16B

Axial Uplift Load Test Results

PTL - **16** Designation: **B**

Section: W6x9

Latitude: 33.658238

Longitude: -114.70375

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 12.18

Pile Damaged? No

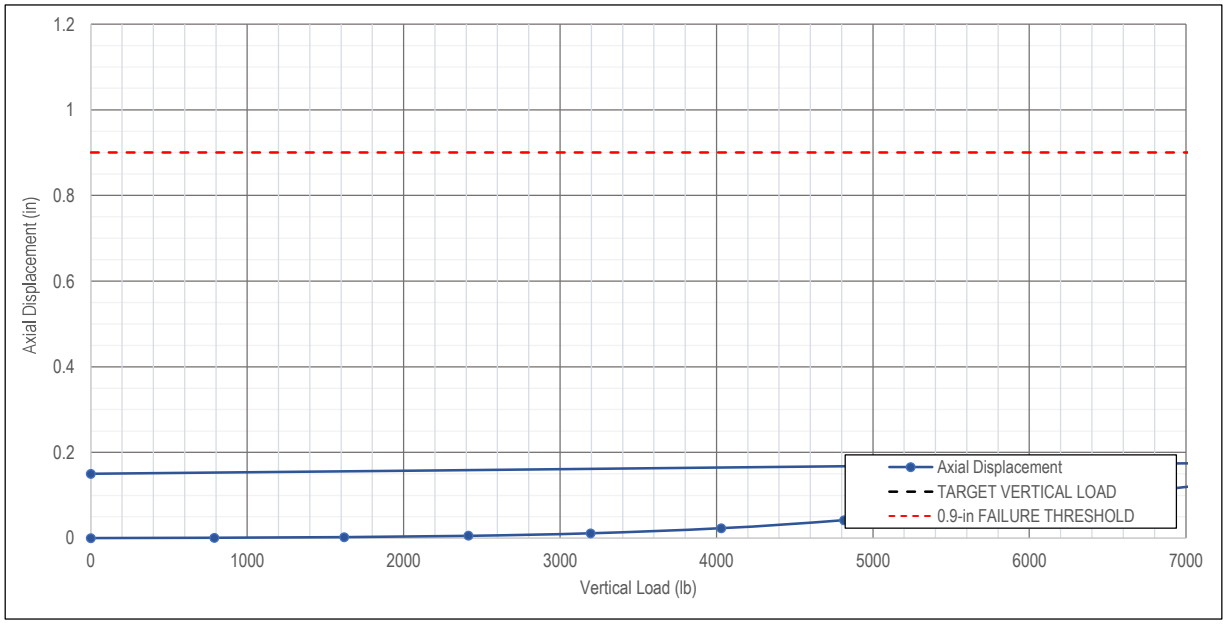
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	790	0.001	0.001	0.00
436%	20%	1,600	1620	0.002	0.003	0.00
654%	30%	2,400	2415	0.0045	0.007	0.01
872%	40%	3,200	3195	0.01	0.0125	0.01
1090%	50%	4,000	4030	0.021	0.025	0.02
1308%	60%	4,800	4815	0.0395	0.044	0.04
1526%	70%	5,600	5640	0.063	0.069	0.07
1744%	80%	6,400	6430	0.092	0.1	0.10
1962%	90%	7,200	7225	0.125	0.135	0.13
2180%	100%	8,000	8040	0.17	0.1805	0.18
0%	0%	0	0	0.149	0.151	0.15

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.15** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-16B

Lateral Load Test Results

PTL- **16** Designation: **B**

Section: W6x9

Latitude: 33.658238

Longitude: -114.70375

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 12.18

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

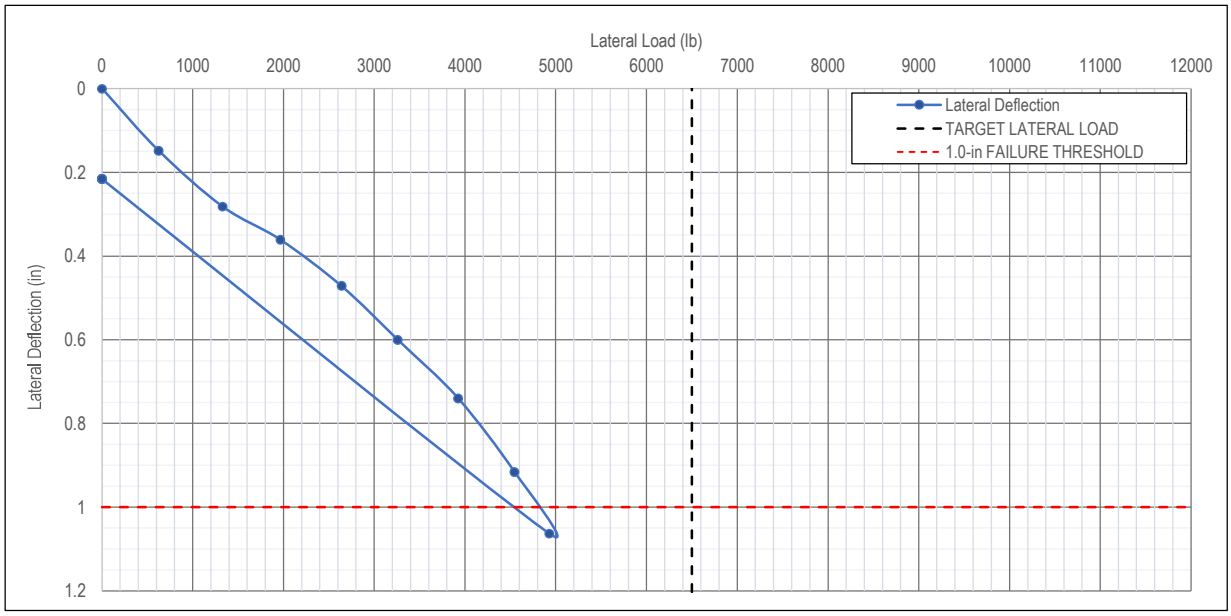
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	625	0.1335	0.163	0.15
62%	20%	1,300	1330	0.263	0.3	0.28
93%	30%	1,950	1965	0.341	0.381	0.36
124%	40%	2,600	2640	0.4495	0.493	0.47
155%	50%	3,250	3260	0.5765	0.6235	0.60
186%	60%	3,900	3925	0.714	0.766	0.74
217%	70%	4,550	4545	0.8855	0.9465	0.92
248%	80%	5,200	4930	1.0265	1.1	1.06
279%	90%	5,850				
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.205	0.2265	0.22

Load at 1.0-in lateral deflection: **4,765** **lb**

Residual lateral deformation: **0.22** **in**



Notes:

Failure reached at 4,930 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-16D

Axial Uplift Load Test Results

PTL - **16** Designation: **D**

Section: **W6x12**

Latitude: 33.658238

Longitude: -114.70375

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 9.91

Pile Damaged? No

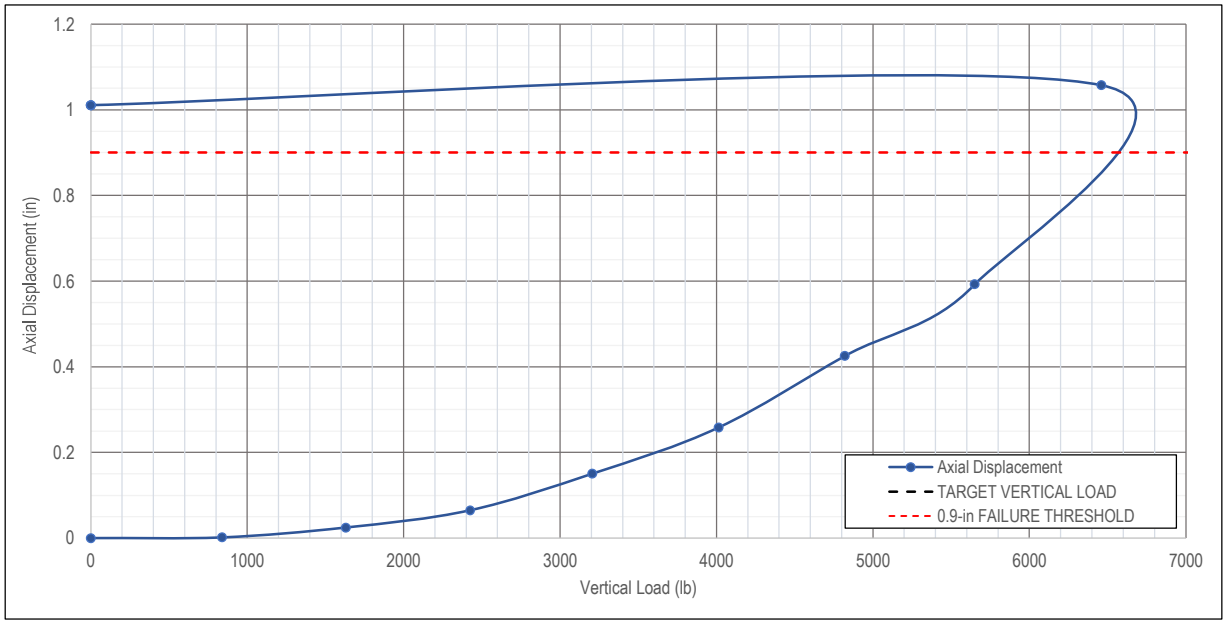
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	840	0.0015	0.0025	0.00
436%	20%	1,600	1630	0.023	0.026	0.02
654%	30%	2,400	2425	0.063	0.067	0.07
872%	40%	3,200	3205	0.1475	0.153	0.15
1090%	50%	4,000	4015	0.255	0.262	0.26
1308%	60%	4,800	4820	0.421	0.4295	0.43
1526%	70%	5,600	5650	0.587	0.598	0.59
1744%	80%	6,400	6460	1.0515	1.0635	1.06
1962%	90%	7,200				
2180%	100%	8,000				
0%	0%	0	0	1.01	1.011	1.01

Load at 0.9-in axial displacement: **6,186** **lb**

Residual axial displacement: **1.01** **in**



Notes:

Failure reached at 6,460 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-16D

Lateral Load Test Results

PTL- **16** Designation: **D**

Section: **W6x12**

Latitude: 33.658238

Longitude: -114.70375

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 9.91

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

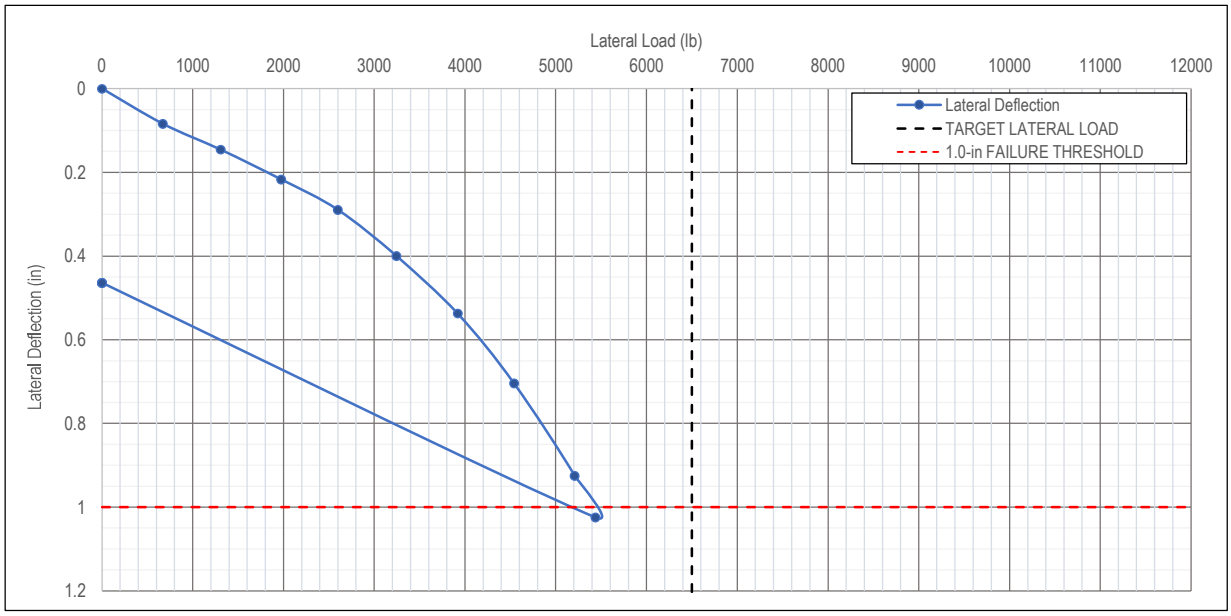
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	670	0.0825	0.0855	0.08
62%	20%	1,300	1310	0.1435	0.148	0.15
93%	30%	1,950	1975	0.214	0.22	0.22
124%	40%	2,600	2600	0.286	0.2925	0.29
155%	50%	3,250	3245	0.396	0.404	0.40
186%	60%	3,900	3920	0.533	0.5415	0.54
217%	70%	4,550	4540	0.699	0.709	0.70
248%	80%	5,200	5210	0.92	0.93	0.93
279%	90%	5,850	5440	1.0195	1.0295	1.02
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.462	0.466	0.46

Load at 1.0-in lateral deflection: **5,383** **lb**

Residual lateral deformation: **0.46** **in**



Notes:

Failure reached at 5,440 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-17A

Axial Uplift Load Test Results

PTL - **17** Designation: **A**

Section: W6x9

Latitude: 33.658974

Longitude: -114.69941

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 9.02

Pile Damaged? No

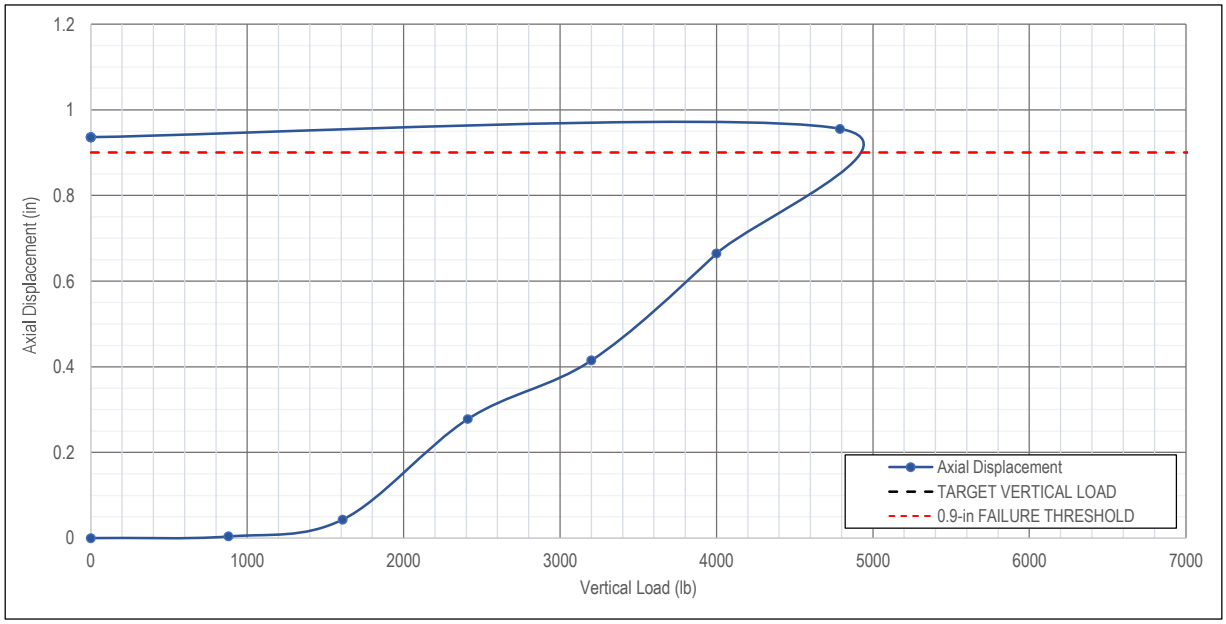
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	880	0.003	0.005	0.00
13594%	20%	1,600	1610	0.0415	0.045	0.04
20391%	30%	2,400	2410	0.275	0.281	0.28
27188%	40%	3,200	3200	0.411	0.4185	0.41
33985%	50%	4,000	4000	0.66	0.669	0.66
40782%	60%	4,800	4790	0.9515	0.959	0.96
47579%	70%	5,600				
54376%	80%	6,400				
61173%	90%	7,200				
67970%	100%	8,000				
0%	0%	0	0	0.9335	0.9385	0.94

Load at 0.9-in axial displacement: **4,640** **lb**

Residual axial displacement: **0.94** **in**



Notes:

Failure reached at 4,790 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-17A

Lateral Load Test Results

PTL- **17** Designation: **A**

Section: W6x9

Latitude: 33.658974

Longitude: -114.69941

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 9.02

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

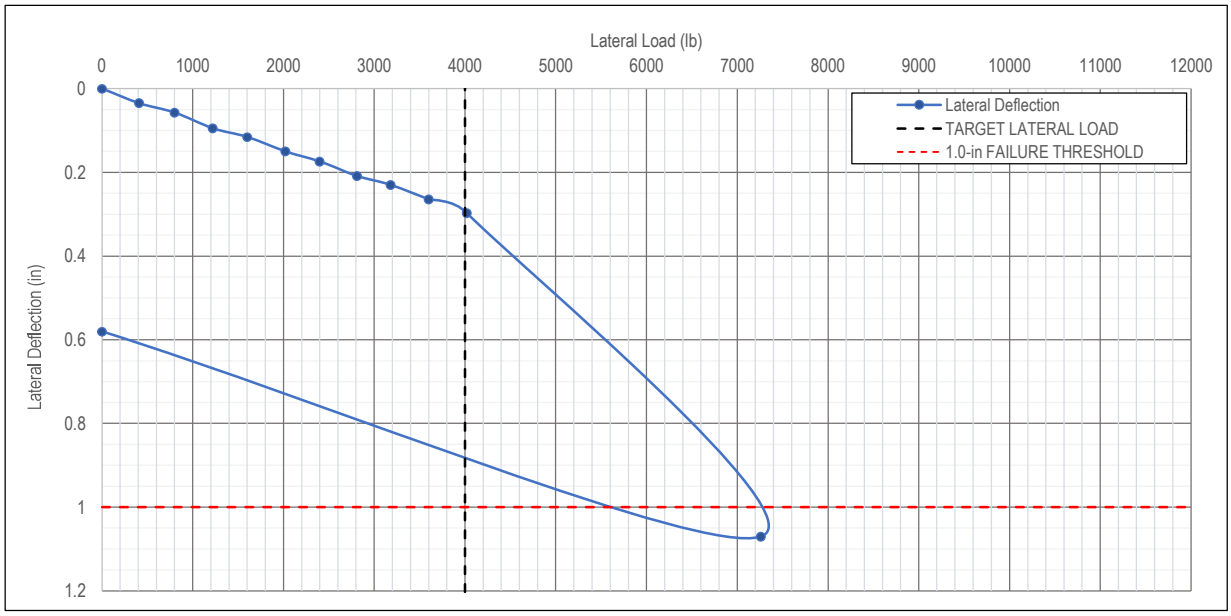
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	410	0.036	0.033	0.03
62%	20%	800	800	0.06	0.0545	0.06
93%	30%	1,200	1220	0.099	0.09	0.09
124%	40%	1,600	1600	0.1215	0.1095	0.12
155%	50%	2,000	2020	0.1575	0.1415	0.15
186%	60%	2,400	2400	0.1835	0.164	0.17
217%	70%	2,800	2810	0.22	0.197	0.21
248%	80%	3,200	3180	0.242	0.217	0.23
279%	90%	3,600	3600	0.279	0.25	0.26
310%	100%	4,000	4020	0.314	0.281	0.30
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7260	1.182	0.959	1.07
0%	0%	0	0	0.6605	0.5005	0.58

Load at 1.0-in lateral deflection: **6,965** **lb**

Residual lateral deformation: **0.58** **in**



Notes:

Failure reached at 7,260 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-17B

Axial Uplift Load Test Results

PTL - **17** Designation: **B**

Section: W6x9

Latitude: 33.658974

Longitude: -114.69941

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 17.01

Pile Damaged? No

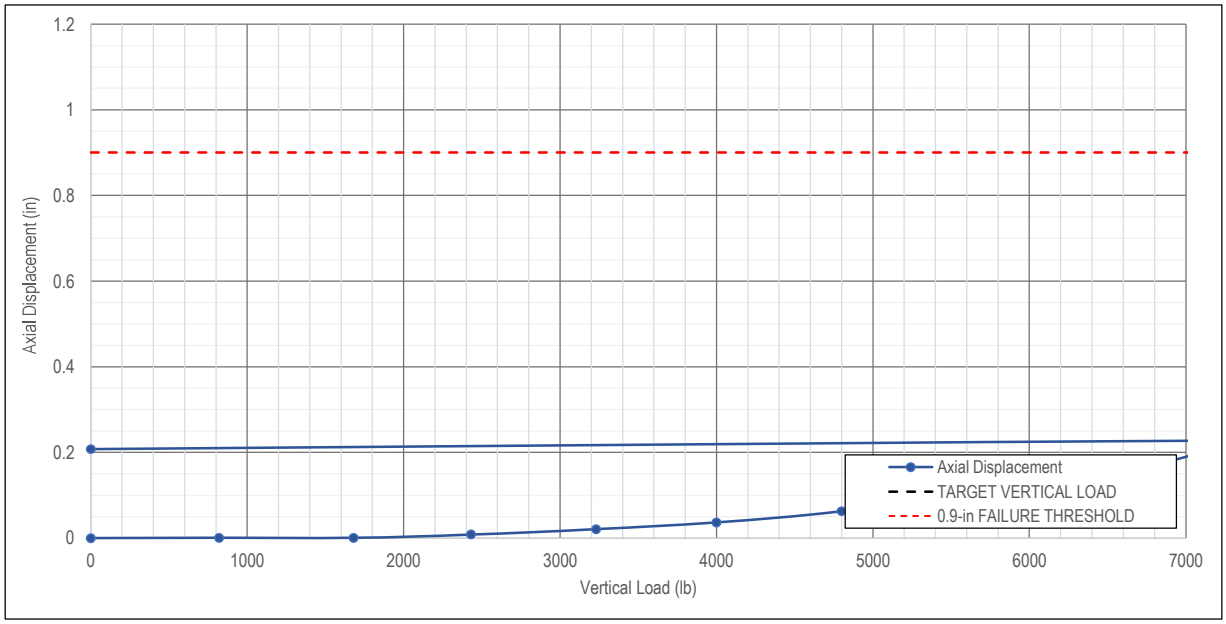
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	820	0.0005	0.001	0.00
13594%	20%	1,600	1680	0.0005	0.001	0.00
20391%	30%	2,400	2430	0.007	0.0095	0.01
27188%	40%	3,200	3230	0.0195	0.022	0.02
33985%	50%	4,000	4000	0.0355	0.038	0.04
40782%	60%	4,800	4800	0.0625	0.063	0.06
47579%	70%	5,600	5630	0.11	0.109	0.11
54376%	80%	6,400	6380	0.129	0.1285	0.13
61173%	90%	7,200	7240	0.21	0.2105	0.21
67970%	100%	8,000	8020	0.2285	0.2295	0.23
0%	0%	0	0	0.2085	0.2075	0.21

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.21** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-17B

Lateral Load Test Results

PTL- **17** Designation: **B**

Section: W6x9

Latitude: 33.658974

Longitude: -114.69941

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 17.01

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

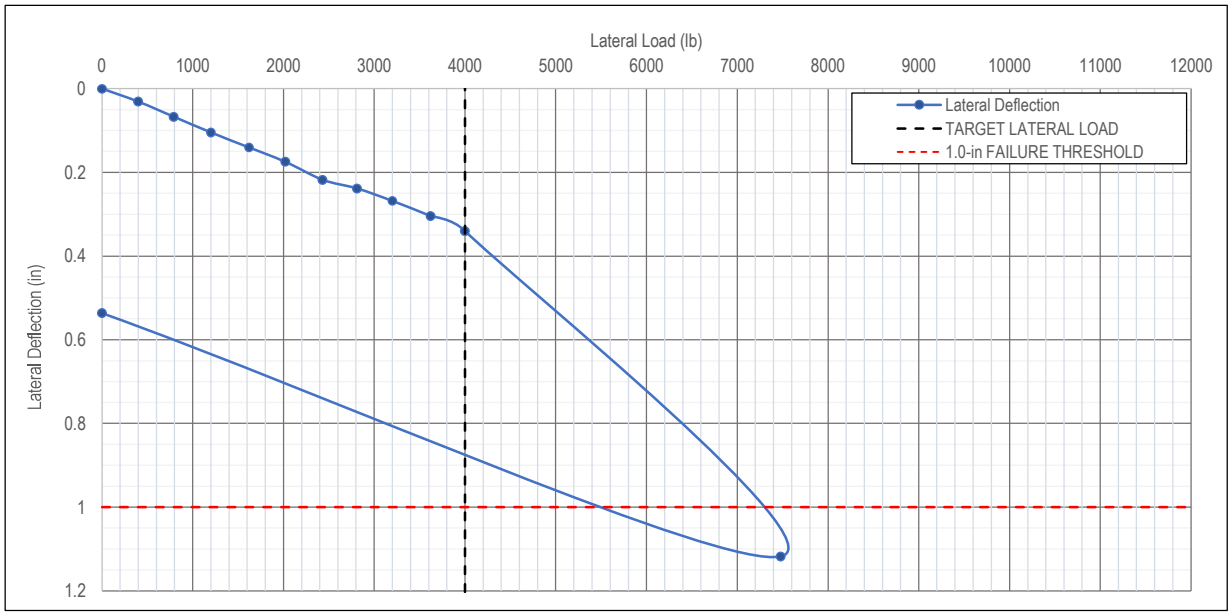
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.031	0.0305	0.03
62%	20%	800	790	0.0675	0.0665	0.07
93%	30%	1,200	1200	0.1055	0.103	0.10
124%	40%	1,600	1620	0.1425	0.138	0.14
155%	50%	2,000	2020	0.1775	0.1715	0.17
186%	60%	2,400	2430	0.222	0.213	0.22
217%	70%	2,800	2810	0.244	0.233	0.24
248%	80%	3,200	3200	0.272	0.264	0.27
279%	90%	3,600	3620	0.312	0.2965	0.30
310%	100%	4,000	4000	0.3475	0.3325	0.34
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7480	0.988	1.247	1.12
0%	0%	0	0	0.39	0.6825	0.54

Load at 1.0-in lateral deflection: **6,954** **lb**

Residual lateral deformation: **0.54** **in**



Notes:

Failure reached at 7,480 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-17C

Axial Compression Load Test Results (supplemental pile)

PTL - **17** Designation: **C**

Section: W6x9

Latitude: 33.658974

Longitude: -114.69941

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **15,000** (≈ 1180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 9.42

Pile Damaged? No

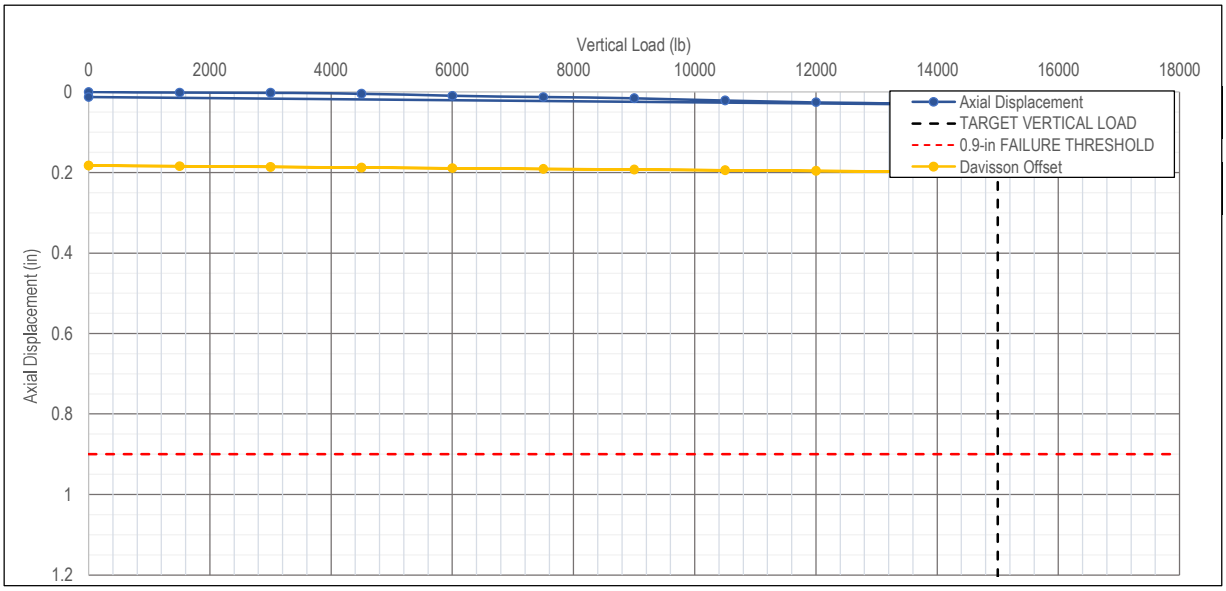
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL COMPRESSION** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
118%	10%	1,500	1500	0.001	0.0025	0.00
236%	20%	3,000	3000	0.001	0.003	0.00
354%	30%	4,500	4500	0.003	0.006	0.00
472%	40%	6,000	6000	0.007	0.012	0.01
590%	50%	7,500	7500	0.01	0.015	0.01
708%	60%	9,000	9000	0.014	0.0175	0.02
826%	70%	10,500	10500	0.0195	0.0225	0.02
944%	80%	12,000	12000	0.024	0.0275	0.03
1062%	90%	13,500	13500	0.0275	0.0305	0.03
1180%	100%	15,000	15000	0.0305	0.0345	0.03
0%	0%	0	0	0.0105	0.0145	0.01

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-18A

Axial Uplift Load Test Results

PTL - **18** Designation: **A**

Section: W6x9

Latitude: 33.659052

Longitude: -114.69506

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 9.25

Pile Damaged? No

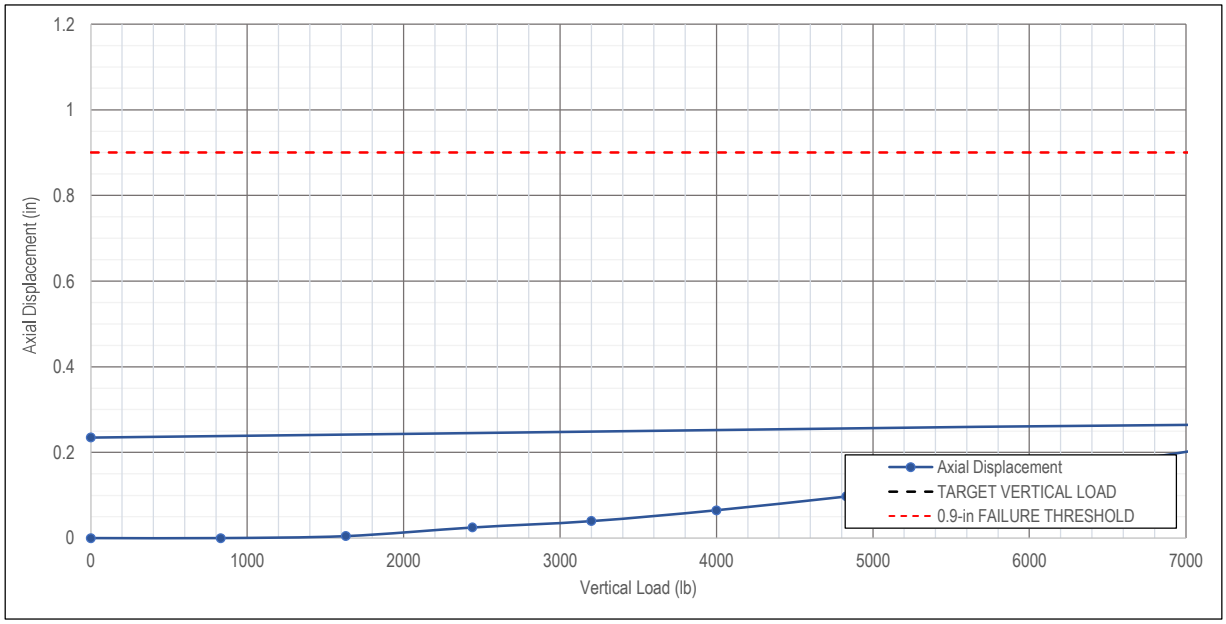
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	830	0	0	0.00
436%	20%	1,600	1630	0.005	0.005	0.01
654%	30%	2,400	2440	0.025	0.025	0.03
872%	40%	3,200	3200	0.04	0.04	0.04
1090%	50%	4,000	4000	0.065	0.065	0.07
1308%	60%	4,800	4830	0.1	0.095	0.10
1526%	70%	5,600	5600	0.13	0.13	0.13
1744%	80%	6,400	6430	0.17	0.17	0.17
1962%	90%	7,200	7230	0.215	0.215	0.22
2180%	100%	8,000	8020	0.265	0.265	0.27
0%	0%	0	0	0.235	0.235	0.24

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.24** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-18A

Lateral Load Test Results

PTL- **18** Designation: **A**

Section: W6x9

Latitude: 33.659052

Longitude: -114.69506

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 9.25

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

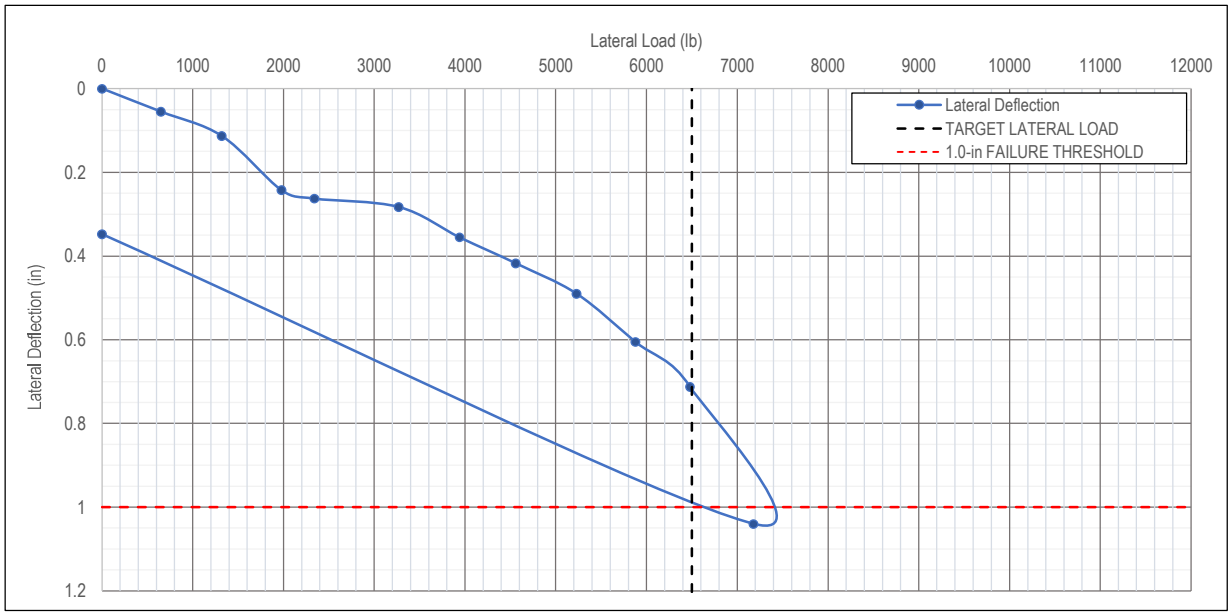
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.05	0.06	0.06
62%	20%	1,300	1320	0.105	0.12	0.11
93%	30%	1,950	1980	0.23	0.255	0.24
124%	40%	2,600	2340	0.25	0.275	0.26
155%	50%	3,250	3270	0.27	0.295	0.28
186%	60%	3,900	3940	0.34	0.37	0.36
217%	70%	4,550	4560	0.4	0.435	0.42
248%	80%	5,200	5230	0.47	0.51	0.49
279%	90%	5,850	5880	0.58	0.63	0.61
310%	100%	6,500	6480	0.68	0.745	0.71
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7180	1.01	1.07	1.04
0%	0%	0	0	0.345	0.35	0.35

Load at 1.0-in lateral deflection: **7,095** **lb**

Residual lateral deformation: **0.35** **in**



Notes:

Failure reached at 7,180 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-18B

Axial Uplift Load Test Results

PTL - **18** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.659052 - - Saturated Soil? No

Longitude: -114.69506 Target Embedment Depth (ft): 8.00 Drive Time (s): 15.47

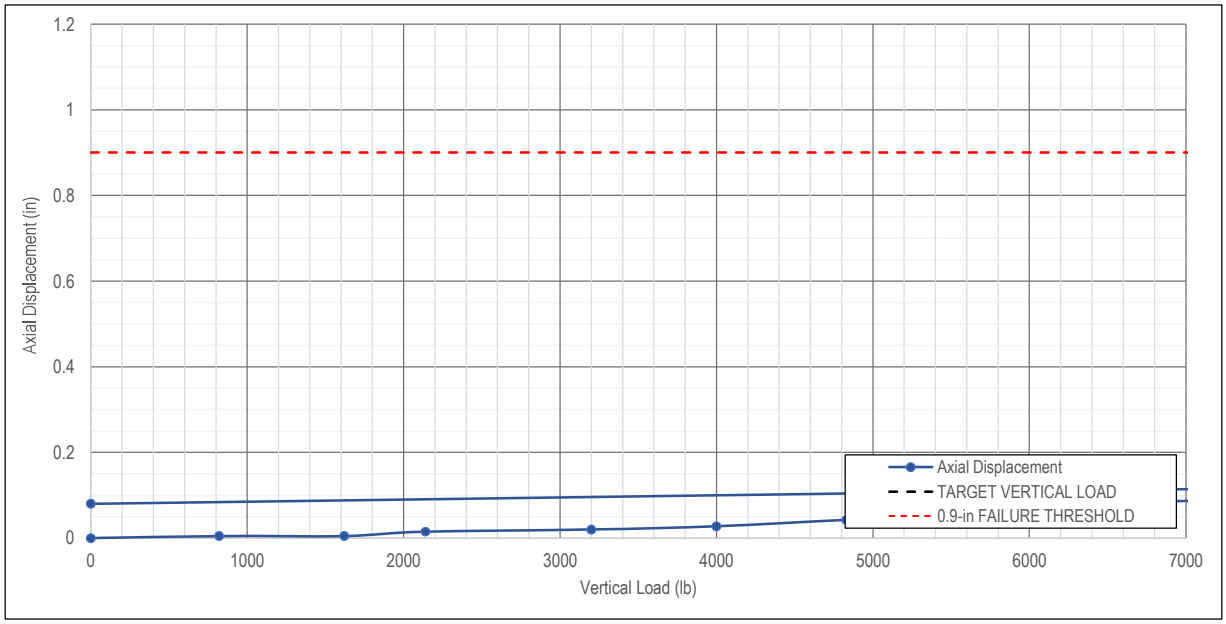
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/24/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	820	0.005	0.005	0.01
436%	20%	1,600	1620	0.005	0.005	0.01
654%	30%	2,400	2140	0.015	0.015	0.02
872%	40%	3,200	3200	0.02	0.02	0.02
1090%	50%	4,000	4000	0.03	0.025	0.03
1308%	60%	4,800	4830	0.045	0.04	0.04
1526%	70%	5,600	5580	0.06	0.05	0.06
1744%	80%	6,400	6440	0.09	0.075	0.08
1962%	90%	7,200	7240	0.095	0.085	0.09
2180%	100%	8,000	8030	0.125	0.11	0.12
0%	0%	0	0	0.08	0.08	0.08

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.08** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-18B

Lateral Load Test Results

PTL- **18** Designation: **B**

Section: W6x9

Latitude: 33.659052

Longitude: -114.69506

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 15.47

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

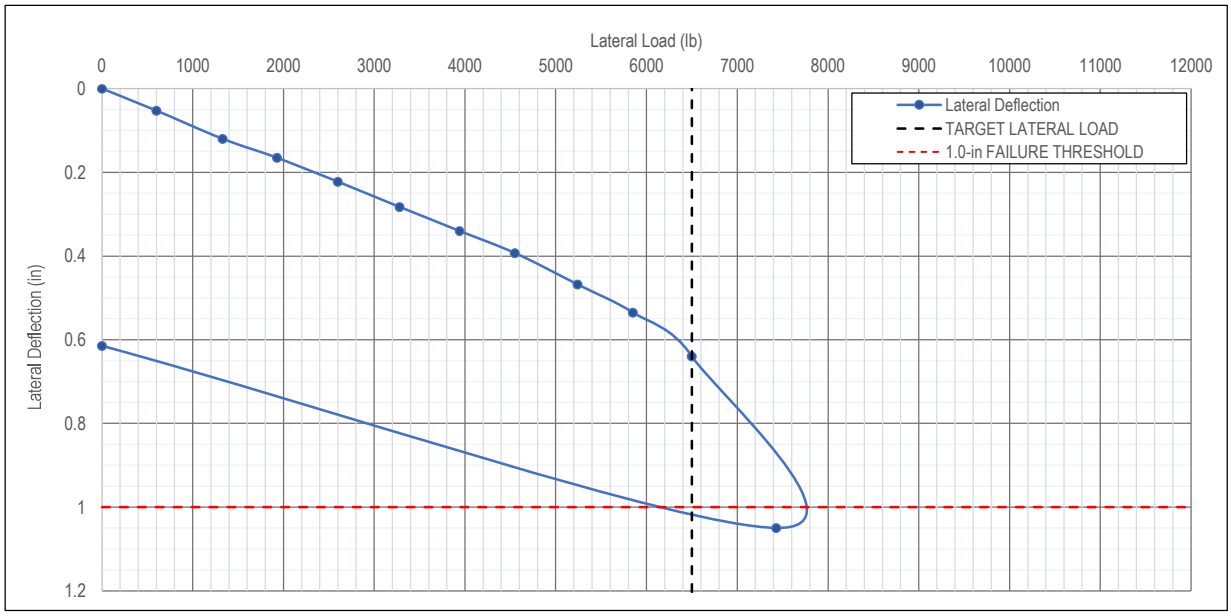
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	600	0.055	0.05	0.05
62%	20%	1,300	1330	0.125	0.115	0.12
93%	30%	1,950	1930	0.17	0.16	0.17
124%	40%	2,600	2600	0.23	0.215	0.22
155%	50%	3,250	3280	0.295	0.27	0.28
186%	60%	3,900	3940	0.35	0.33	0.34
217%	70%	4,550	4550	0.41	0.375	0.39
248%	80%	5,200	5240	0.49	0.445	0.47
279%	90%	5,850	5850	0.56	0.51	0.54
310%	100%	6,500	6500	0.68	0.6	0.64
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7430	1.195	0.905	1.05
0%	0%	0	0	0.709	0.52	0.61

Load at 1.0-in lateral deflection: **7,317** **lb**

Residual lateral deformation: **0.61** **in**



Notes:

Failure reached at 7,430 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-19A

Axial Uplift Load Test Results

PTL - **19** Designation: **A**

Section: W6x9

Latitude: 33.658393

Longitude: -114.69026

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 5.89

Pile Damaged? No

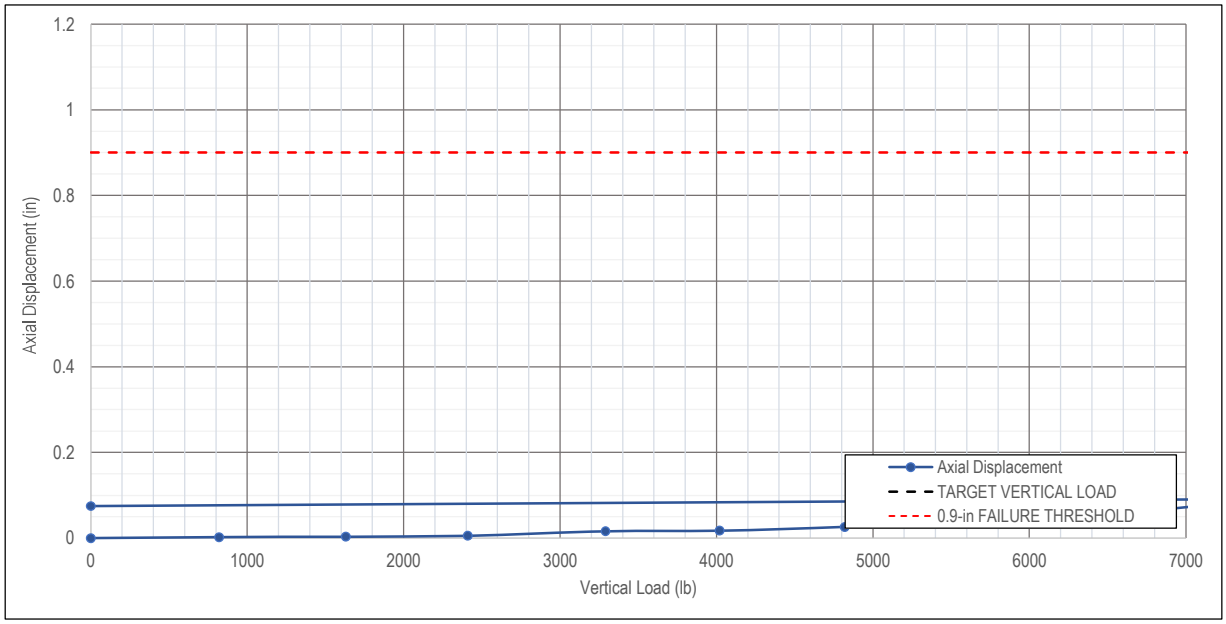
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	820	0.003	0.002	0.00
13594%	20%	1,600	1630	0.004	0.003	0.00
20391%	30%	2,400	2410	0.006	0.0055	0.01
27188%	40%	3,200	3290	0.015	0.017	0.02
33985%	50%	4,000	4020	0.0165	0.0185	0.02
40782%	60%	4,800	4820	0.025	0.0275	0.03
47579%	70%	5,600	5680	0.038	0.0415	0.04
54376%	80%	6,400	6390	0.048	0.0515	0.05
61173%	90%	7,200	7280	0.079	0.084	0.08
67970%	100%	8,000	8100	0.0895	0.094	0.09
0%	0%	0	0	0.075	0.075	0.08

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.08** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-19A

Lateral Load Test Results

PTL- **19** Designation: **A**

Section: W6x9

Latitude: 33.658393

Longitude: -114.69026

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 5.89

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

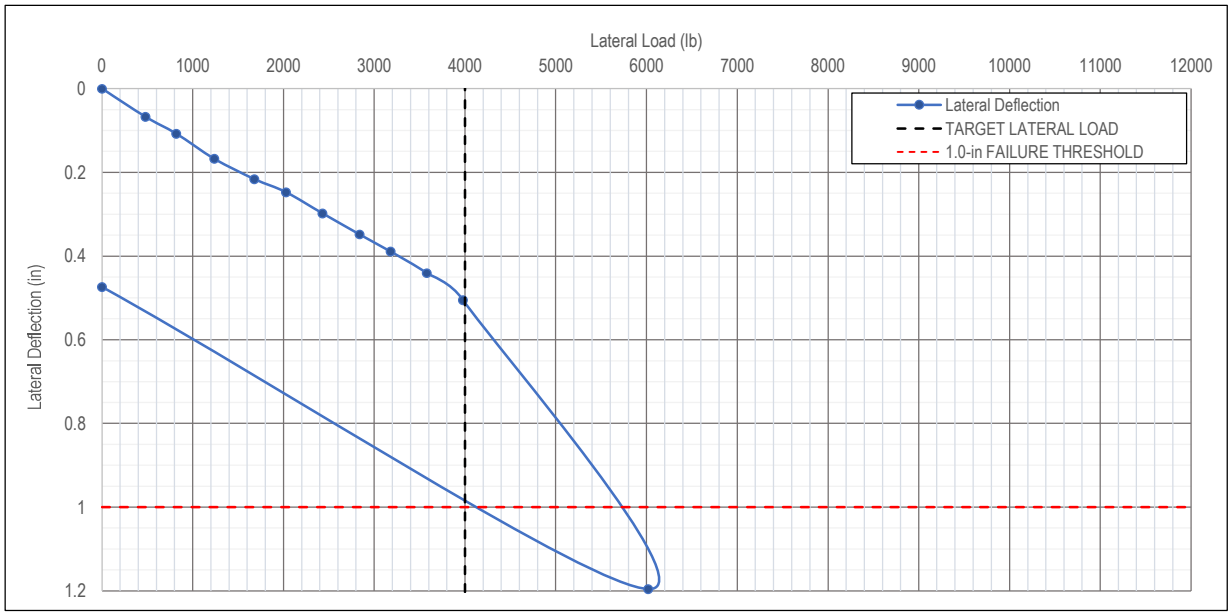
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	480	0.0665	0.068	0.07
62%	20%	800	820	0.105	0.1095	0.11
93%	30%	1,200	1240	0.1675	0.168	0.17
124%	40%	1,600	1680	0.214	0.218	0.22
155%	50%	2,000	2030	0.244	0.2505	0.25
186%	60%	2,400	2430	0.293	0.303	0.30
217%	70%	2,800	2840	0.3415	0.355	0.35
248%	80%	3,200	3180	0.381	0.397	0.39
279%	90%	3,600	3580	0.4315	0.45	0.44
310%	100%	4,000	3980	0.494	0.5165	0.51
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6020	1.1065	1.285	1.20
0%	0%	0	0	0.4415	0.5065	0.47

Load at 1.0-in lateral deflection: **5,442** **lb**

Residual lateral deformation: **0.47** **in**



Notes:

Failure reached at 6,020 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-19B

Axial Uplift Load Test Results

PTL - **19** Designation: **B**

Section: W6x9

Latitude: 33.658393

Longitude: -114.69026

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 15.09

Pile Damaged? No

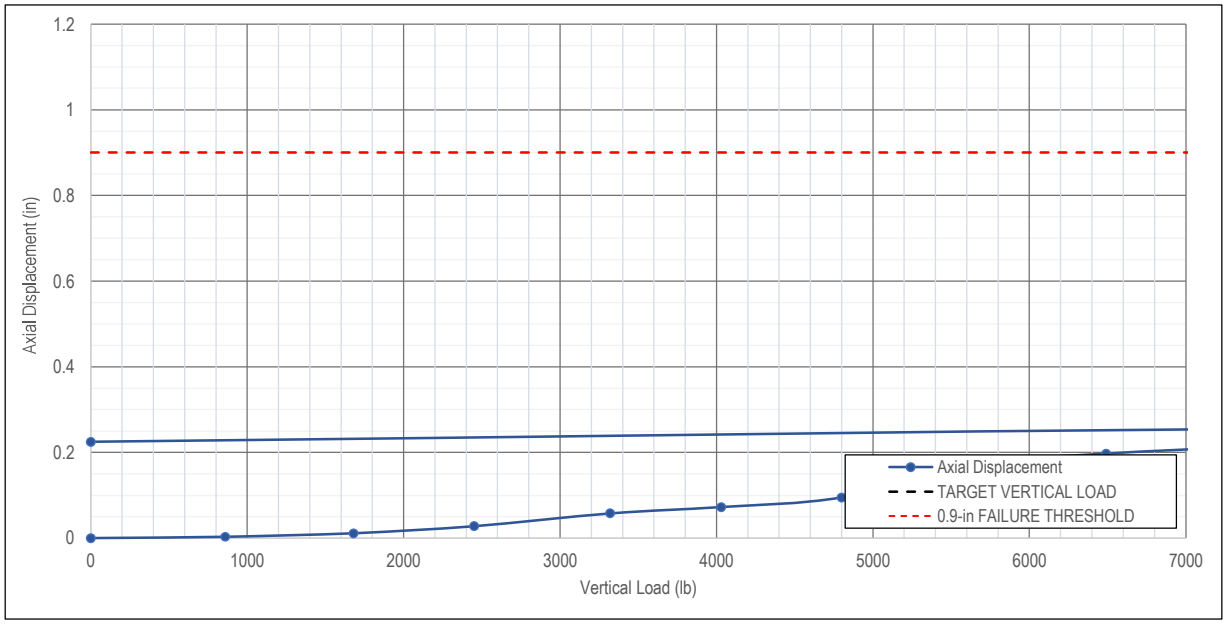
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	860	0.0035	0.003	0.00
13594%	20%	1,600	1680	0.0115	0.0115	0.01
20391%	30%	2,400	2450	0.0275	0.0285	0.03
27188%	40%	3,200	3320	0.0565	0.0595	0.06
33985%	50%	4,000	4030	0.0705	0.075	0.07
40782%	60%	4,800	4800	0.0915	0.099	0.10
47579%	70%	5,600	5700	0.1675	0.1815	0.17
54376%	80%	6,400	6490	0.189	0.206	0.20
61173%	90%	7,200	7310	0.2055	0.2245	0.22
67970%	100%	8,000	8010	0.2435	0.267	0.26
0%	0%	0	0	0.224	0.2255	0.22

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.22** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-19B

Lateral Load Test Results

PTL- **19** Designation: **B**

Section: W6x9

Latitude: 33.658393

Longitude: -114.69026

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 15.09

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

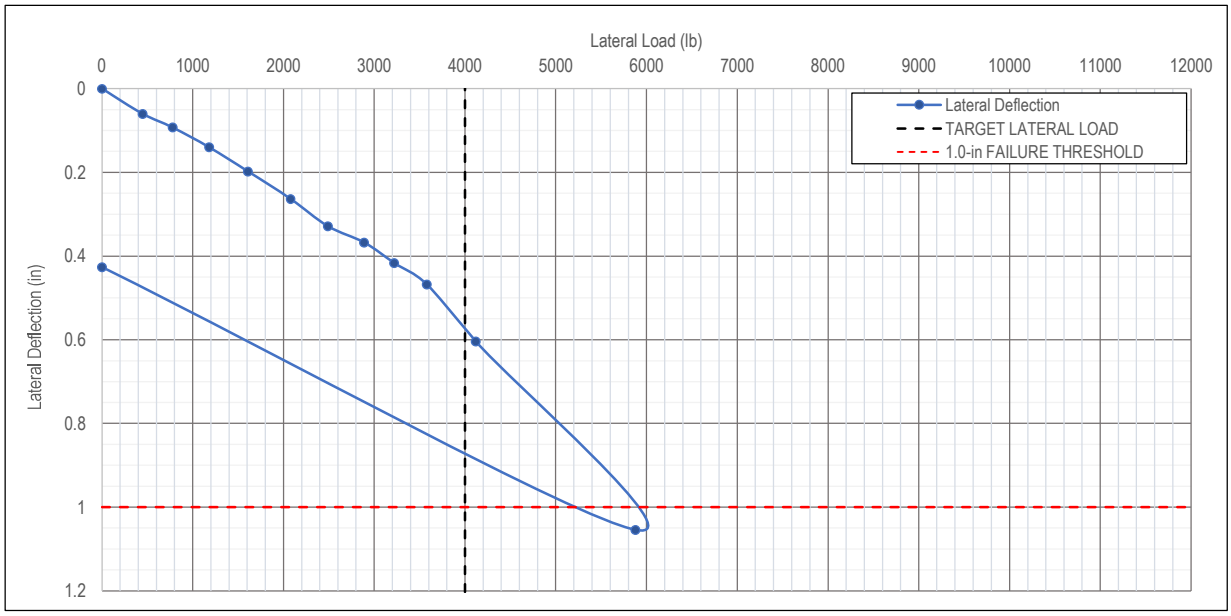
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	450	0.059	0.0615	0.06
62%	20%	800	780	0.09	0.0955	0.09
93%	30%	1,200	1180	0.135	0.1445	0.14
124%	40%	1,600	1610	0.191	0.2045	0.20
155%	50%	2,000	2080	0.254	0.2725	0.26
186%	60%	2,400	2490	0.3175	0.34	0.33
217%	70%	2,800	2890	0.355	0.3805	0.37
248%	80%	3,200	3220	0.4025	0.4305	0.42
279%	90%	3,600	3580	0.4515	0.484	0.47
310%	100%	4,000	4120	0.5845	0.6235	0.60
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5880	1.034	1.0745	1.05
0%	0%	0	0	0.43	0.423	0.43

Load at 1.0-in lateral deflection: **5,668** **lb**

Residual lateral deformation: **0.43** **in**



Notes:

Failure reached at 5,880 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-20A

Axial Uplift Load Test Results

PTL - **20** Designation: **A**

Section: W6x9

Latitude: 33.658393

Longitude: -114.68731

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 17.04

Pile Damaged? No

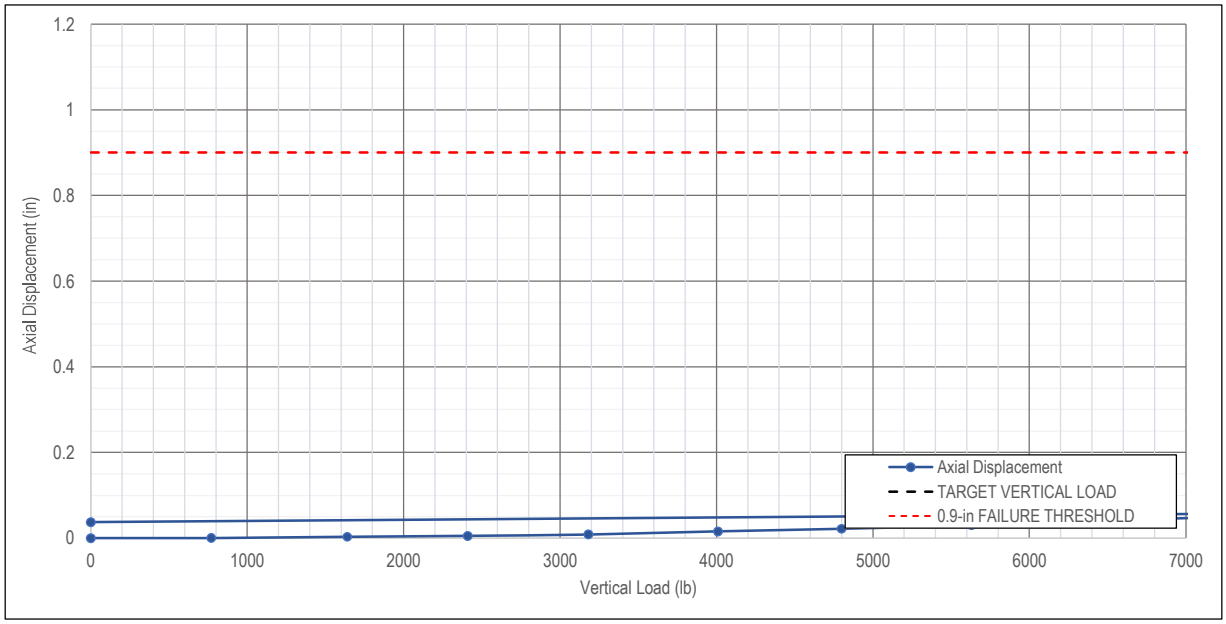
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	770	0.0005	0.0005	0.00
436%	20%	1,600	1640	0.003	0.003	0.00
654%	30%	2,400	2410	0.005	0.0055	0.01
872%	40%	3,200	3180	0.0085	0.009	0.01
1090%	50%	4,000	4010	0.0155	0.016	0.02
1308%	60%	4,800	4800	0.0215	0.022	0.02
1526%	70%	5,600	5630	0.0305	0.03	0.03
1744%	80%	6,400	6430	0.04	0.0425	0.04
1962%	90%	7,200	7220	0.046	0.052	0.05
2180%	100%	8,000	8030	0.0555	0.062	0.06
0%	0%	0	0	0.037	0.0375	0.04

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.04** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-20A

Lateral Load Test Results

PTL- **20** Designation: **A**

Section: W6x9

Latitude: 33.658393

Longitude: -114.68731

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 17.04

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

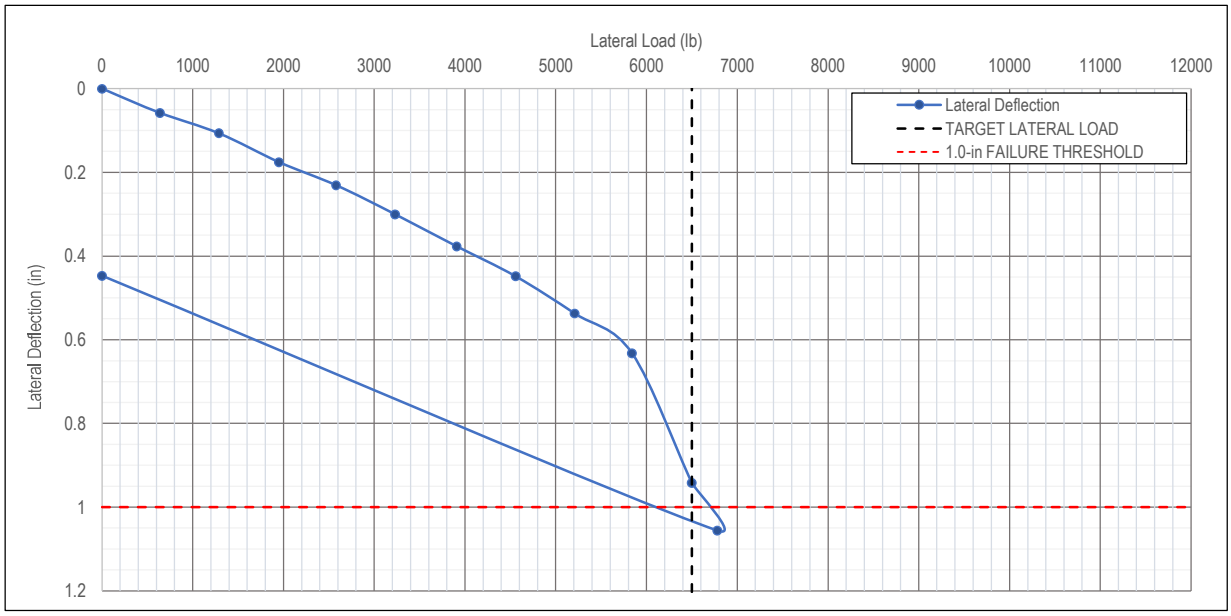
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	640	0.0615	0.0545	0.06
62%	20%	1,300	1290	0.11	0.103	0.11
93%	30%	1,950	1950	0.1805	0.1705	0.18
124%	40%	2,600	2580	0.236	0.225	0.23
155%	50%	3,250	3230	0.3065	0.294	0.30
186%	60%	3,900	3910	0.384	0.3695	0.38
217%	70%	4,550	4560	0.4555	0.441	0.45
248%	80%	5,200	5210	0.5435	0.531	0.54
279%	90%	5,850	5840	0.6355	0.6285	0.63
310%	100%	6,500	6500	0.885	0.9985	0.94
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6780	0.9965	1.115	1.06
0%	0%	0	0	0.357	0.5375	0.45

Load at 1.0-in lateral deflection: **6,643** **lb**

Residual lateral deformation: **0.45** **in**



Notes:

Failure reached at 6,780 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-20B

Axial Uplift Load Test Results

PTL - **20** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.658393 - - Saturated Soil? No

Longitude: -114.68731 Target Embedment Depth (ft): 8.00 Drive Time (s): 17.98

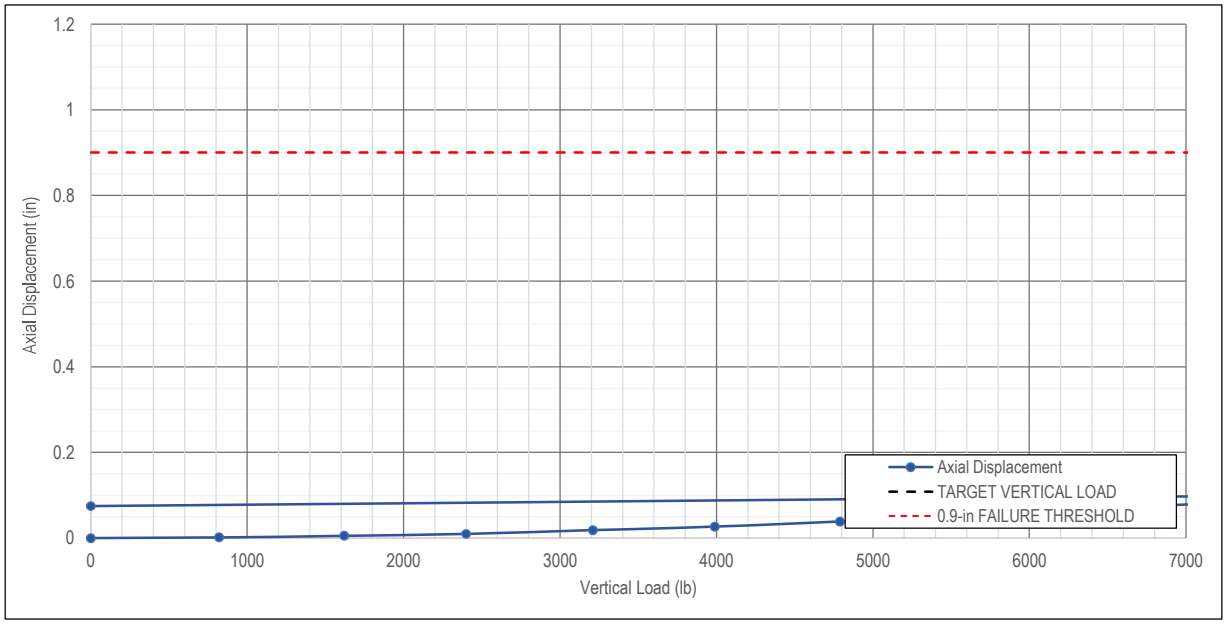
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/22/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	820	0.001	0.0025	0.00
436%	20%	1,600	1620	0.0045	0.0065	0.01
654%	30%	2,400	2400	0.0085	0.0115	0.01
872%	40%	3,200	3210	0.016	0.021	0.02
1090%	50%	4,000	3990	0.023	0.0305	0.03
1308%	60%	4,800	4790	0.0344	0.043	0.04
1526%	70%	5,600	5580	0.045	0.0545	0.05
1744%	80%	6,400	6420	0.066	0.072	0.07
1962%	90%	7,200	7210	0.08	0.085	0.08
2180%	100%	8,000	8040	0.098	0.1015	0.10
0%	0%	0	0	0.0745	0.075	0.07

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.07** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-20B

Lateral Load Test Results

PTL- **20** Designation: **B**

Section: W6x9

Latitude: 33.658393

Longitude: -114.68731

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 17.98

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

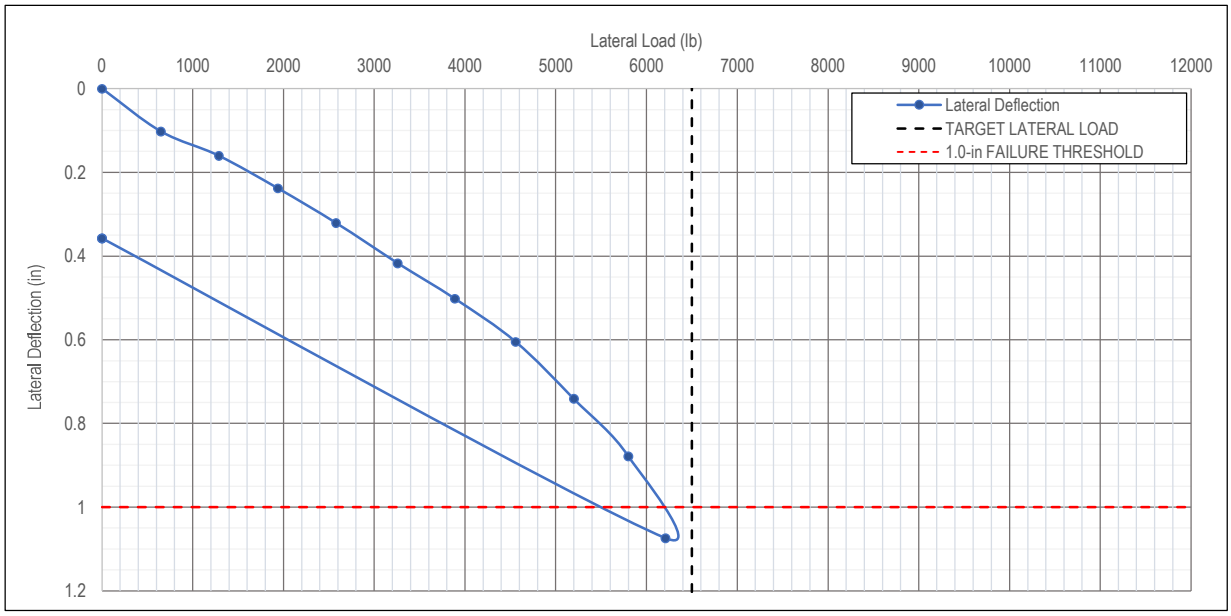
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.1065	0.098	0.10
62%	20%	1,300	1290	0.1645	0.156	0.16
93%	30%	1,950	1940	0.2435	0.2325	0.24
124%	40%	2,600	2580	0.326	0.3155	0.32
155%	50%	3,250	3260	0.422	0.412	0.42
186%	60%	3,900	3890	0.505	0.499	0.50
217%	70%	4,550	4560	0.603	0.607	0.61
248%	80%	5,200	5200	0.722	0.7605	0.74
279%	90%	5,850	5800	0.825	0.9325	0.88
310%	100%	6,500	6210	1.0415	1.1065	1.07
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.275	0.44	0.36

Load at 1.0-in lateral deflection: **6,055** **lb**

Residual lateral deformation: **0.36** **in**



Notes:

Failure reached at 6,210 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-21A

Axial Uplift Load Test Results

PTL - **21** Designation: **A**

Section: W6x9

Latitude: 33.662115

Longitude: -114.68274

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 9.8

Pile Damaged? No

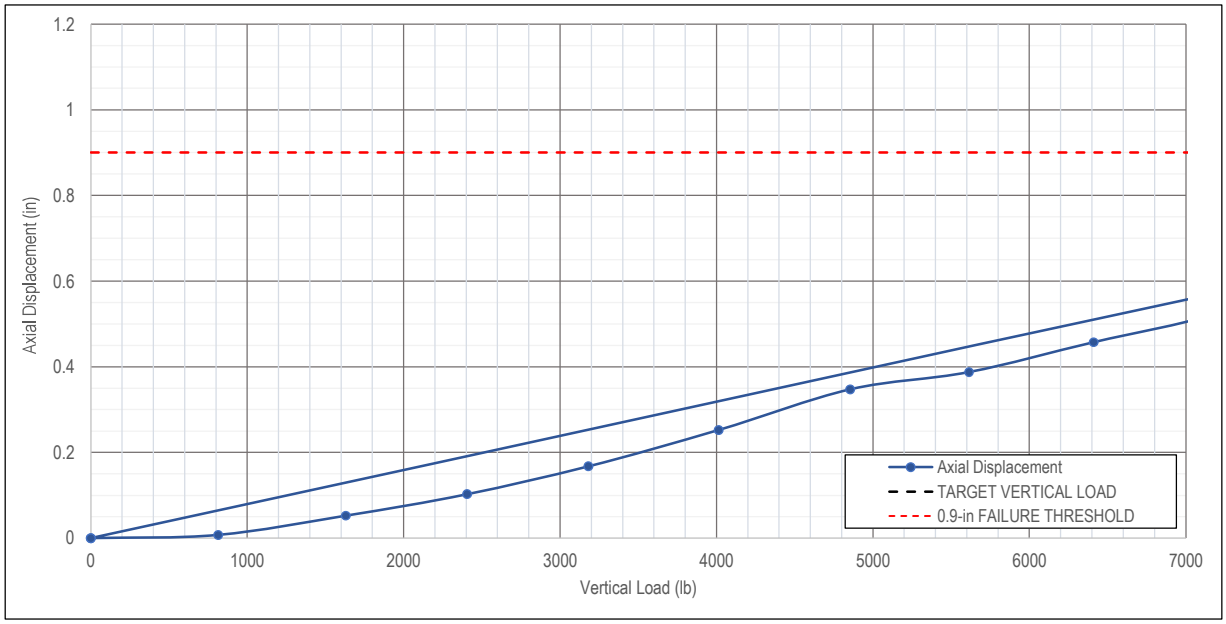
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	815	0.005	0.01	0.01
13594%	20%	1,600	1630	0.05	0.055	0.05
20391%	30%	2,400	2405	0.1	0.105	0.10
27188%	40%	3,200	3180	0.165	0.17	0.17
33985%	50%	4,000	4015	0.25	0.255	0.25
40782%	60%	4,800	4855	0.345	0.35	0.35
47579%	70%	5,600	5615	0.385	0.39	0.39
54376%	80%	6,400	6410	0.455	0.46	0.46
61173%	90%	7,200	7210	0.525	0.525	0.53
67970%	100%	8,000	8030	0.635	0.635	0.64
0%	0%	0	0			

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **TBD** **in**



Notes:

Residual deflection was not recorded.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-21A

Lateral Load Test Results

PTL- **21** Designation: **A**

Section: W6x9

Latitude: 33.662115

Longitude: -114.68274

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 9.8

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

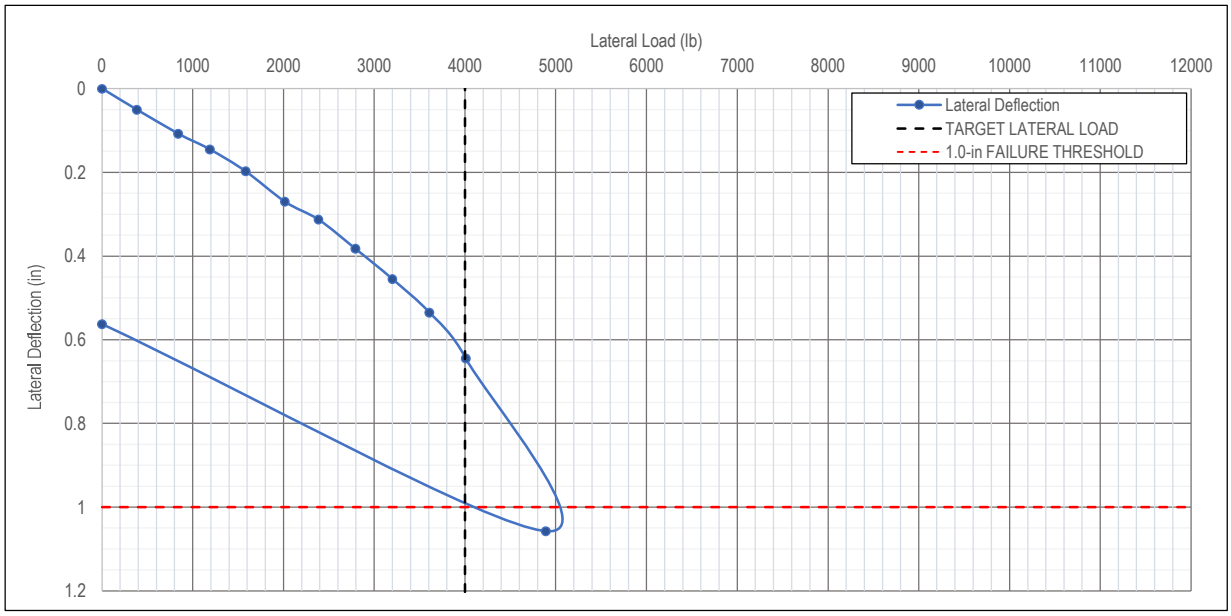
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	385	0.05	0.05	0.05
62%	20%	800	840	0.11	0.105	0.11
93%	30%	1,200	1190	0.15	0.14	0.15
124%	40%	1,600	1585	0.2	0.195	0.20
155%	50%	2,000	2015	0.27	0.27	0.27
186%	60%	2,400	2385	0.315	0.31	0.31
217%	70%	2,800	2795	0.385	0.38	0.38
248%	80%	3,200	3200	0.455	0.455	0.46
279%	90%	3,600	3610	0.535	0.535	0.54
310%	100%	4,000	4010	0.645	0.645	0.65
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			4890	1.055	1.06	1.06
0%	0%	0	0	0.56	0.565	0.56

Load at 1.0-in lateral deflection: **4,767** **lb**

Residual lateral deformation: **0.56** **in**



Notes:

Failure reached at 4,890 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-21B

Axial Uplift Load Test Results

PTL - **21** Designation: **B**

Section: W6x9

Latitude: 33.662115

Longitude: -114.68274

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 25.7

Pile Damaged? No

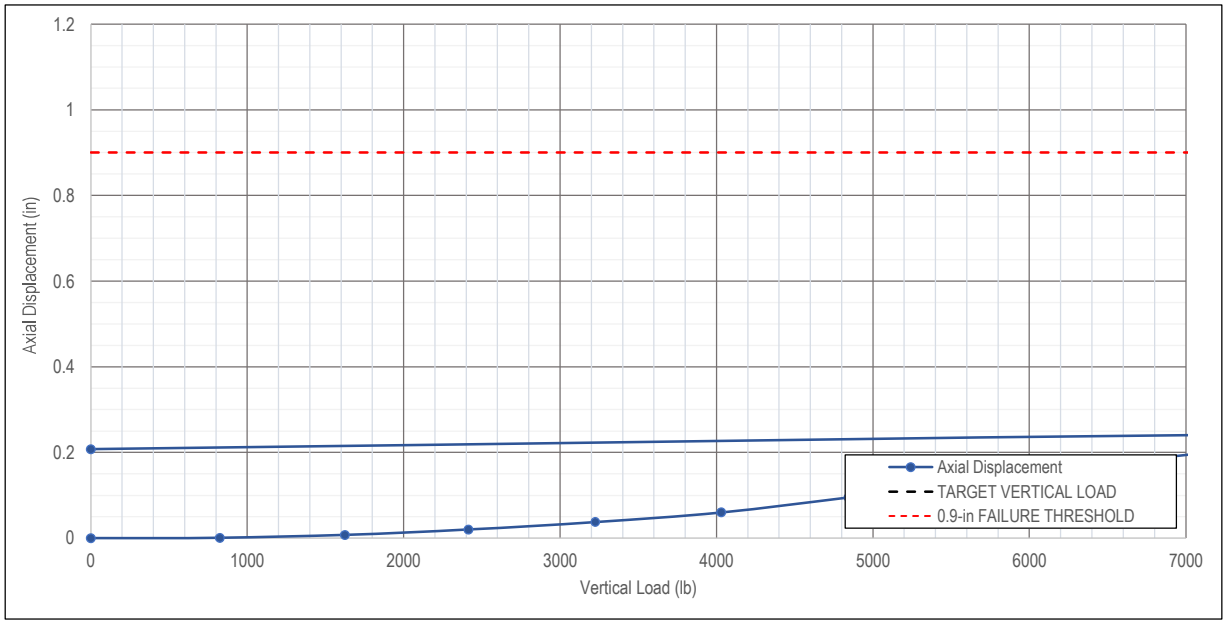
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	825	0	0.002	0.00
13594%	20%	1,600	1625	0.005	0.01	0.01
20391%	30%	2,400	2415	0.02	0.02	0.02
27188%	40%	3,200	3225	0.035	0.04	0.04
33985%	50%	4,000	4030	0.06	0.06	0.06
40782%	60%	4,800	4845	0.095	0.095	0.10
47579%	70%	5,600	5640	0.13	0.13	0.13
54376%	80%	6,400	6405	0.17	0.17	0.17
61173%	90%	7,200	7200	0.205	0.2	0.20
67970%	100%	8,000	8020	0.245	0.24	0.24
0%	0%	0	0	0.21	0.205	0.21

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.21** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-21B

Lateral Load Test Results

PTL- **21** Designation: **B**

Section: W6x9

Latitude: 33.662115

Longitude: -114.68274

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 25.7

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

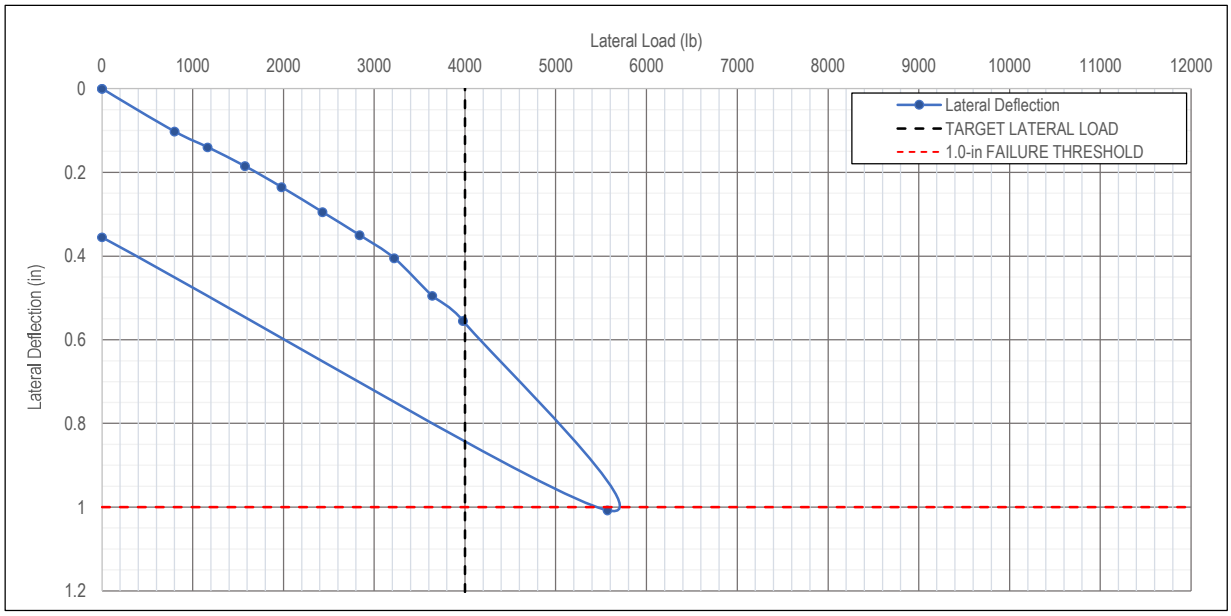
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	0	0	0	0.00
62%	20%	800	800	0.105	0.1	0.10
93%	30%	1,200	1165	0.145	0.135	0.14
124%	40%	1,600	1575	0.19	0.18	0.19
155%	50%	2,000	1980	0.24	0.23	0.24
186%	60%	2,400	2430	0.3	0.29	0.30
217%	70%	2,800	2840	0.355	0.345	0.35
248%	80%	3,200	3220	0.41	0.4	0.41
279%	90%	3,600	3640	0.5	0.49	0.50
310%	100%	4,000	3980	0.56	0.55	0.56
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5570	1	1.015	1.01
0%	0%	0	0	0.35	0.36	0.36

Load at 1.0-in lateral deflection: **5,544** **lb**

Residual lateral deformation: **0.36** **in**



Notes:

Failure reached at 5,570 lbs. 400-lbs load step was skipped.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-21C

Axial Compression Load Test Results (supplemental pile)

PTL - **21** Designation: **C**

Section: W6x9

Latitude: 33.662115

Longitude: -114.68274

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **15,000** (≈ 1180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 8.6

Pile Damaged? No

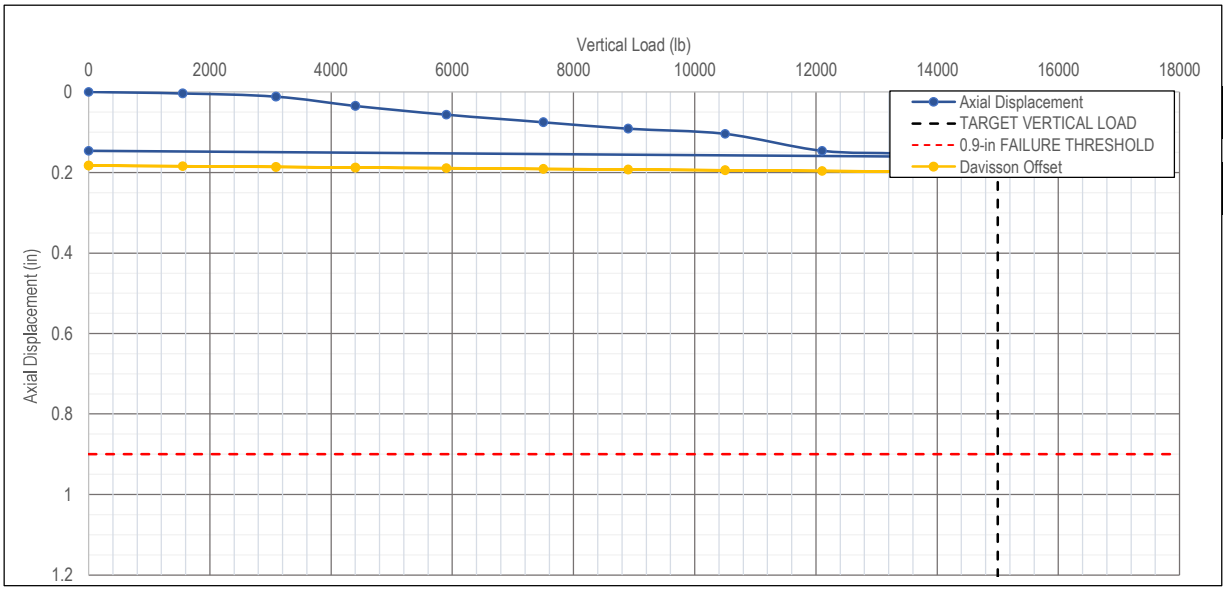
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL COMPRESSION** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
118%	10%	1,500	1550	0.0035	0.0035	0.00
236%	20%	3,000	3090	0.0115	0.0115	0.01
354%	30%	4,500	4400	0.0345	0.035	0.03
472%	40%	6,000	5900	0.0555	0.0575	0.06
590%	50%	7,500	7500	0.074	0.076	0.08
708%	60%	9,000	8900	0.089	0.093	0.09
826%	70%	10,500	10500	0.101	0.1075	0.10
944%	80%	12,000	12100	0.1435	0.149	0.15
1062%	90%	13,500	13450	0.15	0.156	0.15
1180%	100%	15,000	15060	0.1585	0.166	0.16
0%	0%	0	0	0.1465	0.146	0.15

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.15** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-22A

Axial Uplift Load Test Results

PTL - **22** Designation: **A** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.655097 - - Saturated Soil? No

Longitude: -114.70778 Target Embedment Depth (ft): 6.00 Drive Time (s): 19.55

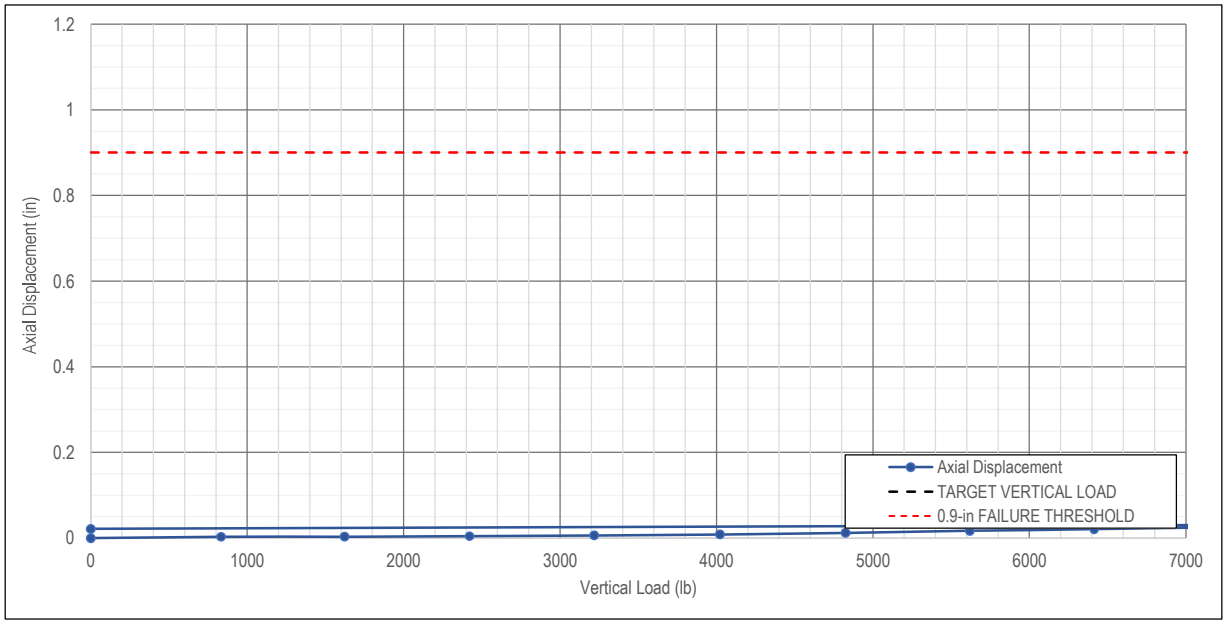
Tested By: ANS Actual Embedment Depth (ft): 6 Pile Damaged? No

Test Date: 10/19/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	833	0.003	0.003	0.00
436%	20%	1,600	1623	0.0035	0.0035	0.00
654%	30%	2,400	2421	0.0045	0.0045	0.00
872%	40%	3,200	3219	0.0065	0.0065	0.01
1090%	50%	4,000	4022	0.009	0.0085	0.01
1308%	60%	4,800	4825	0.013	0.0115	0.01
1526%	70%	5,600	5620	0.018	0.016	0.02
1744%	80%	6,400	6415	0.0225	0.0195	0.02
1962%	90%	7,200	7228	0.027	0.024	0.03
2180%	100%	8,000	8031	0.0335	0.0295	0.03
0%	0%	0	0	0.0215	0.0215	0.02

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.02** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-22A

Lateral Load Test Results

PTL- **22** Designation: **A**

Section: W6x9

Latitude: 33.655097

Longitude: -114.70778

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 19.55

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

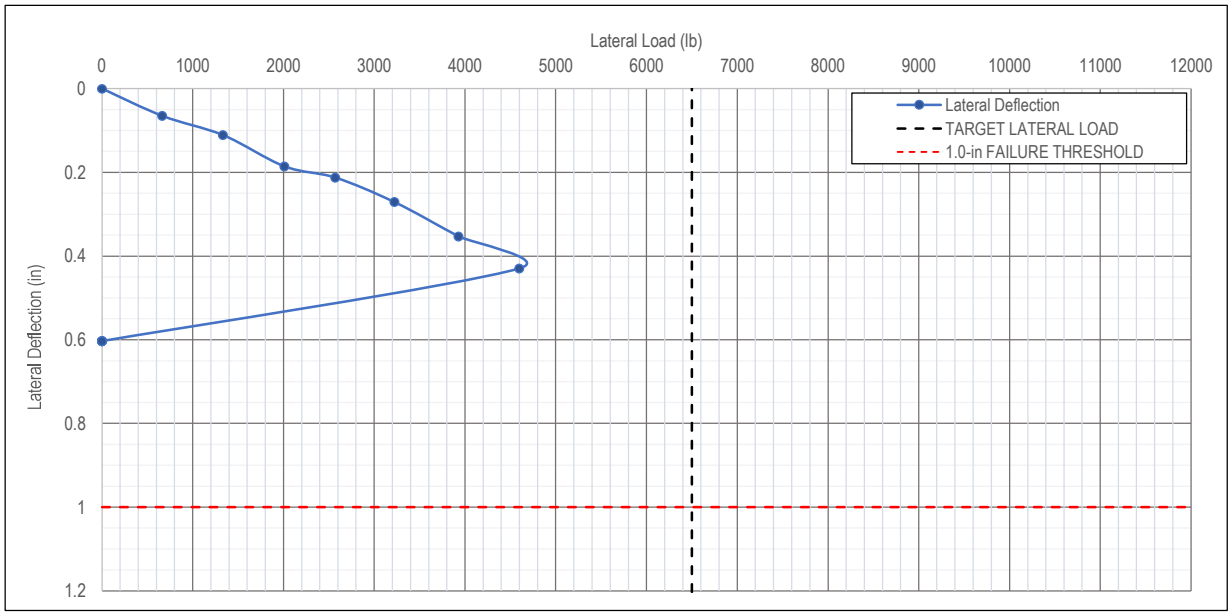
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	665	0.0625	0.0675	0.07
62%	20%	1,300	1331	0.1065	0.115	0.11
93%	30%	1,950	2010	0.1785	0.1925	0.19
124%	40%	2,600	2571	0.204	0.221	0.21
155%	50%	3,250	3223	0.2585	0.2835	0.27
186%	60%	3,900	3929	0.3285	0.378	0.35
217%	70%	4,550	4598	0.3845	0.474	0.43
248%	80%	5,200				
279%	90%	5,850				
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0		0.603	0.60

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.60** **in**



Notes:

Pile twisting and bending caused gauge to slip off at 4,600 lbs. No reliable test results available after this point.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-22B

Axial Uplift Load Test Results

PTL - **22** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.655097 - - Saturated Soil? No

Longitude: -114.70778 Target Embedment Depth (ft): 8.00 Drive Time (s): 23.64

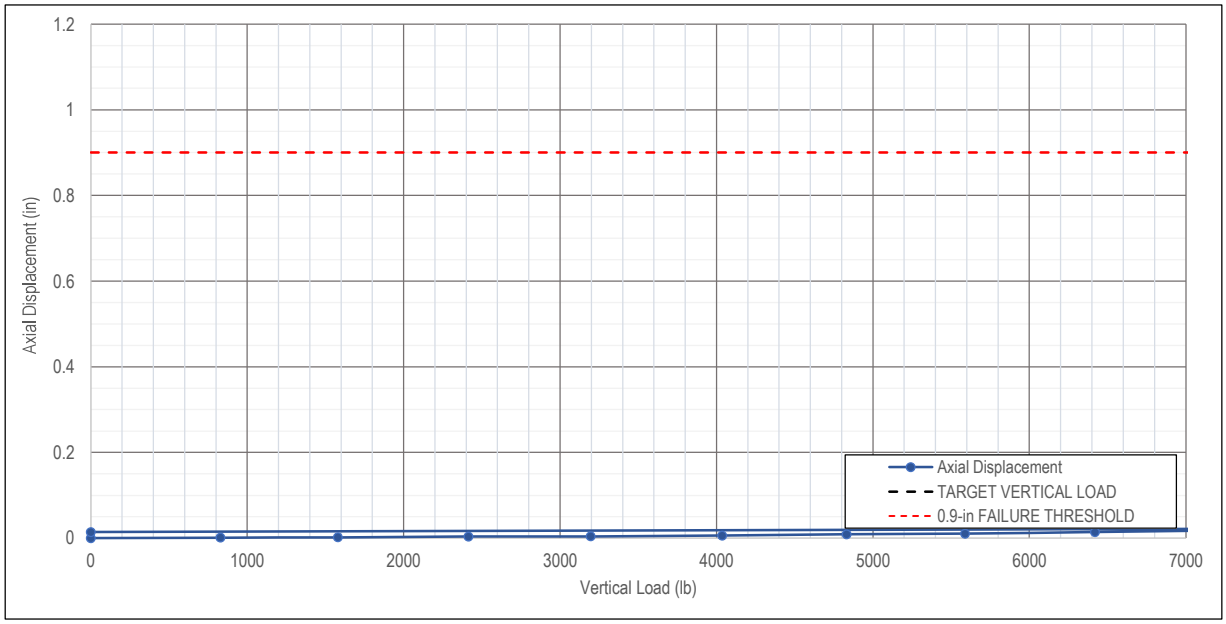
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/19/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	829	0.001	0.0015	0.00
436%	20%	1,600	1580	0.0015	0.0025	0.00
654%	30%	2,400	2415	0.003	0.0045	0.00
872%	40%	3,200	3197	0.0035	0.005	0.00
1090%	50%	4,000	4037	0.0055	0.007	0.01
1308%	60%	4,800	4832	0.0085	0.0095	0.01
1526%	70%	5,600	5590	0.01	0.011	0.01
1744%	80%	6,400	6420	0.014	0.0145	0.01
1962%	90%	7,200	7241	0.018	0.018	0.02
2180%	100%	8,000	8000	0.0225	0.0225	0.02
0%	0%	0	0	0.014	0.014	0.01

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-22B

Lateral Load Test Results

PTL- **22** Designation: **B**

Section: W6x9

Latitude: 33.655097

Longitude: -114.70778

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 23.64

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

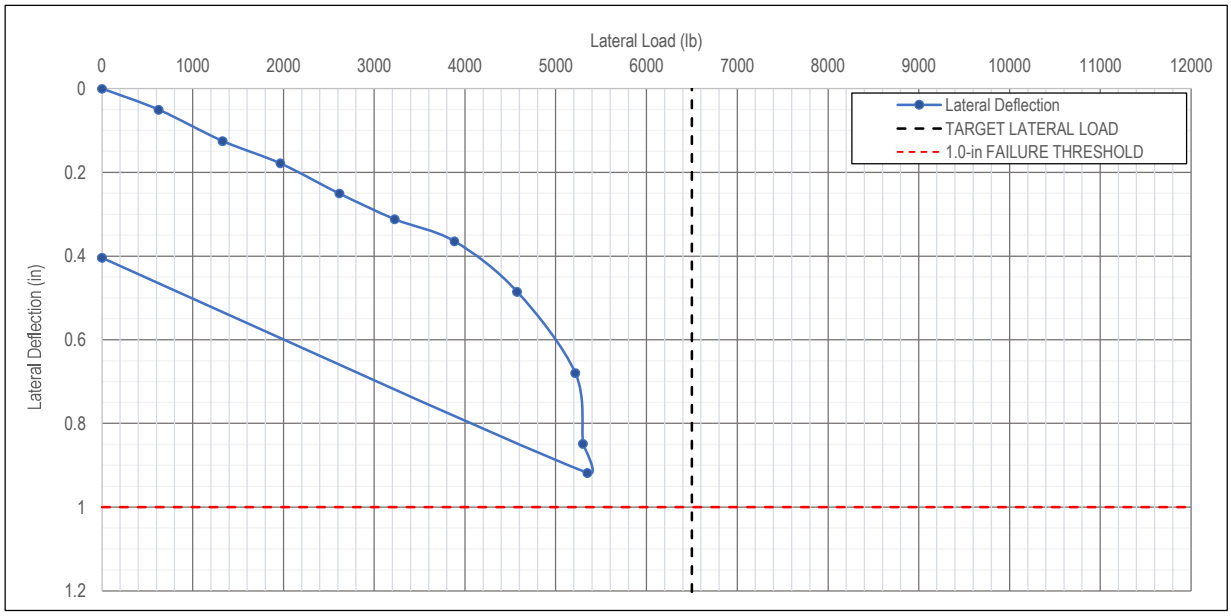
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	625	0.05	0.051	0.05
62%	20%	1,300	1330	0.125	0.125	0.13
93%	30%	1,950	1965	0.1785	0.1785	0.18
124%	40%	2,600	2615	0.2515	0.2495	0.25
155%	50%	3,250	3225	0.314	0.3095	0.31
186%	60%	3,900	3885	0.3675	0.362	0.36
217%	70%	4,550	4575	0.494	0.476	0.49
248%	80%	5,200	5215	0.7415	0.6175	0.68
279%	90%	5,850	5300	0.9255	0.771	0.85
310%	100%	6,500	5350	1.061	0.776	0.92
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.5325	0.2755	0.40

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.40** **in**



Notes:

Pile twisting caused gauge to slip off at 5,350 lbs. No reliable test results available after this point.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-23A

Axial Uplift Load Test Results

PTL - **23** Designation: **A**

Section: W6x9

Latitude: 33.654942

Longitude: -114.70425

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 9.77

Pile Damaged? No

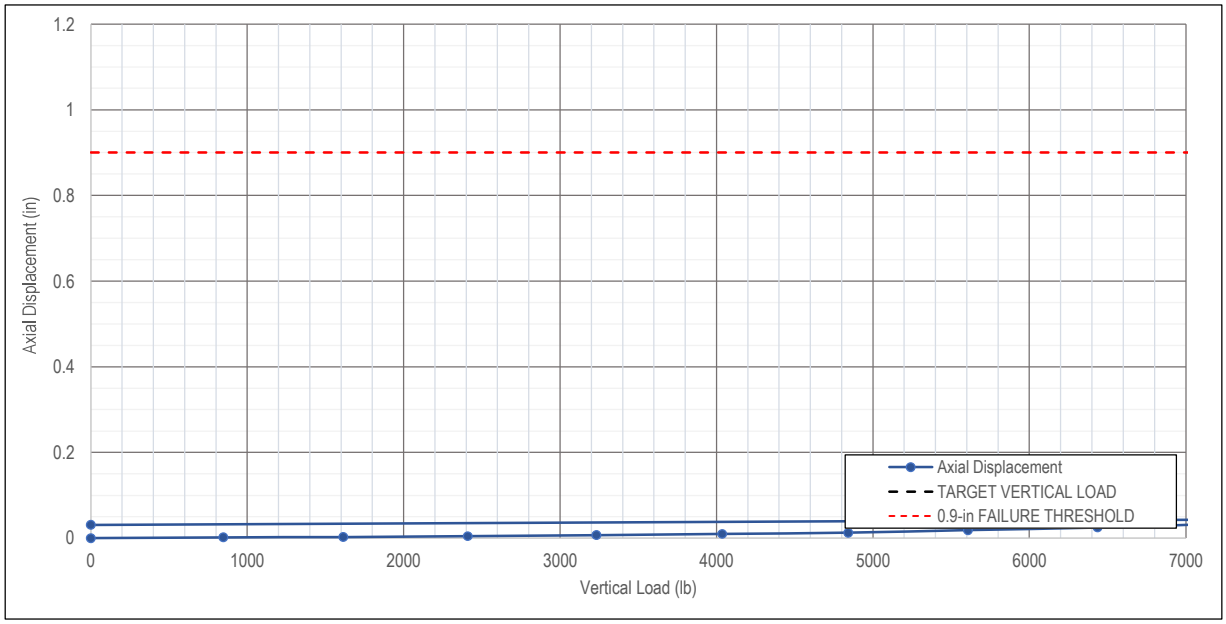
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	847	0.002	0.0015	0.00
13594%	20%	1,600	1614	0.003	0.0025	0.00
20391%	30%	2,400	2409	0.0055	0.004	0.00
27188%	40%	3,200	3233	0.008	0.006	0.01
33985%	50%	4,000	4037	0.01	0.01	0.01
40782%	60%	4,800	4842	0.013	0.013	0.01
47579%	70%	5,600	5608	0.019	0.018	0.02
54376%	80%	6,400	6436	0.0255	0.0245	0.03
61173%	90%	7,200	7224	0.0345	0.0325	0.03
67970%	100%	8,000	7880	0.045	0.0425	0.04
0%	0%	0	0	0.031	0.031	0.03

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-23A

Lateral Load Test Results

PTL- **23** Designation: **A**

Section: W6x9

Latitude: 33.654942

Longitude: -114.70425

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 9.77

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

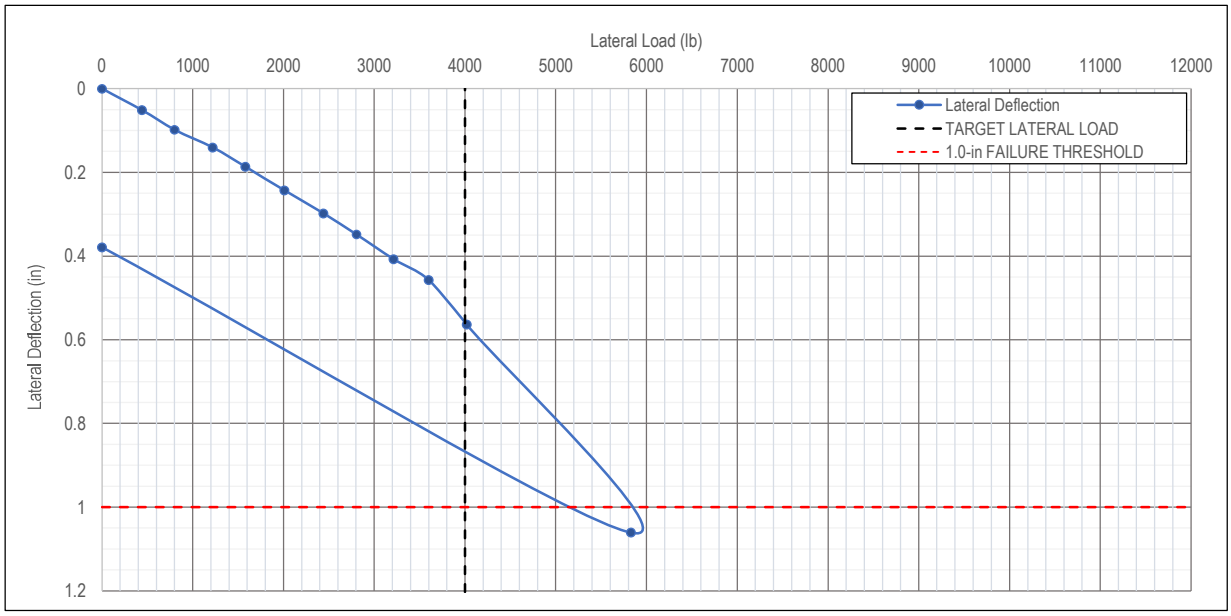
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	440	0.049	0.053	0.05
62%	20%	800	800	0.0945	0.1015	0.10
93%	30%	1,200	1220	0.139	0.142	0.14
124%	40%	1,600	1580	0.1845	0.1885	0.19
155%	50%	2,000	2010	0.2415	0.2445	0.24
186%	60%	2,400	2440	0.2965	0.2995	0.30
217%	70%	2,800	2805	0.346	0.3505	0.35
248%	80%	3,200	3215	0.4035	0.4115	0.41
279%	90%	3,600	3600	0.4515	0.463	0.46
310%	100%	4,000	4020	0.555	0.5725	0.56
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5830	0.987	1.135	1.06
0%	0%	0	0	0.3365	0.4215	0.38

Load at 1.0-in lateral deflection: **5,608** **lb**

Residual lateral deformation: **0.38** **in**



Notes:

Failure reached at 5,830 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-23B

Axial Uplift Load Test Results

PTL - **23** Designation: **B**

Section: W6x9

Latitude: 33.654942

Longitude: -114.70425

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 25.62

Pile Damaged? No

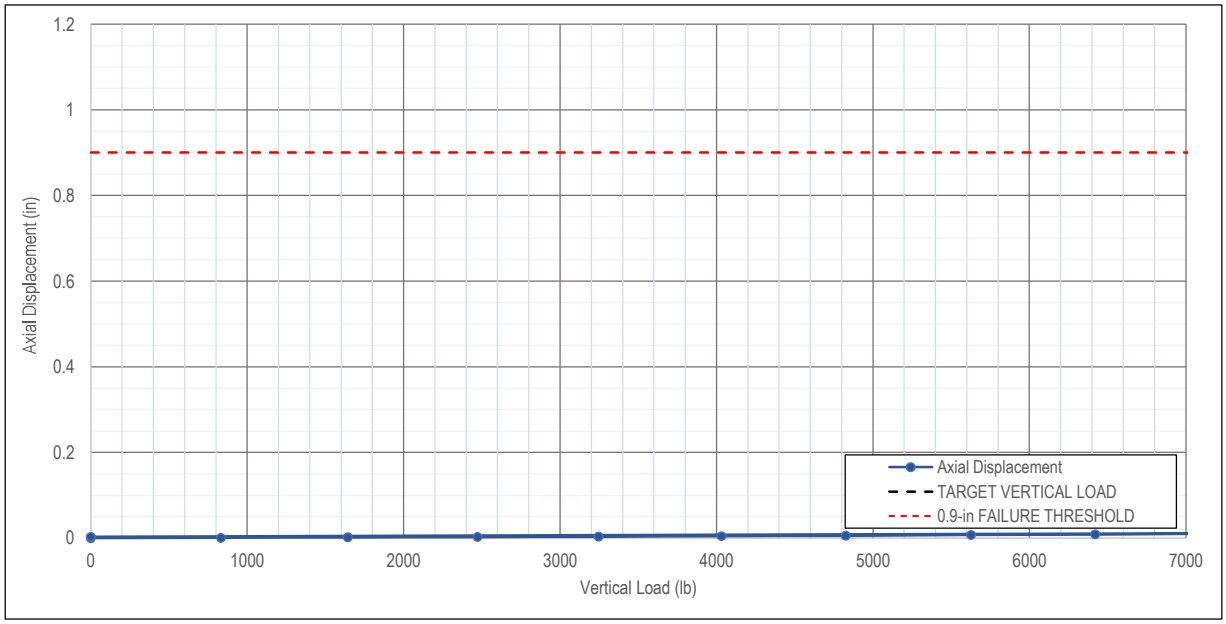
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	832	0.001	0.001	0.00
13594%	20%	1,600	1643	0.002	0.002	0.00
20391%	30%	2,400	2472	0.0025	0.003	0.00
27188%	40%	3,200	3246	0.0035	0.0035	0.00
33985%	50%	4,000	4031	0.005	0.0045	0.00
40782%	60%	4,800	4826	0.0065	0.005	0.01
47579%	70%	5,600	5627	0.0085	0.0065	0.01
54376%	80%	6,400	6421	0.01	0.0075	0.01
61173%	90%	7,200	7228	0.0115	0.0095	0.01
67970%	100%	8,000	8013	0.014	0.0105	0.01
0%	0%	0	0	0.0035	0.003	0.00

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.00** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-23B

Lateral Load Test Results

PTL- **23** Designation: **B**

Section: W6x9

Latitude: 33.654942

Longitude: -114.70425

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 25.62

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

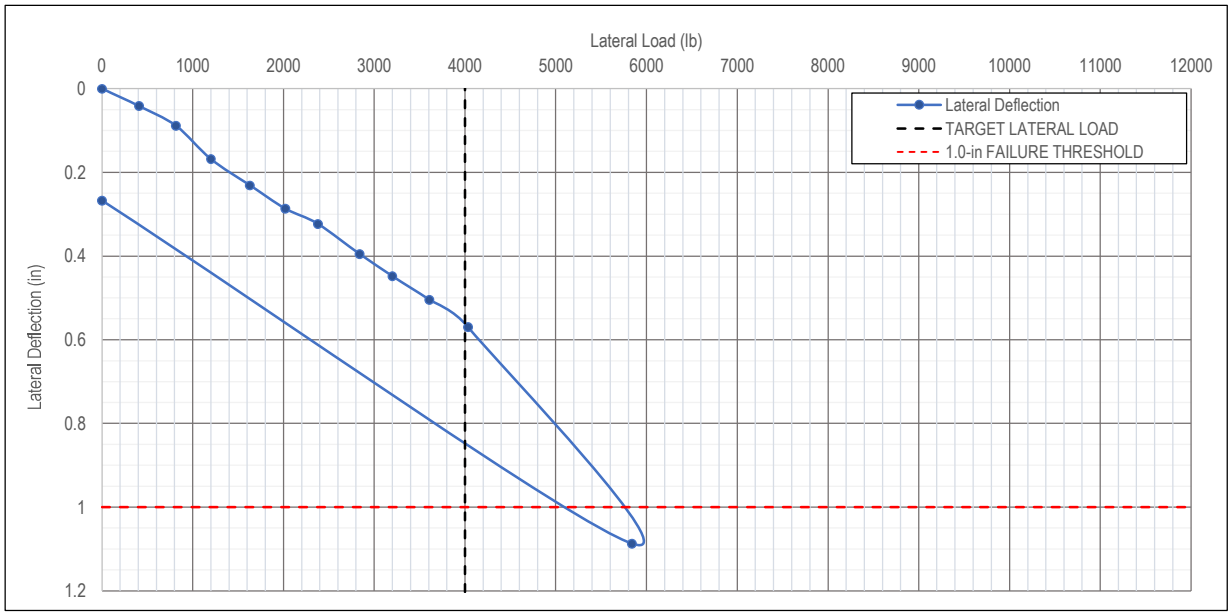
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	410	0.0395	0.0435	0.04
62%	20%	800	815	0.086	0.091	0.09
93%	30%	1,200	1200	0.164	0.172	0.17
124%	40%	1,600	1630	0.2215	0.2405	0.23
155%	50%	2,000	2020	0.278	0.2945	0.29
186%	60%	2,400	2380	0.312	0.3345	0.32
217%	70%	2,800	2840	0.386	0.404	0.40
248%	80%	3,200	3200	0.4325	0.4635	0.45
279%	90%	3,600	3610	0.4915	0.5175	0.50
310%	100%	4,000	4035	0.558	0.582	0.57
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5840	0.973	1.2015	1.09
0%	0%	0	0	0.224	0.311	0.27

Load at 1.0-in lateral deflection: **5,536** **lb**

Residual lateral deformation: **0.27** **in**



Notes:

Failure reached at 5,840 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-24A

Axial Uplift Load Test Results

PTL - **24** Designation: **A**

Section: W6x9

Latitude: 33.655291

Longitude: -114.69941

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 8.96

Pile Damaged? No

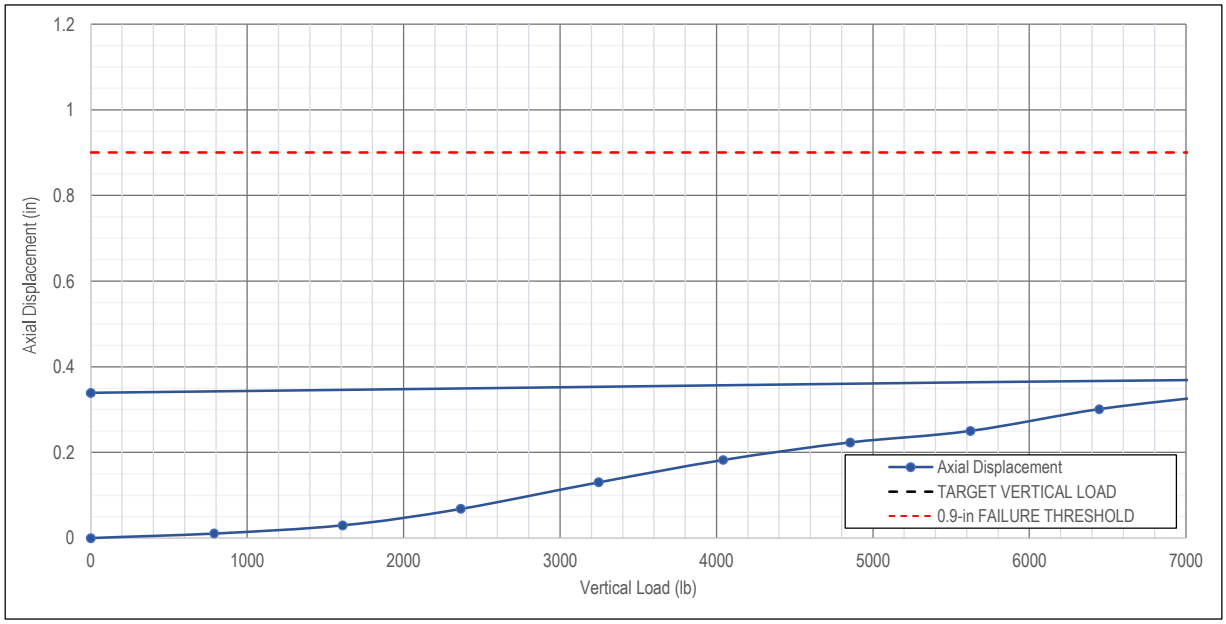
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	789	0.0105	0.011	0.01
436%	20%	1,600	1611	0.029	0.0305	0.03
654%	30%	2,400	2365	0.0675	0.069	0.07
872%	40%	3,200	3247	0.129	0.1315	0.13
1090%	50%	4,000	4042	0.1805	0.1845	0.18
1308%	60%	4,800	4855	0.2215	0.225	0.22
1526%	70%	5,600	5623	0.248	0.2525	0.25
1744%	80%	6,400	6447	0.2995	0.3025	0.30
1962%	90%	7,200	7254	0.3345	0.3375	0.34
2180%	100%	8,000	7980	0.369	0.3725	0.37
0%	0%	0	0	0.339	0.339	0.34

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.34** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-24A

Lateral Load Test Results

PTL- **24** Designation: **A**

Section: W6x9

Latitude: 33.655291

Longitude: -114.69941

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 8.96

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

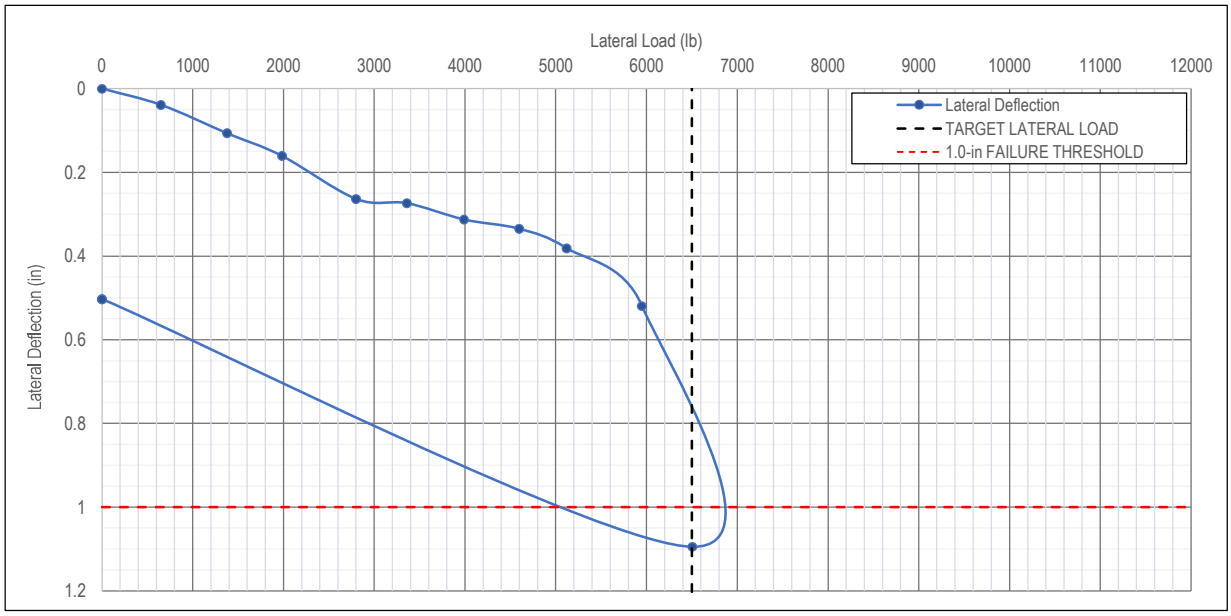
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.047	0.0305	0.04
62%	20%	1,300	1380	0.117	0.096	0.11
93%	30%	1,950	1985	0.1715	0.15	0.16
124%	40%	2,600	2800	0.274	0.253	0.26
155%	50%	3,250	3360	0.286	0.2615	0.27
186%	60%	3,900	3990	0.324	0.3015	0.31
217%	70%	4,550	4597	0.344	0.325	0.33
248%	80%	5,200	5120	0.388	0.375	0.38
279%	90%	5,850	5949	0.506	0.5325	0.52
310%	100%	6,500	6507	1.08	1.1095	1.09
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.381	0.6255	0.50

Load at 1.0-in lateral deflection: **6,415** **lb**

Residual lateral deformation: **0.50** **in**



Notes:

Failure reached at 6,507 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-24B

Axial Uplift Load Test Results

PTL - **24** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.655291 - - Saturated Soil? No

Longitude: -114.69941 Target Embedment Depth (ft): 8.00 Drive Time (s): 19.79

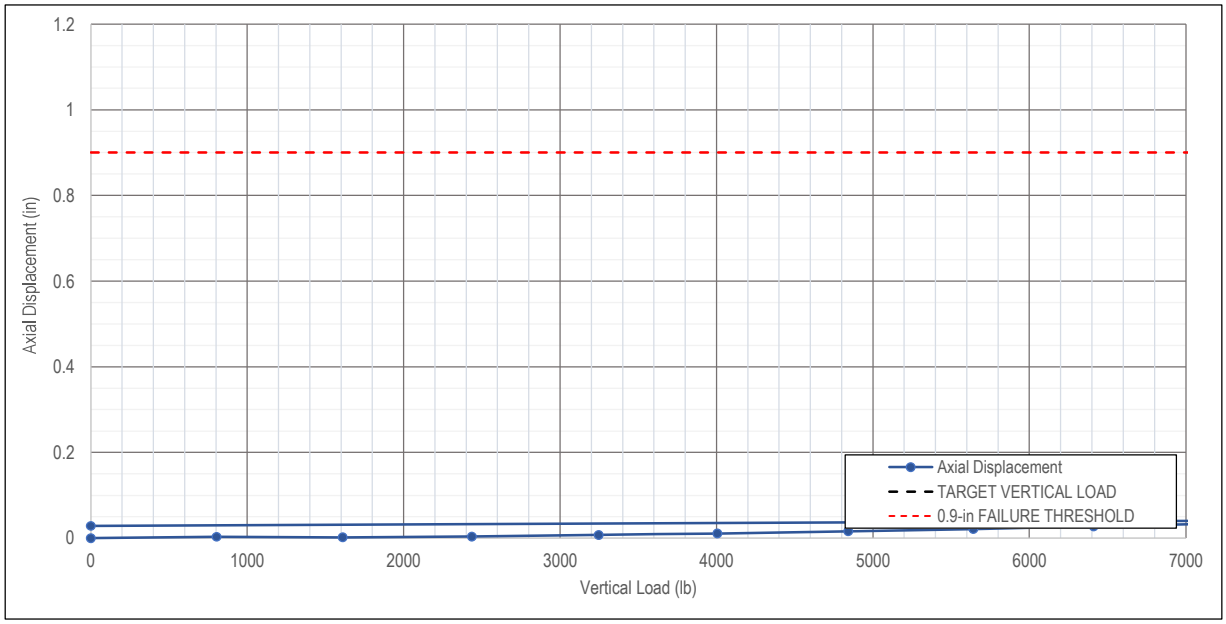
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/19/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	805	0.001	0.005	0.00
436%	20%	1,600	1611	0.0025	0.0015	0.00
654%	30%	2,400	2436	0.0045	0.003	0.00
872%	40%	3,200	3247	0.008	0.0075	0.01
1090%	50%	4,000	4006	0.0115	0.0105	0.01
1308%	60%	4,800	4842	0.017	0.015	0.02
1526%	70%	5,600	5642	0.0225	0.02	0.02
1744%	80%	6,400	6410	0.029	0.0265	0.03
1962%	90%	7,200	7202	0.0355	0.0315	0.03
2180%	100%	8,000	7970	0.0435	0.039	0.04
0%	0%	0	0	0.029	0.0285	0.03

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-24B

Lateral Load Test Results

PTL- **24** Designation: **B**

Section: W6x9

Latitude: 33.655291

Longitude: -114.69941

Tested By: ANS

Test Date: 10/19/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 19.79

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

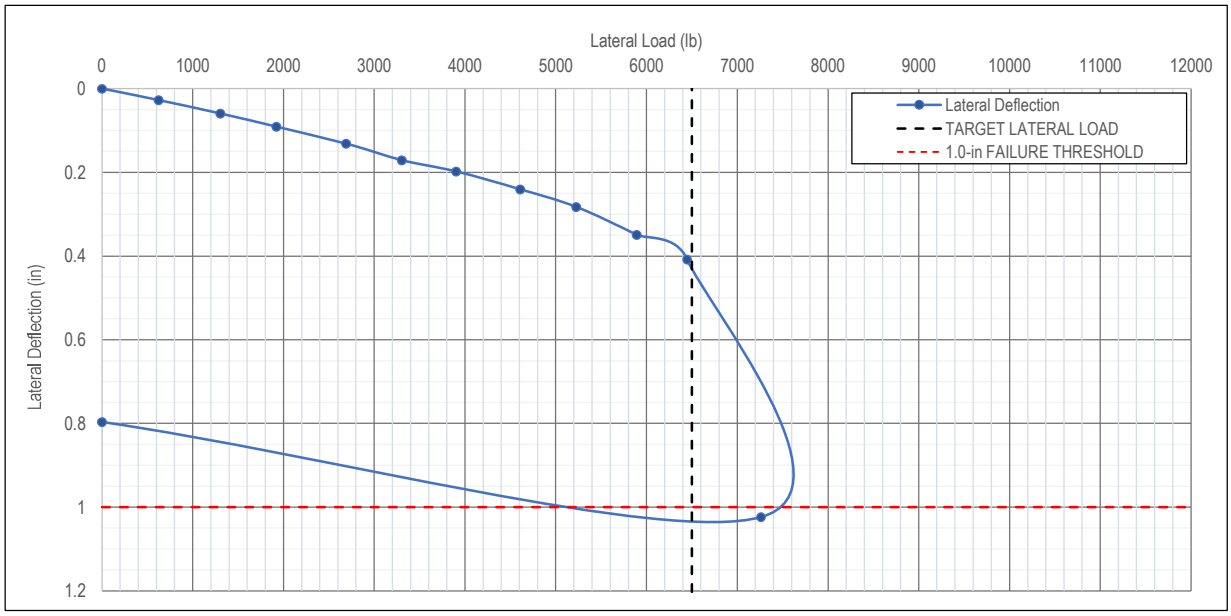
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	625	0.028	0.027	0.03
62%	20%	1,300	1305	0.0605	0.0585	0.06
93%	30%	1,950	1923	0.0915	0.09	0.09
124%	40%	2,600	2690	0.132	0.1305	0.13
155%	50%	3,250	3305	0.1715	0.17	0.17
186%	60%	3,900	3903	0.199	0.1965	0.20
217%	70%	4,550	4609	0.24	0.241	0.24
248%	80%	5,200	5226	0.28	0.285	0.28
279%	90%	5,850	5892	0.34	0.358	0.35
310%	100%	6,500	6449	0.392	0.424	0.41
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7263	1.0035	1.0445	1.02
0%	0%	0	0	0.7625	0.8305	0.80

Load at 1.0-in lateral deflection: **7,231** **lb**

Residual lateral deformation: **0.80** **in**



Notes:

Failure reached at 7,263 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-25A

Axial Uplift Load Test Results

PTL - **25** Designation: **A**

Section: W6x9

Latitude: 33.657074

Longitude: -114.69727

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 7.25

Pile Damaged? No

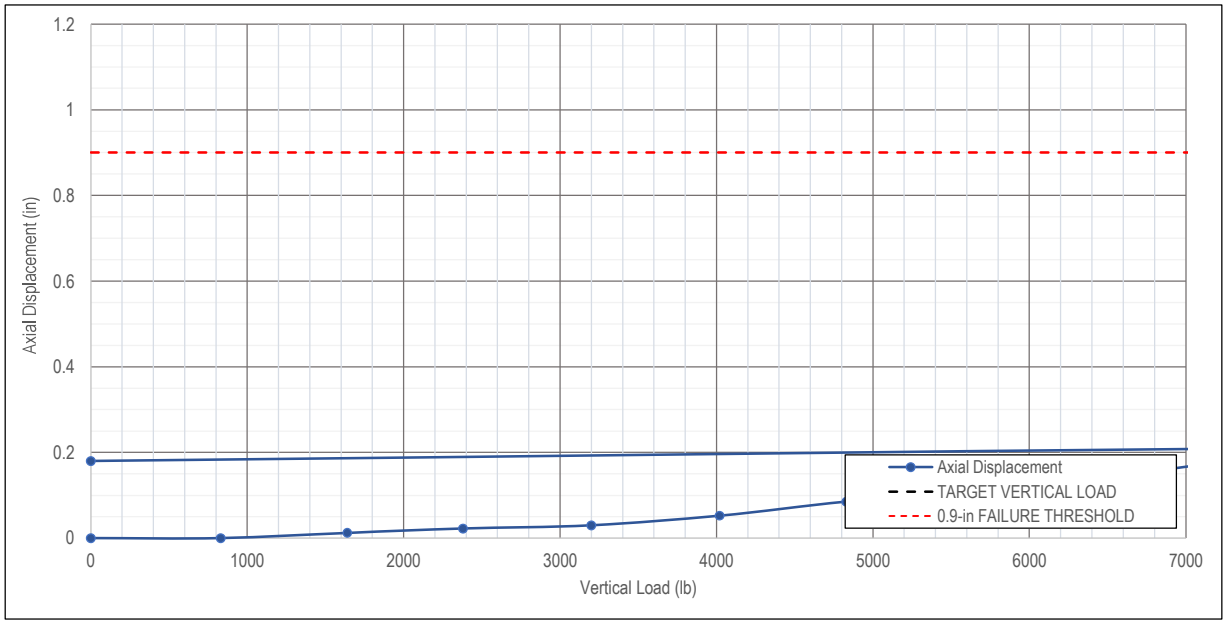
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	830	0	0	0.00
13594%	20%	1,600	1640	0.015	0.01	0.01
20391%	30%	2,400	2380	0.025	0.02	0.02
27188%	40%	3,200	3200	0.03	0.03	0.03
33985%	50%	4,000	4020	0.055	0.05	0.05
40782%	60%	4,800	4830	0.085	0.085	0.09
47579%	70%	5,600	5620	0.105	0.105	0.11
54376%	80%	6,400	6450	0.14	0.14	0.14
61173%	90%	7,200	7230	0.18	0.175	0.18
67970%	100%	8,000	8020	0.21	0.21	0.21
0%	0%	0	0	0.18	0.18	0.18

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.18** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-25A

Lateral Load Test Results

PTL- **25** Designation: **A**

Section: W6x9

Latitude: 33.657074

Longitude: -114.69727

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 7.25

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

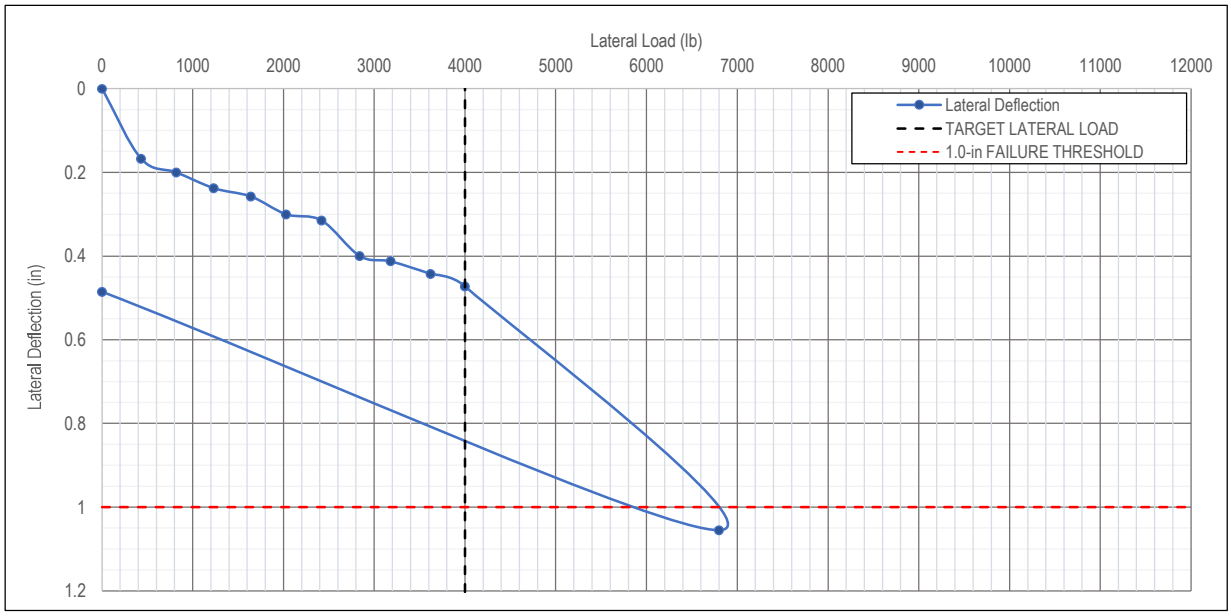
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	430	0.175	0.16	0.17
62%	20%	800	820	0.21	0.19	0.20
93%	30%	1,200	1230	0.245	0.23	0.24
124%	40%	1,600	1640	0.265	0.25	0.26
155%	50%	2,000	2030	0.31	0.29	0.30
186%	60%	2,400	2420	0.325	0.305	0.32
217%	70%	2,800	2840	0.41	0.39	0.40
248%	80%	3,200	3180	0.425	0.4	0.41
279%	90%	3,600	3620	0.455	0.43	0.44
310%	100%	4,000	4000	0.485	0.46	0.47
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6800	1.04	1.07	1.06
0%	0%	0	0	0.47	0.5	0.49

Load at 1.0-in lateral deflection: **6,536** **lb**

Residual lateral deformation: **0.49** **in**



Notes:

Failure reached at 6,800 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-25B

Axial Uplift Load Test Results

PTL - **25** Designation: **B**

Section: W6x9

Latitude: 33.657074

Longitude: -114.69727

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 17.11

Pile Damaged? No

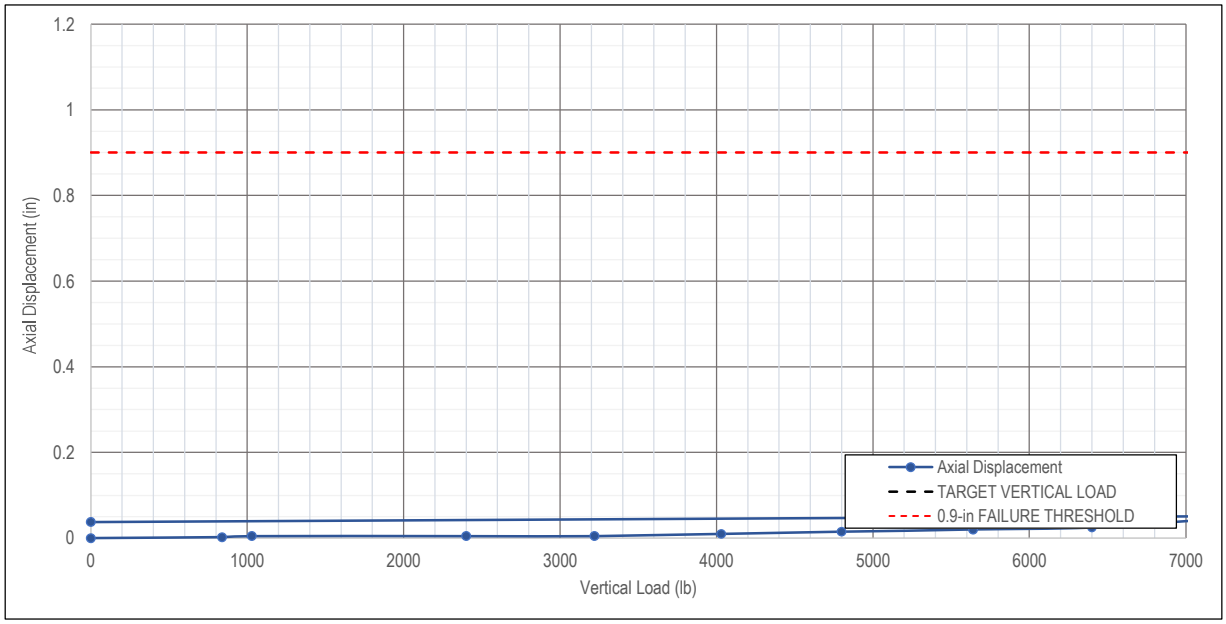
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	840	0.005	0	0.00
13594%	20%	1,600	1030	0.005	0.005	0.01
20391%	30%	2,400	2400	0.005	0.005	0.01
27188%	40%	3,200	3220	0.005	0.005	0.01
33985%	50%	4,000	4030	0.01	0.01	0.01
40782%	60%	4,800	4800	0.015	0.015	0.02
47579%	70%	5,600	5640	0.02	0.02	0.02
54376%	80%	6,400	6400	0.025	0.025	0.03
61173%	90%	7,200	7240	0.04	0.05	0.05
67970%	100%	8,000	8000	0.045	0.06	0.05
0%	0%	0	0	0.035	0.04	0.04

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.04** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-25B

Lateral Load Test Results

PTL- **25** Designation: **B**

Section: W6x9

Latitude: 33.657074

Longitude: -114.69727

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 17.11

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

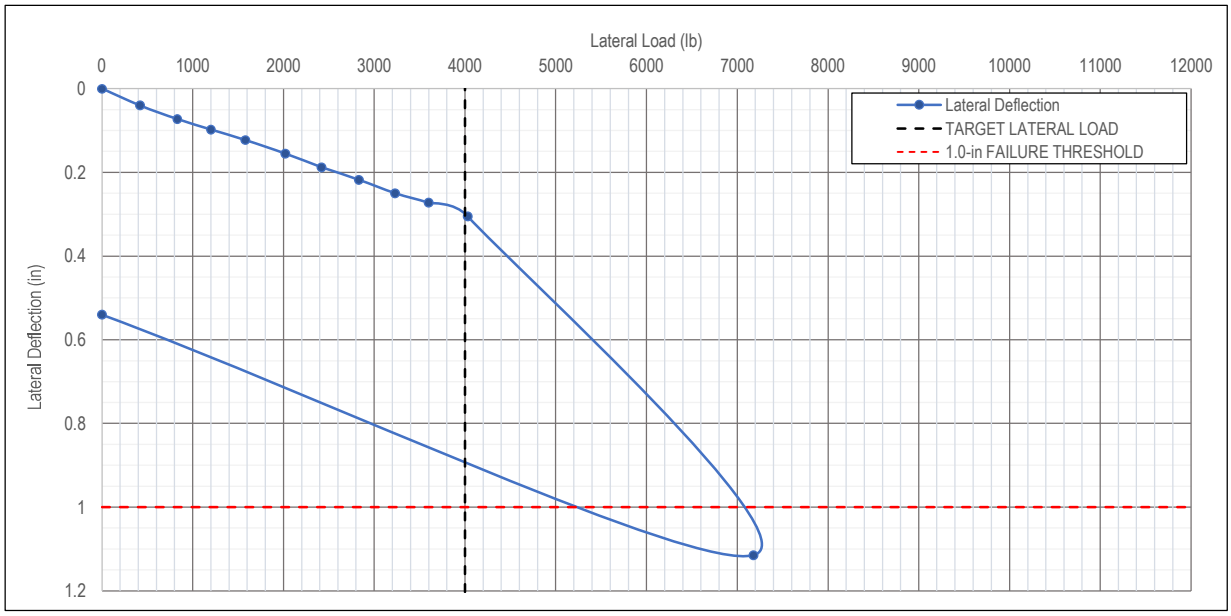
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	420	0.04	0.04	0.04
62%	20%	800	830	0.07	0.075	0.07
93%	30%	1,200	1200	0.095	0.1	0.10
124%	40%	1,600	1580	0.125	0.12	0.12
155%	50%	2,000	2020	0.155	0.155	0.16
186%	60%	2,400	2420	0.19	0.185	0.19
217%	70%	2,800	2830	0.22	0.215	0.22
248%	80%	3,200	3230	0.255	0.245	0.25
279%	90%	3,600	3600	0.275	0.27	0.27
310%	100%	4,000	4030	0.31	0.3	0.31
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7180	1.035	1.195	1.12
0%	0%	0	0	0.445	0.635	0.54

Load at 1.0-in lateral deflection: **6,733** **lb**

Residual lateral deformation: **0.54** **in**



Notes:

Failure reached at 7,180 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-26A

Axial Uplift Load Test Results

PTL - **26** Designation: **A**

Section: W6x9

Latitude: 33.65502

Longitude: -114.69506

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 11.6

Pile Damaged? No

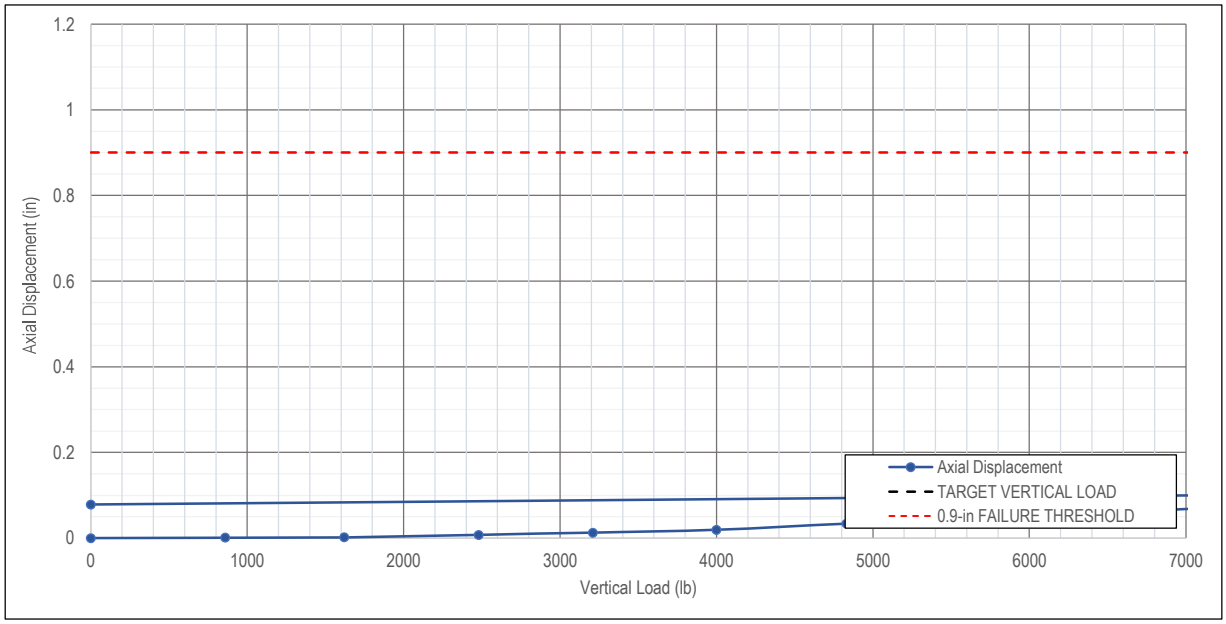
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	860	0.0015	0.001	0.00
436%	20%	1,600	1620	0.003	0.001	0.00
654%	30%	2,400	2480	0.009	0.0065	0.01
872%	40%	3,200	3210	0.014	0.012	0.01
1090%	50%	4,000	4000	0.0215	0.0175	0.02
1308%	60%	4,800	4830	0.035	0.0325	0.03
1526%	70%	5,600	5600	0.0435	0.0415	0.04
1744%	80%	6,400	6420	0.06	0.059	0.06
1962%	90%	7,200	7230	0.073	0.073	0.07
2180%	100%	8,000	8030	0.095	0.107	0.10
0%	0%	0	0	0.078	0.0785	0.08

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.08** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-26A

Lateral Load Test Results

PTL- **26** Designation: **A**

Section: W6x9

Latitude: 33.65502

Longitude: -114.69506

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.6

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

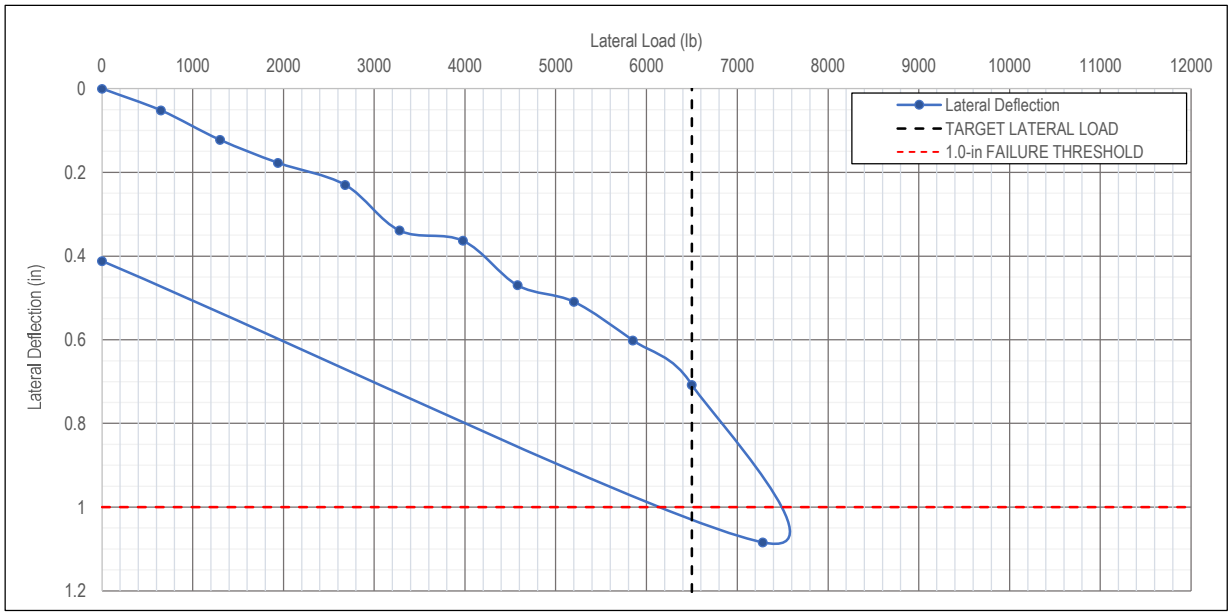
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.053	0.0505	0.05
62%	20%	1,300	1300	0.126	0.118	0.12
93%	30%	1,950	1940	0.1835	0.171	0.18
124%	40%	2,600	2680	0.238	0.2225	0.23
155%	50%	3,250	3280	0.35	0.328	0.34
186%	60%	3,900	3980	0.376	0.351	0.36
217%	70%	4,550	4580	0.4835	0.456	0.47
248%	80%	5,200	5200	0.524	0.4945	0.51
279%	90%	5,850	5850	0.6165	0.587	0.60
310%	100%	6,500	6500	0.7235	0.692	0.71
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7280	1.088	1.0805	1.08
0%	0%	0	0	0.399	0.425	0.41

Load at 1.0-in lateral deflection: **7,105** **lb**

Residual lateral deformation: **0.41** **in**



Notes:

Failure reached at 7,280 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-26B

Axial Uplift Load Test Results

PTL - **26** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.65502 - - Saturated Soil? No

Longitude: -114.69506 Target Embedment Depth (ft): 8.00 Drive Time (s): 17.56

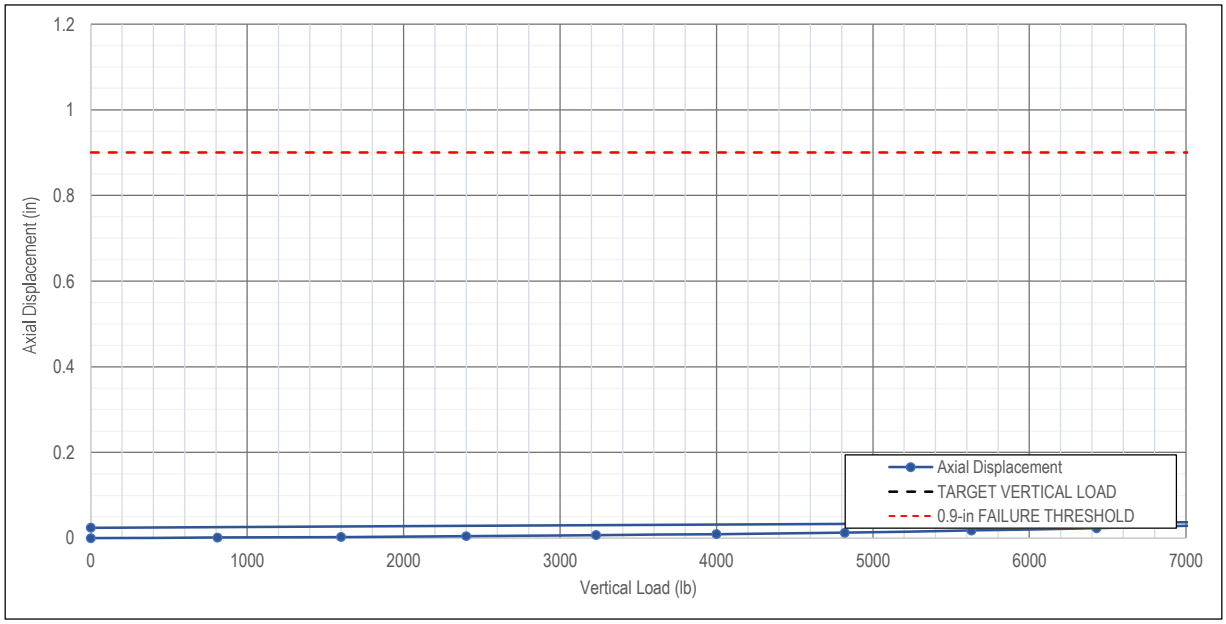
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/23/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	810	0.001	0.002	0.00
436%	20%	1,600	1600	0.0015	0.004	0.00
654%	30%	2,400	2400	0.003	0.0065	0.00
872%	40%	3,200	3230	0.0045	0.01	0.01
1090%	50%	4,000	4000	0.0065	0.0125	0.01
1308%	60%	4,800	4820	0.009	0.0165	0.01
1526%	70%	5,600	5630	0.013	0.0225	0.02
1744%	80%	6,400	6430	0.018	0.0285	0.02
1962%	90%	7,200	7200	0.0245	0.037	0.03
2180%	100%	8,000	8020	0.0315	0.0455	0.04
0%	0%	0	0	0.0235	0.025	0.02

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.02** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-26B

Lateral Load Test Results

PTL- **26** Designation: **B**

Section: W6x9

Latitude: 33.65502

Longitude: -114.69506

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 17.56

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

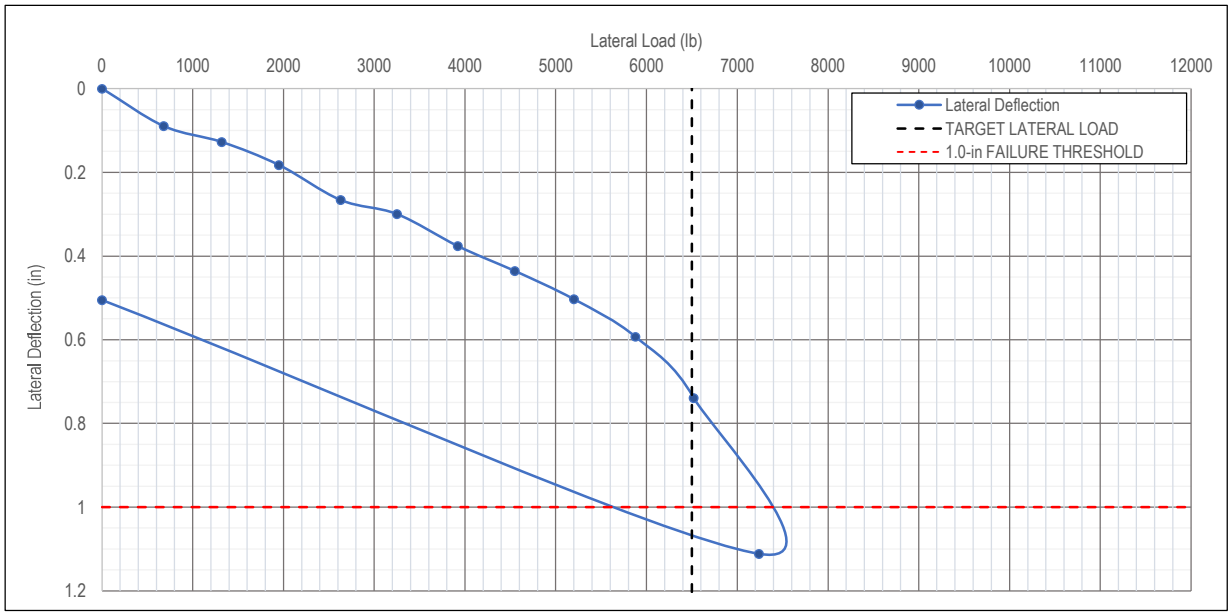
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	680	0.0895	0.089	0.09
62%	20%	1,300	1320	0.1285	0.1265	0.13
93%	30%	1,950	1950	0.1845	0.1805	0.18
124%	40%	2,600	2630	0.269	0.263	0.27
155%	50%	3,250	3250	0.303	0.296	0.30
186%	60%	3,900	3920	0.3805	0.3715	0.38
217%	70%	4,550	4550	0.441	0.4305	0.44
248%	80%	5,200	5200	0.508	0.4975	0.50
279%	90%	5,850	5880	0.5965	0.589	0.59
310%	100%	6,500	6520	0.738	0.74	0.74
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7240	0.9785	1.2445	1.11
0%	0%	0	0	0.4155	0.595	0.51

Load at 1.0-in lateral deflection: **7,024** **lb**

Residual lateral deformation: **0.51** **in**



Notes:

Failure reached at 7,240 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-26D

Axial Uplift Load Test Results

PTL - **26** Designation: **D**

Section: **W6x12**

Latitude: 33.65502

Longitude: -114.69506

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 27.2

Pile Damaged? No

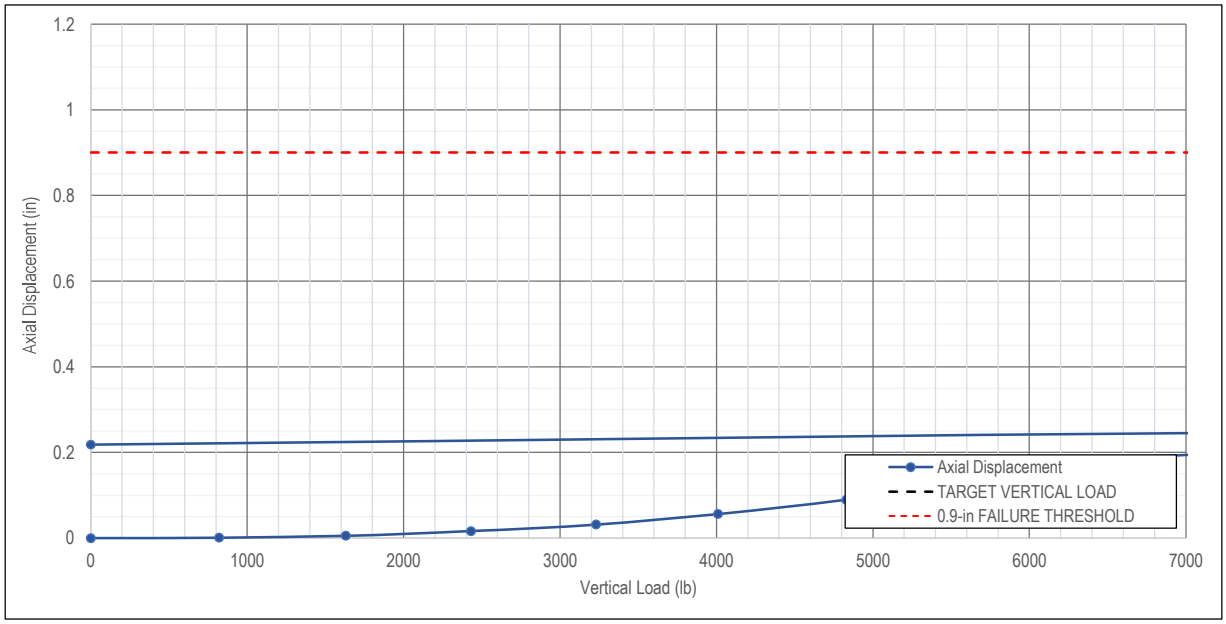
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	820	0.001	0.0015	0.00
436%	20%	1,600	1630	0.005	0.006	0.01
654%	30%	2,400	2430	0.0145	0.018	0.02
872%	40%	3,200	3230	0.029	0.0345	0.03
1090%	50%	4,000	4010	0.053	0.06	0.06
1308%	60%	4,800	4830	0.0855	0.0935	0.09
1526%	70%	5,600	5630	0.125	0.1355	0.13
1744%	80%	6,400	6440	0.177	0.1885	0.18
1962%	90%	7,200	7200	0.1935	0.206	0.20
2180%	100%	8,000	8030	0.239	0.253	0.25
0%	0%	0	0	0.217	0.2195	0.22

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.22** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-26D

Lateral Load Test Results

PTL- **26** Designation: **D**

Section: **W6x12**

Latitude: 33.65502

Longitude: -114.69506

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 27.2

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

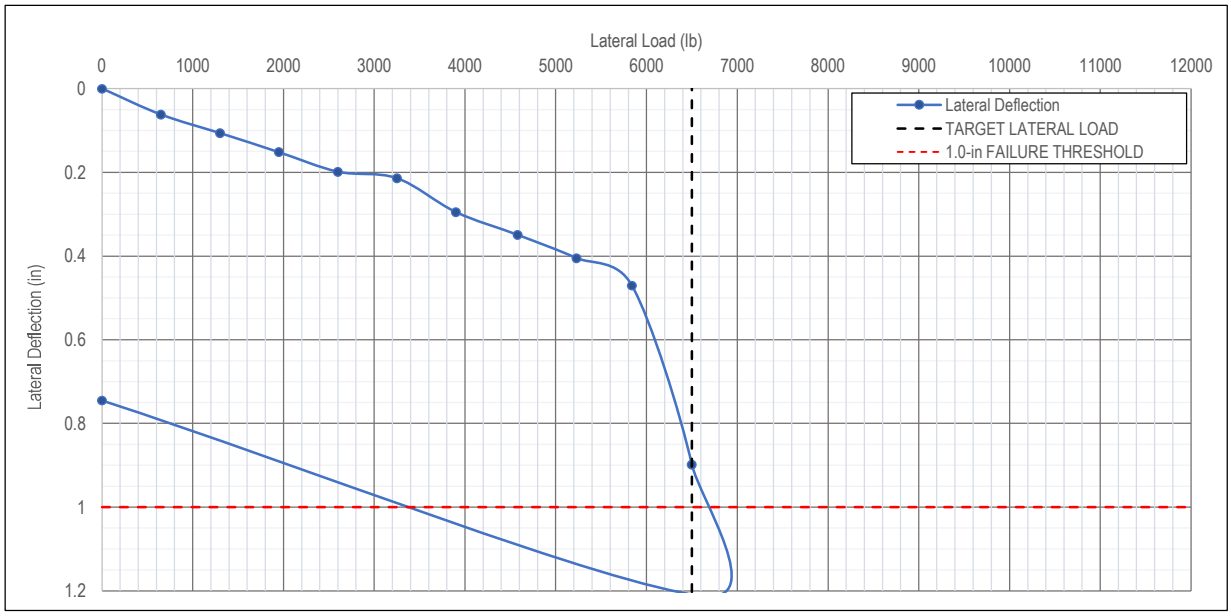
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.066	0.0575	0.06
62%	20%	1,300	1300	0.1045	0.108	0.11
93%	30%	1,950	1950	0.1485	0.1545	0.15
124%	40%	2,600	2600	0.1935	0.203	0.20
155%	50%	3,250	3250	0.2095	0.2185	0.21
186%	60%	3,900	3900	0.2865	0.3035	0.30
217%	70%	4,550	4580	0.3125	0.386	0.35
248%	80%	5,200	5230	0.3915	0.418	0.40
279%	90%	5,850	5840	0.451	0.49	0.47
310%	100%	6,500	6500	0.971	0.8255	0.90
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6630	1.2005	1.225	1.21
0%	0%	0	0	0.7	0.79	0.75

Load at 1.0-in lateral deflection: **6,542** **lb**

Residual lateral deformation: **0.75** **in**



Notes:

Failure reached at 6,630 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-27A

Axial Uplift Load Test Results

PTL - **27** Designation: **A**

Section: W6x9

Latitude: 33.655448

Longitude: -114.69173

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 10.68

Pile Damaged? No

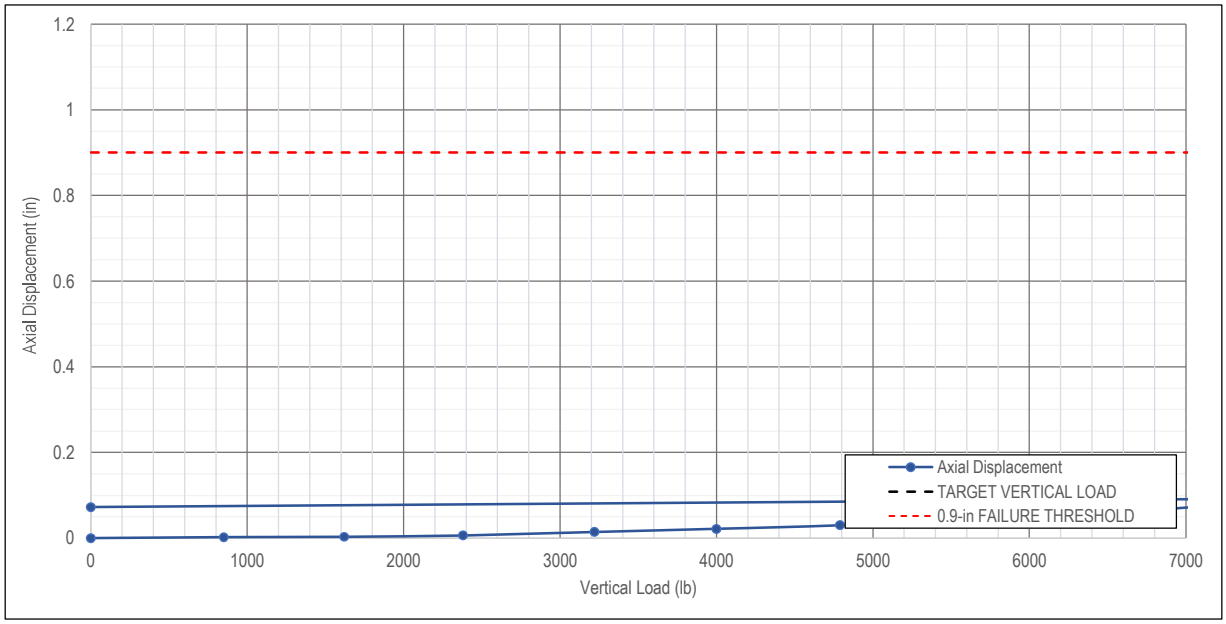
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	850	0.002	0.003	0.00
13594%	20%	1,600	1620	0.0035	0.003	0.00
20391%	30%	2,400	2380	0.0065	0.0065	0.01
27188%	40%	3,200	3220	0.0145	0.0145	0.01
33985%	50%	4,000	4000	0.0215	0.0215	0.02
40782%	60%	4,800	4790	0.03	0.0305	0.03
47579%	70%	5,600	5620	0.0435	0.0455	0.04
54376%	80%	6,400	6410	0.0565	0.058	0.06
61173%	90%	7,200	7280	0.077	0.079	0.08
67970%	100%	8,000	8040	0.0905	0.0945	0.09
0%	0%	0	0	0.0725	0.073	0.07

Load at 0.9-in axial displacement: N/A lb

Residual axial displacement: 0.07 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-27A

Lateral Load Test Results

PTL- **27** Designation: **A**

Section: W6x9

Latitude: 33.655448

Longitude: -114.69173

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 10.68

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

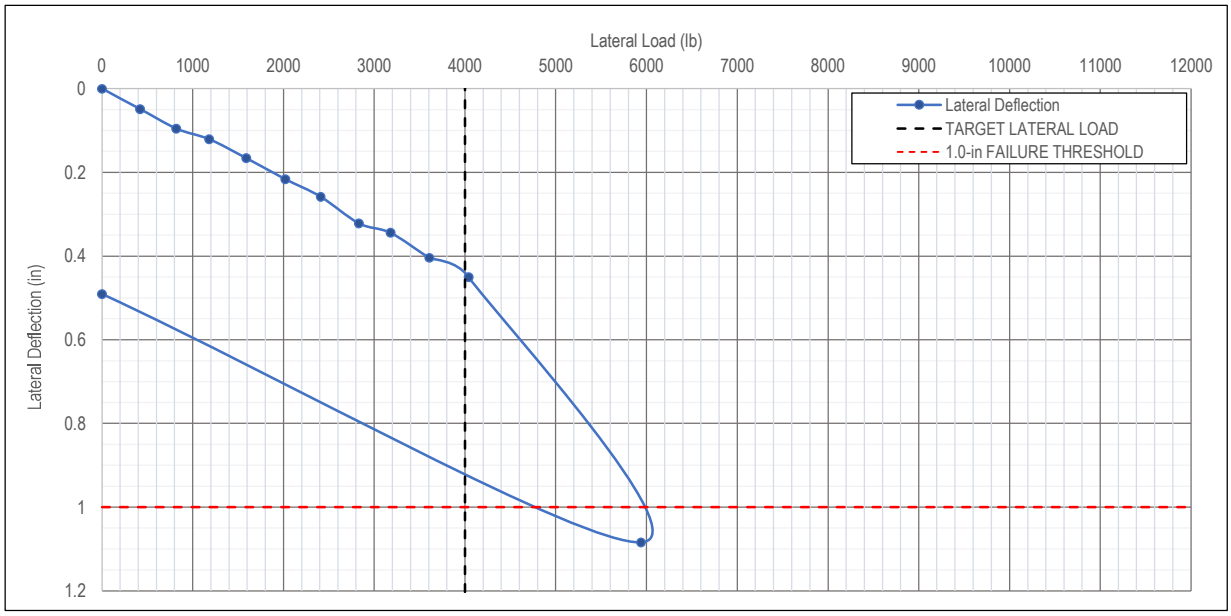
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	420	0.0505	0.047	0.05
62%	20%	800	820	0.098	0.0925	0.10
93%	30%	1,200	1180	0.1235	0.1175	0.12
124%	40%	1,600	1590	0.17	0.162	0.17
155%	50%	2,000	2020	0.2215	0.2105	0.22
186%	60%	2,400	2410	0.264	0.2525	0.26
217%	70%	2,800	2830	0.3285	0.315	0.32
248%	80%	3,200	3180	0.351	0.3375	0.34
279%	90%	3,600	3610	0.4115	0.3965	0.40
310%	100%	4,000	4040	0.4575	0.4425	0.45
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5940	1.036	1.1325	1.08
0%	0%	0	0	0.443	0.538	0.49

Load at 1.0-in lateral deflection: **5,688** **lb**

Residual lateral deformation: **0.49** **in**



Notes:

Failure reached at 5,940 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-27B

Axial Uplift Load Test Results

PTL - **27** Designation: **B**

Section: W6x9

Latitude: 33.655448

Longitude: -114.69173

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 14.54

Pile Damaged? No

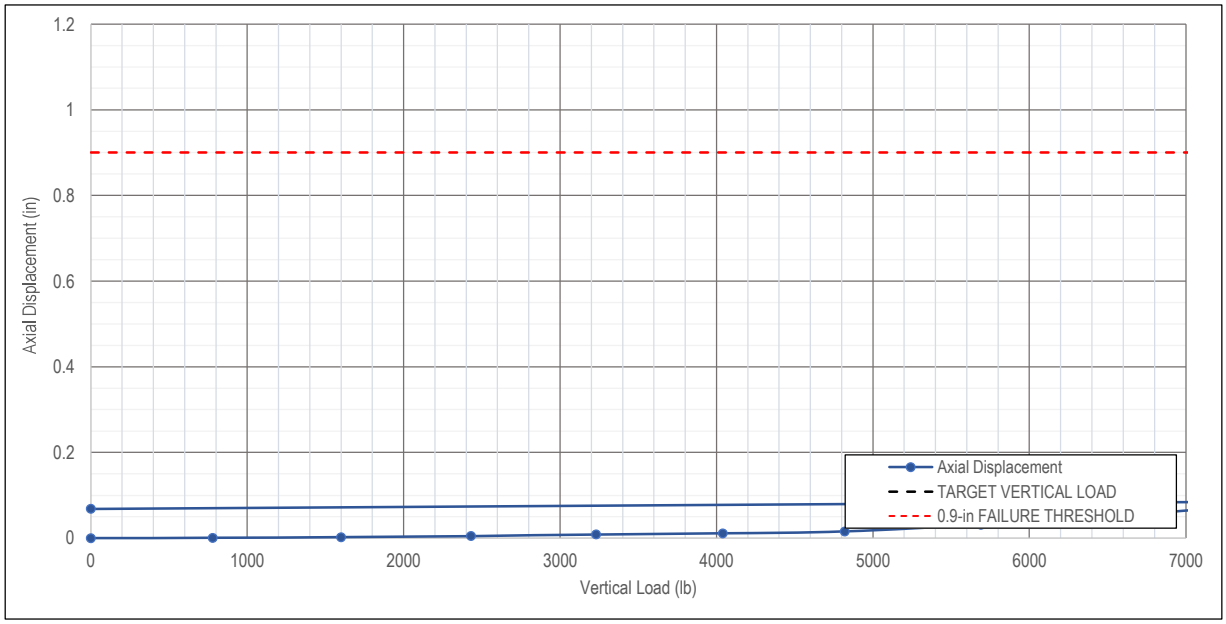
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	780	0.001	0.0005	0.00
13594%	20%	1,600	1600	0.003	0.002	0.00
20391%	30%	2,400	2430	0.0055	0.0045	0.01
27188%	40%	3,200	3230	0.0085	0.0085	0.01
33985%	50%	4,000	4040	0.0115	0.011	0.01
40782%	60%	4,800	4820	0.0155	0.0155	0.02
47579%	70%	5,600	5690	0.0305	0.0325	0.03
54376%	80%	6,400	6400	0.0435	0.048	0.05
61173%	90%	7,200	7200	0.066	0.074	0.07
67970%	100%	8,000	8020	0.0805	0.09	0.09
0%	0%	0	0	0.0675	0.0695	0.07

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.07** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-27B

Lateral Load Test Results

PTL- **27** Designation: **B**

Section: W6x9

Latitude: 33.655448

Longitude: -114.69173

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 14.54

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

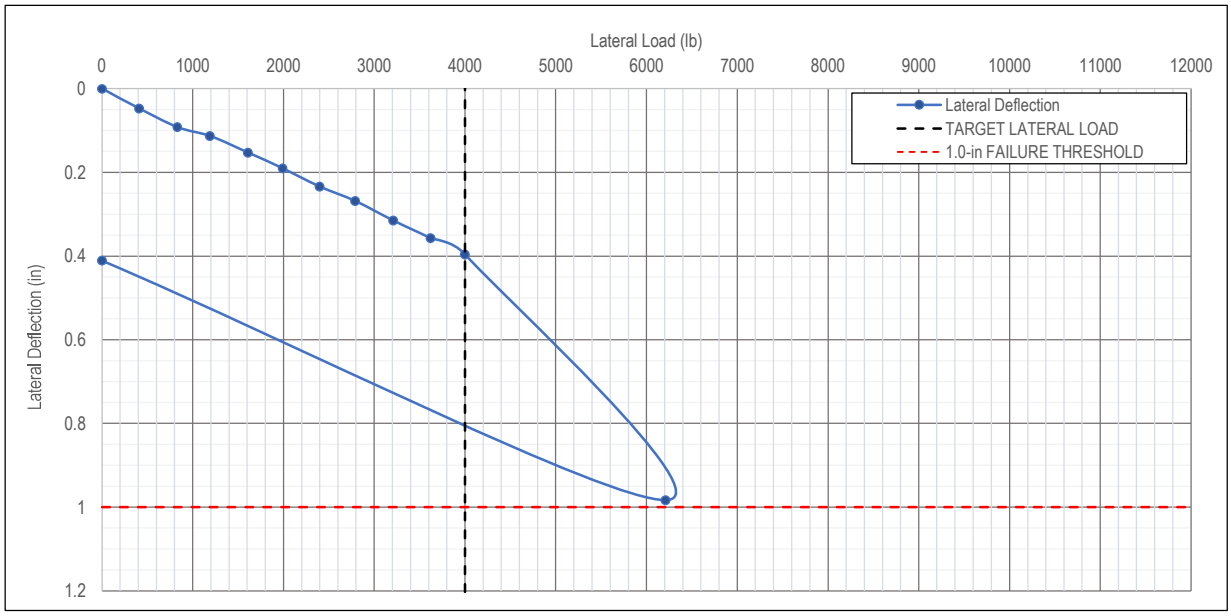
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	410	0.049	0.0455	0.05
62%	20%	800	830	0.0955	0.088	0.09
93%	30%	1,200	1190	0.117	0.1085	0.11
124%	40%	1,600	1610	0.1585	0.147	0.15
155%	50%	2,000	1990	0.1975	0.183	0.19
186%	60%	2,400	2400	0.243	0.225	0.23
217%	70%	2,800	2790	0.2785	0.258	0.27
248%	80%	3,200	3210	0.327	0.303	0.32
279%	90%	3,600	3620	0.371	0.3425	0.36
310%	100%	4,000	4000	0.4115	0.3805	0.40
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6210	1.042	0.9245	0.98
0%	0%	0	0	0.419	0.403	0.41

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.41** **in**



Notes:

Failure reached at 6,210 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-28A

Axial Uplift Load Test Results

PTL - **28** Designation: **A**

Section: W6x9

Latitude: 33.655448

Longitude: -114.68878

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 10.17

Pile Damaged? No

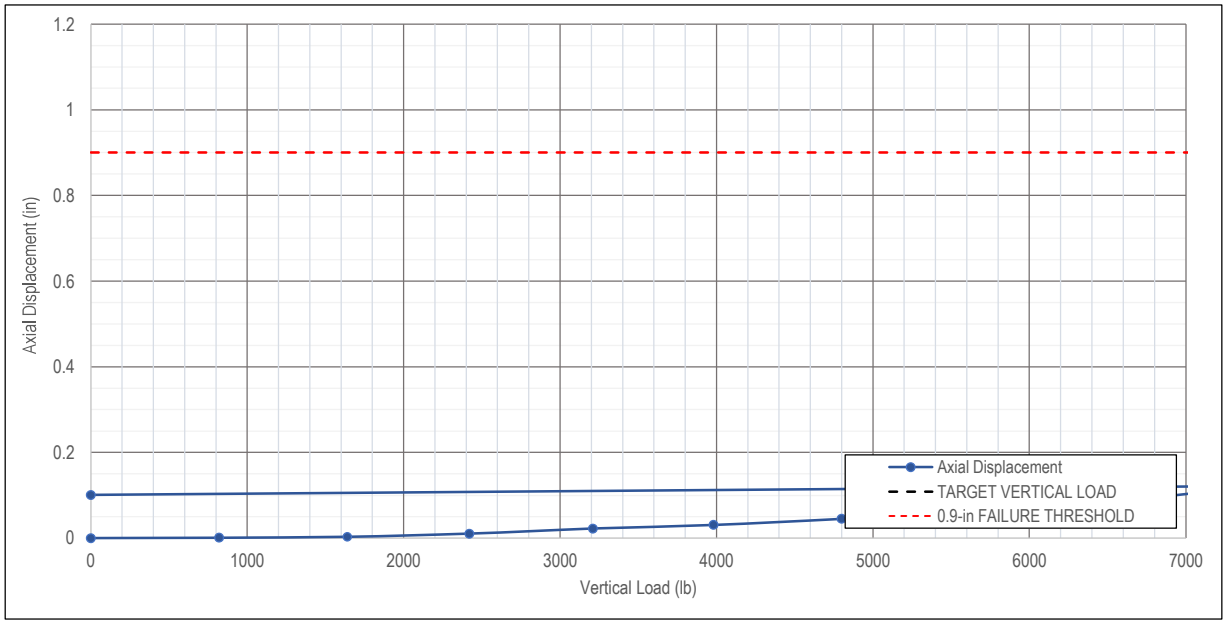
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	820	0.0015	0.001	0.00
436%	20%	1,600	1640	0.0035	0.003	0.00
654%	30%	2,400	2420	0.011	0.01	0.01
872%	40%	3,200	3210	0.0225	0.022	0.02
1090%	50%	4,000	3980	0.031	0.0305	0.03
1308%	60%	4,800	4800	0.0455	0.045	0.05
1526%	70%	5,600	5630	0.062	0.061	0.06
1744%	80%	6,400	6410	0.079	0.0785	0.08
1962%	90%	7,200	7240	0.112	0.111	0.11
2180%	100%	8,000	8060	0.123	0.123	0.12
0%	0%	0	0	0.101	0.1005	0.10

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.10** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-28A

Lateral Load Test Results

PTL- **28** Designation: **A**

Section: W6x9

Latitude: 33.655448

Longitude: -114.68878

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 10.17

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

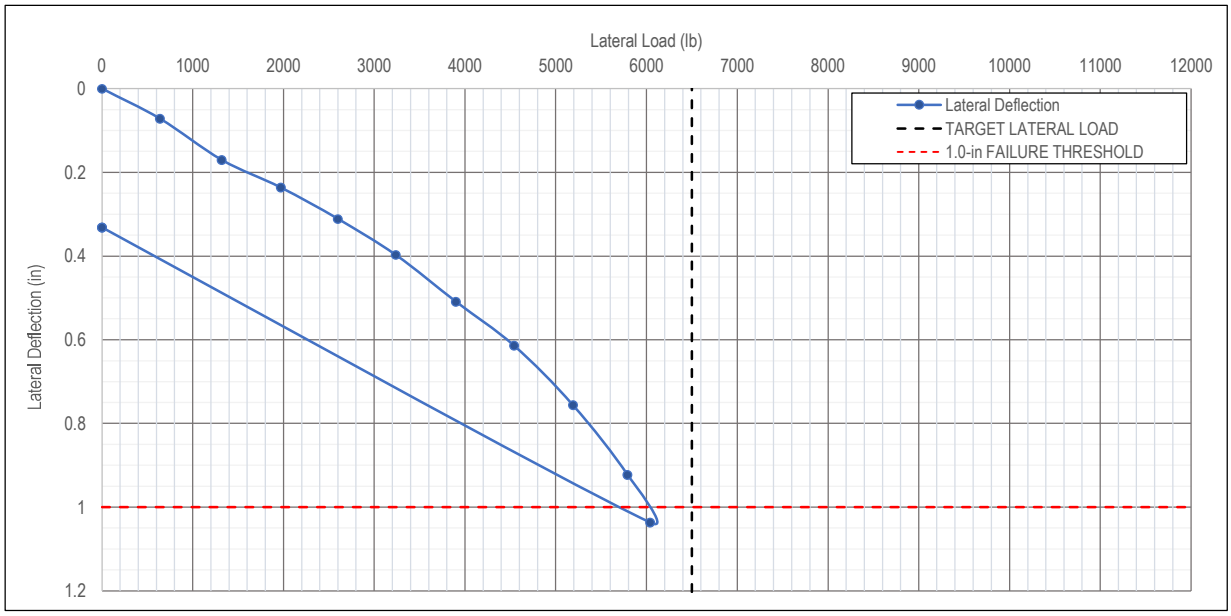
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	640	0.0685	0.075	0.07
62%	20%	1,300	1320	0.1635	0.177	0.17
93%	30%	1,950	1970	0.227	0.245	0.24
124%	40%	2,600	2600	0.3	0.3225	0.31
155%	50%	3,250	3240	0.3845	0.41	0.40
186%	60%	3,900	3900	0.4935	0.525	0.51
217%	70%	4,550	4540	0.5955	0.632	0.61
248%	80%	5,200	5190	0.7345	0.777	0.76
279%	90%	5,850	5790	0.899	0.947	0.92
310%	100%	6,500	6040	1.023	1.051	1.04
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.335	0.3285	0.33

Load at 1.0-in lateral deflection: **5,959** **lb**

Residual lateral deformation: **0.33** **in**



Notes:

Failure reached at 6,040 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-28B

Axial Uplift Load Test Results

PTL - **28** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.655448 - - Saturated Soil? No

Longitude: -114.68878 Target Embedment Depth (ft): 8.00 Drive Time (s): 16.01

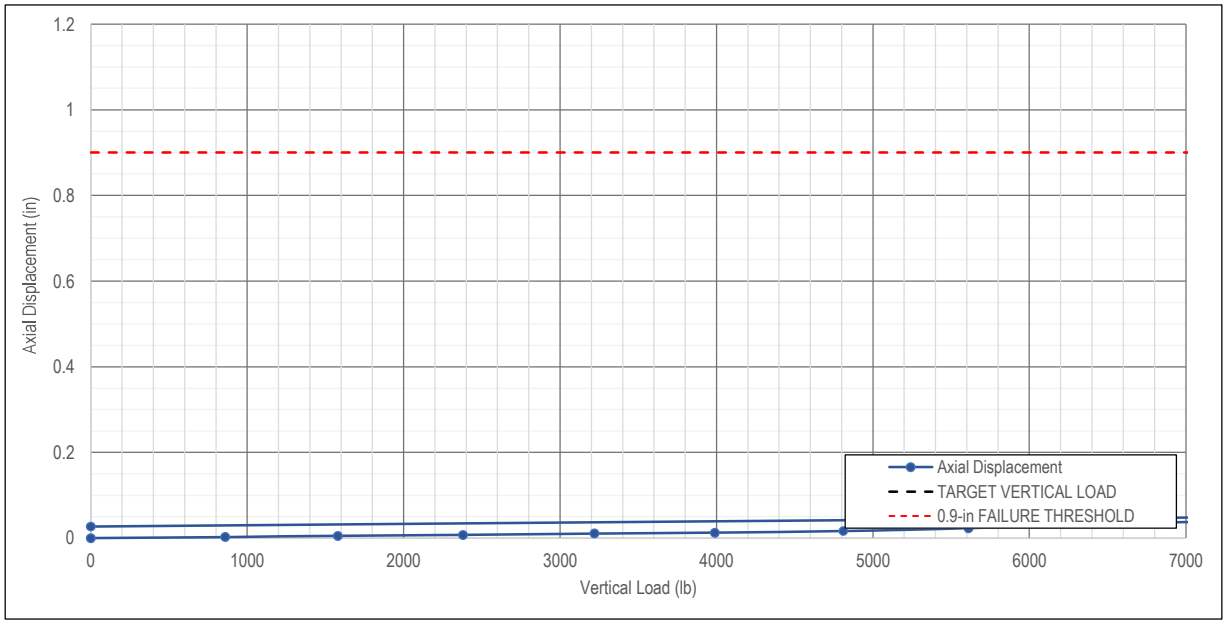
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/22/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	860	0.0035	0.002	0.00
436%	20%	1,600	1580	0.006	0.0045	0.01
654%	30%	2,400	2380	0.0085	0.0065	0.01
872%	40%	3,200	3220	0.0115	0.01	0.01
1090%	50%	4,000	3990	0.014	0.012	0.01
1308%	60%	4,800	4810	0.0175	0.0155	0.02
1526%	70%	5,600	5610	0.0245	0.0215	0.02
1744%	80%	6,400	6420	0.034	0.032	0.03
1962%	90%	7,200	7240	0.04	0.039	0.04
2180%	100%	8,000	8040	0.051	0.05	0.05
0%	0%	0	0	0.0275	0.027	0.03

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-28B

Lateral Load Test Results

PTL- **28** Designation: **B**

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Section: W6x9

Installation Method: Direct Drive (DD)

Saturated Soil? No

Latitude: 33.655448

- -

Drive Time (s): 16.01

Longitude: -114.68878

Target Embedment Depth (ft): 8.00

Pile Damaged? No

Tested By: ANS

Actual Embedment Depth (ft): 8

Gauge 1 Height (in): 6

Test Date: 10/22/2025

Lateral Load Application Height (ft): 5

Gauge 2 Height (in): 6

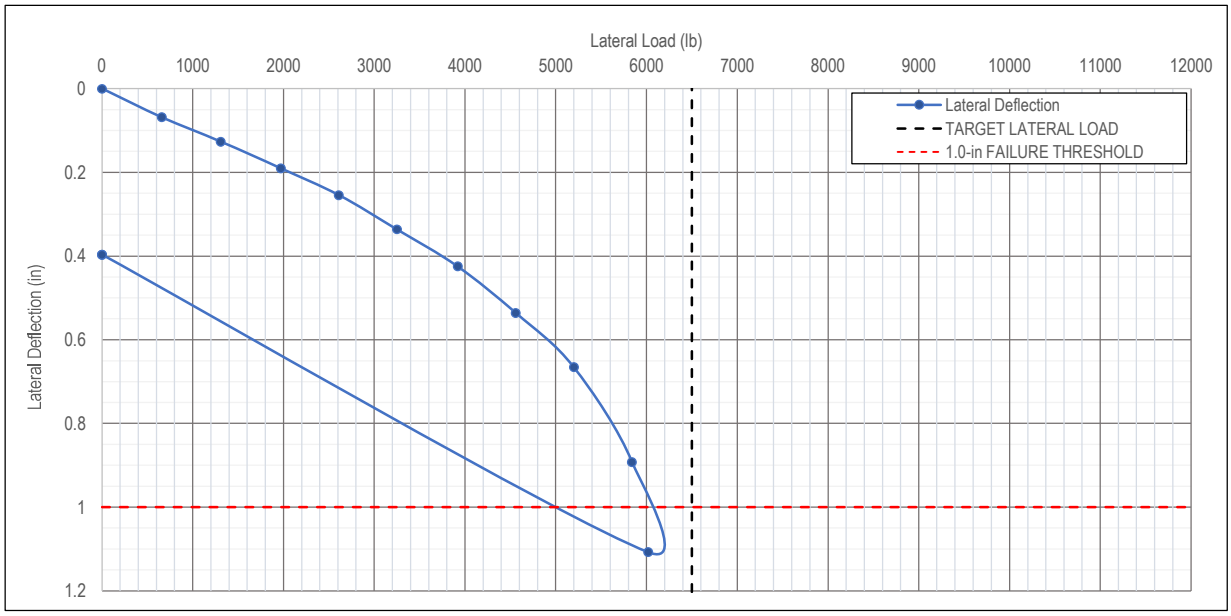
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	660	0.0655	0.071	0.07
62%	20%	1,300	1310	0.121	0.132	0.13
93%	30%	1,950	1970	0.182	0.1985	0.19
124%	40%	2,600	2610	0.2435	0.2645	0.25
155%	50%	3,250	3250	0.323	0.349	0.34
186%	60%	3,900	3920	0.411	0.4385	0.42
217%	70%	4,550	4560	0.5215	0.5505	0.54
248%	80%	5,200	5200	0.6505	0.6805	0.67
279%	90%	5,850	5840	0.893	0.8915	0.89
310%	100%	6,500	6020	1.153	1.062	1.11
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.407	0.386	0.40

Load at 1.0-in lateral deflection: **5,930** **lb**

Residual lateral deformation: **0.40** **in**



Notes:

Failure reached at 6,020 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-29A

Axial Uplift Load Test Results

PTL - **29** Designation: **A**

Section: W6x9

Latitude: 33.655448

Longitude: -114.58584

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 8.42

Pile Damaged? No

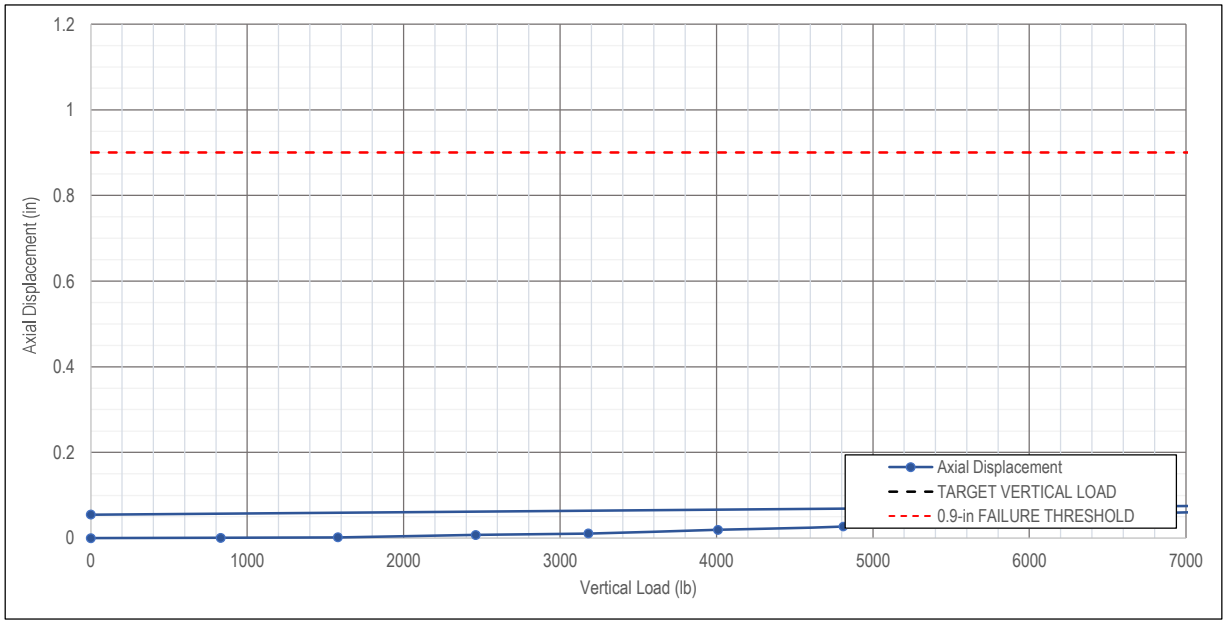
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	830	0.001	0.001	0.00
13594%	20%	1,600	1580	0.0015	0.0025	0.00
20391%	30%	2,400	2460	0.0055	0.01	0.01
27188%	40%	3,200	3180	0.0085	0.0135	0.01
33985%	50%	4,000	4010	0.0155	0.023	0.02
40782%	60%	4,800	4810	0.023	0.0315	0.03
47579%	70%	5,600	5630	0.0365	0.0475	0.04
54376%	80%	6,400	6440	0.049	0.0605	0.05
61173%	90%	7,200	7200	0.0565	0.0685	0.06
67970%	100%	8,000	7990	0.0695	0.084	0.08
0%	0%	0	0	0.0535	0.056	0.05

Load at 0.9-in axial displacement: N/A lb

Residual axial displacement: 0.05 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-29A

Lateral Load Test Results

PTL- **29** Designation: **A**

Section: W6x9

Latitude: 33.655448

Longitude: -114.58584

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 8.42

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

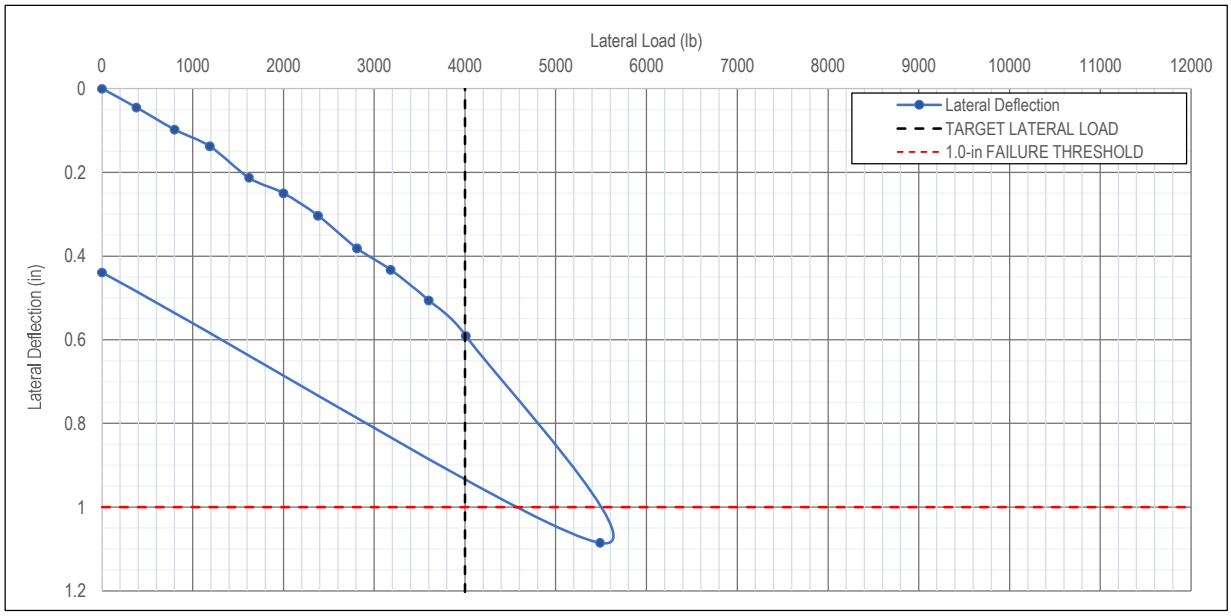
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	380	0.046	0.0435	0.04
62%	20%	800	800	0.101	0.0945	0.10
93%	30%	1,200	1190	0.142	0.1335	0.14
124%	40%	1,600	1620	0.219	0.2065	0.21
155%	50%	2,000	2000	0.2575	0.242	0.25
186%	60%	2,400	2380	0.313	0.2935	0.30
217%	70%	2,800	2810	0.3945	0.3685	0.38
248%	80%	3,200	3180	0.4475	0.418	0.43
279%	90%	3,600	3600	0.5235	0.488	0.51
310%	100%	4,000	4010	0.612	0.5695	0.59
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5490	1.162	1.009	1.09
0%	0%	0	0	0.4805	0.398	0.44

Load at 1.0-in lateral deflection: **5,234** **lb**

Residual lateral deformation: **0.44** **in**



Notes:

Failure reached at 5,490 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-29B

Axial Uplift Load Test Results

PTL - **29** Designation: **B**

Section: W6x9

Latitude: 33.655448

Longitude: -114.58584

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 11.37

Pile Damaged? No

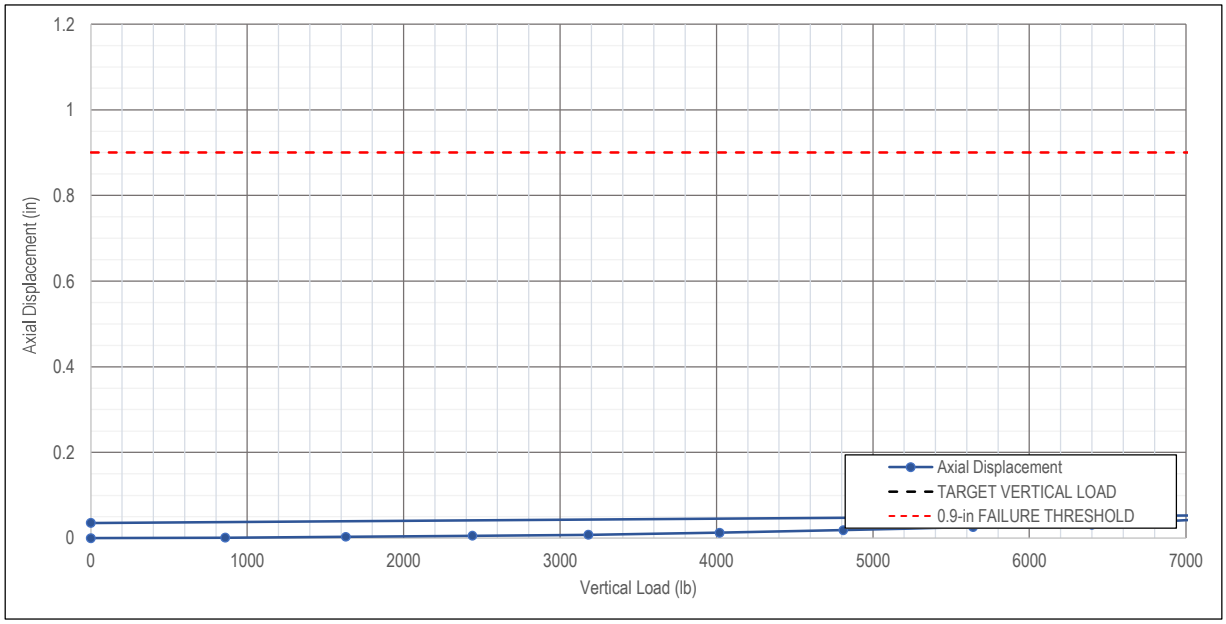
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	860	0.0015	0.001	0.00
13594%	20%	1,600	1630	0.0035	0.0025	0.00
20391%	30%	2,400	2440	0.0065	0.0045	0.01
27188%	40%	3,200	3180	0.0085	0.0075	0.01
33985%	50%	4,000	4020	0.014	0.012	0.01
40782%	60%	4,800	4810	0.0195	0.0175	0.02
47579%	70%	5,600	5640	0.027	0.025	0.03
54376%	80%	6,400	6400	0.0315	0.0305	0.03
61173%	90%	7,200	7230	0.0455	0.0465	0.05
67970%	100%	8,000	8020	0.054	0.0555	0.05
0%	0%	0	0	0.0355	0.0355	0.04

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.04** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-29B

Lateral Load Test Results

PTL- **29** Designation: **B**

Section: W6x9

Latitude: 33.655448

Longitude: -114.58584

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.37

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

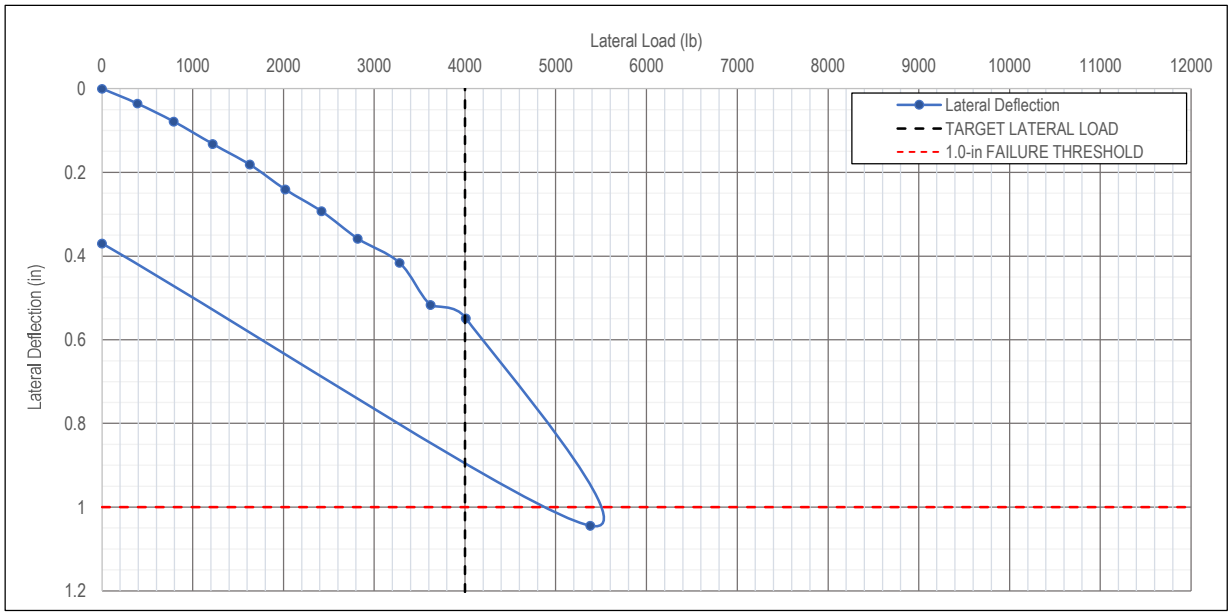
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	390	0.0365	0.034	0.04
62%	20%	800	790	0.0815	0.075	0.08
93%	30%	1,200	1220	0.137	0.127	0.13
124%	40%	1,600	1630	0.1925	0.1705	0.18
155%	50%	2,000	2020	0.248	0.2335	0.24
186%	60%	2,400	2420	0.301	0.285	0.29
217%	70%	2,800	2820	0.367	0.35	0.36
248%	80%	3,200	3280	0.426	0.4065	0.42
279%	90%	3,600	3620	0.527	0.5065	0.52
310%	100%	4,000	4010	0.59	0.508	0.55
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5380	1.05	1.038	1.04
0%	0%	0	0	0.363	0.377	0.37

Load at 1.0-in lateral deflection: **5,258** **lb**

Residual lateral deformation: **0.37** **in**



Notes:

Failure reached at 5,380 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-30A

Axial Uplift Load Test Results

PTL - **30** Designation: **A**

Section: W6x9

Latitude: 33.658548

Longitude: -114.68254

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 11.38

Pile Damaged? No

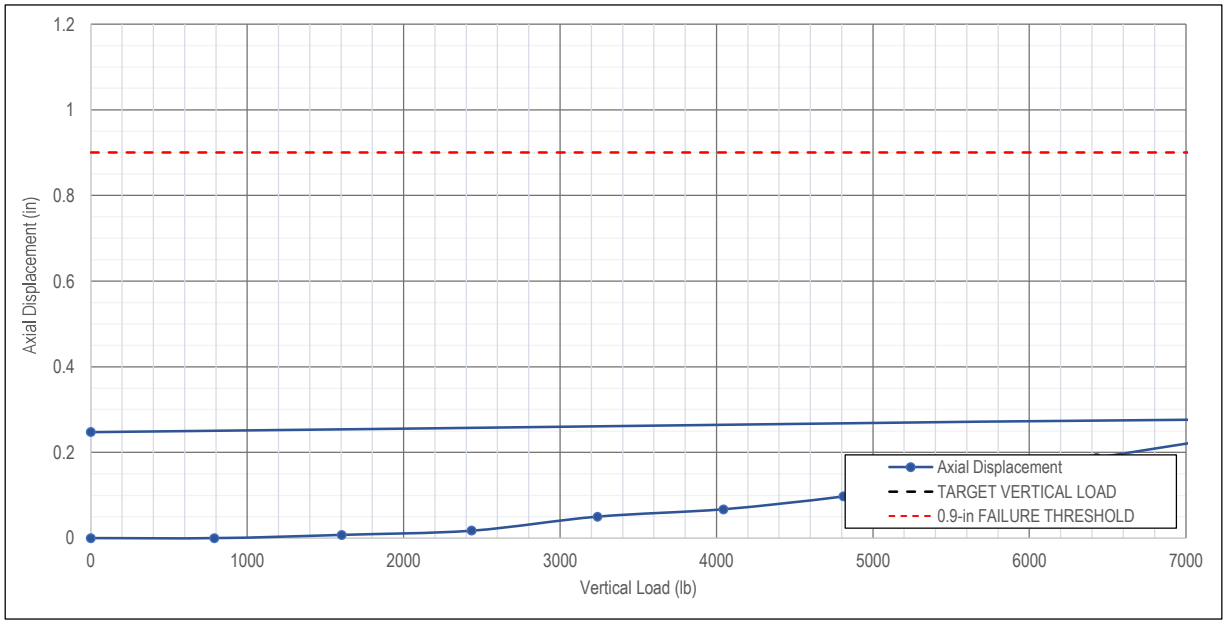
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	790	0	0	0.00
436%	20%	1,600	1605	0.01	0.005	0.01
654%	30%	2,400	2435	0.02	0.015	0.02
872%	40%	3,200	3240	0.055	0.045	0.05
1090%	50%	4,000	4045	0.07	0.065	0.07
1308%	60%	4,800	4810	0.1	0.095	0.10
1526%	70%	5,600	5640	0.145	0.14	0.14
1744%	80%	6,400	6430	0.19	0.185	0.19
1962%	90%	7,200	7210	0.235	0.23	0.23
2180%	100%	8,000	8025	0.28	0.275	0.28
0%	0%	0	0	0.25	0.245	0.25

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.25** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-30A

Lateral Load Test Results

PTL- **30** Designation: **A**

Section: W6x9

Latitude: 33.658548

Longitude: -114.68254

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.38

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

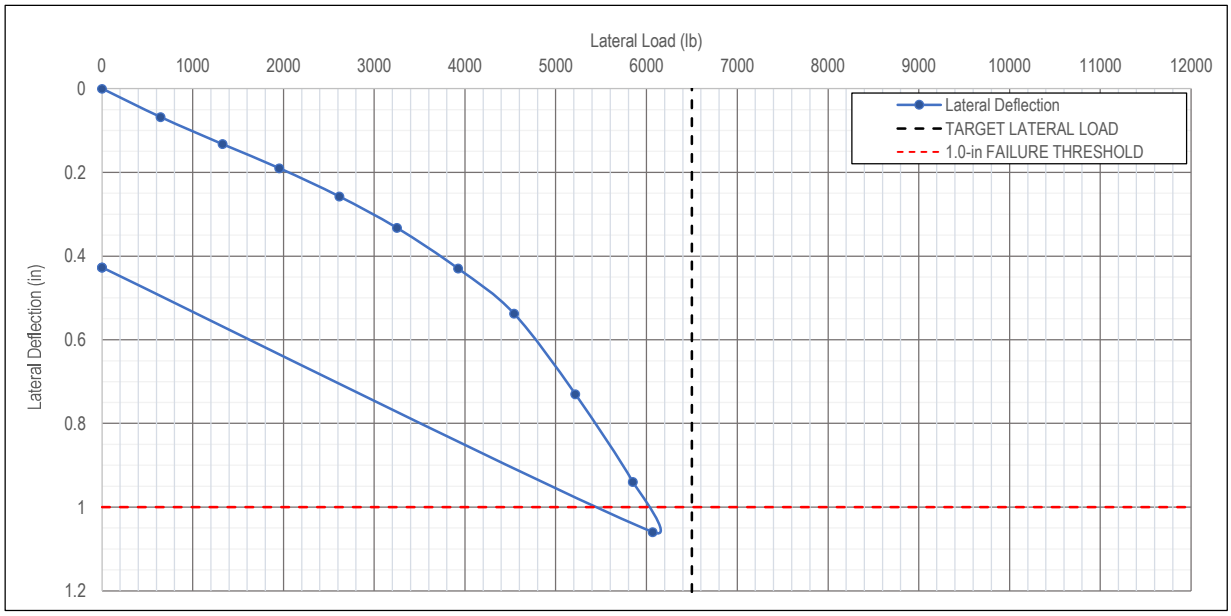
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	645	0.07	0.065	0.07
62%	20%	1,300	1330	0.135	0.13	0.13
93%	30%	1,950	1955	0.19	0.19	0.19
124%	40%	2,600	2615	0.255	0.26	0.26
155%	50%	3,250	3250	0.33	0.335	0.33
186%	60%	3,900	3925	0.425	0.435	0.43
217%	70%	4,550	4540	0.53	0.545	0.54
248%	80%	5,200	5215	0.71	0.75	0.73
279%	90%	5,850	5850	0.895	0.985	0.94
310%	100%	6,500	6070	0.985	1.135	1.06
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.38	0.475	0.43

Load at 1.0-in lateral deflection: **5,960** **lb**

Residual lateral deformation: **0.43** **in**



Notes:

Failure reached at 6,070 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-30B

Axial Uplift Load Test Results

PTL - **30** Designation: **B**

Section: W6x9

Latitude: 33.658548

Longitude: -114.68254

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 14.11

Pile Damaged? No

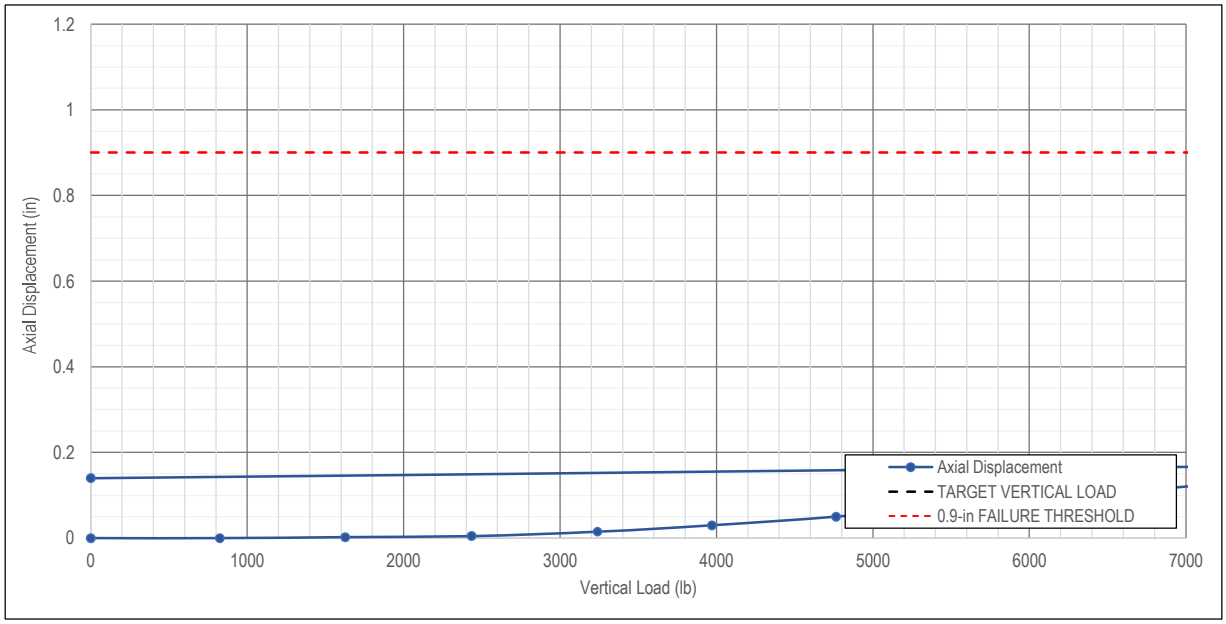
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	825	0	0	0.00
436%	20%	1,600	1627	0.005	0	0.00
654%	30%	2,400	2435	0.005	0.005	0.01
872%	40%	3,200	3240	0.015	0.015	0.02
1090%	50%	4,000	3970	0.03	0.03	0.03
1308%	60%	4,800	4765	0.05	0.05	0.05
1526%	70%	5,600	5630	0.075	0.08	0.08
1744%	80%	6,400	6430	0.1	0.105	0.10
1962%	90%	7,200	7180	0.125	0.13	0.13
2180%	100%	8,000	8040	0.165	0.17	0.17
0%	0%	0	0	0.14	0.14	0.14

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.14** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-30B

Lateral Load Test Results

PTL- **30** Designation: **B**

Section: W6x9

Latitude: 33.658548

Longitude: -114.68254

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 14.11

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

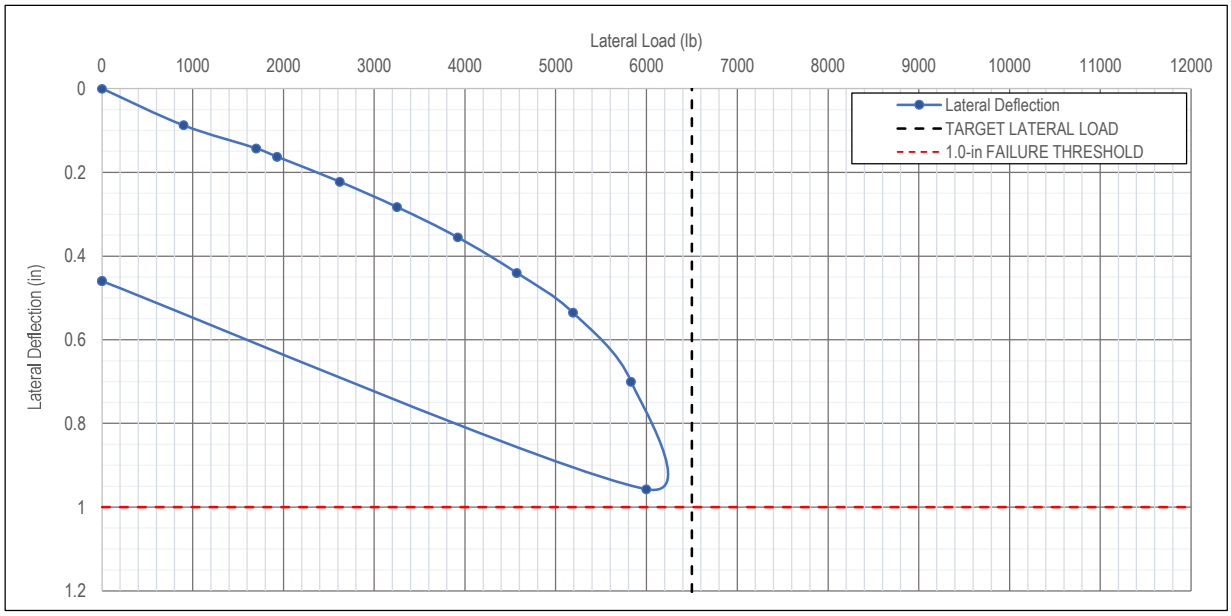
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	900	0.09	0.085	0.09
62%	20%	1,300	1700	0.145	0.14	0.14
93%	30%	1,950	1930	0.165	0.16	0.16
124%	40%	2,600	2620	0.225	0.22	0.22
155%	50%	3,250	3250	0.285	0.28	0.28
186%	60%	3,900	3920	0.355	0.355	0.36
217%	70%	4,550	4570	0.44	0.44	0.44
248%	80%	5,200	5190	0.525	0.545	0.54
279%	90%	5,850	5830	0.66	0.74	0.70
310%	100%	6,500	6000	0.845	1.07	0.96
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.36	0.56	0.46

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.46** **in**



Notes:

Pile bending at 6,000 lbs. No reliable test results available after this point.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-30D

Axial Uplift Load Test Results

PTL - **30** Designation: **D**

Section: **W6x12**

Latitude: 33.658548

Longitude: -114.68254

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 15.48

Pile Damaged? No

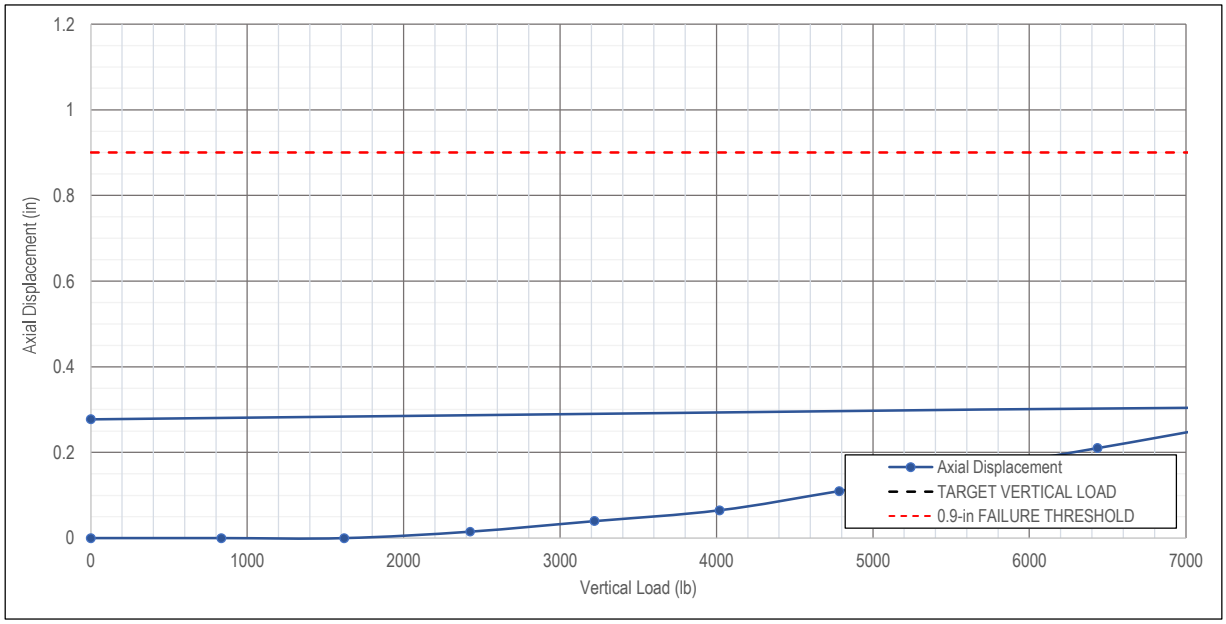
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	835	0	0	0.00
436%	20%	1,600	1620	0	0	0.00
654%	30%	2,400	2425	0.015	0.015	0.02
872%	40%	3,200	3220	0.04	0.04	0.04
1090%	50%	4,000	4020	0.065	0.065	0.07
1308%	60%	4,800	4785	0.11	0.11	0.11
1526%	70%	5,600	5630	0.16	0.16	0.16
1744%	80%	6,400	6435	0.21	0.21	0.21
1962%	90%	7,200	7220	0.26	0.26	0.26
2180%	100%	8,000	8035	0.305	0.305	0.31
0%	0%	0	0	0.275	0.28	0.28

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.28** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-30D

Lateral Load Test Results

PTL- **30** Designation: **D**

Section: **W6x12**

Latitude: 33.658548

Longitude: -114.68254

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 15.48

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

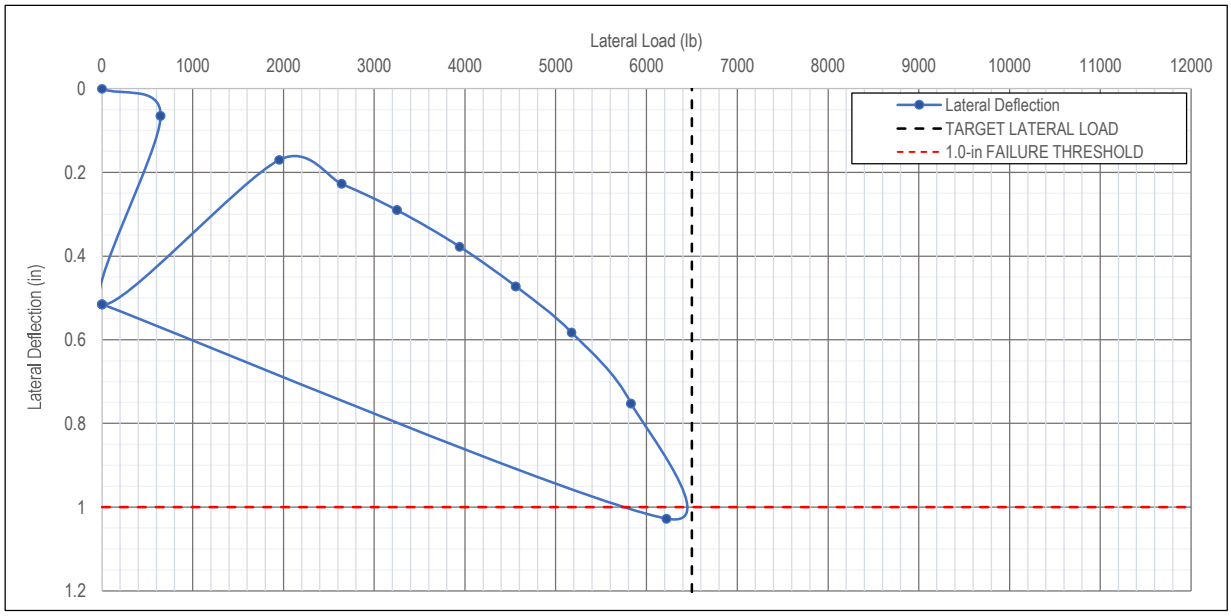
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	645	0.06	0.07	0.07
62%	20%	1,300				0.00
93%	30%	1,950	1955	0.16	0.18	0.17
124%	40%	2,600	2640	0.215	0.24	0.23
155%	50%	3,250	3250	0.28	0.3	0.29
186%	60%	3,900	3940	0.365	0.39	0.38
217%	70%	4,550	4560	0.46	0.485	0.47
248%	80%	5,200	5175	0.57	0.595	0.58
279%	90%	5,850	5830	0.75	0.755	0.75
310%	100%	6,500	6220	1.06	0.995	1.03
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.54	0.49	0.52

Load at 1.0-in lateral deflection: **6,181** **lb**

Residual lateral deformation: **0.52** **in**



Notes:

Failure reached at 6,220 lbs. 1,300-lbs load step skipped.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-31A

Axial Uplift Load Test Results

PTL - **31** Designation: **A**

Section: W6x9

Latitude: 33.655213

Longitude: -114.68208

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 13.3

Pile Damaged? No

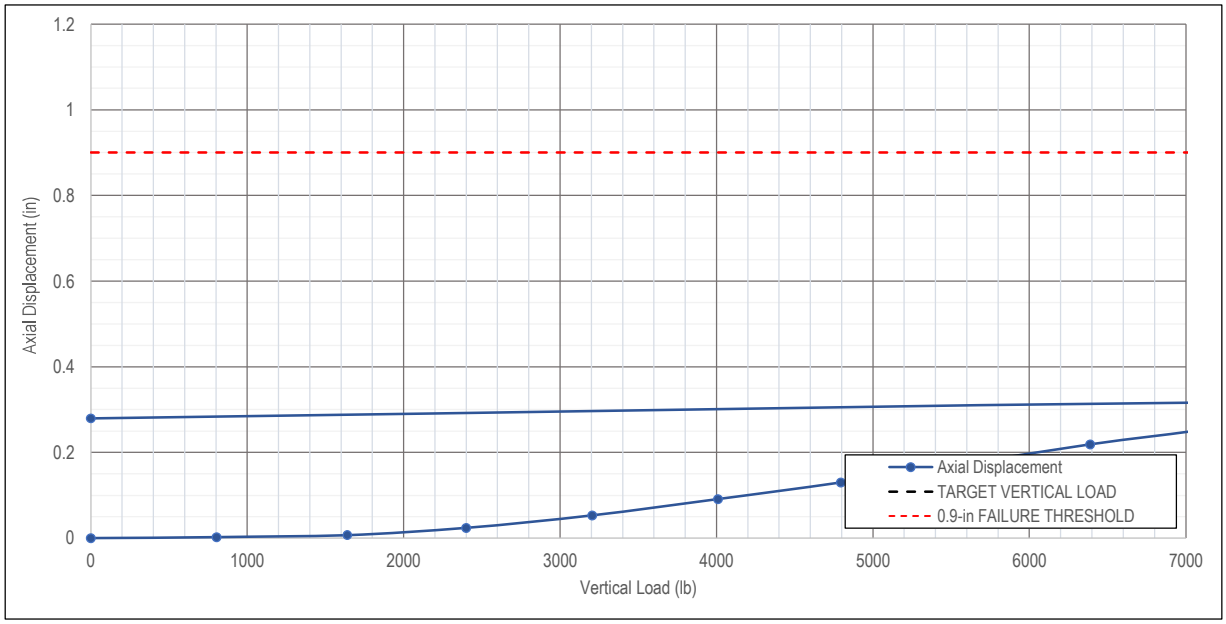
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	805	0.003	0.002	0.00
13594%	20%	1,600	1640	0.0065	0.008	0.01
20391%	30%	2,400	2400	0.023	0.025	0.02
27188%	40%	3,200	3205	0.053	0.053	0.05
33985%	50%	4,000	4010	0.091	0.0915	0.09
40782%	60%	4,800	4795	0.127	0.133	0.13
47579%	70%	5,600	5620	0.171	0.18	0.18
54376%	80%	6,400	6390	0.215	0.223	0.22
61173%	90%	7,200	7210	0.2545	0.2635	0.26
67970%	100%	8,000	8025	0.314	0.321	0.32
0%	0%	0	0	0.281	0.278	0.28

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.28** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-31A

Lateral Load Test Results

PTL- **31** Designation: **A**

Section: W6x9

Latitude: 33.655213

Longitude: -114.68208

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 13.3

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

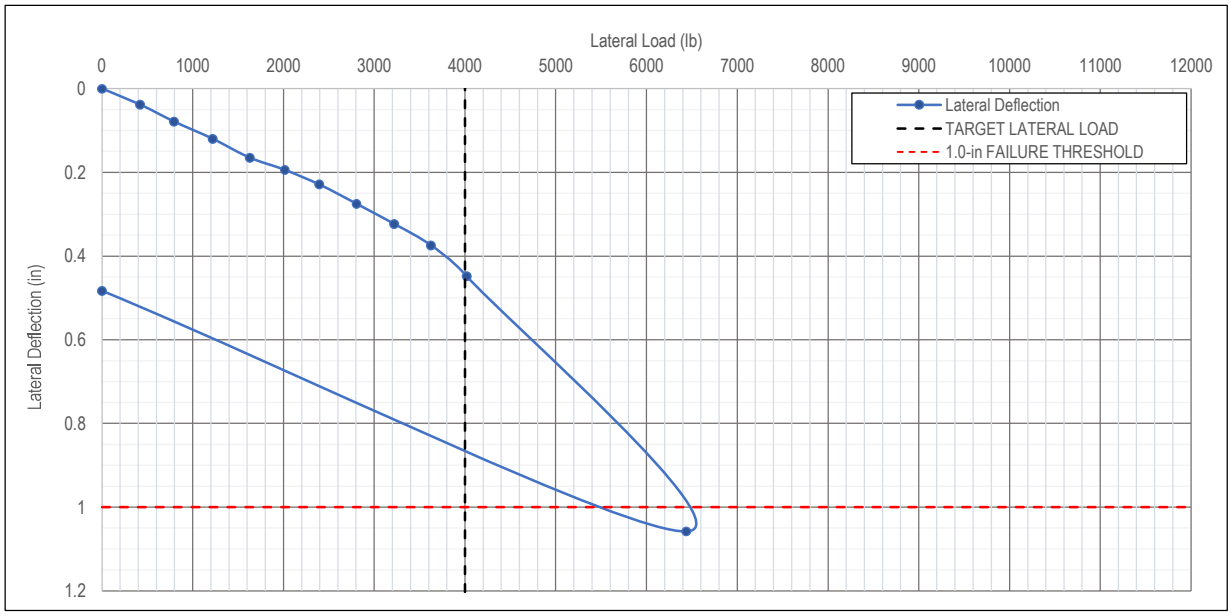
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	420	0.038	0.038	0.04
62%	20%	800	795	0.077	0.08	0.08
93%	30%	1,200	1220	0.117	0.122	0.12
124%	40%	1,600	1630	0.1625	0.1675	0.17
155%	50%	2,000	2015	0.1905	0.197	0.19
186%	60%	2,400	2395	0.2265	0.231	0.23
217%	70%	2,800	2805	0.272	0.278	0.28
248%	80%	3,200	3220	0.3205	0.3255	0.32
279%	90%	3,600	3625	0.371	0.378	0.37
310%	100%	4,000	4020	0.445	0.4515	0.45
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6440	1.0815	1.034	1.06
0%	0%	0	0	0.5015	0.465	0.48

Load at 1.0-in lateral deflection: **6,211** **lb**

Residual lateral deformation: **0.48** **in**



Notes:

Failure reached at 6,440 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-31B

Axial Uplift Load Test Results

PTL - **31** Designation: **B**

Section: W6x9

Latitude: 33.655213

Longitude: -114.68208

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 23.15

Pile Damaged? No

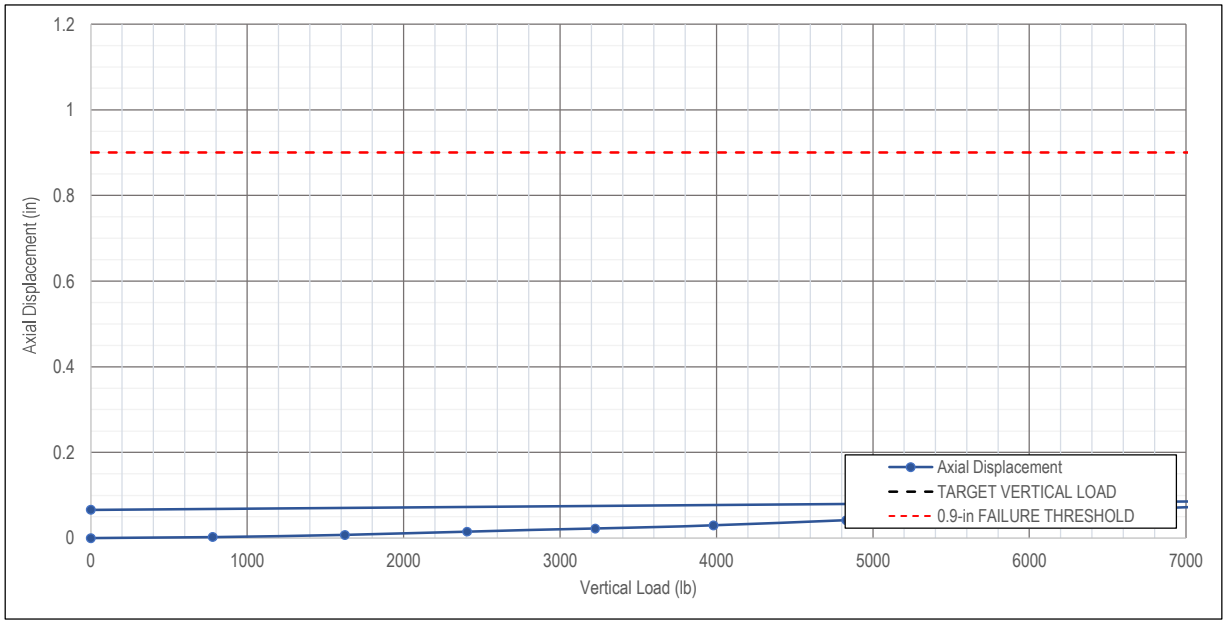
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	780	0.0025	0.003	0.00
13594%	20%	1,600	1625	0.007	0.008	0.01
20391%	30%	2,400	2405	0.0145	0.016	0.02
27188%	40%	3,200	3225	0.0205	0.0245	0.02
33985%	50%	4,000	3980	0.028	0.0315	0.03
40782%	60%	4,800	4830	0.041	0.0425	0.04
47579%	70%	5,600	5615	0.0495	0.0535	0.05
54376%	80%	6,400	6430	0.063	0.067	0.07
61173%	90%	7,200	7220	0.0715	0.078	0.07
67970%	100%	8,000	7990	0.0835	0.092	0.09
0%	0%	0	0	0.067	0.0655	0.07

Load at 0.9-in axial displacement: N/A lb

Residual axial displacement: 0.07 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-31B

Lateral Load Test Results

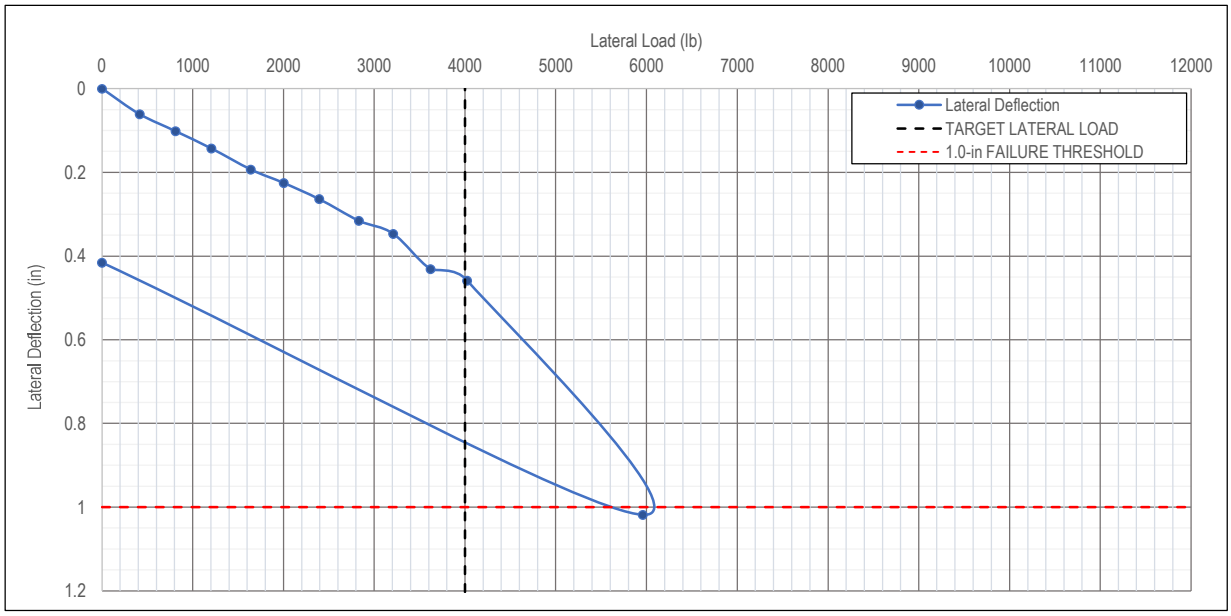
PTL-	**31**	Designation:	**B**	Target Test Load (lb):	**4,000** (≈ 310% Design Load)
Section:	W6x9			Installation Method:	Direct Drive (DD)
Latitude:	33.655213				- -
Longitude:	-114.68208			Target Embedment Depth (ft):	7.00
Tested By:	ANS			Actual Embedment Depth (ft):	7
Test Date:	10/26/2025			Lateral Load Application Height (ft):	5

Saturated Soil?	No	Strong Axis Plumb. (deg):	
Drive Time (s):	23.15		
Pile Damaged?	No	Weak Axis Plumb. (deg):	
Gauge 1 Height (in):	6		
Gauge 2 Height (in):	6		

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	415	0.06	0.0615	0.06
62%	20%	800	810	0.099	0.104	0.10
93%	30%	1,200	1205	0.141	0.145	0.14
124%	40%	1,600	1640	0.19	0.196	0.19
155%	50%	2,000	2005	0.2235	0.2275	0.23
186%	60%	2,400	2395	0.2605	0.2675	0.26
217%	70%	2,800	2830	0.3125	0.318	0.32
248%	80%	3,200	3210	0.343	0.35	0.35
279%	90%	3,600	3620	0.4285	0.434	0.43
310%	100%	4,000	4025	0.4545	0.463	0.46
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5960	0.9785	1.0575	1.02
0%	0%	0	0	0.3935	0.4375	0.42

Load at 1.0-in lateral deflection: **5,898** **lb**

Residual lateral deformation: **0.42** **in**



Notes:

Failure reached at 5,960 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-31C

Axial Compression Load Test Results (supplemental pile)

PTL - **31** Designation: **C**

Section: W6x9

Latitude: 33.655213

Longitude: -114.68208

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **15,000** (≈ 1180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 6.5

Pile Damaged? No

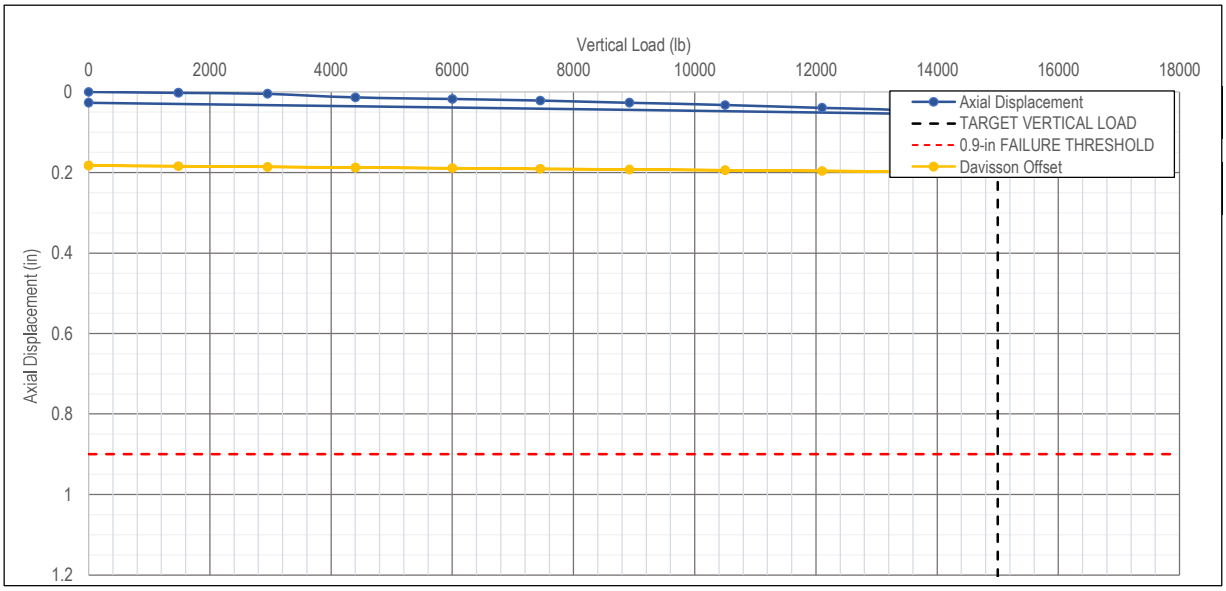
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL COMPRESSION** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
118%	10%	1,500	1480	0.002	0.002	0.00
236%	20%	3,000	2950	0.005	0.0045	0.00
354%	30%	4,500	4400	0.0145	0.013	0.01
472%	40%	6,000	6000	0.0195	0.0155	0.02
590%	50%	7,500	7450	0.024	0.019	0.02
708%	60%	9,000	8920	0.029	0.0245	0.03
826%	70%	10,500	10500	0.0345	0.0305	0.03
944%	80%	12,000	12100	0.0415	0.037	0.04
1062%	90%	13,500	13400	0.0475	0.0425	0.05
1180%	100%	15,000	14900	0.059	0.0545	0.06
0%	0%	0	0	0.025	0.0285	0.03

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-32A

Axial Uplift Load Test Results

PTL - **32** Designation: **A**

Section: W6x9

Latitude: 33.652344

Longitude: -114.7029

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 11.11

Pile Damaged? No

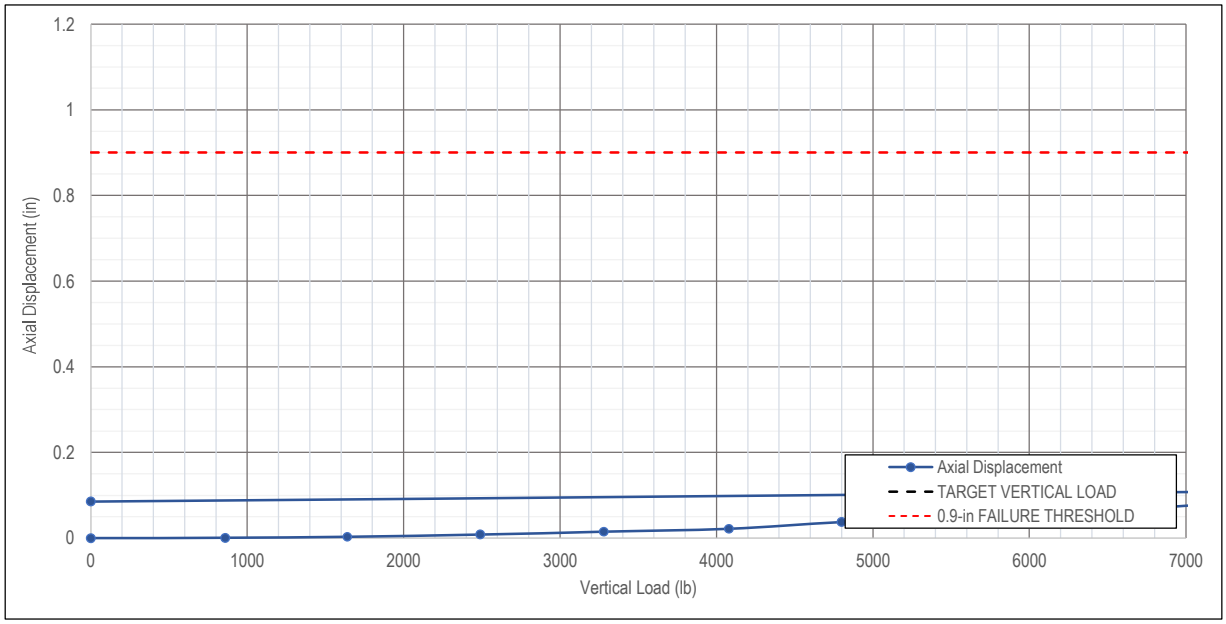
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	860	0.001	0.001	0.00
436%	20%	1,600	1640	0.0035	0.003	0.00
654%	30%	2,400	2490	0.01	0.0065	0.01
872%	40%	3,200	3280	0.018	0.0125	0.02
1090%	50%	4,000	4080	0.025	0.0185	0.02
1308%	60%	4,800	4800	0.0425	0.0325	0.04
1526%	70%	5,600	5570	0.0555	0.0435	0.05
1744%	80%	6,400	6430	0.072	0.057	0.06
1962%	90%	7,200	7120	0.088	0.0695	0.08
2180%	100%	8,000	8100	0.123	0.095	0.11
0%	0%	0	0	0.086	0.0845	0.09

Load at 0.9-in axial displacement: N/A lb

Residual axial displacement: 0.09 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-32A

Lateral Load Test Results

PTL- **32** Designation: **A**

Section: W6x9

Latitude: 33.652344

Longitude: -114.7029

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.11

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

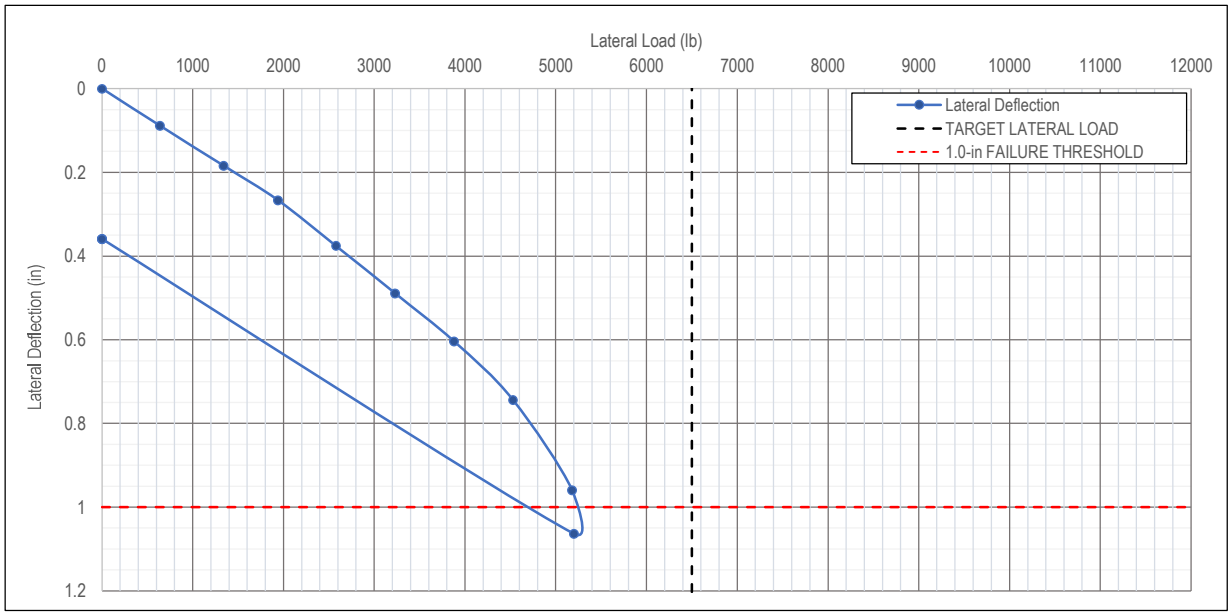
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	640	0.086	0.091	0.09
62%	20%	1,300	1340	0.179	0.1895	0.18
93%	30%	1,950	1940	0.263	0.27	0.27
124%	40%	2,600	2580	0.3675	0.384	0.38
155%	50%	3,250	3230	0.4785	0.5	0.49
186%	60%	3,900	3880	0.588	0.6195	0.60
217%	70%	4,550	4530	0.715	0.773	0.74
248%	80%	5,200	5180	0.8805	1.0385	0.96
279%	90%	5,850	5200	0.952	1.175	1.06
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.265	0.453	0.36

Load at 1.0-in lateral deflection: **5,188** **lb**

Residual lateral deformation: **0.36** **in**



Notes:

Failure reached at 5,200 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-32B

Axial Uplift Load Test Results

PTL - **32** Designation: **B**

Section: W6x9

Latitude: 33.652344

Longitude: -114.7029

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 27.26

Pile Damaged? No

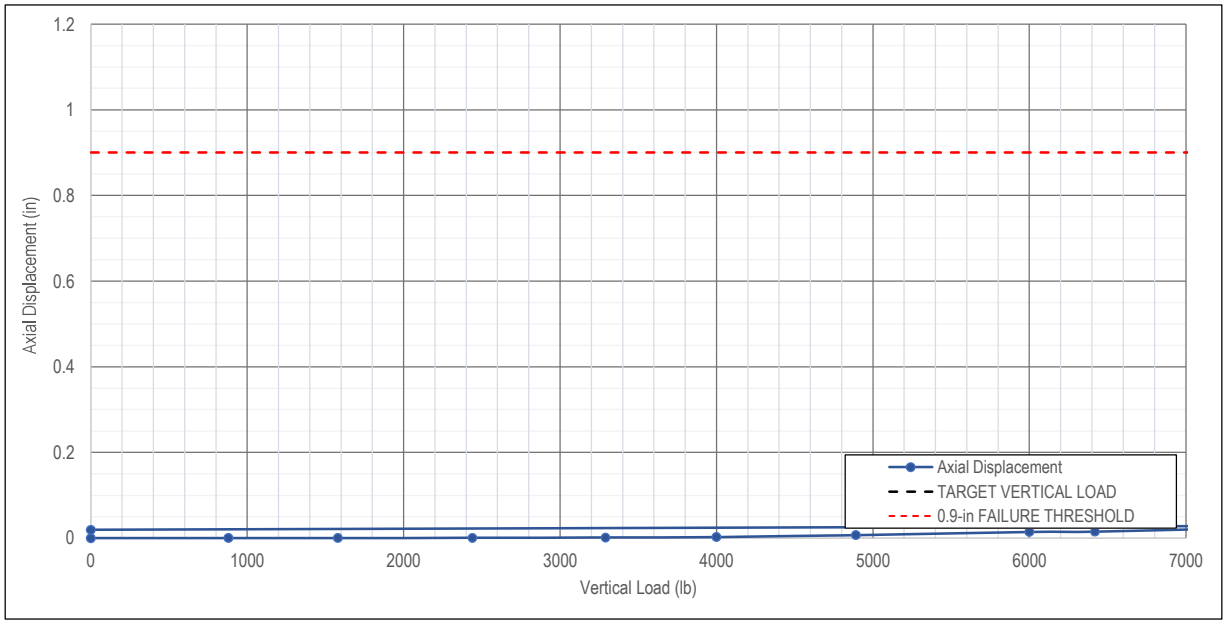
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	880	0	0.0005	0.00
436%	20%	1,600	1580	0	0.0005	0.00
654%	30%	2,400	2440	0.001	0.0005	0.00
872%	40%	3,200	3290	0.0025	0.0005	0.00
1090%	50%	4,000	4000	0.0045	0.001	0.00
1308%	60%	4,800	4890	0.0105	0.004	0.01
1526%	70%	5,600	6000	0.0195	0.0095	0.01
1744%	80%	6,400	6420	0.02	0.01	0.02
1962%	90%	7,200	7290	0.029	0.016	0.02
2180%	100%	8,000	8100	0.036	0.0215	0.03
0%	0%	0	0	0.02	0.019	0.02

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.02** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-32B

Lateral Load Test Results

PTL- **32** Designation: **B**

Section: W6x9

Latitude: 33.652344

Longitude: -114.7029

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 27.26

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

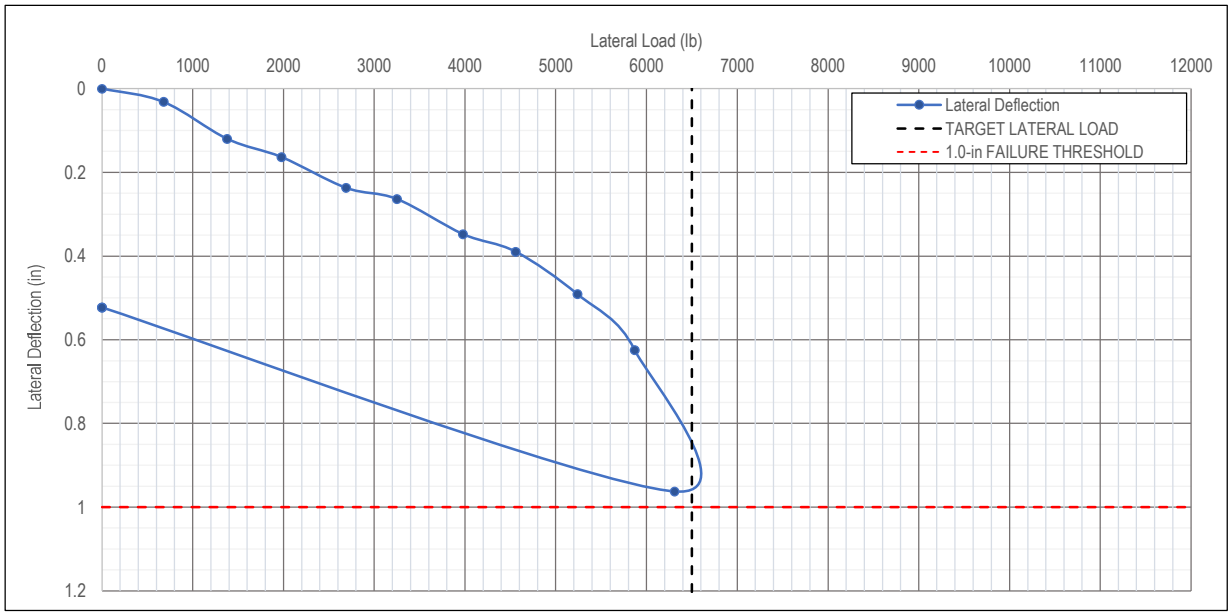
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	680	0.061	0.002	0.03
62%	20%	1,300	1380	0.1205	0.1195	0.12
93%	30%	1,950	1980	0.1655	0.1615	0.16
124%	40%	2,600	2690	0.24	0.233	0.24
155%	50%	3,250	3250	0.2675	0.26	0.26
186%	60%	3,900	3980	0.3525	0.3425	0.35
217%	70%	4,550	4560	0.3935	0.386	0.39
248%	80%	5,200	5240	0.489	0.4935	0.49
279%	90%	5,850	5870	0.602	0.6475	0.62
310%	100%	6,500	6310	0.8	1.125	0.96
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.3805	0.665	0.52

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.52** **in**



Notes:

Pile twisting at 6,310 lbs. No reliable test results available after this point.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-33A

Axial Uplift Load Test Results

PTL - **33** Designation: **A**

Section: W6x9

Latitude: 33.652503

Longitude: -114.69026

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 7.17

Pile Damaged? No

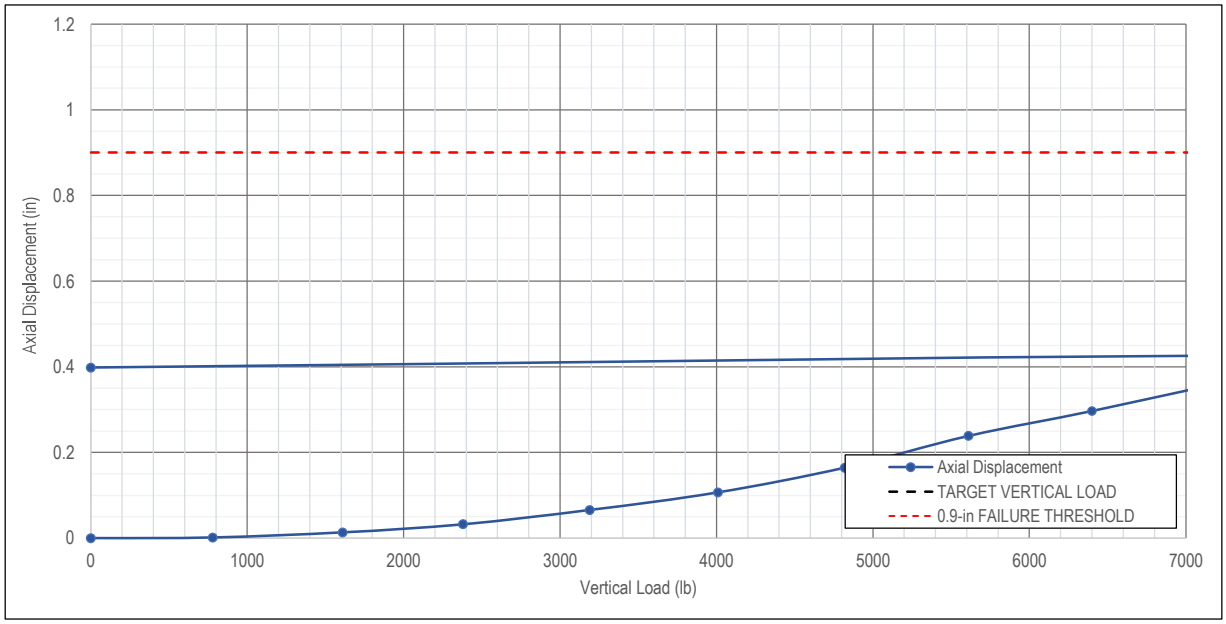
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	780	0.002	0.001	0.00
13594%	20%	1,600	1610	0.0145	0.012	0.01
20391%	30%	2,400	2380	0.0355	0.0295	0.03
27188%	40%	3,200	3190	0.069	0.0625	0.07
33985%	50%	4,000	4010	0.111	0.103	0.11
40782%	60%	4,800	4820	0.169	0.16	0.16
47579%	70%	5,600	5610	0.243	0.2345	0.24
54376%	80%	6,400	6400	0.3	0.2935	0.30
61173%	90%	7,200	7230	0.3645	0.36	0.36
67970%	100%	8,000	8060	0.427	0.424	0.43
0%	0%	0	0	0.399	0.397	0.40

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.40** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-33A

Lateral Load Test Results

PTL- **33** Designation: **A**

Section: W6x9

Latitude: 33.652503

Longitude: -114.69026

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 7.17

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

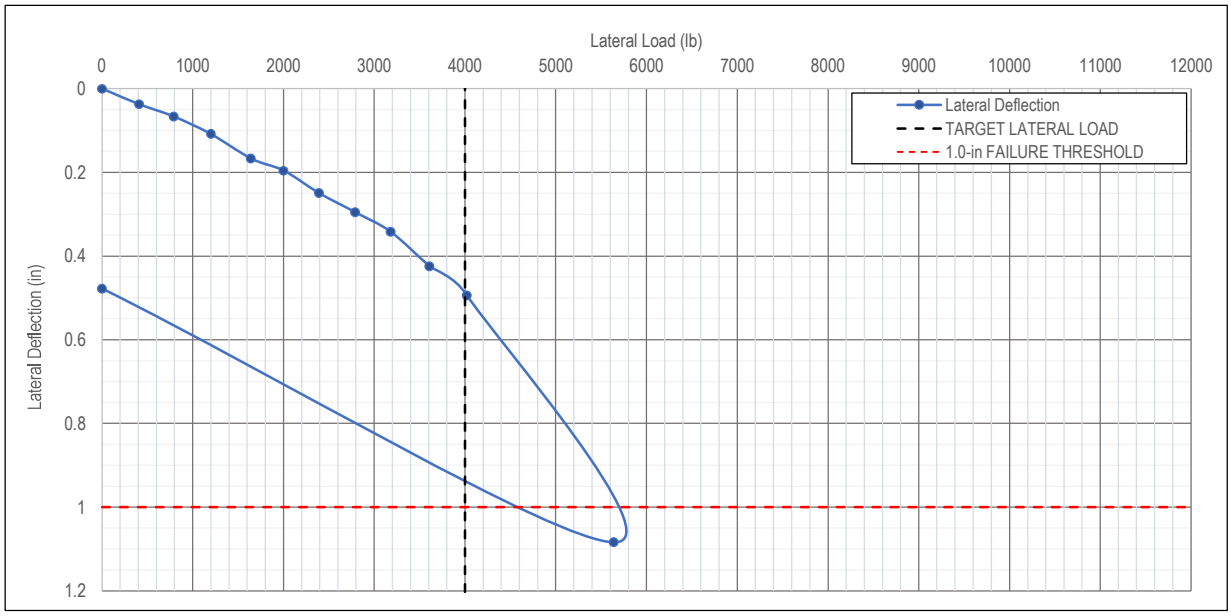
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	410	0.036	0.038	0.04
62%	20%	800	790	0.064	0.0685	0.07
93%	30%	1,200	1200	0.104	0.112	0.11
124%	40%	1,600	1640	0.1605	0.173	0.17
155%	50%	2,000	2000	0.1885	0.203	0.20
186%	60%	2,400	2390	0.24	0.258	0.25
217%	70%	2,800	2790	0.2845	0.306	0.30
248%	80%	3,200	3180	0.329	0.3545	0.34
279%	90%	3,600	3610	0.408	0.44	0.42
310%	100%	4,000	4020	0.475	0.512	0.49
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5640	1.0425	1.125	1.08
0%	0%	0	0	0.465	0.4905	0.48

Load at 1.0-in lateral deflection: 5,410 lb

Residual lateral deformation: 0.48 in



Notes:

Failure reached at 5,640 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-33B

Axial Uplift Load Test Results

PTL - **33** Designation: **B**

Section: W6x9

Latitude: 33.652503

Longitude: -114.69026

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 11.28

Pile Damaged? No

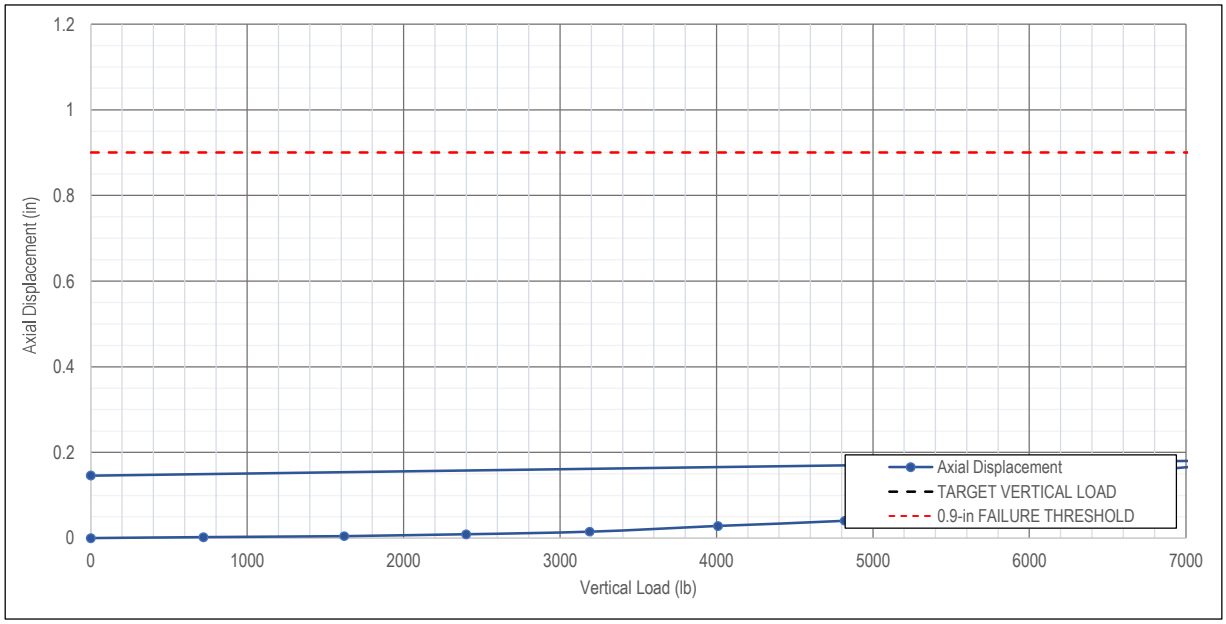
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	720	0.0025	0.0025	0.00
13594%	20%	1,600	1620	0.0046	0.005	0.00
20391%	30%	2,400	2400	0.0085	0.0095	0.01
27188%	40%	3,200	3190	0.014	0.016	0.02
33985%	50%	4,000	4010	0.0275	0.029	0.03
40782%	60%	4,800	4820	0.04	0.04115	0.04
47579%	70%	5,600	5620	0.054	0.0565	0.06
54376%	80%	6,400	6490	0.1375	0.147	0.14
61173%	90%	7,200	7240	0.167	0.177	0.17
67970%	100%	8,000	8090	0.1795	0.1905	0.19
0%	0%	0	0	0.1455	0.1465	0.15

Load at 0.9-in axial displacement: N/A lb

Residual axial displacement: 0.15 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-33B

Lateral Load Test Results

PTL- **33** Designation: **B**

Section: W6x9

Latitude: 33.652503

Longitude: -114.69026

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.28

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

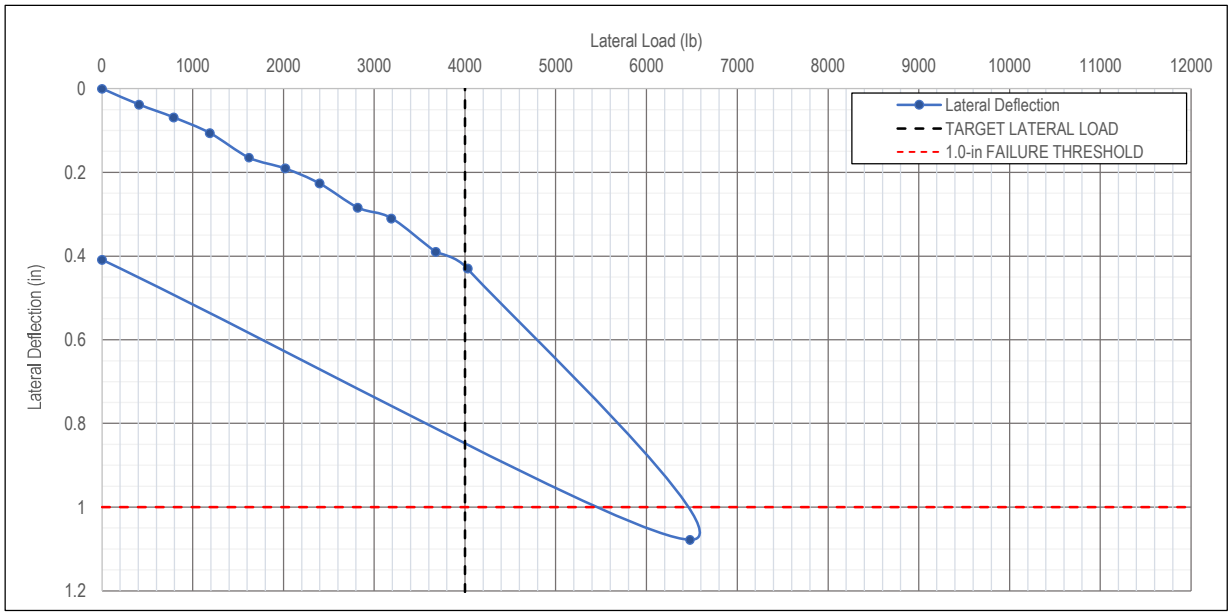
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	410	0.0375	0.039	0.04
62%	20%	800	790	0.0675	0.07	0.07
93%	30%	1,200	1190	0.104	0.1085	0.11
124%	40%	1,600	1620	0.1595	0.1695	0.16
155%	50%	2,000	2020	0.185	0.1965	0.19
186%	60%	2,400	2400	0.2195	0.2335	0.23
217%	70%	2,800	2820	0.275	0.2935	0.28
248%	80%	3,200	3190	0.3	0.3195	0.31
279%	90%	3,600	3680	0.3775	0.4025	0.39
310%	100%	4,000	4030	0.415	0.445	0.43
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6480	1.0785	1.0775	1.08
0%	0%	0	0	0.399	0.419	0.41

Load at 1.0-in lateral deflection: 6,185 lb

Residual lateral deformation: 0.41 in



Notes:

Failure reached at 6,480 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-34A

Axial Uplift Load Test Results

PTL - **34** Designation: **A**

Section: W6x9

Latitude: 33.652503

Longitude: -114.68731

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 10.54

Pile Damaged? No

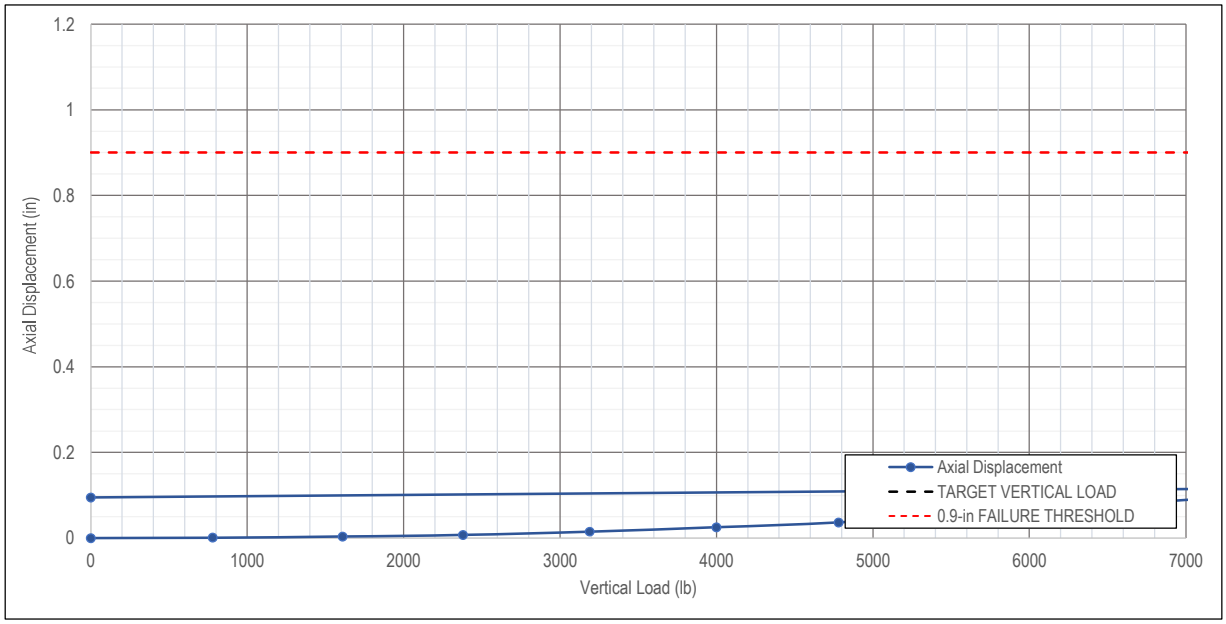
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	780	0.001	0.0015	0.00
436%	20%	1,600	1610	0.003	0.0045	0.00
654%	30%	2,400	2380	0.0065	0.0085	0.01
872%	40%	3,200	3190	0.014	0.0165	0.02
1090%	50%	4,000	4000	0.024	0.0265	0.03
1308%	60%	4,800	4780	0.0355	0.038	0.04
1526%	70%	5,600	5620	0.0545	0.0565	0.06
1744%	80%	6,400	6410	0.074	0.0755	0.07
1962%	90%	7,200	7230	0.0975	0.092	0.09
2180%	100%	8,000	8080	0.1215	0.111	0.12
0%	0%	0	0	0.0955	0.095	0.10

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.10** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-34A

Lateral Load Test Results

PTL- **34** Designation: **A**

Section: W6x9

Latitude: 33.652503

Longitude: -114.68731

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 10.54

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

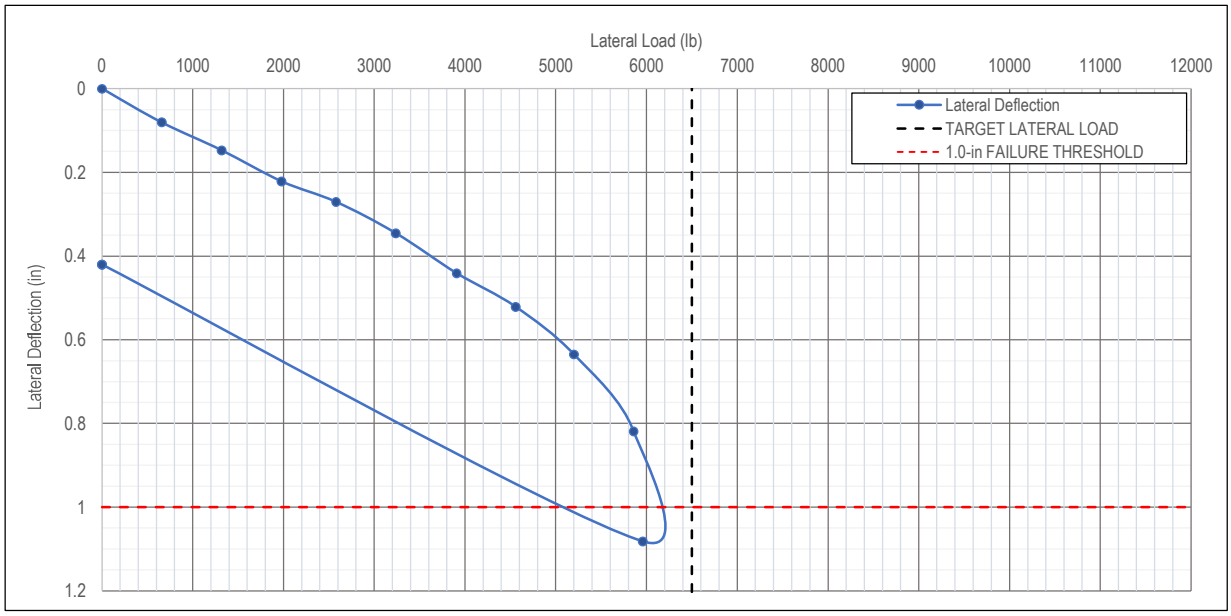
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	660	0.0845	0.0765	0.08
62%	20%	1,300	1320	0.1545	0.14	0.15
93%	30%	1,950	1980	0.231	0.2115	0.22
124%	40%	2,600	2580	0.2825	0.258	0.27
155%	50%	3,250	3240	0.3615	0.3295	0.35
186%	60%	3,900	3910	0.462	0.42	0.44
217%	70%	4,550	4560	0.5485	0.4945	0.52
248%	80%	5,200	5200	0.674	0.5955	0.63
279%	90%	5,850	5860	0.901	0.7375	0.82
310%	100%	6,500	5960	1.1545	1.009	1.08
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.491	0.349	0.42

Load at 1.0-in lateral deflection: **5,929** **lb**

Residual lateral deformation: **0.42** **in**



Notes:

Failure reached at 5,960 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-34B

Axial Uplift Load Test Results

PTL - **34** Designation: **B**

Section: W6x9

Latitude: 33.652503

Longitude: -114.68731

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 20.37

Pile Damaged? No

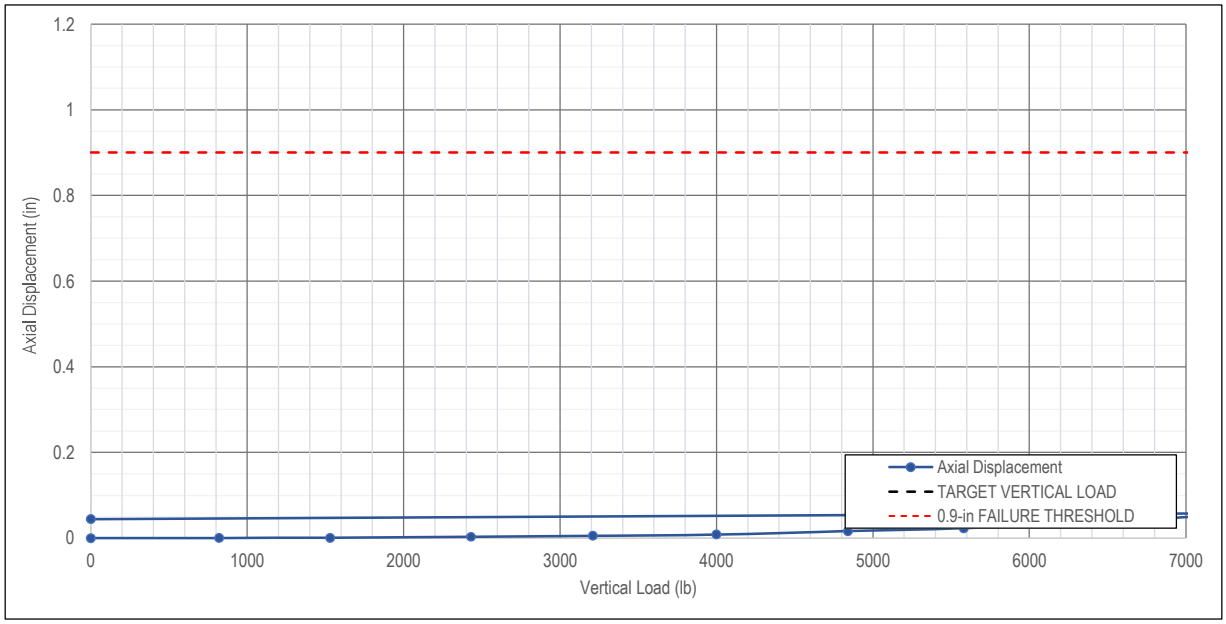
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	820	0.001	0	0.00
436%	20%	1,600	1530	0.001	0.001	0.00
654%	30%	2,400	2430	0.003	0.003	0.00
872%	40%	3,200	3210	0.0065	0.0045	0.01
1090%	50%	4,000	4000	0.01	0.0065	0.01
1308%	60%	4,800	4840	0.0195	0.013	0.02
1526%	70%	5,600	5580	0.0265	0.0195	0.02
1744%	80%	6,400	6480	0.041	0.0325	0.04
1962%	90%	7,200	7210	0.0575	0.049	0.05
2180%	100%	8,000	8020	0.063	0.056	0.06
0%	0%	0	0	0.0445	0.0445	0.04

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.04** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-34B

Lateral Load Test Results

PTL- **34** Designation: **B**

Section: W6x9

Latitude: 33.652503

Longitude: -114.68731

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 20.37

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

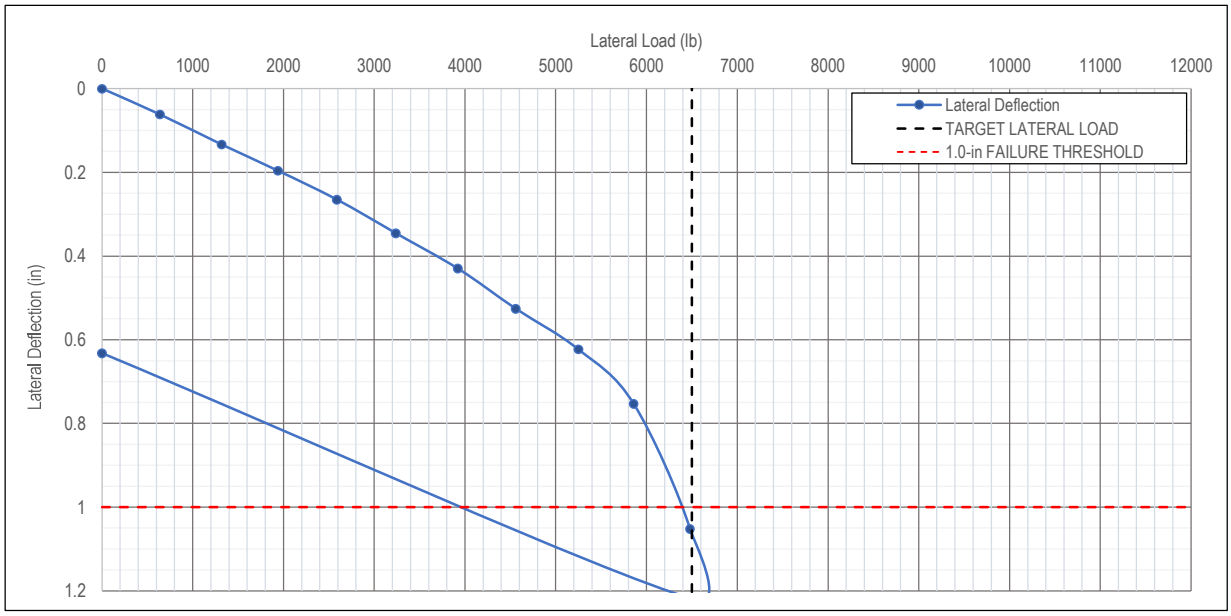
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	640	0.062	0.061	0.06
62%	20%	1,300	1320	0.136	0.1305	0.13
93%	30%	1,950	1940	0.2005	0.1915	0.20
124%	40%	2,600	2590	0.272	0.2585	0.27
155%	50%	3,250	3240	0.354	0.3365	0.35
186%	60%	3,900	3920	0.441	0.418	0.43
217%	70%	4,550	4560	0.5415	0.51	0.53
248%	80%	5,200	5250	0.6435	0.602	0.62
279%	90%	5,850	5860	0.7865	0.719	0.75
310%	100%	6,500	6480	1.183	0.9205	1.05
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6530	1.364	1.078	1.22
0%	0%	0	0	0.732	0.532	0.63

Load at 1.0-in lateral deflection: **6,373** **lb**

Residual lateral deformation: **0.63** **in**



Notes:

Failure reached at 6,480 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-34D

Axial Uplift Load Test Results

PTL - **34** Designation: **D**

Section: **W6x12**

Latitude: 33.652503

Longitude: -114.68731

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 26.27

Pile Damaged? No

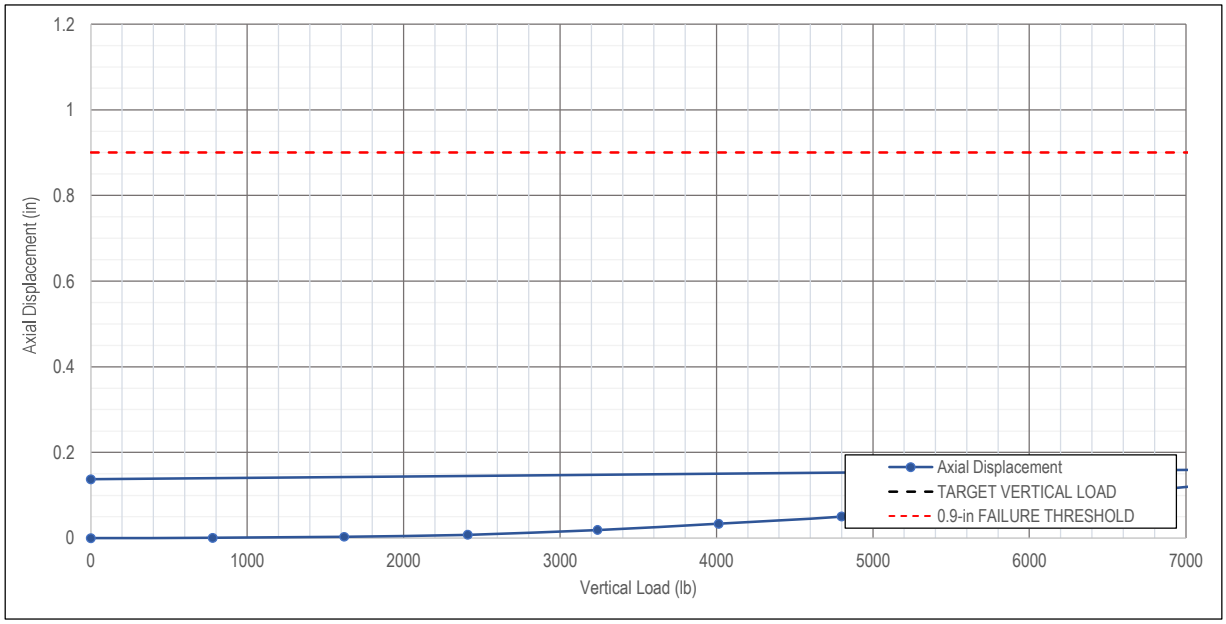
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	780	0.001	0.001	0.00
436%	20%	1,600	1620	0.0035	0.003	0.00
654%	30%	2,400	2410	0.0085	0.0075	0.01
872%	40%	3,200	3240	0.0195	0.0185	0.02
1090%	50%	4,000	4015	0.034	0.033	0.03
1308%	60%	4,800	4800	0.051	0.05	0.05
1526%	70%	5,600	5580	0.0745	0.0735	0.07
1744%	80%	6,400	6390	0.0995	0.0985	0.10
1962%	90%	7,200	7220	0.128	0.127	0.13
2180%	100%	8,000	8040	0.1605	0.16	0.16
0%	0%	0	0	0.138	0.1375	0.14

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.14** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-34D

Lateral Load Test Results

PTL- **34** Designation: **D**

Section: **W6x12**

Latitude: 33.652503

Longitude: -114.68731

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 26.27

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

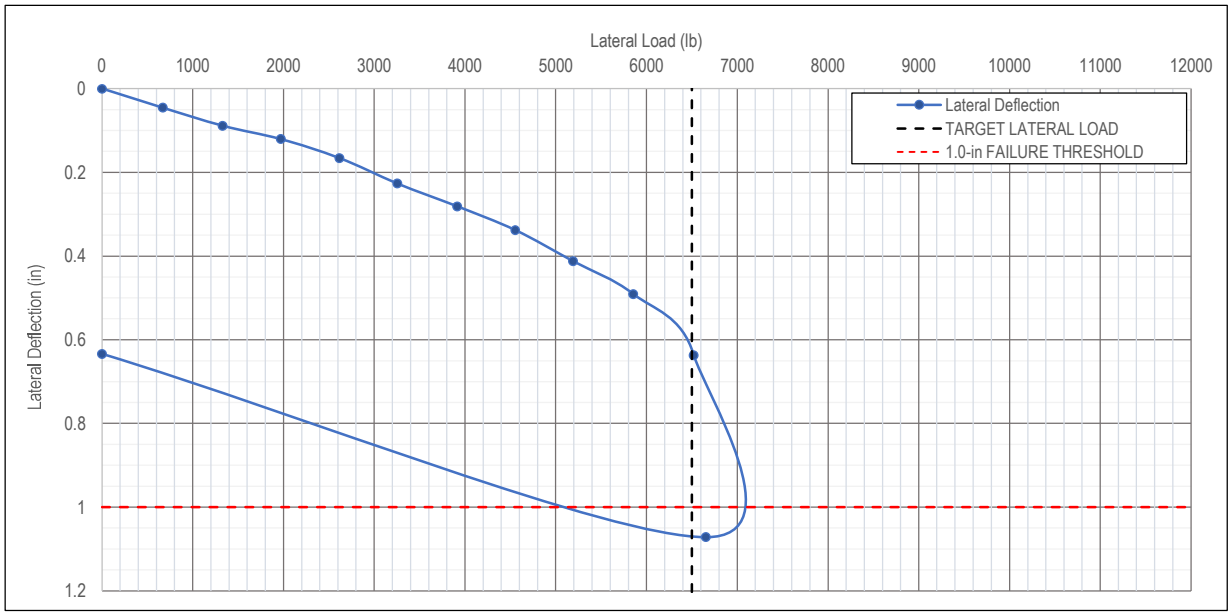
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	670	0.0435	0.047	0.05
62%	20%	1,300	1330	0.086	0.0915	0.09
93%	30%	1,950	1970	0.117	0.1235	0.12
124%	40%	2,600	2615	0.162	0.1695	0.17
155%	50%	3,250	3255	0.2225	0.2305	0.23
186%	60%	3,900	3915	0.2775	0.2845	0.28
217%	70%	4,550	4555	0.3345	0.341	0.34
248%	80%	5,200	5190	0.407	0.417	0.41
279%	90%	5,850	5855	0.4875	0.494	0.49
310%	100%	6,500	6520	0.6325	0.6415	0.64
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6655	1.092	1.051	1.07
0%	0%	0	0	0.651	0.6165	0.63

Load at 1.0-in lateral deflection: **6,633** **lb**

Residual lateral deformation: **0.63** **in**



Notes:

Failure reached at 6,650 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-35A

Axial Uplift Load Test Results

PTL - **35** Designation: **A**

Section: W6x9

Latitude: 33.652771

Longitude: -114.67971

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 9.18

Pile Damaged? No

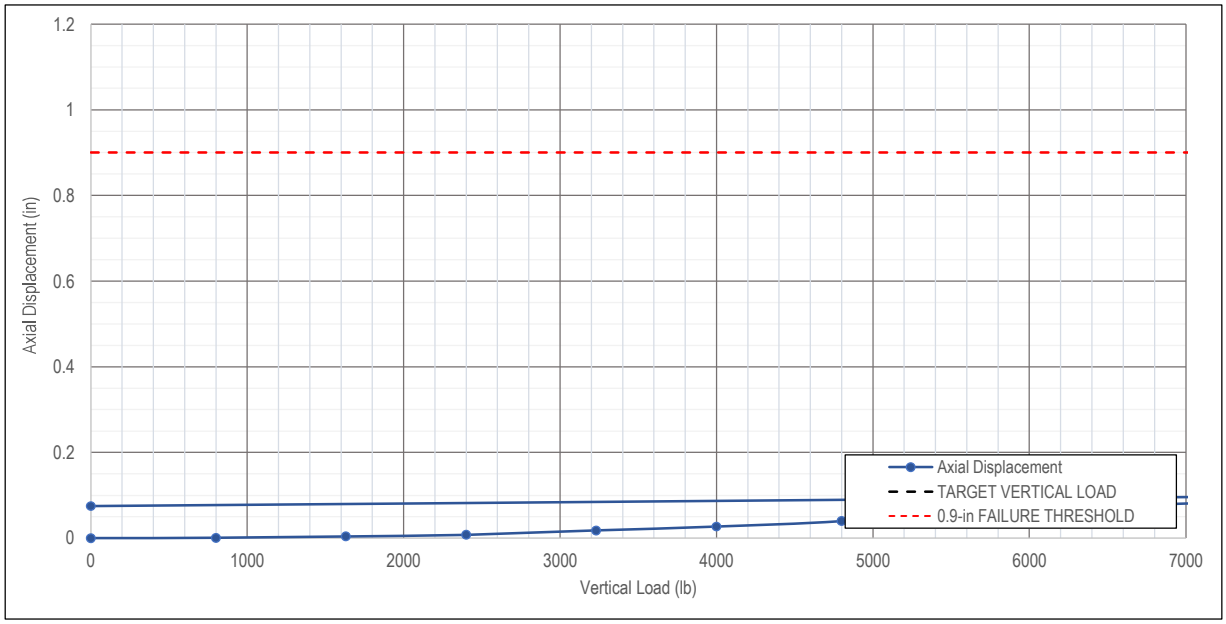
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	800	0	0.002	0.00
13594%	20%	1,600	1630	0.002	0.006	0.00
20391%	30%	2,400	2400	0.006	0.01	0.01
27188%	40%	3,200	3230	0.018	0.018	0.02
33985%	50%	4,000	4000	0.026	0.028	0.03
40782%	60%	4,800	4800	0.04	0.04	0.04
47579%	70%	5,600	5640	0.068	0.068	0.07
54376%	80%	6,400	6380	0.074	0.074	0.07
61173%	90%	7,200	7230	0.084	0.084	0.08
67970%	100%	8,000	8000	0.098	0.098	0.10
0%	0%	0	0	0.076	0.074	0.08

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.08** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-35A

Lateral Load Test Results

PTL- **35** Designation: **A**

Section: W6x9

Latitude: 33.652771

Longitude: -114.67971

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 9.18

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

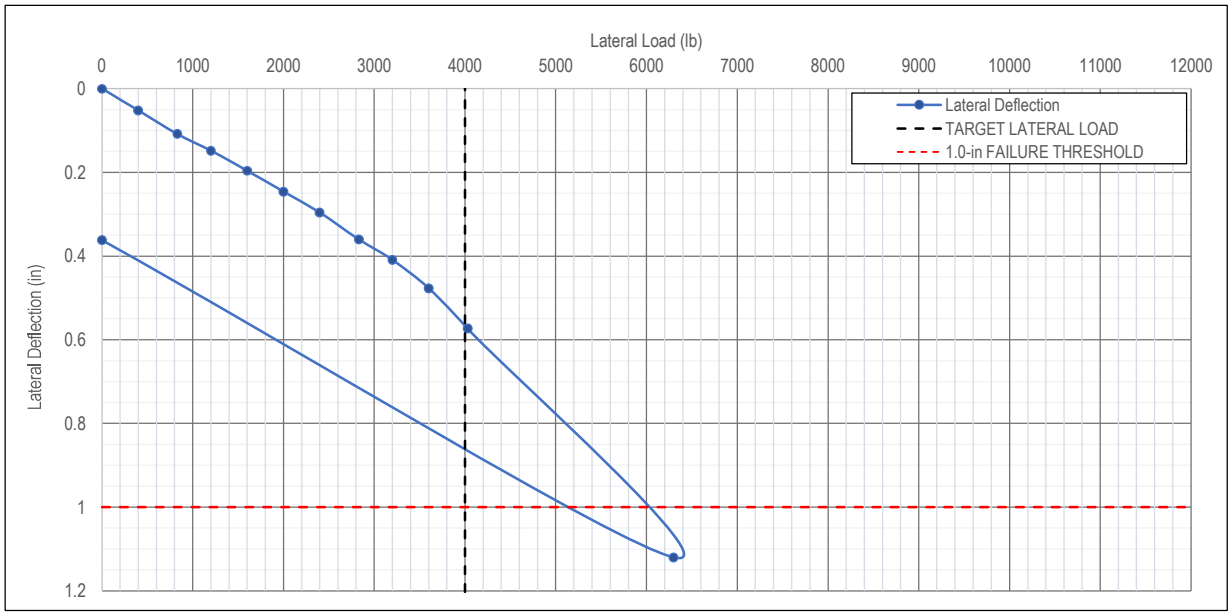
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.054	0.05	0.05
62%	20%	800	830	0.112	0.104	0.11
93%	30%	1,200	1200	0.154	0.142	0.15
124%	40%	1,600	1600	0.204	0.188	0.20
155%	50%	2,000	2000	0.256	0.236	0.25
186%	60%	2,400	2400	0.308	0.284	0.30
217%	70%	2,800	2830	0.374	0.346	0.36
248%	80%	3,200	3200	0.426	0.392	0.41
279%	90%	3,600	3600	0.496	0.458	0.48
310%	100%	4,000	4030	0.596	0.55	0.57
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6300	1.15	1.09	1.12
0%	0%	0	0	0.37	0.354	0.36

Load at 1.0-in lateral deflection: **5,802** **lb**

Residual lateral deformation: **0.36** **in**



Notes:

Failure reached at 6,300 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-35B

Axial Uplift Load Test Results

PTL - **35** Designation: **B**

Section: W6x9

Latitude: 33.652771

Longitude: -114.67971

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 10.1

Pile Damaged? No

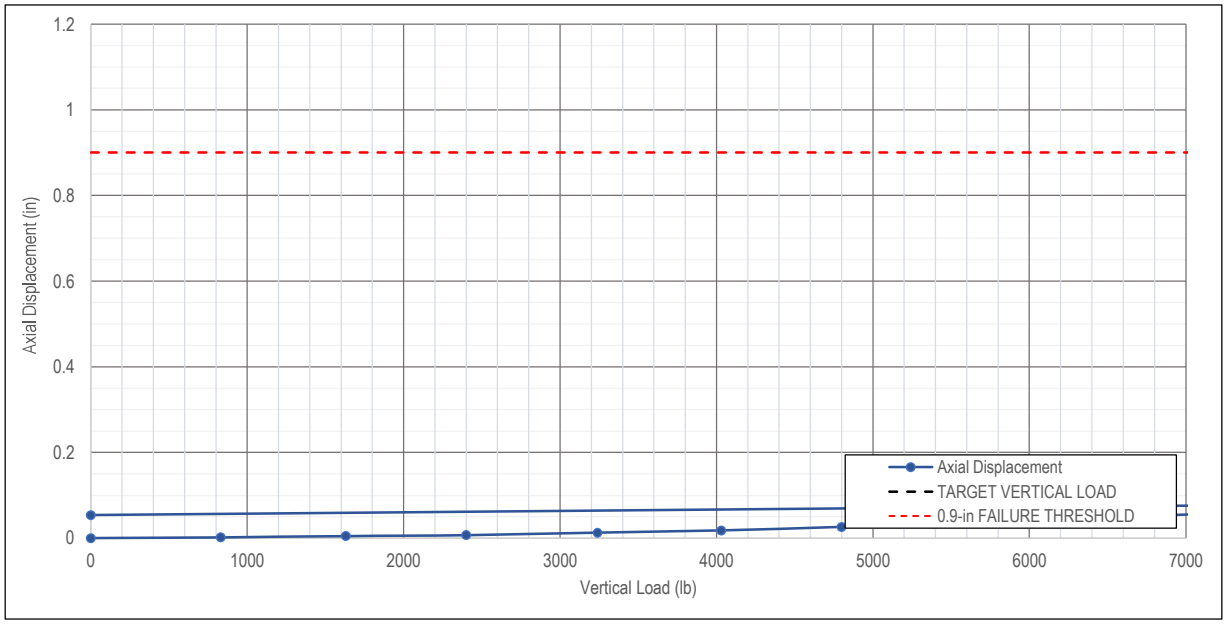
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	830	0.002	0.002	0.00
13594%	20%	1,600	1630	0.004	0.006	0.01
20391%	30%	2,400	2400	0.006	0.008	0.01
27188%	40%	3,200	3240	0.012	0.014	0.01
33985%	50%	4,000	4030	0.018	0.018	0.02
40782%	60%	4,800	4800	0.026	0.026	0.03
47579%	70%	5,600	5600	0.036	0.036	0.04
54376%	80%	6,400	6430	0.05	0.048	0.05
61173%	90%	7,200	7180	0.06	0.056	0.06
67970%	100%	8,000	8040	0.08	0.076	0.08
0%	0%	0	0	0.056	0.052	0.05

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.05** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-35B

Lateral Load Test Results

PTL- **35** Designation: **B**

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Section: W6x9

Installation Method: Direct Drive (DD)

Saturated Soil? No

Latitude: 33.652771

- -

Drive Time (s): 10.1

Longitude: -114.67971

Target Embedment Depth (ft): 7.00

Pile Damaged? No

Tested By: ANS

Actual Embedment Depth (ft): 7

Gauge 1 Height (in): 6

Test Date: 10/28/2025

Lateral Load Application Height (ft): 5

Gauge 2 Height (in): 6

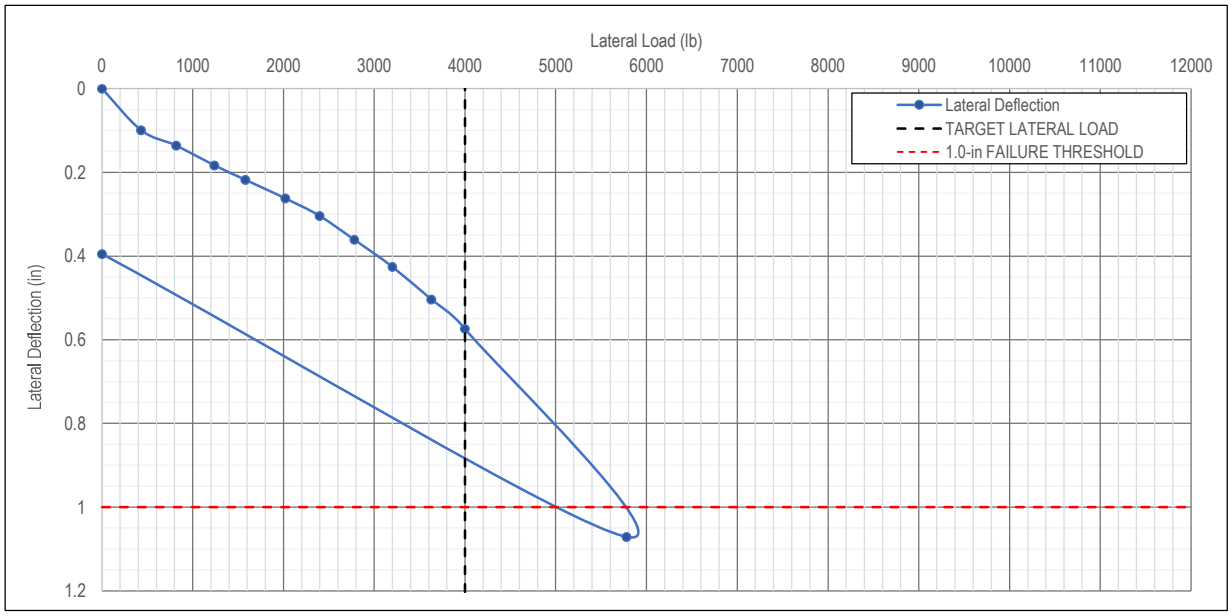
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	430	0.104	0.094	0.10
62%	20%	800	820	0.142	0.13	0.14
93%	30%	1,200	1240	0.19	0.176	0.18
124%	40%	1,600	1580	0.226	0.21	0.22
155%	50%	2,000	2020	0.272	0.252	0.26
186%	60%	2,400	2400	0.316	0.292	0.30
217%	70%	2,800	2780	0.374	0.348	0.36
248%	80%	3,200	3200	0.442	0.41	0.43
279%	90%	3,600	3630	0.522	0.486	0.50
310%	100%	4,000	4000	0.594	0.554	0.57
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5780	1.126	1.016	1.07
0%	0%	0	0	0.424	0.366	0.40

Load at 1.0-in lateral deflection: **5,526** **lb**

Residual lateral deformation: **0.40** **in**



Notes:

Failure reached at 5,780 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-36A

Axial Uplift Load Test Results

PTL - **36** Designation: **A**

Section: W6x9

Latitude: 33.656183

Longitude: -114.67913

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 11.16

Pile Damaged? No

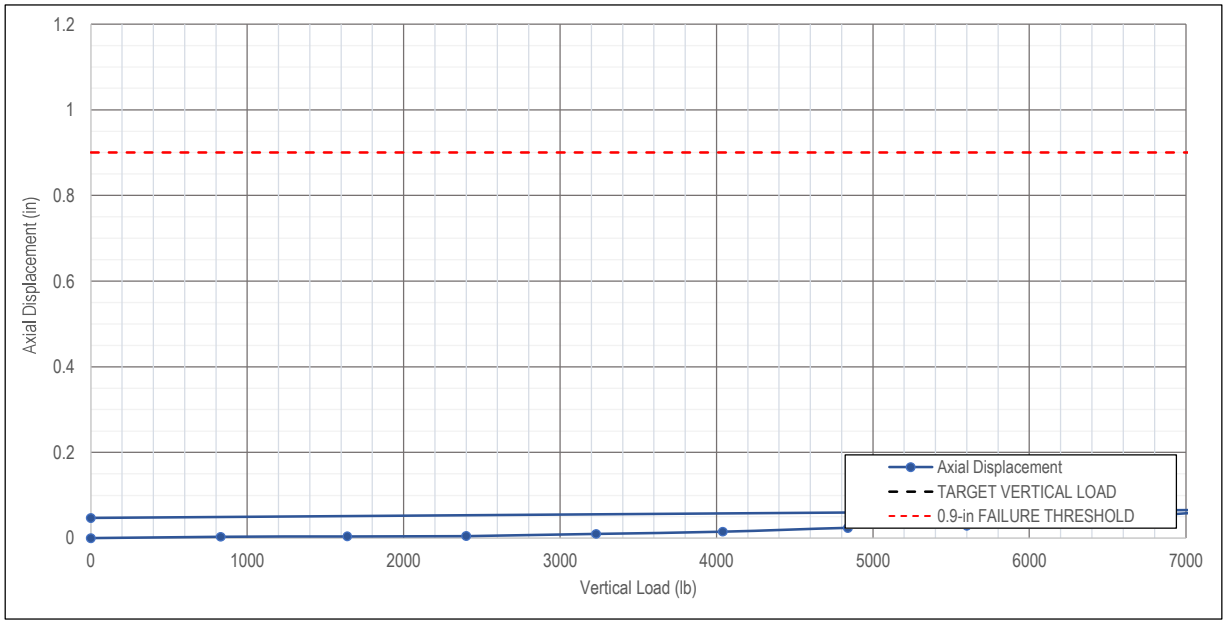
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	830	0.002	0.004	0.00
436%	20%	1,600	1640	0.004	0.004	0.00
654%	30%	2,400	2400	0.004	0.006	0.01
872%	40%	3,200	3230	0.01	0.01	0.01
1090%	50%	4,000	4040	0.016	0.014	0.02
1308%	60%	4,800	4840	0.026	0.022	0.02
1526%	70%	5,600	5600	0.032	0.026	0.03
1744%	80%	6,400	6430	0.048	0.038	0.04
1962%	90%	7,200	7240	0.068	0.058	0.06
2180%	100%	8,000	8020	0.074	0.062	0.07
0%	0%	0	0	0.048	0.046	0.05

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.05** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-36A

Lateral Load Test Results

PTL- **36** Designation: **A**

Section: W6x9

Latitude: 33.656183

Longitude: -114.67913

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.16

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

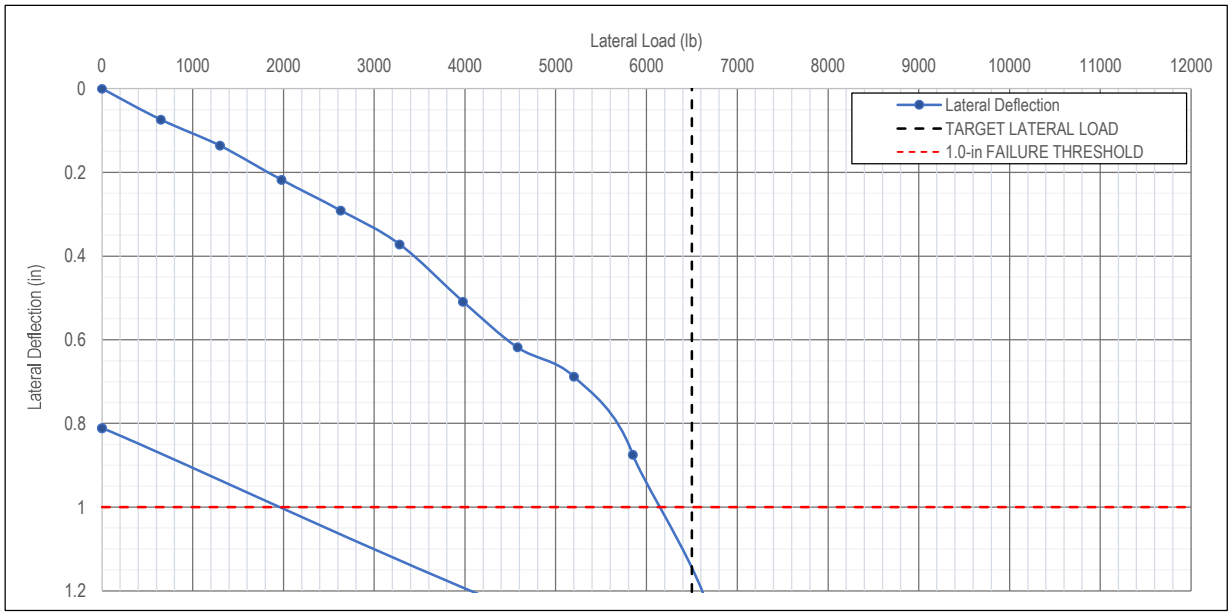
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.076	0.072	0.07
62%	20%	1,300	1300	0.14	0.132	0.14
93%	30%	1,950	1980	0.224	0.212	0.22
124%	40%	2,600	2630	0.3	0.282	0.29
155%	50%	3,250	3280	0.384	0.36	0.37
186%	60%	3,900	3980	0.526	0.492	0.51
217%	70%	4,550	4580	0.642	0.594	0.62
248%	80%	5,200	5200	0.72	0.656	0.69
279%	90%	5,850	5850	0.934	0.816	0.88
310%	100%	6,500	6380	1.518	1.224	1.37
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.918	0.704	0.81

Load at 1.0-in lateral deflection: **5,984** **lb**

Residual lateral deformation: **0.81** **in**



Notes:

Failure reached at 6,380 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-36B

Axial Uplift Load Test Results

PTL - **36** Designation: **B**

Section: W6x9

Latitude: 33.656183

Longitude: -114.67913

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 16.63

Pile Damaged? No

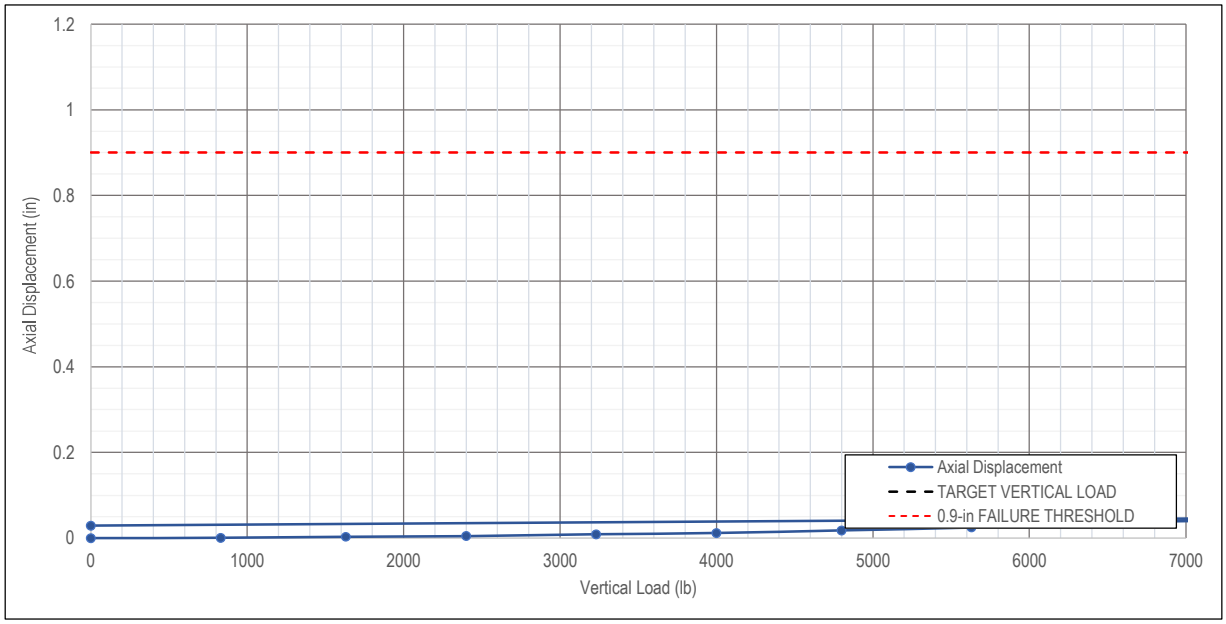
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	830	0	0.002	0.00
436%	20%	1,600	1630	0.002	0.004	0.00
654%	30%	2,400	2400	0.004	0.006	0.01
872%	40%	3,200	3230	0.008	0.01	0.01
1090%	50%	4,000	4000	0.012	0.012	0.01
1308%	60%	4,800	4800	0.018	0.018	0.02
1526%	70%	5,600	5630	0.026	0.024	0.03
1744%	80%	6,400	6430	0.038	0.034	0.04
1962%	90%	7,200	7230	0.042	0.04	0.04
2180%	100%	8,000	8000	0.05	0.046	0.05
0%	0%	0	0	0.03	0.028	0.03

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-36B

Lateral Load Test Results

PTL- **36** Designation: **B**

Section: W6x9

Latitude: 33.656183

Longitude: -114.67913

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 16.63

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

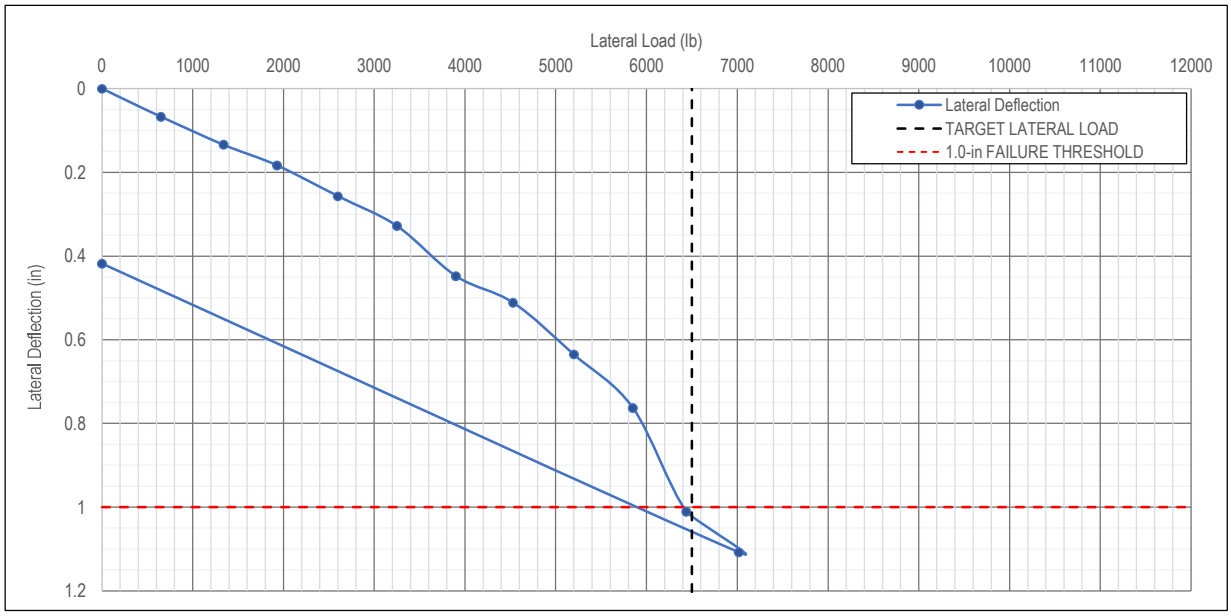
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.07	0.064	0.07
62%	20%	1,300	1340	0.138	0.13	0.13
93%	30%	1,950	1930	0.19	0.176	0.18
124%	40%	2,600	2600	0.266	0.248	0.26
155%	50%	3,250	3250	0.338	0.318	0.33
186%	60%	3,900	3900	0.462	0.434	0.45
217%	70%	4,550	4530	0.526	0.496	0.51
248%	80%	5,200	5200	0.652	0.618	0.64
279%	90%	5,850	5850	0.782	0.744	0.76
310%	100%	6,500	6440	1.036	0.986	1.01
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7020	1.136	1.08	1.11
0%	0%	0	0	0.426	0.41	0.42

Load at 1.0-in lateral deflection: **6,414** **lb**

Residual lateral deformation: **0.42** **in**



Notes:

Failure reached at 6,440 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-37A

Axial Uplift Load Test Results

PTL - **37** Designation: **A**

Section: W6x9

Latitude: 33.651298

Longitude: -114.71596

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 13.3

Pile Damaged? No

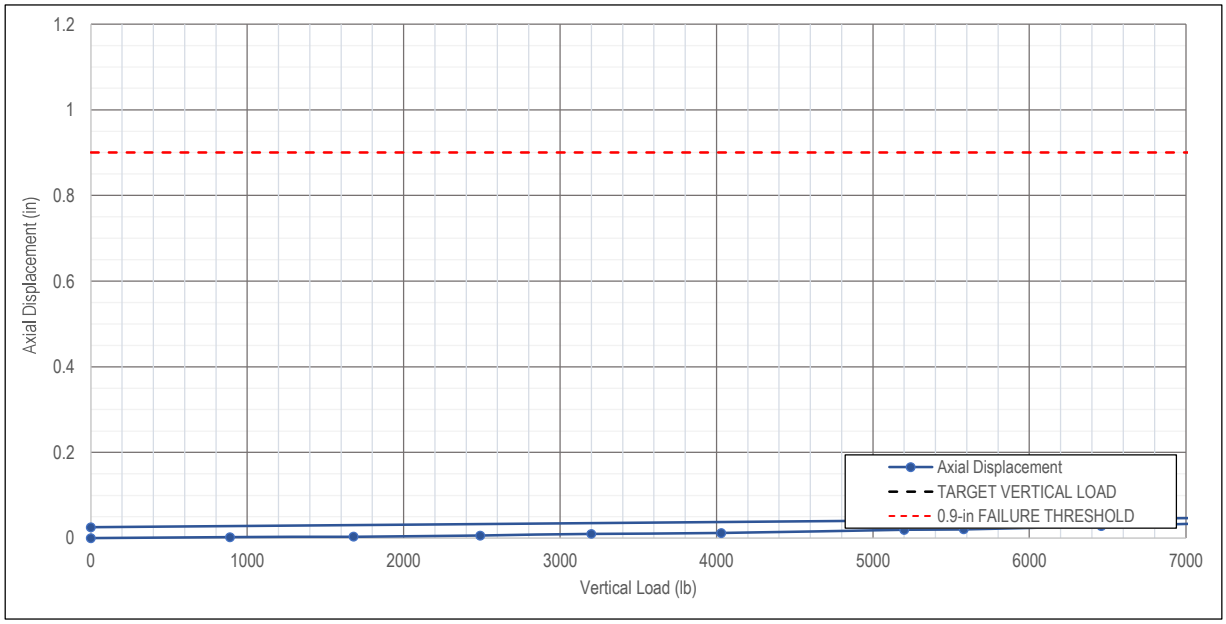
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	890	0.003	0.0015	0.00
13594%	20%	1,600	1680	0.004	0.003	0.00
20391%	30%	2,400	2490	0.007	0.0055	0.01
27188%	40%	3,200	3200	0.0105	0.009	0.01
33985%	50%	4,000	4030	0.013	0.011	0.01
40782%	60%	4,800	5200	0.02	0.0185	0.02
47579%	70%	5,600	5580	0.0215	0.0195	0.02
54376%	80%	6,400	6460	0.0295	0.027	0.03
61173%	90%	7,200	7280	0.038	0.0345	0.04
67970%	100%	8,000	8100	0.051	0.0475	0.05
0%	0%	0	0	0.0255	0.025	0.03

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-37A

Lateral Load Test Results

PTL- **37** Designation: **A**

Section: W6x9

Latitude: 33.651298

Longitude: -114.71596

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 13.3

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

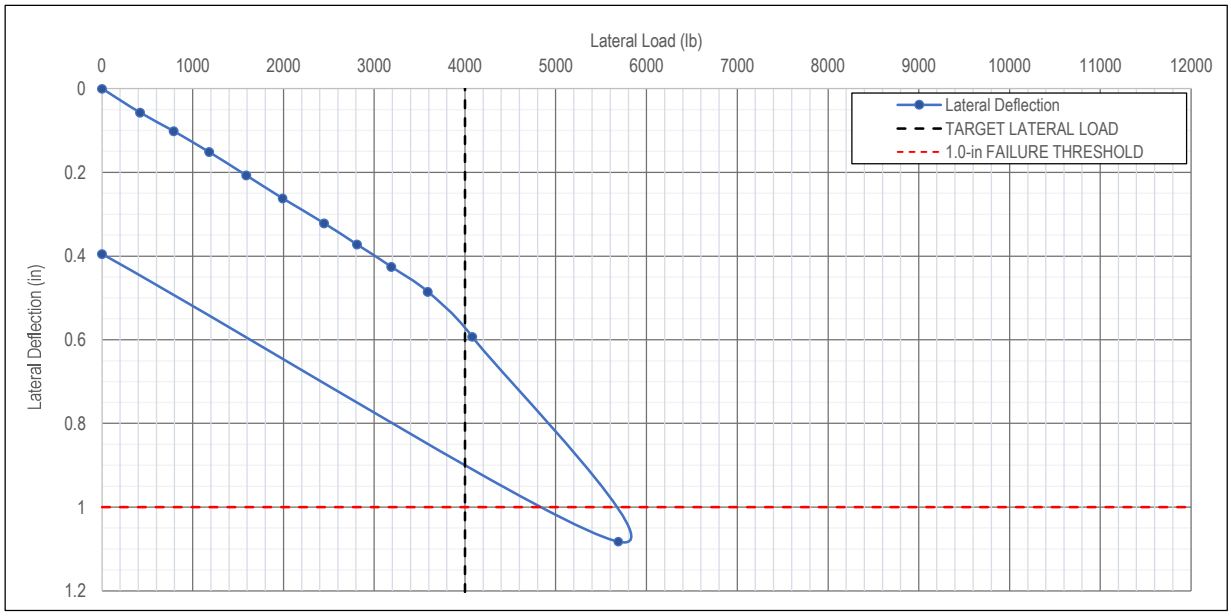
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	420	0.0545	0.0595	0.06
62%	20%	800	790	0.0975	0.1055	0.10
93%	30%	1,200	1180	0.1455	0.1565	0.15
124%	40%	1,600	1590	0.1985	0.2155	0.21
155%	50%	2,000	1990	0.251	0.2735	0.26
186%	60%	2,400	2450	0.309	0.3345	0.32
217%	70%	2,800	2810	0.3575	0.3865	0.37
248%	80%	3,200	3190	0.41	0.441	0.43
279%	90%	3,600	3590	0.4685	0.5025	0.49
310%	100%	4,000	4080	0.5975	0.588	0.59
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5690	0.995	1.1695	1.08
0%	0%	0	0	0.344	0.4465	0.40

Load at 1.0-in lateral deflection: **5,419** **lb**

Residual lateral deformation: **0.40** **in**



Notes:

Failure reached at 5,690 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-37B

Axial Uplift Load Test Results

PTL - **37** Designation: **B**

Section: W6x9

Latitude: 33.651298

Longitude: -114.71596

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 23.25

Pile Damaged? No

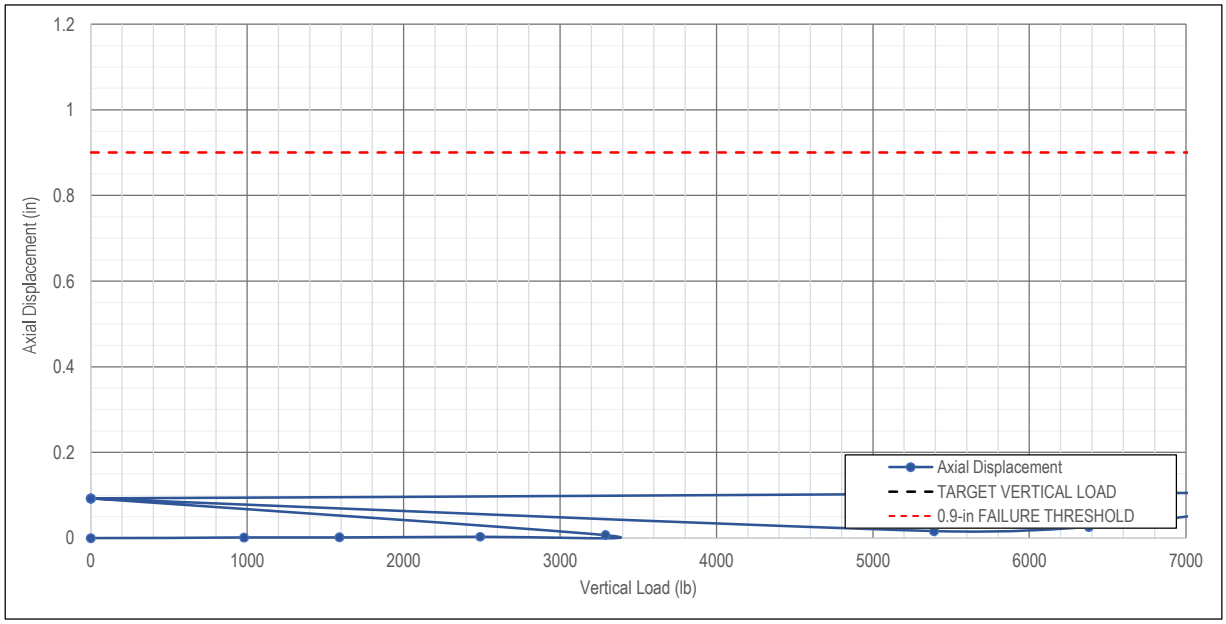
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	980	0.002	0.001	0.00
13594%	20%	1,600	1590	0.003	0.001	0.00
20391%	30%	2,400	2490	0.003	0.0035	0.00
27188%	40%	3,200	3290	0.006	0.008	0.01
33985%	50%	4,000				0.00
40782%	60%	4,800				0.00
47579%	70%	5,600	5390	0.0135	0.019	0.02
54376%	80%	6,400	6380	0.0225	0.0295	0.03
61173%	90%	7,200	7290	0.0595	0.0685	0.06
67970%	100%	8,000	8090	0.099	0.11	0.10
0%	0%	0	0	0.0925	0.0925	0.09

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.09** **in**



Notes:

4,000-lbs and 4,800-lbs load steps skipped.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-37B

Lateral Load Test Results

PTL- **37** Designation: **B**

Section: W6x9

Latitude: 33.651298

Longitude: -114.71596

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 23.25

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

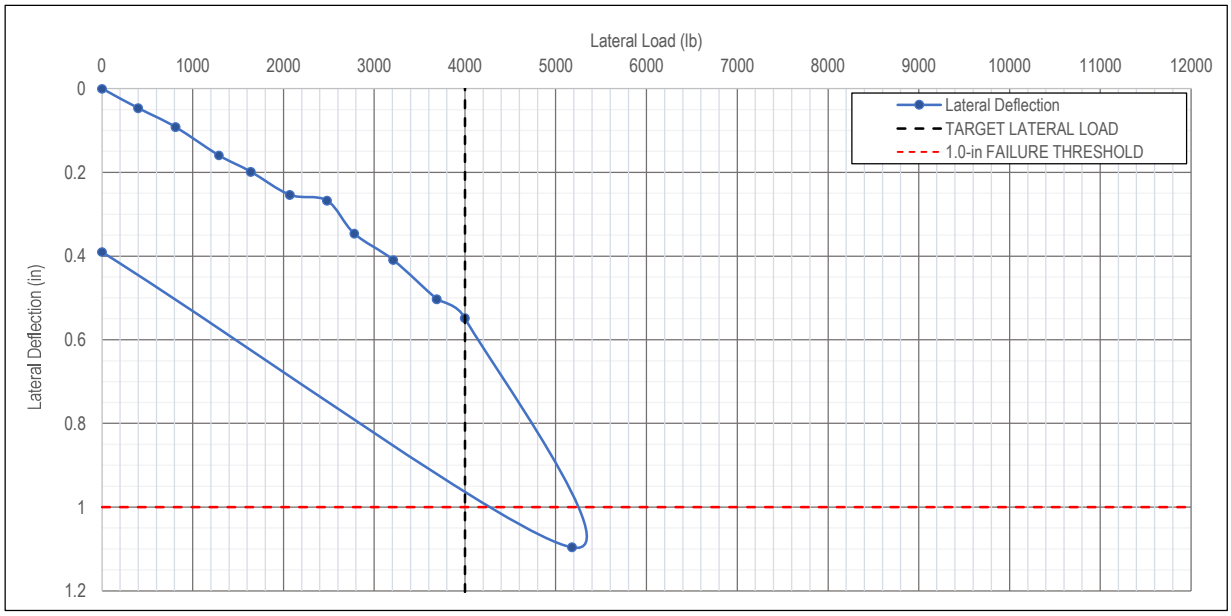
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.0455	0.047	0.05
62%	20%	800	810	0.0895	0.0935	0.09
93%	30%	1,200	1290	0.1525	0.166	0.16
124%	40%	1,600	1640	0.1895	0.2075	0.20
155%	50%	2,000	2070	0.242	0.265	0.25
186%	60%	2,400	2480	0.288	0.2465	0.27
217%	70%	2,800	2780	0.33	0.362	0.35
248%	80%	3,200	3210	0.39	0.4285	0.41
279%	90%	3,600	3690	0.477	0.5285	0.50
310%	100%	4,000	4000	0.5175	0.5795	0.55
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5180	0.926	1.266	1.10
0%	0%	0	0	0.1515	0.629	0.39

Load at 1.0-in lateral deflection: **4,973** **lb**

Residual lateral deformation: **0.39** **in**



Notes:

Failure reached at 5,180 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-38A

Axial Uplift Load Test Results

PTL - **38** Designation: **A** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.651142 - - Saturated Soil? No

Longitude: -114.71181 Target Embedment Depth (ft): 6.00 Drive Time (s): 13.5

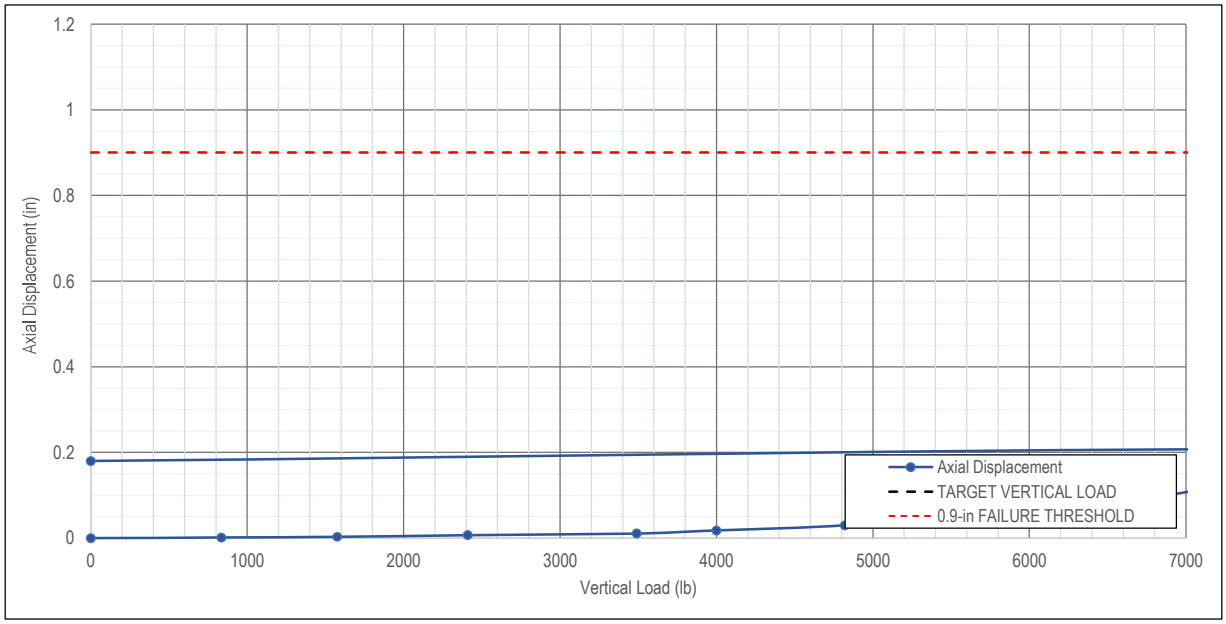
Tested By: ANS Actual Embedment Depth (ft): 6 Pile Damaged? No

Test Date: 10/24/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	835	0.001	0.002	0.00
436%	20%	1,600	1575	0.003	0.0035	0.00
654%	30%	2,400	2410	0.0075	0.0065	0.01
872%	40%	3,200	3490	0.0115	0.01	0.01
1090%	50%	4,000	4000	0.0195	0.0165	0.02
1308%	60%	4,800	4820	0.0325	0.027	0.03
1526%	70%	5,600	5625	0.0595	0.0535	0.06
1744%	80%	6,400	6430	0.0845	0.0765	0.08
1962%	90%	7,200	7235	0.128	0.119	0.12
2180%	100%	8,000	7995	0.2105	0.2	0.21
0%	0%	0	0	0.1815	0.179	0.18

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.18** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-38A

Lateral Load Test Results

PTL- **38** Designation: **A**

Section: W6x9

Latitude: 33.651142

Longitude: -114.71181

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 13.5

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

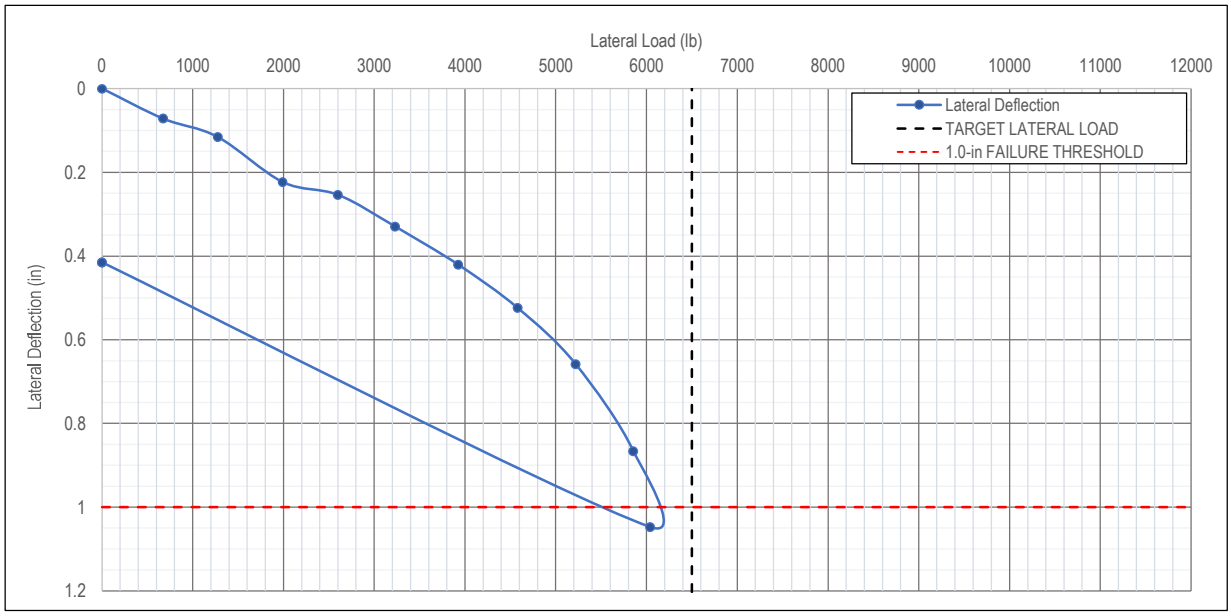
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	675	0.0715	0.071	0.07
62%	20%	1,300	1275	0.1145	0.1155	0.12
93%	30%	1,950	1990	0.2215	0.224	0.22
124%	40%	2,600	2600	0.2525	0.255	0.25
155%	50%	3,250	3230	0.327	0.332	0.33
186%	60%	3,900	3925	0.4165	0.4245	0.42
217%	70%	4,550	4580	0.516	0.531	0.52
248%	80%	5,200	5220	0.6415	0.6755	0.66
279%	90%	5,850	5855	0.811	0.9215	0.87
310%	100%	6,500	6040	0.9125	1.1825	1.05
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.3275	0.5025	0.42

Load at 1.0-in lateral deflection: **5,992** **lb**

Residual lateral deformation: **0.42** **in**



Notes:

Failure reached at 6,040 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-38B

Axial Uplift Load Test Results

PTL - **38** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.651142 - - Saturated Soil? No

Longitude: -114.71181 Target Embedment Depth (ft): 8.00 Drive Time (s): 24.76

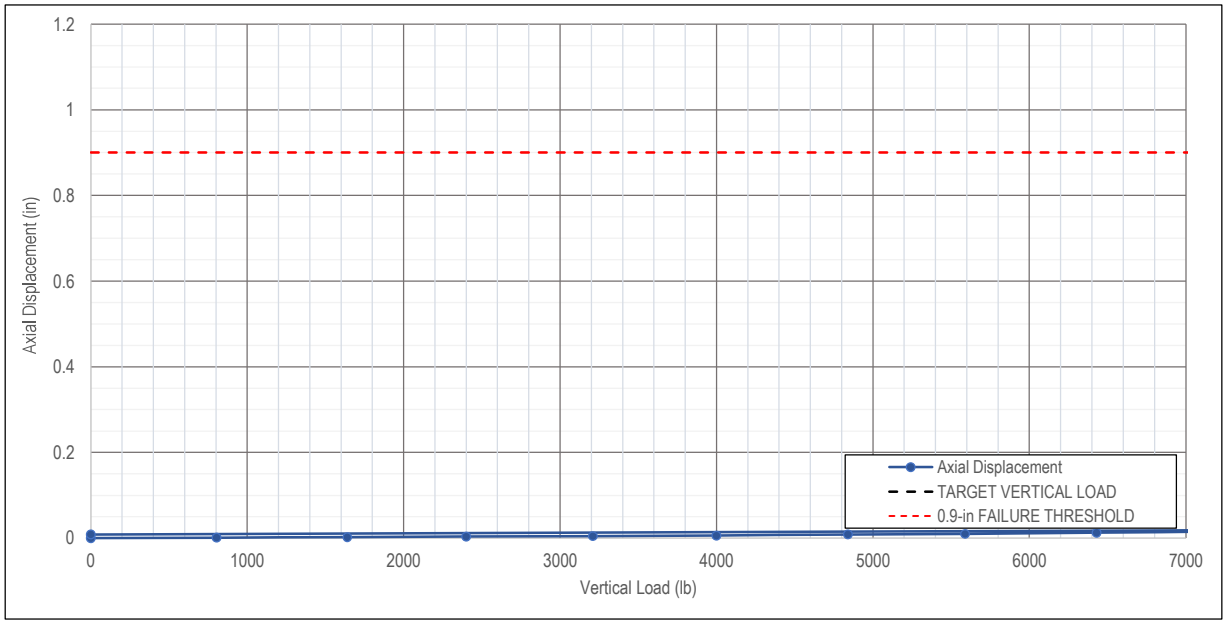
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/25/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	805	0.001	0.0015	0.00
436%	20%	1,600	1640	0.002	0.003	0.00
654%	30%	2,400	2400	0.0035	0.004	0.00
872%	40%	3,200	3210	0.005	0.0045	0.00
1090%	50%	4,000	4000	0.007	0.0055	0.01
1308%	60%	4,800	4840	0.011	0.0065	0.01
1526%	70%	5,600	5590	0.012	0.008	0.01
1744%	80%	6,400	6430	0.0155	0.01	0.01
1962%	90%	7,200	7240	0.019	0.0125	0.02
2180%	100%	8,000	7990	0.023	0.0155	0.02
0%	0%	0	0	0.0085	0.0085	0.01

Load at 0.9-in axial displacement: N/A lb **Residual axial displacement:** 0.01 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-38B

Lateral Load Test Results

PTL-	**38**	Designation:	**B**		
Section:	W6x9				
Latitude:	33.651142				
Longitude:	-114.71181				
Tested By:	ANS				
Test Date:	10/25/2025				

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 24.76

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

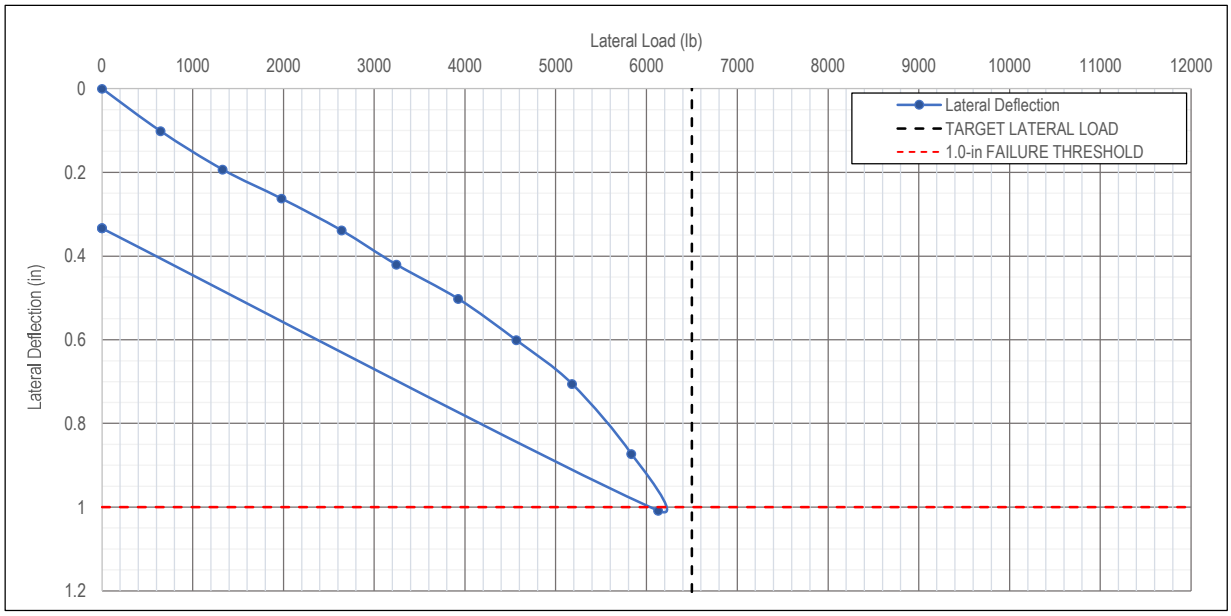
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	645	0.102	0.101	0.10
62%	20%	1,300	1330	0.1945	0.1925	0.19
93%	30%	1,950	1980	0.264	0.261	0.26
124%	40%	2,600	2640	0.34	0.3375	0.34
155%	50%	3,250	3245	0.4215	0.4195	0.42
186%	60%	3,900	3925	0.503	0.5015	0.50
217%	70%	4,550	4565	0.601	0.6005	0.60
248%	80%	5,200	5180	0.705	0.7065	0.71
279%	90%	5,850	5835	0.8735	0.872	0.87
310%	100%	6,500	6130	1.0125	1.005	1.01
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.339	0.328	0.33

Load at 1.0-in lateral deflection: **6,111** **lb**

Residual lateral deformation: **0.33** **in**



Notes:

Failure reached at 6,130 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-38D

Axial Uplift Load Test Results

PTL - **38** Designation: **D**

Section: **W6x12**

Latitude: 33.651142

Longitude: -114.71181

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 13.45

Pile Damaged? No

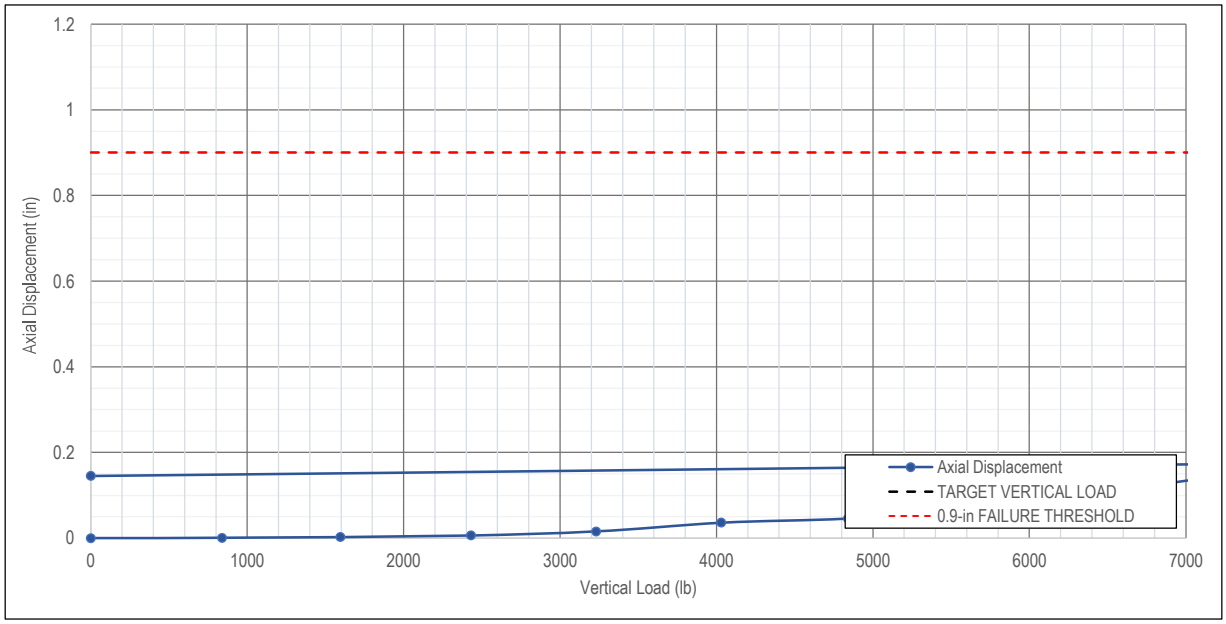
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	840	0.001	0.001	0.00
436%	20%	1,600	1595	0.003	0.0025	0.00
654%	30%	2,400	2430	0.008	0.005	0.01
872%	40%	3,200	3230	0.0175	0.014	0.02
1090%	50%	4,000	4030	0.039	0.033	0.04
1308%	60%	4,800	4840	0.0495	0.0425	0.05
1526%	70%	5,600	5615	0.074	0.0655	0.07
1744%	80%	6,400	6405	0.1085	0.0985	0.10
1962%	90%	7,200	7230	0.151	0.1395	0.15
2180%	100%	8,000	8030	0.181	0.168	0.17
0%	0%	0	0	0.1455	0.1445	0.15

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.15** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-38D

Lateral Load Test Results

PTL- **38** Designation: **D**

Section: **W6x12**

Latitude: 33.651142

Longitude: -114.71181

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 13.45

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

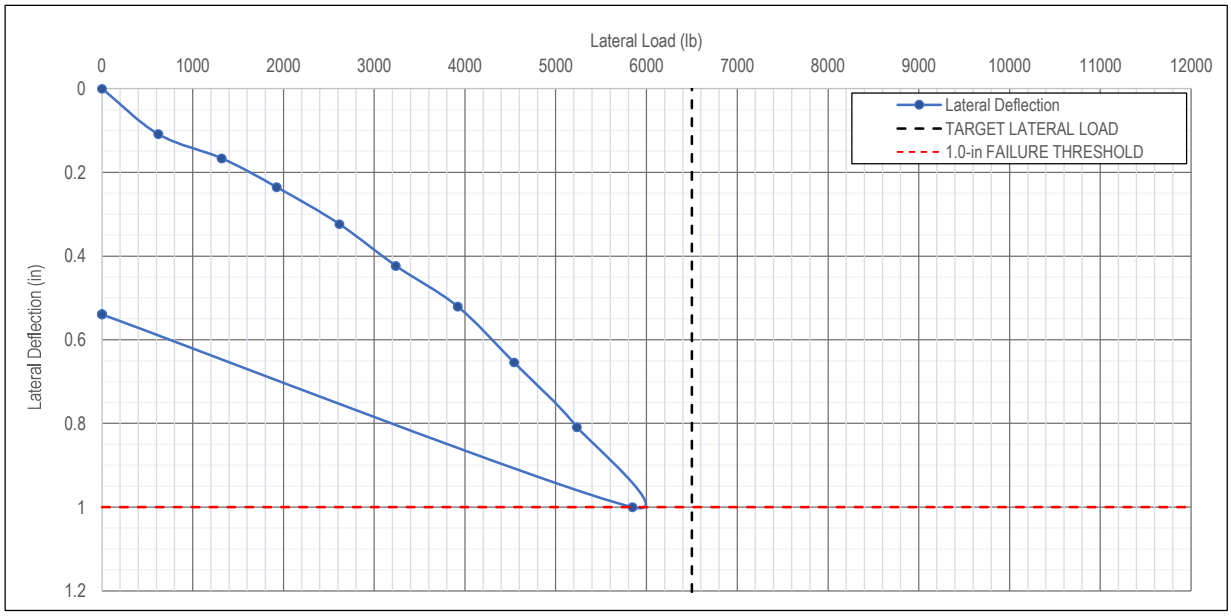
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	620	0.1045	0.1125	0.11
62%	20%	1,300	1320	0.161	0.172	0.17
93%	30%	1,950	1925	0.2285	0.2415	0.24
124%	40%	2,600	2615	0.3155	0.3315	0.32
155%	50%	3,250	3240	0.414	0.433	0.42
186%	60%	3,900	3920	0.51	0.5315	0.52
217%	70%	4,550	4540	0.6425	0.6655	0.65
248%	80%	5,200	5235	0.796	0.821	0.81
279%	90%	5,850	5845	0.9875	1.0125	1.00
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.536	0.5425	0.54

Load at 1.0-in lateral deflection: **5,845** **lb**

Residual lateral deformation: **0.54** **in**



Notes:

Failure reached at 5,845 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-39A

Axial Uplift Load Test Results

PTL - **39** Designation: **A**

Section: W6x9

Latitude: 33.651491

Longitude: -114.70736

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 4.68

Pile Damaged? No

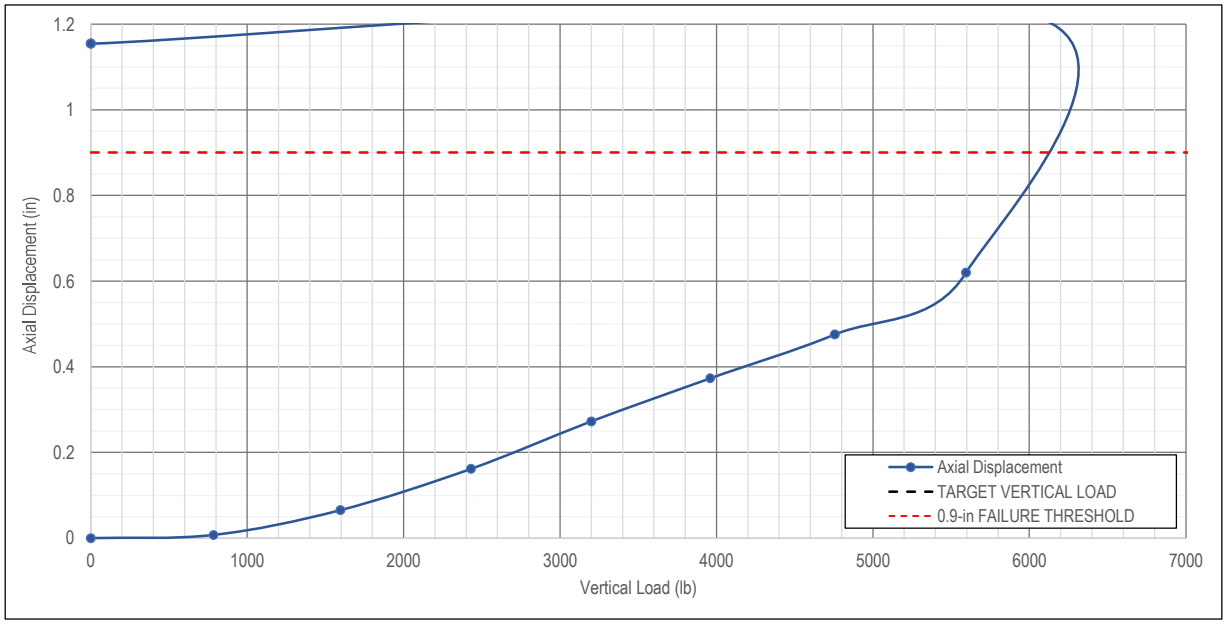
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	785	0.0065	0.0085	0.01
13594%	20%	1,600	1595	0.063	0.0675	0.07
20391%	30%	2,400	2430	0.1595	0.164	0.16
27188%	40%	3,200	3200	0.27	0.275	0.27
33985%	50%	4,000	3960	0.371	0.3755	0.37
40782%	60%	4,800	4755	0.473	0.478	0.48
47579%	70%	5,600	5595	0.618	0.622	0.62
54376%	80%	6,400	6005	1.2195	1.2215	1.22
61173%	90%	7,200				
67970%	100%	8,000				
0%	0%	0	0	1.156	1.1525	1.15

Load at 0.9-in axial displacement: **5,786** **lb**

Residual axial displacement: **1.15** **in**



Notes:

Failure reached at 6,005 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-39A

Lateral Load Test Results

PTL- **39** Designation: **A**

Section: W6x9

Latitude: 33.651491

Longitude: -114.70736

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 4.68

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

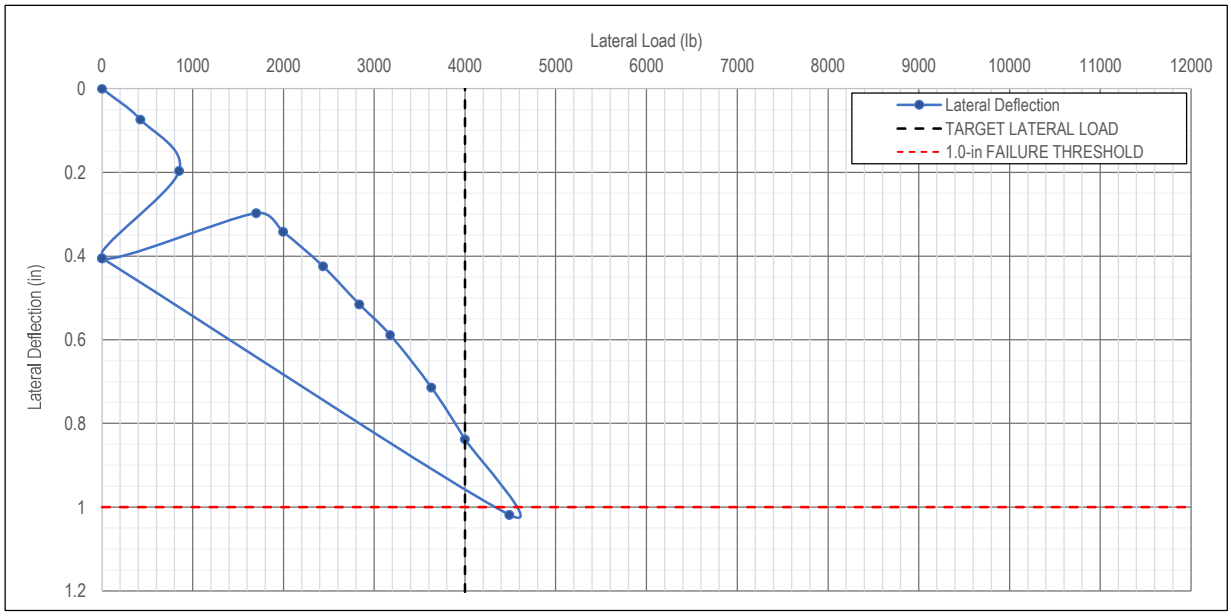
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	425	0.0765	0.071	0.07
62%	20%	800	850	0.2	0.1931	0.20
93%	30%	1,200				
124%	40%	1,600	1700	0.303	0.292	0.30
155%	50%	2,000	1995	0.3485	0.335	0.34
186%	60%	2,400	2435	0.4325	0.4155	0.42
217%	70%	2,800	2835	0.525	0.506	0.52
248%	80%	3,200	3175	0.5985	0.578	0.59
279%	90%	3,600	3630	0.7255	0.7025	0.71
310%	100%	4,000	4000	0.8505	0.825	0.84
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			4490	1.029	1.007	1.02
0%	0%	0	0	0.4045	0.406	0.41

Load at 1.0-in lateral deflection: **4,441** **lb**

Residual lateral deformation: **0.41** **in**



Notes:

Failure reached at 4,490 lbs. 1,200-lbs load step skipped.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-39B

Axial Uplift Load Test Results

PTL - **39** Designation: **B**

Section: W6x9

Latitude: 33.651491

Longitude: -114.70736

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 11.03

Pile Damaged? No

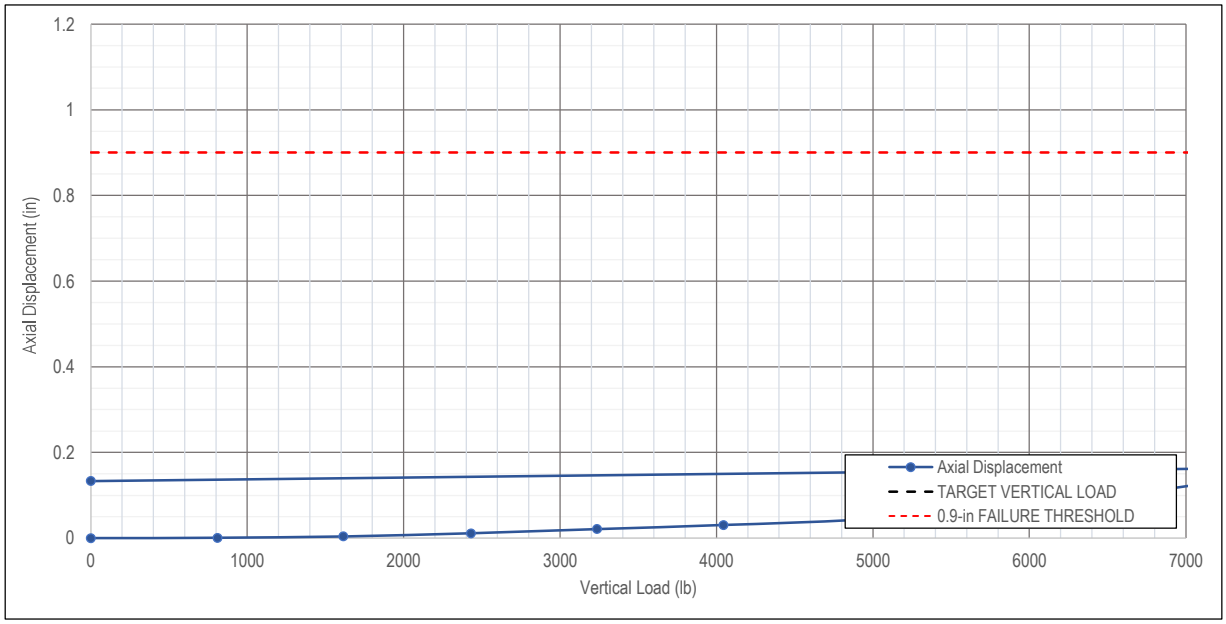
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	810	0.0005	0.0015	0.00
13594%	20%	1,600	1615	0.003	0.0055	0.00
20391%	30%	2,400	2430	0.01	0.013	0.01
27188%	40%	3,200	3235	0.0195	0.023	0.02
33985%	50%	4,000	4045	0.0285	0.0325	0.03
40782%	60%	4,800	4895	0.041	0.045	0.04
47579%	70%	5,600	5615	0.0635	0.0655	0.06
54376%	80%	6,400	6385	0.0895	0.091	0.09
61173%	90%	7,200	7245	0.1335	0.1325	0.13
67970%	100%	8,000	8035	0.164	0.163	0.16
0%	0%	0	0	0.134	0.133	0.13

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.13** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-39B

Lateral Load Test Results

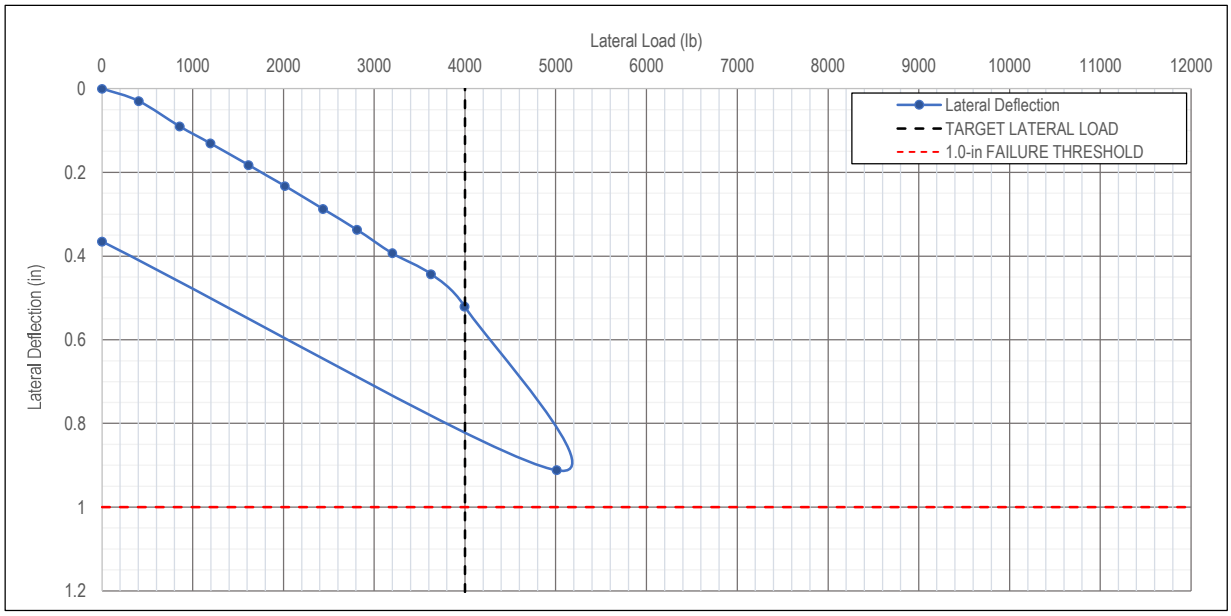
PTL-	**39**	Designation:	**B**		
Section:	W6x9				
Latitude:	33.651491				
Longitude:	-114.70736				
Tested By:	ANS				
Test Date:	10/21/2025				

Target Test Load (lb):	**4,000**	(≈ 310% Design Load)	
Installation Method:	Direct Drive (DD)	Saturated Soil?	No
	- -	Drive Time (s):	11.03
Target Embedment Depth (ft):	7.00	Pile Damaged?	No
Actual Embedment Depth (ft):	7	Gauge 1 Height (in):	6
Lateral Load Application Height (ft):	5	Gauge 2 Height (in):	6

Strong Axis Plumb. (deg): _____ Weak Axis Plumb. (deg): _____

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	405	0.0305	0.0285	0.03
62%	20%	800	855	0.091	0.089	0.09
93%	30%	1,200	1195	0.1315	0.129	0.13
124%	40%	1,600	1615	0.1825	0.1825	0.18
155%	50%	2,000	2015	0.2325	0.2325	0.23
186%	60%	2,400	2435	0.287	0.288	0.29
217%	70%	2,800	2810	0.336	0.338	0.34
248%	80%	3,200	3200	0.391	0.3955	0.39
279%	90%	3,600	3625	0.4225	0.4635	0.44
310%	100%	4,000	3995	0.509	0.5325	0.52
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5010	0.81	1.0135	0.91
0%	0%	0	0	0.3225	0.408	0.37

Load at 1.0-in lateral deflection: **N/A** **lb** **Residual lateral deformation:** **0.37** **in**



Notes:

Pile twisting at 5,010 lbs. No reliable test results available after this point.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-39C

Axial Compression Load Test Results (supplemental pile)

PTL - **39** Designation: **C**

Section: W6x9

Latitude: 33.651491

Longitude: -114.70736

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **15,000** (≈ 1180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 7.7

Pile Damaged? No

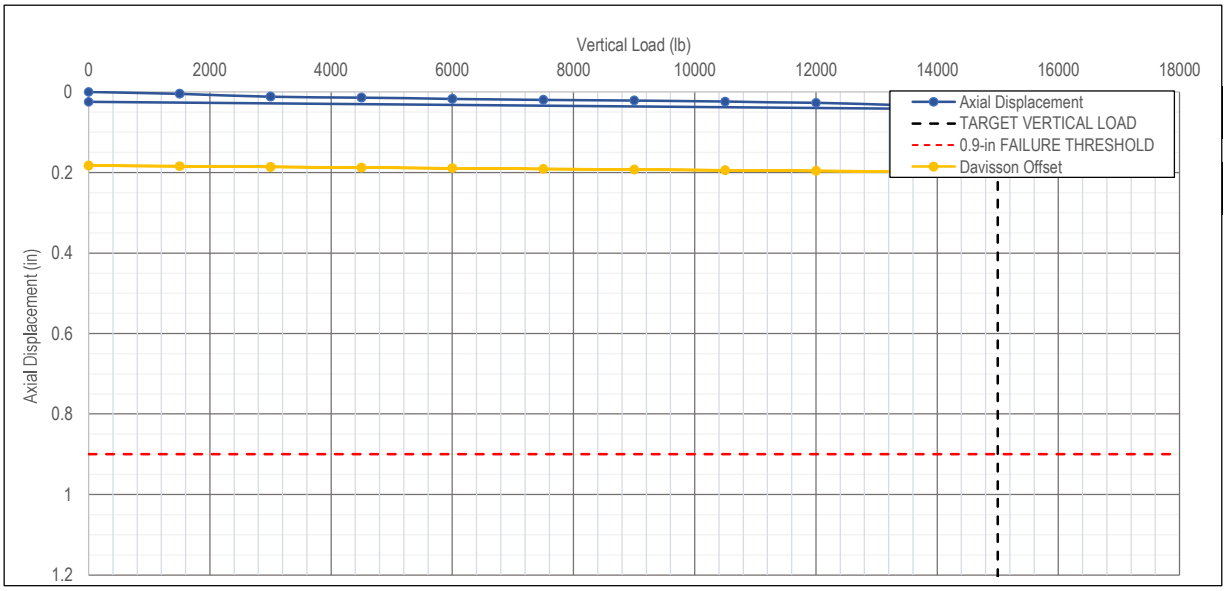
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL COMPRESSION** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
118%	10%	1,500	1500	0.0065	0.003	0.00
236%	20%	3,000	3000	0.017	0.0065	0.01
354%	30%	4,500	4500	0.018	0.01	0.01
472%	40%	6,000	6000	0.022	0.012	0.02
590%	50%	7,500	7500	0.0255	0.0135	0.02
708%	60%	9,000	9000	0.0285	0.014	0.02
826%	70%	10,500	10500	0.0325	0.0155	0.02
944%	80%	12,000	12000	0.0375	0.017	0.03
1062%	90%	13,500	13500	0.0455	0.021	0.03
1180%	100%	15,000	15000	0.0575	0.03	0.04
0%	0%	0	0	0.0245	0.0245	0.02

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.02** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-40A

Axial Uplift Load Test Results

PTL - **40** Designation: **A**

Section: W6x9

Latitude: 33.651453

Longitude: -114.69933

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 8.84

Pile Damaged? No

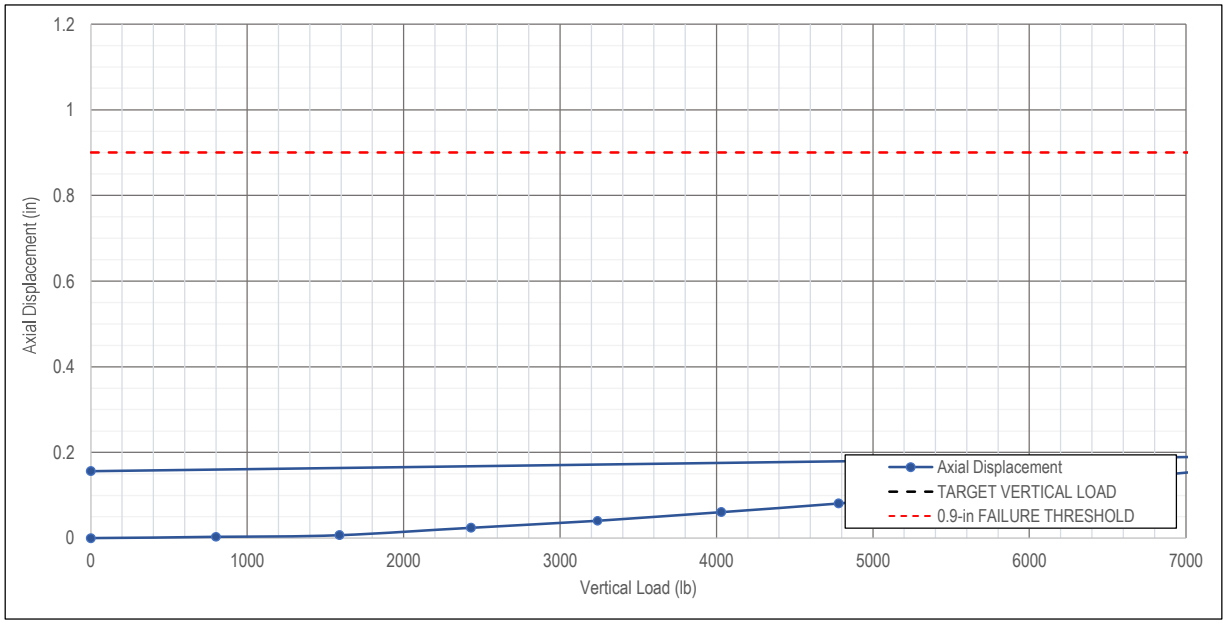
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0.0035	0.0025	0.00
436%	20%	1,600	1590	0.0085	0.0055	0.01
654%	30%	2,400	2430	0.027	0.0215	0.02
872%	40%	3,200	3240	0.045	0.0365	0.04
1090%	50%	4,000	4030	0.0655	0.0565	0.06
1308%	60%	4,800	4780	0.086	0.076	0.08
1526%	70%	5,600	5600	0.114	0.1015	0.11
1744%	80%	6,400	6420	0.141	0.127	0.13
1962%	90%	7,200	7230	0.1685	0.153	0.16
2180%	100%	8,000	8040	0.201	0.183	0.19
0%	0%	0	0	0.1585	0.1545	0.16

Load at 0.9-in axial displacement: N/A lb

Residual axial displacement: 0.16 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-40A

Lateral Load Test Results

PTL- **40** Designation: **A**

Section: W6x9

Latitude: 33.651453

Longitude: -114.69933

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 8.84

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

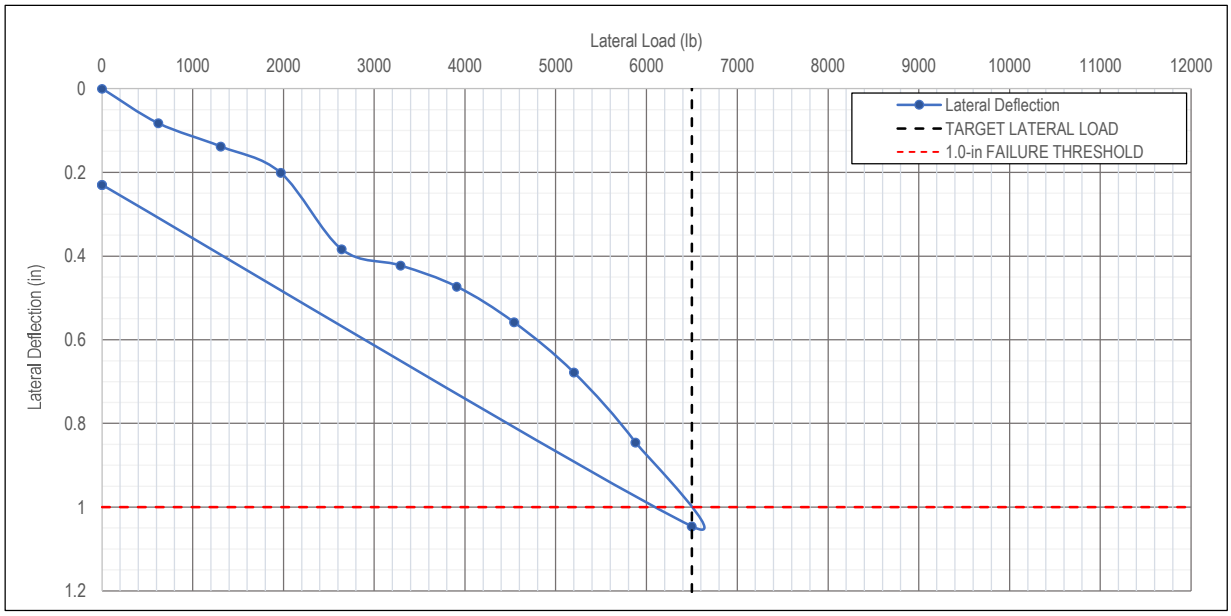
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	620	0.0785	0.086	0.08
62%	20%	1,300	1310	0.133	0.1435	0.14
93%	30%	1,950	1970	0.1935	0.2085	0.20
124%	40%	2,600	2640	0.371	0.3955	0.38
155%	50%	3,250	3290	0.41	0.435	0.42
186%	60%	3,900	3910	0.46	0.4855	0.47
217%	70%	4,550	4540	0.545	0.5715	0.56
248%	80%	5,200	5200	0.664	0.6925	0.68
279%	90%	5,850	5880	0.832	0.8585	0.85
310%	100%	6,500	6500	1.053	1.0395	1.05
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.2415	0.2175	0.23

Load at 1.0-in lateral deflection: **6,357** **lb**

Residual lateral deformation: **0.23** **in**



Notes:

Failure reached at 6,500 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-40B

Axial Uplift Load Test Results

PTL - **40** Designation: **B**

Section: W6x9

Latitude: 33.651453

Longitude: -114.69933

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 17.28

Pile Damaged? No

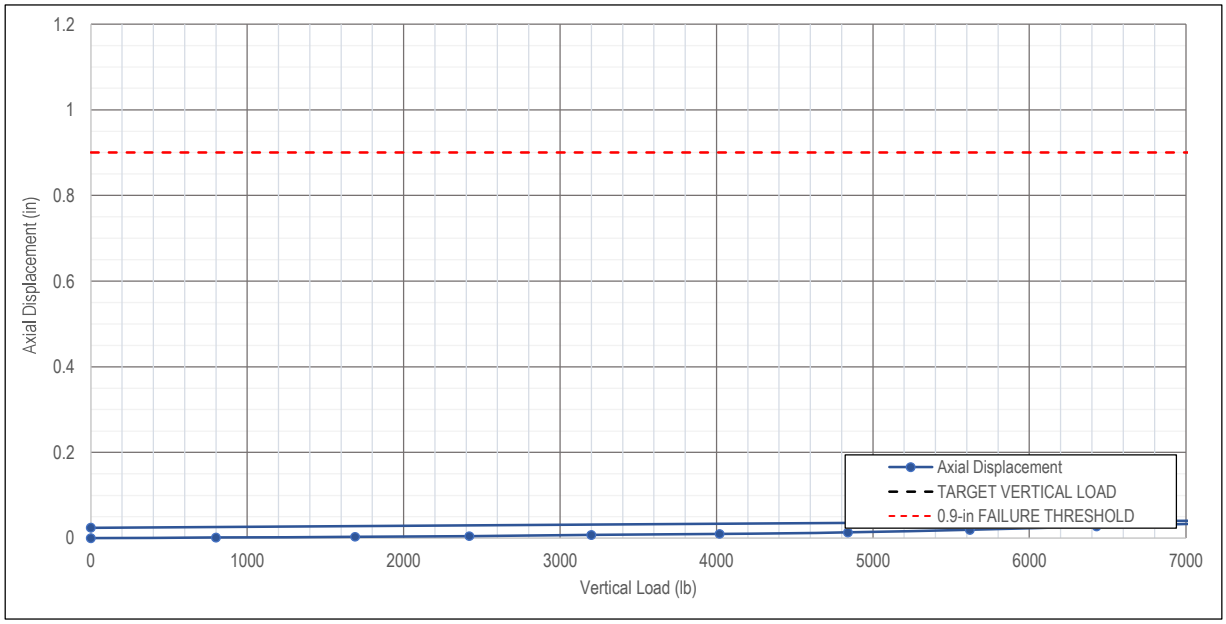
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0.001	0.002	0.00
436%	20%	1,600	1690	0.0025	0.0035	0.00
654%	30%	2,400	2420	0.0045	0.005	0.00
872%	40%	3,200	3200	0.007	0.008	0.01
1090%	50%	4,000	4020	0.01	0.01	0.01
1308%	60%	4,800	4840	0.0135	0.013	0.01
1526%	70%	5,600	5620	0.0195	0.0195	0.02
1744%	80%	6,400	6430	0.0275	0.028	0.03
1962%	90%	7,200	7240	0.035	0.036	0.04
2180%	100%	8,000	7990	0.0425	0.043	0.04
0%	0%	0	0	0.027	0.0215	0.02

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.02** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-40B

Lateral Load Test Results

PTL- **40** Designation: **B**

Section: W6x9

Latitude: 33.651453

Longitude: -114.69933

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 17.28

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

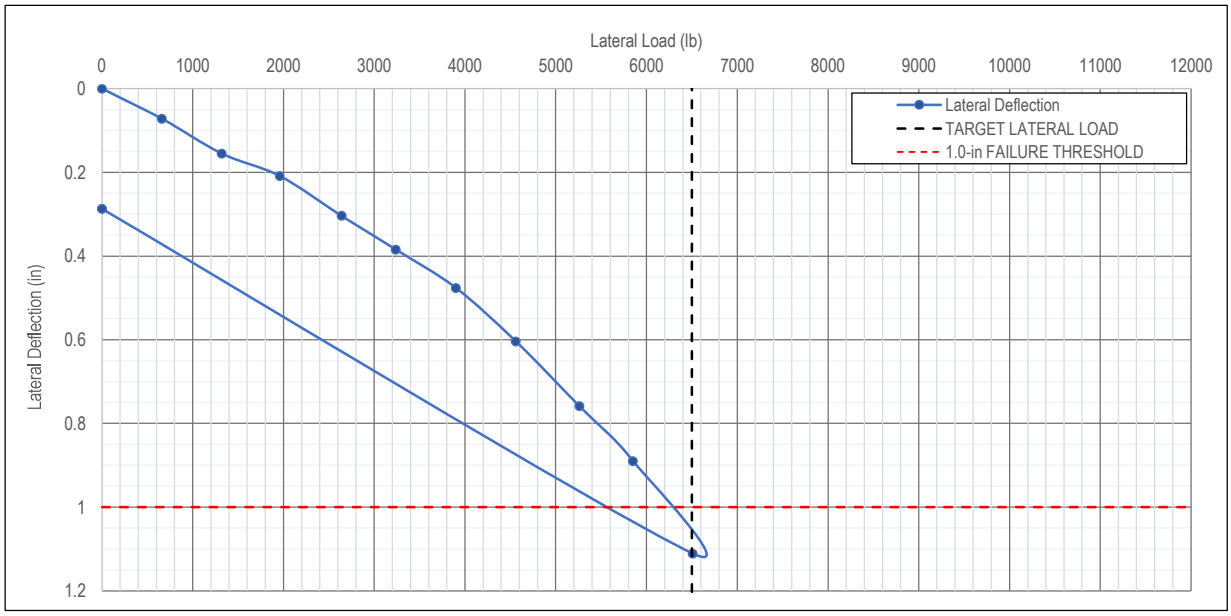
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	660	0.075	0.0685	0.07
62%	20%	1,300	1320	0.149	0.1605	0.15
93%	30%	1,950	1960	0.2015	0.2155	0.21
124%	40%	2,600	2640	0.2925	0.315	0.30
155%	50%	3,250	3240	0.3745	0.3945	0.38
186%	60%	3,900	3900	0.464	0.4885	0.48
217%	70%	4,550	4560	0.59	0.6175	0.60
248%	80%	5,200	5260	0.74	0.7765	0.76
279%	90%	5,850	5850	0.8695	0.9105	0.89
310%	100%	6,500	6510	1.105	1.117	1.11
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.2915	0.2825	0.29

Load at 1.0-in lateral deflection: **6,179** **lb**

Residual lateral deformation: **0.29** **in**



Notes:

Failure reached at 6,510 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-41A

Axial Uplift Load Test Results

PTL - **41** Designation: **A**

Section: W6x9

Latitude: 33.650057

Longitude: -114.6972

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 8.51

Pile Damaged? No

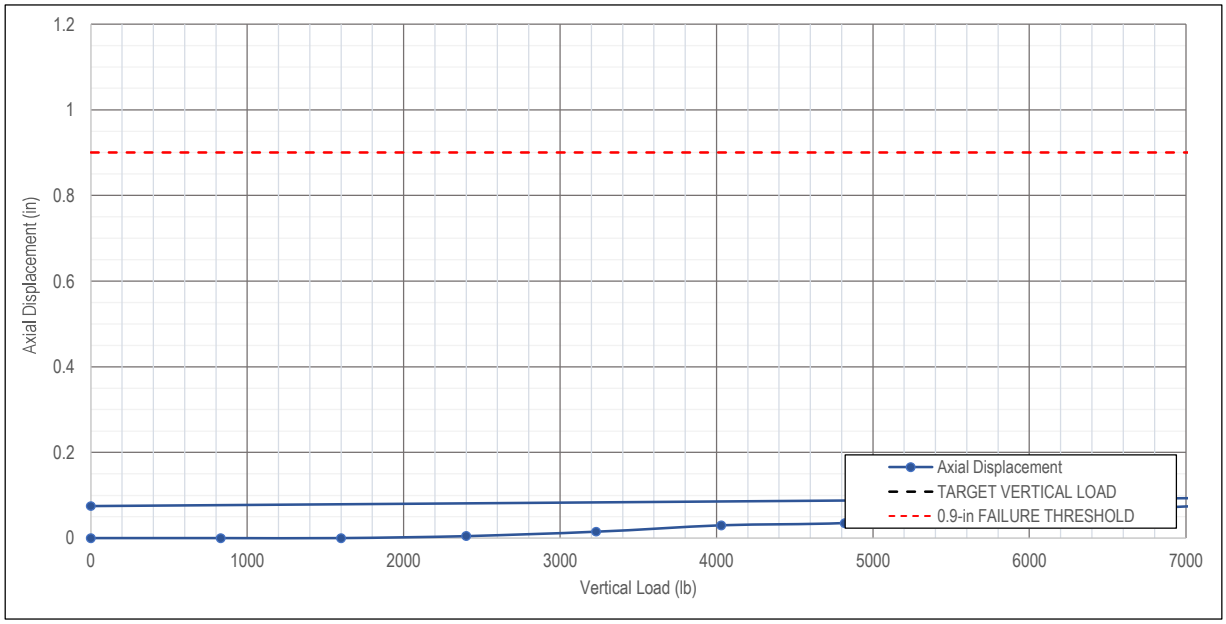
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	830	0	0	0.00
13594%	20%	1,600	1600	0	0	0.00
20391%	30%	2,400	2400	0.005	0.005	0.01
27188%	40%	3,200	3230	0.015	0.015	0.02
33985%	50%	4,000	4030	0.03	0.03	0.03
40782%	60%	4,800	4820	0.035	0.035	0.04
47579%	70%	5,600	5640	0.055	0.05	0.05
54376%	80%	6,400	6430	0.065	0.065	0.07
61173%	90%	7,200	7210	0.08	0.075	0.08
67970%	100%	8,000	8040	0.095	0.095	0.10
0%	0%	0	0	0.075	0.075	0.08

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.08** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-41A

Lateral Load Test Results

PTL- **41** Designation: **A**

Section: W6x9

Latitude: 33.650057

Longitude: -114.6972

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 8.51

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

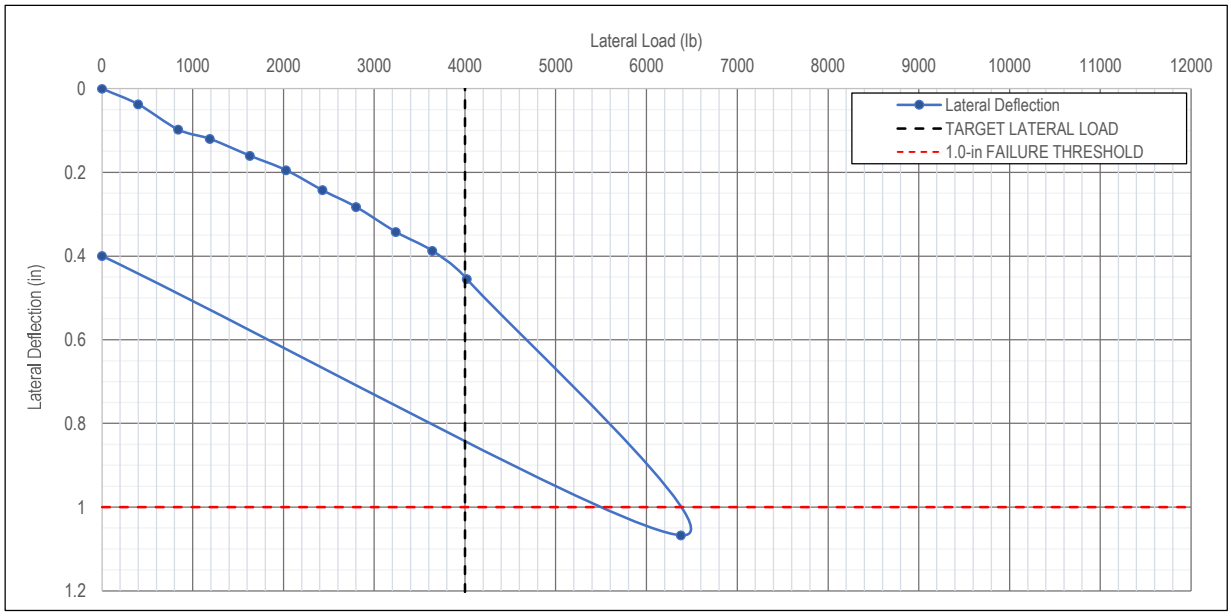
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.04	0.035	0.04
62%	20%	800	840	0.1	0.095	0.10
93%	30%	1,200	1190	0.125	0.115	0.12
124%	40%	1,600	1630	0.165	0.155	0.16
155%	50%	2,000	2030	0.2	0.19	0.20
186%	60%	2,400	2430	0.25	0.235	0.24
217%	70%	2,800	2800	0.29	0.275	0.28
248%	80%	3,200	3240	0.35	0.335	0.34
279%	90%	3,600	3640	0.395	0.38	0.39
310%	100%	4,000	4020	0.465	0.445	0.46
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6380	1.015	1.12	1.07
0%	0%	0	0	0.37	0.43	0.40

Load at 1.0-in lateral deflection: **6,120** **lb**

Residual lateral deformation: **0.40** **in**



Notes:

Failure reached at 6,380 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-41B

Axial Uplift Load Test Results

PTL - **41** Designation: **B**

Section: W6x9

Latitude: 33.650057

Longitude: -114.6972

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 10.7

Pile Damaged? No

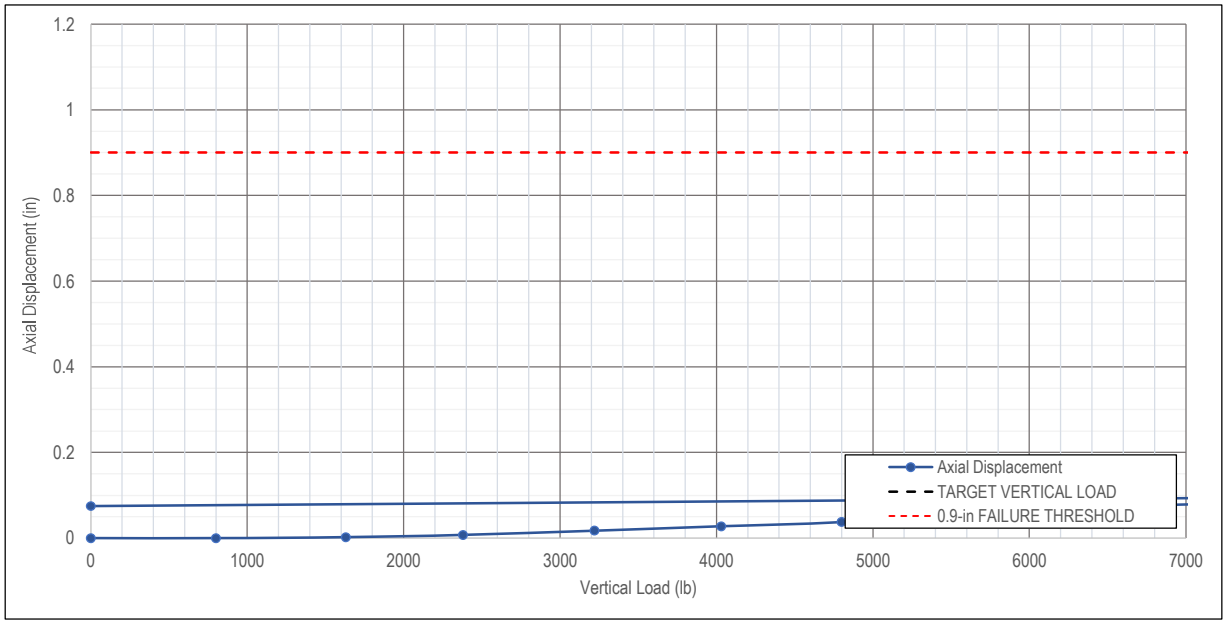
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	800	0	0	0.00
13594%	20%	1,600	1630	0	0.005	0.00
20391%	30%	2,400	2380	0.005	0.01	0.01
27188%	40%	3,200	3220	0.01	0.025	0.02
33985%	50%	4,000	4030	0.02	0.035	0.03
40782%	60%	4,800	4800	0.03	0.045	0.04
47579%	70%	5,600	5630	0.05	0.07	0.06
54376%	80%	6,400	5400	0.06	0.075	0.07
61173%	90%	7,200	7180	0.07	0.09	0.08
67970%	100%	8,000	8000	0.085	0.105	0.10
0%	0%	0	0	0.075	0.075	0.08

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.08** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-41B

Lateral Load Test Results

PTL- **41** Designation: **B**

Section: W6x9

Latitude: 33.650057

Longitude: -114.6972

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 10.7

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

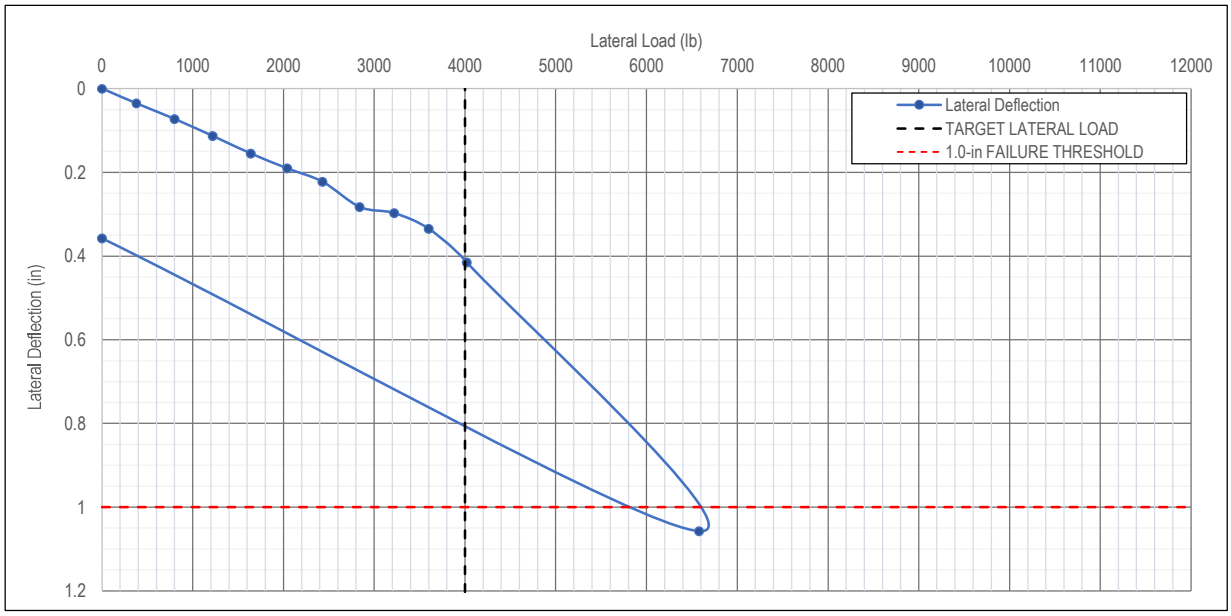
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	380	0.035	0.035	0.04
62%	20%	800	800	0.075	0.07	0.07
93%	30%	1,200	1220	0.115	0.11	0.11
124%	40%	1,600	1640	0.16	0.15	0.16
155%	50%	2,000	2040	0.195	0.185	0.19
186%	60%	2,400	2430	0.23	0.215	0.22
217%	70%	2,800	2840	0.285	0.28	0.28
248%	80%	3,200	3220	0.305	0.29	0.30
279%	90%	3,600	3600	0.345	0.325	0.34
310%	100%	4,000	4020	0.425	0.405	0.42
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6580	1.025	1.09	1.06
0%	0%	0	0	0.335	0.38	0.36

Load at 1.0-in lateral deflection: **6,351** **lb**

Residual lateral deformation: **0.36** **in**



Notes:

Failure reached at 6,580 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-42A

Axial Uplift Load Test Results

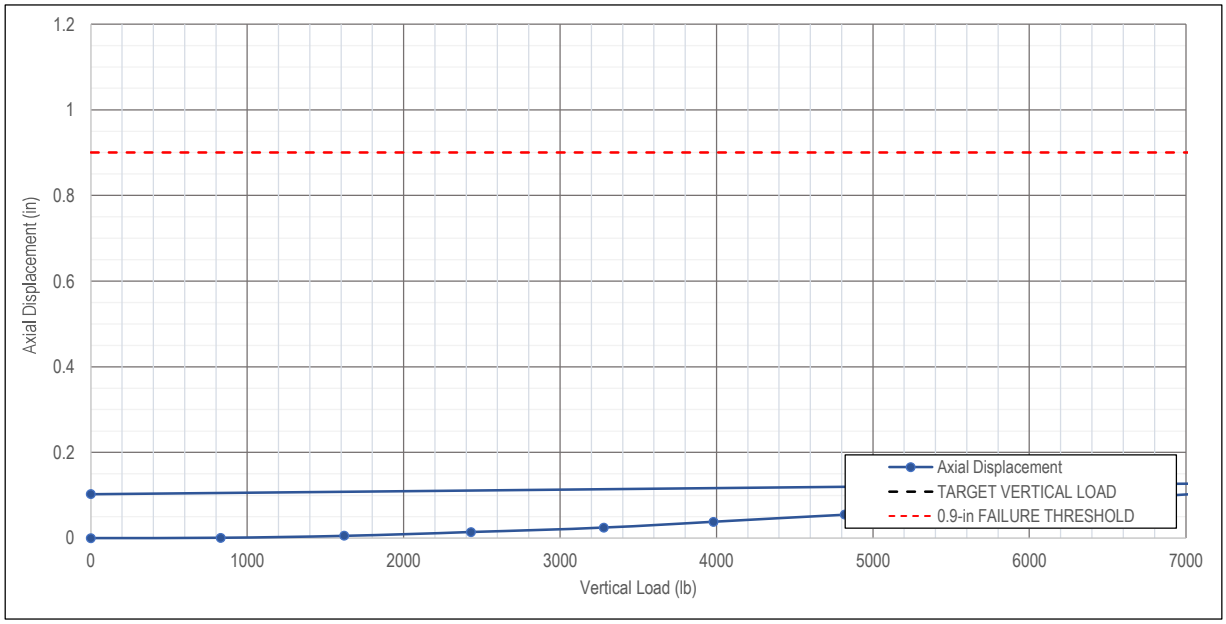
PTL - **42**
Designation: **A**
Target Test Load (lb): **8,000** (≈ 2180% Design Load)
Section: W6x9
Installation Method: Direct Drive (DD)
Latitude: 33.651802
- -
Saturated Soil? No
Longitude: -114.69503
Target Embedment Depth (ft): 6.00
Drive Time (s): 10.2
Tested By: ANS
Actual Embedment Depth (ft): 6
Pile Damaged? No
Test Date: 10/23/2025

Strong Axis Plumb. (deg):
Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	830	0.001	0.001	0.00
436%	20%	1,600	1620	0.0055	0.0055	0.01
654%	30%	2,400	2430	0.014	0.014	0.01
872%	40%	3,200	3280	0.024	0.025	0.02
1090%	50%	4,000	3980	0.037	0.0395	0.04
1308%	60%	4,800	4820	0.0525	0.057	0.05
1526%	70%	5,600	5630	0.0675	0.0755	0.07
1744%	80%	6,400	6440	0.085	0.0955	0.09
1962%	90%	7,200	7200	0.1	0.113	0.11
2180%	100%	8,000	8040	0.121	0.1375	0.13
0%	0%	0	0	0.101	0.104	0.10

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.10** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-42A

Lateral Load Test Results

PTL- **42** Designation: **A**

Section: W6x9

Latitude: 33.651802

Longitude: -114.69503

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 10.2

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

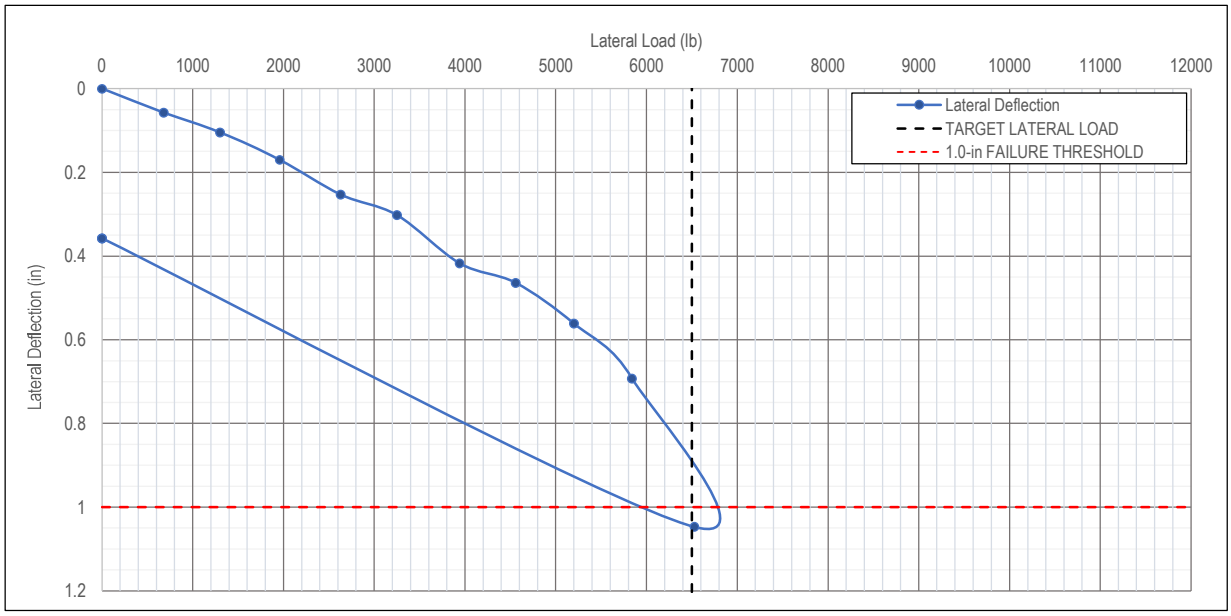
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	680	0.06	0.0535	0.06
62%	20%	1,300	1300	0.1095	0.099	0.10
93%	30%	1,950	1960	0.178	0.162	0.17
124%	40%	2,600	2630	0.2635	0.2425	0.25
155%	50%	3,250	3250	0.314	0.29	0.30
186%	60%	3,900	3940	0.432	0.402	0.42
217%	70%	4,550	4560	0.4805	0.448	0.46
248%	80%	5,200	5200	0.5805	0.5415	0.56
279%	90%	5,850	5840	0.72	0.6655	0.69
310%	100%	6,500	6530	1.123	0.9715	1.05
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.3905	0.324	0.36

Load at 1.0-in lateral deflection: **6,438** **lb**

Residual lateral deformation: **0.36** **in**



Notes:

Failure reached at 6,530 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-42B

Axial Uplift Load Test Results

PTL - **42** Designation: **B**

Section: W6x9

Latitude: 33.651802

Longitude: -114.69503

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 13.02

Pile Damaged? No

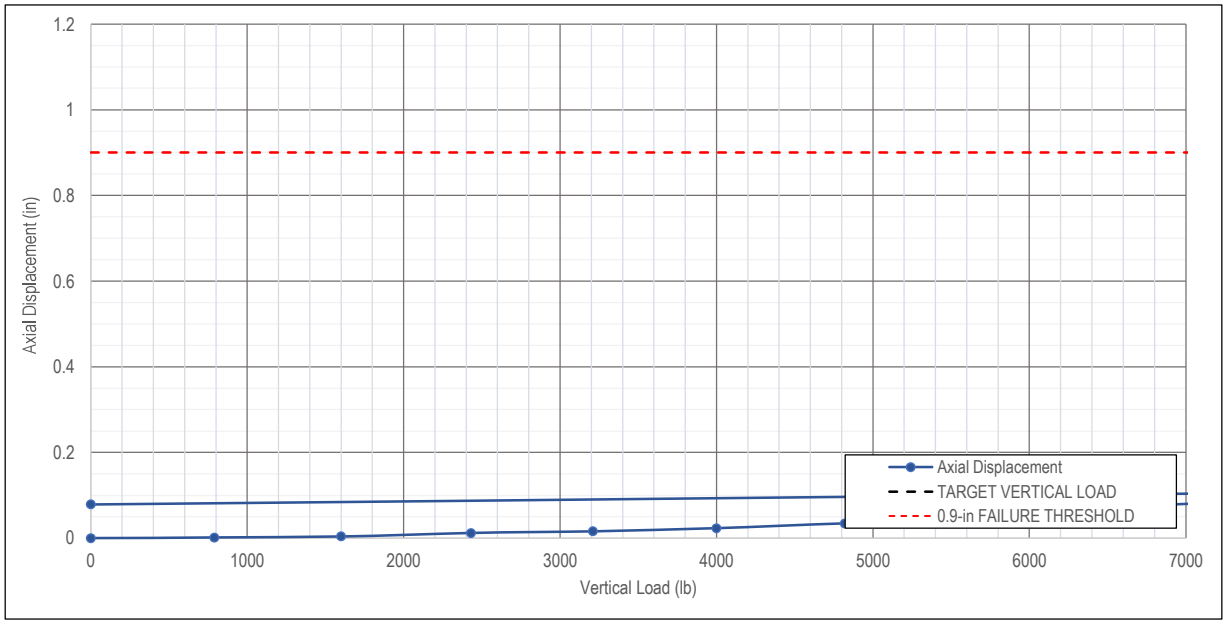
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	790	0.001	0.002	0.00
436%	20%	1,600	1600	0.003	0.005	0.00
654%	30%	2,400	2430	0.01	0.014	0.01
872%	40%	3,200	3210	0.014	0.018	0.02
1090%	50%	4,000	4000	0.021	0.026	0.02
1308%	60%	4,800	4820	0.031	0.038	0.03
1526%	70%	5,600	5610	0.0385	0.047	0.04
1744%	80%	6,400	6430	0.064	0.0775	0.07
1962%	90%	7,200	7230	0.076	0.091	0.08
2180%	100%	8,000	8060	0.097	0.1155	0.11
0%	0%	0	0	0.0775	0.08	0.08

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.08** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-42B

Lateral Load Test Results

PTL- **42** Designation: **B**

Section: W6x9

Latitude: 33.651802

Longitude: -114.69503

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 13.02

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

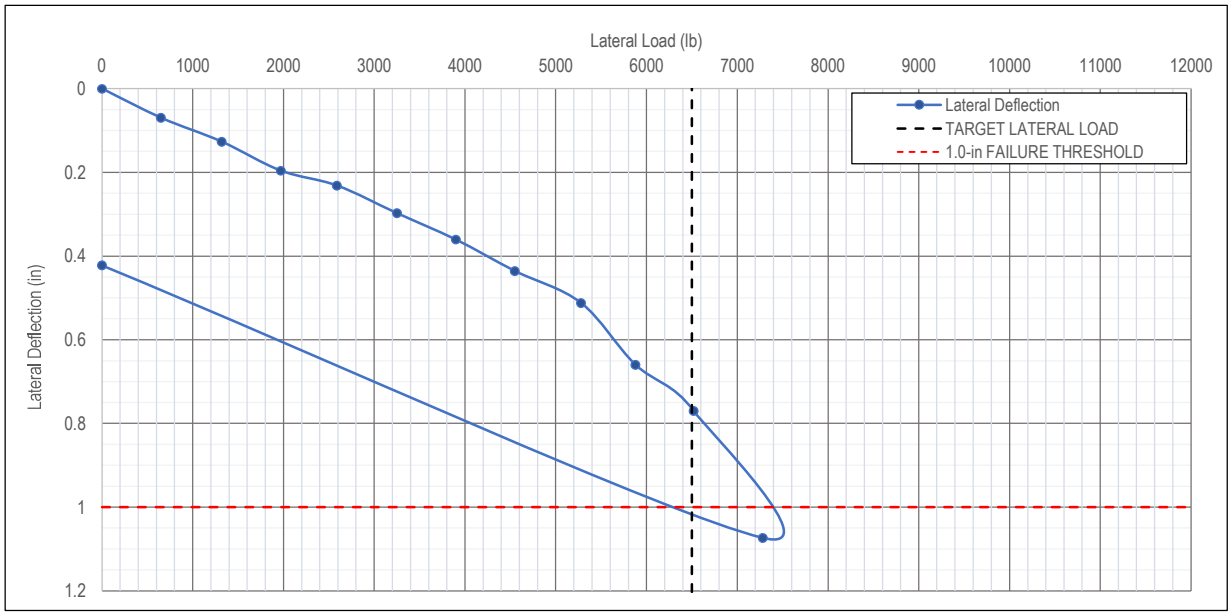
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.0665	0.072	0.07
62%	20%	1,300	1320	0.124	0.129	0.13
93%	30%	1,950	1970	0.1945	0.197	0.20
124%	40%	2,600	2590	0.2305	0.2325	0.23
155%	50%	3,250	3250	0.2975	0.297	0.30
186%	60%	3,900	3900	0.362	0.359	0.36
217%	70%	4,550	4550	0.4395	0.432	0.44
248%	80%	5,200	5280	0.517	0.507	0.51
279%	90%	5,850	5880	0.6655	0.654	0.66
310%	100%	6,500	6520	0.775	0.7655	0.77
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7280	1.038	1.109	1.07
0%	0%	0	0	0.397	0.4475	0.42

Load at 1.0-in lateral deflection: **7,096** **lb**

Residual lateral deformation: **0.42** **in**



Notes:

Failure reached at 7,280 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-43A

Axial Uplift Load Test Results

PTL - **43** Designation: **A**

Section: W6x9

Latitude: 33.650009

Longitude: -114.69111

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 10.87

Pile Damaged? No

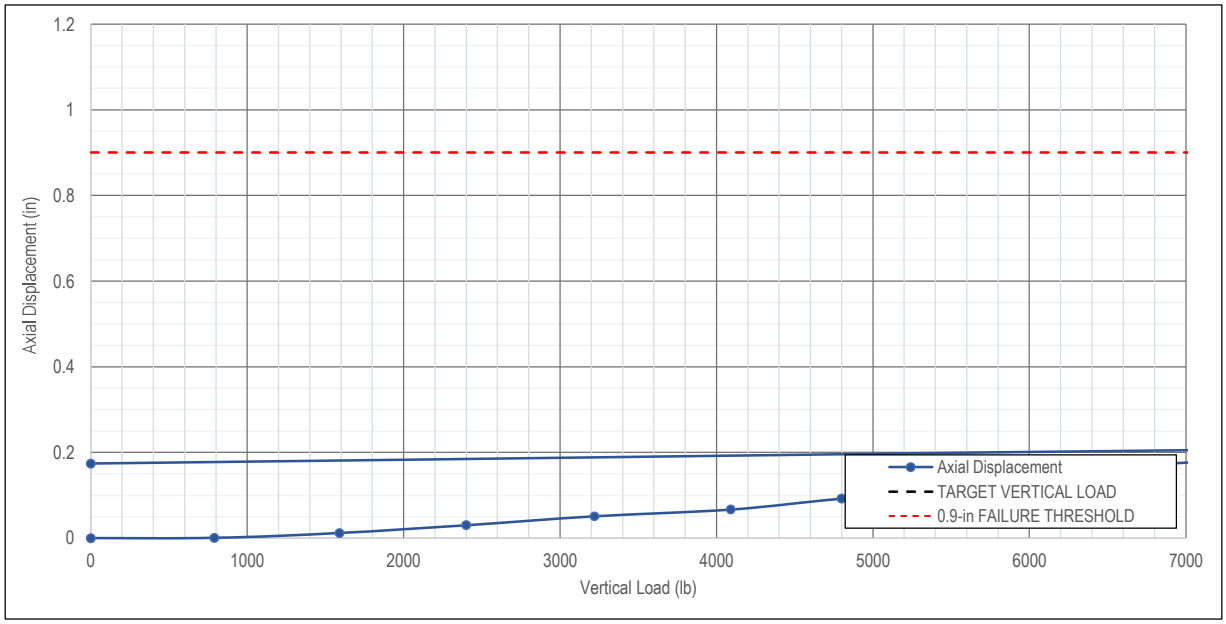
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	790	0.0005	0.001	0.00
13594%	20%	1,600	1590	0.012	0.012	0.01
20391%	30%	2,400	2400	0.03	0.0305	0.03
27188%	40%	3,200	3220	0.051	0.051	0.05
33985%	50%	4,000	4090	0.067	0.0665	0.07
40782%	60%	4,800	4800	0.0925	0.0915	0.09
47579%	70%	5,600	5580	0.112	0.112	0.11
54376%	80%	6,400	6490	0.1625	0.1625	0.16
61173%	90%	7,200	7200	0.181	0.181	0.18
67970%	100%	8,000	8090	0.215	0.2015	0.21
0%	0%	0	0	0.175	0.173	0.17

Load at 0.9-in axial displacement: N/A lb

Residual axial displacement: 0.17 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-43A

Lateral Load Test Results

PTL- **43** Designation: **A**

Section: W6x9

Latitude: 33.650009

Longitude: -114.69111

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 10.87

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

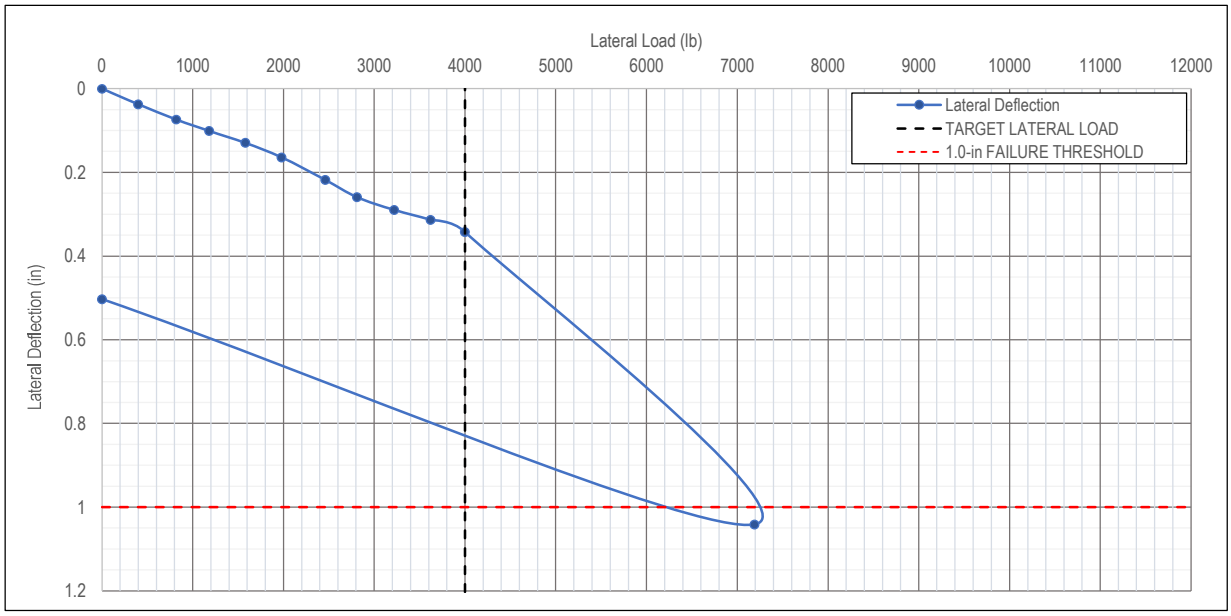
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.038	0.0365	0.04
62%	20%	800	820	0.0755	0.072	0.07
93%	30%	1,200	1180	0.1035	0.0985	0.10
124%	40%	1,600	1580	0.1325	0.1265	0.13
155%	50%	2,000	1980	0.168	0.16	0.16
186%	60%	2,400	2460	0.223	0.213	0.22
217%	70%	2,800	2810	0.2655	0.253	0.26
248%	80%	3,200	3220	0.297	0.2825	0.29
279%	90%	3,600	3620	0.3215	0.305	0.31
310%	100%	4,000	4000	0.351	0.334	0.34
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7190	1.012	1.071	1.04
0%	0%	0	0	0.4755	0.5309	0.50

Load at 1.0-in lateral deflection: **7,001** **lb**

Residual lateral deformation: **0.50** **in**



Notes:

Failure reached at 7,190 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-43B

Axial Uplift Load Test Results

PTL - **43** Designation: **B**

Section: W6x9

Latitude: 33.650009

Longitude: -114.69111

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 16.84

Pile Damaged? No

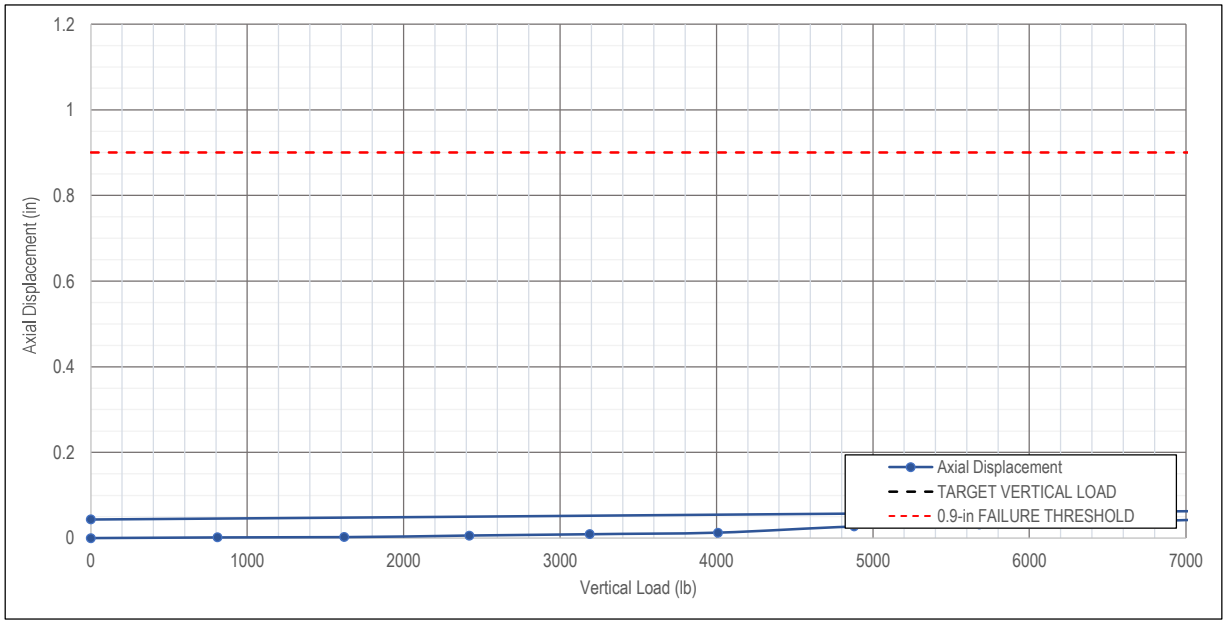
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	810	0.001	0.0025	0.00
13594%	20%	1,600	1620	0.002	0.0035	0.00
20391%	30%	2,400	2420	0.006	0.006	0.01
27188%	40%	3,200	3190	0.0095	0.009	0.01
33985%	50%	4,000	4010	0.013	0.0125	0.01
40782%	60%	4,800	4880	0.028	0.0265	0.03
47579%	70%	5,600	5680	0.0325	0.0315	0.03
54376%	80%	6,400	6420	0.038	0.037	0.04
61173%	90%	7,200	7210	0.0455	0.044	0.04
67970%	100%	8,000	8030	0.0645	0.064	0.06
0%	0%	0	0	0.044	0.0435	0.04

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.04** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-43B

Lateral Load Test Results

PTL- **43** Designation: **B**

Section: W6x9

Latitude: 33.650009

Longitude: -114.69111

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 16.84

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

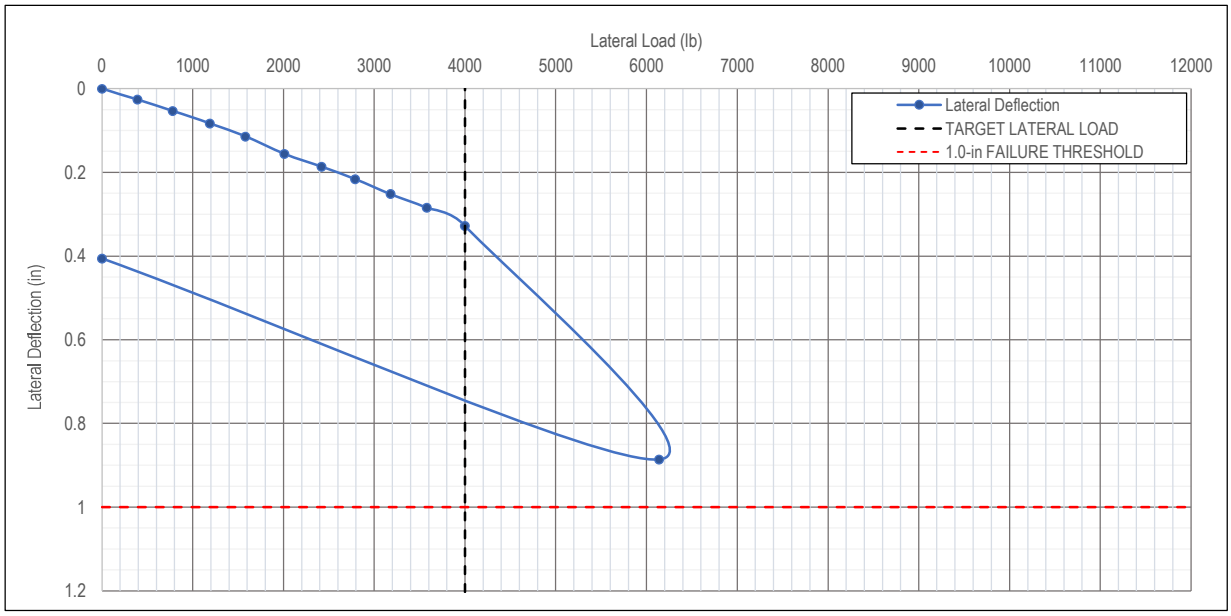
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	390	0.0285	0.0235	0.03
62%	20%	800	780	0.057	0.049	0.05
93%	30%	1,200	1190	0.088	0.0775	0.08
124%	40%	1,600	1580	0.12	0.108	0.11
155%	50%	2,000	2010	0.164	0.1475	0.16
186%	60%	2,400	2420	0.195	0.177	0.19
217%	70%	2,800	2790	0.2255	0.2065	0.22
248%	80%	3,200	3180	0.262	0.2415	0.25
279%	90%	3,600	3580	0.2955	0.274	0.28
310%	100%	4,000	4000	0.3375	0.3175	0.33
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6140	0.8765	0.8955	0.89
0%	0%	0	0	0.319	0.4925	0.41

Load at 1.0-in lateral deflection: N/A lb

Residual lateral deformation: 0.41 in



Notes:

Pile bending at 6,140 lbs. No reliable test results available after this point.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-44A

Axial Uplift Load Test Results

PTL - **44** Designation: **A**

Section: W6x9

Latitude: 33.647911

Longitude: -114.68882

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 18.28

Pile Damaged? No

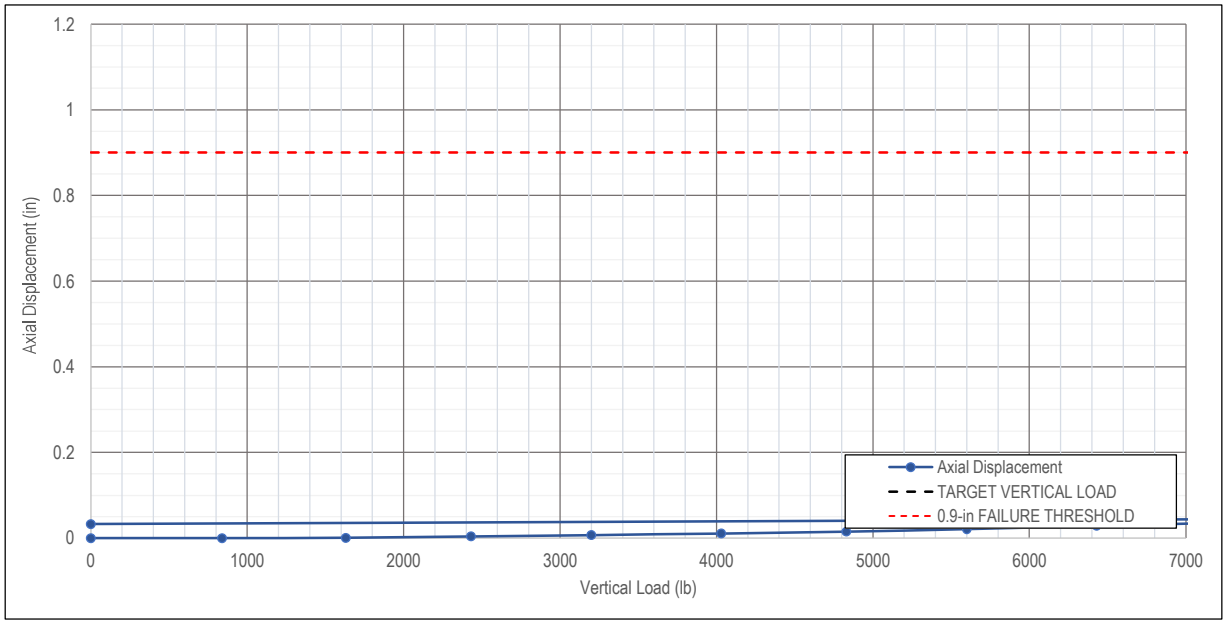
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	840	0	0	0.00
436%	20%	1,600	1630	0	0.002	0.00
654%	30%	2,400	2430	0.004	0.004	0.00
872%	40%	3,200	3200	0.006	0.008	0.01
1090%	50%	4,000	4030	0.01	0.012	0.01
1308%	60%	4,800	4830	0.014	0.016	0.02
1526%	70%	5,600	5600	0.02	0.022	0.02
1744%	80%	6,400	6430	0.028	0.03	0.03
1962%	90%	7,200	7240	0.036	0.036	0.04
2180%	100%	8,000	8040	0.044	0.046	0.05
0%	0%	0	0	0.034	0.032	0.03

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-44A

Lateral Load Test Results

PTL- **44** Designation: **A**

Section: W6x9

Latitude: 33.647911

Longitude: -114.68882

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 18.28

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

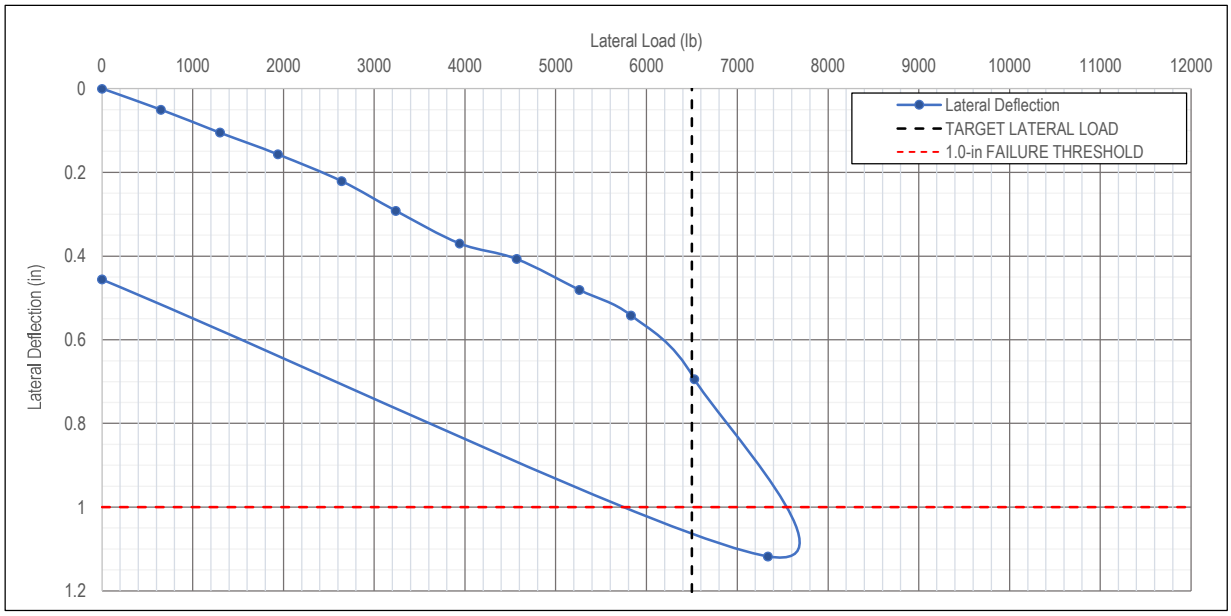
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.052	0.048	0.05
62%	20%	1,300	1300	0.11	0.1	0.11
93%	30%	1,950	1940	0.164	0.15	0.16
124%	40%	2,600	2640	0.23	0.212	0.22
155%	50%	3,250	3240	0.304	0.28	0.29
186%	60%	3,900	3940	0.384	0.356	0.37
217%	70%	4,550	4570	0.422	0.392	0.41
248%	80%	5,200	5260	0.496	0.466	0.48
279%	90%	5,850	5830	0.554	0.53	0.54
310%	100%	6,500	6530	0.696	0.692	0.69
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7340	1.168	1.068	1.12
0%	0%	0	0	0.258	0.654	0.46

Load at 1.0-in lateral deflection: **7,115** **lb**

Residual lateral deformation: **0.46** **in**



Notes:

Failure reached at 7,340 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-44B

Axial Uplift Load Test Results

PTL - **44** Designation: **B**

Section: W6x9

Latitude: 33.647911

Longitude: -114.68882

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 26.73

Pile Damaged? No

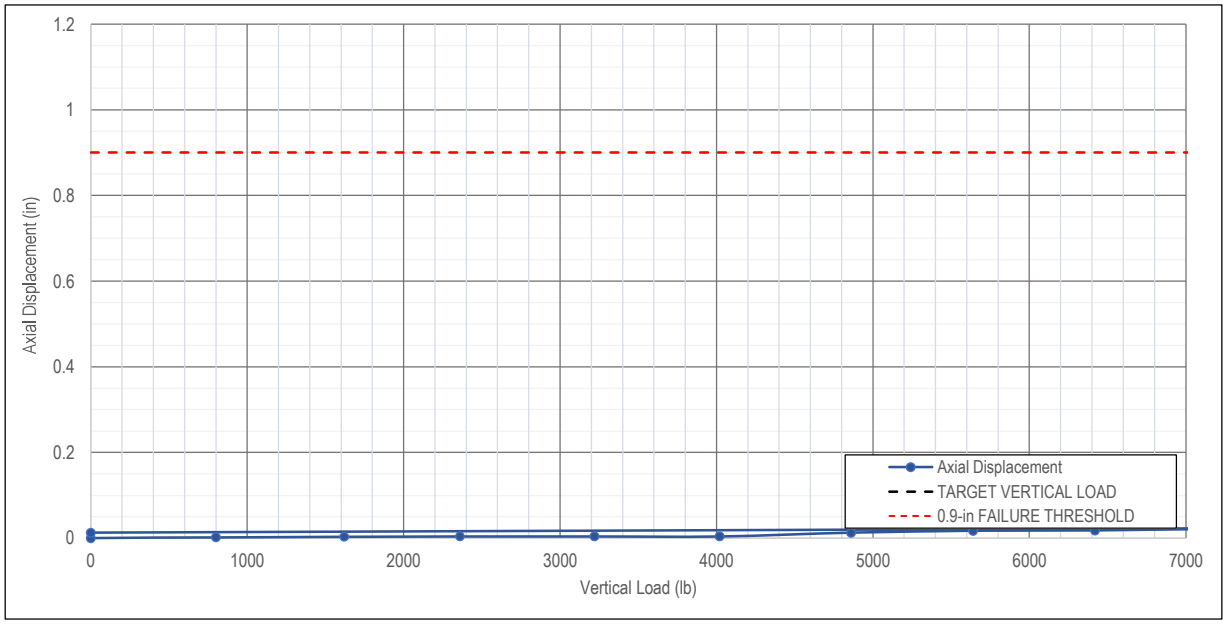
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0.002	0.002	0.00
436%	20%	1,600	1620	0.002	0.004	0.00
654%	30%	2,400	2360	0.004	0.004	0.00
872%	40%	3,200	3220	0.004	0.004	0.00
1090%	50%	4,000	4020	0.004	0.004	0.00
1308%	60%	4,800	4860	0.014	0.012	0.01
1526%	70%	5,600	5640	0.018	0.016	0.02
1744%	80%	6,400	6420	0.02	0.016	0.02
1962%	90%	7,200	7250	0.024	0.02	0.02
2180%	100%	8,000	8030	0.026	0.022	0.02
0%	0%	0	0	0.014	0.012	0.01

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-44B

Lateral Load Test Results

PTL- **44** Designation: **B**

Section: W6x9

Latitude: 33.647911

Longitude: -114.68882

Tested By: ANS

Test Date: 10/24/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 26.73

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

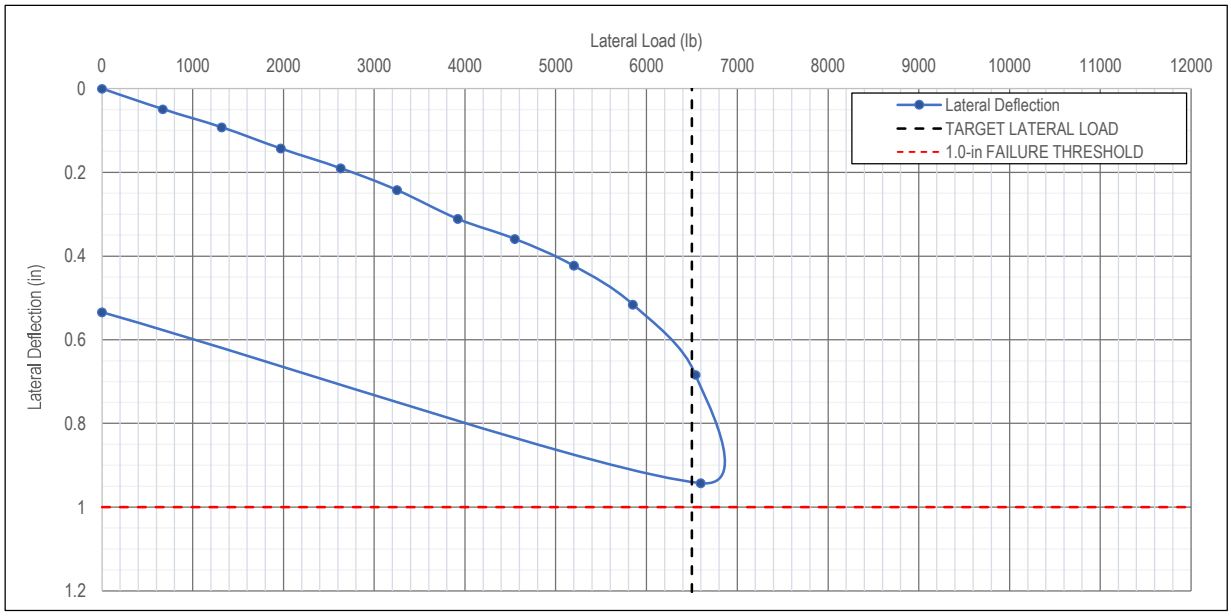
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	670	0.042	0.056	0.05
62%	20%	1,300	1320	0.088	0.096	0.09
93%	30%	1,950	1970	0.14	0.146	0.14
124%	40%	2,600	2630	0.188	0.192	0.19
155%	50%	3,250	3250	0.242	0.242	0.24
186%	60%	3,900	3920	0.312	0.31	0.31
217%	70%	4,550	4550	0.36	0.358	0.36
248%	80%	5,200	5200	0.422	0.424	0.42
279%	90%	5,850	5850	0.506	0.526	0.52
310%	100%	6,500	6540	0.636	0.732	0.68
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6600	0.786	1.1	0.94
0%	0%	0	0	0.366	0.702	0.53

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.53** **in**



Notes:

Pile bending at 6,600 lbs. No reliable test results available after this point.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-44D

Axial Uplift Load Test Results

PTL - **44** Designation: **D**

Section: **W6x12**

Latitude: 33.647911

Longitude: -114.68882

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 33.12

Pile Damaged? No

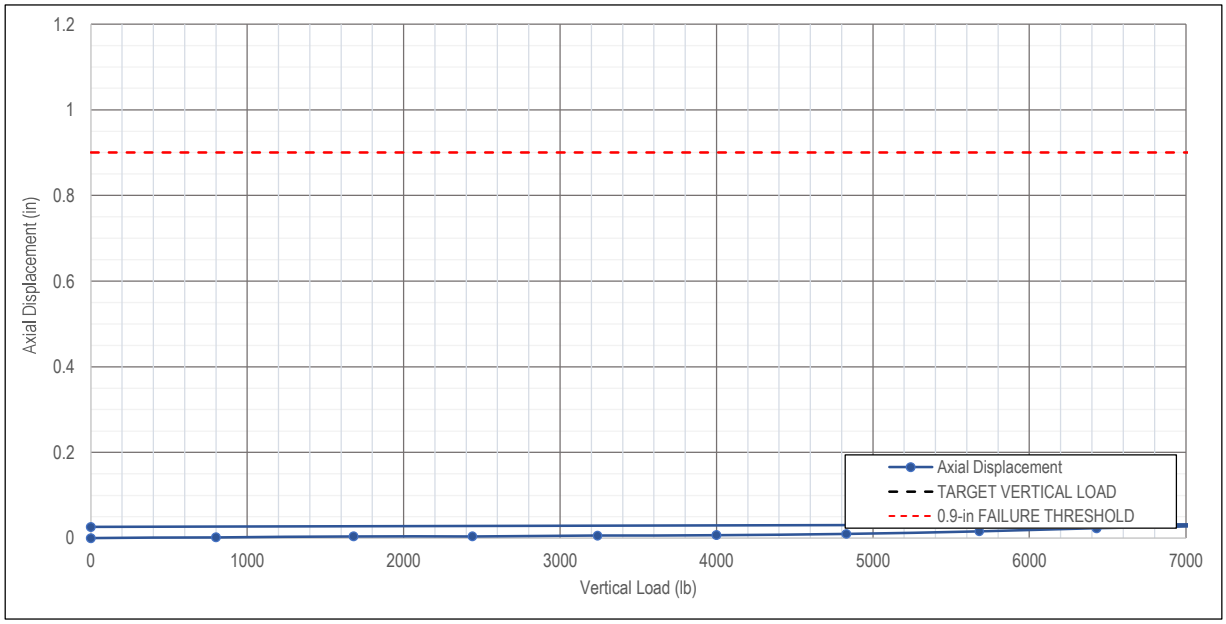
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0.002	0.002	0.00
436%	20%	1,600	1680	0.004	0.004	0.00
654%	30%	2,400	2440	0.004	0.004	0.00
872%	40%	3,200	3240	0.006	0.006	0.01
1090%	50%	4,000	4000	0.008	0.006	0.01
1308%	60%	4,800	4830	0.01	0.01	0.01
1526%	70%	5,600	5680	0.018	0.014	0.02
1744%	80%	6,400	6430	0.026	0.02	0.02
1962%	90%	7,200	7240	0.032	0.026	0.03
2180%	100%	8,000	8040	0.036	0.03	0.03
0%	0%	0	0	0.026	0.026	0.03

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-44D

Lateral Load Test Results

PTL- **44** Designation: **D**

Section: **W6x12**

Latitude: 33.647911

Longitude: -114.68882

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 33.12

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

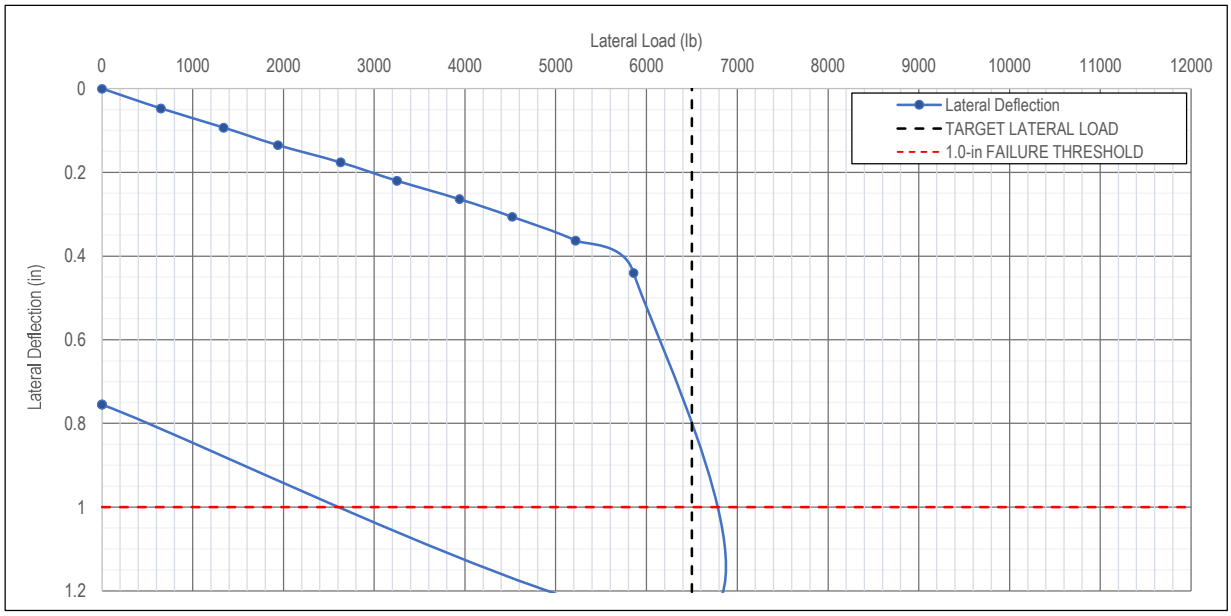
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.048	0.046	0.05
62%	20%	1,300	1340	0.094	0.092	0.09
93%	30%	1,950	1940	0.138	0.132	0.14
124%	40%	2,600	2630	0.18	0.172	0.18
155%	50%	3,250	3250	0.224	0.216	0.22
186%	60%	3,900	3940	0.27	0.258	0.26
217%	70%	4,550	4520	0.314	0.298	0.31
248%	80%	5,200	5220	0.37	0.356	0.36
279%	90%	5,850	5860	0.454	0.426	0.44
310%	100%	6,500	6540	1.136	1.408	1.27
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.742	0.768	0.76

Load at 1.0-in lateral deflection: **6,318** **lb**

Residual lateral deformation: **0.76** **in**



Notes:

Failure reached at 6,540 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-45A

Axial Uplift Load Test Results

PTL - **45** Designation: **A**

Section: W6x9

Latitude: 33.649559

Longitude: -114.68584

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 8.5

Pile Damaged? No

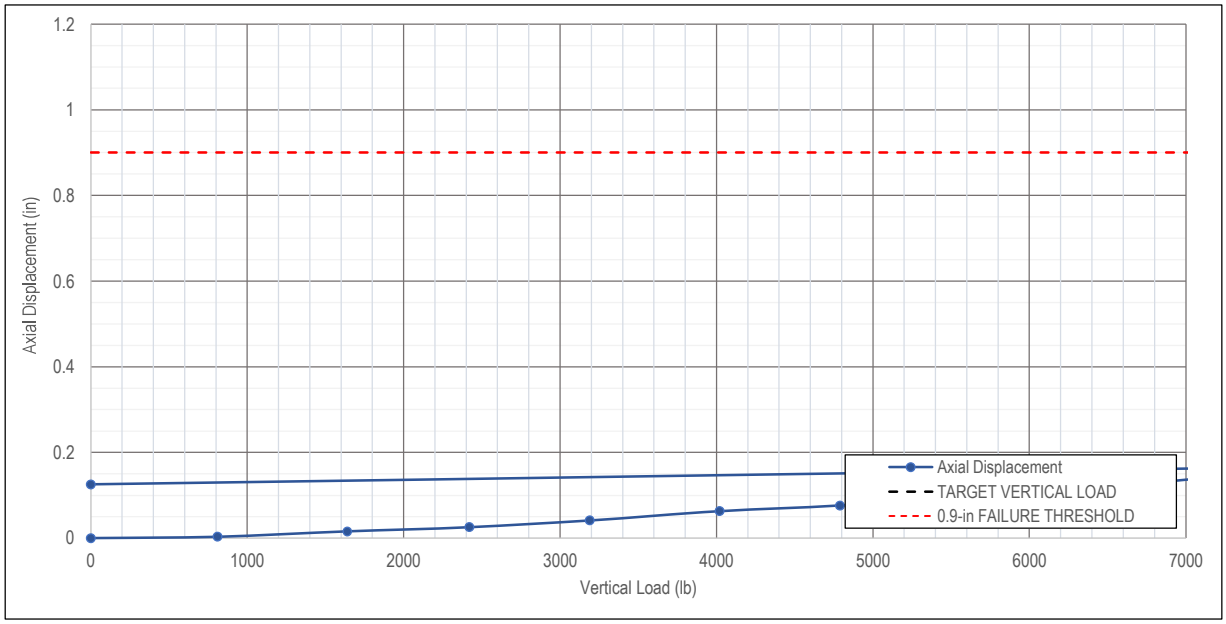
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	810	0.0035	0.0035	0.00
13594%	20%	1,600	1640	0.015	0.0165	0.02
20391%	30%	2,400	2420	0.0245	0.0265	0.03
27188%	40%	3,200	3190	0.04	0.0425	0.04
33985%	50%	4,000	4020	0.061	0.0655	0.06
40782%	60%	4,800	4790	0.073	0.079	0.08
47579%	70%	5,600	5590	0.0915	0.098	0.09
54376%	80%	6,400	6430	0.114	0.1205	0.12
61173%	90%	7,200	7210	0.1395	0.147	0.14
67970%	100%	8,000	8080	0.1615	0.172	0.17
0%	0%	0	0	0.126	0.1255	0.13

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.13** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-45A

Lateral Load Test Results

PTL- **45** Designation: **A**

Section: W6x9

Latitude: 33.649559

Longitude: -114.68584

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 8.5

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

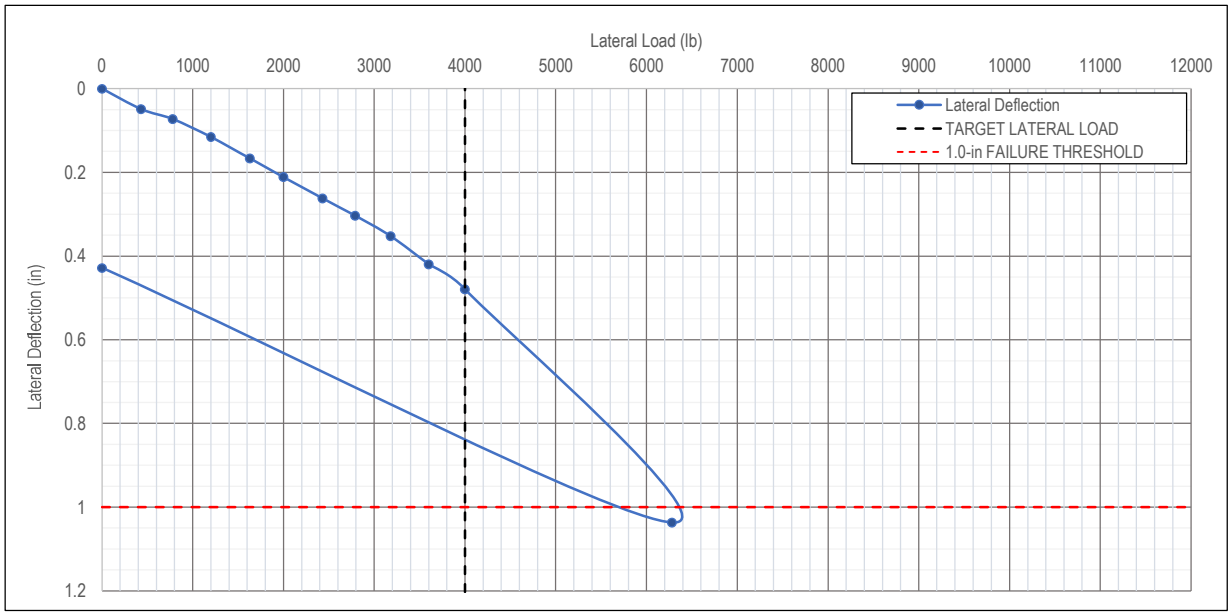
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	430	0.0475	0.05	0.05
62%	20%	800	780	0.0715	0.0735	0.07
93%	30%	1,200	1200	0.114	0.1165	0.12
124%	40%	1,600	1630	0.166	0.1675	0.17
155%	50%	2,000	2000	0.2105	0.212	0.21
186%	60%	2,400	2430	0.262	0.262	0.26
217%	70%	2,800	2790	0.3035	0.3035	0.30
248%	80%	3,200	3180	0.352	0.352	0.35
279%	90%	3,600	3600	0.4195	0.4195	0.42
310%	100%	4,000	4000	0.4795	0.4795	0.48
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6280	1.074	1	1.04
0%	0%	0	0	0.4565	0.4	0.43

Load at 1.0-in lateral deflection: **6,129** **lb**

Residual lateral deformation: **0.43** **in**



Notes:

Failure reached at 6,280 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-45B

Axial Uplift Load Test Results

PTL - **45** Designation: **B**

Section: W6x9

Latitude: 33.649559

Longitude: -114.68584

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 11.47

Pile Damaged? No

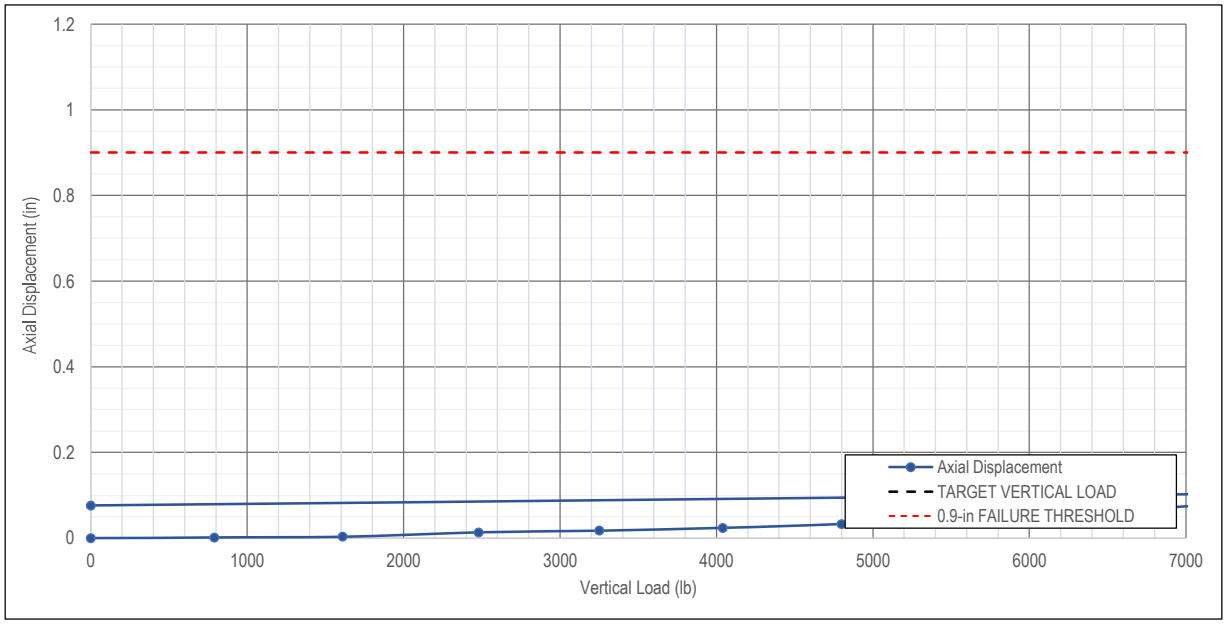
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	790	0.0015	0.0015	0.00
13594%	20%	1,600	1610	0.003	0.004	0.00
20391%	30%	2,400	2480	0.0115	0.015	0.01
27188%	40%	3,200	3250	0.015	0.02	0.02
33985%	50%	4,000	4040	0.021	0.0265	0.02
40782%	60%	4,800	4800	0.0295	0.036	0.03
47579%	70%	5,600	5580	0.0405	0.0485	0.04
54376%	80%	6,400	6430	0.0565	0.0655	0.06
61173%	90%	7,200	7240	0.075	0.086	0.08
67970%	100%	8,000	8010	0.098	0.1115	0.10
0%	0%	0	0	0.075	0.077	0.08

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.08** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-45B

Lateral Load Test Results

PTL- **45** Designation: **B**

Section: W6x9

Latitude: 33.649559

Longitude: -114.68584

Tested By: ANS

Test Date: 10/22/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.47

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

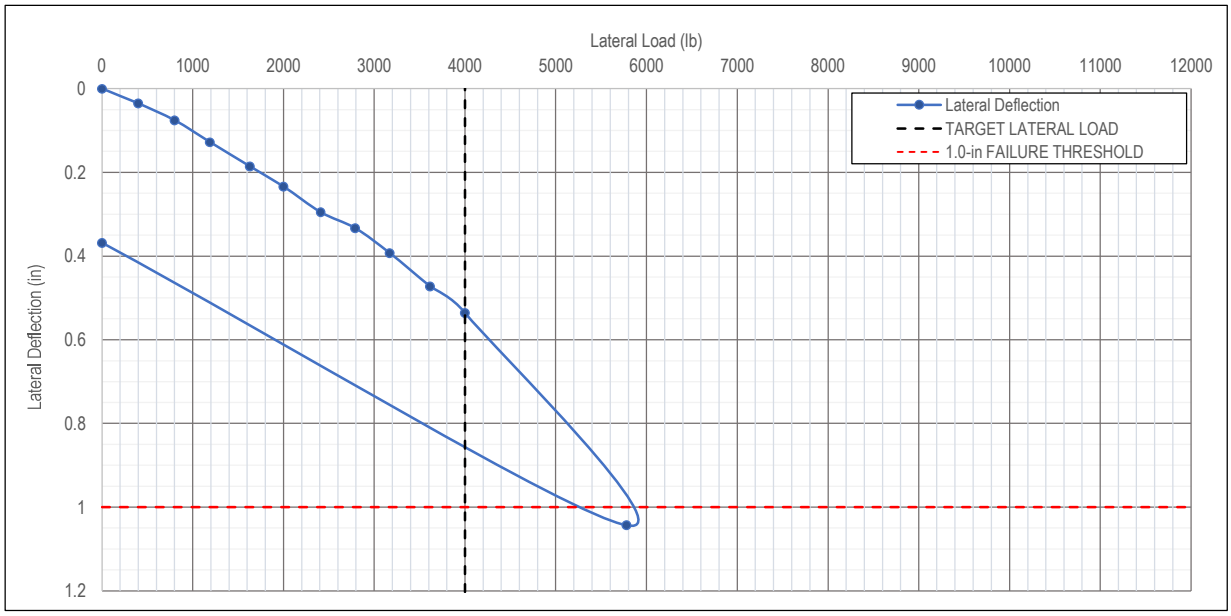
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.035	0.035	0.04
62%	20%	800	800	0.0745	0.0765	0.08
93%	30%	1,200	1190	0.1265	0.129	0.13
124%	40%	1,600	1630	0.1835	0.1875	0.19
155%	50%	2,000	2000	0.2325	0.2356	0.23
186%	60%	2,400	2410	0.2935	0.297	0.30
217%	70%	2,800	2790	0.3315	0.335	0.33
248%	80%	3,200	3170	0.391	0.3945	0.39
279%	90%	3,600	3616	0.471	0.474	0.47
310%	100%	4,000	4000	0.537	0.534	0.54
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5780	1.054	1.0325	1.04
0%	0%	0	0	0.3715	0.3655	0.37

Load at 1.0-in lateral deflection: **5,628** **lb**

Residual lateral deformation: **0.37** **in**



Notes:

Failure reached at 5,780 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-46A

Axial Uplift Load Test Results

PTL - **46** Designation: **A**

Section: W6x9

Latitude: 33.633657

Longitude: -114.69441

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 8.54

Pile Damaged? No

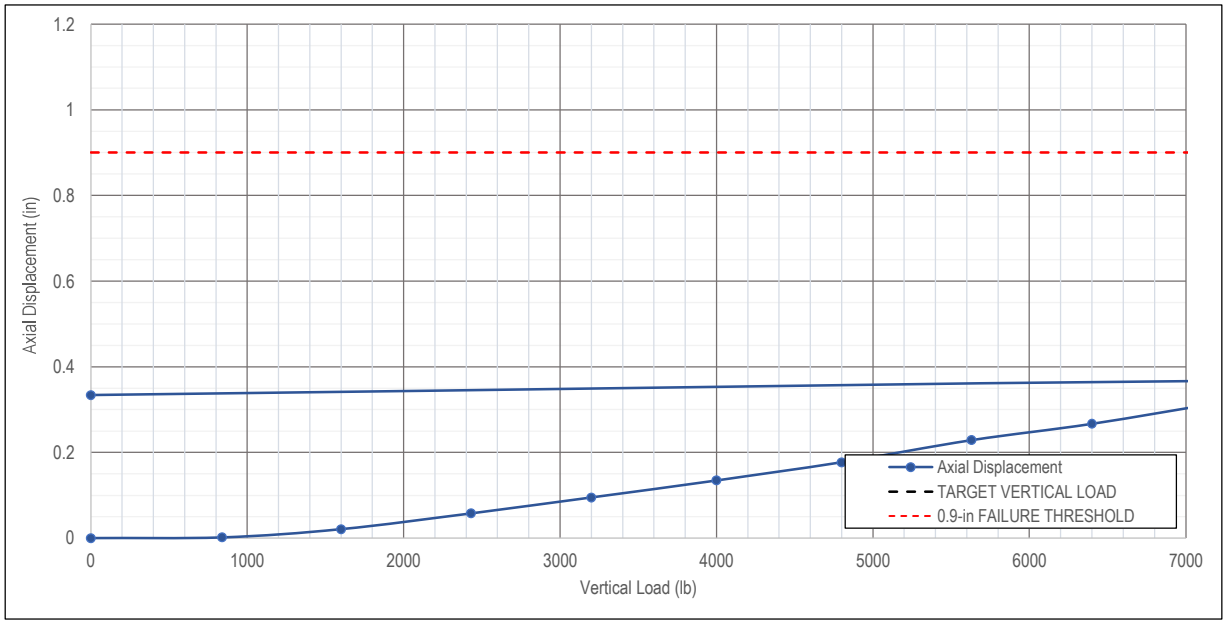
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	840	0.002	0.002	0.00
436%	20%	1,600	1600	0.02	0.022	0.02
654%	30%	2,400	2430	0.056	0.06	0.06
872%	40%	3,200	3200	0.094	0.096	0.10
1090%	50%	4,000	4000	0.134	0.136	0.14
1308%	60%	4,800	4800	0.178	0.176	0.18
1526%	70%	5,600	5630	0.23	0.228	0.23
1744%	80%	6,400	6400	0.268	0.266	0.27
1962%	90%	7,200	7240	0.32	0.316	0.32
2180%	100%	8,000	8030	0.37	0.366	0.37
0%	0%	0	0	0.334	0.334	0.33

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.33** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-46A

Lateral Load Test Results

PTL- **46** Designation: **A**

Section: W6x9

Latitude: 33.633657

Longitude: -114.69441

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 8.54

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

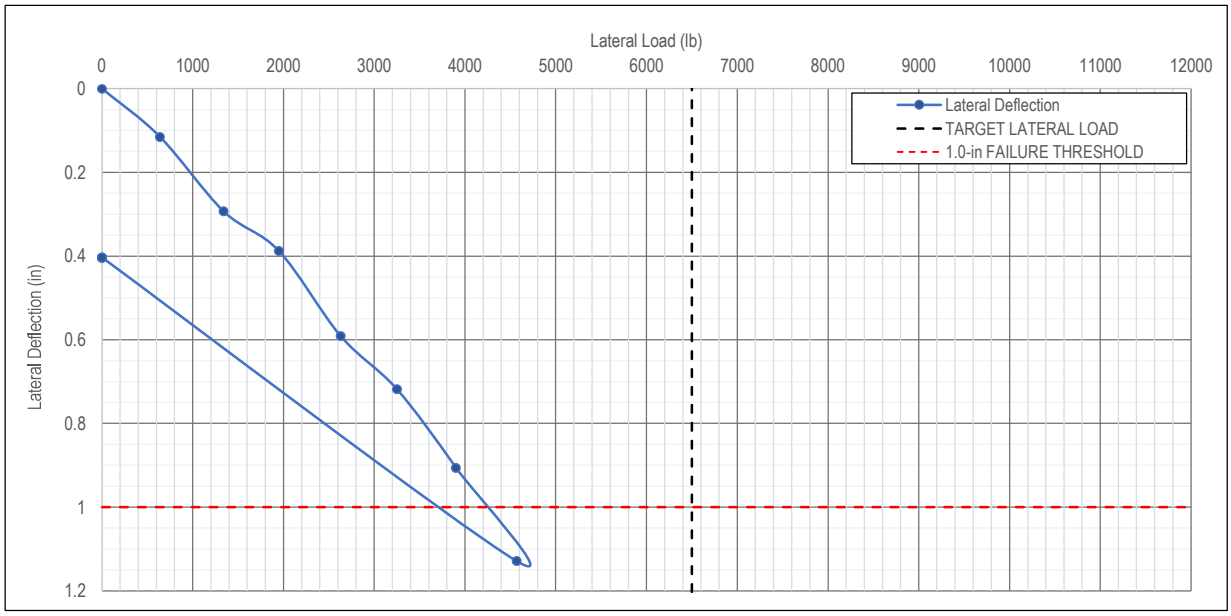
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	640	0.12	0.11	0.12
62%	20%	1,300	1340	0.304	0.282	0.29
93%	30%	1,950	1950	0.402	0.374	0.39
124%	40%	2,600	2630	0.61	0.572	0.59
155%	50%	3,250	3250	0.74	0.696	0.72
186%	60%	3,900	3900	0.932	0.88	0.91
217%	70%	4,550	4570	1.162	1.096	1.13
248%	80%	5,200				
279%	90%	5,850				
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.415	0.392	0.40

Load at 1.0-in lateral deflection: **4,182** **lb**

Residual lateral deformation: **0.40** **in**



Notes:

Failure reached at 4,570 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-46B

Axial Uplift Load Test Results

PTL - **46** Designation: **B**

Section: W6x9

Latitude: 33.633657

Longitude: -114.69441

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 10.79

Pile Damaged? No

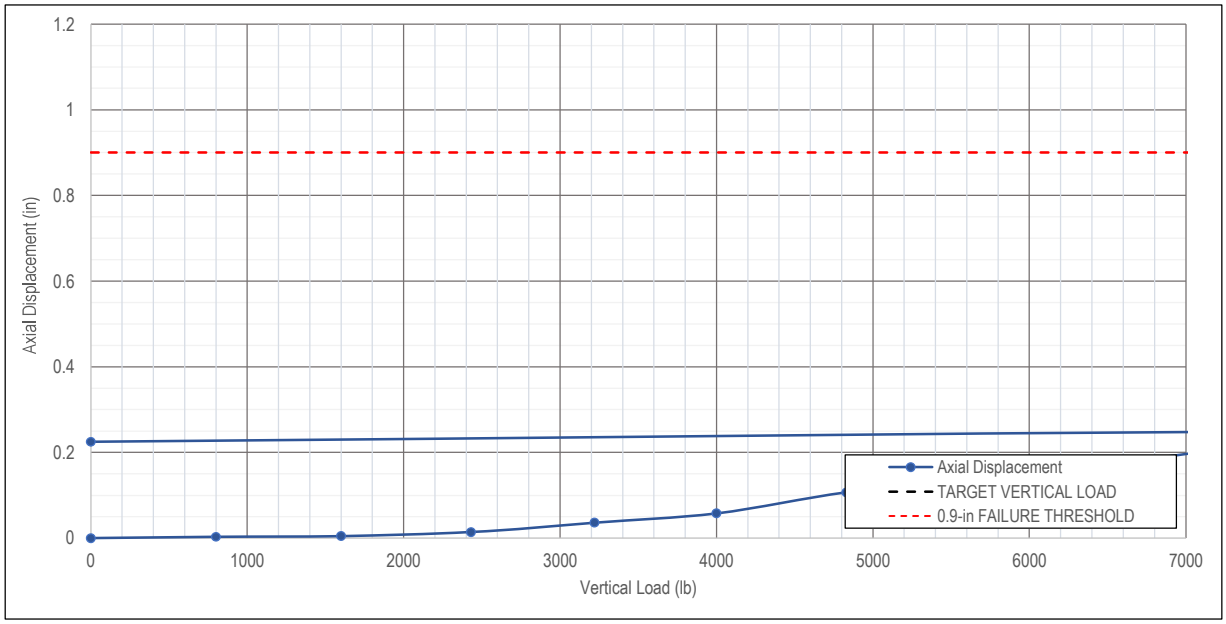
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0.002	0.004	0.00
436%	20%	1,600	1600	0.004	0.006	0.01
654%	30%	2,400	2430	0.012	0.016	0.01
872%	40%	3,200	3220	0.034	0.038	0.04
1090%	50%	4,000	4000	0.056	0.06	0.06
1308%	60%	4,800	4830	0.106	0.108	0.11
1526%	70%	5,600	5600	0.136	0.136	0.14
1744%	80%	6,400	6400	0.168	0.168	0.17
1962%	90%	7,200	7200	0.206	0.206	0.21
2180%	100%	8,000	8040	0.248	0.248	0.25
0%	0%	0	0	0.226	0.224	0.23

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.23** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-46B

Lateral Load Test Results

PTL- **46** Designation: **B**

Section: W6x9

Latitude: 33.633657

Longitude: -114.69441

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 10.79

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

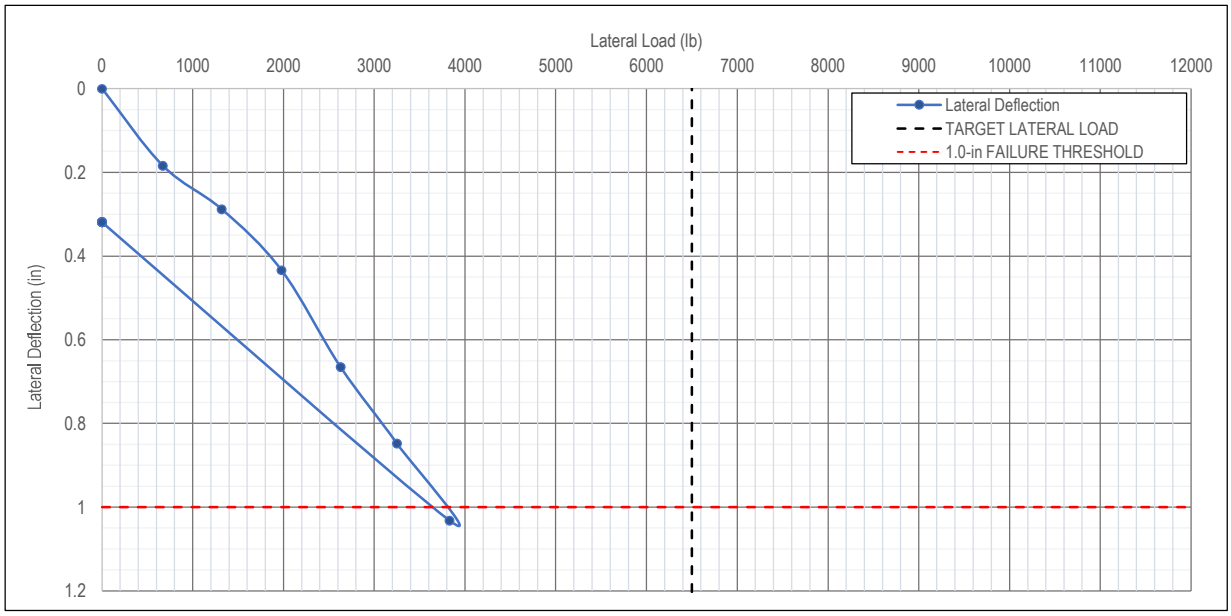
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	670	0.188	0.18	0.18
62%	20%	1,300	1320	0.294	0.282	0.29
93%	30%	1,950	1980	0.442	0.426	0.43
124%	40%	2,600	2630	0.676	0.654	0.67
155%	50%	3,250	3250	0.86	0.836	0.85
186%	60%	3,900	3830	1.044	1.02	1.03
217%	70%	4,550				
248%	80%	5,200				
279%	90%	5,850				
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.316	0.322	0.32

Load at 1.0-in lateral deflection: **3,729** **lb**

Residual lateral deformation: **0.32** **in**



Notes:

Failure reached at 3,830 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-47A

Axial Uplift Load Test Results

PTL - **47** Designation: **A**

Section: W6x9

Latitude: 33.649905

Longitude: -114.68116

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 7.35

Pile Damaged? No

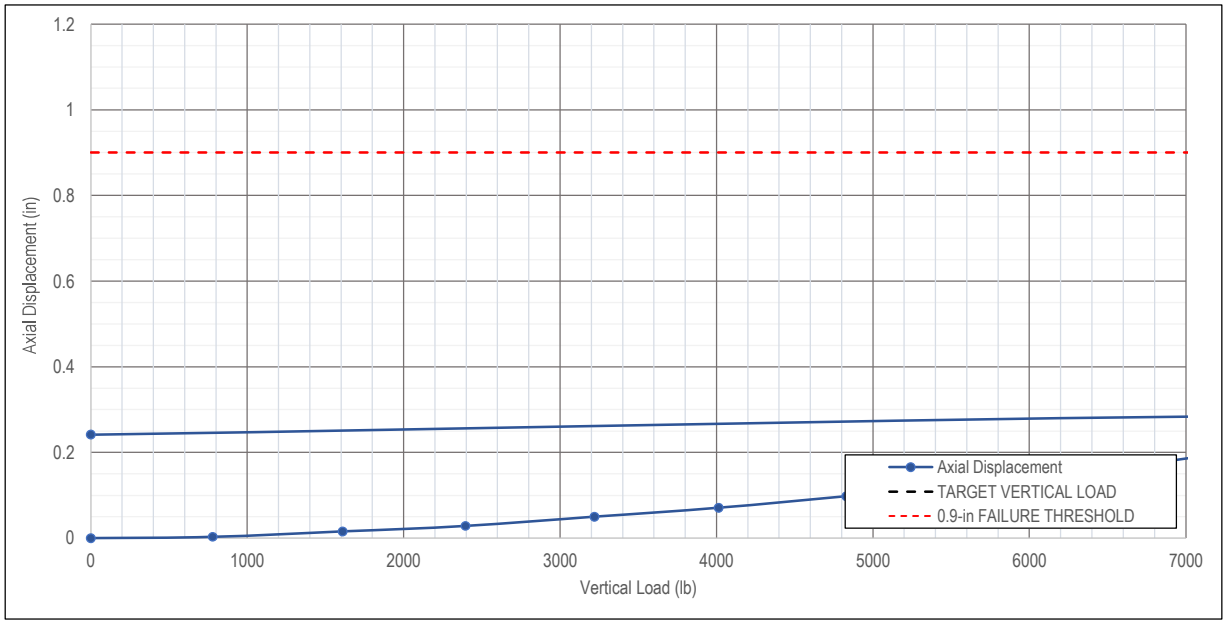
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	780	0.003	0.003	0.00
13594%	20%	1,600	1610	0.011	0.02	0.02
20391%	30%	2,400	2395	0.027	0.03	0.03
27188%	40%	3,200	3220	0.049	0.051	0.05
33985%	50%	4,000	4015	0.069	0.073	0.07
40782%	60%	4,800	4830	0.095	0.1005	0.10
47579%	70%	5,600	5625	0.119	0.1255	0.12
54376%	80%	6,400	6420	0.157	0.1655	0.16
61173%	90%	7,200	7220	0.195	0.204	0.20
67970%	100%	8,000	8040	0.278	0.29	0.28
0%	0%	0	0	0.24	0.2435	0.24

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.24** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-47A

Lateral Load Test Results

PTL- **47** Designation: **A**

Section: W6x9

Latitude: 33.649905

Longitude: -114.68116

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 7.35

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

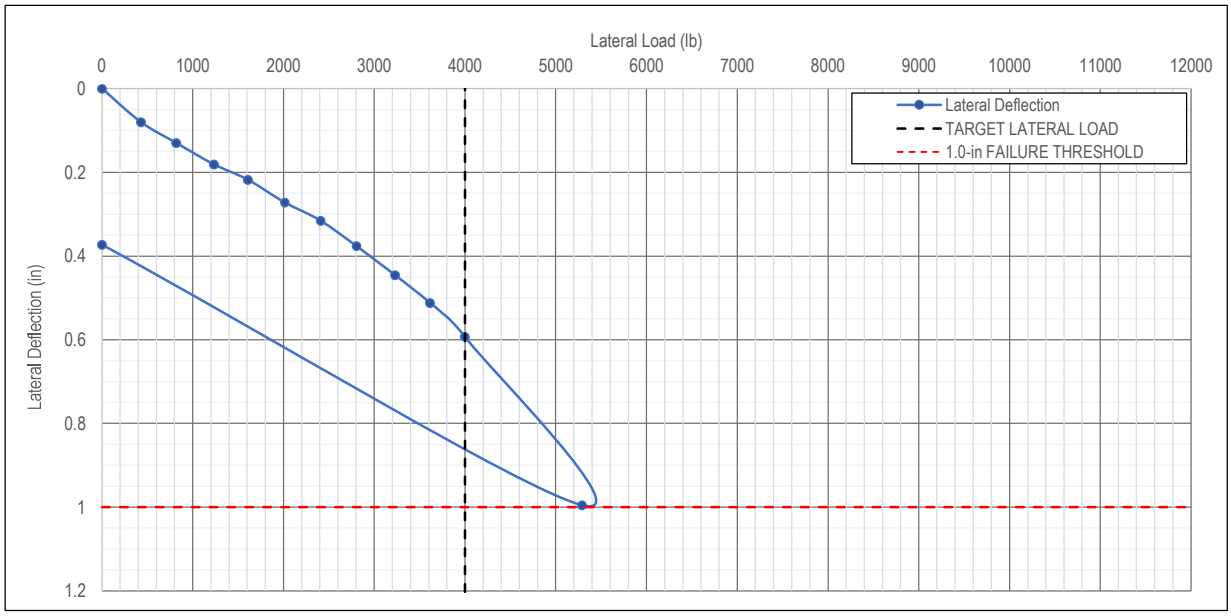
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	430	0.072	0.088	0.08
62%	20%	800	820	0.1185	0.141	0.13
93%	30%	1,200	1235	0.168	0.194	0.18
124%	40%	1,600	1610	0.2035	0.232	0.22
155%	50%	2,000	2015	0.2565	0.2875	0.27
186%	60%	2,400	2410	0.2985	0.3325	0.32
217%	70%	2,800	2805	0.3575	0.395	0.38
248%	80%	3,200	3230	0.4245	0.467	0.45
279%	90%	3,600	3615	0.488	0.536	0.51
310%	100%	4,000	4000	0.564	0.6215	0.59
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5290	0.924	1.067	1.00
0%	0%	0	0	0.333	0.413	0.37

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.37** **in**



Notes:

Failure reached at 5,290 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-47B

Axial Uplift Load Test Results

PTL - **47** Designation: **B**

Section: W6x9

Latitude: 33.649905

Longitude: -114.68116

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 15.66

Pile Damaged? No

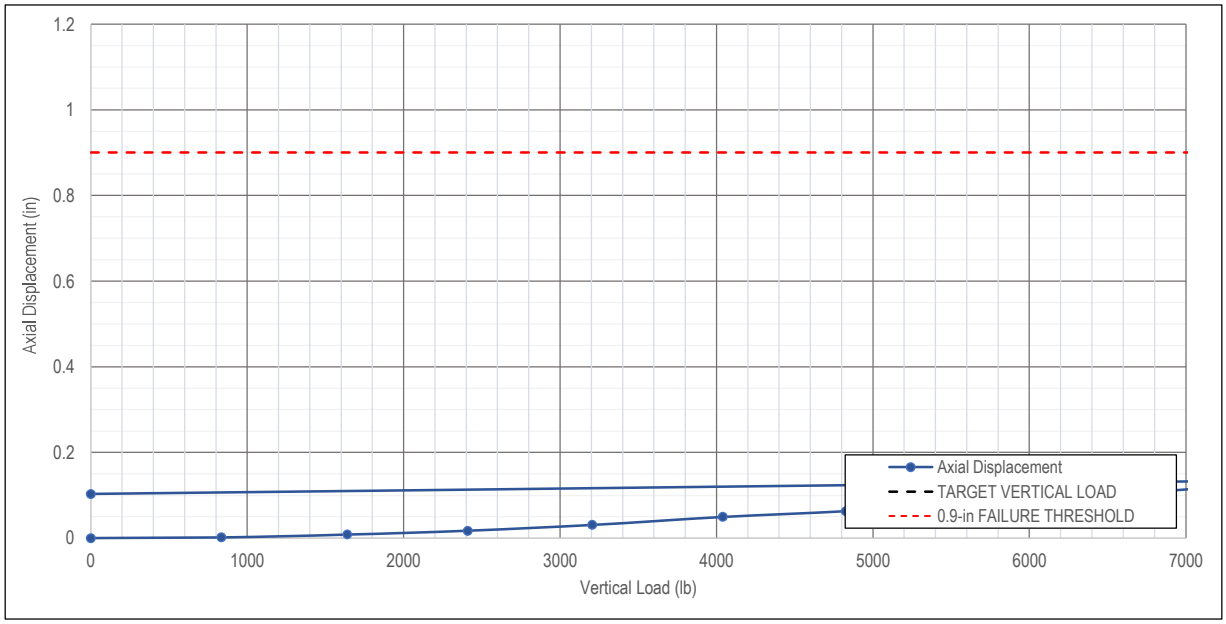
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	835	0.0015	0.0025	0.00
13594%	20%	1,600	1640	0.008	0.0085	0.01
20391%	30%	2,400	2410	0.017	0.0175	0.02
27188%	40%	3,200	3205	0.0305	0.0315	0.03
33985%	50%	4,000	4040	0.049	0.05005	0.05
40782%	60%	4,800	4825	0.062	0.064	0.06
47579%	70%	5,600	5635	0.0815	0.0845	0.08
54376%	80%	6,400	6445	0.0975	0.1	0.10
61173%	90%	7,200	7260	0.1195	0.122	0.12
67970%	100%	8,000	7990	0.1345	0.137	0.14
0%	0%	0	0	0.1035	0.103	0.10

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.10** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-47B

Lateral Load Test Results

PTL- **47** Designation: **B**

Section: W6x9

Latitude: 33.649905

Longitude: -114.68116

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 15.66

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

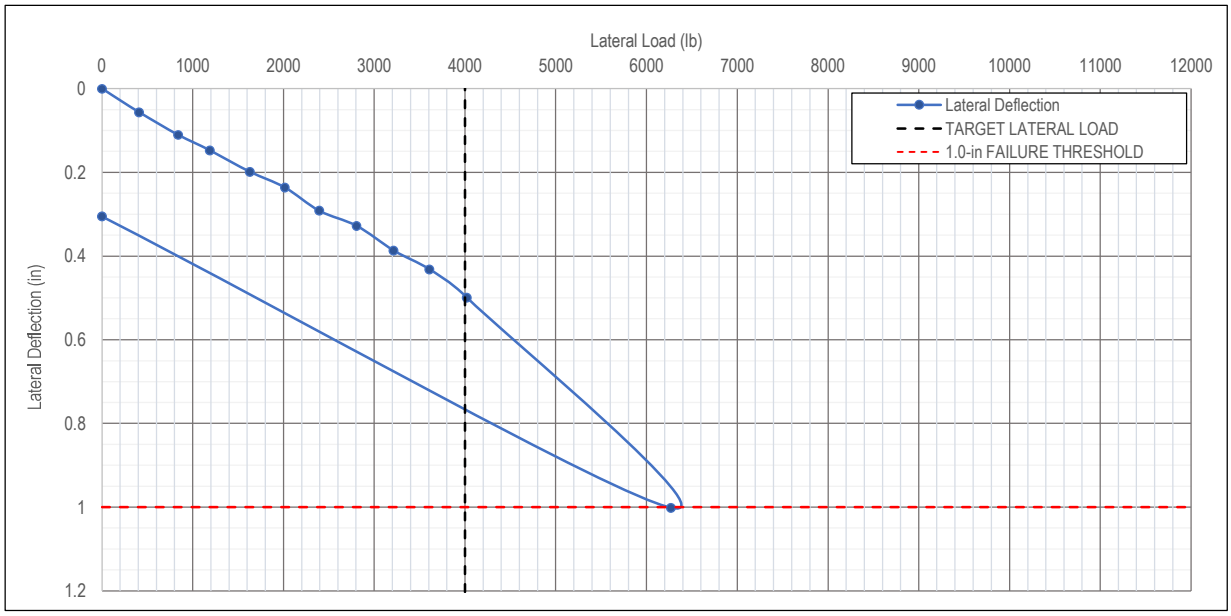
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	410	0.0555	0.0575	0.06
62%	20%	800	840	0.1085	0.112	0.11
93%	30%	1,200	1190	0.1435	0.1515	0.15
124%	40%	1,600	1630	0.1935	0.2035	0.20
155%	50%	2,000	2015	0.231	0.241	0.24
186%	60%	2,400	2395	0.286	0.297	0.29
217%	70%	2,800	2805	0.322	0.333	0.33
248%	80%	3,200	3215	0.381	0.3925	0.39
279%	90%	3,600	3610	0.426	0.4375	0.43
310%	100%	4,000	4020	0.49335	0.505	0.50
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6270	0.955	1.0485	1.00
0%	0%	0	0	0.275	0.3355	0.31

Load at 1.0-in lateral deflection: **6,262** **lb**

Residual lateral deformation: **0.31** **in**



Notes:

Failure reached at 6,270 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-48A

Axial Uplift Load Test Results

PTL - **48** Designation: **A**

Section: W6x9

Latitude: 33.650181

Longitude: -114.67692

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 9.19

Pile Damaged? No

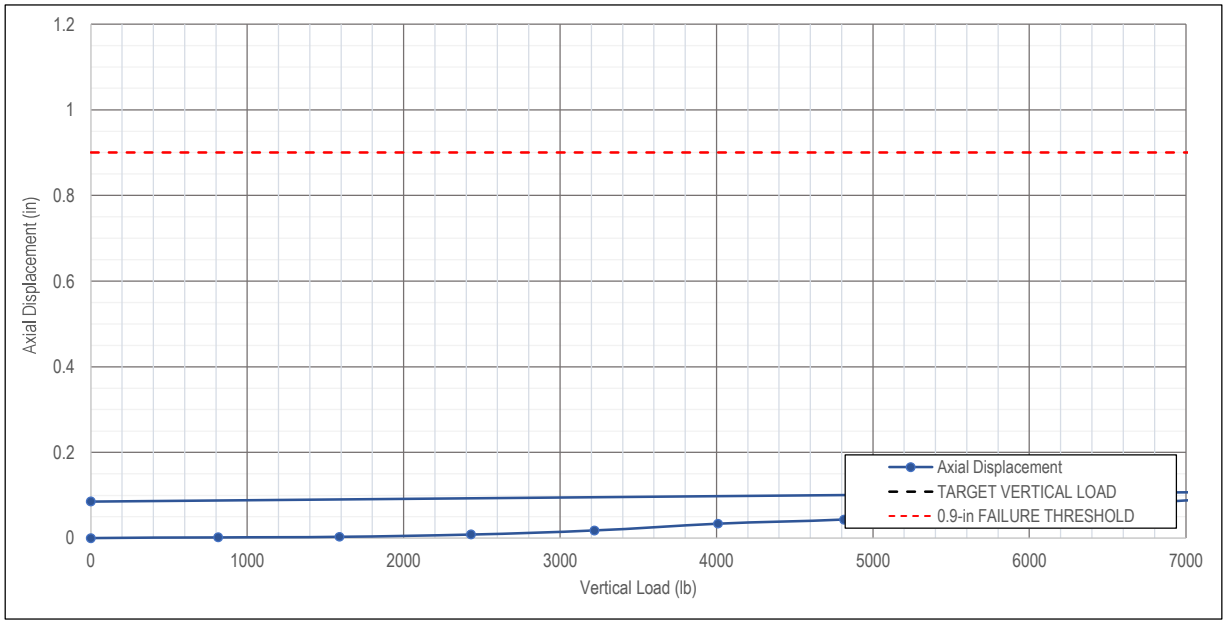
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	815	0.002	0.002	0.00
436%	20%	1,600	1590	0.003	0.003	0.00
654%	30%	2,400	2430	0.0095	0.0075	0.01
872%	40%	3,200	3220	0.02	0.016	0.02
1090%	50%	4,000	4010	0.037	0.0305	0.03
1308%	60%	4,800	4815	0.047	0.04	0.04
1526%	70%	5,600	5600	0.0655	0.056	0.06
1744%	80%	6,400	6390	0.0805	0.069	0.07
1962%	90%	7,200	7210	0.099	0.086	0.09
2180%	100%	8,000	8005	0.117	0.101	0.11
0%	0%	0	0	0.087	0.084	0.09

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.09** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-48A

Lateral Load Test Results

PTL- **48** Designation: **A**

Section: W6x9

Latitude: 33.650181

Longitude: -114.67692

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 9.19

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

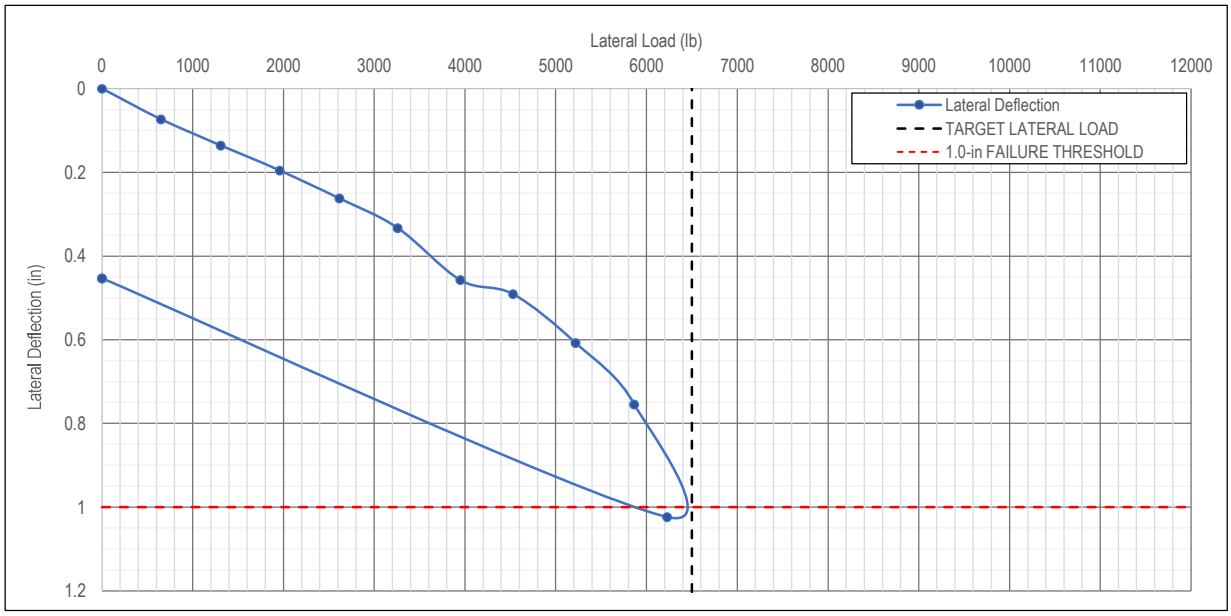
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.0755	0.0705	0.07
62%	20%	1,300	1310	0.1385	0.1325	0.14
93%	30%	1,950	1960	0.199	0.192	0.20
124%	40%	2,600	2615	0.2655	0.2585	0.26
155%	50%	3,250	3260	0.336	0.3295	0.33
186%	60%	3,900	3950	0.4599	0.455	0.46
217%	70%	4,550	4530	0.492	0.49	0.49
248%	80%	5,200	5220	0.601	0.614	0.61
279%	90%	5,850	5865	0.7285	0.781	0.75
310%	100%	6,500	6230	0.884	1.163	1.02
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.352	0.5545	0.45

Load at 1.0-in lateral deflection: **6,198** **lb**

Residual lateral deformation: **0.45** **in**



Notes:

Failure reached at 6,230 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-48B

Axial Uplift Load Test Results

PTL - **48** Designation: **B**

Section: W6x9

Latitude: 33.650181

Longitude: -114.67692

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 17.31

Pile Damaged? No

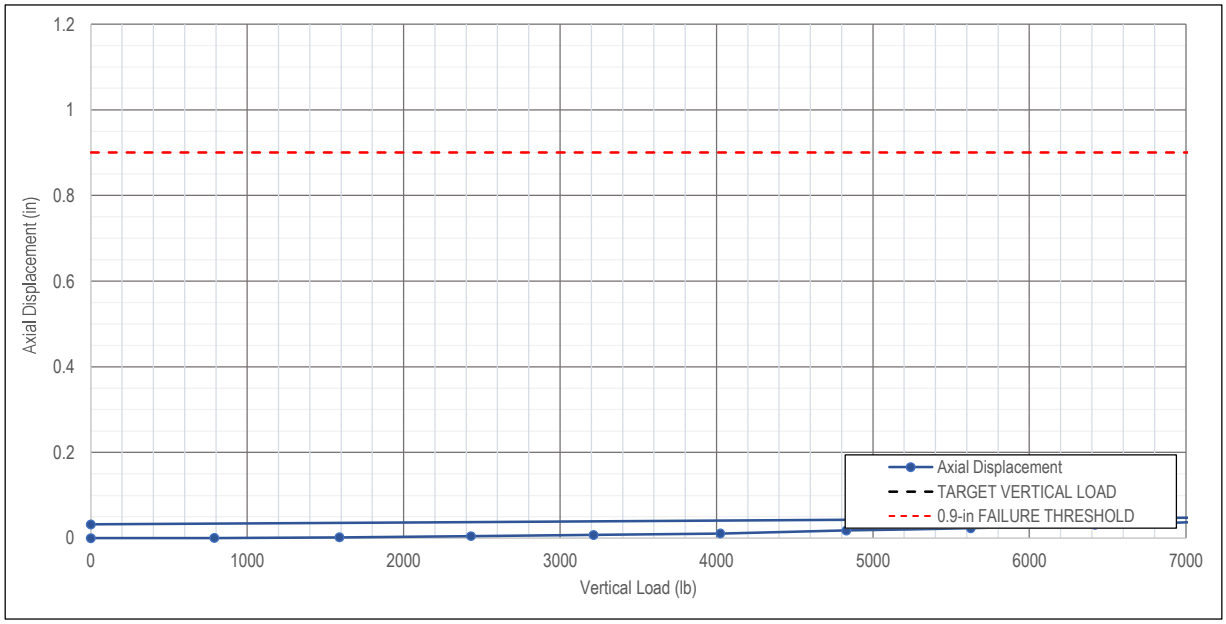
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	790	0.0005	0.0005	0.00
436%	20%	1,600	1590	0.002	0.002	0.00
654%	30%	2,400	2430	0.0045	0.005	0.00
872%	40%	3,200	3215	0.0075	0.008	0.01
1090%	50%	4,000	4025	0.011	0.011	0.01
1308%	60%	4,800	4830	0.018	0.0175	0.02
1526%	70%	5,600	5625	0.024	0.0225	0.02
1744%	80%	6,400	6420	0.032	0.0305	0.03
1962%	90%	7,200	7210	0.04	0.038	0.04
2180%	100%	8,000	8035	0.051	0.0475	0.05
0%	0%	0	0	0.0325	0.0315	0.03

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-48B

Lateral Load Test Results

PTL- **48** Designation: **B**

Section: W6x9

Latitude: 33.650181

Longitude: -114.67692

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 17.31

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

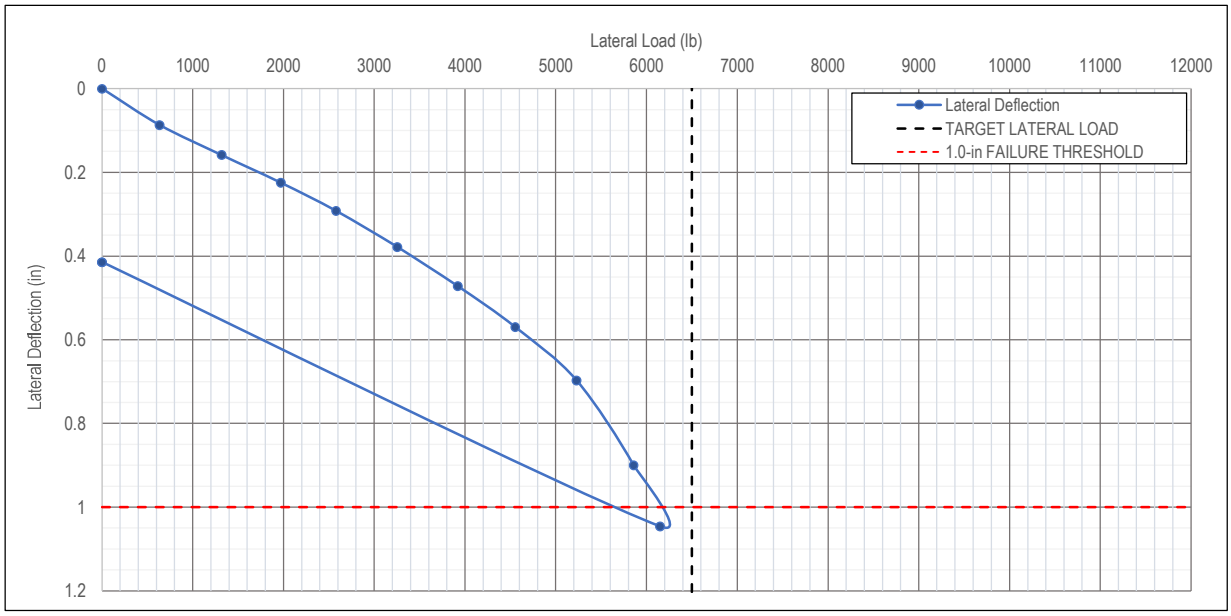
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	635	0.0875	0.086	0.09
62%	20%	1,300	1320	0.16	0.157	0.16
93%	30%	1,950	1970	0.2265	0.222	0.22
124%	40%	2,600	2580	0.295	0.289	0.29
155%	50%	3,250	3255	0.382	0.375	0.38
186%	60%	3,900	3920	0.476	0.4675	0.47
217%	70%	4,550	4555	0.5755	0.564	0.57
248%	80%	5,200	5230	0.7045	0.6895	0.70
279%	90%	5,850	5860	0.918	0.8815	0.90
310%	100%	6,500	6150	1.0755	1.017	1.05
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.4345	0.3945	0.41

Load at 1.0-in lateral deflection: **6,058** **lb**

Residual lateral deformation: **0.41** **in**



Notes:

Failure reached at 6,150 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-48D

Axial Uplift Load Test Results

PTL - **48** Designation: **D**

Section: **W6x12**

Latitude: 33.650181

Longitude: -114.67692

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 18.05

Pile Damaged? No

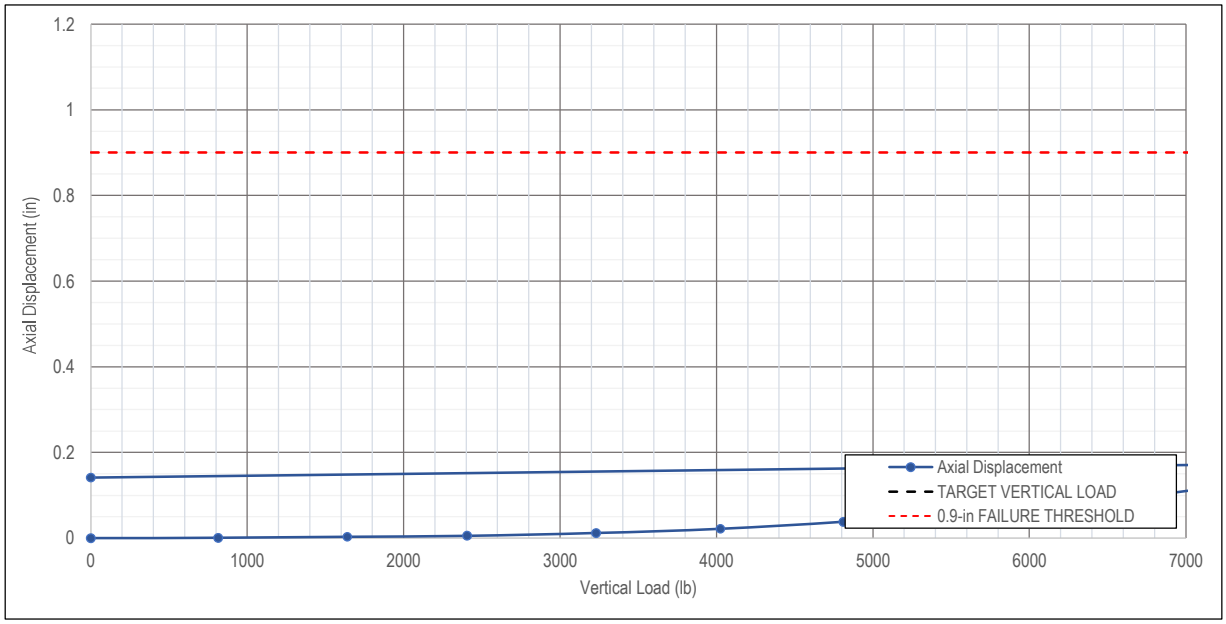
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	815	0.001	0.001	0.00
436%	20%	1,600	1640	0.003	0.003	0.00
654%	30%	2,400	2405	0.0055	0.0055	0.01
872%	40%	3,200	3230	0.0125	0.0115	0.01
1090%	50%	4,000	4025	0.0225	0.021	0.02
1308%	60%	4,800	4810	0.039	0.0375	0.04
1526%	70%	5,600	5630	0.062	0.0595	0.06
1744%	80%	6,400	6410	0.085	0.0825	0.08
1962%	90%	7,200	7215	0.1225	0.1205	0.12
2180%	100%	8,000	8010	0.1735	0.1695	0.17
0%	0%	0	0	0.1425	0.1405	0.14

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.14** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-48D

Lateral Load Test Results

PTL- **48** Designation: **D**

Section: **W6x12**

Latitude: 33.650181

Longitude: -114.67692

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 18.05

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

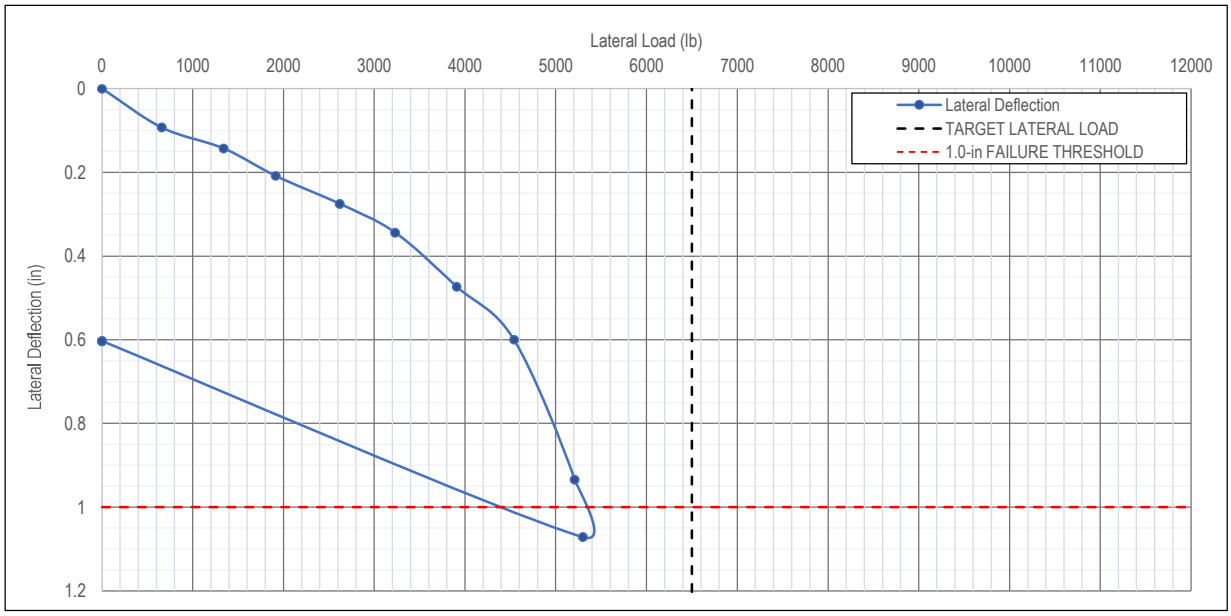
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	660	0.092	0.0935	0.09
62%	20%	1,300	1340	0.1415	0.1435	0.14
93%	30%	1,950	1915	0.2065	0.21	0.21
124%	40%	2,600	2620	0.2735	0.277	0.28
155%	50%	3,250	3230	0.342	0.3455	0.34
186%	60%	3,900	3910	0.4715	0.475	0.47
217%	70%	4,550	4540	0.597	0.6025	0.60
248%	80%	5,200	5210	0.929	0.939	0.93
279%	90%	5,850	5300	1.066	1.077	1.07
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.6015	0.6045	0.60

Load at 1.0-in lateral deflection: **5,253** **lb**

Residual lateral deformation: **0.60** **in**



Notes:

Failure reached at 5,300 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-49A

Axial Uplift Load Test Results

PTL - **49** Designation: **A**

Section: W6x9

Latitude: 33.651724

Longitude: -114.67393

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 8.45

Pile Damaged? No

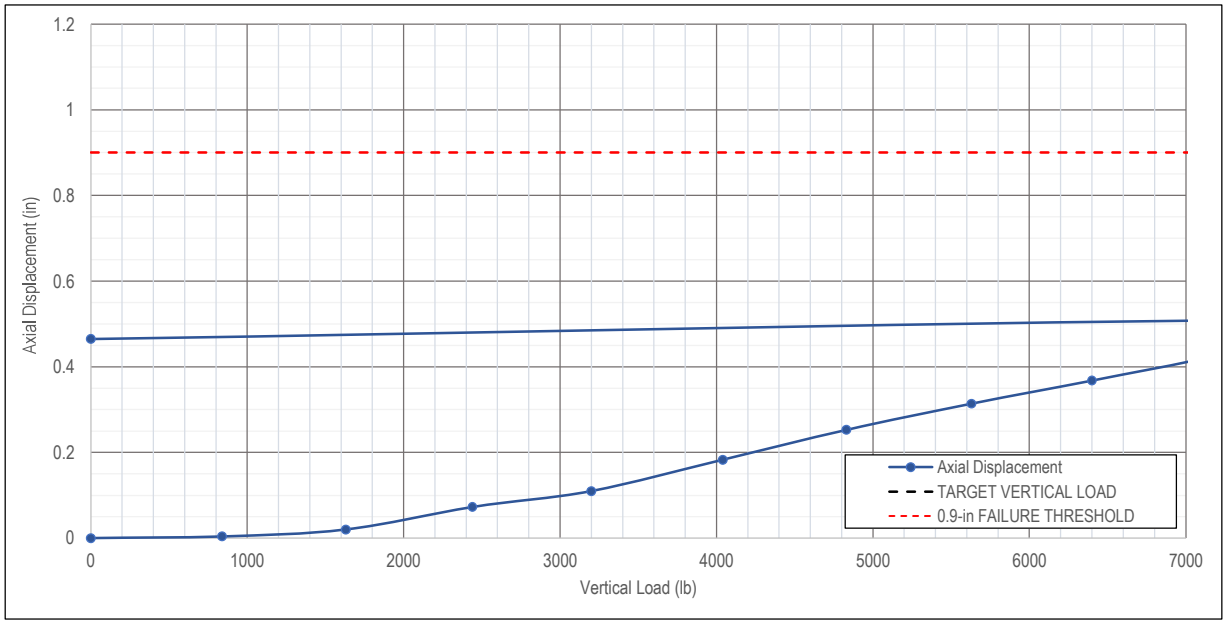
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	840	0.004	0.004	0.00
13594%	20%	1,600	1630	0.02	0.02	0.02
20391%	30%	2,400	2440	0.072	0.074	0.07
27188%	40%	3,200	3200	0.108	0.112	0.11
33985%	50%	4,000	4040	0.18	0.186	0.18
40782%	60%	4,800	4830	0.25	0.256	0.25
47579%	70%	5,600	5630	0.31	0.318	0.31
54376%	80%	6,400	6400	0.364	0.372	0.37
61173%	90%	7,200	7200	0.422	0.432	0.43
67970%	100%	8,000	8000	0.504	0.512	0.51
0%	0%	0	0	0.466	0.464	0.47

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.47** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-49A

Lateral Load Test Results

PTL- **49** Designation: **A**

Section: W6x9

Latitude: 33.651724

Longitude: -114.67393

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 8.45

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

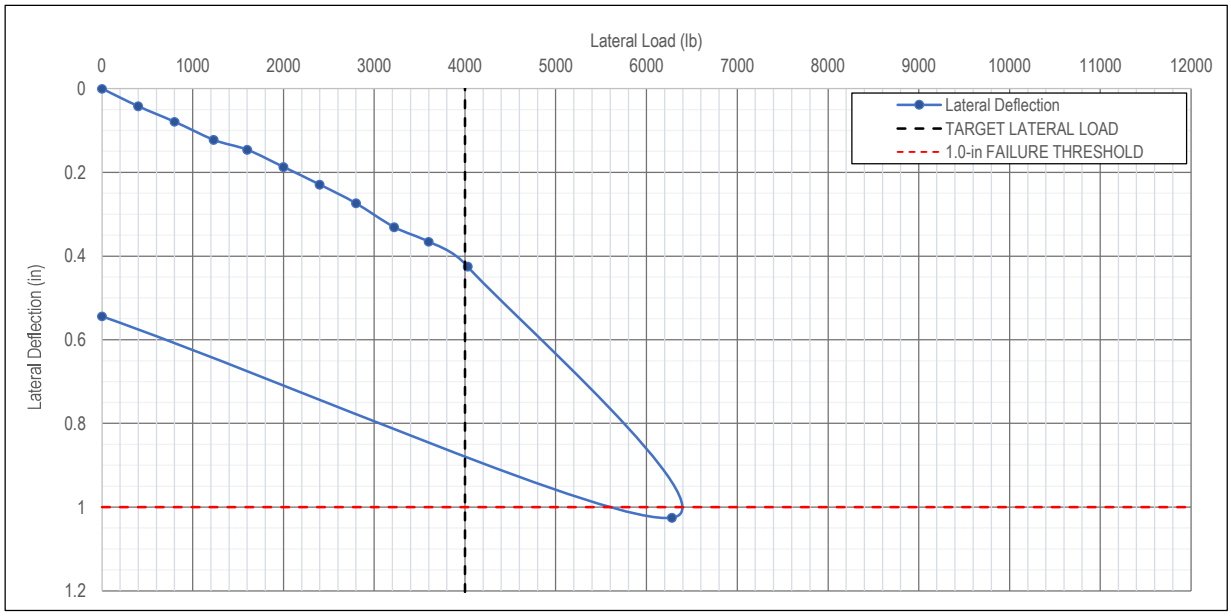
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.042	0.042	0.04
62%	20%	800	800	0.08	0.078	0.08
93%	30%	1,200	1230	0.126	0.118	0.12
124%	40%	1,600	1600	0.15	0.142	0.15
155%	50%	2,000	2000	0.192	0.182	0.19
186%	60%	2,400	2400	0.236	0.222	0.23
217%	70%	2,800	2800	0.282	0.266	0.27
248%	80%	3,200	3220	0.34	0.322	0.33
279%	90%	3,600	3600	0.376	0.356	0.37
310%	100%	4,000	4030	0.436	0.414	0.43
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6280	0.914	1.136	1.03
0%	0%	0	0	0.464	0.624	0.54

Load at 1.0-in lateral deflection: **6,186** **lb**

Residual lateral deformation: **0.54** **in**



Notes:

Failure reached at 6,280 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-49B

Axial Uplift Load Test Results

PTL - **49** Designation: **B**

Section: W6x9

Latitude: 33.651724

Longitude: -114.67393

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 14.04

Pile Damaged? No

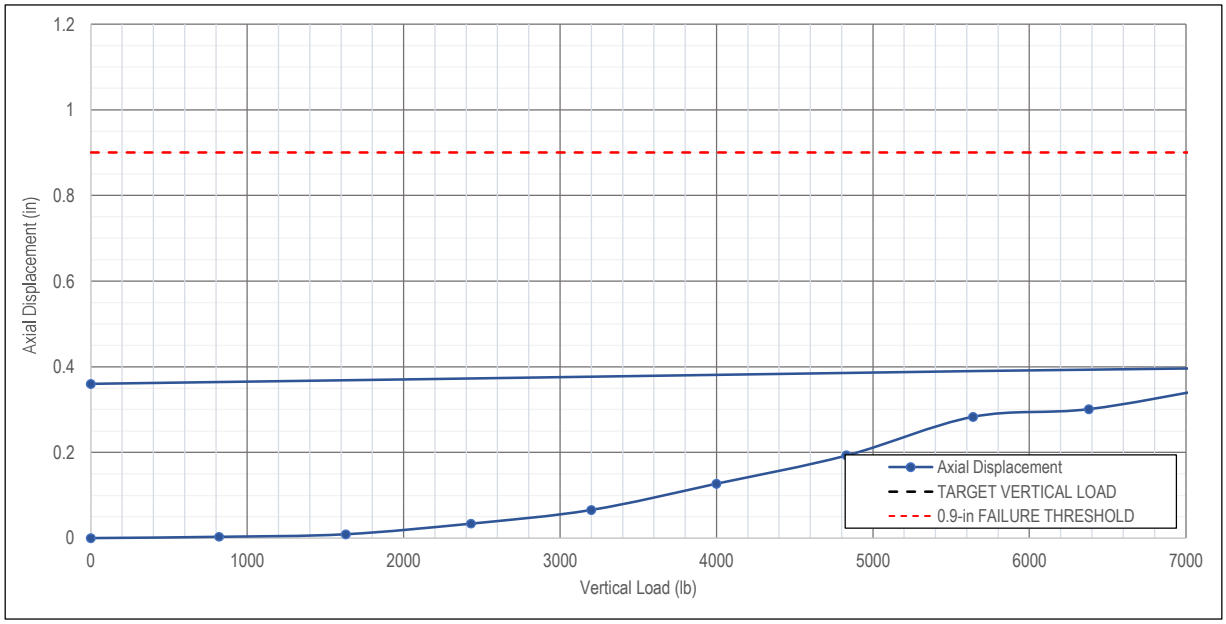
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	820	0.004	0.002	0.00
13594%	20%	1,600	1630	0.01	0.008	0.01
20391%	30%	2,400	2430	0.036	0.032	0.03
27188%	40%	3,200	3200	0.068	0.064	0.07
33985%	50%	4,000	4000	0.13	0.124	0.13
40782%	60%	4,800	4830	0.196	0.19	0.19
47579%	70%	5,600	5640	0.286	0.28	0.28
54376%	80%	6,400	6380	0.304	0.298	0.30
61173%	90%	7,200	7200	0.354	0.349	0.35
67970%	100%	8,000	8000	0.4	0.396	0.40
0%	0%	0	0	0.362	0.358	0.36

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.36** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-49B

Lateral Load Test Results

PTL- **49** Designation: **B**

Section: W6x9

Latitude: 33.651724

Longitude: -114.67393

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 14.04

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

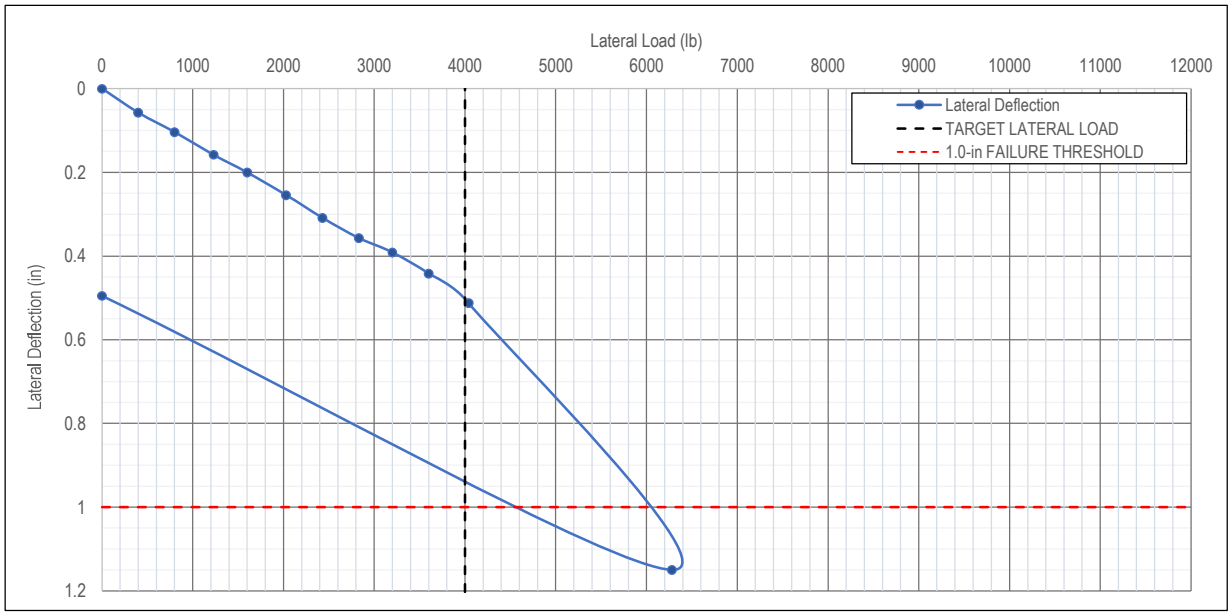
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.06	0.054	0.06
62%	20%	800	800	0.11	0.098	0.10
93%	30%	1,200	1230	0.166	0.15	0.16
124%	40%	1,600	1600	0.21	0.19	0.20
155%	50%	2,000	2030	0.266	0.242	0.25
186%	60%	2,400	2430	0.322	0.296	0.31
217%	70%	2,800	2830	0.372	0.342	0.36
248%	80%	3,200	3200	0.408	0.374	0.39
279%	90%	3,600	3600	0.46	0.424	0.44
310%	100%	4,000	4040	0.532	0.492	0.51
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6280	1.172	1.128	1.15
0%	0%	0	0	0.428	0.562	0.50

Load at 1.0-in lateral deflection: **5,753** **lb**

Residual lateral deformation: **0.50** **in**



Notes:

Failure reached at 6,280 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-50A

Axial Uplift Load Test Results

PTL - **50** Designation: **A**

Section: W6x9

Latitude: 33.650638

Longitude: -114.67017

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 11.35

Pile Damaged? No

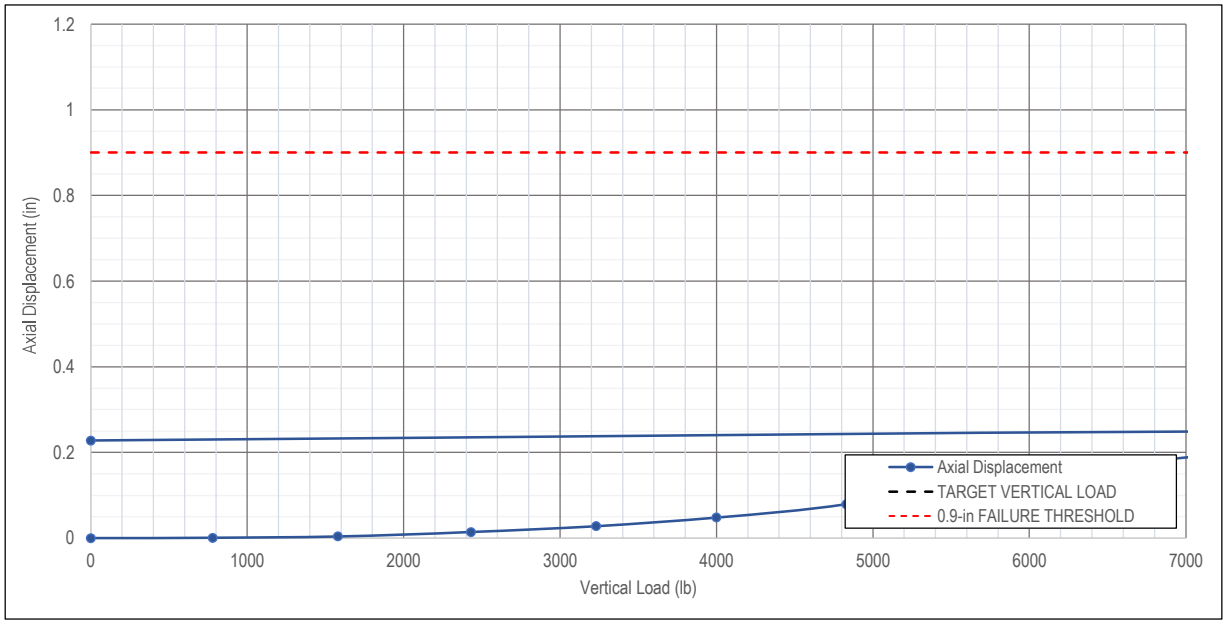
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	780	0	0.002	0.00
436%	20%	1,600	1580	0.004	0.004	0.00
654%	30%	2,400	2430	0.014	0.014	0.01
872%	40%	3,200	3230	0.028	0.028	0.03
1090%	50%	4,000	4000	0.048	0.048	0.05
1308%	60%	4,800	4830	0.08	0.078	0.08
1526%	70%	5,600	5640	0.136	0.132	0.13
1744%	80%	6,400	6440	0.172	0.166	0.17
1962%	90%	7,200	7200	0.2	0.194	0.20
2180%	100%	8,000	8030	0.252	0.244	0.25
0%	0%	0	0	0.23	0.226	0.23

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.23** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-50A

Lateral Load Test Results

PTL- **50** Designation: **A**

Section: W6x9

Latitude: 33.650638

Longitude: -114.67017

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.35

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

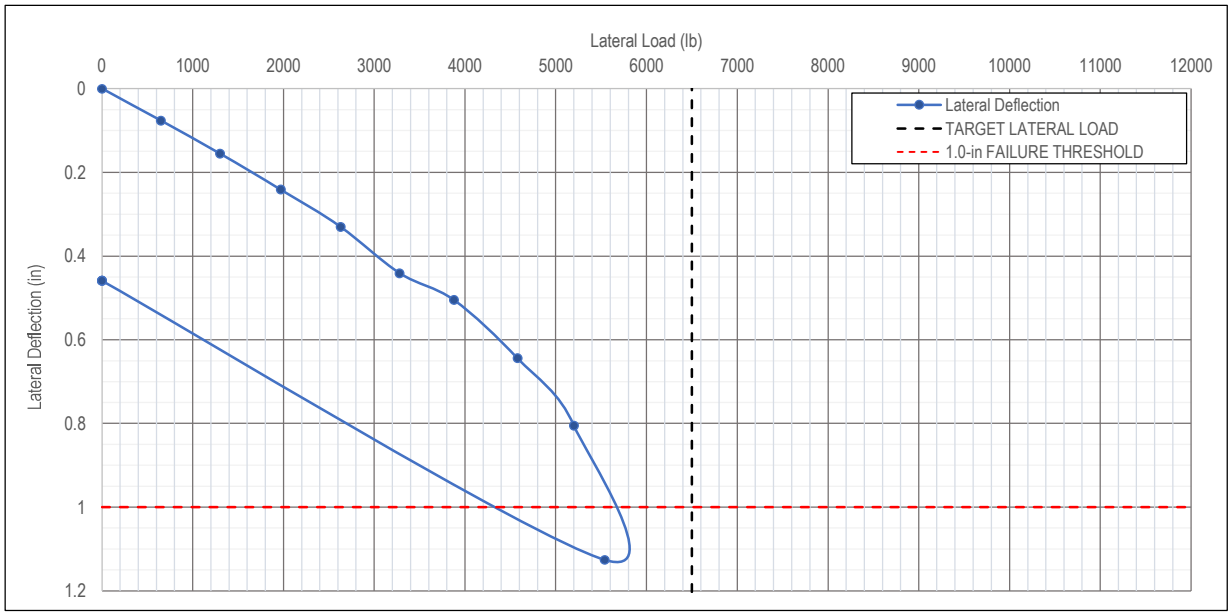
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.08	0.072	0.08
62%	20%	1,300	1300	0.164	0.146	0.16
93%	30%	1,950	1970	0.254	0.228	0.24
124%	40%	2,600	2630	0.346	0.314	0.33
155%	50%	3,250	3280	0.464	0.418	0.44
186%	60%	3,900	3880	0.532	0.478	0.51
217%	70%	4,550	4580	0.684	0.604	0.64
248%	80%	5,200	5200	0.87	0.74	0.81
279%	90%	5,850	5540	1.208	1.044	1.13
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.572	0.346	0.46

Load at 1.0-in lateral deflection: **5,407** **lb**

Residual lateral deformation: **0.46** **in**



Notes:

Failure reached at 5,540 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-50B

Axial Uplift Load Test Results

PTL - **50** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.650638 - - Saturated Soil? No

Longitude: -114.67017 Target Embedment Depth (ft): 8.00 Drive Time (s): 15.53

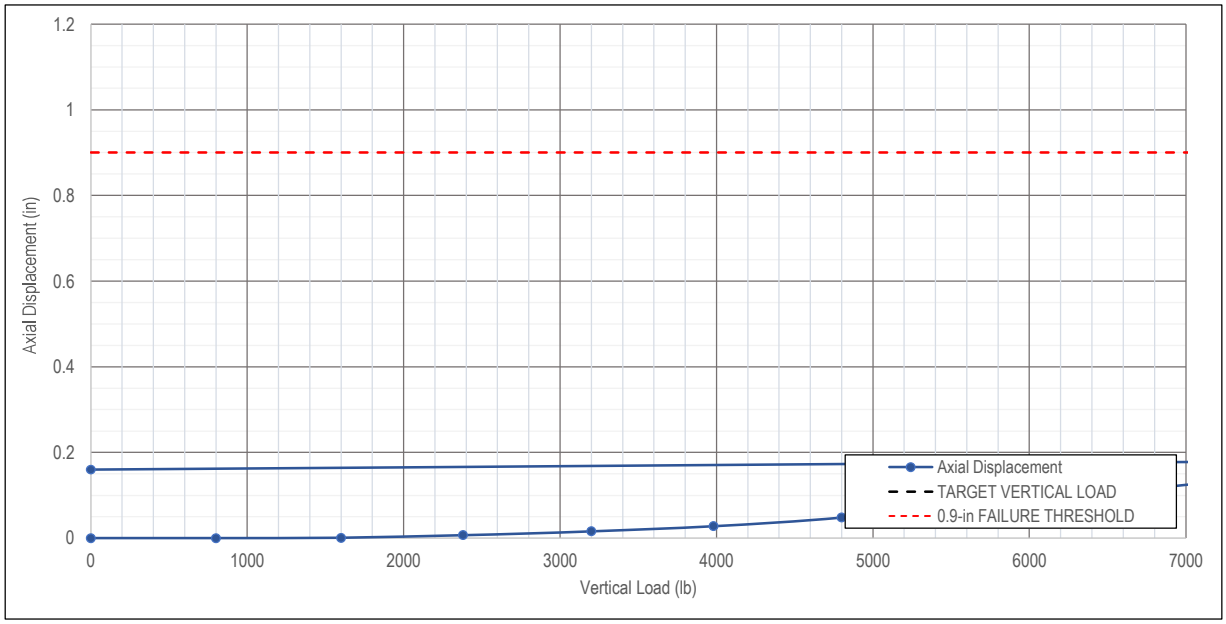
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/28/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0	0	0.00
436%	20%	1,600	1600	0	0.002	0.00
654%	30%	2,400	2380	0.006	0.008	0.01
872%	40%	3,200	3200	0.016	0.016	0.02
1090%	50%	4,000	3980	0.028	0.028	0.03
1308%	60%	4,800	4800	0.048	0.048	0.05
1526%	70%	5,600	5630	0.084	0.084	0.08
1744%	80%	6,400	6430	0.104	0.104	0.10
1962%	90%	7,200	7220	0.134	0.134	0.13
2180%	100%	8,000	8030	0.176	0.178	0.18
0%	0%	0	0	0.16	0.16	0.16

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.16** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-50B

Lateral Load Test Results

PTL- **50** Designation: **B**

Section: W6x9

Latitude: 33.650638

Longitude: -114.67017

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 15.53

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

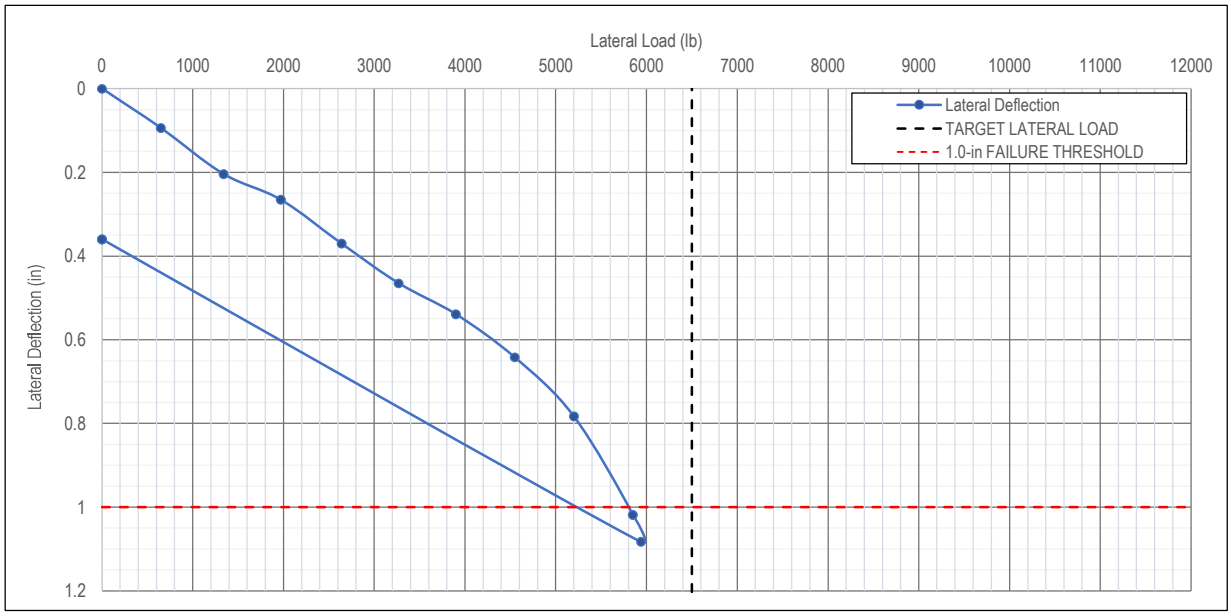
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.094	0.094	0.09
62%	20%	1,300	1340	0.208	0.2	0.20
93%	30%	1,950	1970	0.27	0.26	0.27
124%	40%	2,600	2640	0.378	0.362	0.37
155%	50%	3,250	3270	0.476	0.454	0.47
186%	60%	3,900	3900	0.554	0.524	0.54
217%	70%	4,550	4550	0.662	0.622	0.64
248%	80%	5,200	5200	0.814	0.752	0.78
279%	90%	5,850	5850	1.08	0.956	1.02
310%	100%	6,500	5940	1.16	1.006	1.08
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.406	0.314	0.36

Load at 1.0-in lateral deflection: **5,800** **lb**

Residual lateral deformation: **0.36** **in**



Notes:

Failure reached at 5,850 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-51A

Axial Uplift Load Test Results

PTL - **51** Designation: **A**

Section: W6x9

Latitude: 33.648015

Longitude: -114.71432

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 6.48

Pile Damaged? No

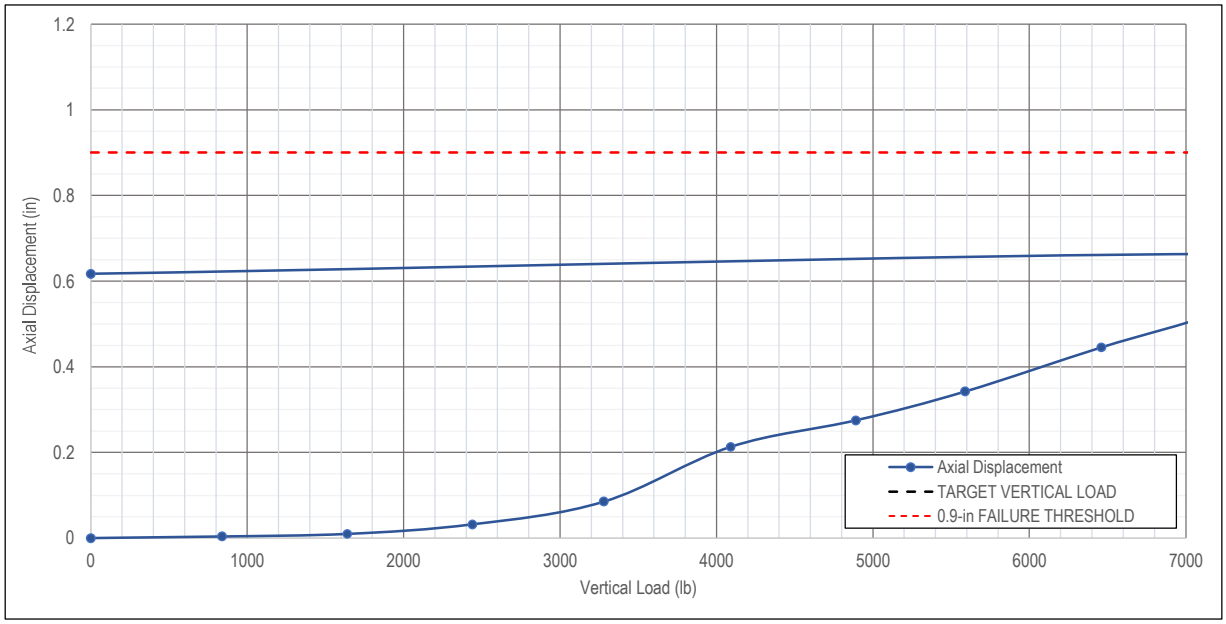
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	840	0.003	0.005	0.00
13594%	20%	1,600	1640	0.008	0.012	0.01
20391%	30%	2,400	2440	0.0295	0.0345	0.03
27188%	40%	3,200	3280	0.0825	0.088	0.09
33985%	50%	4,000	4090	0.2105	0.216	0.21
40782%	60%	4,800	4890	0.272	0.2775	0.27
47579%	70%	5,600	5590	0.3395	0.3455	0.34
54376%	80%	6,400	6460	0.443	0.448	0.45
61173%	90%	7,200	7230	0.527	0.5325	0.53
67970%	100%	8,000	8090	0.6585	0.663	0.66
0%	0%	0		0.617	0.6175	0.62

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.62** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-51A

Lateral Load Test Results

PTL- **51** Designation: **A**

Section: W6x9

Latitude: 33.648015

Longitude: -114.71432

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 6.48

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

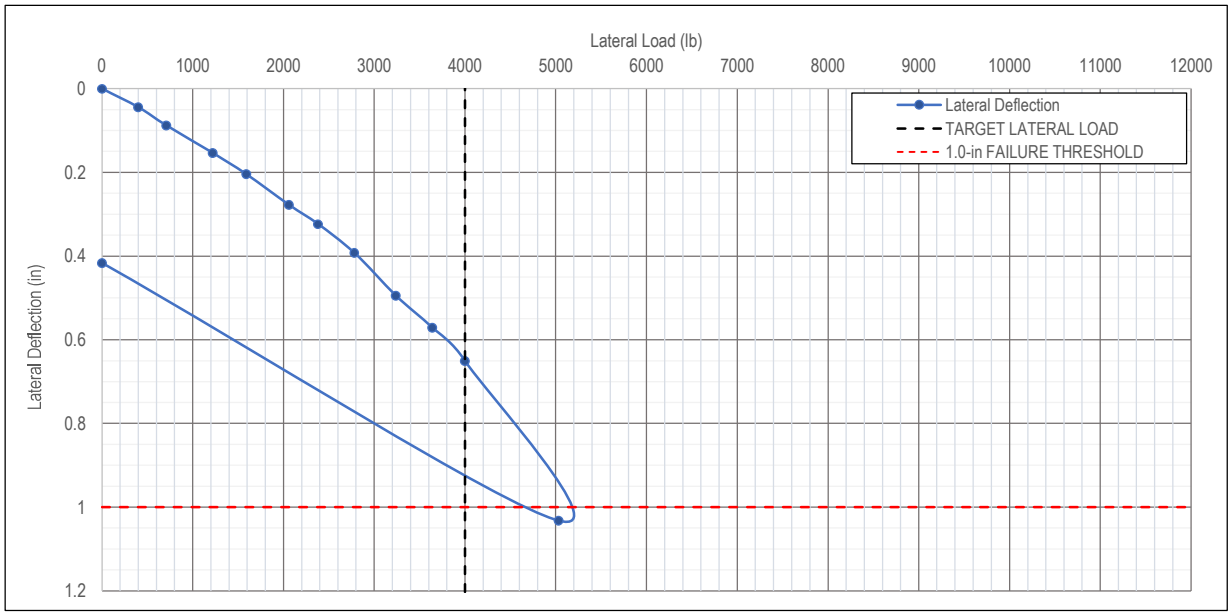
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.047	0.041	0.04
62%	20%	800	710	0.0815	0.0945	0.09
93%	30%	1,200	1220	0.142	0.1655	0.15
124%	40%	1,600	1590	0.1885	0.2195	0.20
155%	50%	2,000	2060	0.2565	0.2985	0.28
186%	60%	2,400	2380	0.299	0.348	0.32
217%	70%	2,800	2780	0.362	0.422	0.39
248%	80%	3,200	3240	0.4565	0.5325	0.49
279%	90%	3,600	3640	0.5265	0.615	0.57
310%	100%	4,000	4000	0.599	0.702	0.65
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5030	0.919	1.1445	1.03
0%	0%	0	0	0.3745	0.4585	0.42

Load at 1.0-in lateral deflection: **4,944** **lb**

Residual lateral deformation: **0.42** **in**



Notes:

Failure reached at 5,030 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-51B

Axial Uplift Load Test Results

PTL - **51** Designation: **B**

Section: W6x9

Latitude: 33.648015

Longitude: -114.71432

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 10.4

Pile Damaged? No

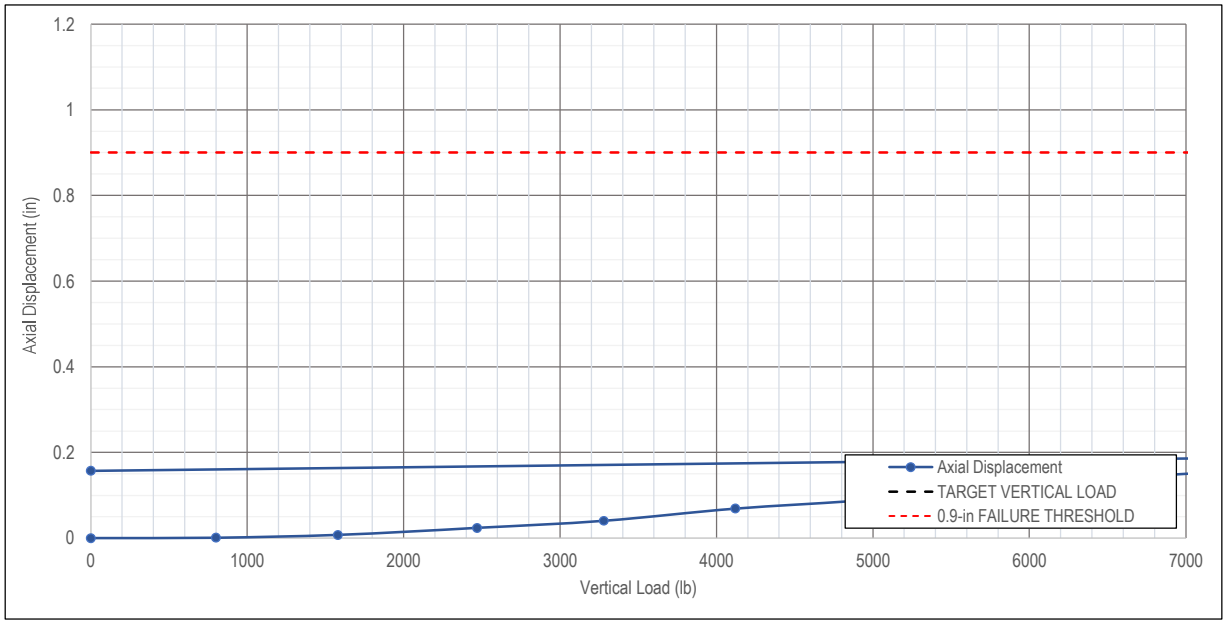
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	800	0.0015	0.001	0.00
13594%	20%	1,600	1580	0.009	0.006	0.01
20391%	30%	2,400	2470	0.027	0.0205	0.02
27188%	40%	3,200	3280	0.045	0.036	0.04
33985%	50%	4,000	4120	0.0745	0.0635	0.07
40782%	60%	4,800	4880	0.093	0.08	0.09
47579%	70%	5,600	5680	0.1145	0.1015	0.11
54376%	80%	6,400	6540	0.1485	0.1315	0.14
61173%	90%	7,200	7190	0.161	0.149	0.16
67970%	100%	8,000	8010	0.197	0.1785	0.19
0%	0%	0	0	0.1575	0.1565	0.16

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.16** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-51B

Lateral Load Test Results

PTL- **51** Designation: **B**

Section: W6x9

Latitude: 33.648015

Longitude: -114.71432

Tested By: ANS

Test Date: 10/29/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 10.4

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

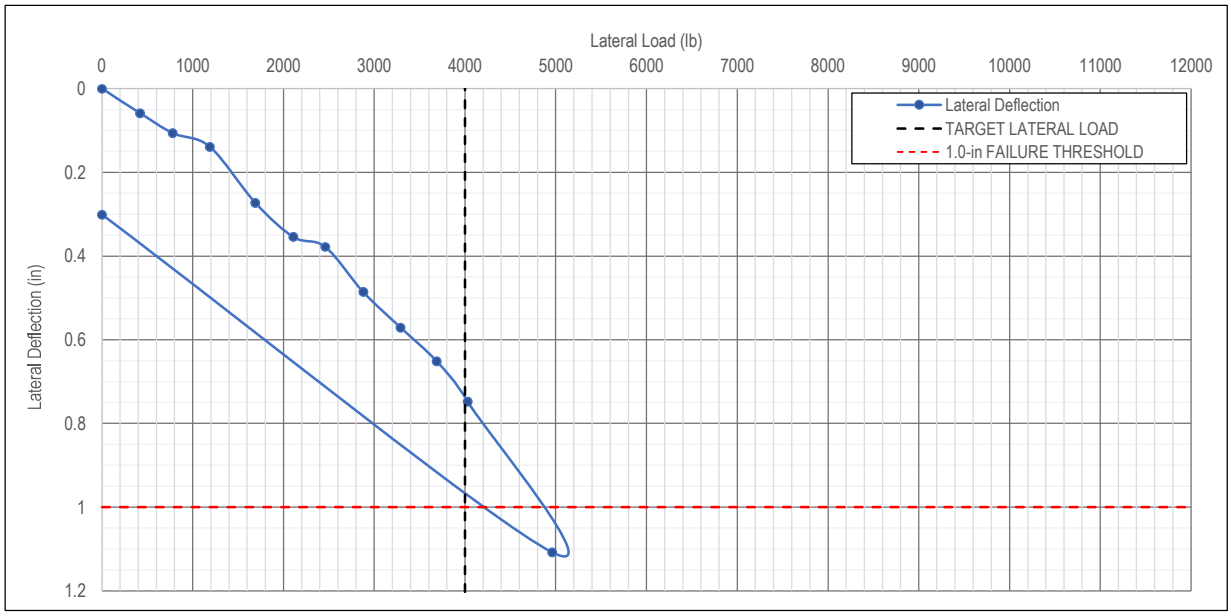
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	420	0.0575	0.06	0.06
62%	20%	800	780	0.101	0.111	0.11
93%	30%	1,200	1190	0.103	0.175	0.14
124%	40%	1,600	1690	0.261	0.2845	0.27
155%	50%	2,000	2110	0.3375	0.371	0.35
186%	60%	2,400	2460	0.361	0.395	0.38
217%	70%	2,800	2880	0.467	0.5045	0.49
248%	80%	3,200	3290	0.547	0.595	0.57
279%	90%	3,600	3690	0.6265	0.677	0.65
310%	100%	4,000	4030	0.718	0.777	0.75
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			4960	1.0365	1.179	1.11
0%	0%	0	0	0.275	0.327	0.30

Load at 1.0-in lateral deflection: **4,682** **lb**

Residual lateral deformation: **0.30** **in**



Notes:

Failure reached at 4,960 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-51C

Axial Compression Load Test Results (supplemental pile)

PTL - **51** Designation: **C**

Section: W6x9

Latitude: 33.648015

Longitude: -114.71432

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **15,000** (≈ 1180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 5.46

Pile Damaged? No

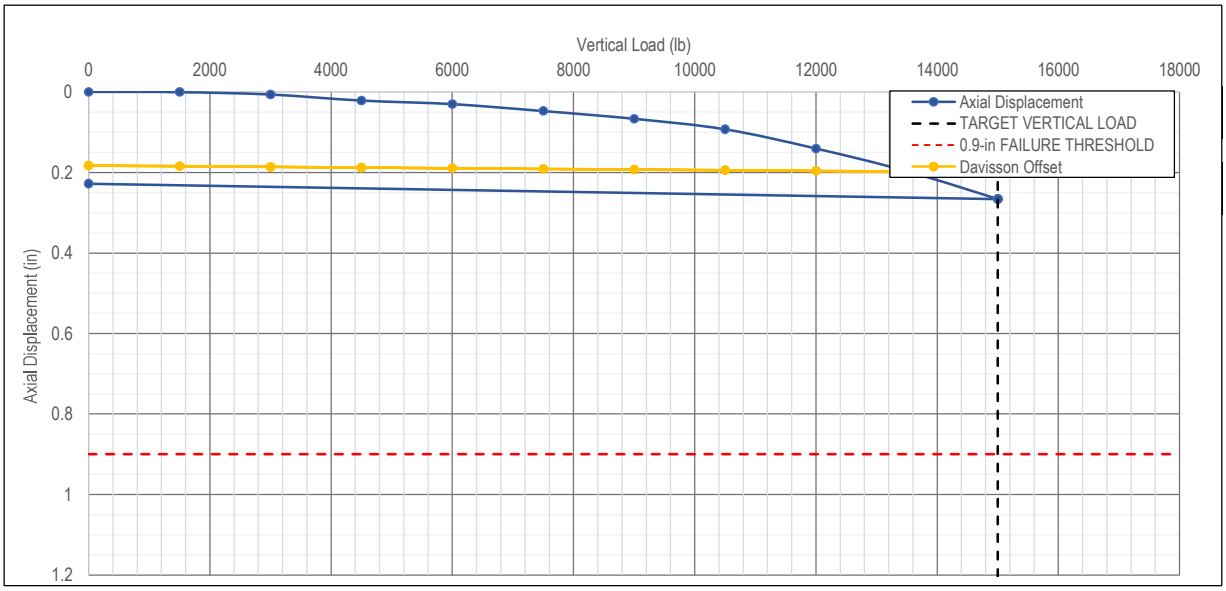
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL COMPRESSION** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
118%	10%	1,500	1500	0.0005	0	0.00
236%	20%	3,000	3000	0.0075	0.0055	0.01
354%	30%	4,500	4500	0.023	0.02	0.02
472%	40%	6,000	6000	0.0325	0.028	0.03
590%	50%	7,500	7500	0.0505	0.044	0.05
708%	60%	9,000	9000	0.0695	0.064	0.07
826%	70%	10,500	10500	0.097	0.088	0.09
944%	80%	12,000	12000	0.1455	0.136	0.14
1062%	90%	13,500	13500	0.202	0.1915	0.20
1180%	100%	15,000	15000	0.272	0.26	0.27
0%	0%	0	0	0.229	0.227	0.23

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.23** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-52A

Axial Uplift Load Test Results

PTL - **52** Designation: **A**

Section: W6x9

Latitude: 33.647963

Longitude: -114.70809

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 12.99

Pile Damaged? No

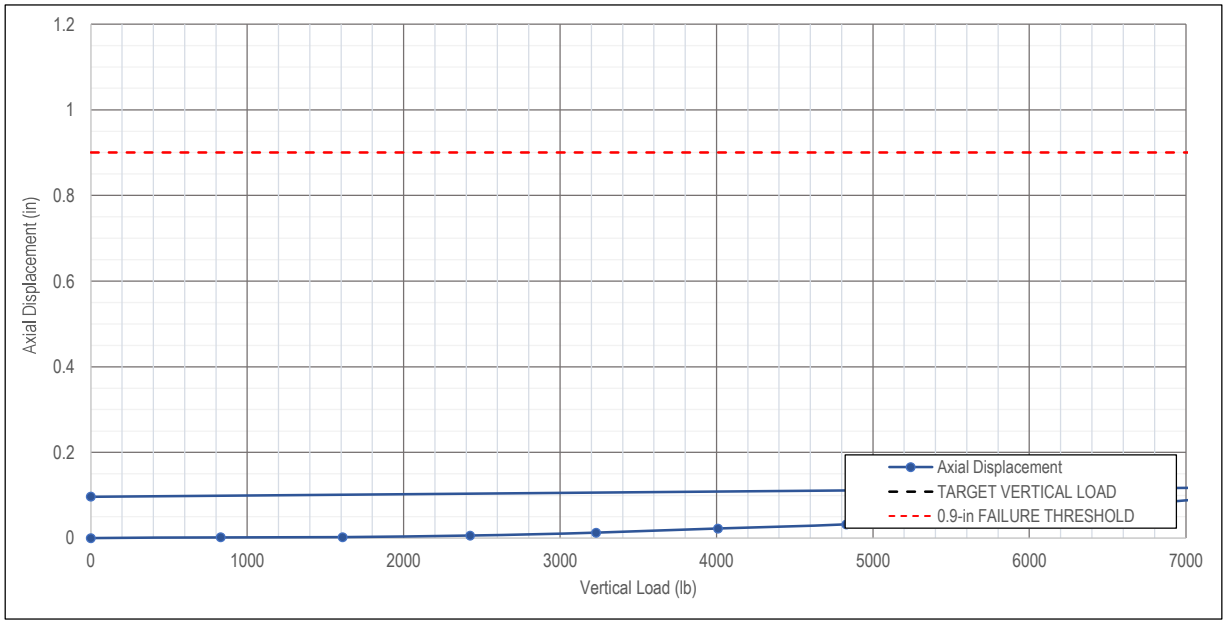
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	830	0.0025	0.001	0.00
436%	20%	1,600	1610	0.004	0.001	0.00
654%	30%	2,400	2425	0.0085	0.0035	0.01
872%	40%	3,200	3230	0.0165	0.009	0.01
1090%	50%	4,000	4010	0.0265	0.0185	0.02
1308%	60%	4,800	4830	0.036	0.028	0.03
1526%	70%	5,600	5645	0.05	0.0425	0.05
1744%	80%	6,400	6430	0.0745	0.0655	0.07
1962%	90%	7,200	7240	0.101	0.091	0.10
2180%	100%	8,000	8000	0.1245	0.1135	0.12
0%	0%	0	0	0.0965	0.097	0.10

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.10** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-52A

Lateral Load Test Results

PTL- **52** Designation: **A**

Section: W6x9

Latitude: 33.647963

Longitude: -114.70809

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 12.99

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

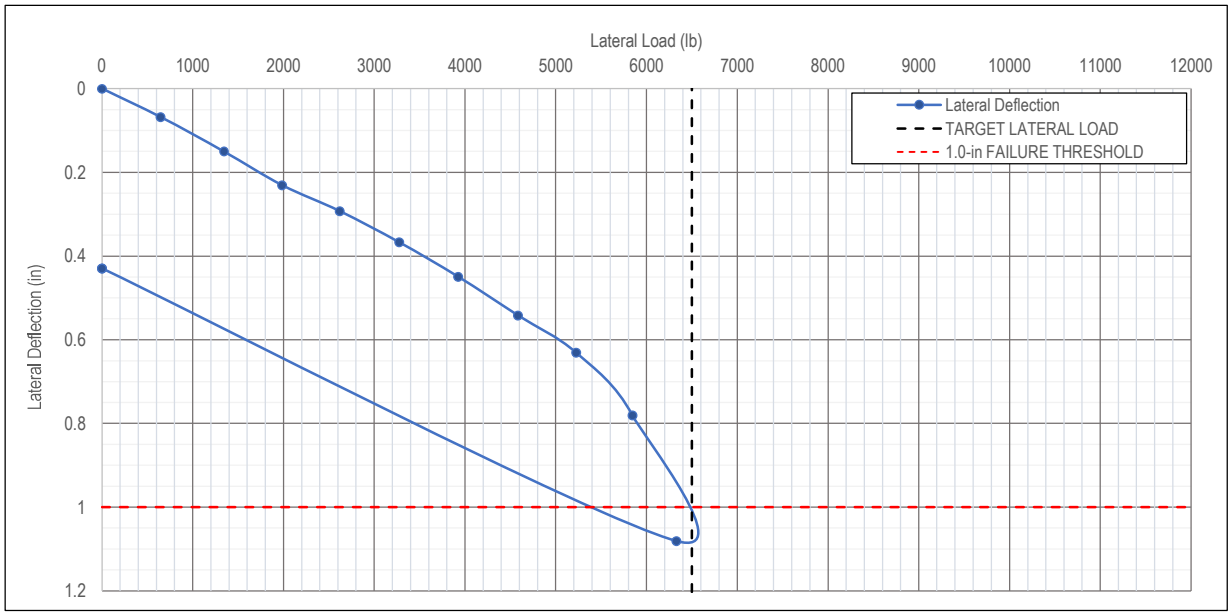
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	645	0.0715	0.0635	0.07
62%	20%	1,300	1345	0.153	0.147	0.15
93%	30%	1,950	1985	0.2355	0.2255	0.23
124%	40%	2,600	2620	0.2995	0.286	0.29
155%	50%	3,250	3275	0.3765	0.3575	0.37
186%	60%	3,900	3925	0.4595	0.4395	0.45
217%	70%	4,550	4585	0.5495	0.534	0.54
248%	80%	5,200	5225	0.6275	0.634	0.63
279%	90%	5,850	5845	0.7645	0.797	0.78
310%	100%	6,500	6330	1.058	1.1035	1.08
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.359	0.4995	0.43

Load at 1.0-in lateral deflection: **6,199** **lb**

Residual lateral deformation: **0.43** **in**



Notes:

Failure reached at 6,330 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-52B

Axial Uplift Load Test Results

PTL - **52** Designation: **B**

Section: W6x9

Latitude: 33.647963

Longitude: -114.70809

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 21.97

Pile Damaged? No

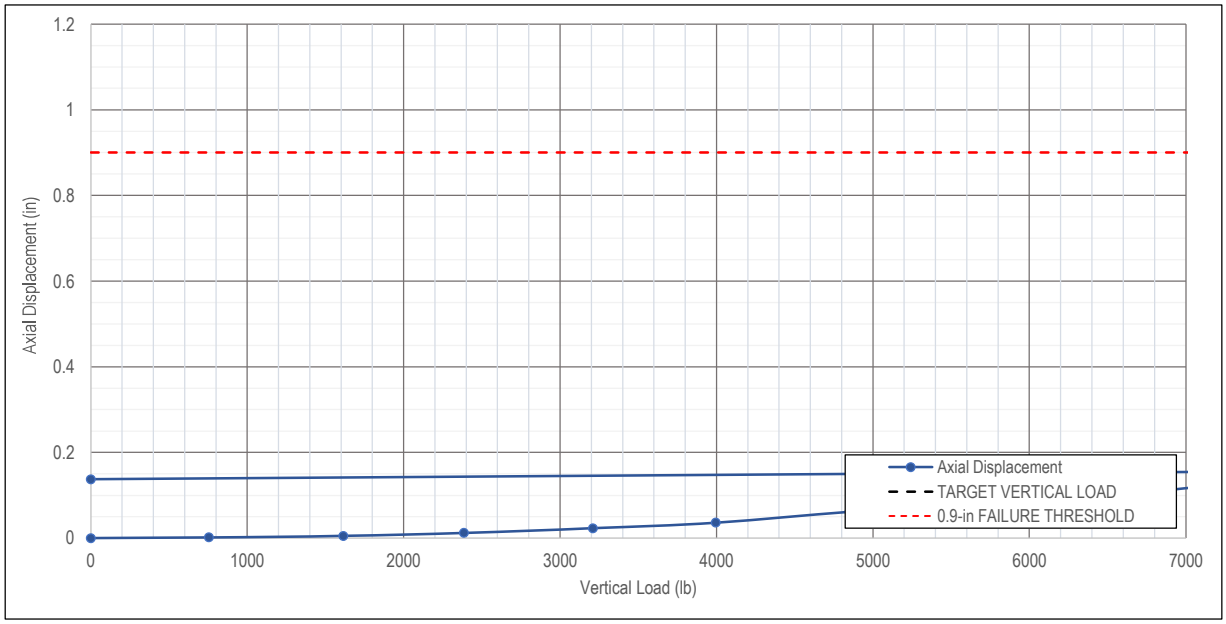
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	755	0.0025	0.001	0.00
436%	20%	1,600	1615	0.0055	0.005	0.01
654%	30%	2,400	2385	0.013	0.0115	0.01
872%	40%	3,200	3210	0.0245	0.0215	0.02
1090%	50%	4,000	3995	0.0375	0.0345	0.04
1308%	60%	4,800	4850	0.063	0.0605	0.06
1526%	70%	5,600	5645	0.0825	0.0805	0.08
1744%	80%	6,400	6525	0.101	0.0995	0.10
1962%	90%	7,200	7245	0.1265	0.125	0.13
2180%	100%	8,000	8040	0.1555	0.1545	0.16
0%	0%	0	0	0.138	0.137	0.14

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.14** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-52B

Lateral Load Test Results

PTL- **52** Designation: **B**

Section: W6x9

Latitude: 33.647963

Longitude: -114.70809

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 21.97

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

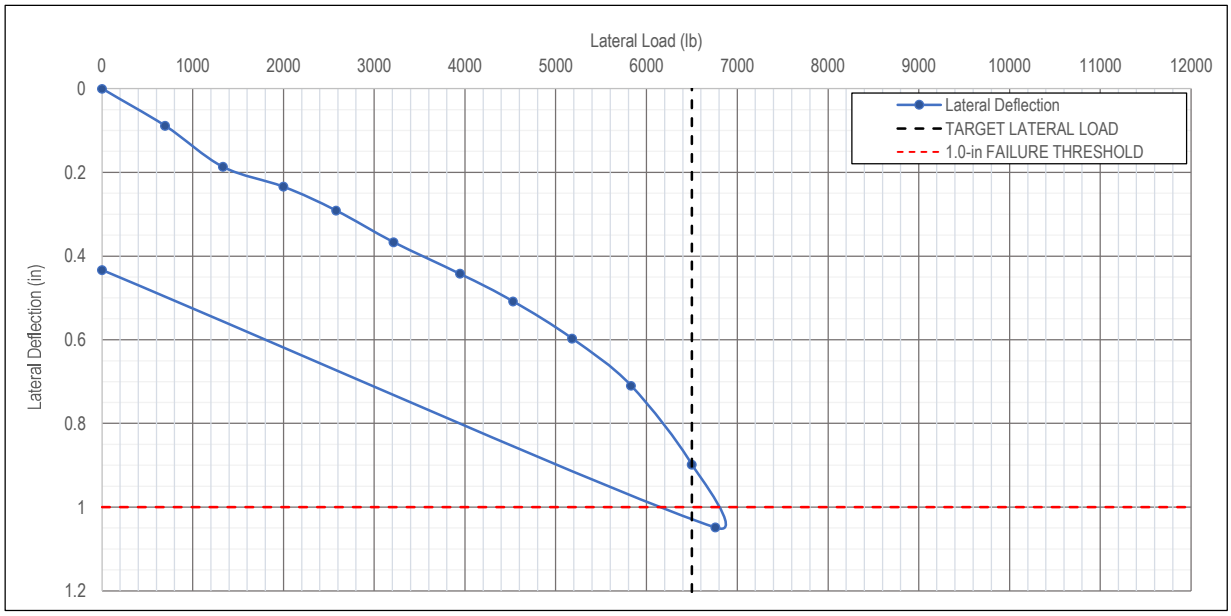
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	695	0.092	0.0855	0.09
62%	20%	1,300	1335	0.191	0.1825	0.19
93%	30%	1,950	2000	0.24	0.2285	0.23
124%	40%	2,600	2580	0.2985	0.283	0.29
155%	50%	3,250	3215	0.3765	0.3575	0.37
186%	60%	3,900	3945	0.454	0.4305	0.44
217%	70%	4,550	4530	0.521	0.495	0.51
248%	80%	5,200	5180	0.608	0.5855	0.60
279%	90%	5,850	5830	0.715	0.7045	0.71
310%	100%	6,500	6500	0.8785	0.9185	0.90
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6760	0.991	1.1065	1.05
0%	0%	0	0	0.375	0.4915	0.43

Load at 1.0-in lateral deflection: **6,676** **lb**

Residual lateral deformation: **0.43** **in**



Notes:

Failure reached at 6,760 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-53A

Axial Uplift Load Test Results

PTL - **53** Designation: **A**

Section: W6x9

Latitude: 33.649049

Longitude: -114.70495

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 8.57

Pile Damaged? No

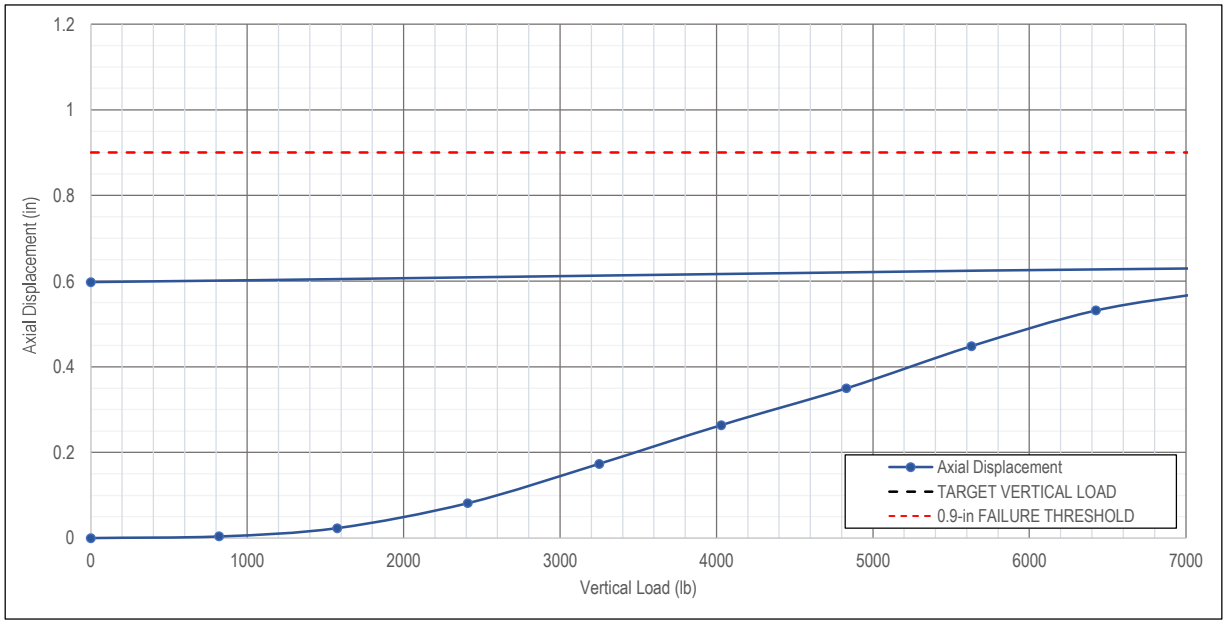
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	820	0.003	0.005	0.00
13594%	20%	1,600	1575	0.022	0.025	0.02
20391%	30%	2,400	2410	0.08	0.0835	0.08
27188%	40%	3,200	3250	0.1715	0.1755	0.17
33985%	50%	4,000	4030	0.2615	0.2665	0.26
40782%	60%	4,800	4830	0.347	0.3525	0.35
47579%	70%	5,600	5630	0.4455	0.451	0.45
54376%	80%	6,400	6425	0.528	0.535	0.53
61173%	90%	7,200	7215	0.576	0.5805	0.58
67970%	100%	8,000	8025	0.628	0.6325	0.63
0%	0%	0	0	0.597	0.598	0.60

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.60** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-53A

Lateral Load Test Results

PTL- **53** Designation: **A**

Section: W6x9

Latitude: 33.649049

Longitude: -114.70495

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 8.57

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

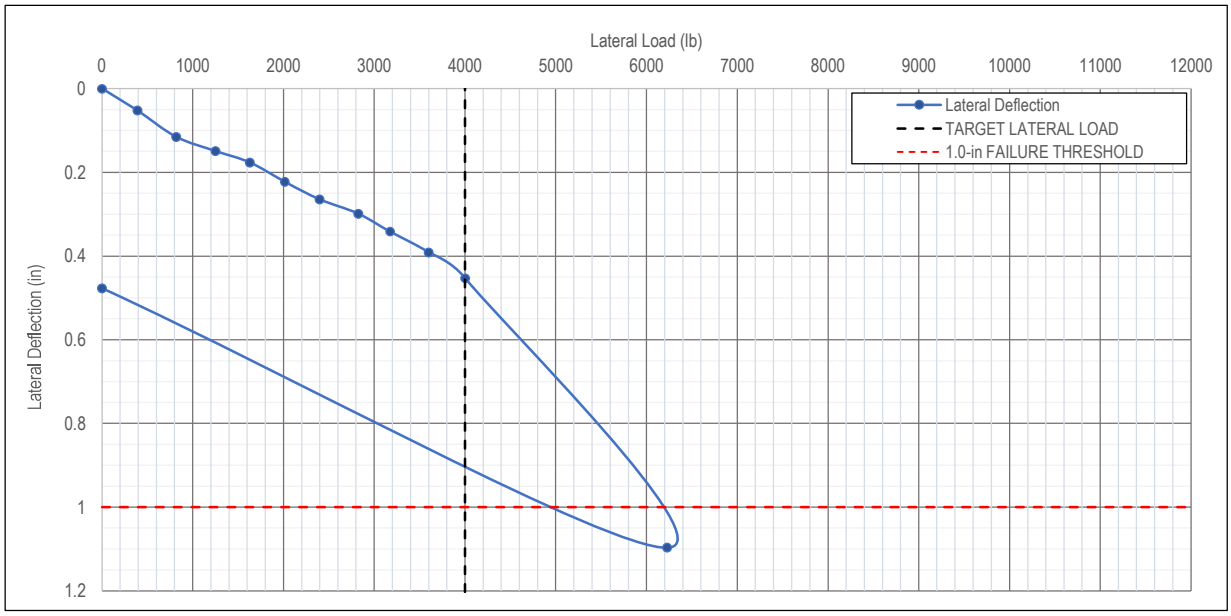
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	390	0.053	0.051	0.05
62%	20%	800	820	0.111	0.1195	0.12
93%	30%	1,200	1250	0.145	0.1525	0.15
124%	40%	1,600	1630	0.1725	0.18	0.18
155%	50%	2,000	2015	0.2195	0.2255	0.22
186%	60%	2,400	2400	0.262	0.267	0.26
217%	70%	2,800	2825	0.2965	0.301	0.30
248%	80%	3,200	3175	0.3395	0.343	0.34
279%	90%	3,600	3600	0.39	0.392	0.39
310%	100%	4,000	4005	0.4525	0.4535	0.45
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6230	1.045	1.1485	1.10
0%	0%	0	0	0.4345	0.5195	0.48

Load at 1.0-in lateral deflection: **5,896** **lb**

Residual lateral deformation: **0.48** **in**



Notes:

Failure reached at 6,230 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-53B

Axial Uplift Load Test Results

PTL - **53** Designation: **B**

Section: W6x9

Latitude: 33.649049

Longitude: -114.70495

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 12.58

Pile Damaged? No

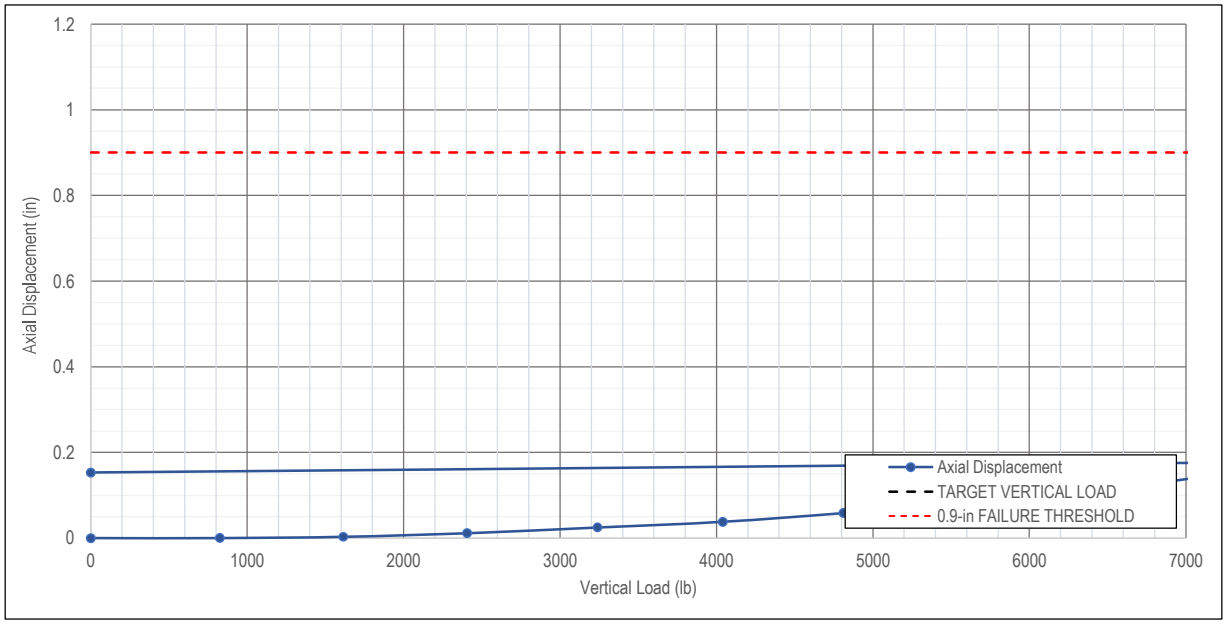
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	825	0.0005	0	0.00
13594%	20%	1,600	1615	0.0035	0.0025	0.00
20391%	30%	2,400	2405	0.011	0.0125	0.01
27188%	40%	3,200	3240	0.0235	0.0265	0.03
33985%	50%	4,000	4040	0.0365	0.0395	0.04
40782%	60%	4,800	4810	0.0575	0.06	0.06
47579%	70%	5,600	5625	0.082	0.0845	0.08
54376%	80%	6,400	6415	0.107	0.109	0.11
61173%	90%	7,200	7130	0.1435	0.1435	0.14
67970%	100%	8,000	8035	0.177	0.1765	0.18
0%	0%	0	0	0.153	0.153	0.15

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.15** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-53B

Lateral Load Test Results

PTL- **53** Designation: **B**

Section: W6x9

Latitude: 33.649049

Longitude: -114.70495

Tested By: ANS

Test Date: 10/21/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 12.58

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

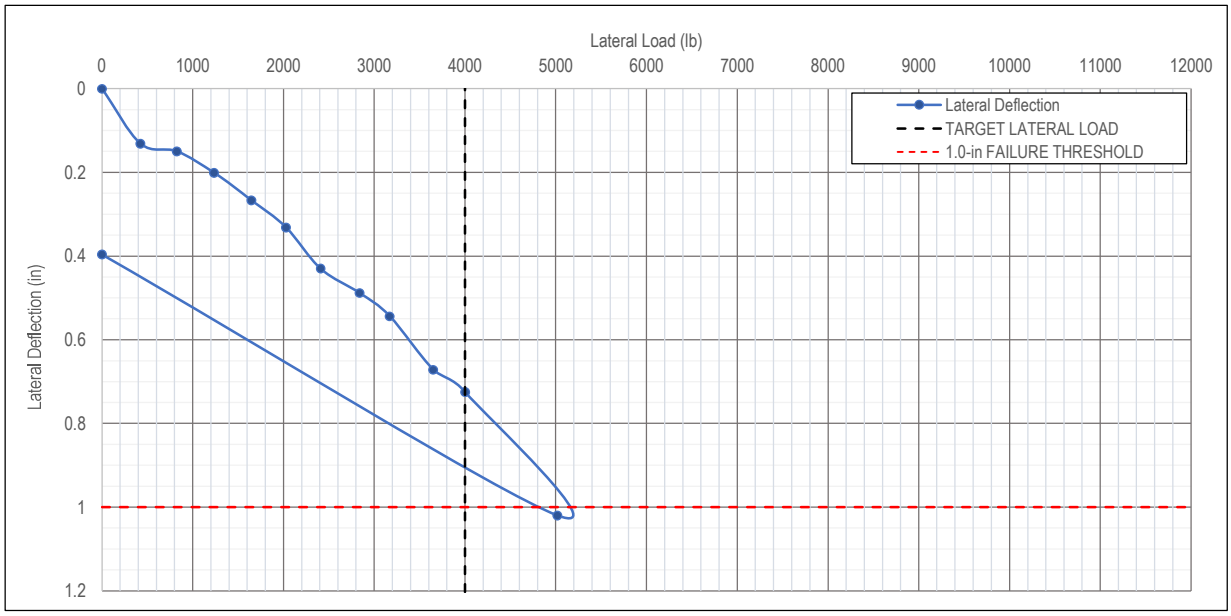
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	425	0.127	0.1355	0.13
62%	20%	800	825	0.1455	0.154	0.15
93%	30%	1,200	1235	0.197	0.2045	0.20
124%	40%	1,600	1645	0.2625	0.27	0.27
155%	50%	2,000	2030	0.3285	0.3345	0.33
186%	60%	2,400	2410	0.425	0.435	0.43
217%	70%	2,800	2840	0.486	0.49	0.49
248%	80%	3,200	3170	0.542	0.5455	0.54
279%	90%	3,600	3650	0.6715	0.6715	0.67
310%	100%	4,000	4005	0.7255	0.725	0.73
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5020	1.01	1.0295	1.02
0%	0%	0	0	0.3835	0.409	0.40

Load at 1.0-in lateral deflection: **4,952** **lb**

Residual lateral deformation: **0.40** **in**



Notes:

Failure reached at 5,020 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-54A

Axial Uplift Load Test Results

PTL - **54** Designation: **A**

Section: W6x9

Latitude: 33.647924

Longitude: -114.69925

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 13.42

Pile Damaged? No

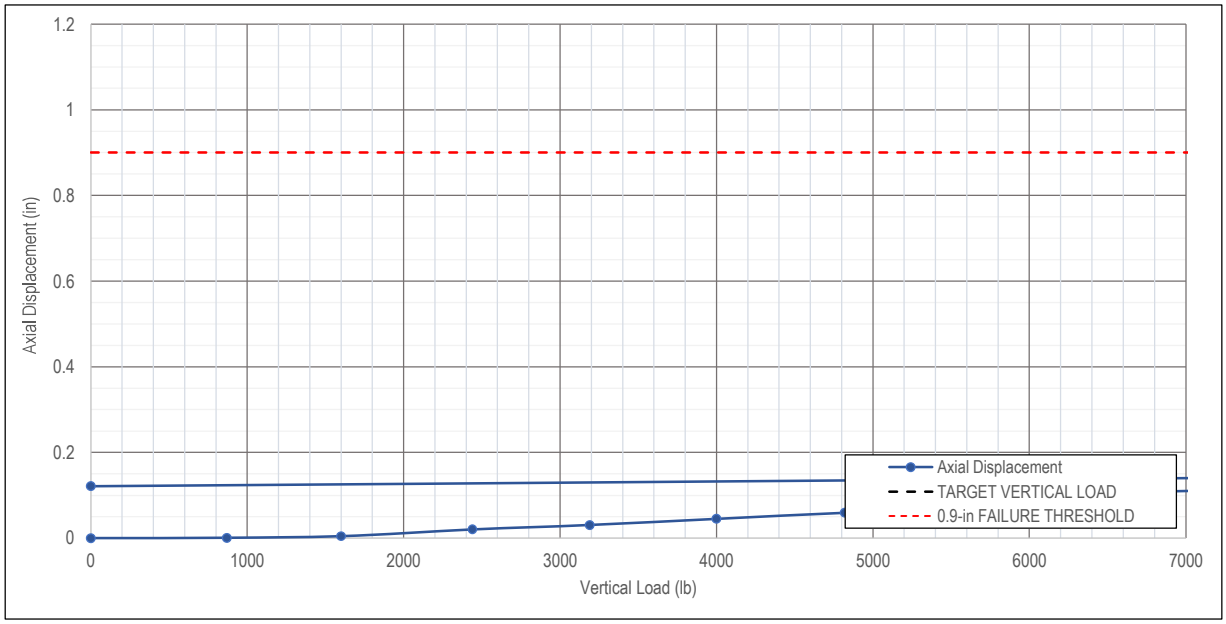
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	870	0.001	0.001	0.00
436%	20%	1,600	1600	0.0045	0.005	0.00
654%	30%	2,400	2440	0.0195	0.0215	0.02
872%	40%	3,200	3190	0.029	0.032	0.03
1090%	50%	4,000	4000	0.0435	0.047	0.05
1308%	60%	4,800	4820	0.0575	0.0615	0.06
1526%	70%	5,600	5590	0.0715	0.0765	0.07
1744%	80%	6,400	6420	0.101	0.1085	0.10
1962%	90%	7,200	7230	0.1095	0.117	0.11
2180%	100%	8,000	8040	0.1395	0.1425	0.14
0%	0%	0	0	0.1215	0.1215	0.12

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.12** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-54A

Lateral Load Test Results

PTL- **54** Designation: **A**

Section: W6x9

Latitude: 33.647924

Longitude: -114.69925

Tested By: ANS

Test Date: 10/29/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 13.42

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

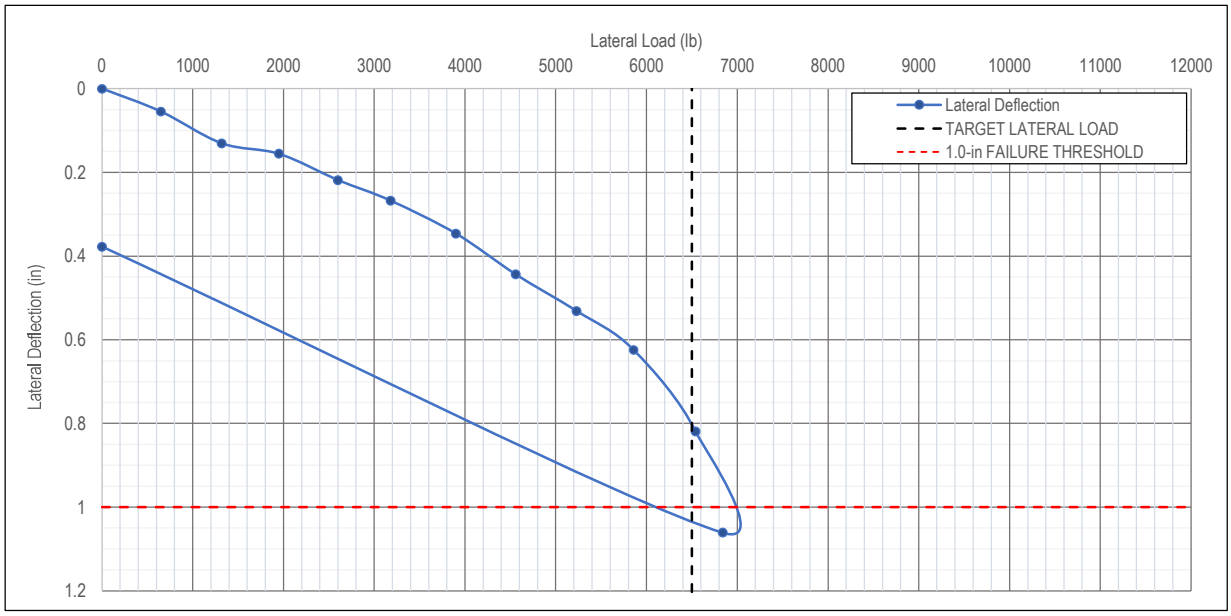
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.0545	0.0545	0.05
62%	20%	1,300	1320	0.1325	0.1285	0.13
93%	30%	1,950	1950	0.158	0.153	0.16
124%	40%	2,600	2600	0.2225	0.214	0.22
155%	50%	3,250	3180	0.273	0.2615	0.27
186%	60%	3,900	3900	0.3545	0.338	0.35
217%	70%	4,550	4560	0.455	0.4325	0.44
248%	80%	5,200	5230	0.548	0.5145	0.53
279%	90%	5,850	5860	0.651	0.5975	0.62
310%	100%	6,500	6540	0.8845	0.7535	0.82
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6840	1.06	1.062	1.06
0%	0%	0	0	0.408	0.3475	0.38

Load at 1.0-in lateral deflection: **6,764** **lb**

Residual lateral deformation: **0.38** **in**



Notes:

Failure reached at 6,840 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-54B

Axial Uplift Load Test Results

PTL - **54** Designation: **B**

Section: W6x9

Latitude: 33.647924

Longitude: -114.69925

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 17.19

Pile Damaged? No

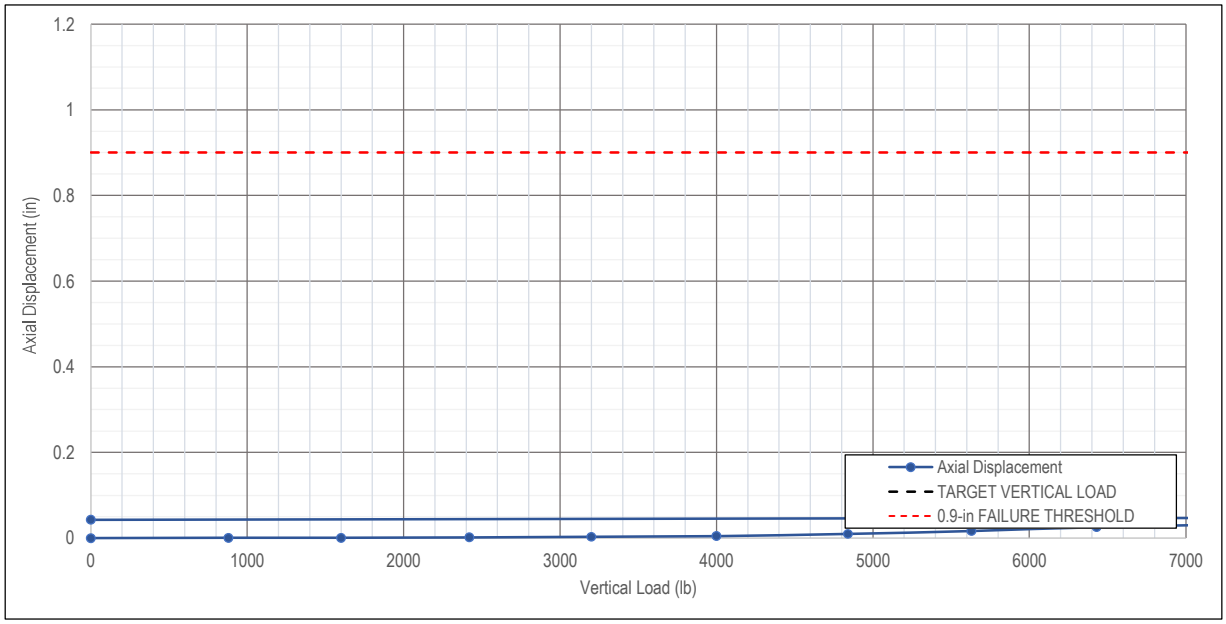
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	880	0.001	0.001	0.00
436%	20%	1,600	1600	0.001	0.001	0.00
654%	30%	2,400	2420	0.0015	0.002	0.00
872%	40%	3,200	3200	0.003	0.0035	0.00
1090%	50%	4,000	4000	0.0045	0.0055	0.01
1308%	60%	4,800	4840	0.0095	0.01	0.01
1526%	70%	5,600	5630	0.016	0.0175	0.02
1744%	80%	6,400	6430	0.0245	0.027	0.03
1962%	90%	7,200	7200	0.0305	0.034	0.03
2180%	100%	8,000	8040	0.0445	0.049	0.05
0%	0%	0	0	0.047	0.039	0.04

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.04** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-54B

Lateral Load Test Results

PTL- **54** Designation: **B**

Section: W6x9

Latitude: 33.647924

Longitude: -114.69925

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 17.19

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

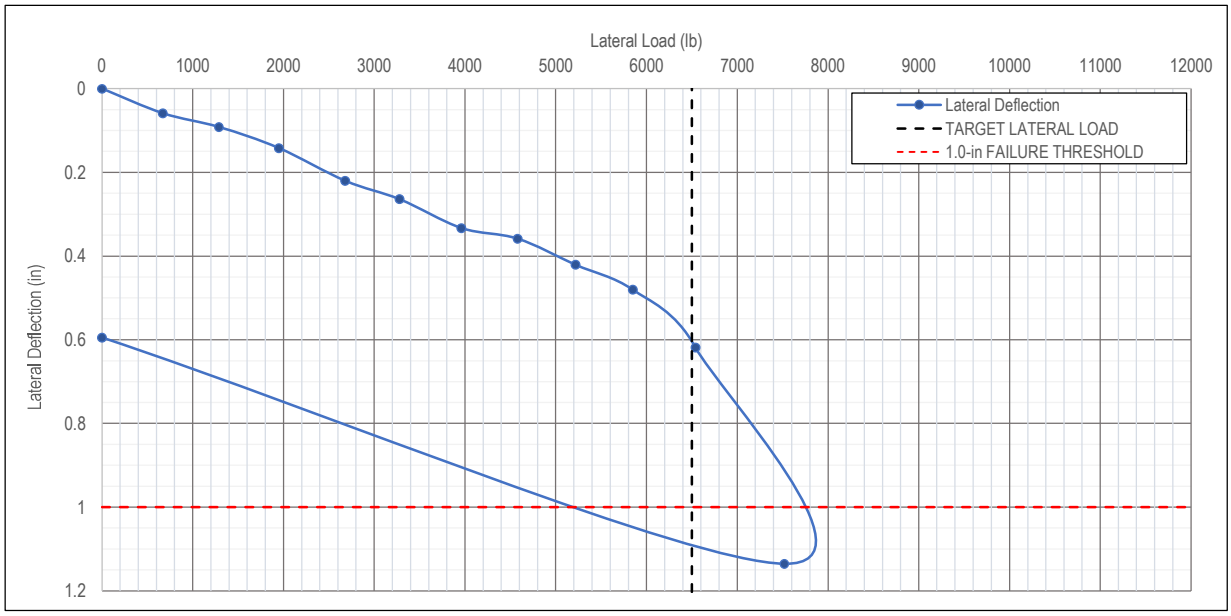
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	670	0.059	0.0585	0.06
62%	20%	1,300	1290	0.092	0.091	0.09
93%	30%	1,950	1950	0.1435	0.141	0.14
124%	40%	2,600	2680	0.223	0.217	0.22
155%	50%	3,250	3280	0.268	0.26	0.26
186%	60%	3,900	3960	0.338	0.3275	0.33
217%	70%	4,550	4580	0.3645	0.3525	0.36
248%	80%	5,200	5220	0.4295	0.412	0.42
279%	90%	5,850	5850	0.4915	0.4695	0.48
310%	100%	6,500	6540	0.641	0.5955	0.62
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7520	1.2515	1.0185	1.14
0%	0%	0	0	0.6405	0.549	0.59

Load at 1.0-in lateral deflection: **7,264** **lb**

Residual lateral deformation: **0.59** **in**



Notes:

Failure reached at 7,520 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-54D

Axial Uplift Load Test Results

PTL - **54** Designation: **D**

Section: **W6x12**

Latitude: 33.647924

Longitude: -114.69925

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 21.37

Pile Damaged? No

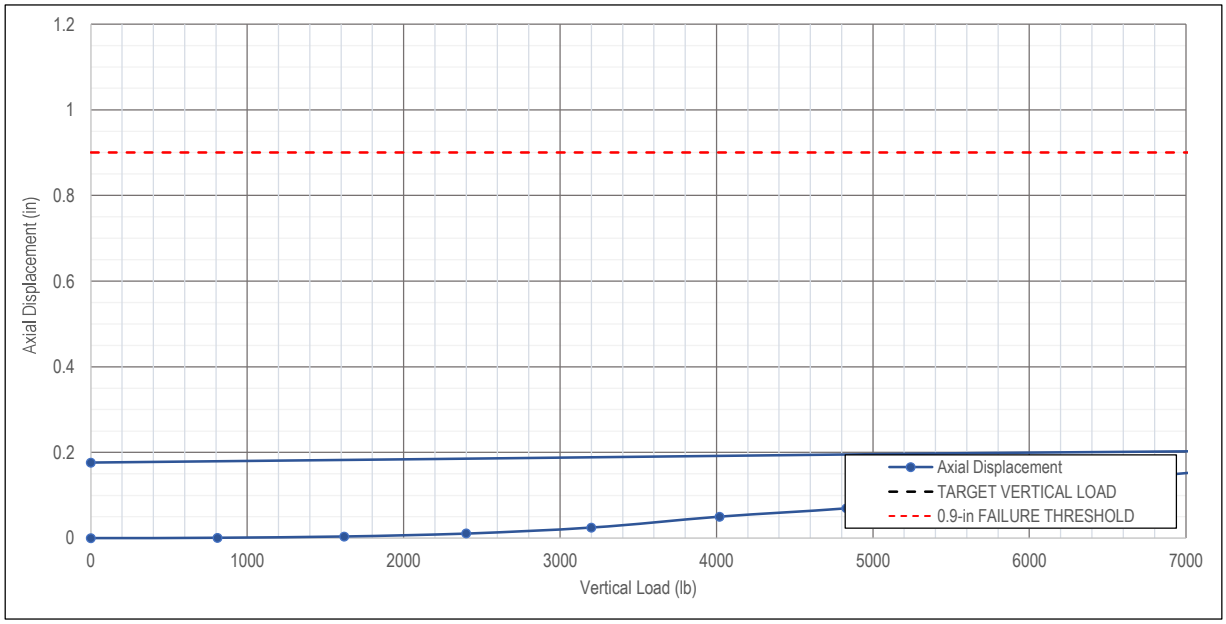
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	810	0.001	0.001	0.00
436%	20%	1,600	1620	0.0035	0.004	0.00
654%	30%	2,400	2400	0.01	0.012	0.01
872%	40%	3,200	3200	0.023	0.026	0.02
1090%	50%	4,000	4020	0.048	0.052	0.05
1308%	60%	4,800	4830	0.0675	0.0715	0.07
1526%	70%	5,600	5610	0.0925	0.097	0.09
1744%	80%	6,400	6430	0.1315	0.1375	0.13
1962%	90%	7,200	7200	0.1565	0.1615	0.16
2180%	100%	8,000	8010	0.201	0.206	0.20
0%	0%	0	0	0.1755	0.177	0.18

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.18** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-54D

Lateral Load Test Results

PTL- **54** Designation: **D**

Section: **W6x12**

Latitude: 33.647924

Longitude: -114.69925

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 21.37

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

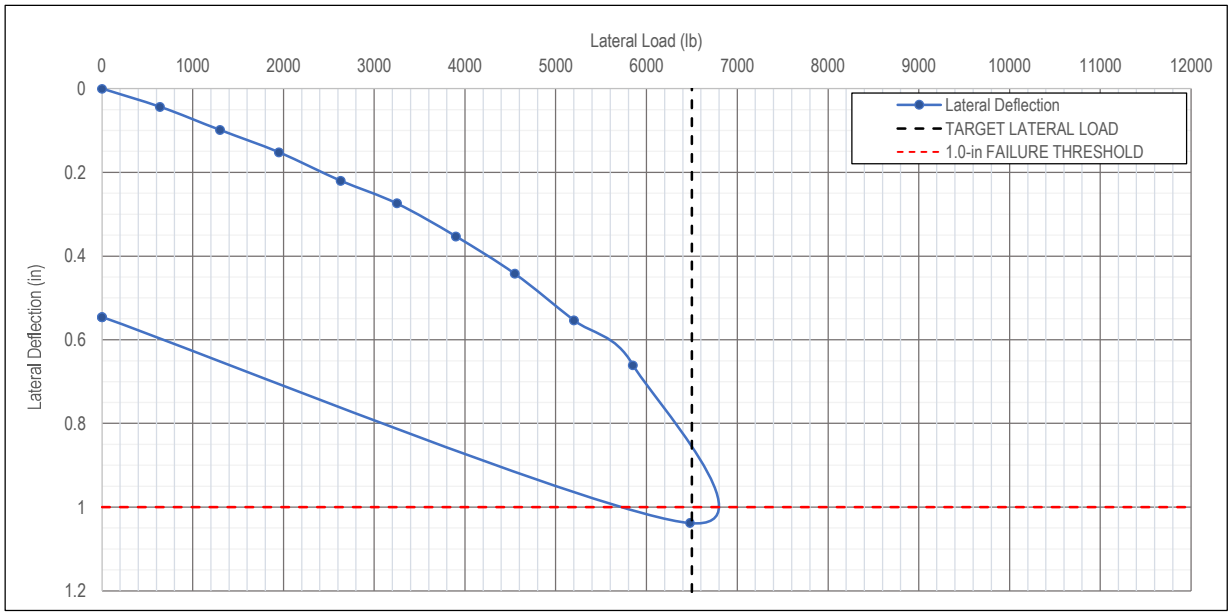
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	640	0.044	0.0425	0.04
62%	20%	1,300	1300	0.1015	0.0955	0.10
93%	30%	1,950	1950	0.157	0.147	0.15
124%	40%	2,600	2630	0.2275	0.212	0.22
155%	50%	3,250	3250	0.284	0.264	0.27
186%	60%	3,900	3900	0.3655	0.3405	0.35
217%	70%	4,550	4550	0.4575	0.427	0.44
248%	80%	5,200	5200	0.571	0.536	0.55
279%	90%	5,850	5850	0.6845	0.637	0.66
310%	100%	6,500	6480	1.0955	0.9805	1.04
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.5785	0.513	0.55

Load at 1.0-in lateral deflection: **6,417** **lb**

Residual lateral deformation: **0.55** **in**



Notes:

Failure reached at 6,480 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-55A

Axial Uplift Load Test Results

PTL - **55** Designation: **A**

Section: W6x9

Latitude: 33.648118

Longitude: -114.69491

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 6.55

Pile Damaged? No

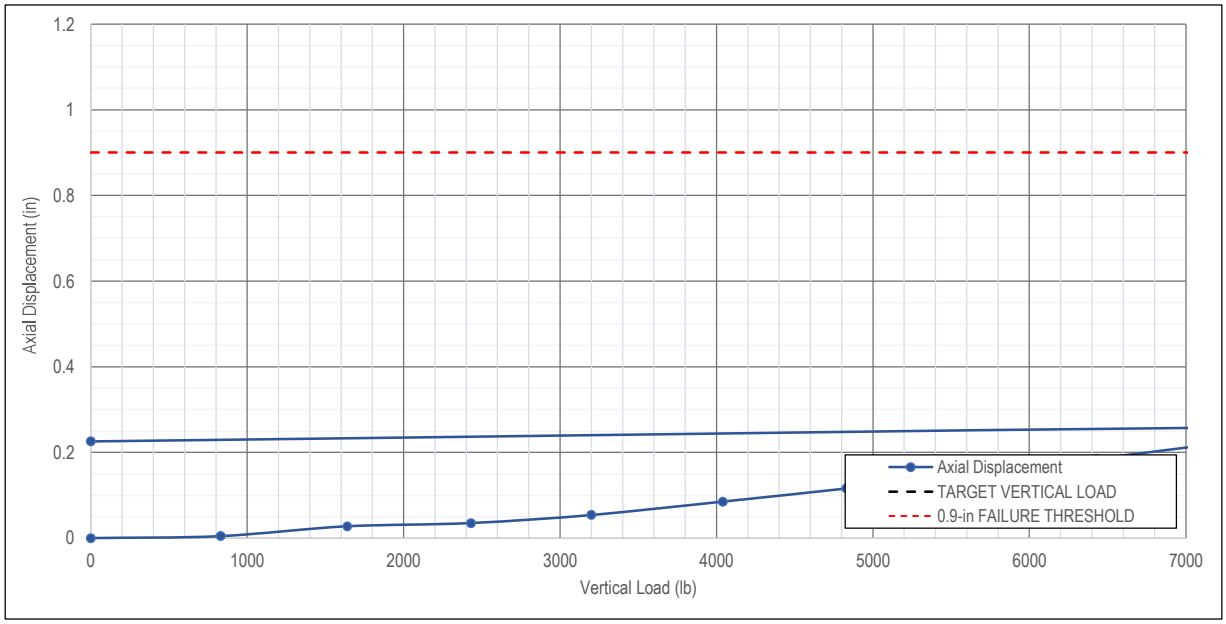
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	830	0.005	0.005	0.01
13594%	20%	1,600	1640	0.0285	0.02665	0.03
20391%	30%	2,400	2430	0.036	0.034	0.04
27188%	40%	3,200	3200	0.0555	0.053	0.05
33985%	50%	4,000	4040	0.0875	0.0825	0.09
40782%	60%	4,800	4830	0.119	0.1135	0.12
47579%	70%	5,600	5600	0.147	0.14	0.14
54376%	80%	6,400	6430	0.189	0.1805	0.18
61173%	90%	7,200	7240	0.2275	0.217	0.22
67970%	100%	8,000	8060	0.2655	0.253	0.26
0%	0%	0	0	0.227	0.225	0.23

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.23** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-55A

Lateral Load Test Results

PTL- **55** Designation: **A**

Section: W6x9

Latitude: 33.648118

Longitude: -114.69491

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 6.55

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

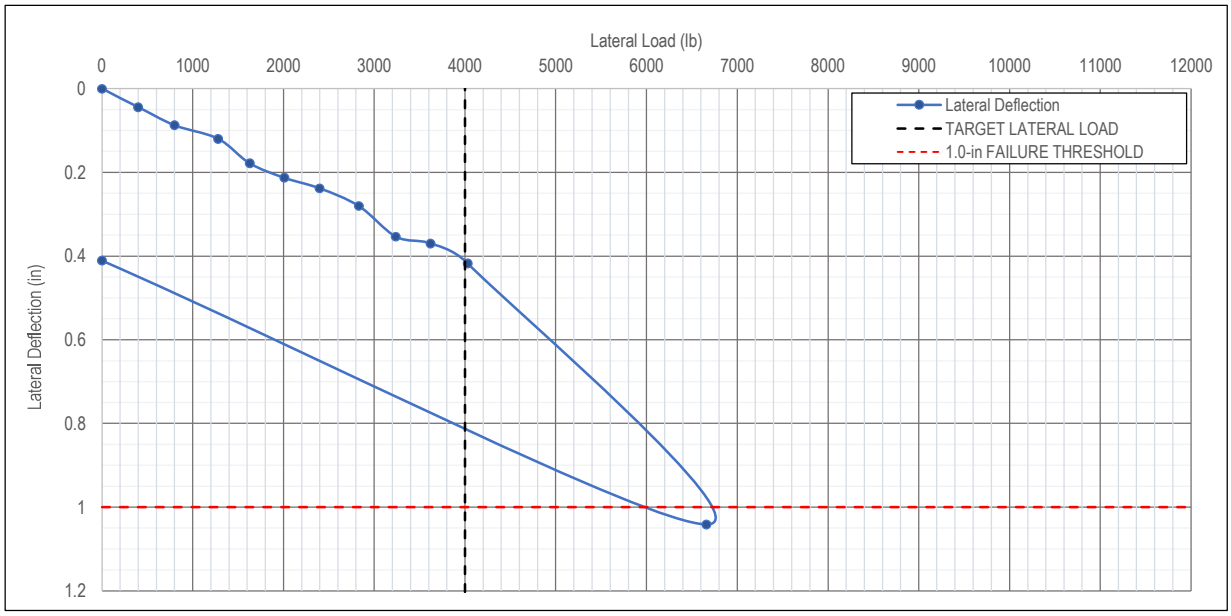
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.0435	0.045	0.04
62%	20%	800	800	0.0845	0.09	0.09
93%	30%	1,200	1280	0.116	0.1235	0.12
124%	40%	1,600	1630	0.1725	0.1845	0.18
155%	50%	2,000	2010	0.206	0.2195	0.21
186%	60%	2,400	2400	0.2305	0.2455	0.24
217%	70%	2,800	2830	0.2715	0.289	0.28
248%	80%	3,200	3240	0.343	0.364	0.35
279%	90%	3,600	3620	0.3595	0.3805	0.37
310%	100%	4,000	4030	0.4065	0.4285	0.42
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6660	1.059	1.023	1.04
0%	0%	0	0	0.43	0.3915	0.41

Load at 1.0-in lateral deflection: **6,487** **lb**

Residual lateral deformation: **0.41** **in**



Notes:

Failure reached at 6,660 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-55B

Axial Uplift Load Test Results

PTL - **55** Designation: **B**

Section: W6x9

Latitude: 33.648118

Longitude: -114.69491

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 12.39

Pile Damaged? No

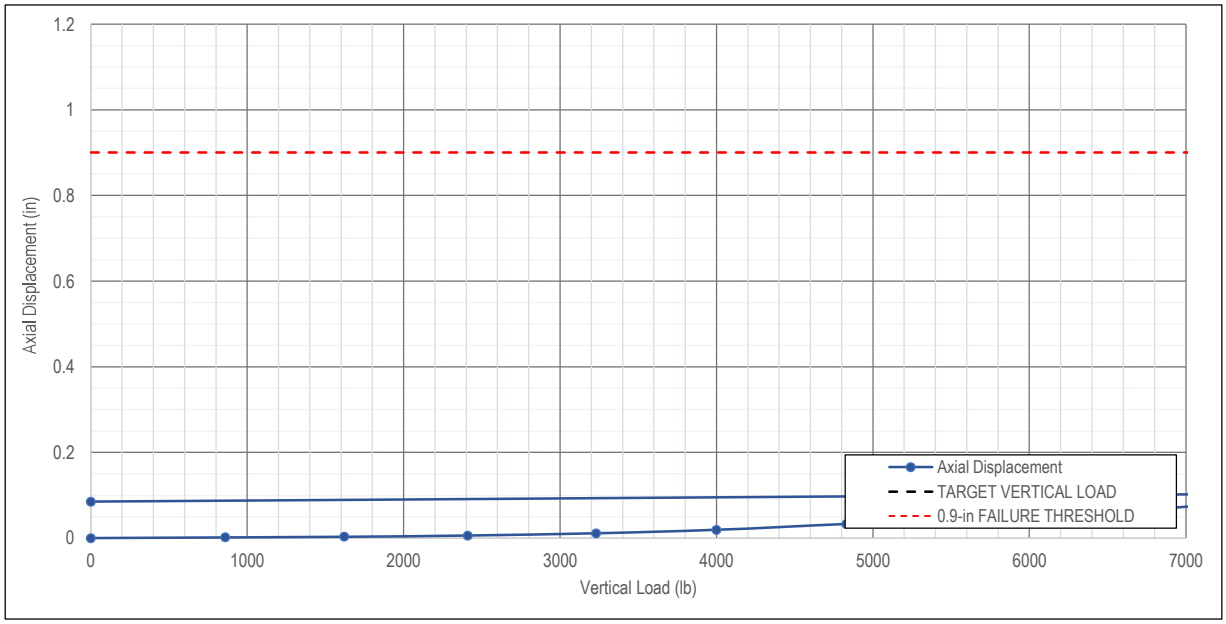
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	860	0.001	0.0025	0.00
13594%	20%	1,600	1620	0.003	0.004	0.00
20391%	30%	2,400	2410	0.006	0.0065	0.01
27188%	40%	3,200	3230	0.0105	0.012	0.01
33985%	50%	4,000	4000	0.019	0.0195	0.02
40782%	60%	4,800	4830	0.0325	0.033	0.03
47579%	70%	5,600	5580	0.0415	0.0415	0.04
54376%	80%	6,400	6430	0.058	0.0585	0.06
61173%	90%	7,200	7240	0.08	0.08005	0.08
67970%	100%	8,000	8030	0.103	0.103	0.10
0%	0%	0	0	0.086	0.084	0.09

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.09** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-55B

Lateral Load Test Results

PTL- **55** Designation: **B**

Section: W6x9

Latitude: 33.648118

Longitude: -114.69491

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 12.39

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

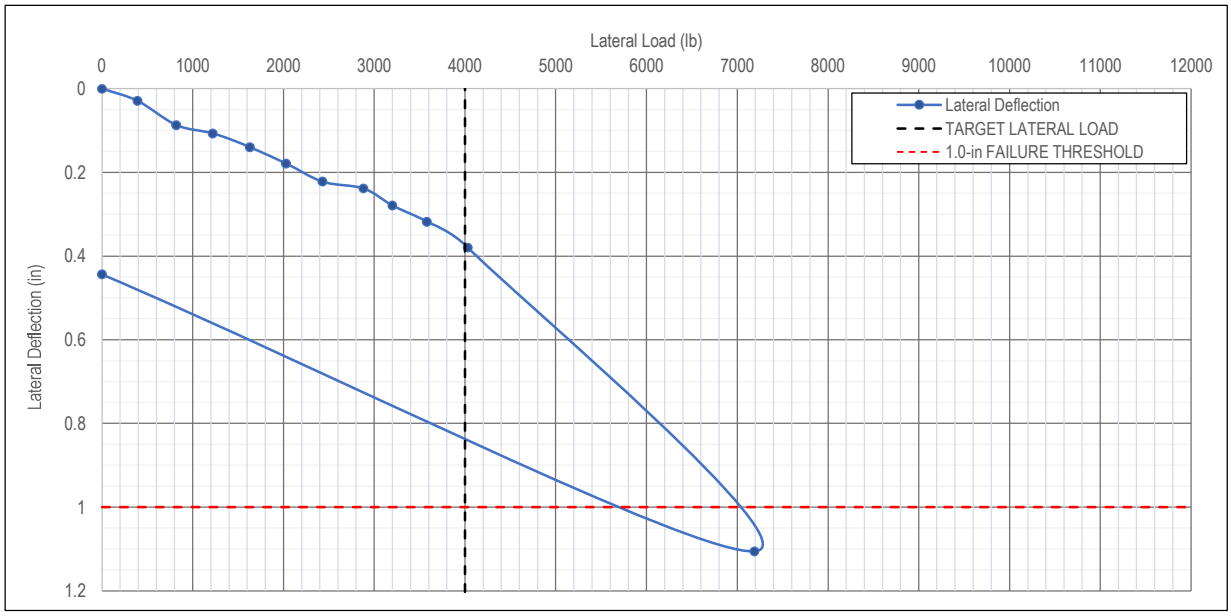
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	390	0.0275	0.03	0.03
62%	20%	800	820	0.084	0.09	0.09
93%	30%	1,200	1220	0.103	0.1105	0.11
124%	40%	1,600	1630	0.1345	0.1445	0.14
155%	50%	2,000	2030	0.173	0.185	0.18
186%	60%	2,400	2430	0.215	0.229	0.22
217%	70%	2,800	2880	0.2305	0.246	0.24
248%	80%	3,200	3200	0.27	0.288	0.28
279%	90%	3,600	3580	0.308	0.3275	0.32
310%	100%	4,000	4030	0.3675	0.391	0.38
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7190	1.068	1.1425	1.11
0%	0%	0	0	0.431	0.4565	0.44

Load at 1.0-in lateral deflection: **6,732** **lb**

Residual lateral deformation: **0.44** **in**



Notes:

Failure reached at 7,190 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-55C

Axial Compression Load Test Results (supplemental pile)

PTL - **55** Designation: **C**

Section: W6x9

Latitude: 33.648118

Longitude: -114.69491

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **15,000** (≈ 1180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 6.37

Pile Damaged? No

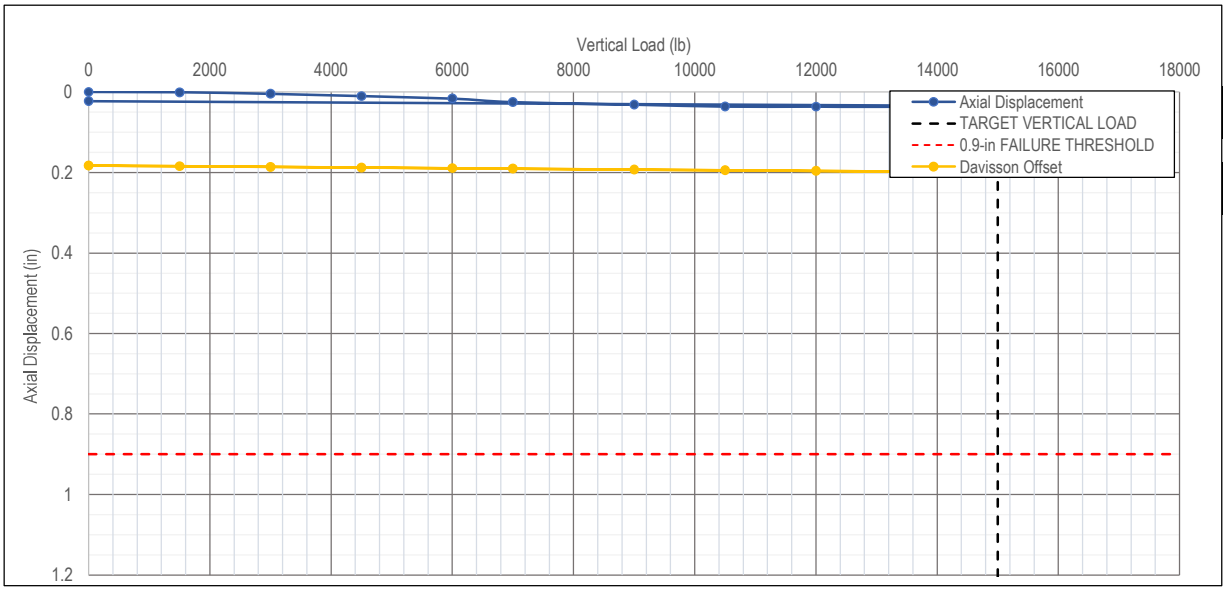
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL COMPRESSION** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
118%	10%	1,500	1500	0.0015	0.0005	0.00
236%	20%	3,000	3000	0.0055	0.004	0.00
354%	30%	4,500	4500	0.012	0.009	0.01
472%	40%	6,000	6000	0.018	0.0145	0.02
590%	50%	7,500	7000	0.0285	0.022	0.03
708%	60%	9,000	9000	0.035	0.028	0.03
826%	70%	10,500	10500	0.04	0.0325	0.04
944%	80%	12,000	12000	0.04	0.0335	0.04
1062%	90%	13,500	13500	0.039	0.0345	0.04
1180%	100%	15,000	15000	0.0355	0.0345	0.04
0%	0%	0	0	0.023	0.022	0.02

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.02** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-56A

Axial Uplift Load Test Results

PTL - **56** Designation: **A**

Section: W6x9

Latitude: 33.641915

Longitude: -114.68312

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 23.9

Pile Damaged? No

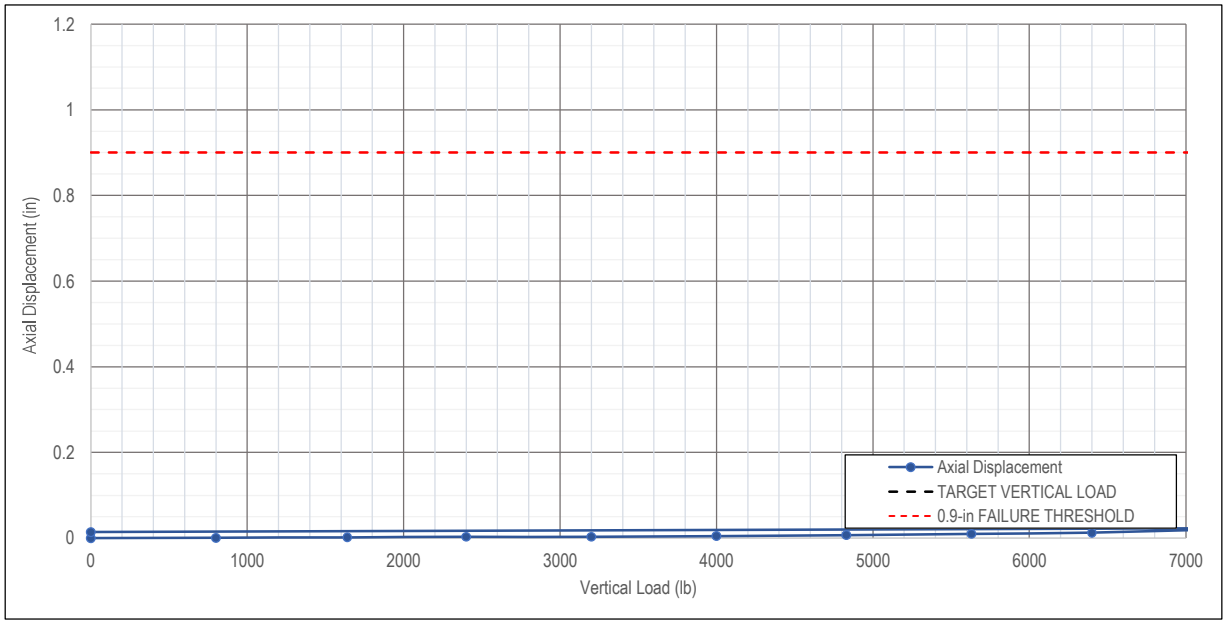
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0.002	0	0.00
436%	20%	1,600	1640	0.002	0.002	0.00
654%	30%	2,400	2400	0.004	0.002	0.00
872%	40%	3,200	3200	0.004	0.002	0.00
1090%	50%	4,000	4000	0.006	0.004	0.01
1308%	60%	4,800	4830	0.008	0.006	0.01
1526%	70%	5,600	5630	0.01	0.01	0.01
1744%	80%	6,400	6400	0.014	0.012	0.01
1962%	90%	7,200	7280	0.016	0.026	0.02
2180%	100%	8,000	7980	0.02	0.028	0.02
0%	0%	0	0	0.014	0.014	0.01

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-56A

Lateral Load Test Results

PTL- **56** Designation: **A**

Section: W6x9

Latitude: 33.641915

Longitude: -114.68312

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 23.9

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

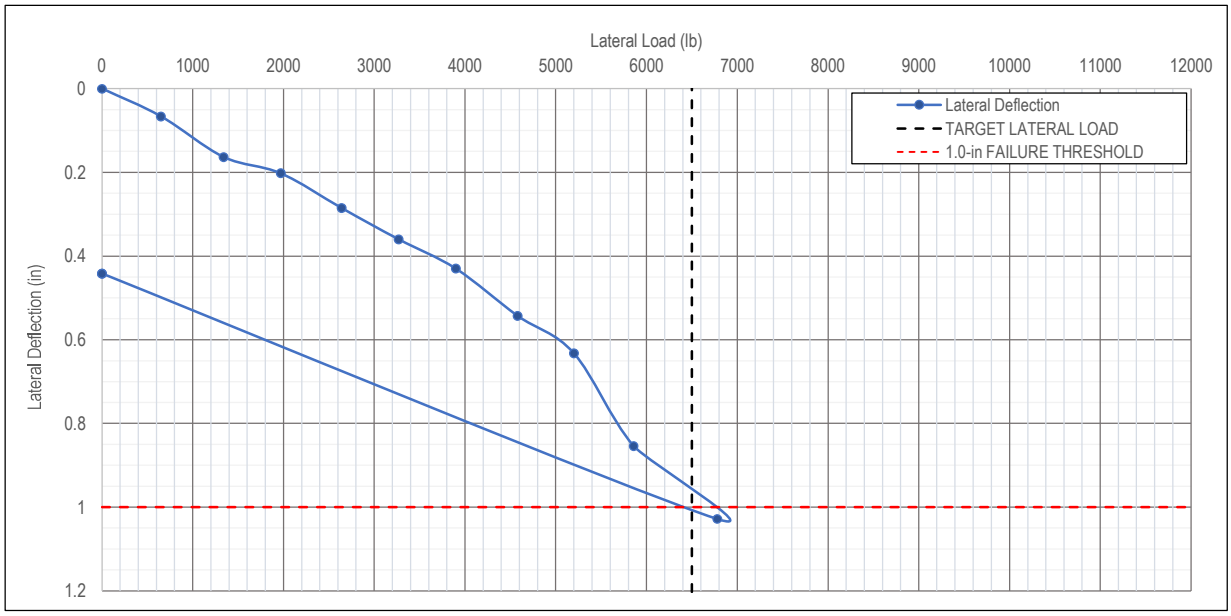
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.068	0.064	0.07
62%	20%	1,300	1340	0.168	0.158	0.16
93%	30%	1,950	1970	0.208	0.196	0.20
124%	40%	2,600	2640	0.294	0.276	0.29
155%	50%	3,250	3270	0.376	0.344	0.36
186%	60%	3,900	3900	0.446	0.414	0.43
217%	70%	4,550	4580	0.568	0.518	0.54
248%	80%	5,200	5200	0.666	0.598	0.63
279%	90%	5,850	5860	0.93	0.778	0.85
310%	100%	6,500	6780	1.134	0.922	1.03
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.522	0.362	0.44

Load at 1.0-in lateral deflection: **6,632** **lb**

Residual lateral deformation: **0.44** **in**



Notes:

Failure reached at 6,780 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-56B

Axial Uplift Load Test Results

PTL - **56** Designation: **B**

Section: W6x9

Latitude: 33.641915

Longitude: -114.68312

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 34.9

Pile Damaged? No

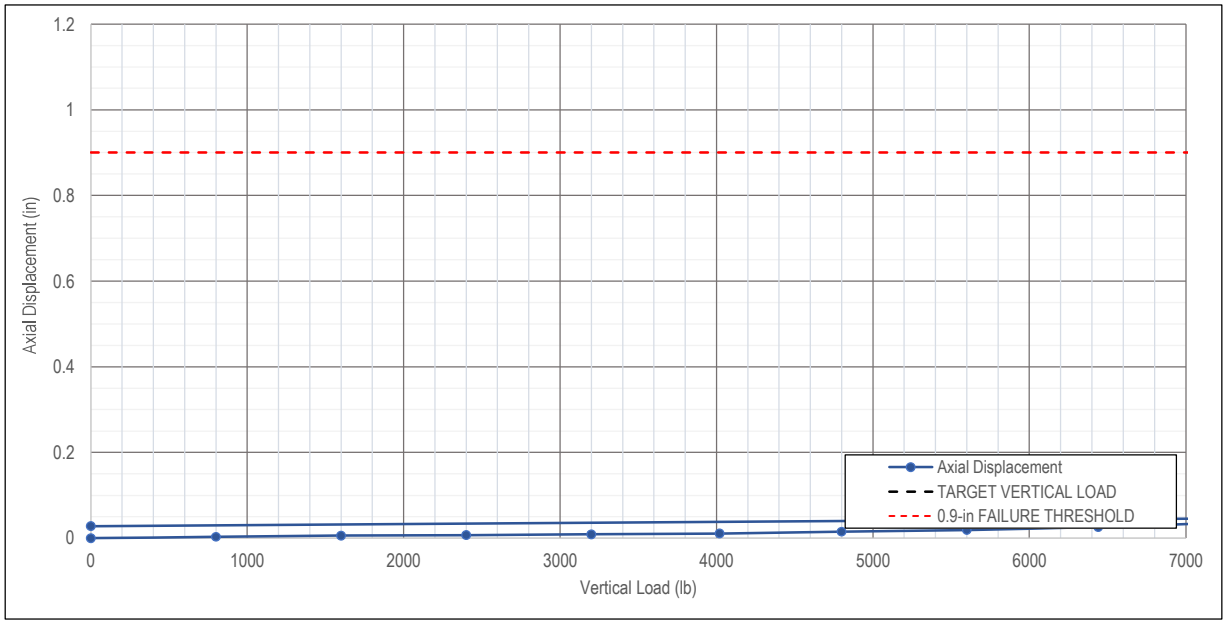
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0.004	0.002	0.00
436%	20%	1,600	1600	0.008	0.004	0.01
654%	30%	2,400	2400	0.01	0.004	0.01
872%	40%	3,200	3200	0.012	0.006	0.01
1090%	50%	4,000	4020	0.014	0.008	0.01
1308%	60%	4,800	4800	0.018	0.012	0.02
1526%	70%	5,600	5600	0.022	0.016	0.02
1744%	80%	6,400	6440	0.03	0.022	0.03
1962%	90%	7,200	7230	0.04	0.032	0.04
2180%	100%	8,000	8030	0.052	0.042	0.05
0%	0%	0	0	0.028	0.028	0.03

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-56B

Lateral Load Test Results

PTL- **56** Designation: **B**

Section: W6x9

Latitude: 33.641915

Longitude: -114.68312

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 34.9

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

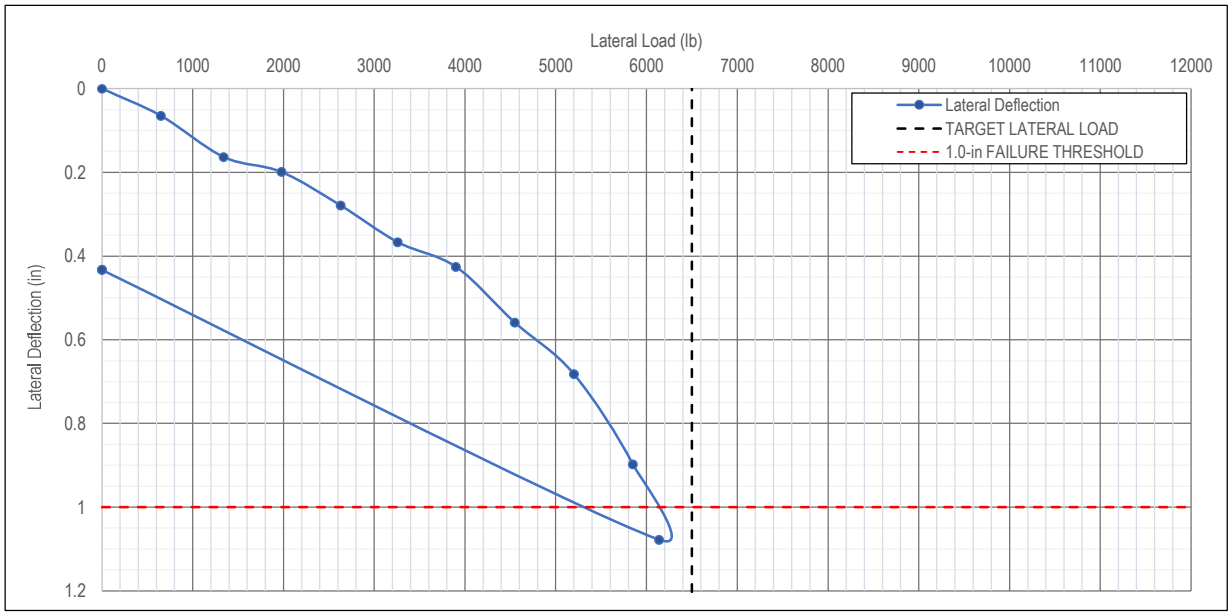
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.064	0.066	0.07
62%	20%	1,300	1340	0.164	0.162	0.16
93%	30%	1,950	1980	0.202	0.196	0.20
124%	40%	2,600	2630	0.284	0.274	0.28
155%	50%	3,250	3260	0.376	0.358	0.37
186%	60%	3,900	3900	0.408	0.444	0.43
217%	70%	4,550	4550	0.576	0.542	0.56
248%	80%	5,200	5200	0.71	0.654	0.68
279%	90%	5,850	5850	0.956	0.84	0.90
310%	100%	6,500	6140	1.182	0.974	1.08
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.496	0.37	0.43

Load at 1.0-in lateral deflection: **6,014** **lb**

Residual lateral deformation: **0.43** **in**



Notes:

Failure reached at 6,140 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-57A

Axial Uplift Load Test Results

PTL - **57** Designation: **A**

Section: W6x9

Latitude: 33.647237

Longitude: -114.68134

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 7.85

Pile Damaged? No

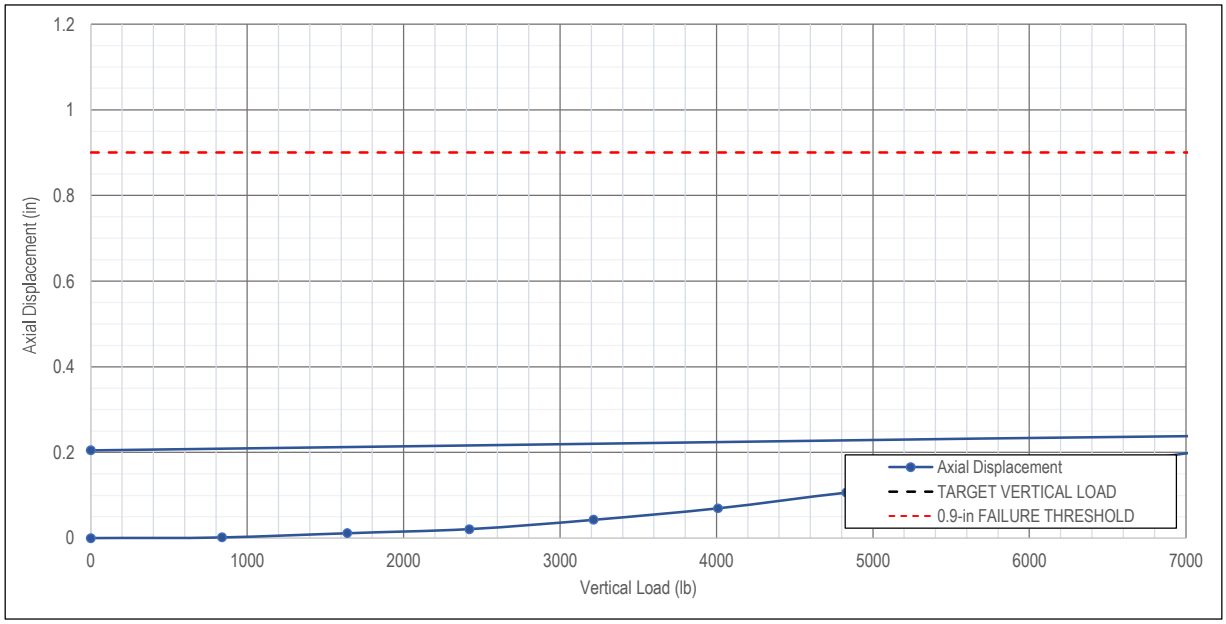
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	840	0.0015	0.002	0.00
13594%	20%	1,600	1640	0.013	0.0105	0.01
20391%	30%	2,400	2420	0.0205	0.0215	0.02
27188%	40%	3,200	3215	0.043	0.0425	0.04
33985%	50%	4,000	4010	0.07	0.0695	0.07
40782%	60%	4,800	4830	0.107	0.106	0.11
47579%	70%	5,600	5590	0.1345	0.132	0.13
54376%	80%	6,400	6420	0.1705	0.1675	0.17
61173%	90%	7,200	7210	0.21	0.206	0.21
67970%	100%	8,000	7985	0.243	0.238	0.24
0%	0%	0	0	0.205	0.2045	0.20

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.20** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-57A

Lateral Load Test Results

PTL- **57** Designation: **A**

Section: W6x9

Latitude: 33.647237

Longitude: -114.68134

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 7.85

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

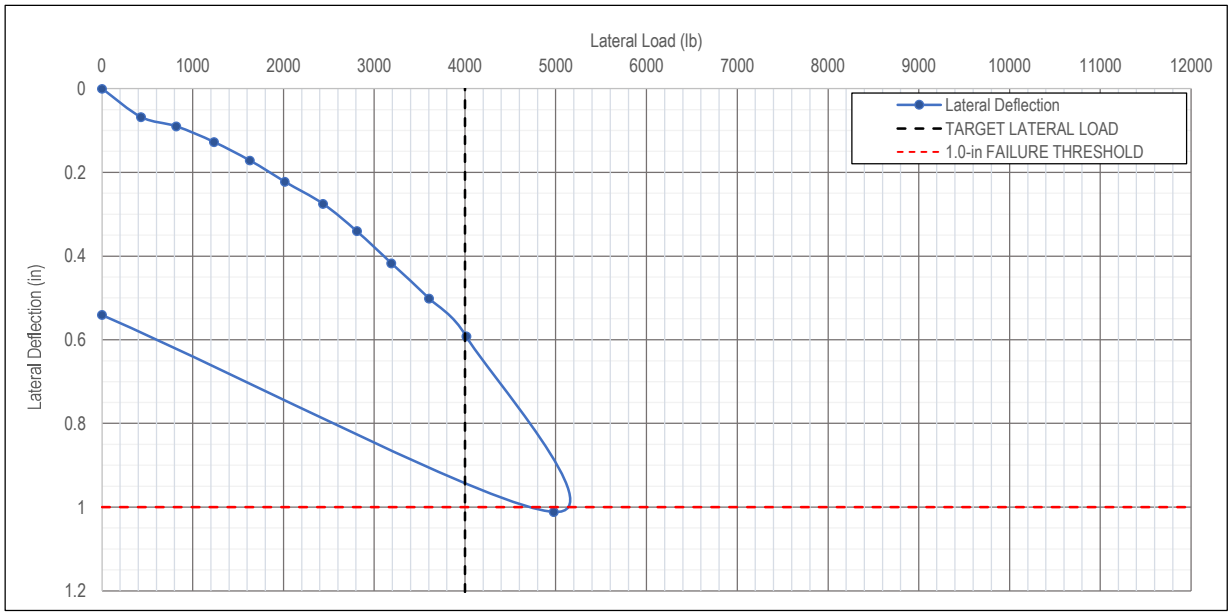
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	430	0.0675	0.0675	0.07
62%	20%	800	820	0.091	0.0885	0.09
93%	30%	1,200	1235	0.13	0.1245	0.13
124%	40%	1,600	1630	0.175	0.168	0.17
155%	50%	2,000	2015	0.2255	0.219	0.22
186%	60%	2,400	2435	0.2785	0.2715	0.28
217%	70%	2,800	2810	0.3445	0.3365	0.34
248%	80%	3,200	3190	0.4365	0.398	0.42
279%	90%	3,600	3605	0.506	0.497	0.50
310%	100%	4,000	4015	0.5955	0.5885	0.59
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			4980	1.0125	1.0105	1.01
0%	0%	0	0	0.545	0.5365	0.54

Load at 1.0-in lateral deflection: **4,954** **lb**

Residual lateral deformation: **0.54** **in**



Notes:

Failure reached at 4,980 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-57B

Axial Uplift Load Test Results

PTL - **57** Designation: **B**

Section: W6x9

Latitude: 33.647237

Longitude: -114.68134

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 9.45

Pile Damaged? No

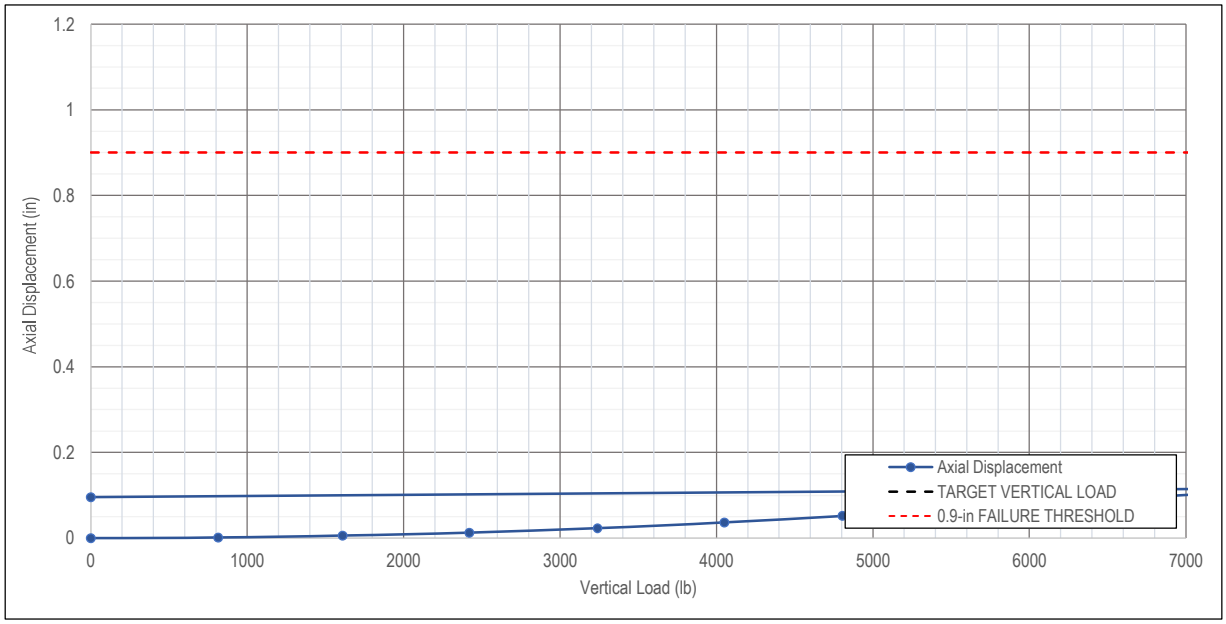
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	815	0.0015	0.0015	0.00
13594%	20%	1,600	1610	0.0065	0.0055	0.01
20391%	30%	2,400	2420	0.014	0.012	0.01
27188%	40%	3,200	3240	0.025	0.022	0.02
33985%	50%	4,000	4050	0.039	0.034	0.04
40782%	60%	4,800	4805	0.055	0.0485	0.05
47579%	70%	5,600	5615	0.073	0.065	0.07
54376%	80%	6,400	6420	0.093	0.083	0.09
61173%	90%	7,200	7220	0.111	0.099	0.11
67970%	100%	8,000	8030	0.1305	0.1035	0.12
0%	0%	0	0	0.097	0.094	0.10

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.10** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-57B

Lateral Load Test Results

PTL- **57** Designation: **B**

Section: W6x9

Latitude: 33.647237

Longitude: -114.68134

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 9.45

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

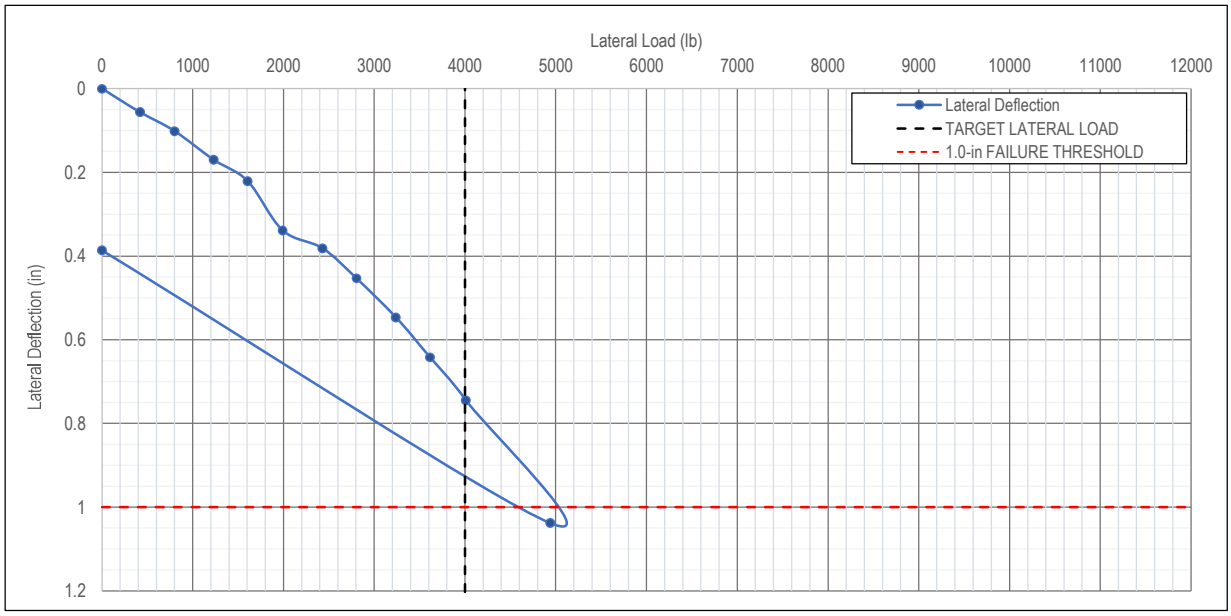
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	420	0.0555	0.0565	0.06
62%	20%	800	800	0.099	0.1035	0.10
93%	30%	1,200	1230	0.1675	0.1715	0.17
124%	40%	1,600	1605	0.218	0.224	0.22
155%	50%	2,000	1990	0.2845	0.393	0.34
186%	60%	2,400	2430	0.377	0.3855	0.38
217%	70%	2,800	2805	0.4475	0.4585	0.45
248%	80%	3,200	3240	0.54	0.553	0.55
279%	90%	3,600	3615	0.6345	0.6495	0.64
310%	100%	4,000	4010	0.7355	0.754	0.74
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			4940	1.023	1.053	1.04
0%	0%	0	0	0.373	0.4	0.39

Load at 1.0-in lateral deflection: **4,819** **lb**

Residual lateral deformation: **0.39** **in**



Notes:

Failure reached at 4,940 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-58A

Axial Uplift Load Test Results

PTL - **58** Designation: **A**

Section: W6x9

Latitude: 33.647237

Longitude: -114.67839

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 11.65

Pile Damaged? No

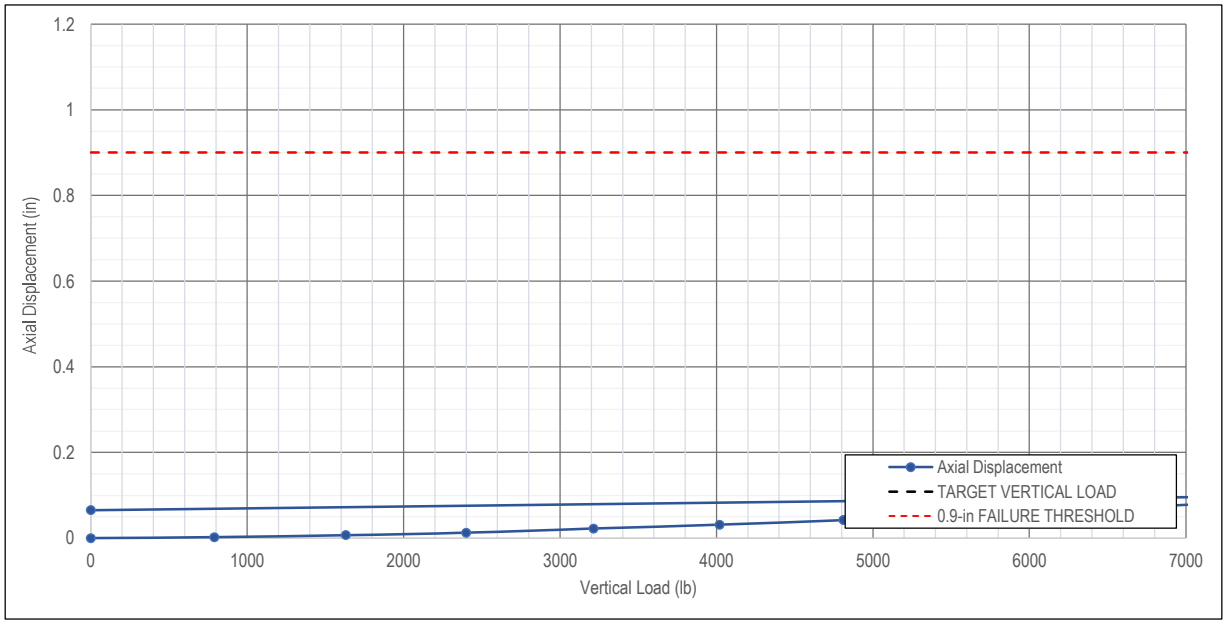
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	790	0.002	0.0025	0.00
436%	20%	1,600	1630	0.0065	0.0075	0.01
654%	30%	2,400	2400	0.0115	0.014	0.01
872%	40%	3,200	3215	0.0205	0.024	0.02
1090%	50%	4,000	4020	0.0285	0.034	0.03
1308%	60%	4,800	4810	0.039	0.0455	0.04
1526%	70%	5,600	5625	0.051	0.0585	0.05
1744%	80%	6,400	6395	0.063	0.0715	0.07
1962%	90%	7,200	7220	0.0765	0.0875	0.08
2180%	100%	8,000	8015	0.0935	0.1045	0.10
0%	0%	0	0	0.065	0.0655	0.07

Load at 0.9-in axial displacement: N/A lb

Residual axial displacement: 0.07 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-58A

Lateral Load Test Results

PTL- **58** Designation: **A**

Section: W6x9

Latitude: 33.647237

Longitude: -114.67839

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.65

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

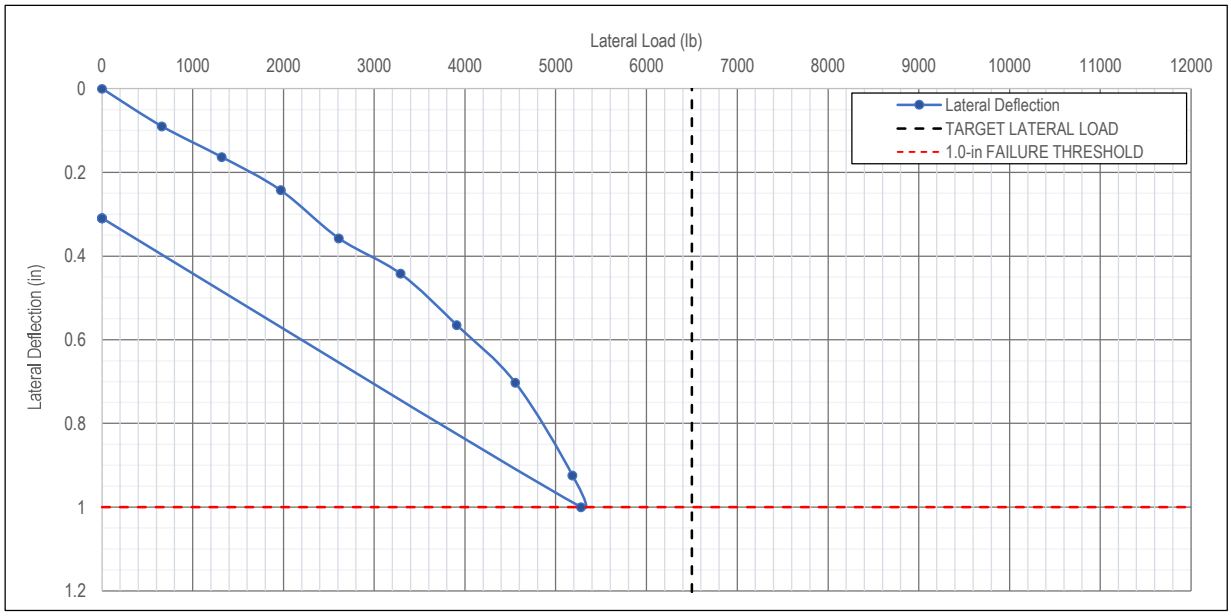
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	660	0.0865	0.093	0.09
62%	20%	1,300	1320	0.1605	0.166	0.16
93%	30%	1,950	1970	0.2415	0.2435	0.24
124%	40%	2,600	2610	0.3555	0.359	0.36
155%	50%	3,250	3290	0.44	0.4445	0.44
186%	60%	3,900	3910	0.5625	0.5675	0.57
217%	70%	4,550	4555	0.701	0.704	0.70
248%	80%	5,200	5185	0.9285	0.92	0.92
279%	90%	5,850	5280	1.0045	0.9955	1.00
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.318	0.301	0.31

Load at 1.0-in lateral deflection: **5,280** **lb**

Residual lateral deformation: **0.31** **in**



Notes:

Failure reached at 5,280 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-58B

Axial Uplift Load Test Results

PTL - **58** Designation: **B**

Section: W6x9

Latitude: 33.647237

Longitude: -114.67839

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 18.11

Pile Damaged? No

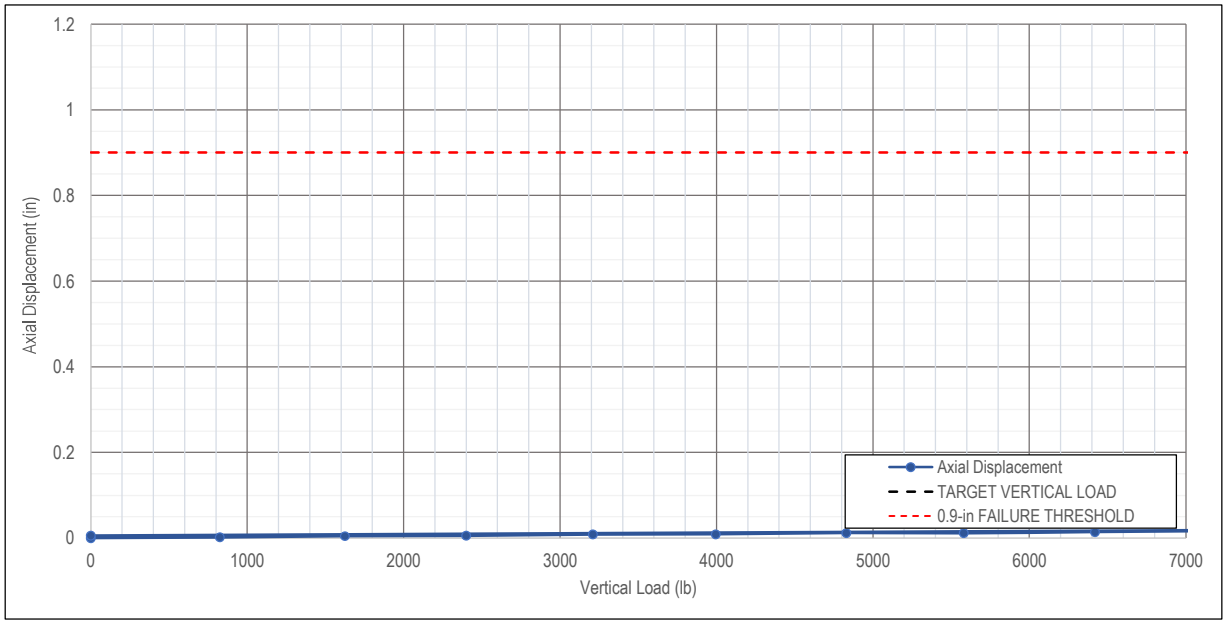
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	825	0.0015	0.002	0.00
436%	20%	1,600	1625	0.0055	0.0035	0.00
654%	30%	2,400	2400	0.0055	0.0055	0.01
872%	40%	3,200	3210	0.0075	0.009	0.01
1090%	50%	4,000	3995	0.008	0.01	0.01
1308%	60%	4,800	4830	0.01	0.014	0.01
1526%	70%	5,600	5580	0.01	0.0135	0.01
1744%	80%	6,400	6420	0.0105	0.0175	0.01
1962%	90%	7,200	7195	0.0125	0.021	0.02
2180%	100%	8,000	7985	0.015	0.025	0.02
0%	0%	0	0	0.0055	0.0065	0.01

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-58B

Lateral Load Test Results

PTL- **58** Designation: **B**

Section: W6x9

Latitude: 33.647237

Longitude: -114.67839

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 18.11

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

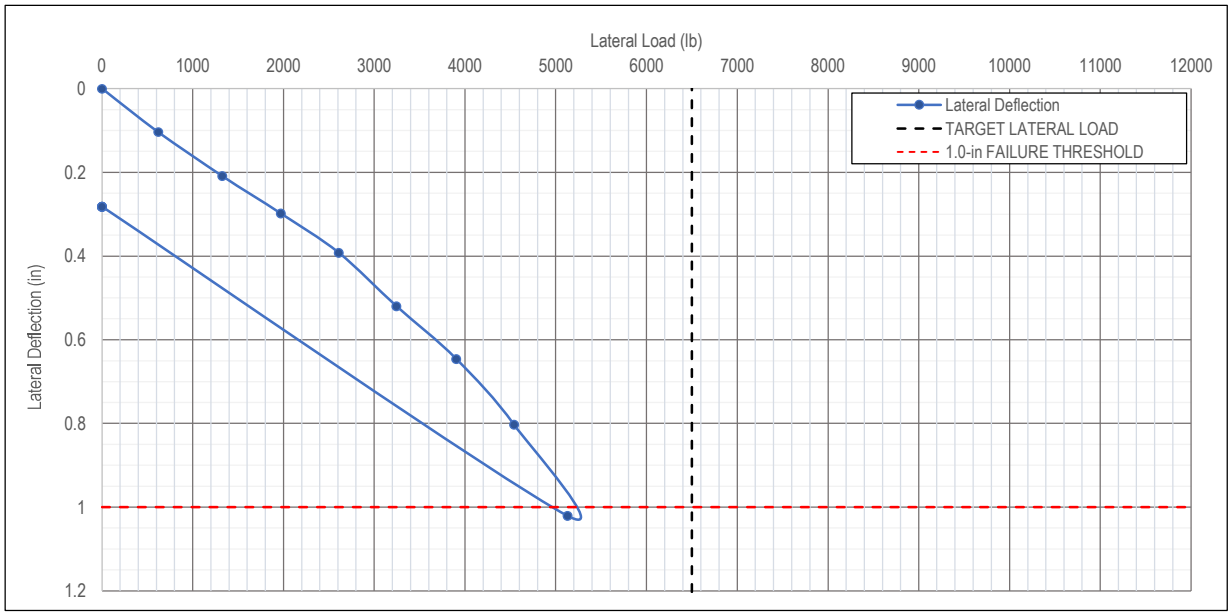
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	620	0.1025	0.1045	0.10
62%	20%	1,300	1325	0.2075	0.21	0.21
93%	30%	1,950	1970	0.2975	0.299	0.30
124%	40%	2,600	2610	0.392	0.3925	0.39
155%	50%	3,250	3245	0.518	0.5215	0.52
186%	60%	3,900	3905	0.643	0.649	0.65
217%	70%	4,550	4540	0.7965	0.8095	0.80
248%	80%	5,200	5130	1.04	1.001	1.02
279%	90%	5,850				
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.293	0.2715	0.28

Load at 1.0-in lateral deflection: **5,074** **lb**

Residual lateral deformation: **0.28** **in**



Notes:

Failure reached at 5,130 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-59A

Axial Uplift Load Test Results

PTL - **59** Designation: **A**

Section: W6x9

Latitude: 33.64904

Longitude: -144.66626

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 11.37

Pile Damaged? No

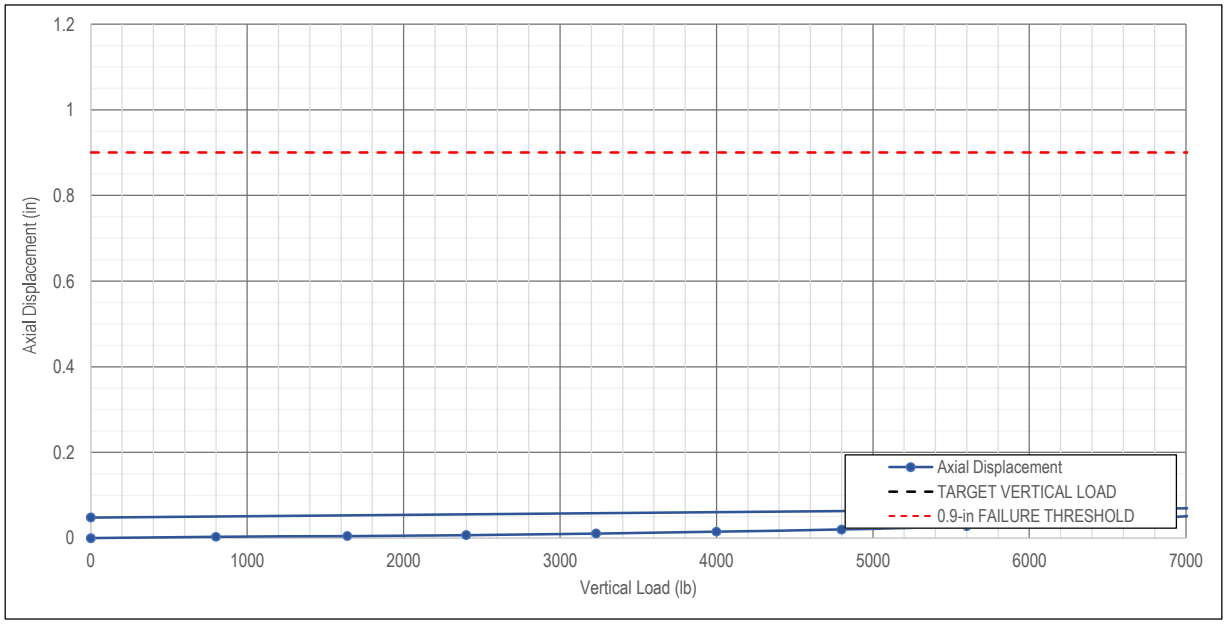
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	800	0.004	0.002	0.00
13594%	20%	1,600	1640	0.006	0.004	0.01
20391%	30%	2,400	2400	0.008	0.006	0.01
27188%	40%	3,200	3230	0.012	0.01	0.01
33985%	50%	4,000	4000	0.016	0.014	0.02
40782%	60%	4,800	4800	0.02	0.02	0.02
47579%	70%	5,600	5600	0.028	0.028	0.03
54376%	80%	6,400	6400	0.038	0.04	0.04
61173%	90%	7,200	7230	0.054	0.058	0.06
67970%	100%	8,000	8000	0.068	0.076	0.07
0%	0%	0	0	0.048	0.048	0.05

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.05** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-59A

Lateral Load Test Results

PTL- **59** Designation: **A**

Section: W6x9

Latitude: 33.64904

Longitude: -144.66626

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.37

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

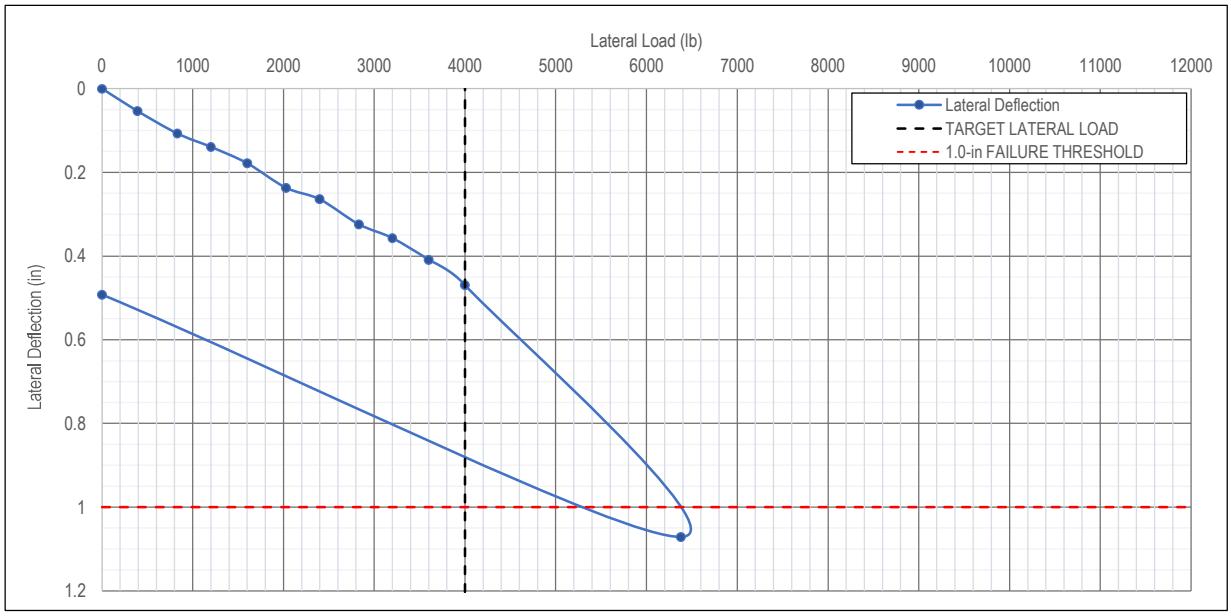
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	390	0.052	0.054	0.05
62%	20%	800	830	0.108	0.106	0.11
93%	30%	1,200	1200	0.14	0.138	0.14
124%	40%	1,600	1600	0.18	0.176	0.18
155%	50%	2,000	2030	0.24	0.234	0.24
186%	60%	2,400	2400	0.268	0.26	0.26
217%	70%	2,800	2830	0.33	0.318	0.32
248%	80%	3,200	3200	0.364	0.35	0.36
279%	90%	3,600	3600	0.418	0.4	0.41
310%	100%	4,000	4000	0.48	0.458	0.47
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6380	1.162	0.98	1.07
0%	0%	0	0	0.548	0.436	0.49

Load at 1.0-in lateral deflection: **6,099** **lb**

Residual lateral deformation: **0.49** **in**



Notes:

Failure reached at 6,380 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-59B

Axial Uplift Load Test Results

PTL - **59** Designation: **B**

Section: W6x9

Latitude: 33.64904

Longitude: -144.66626

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 19.25

Pile Damaged? No

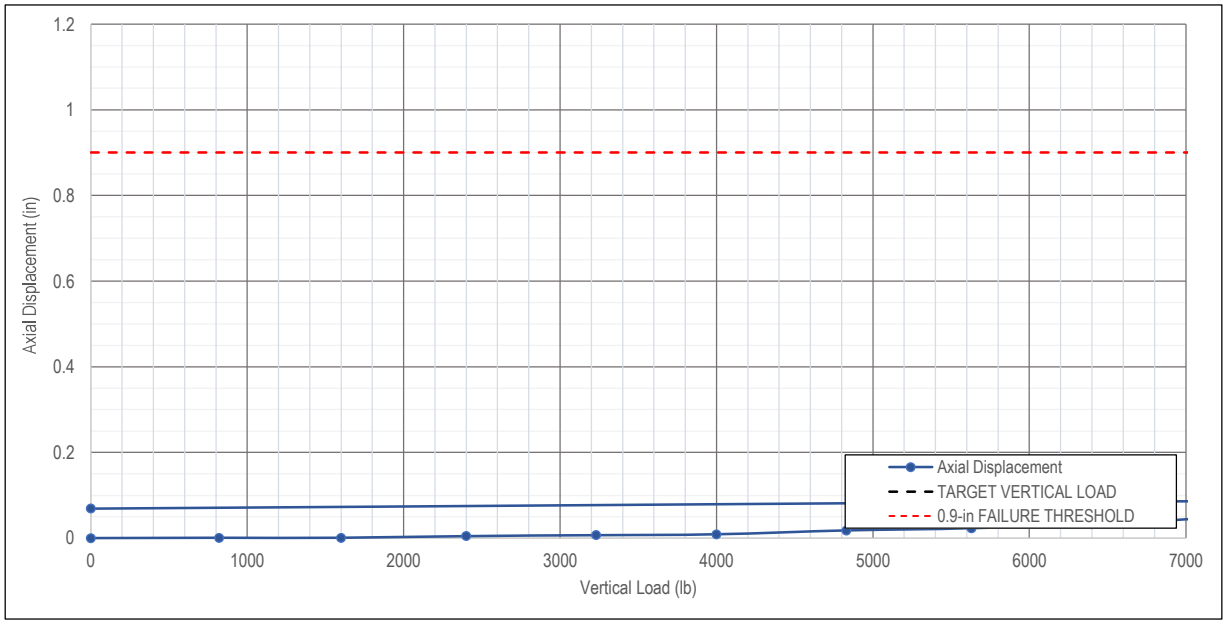
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	820	0	0.002	0.00
13594%	20%	1,600	1600	0	0.002	0.00
20391%	30%	2,400	2400	0.004	0.006	0.01
27188%	40%	3,200	3230	0.006	0.008	0.01
33985%	50%	4,000	4000	0.008	0.01	0.01
40782%	60%	4,800	4830	0.016	0.02	0.02
47579%	70%	5,600	5630	0.02	0.026	0.02
54376%	80%	6,400	6400	0.03	0.036	0.03
61173%	90%	7,200	7180	0.044	0.054	0.05
67970%	100%	8,000	8030	0.078	0.094	0.09
0%	0%	0	0	0.068	0.07	0.07

Load at 0.9-in axial displacement: N/A lb

Residual axial displacement: 0.07 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-59B

Lateral Load Test Results

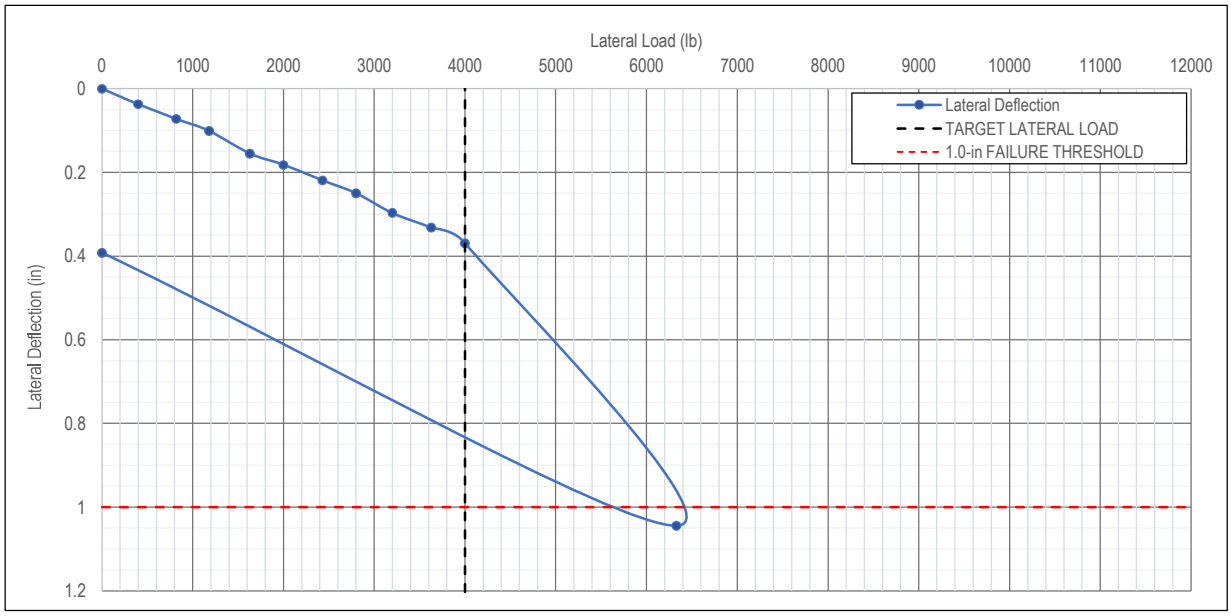
PTL-	**59**	Designation:	**B**		
Section:	W6x9				
Latitude:	33.64904				
Longitude:	-144.66626				
Tested By:	ANS				
Test Date:	10/28/2025				

Target Test Load (lb):	**4,000**	(≈ 310% Design Load)	
Installation Method:	Direct Drive (DD)	Saturated Soil?	No
- -		Drive Time (s):	19.25
Target Embedment Depth (ft):	7.00	Pile Damaged?	No
Actual Embedment Depth (ft):	7	Gauge 1 Height (in):	6
Lateral Load Application Height (ft):	5	Gauge 2 Height (in):	6

Strong Axis Plumb. (deg): _____ Weak Axis Plumb. (deg): _____

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.042	0.032	0.04
62%	20%	800	820	0.08	0.064	0.07
93%	30%	1,200	1180	0.108	0.094	0.10
124%	40%	1,600	1630	0.164	0.146	0.16
155%	50%	2,000	2000	0.192	0.172	0.18
186%	60%	2,400	2430	0.234	0.204	0.22
217%	70%	2,800	2800	0.266	0.234	0.25
248%	80%	3,200	3200	0.31	0.284	0.30
279%	90%	3,600	3630	0.354	0.31	0.33
310%	100%	4,000	4000	0.394	0.344	0.37
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6330	1.15	0.938	1.04
0%	0%	0	0	0.488	0.296	0.39

Load at 1.0-in lateral deflection: **6,178** **lb** **Residual lateral deformation:** **0.39** **in**



Notes:

Failure reached at 6,330 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-59C

Axial Compression Load Test Results (supplemental pile)

PTL - **59**
Section: W6x9
Latitude: 33.64904
Longitude: -144.66626
Tested By: ANS
Test Date: 10/28/2025

Designation: **C**

Target Test Load (lb): **15,000** (≈ 1180% Design Load)
Installation Method: Direct Drive (DD)
- -
Target Embedment Depth (ft): 5.00
Actual Embedment Depth (ft): 5

Saturated Soil? No
Drive Time (s): 11.61
Pile Damaged? No

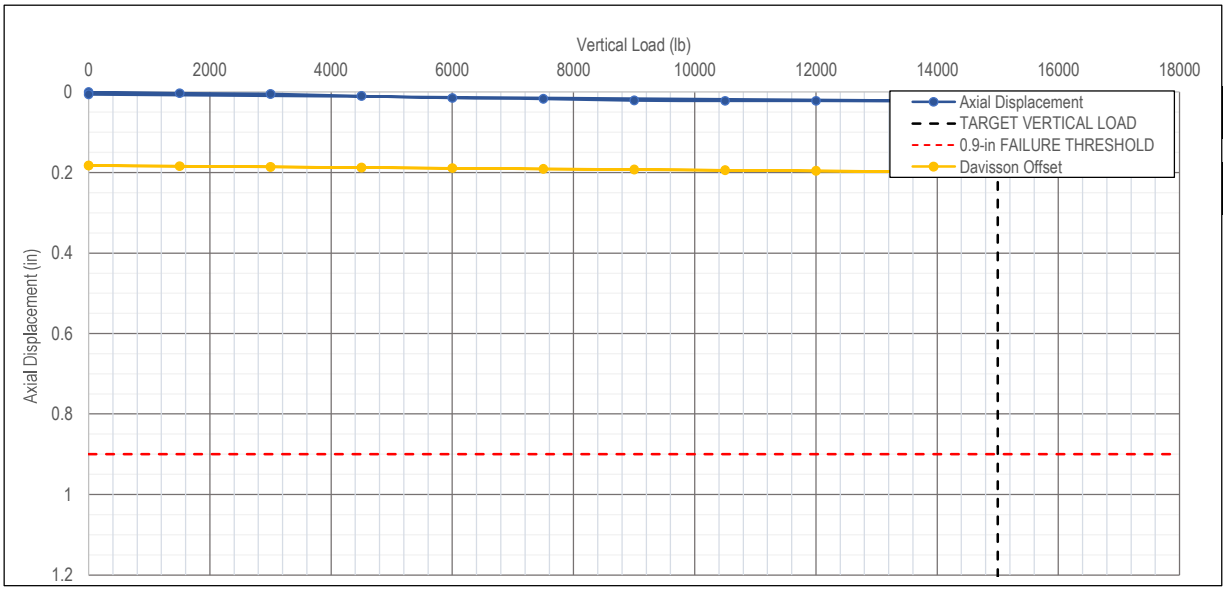
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL COMPRESSION** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
118%	10%	1,500	1500	0.002	0.004	0.00
236%	20%	3,000	3000	0.004	0.006	0.01
354%	30%	4,500	4500	0.008	0.012	0.01
472%	40%	6,000	6000	0.012	0.018	0.02
590%	50%	7,500	7500	0.014	0.02	0.02
708%	60%	9,000	9000	0.018	0.024	0.02
826%	70%	10,500	10500	0.02	0.024	0.02
944%	80%	12,000	12000	0.02	0.024	0.02
1062%	90%	13,500	13500	0.022	0.024	0.02
1180%	100%	15,000	15000	0.022	0.024	0.02
0%	0%	0	0	0.004	0.008	0.01

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-60A

Axial Uplift Load Test Results

PTL - **60** Designation: **A**

Section: W6x9

Latitude: 33.64808

Longitude: -114.66327

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 25.14

Pile Damaged? No

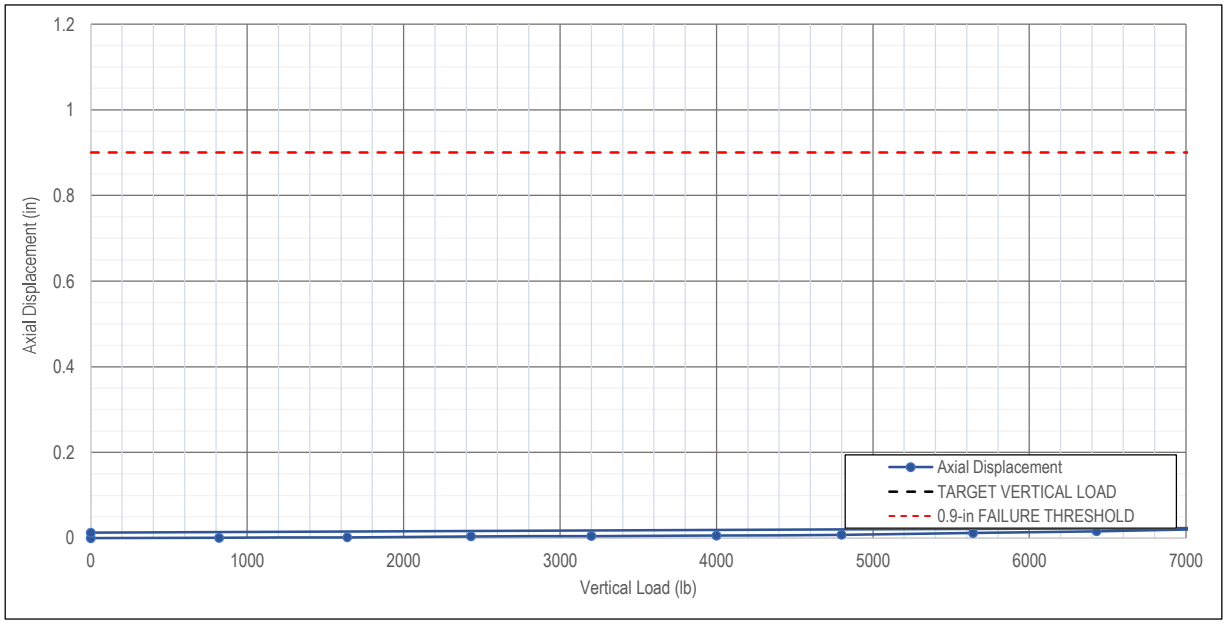
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	820	0.002	0	0.00
436%	20%	1,600	1640	0.002	0.002	0.00
654%	30%	2,400	2430	0.004	0.004	0.00
872%	40%	3,200	3200	0.004	0.006	0.01
1090%	50%	4,000	4000	0.006	0.006	0.01
1308%	60%	4,800	4800	0.006	0.01	0.01
1526%	70%	5,600	5640	0.01	0.014	0.01
1744%	80%	6,400	6430	0.012	0.02	0.02
1962%	90%	7,200	7230	0.014	0.03	0.02
2180%	100%	8,000	8030	0.016	0.034	0.03
0%	0%	0	0	0.012	0.014	0.01

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-60A

Lateral Load Test Results

PTL- **60** Designation: **A**

Section: W6x9

Latitude: 33.64808

Longitude: -114.66327

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 25.14

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

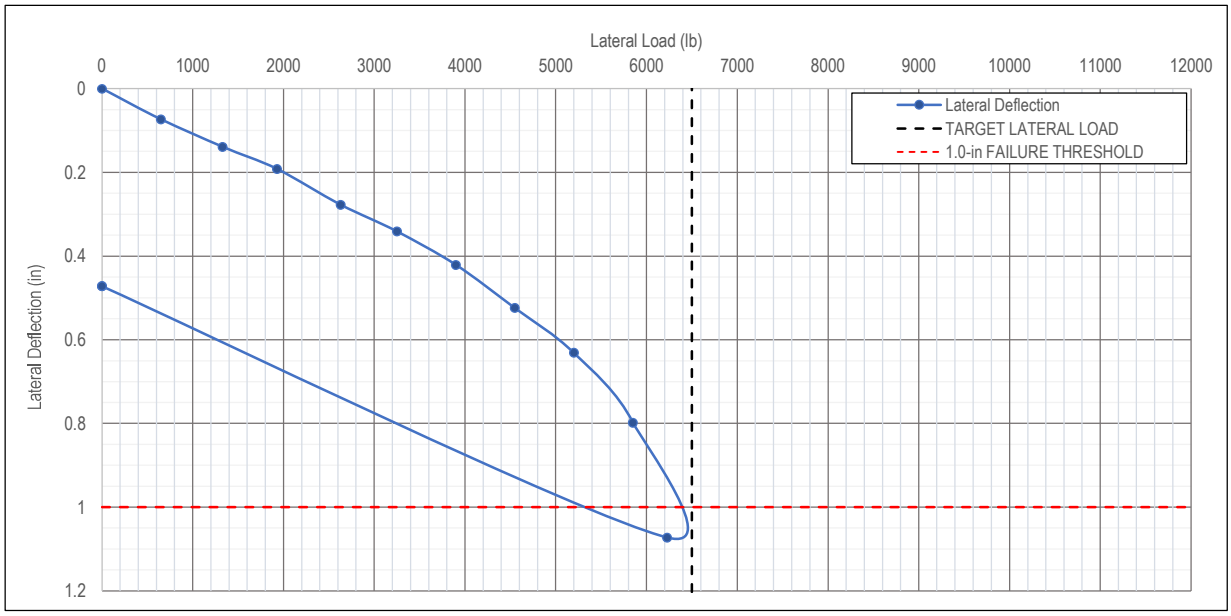
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.076	0.07	0.07
62%	20%	1,300	1330	0.144	0.134	0.14
93%	30%	1,950	1930	0.2	0.184	0.19
124%	40%	2,600	2630	0.288	0.266	0.28
155%	50%	3,250	3250	0.354	0.328	0.34
186%	60%	3,900	3900	0.438	0.404	0.42
217%	70%	4,550	4550	0.546	0.502	0.52
248%	80%	5,200	5200	0.662	0.6	0.63
279%	90%	5,850	5850	0.848	0.748	0.80
310%	100%	6,500	6230	1.202	0.944	1.07
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.558	0.386	0.47

Load at 1.0-in lateral deflection: **6,129** **lb**

Residual lateral deformation: **0.47** **in**



Notes:

Failure reached at 6,230 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-60B

Axial Uplift Load Test Results

PTL - **60** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.64808 - - Saturated Soil? No

Longitude: -114.66327 Target Embedment Depth (ft): 8.00 Drive Time (s): 21.86

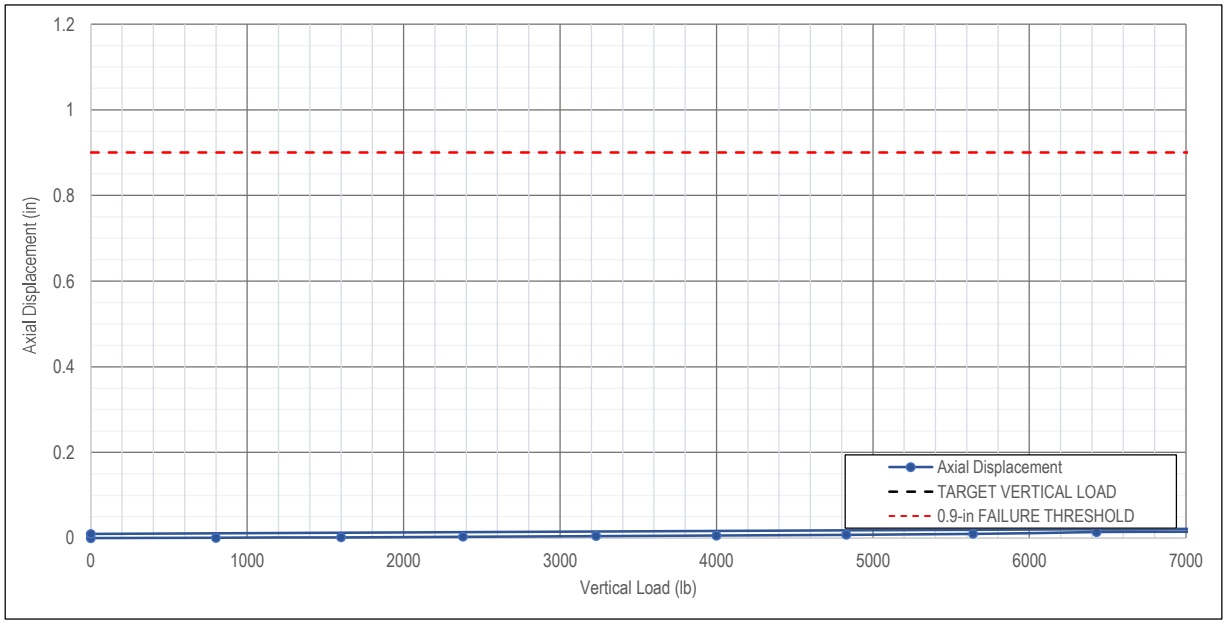
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/28/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0.002	0	0.00
436%	20%	1,600	1600	0.002	0.002	0.00
654%	30%	2,400	2380	0.004	0.002	0.00
872%	40%	3,200	3230	0.006	0.004	0.01
1090%	50%	4,000	4000	0.008	0.004	0.01
1308%	60%	4,800	4830	0.01	0.006	0.01
1526%	70%	5,600	5640	0.012	0.008	0.01
1744%	80%	6,400	6430	0.016	0.012	0.01
1962%	90%	7,200	7230	0.016	0.016	0.02
2180%	100%	8,000	8030	0.018	0.028	0.02
0%	0%	0	0	0.01	0.01	0.01

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-60B

Lateral Load Test Results

PTL- **60** Designation: **B**

Section: W6x9

Latitude: 33.64808

Longitude: -114.66327

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 21.86

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

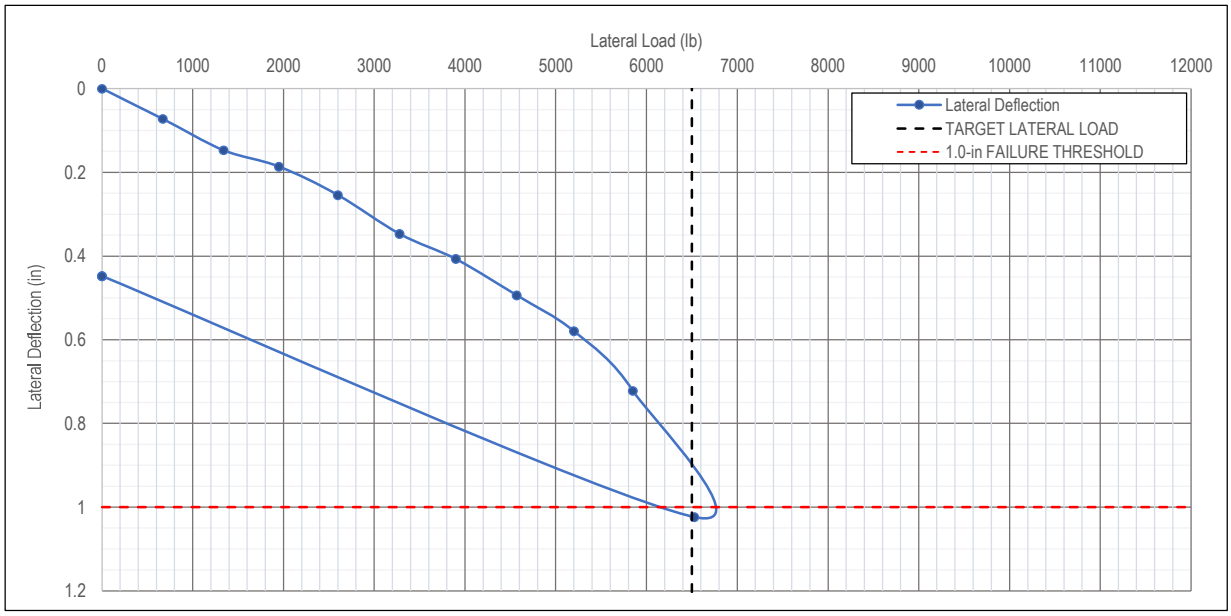
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	670	0.074	0.07	0.07
62%	20%	1,300	1340	0.152	0.142	0.15
93%	30%	1,950	1950	0.192	0.18	0.19
124%	40%	2,600	2600	0.262	0.246	0.25
155%	50%	3,250	3280	0.358	0.336	0.35
186%	60%	3,900	3900	0.42	0.394	0.41
217%	70%	4,550	4570	0.51	0.478	0.49
248%	80%	5,200	5200	0.6	0.558	0.58
279%	90%	5,850	5850	0.756	0.688	0.72
310%	100%	6,500	6530	1.118	0.93	1.02
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.508	0.388	0.45

Load at 1.0-in lateral deflection: **6,476** **lb**

Residual lateral deformation: **0.45** **in**



Notes:

Failure reached at 6,530 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-60D

Axial Uplift Load Test Results

PTL - **60** Designation: **D**

Section: **W6x12**

Latitude: 33.64808

Longitude: -114.66327

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 43.78

Pile Damaged? No

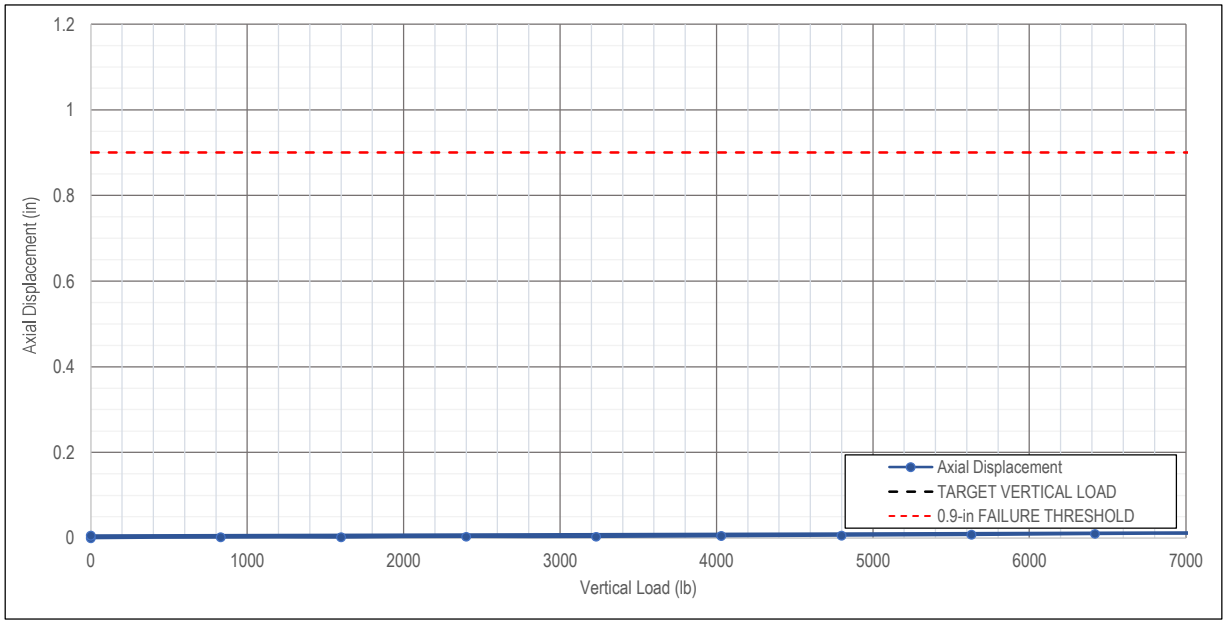
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	830	0.002	0.002	0.00
436%	20%	1,600	1600	0.002	0.002	0.00
654%	30%	2,400	2400	0.002	0.004	0.00
872%	40%	3,200	3230	0.002	0.004	0.00
1090%	50%	4,000	4030	0.004	0.006	0.01
1308%	60%	4,800	4800	0.004	0.008	0.01
1526%	70%	5,600	5630	0.008	0.008	0.01
1744%	80%	6,400	6420	0.01	0.01	0.01
1962%	90%	7,200	7180	0.012	0.01	0.01
2180%	100%	8,000	8030	0.014	0.014	0.01
0%	0%	0	0	0.006	0.006	0.01

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-60D

Lateral Load Test Results

PTL- **60** Designation: **D**

Section: **W6x12**

Latitude: 33.64808

Longitude: -114.66327

Tested By: ANS

Test Date: 10/28/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 43.78

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

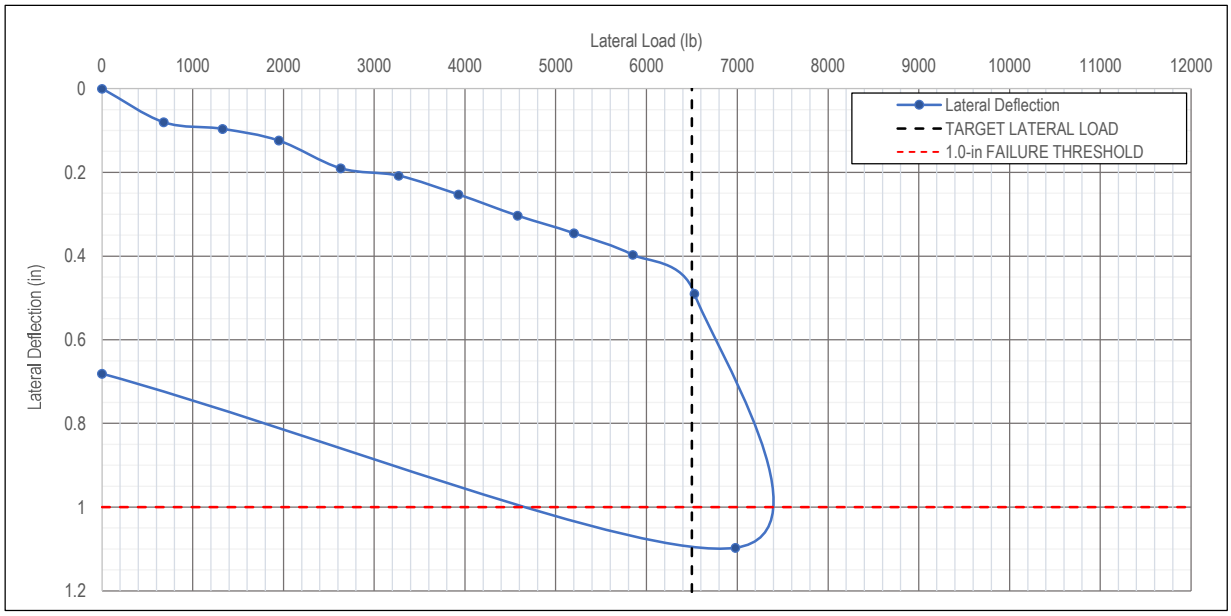
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	680	0.086	0.074	0.08
62%	20%	1,300	1330	0.104	0.088	0.10
93%	30%	1,950	1950	0.134	0.114	0.12
124%	40%	2,600	2630	0.204	0.176	0.19
155%	50%	3,250	3270	0.222	0.194	0.21
186%	60%	3,900	3930	0.268	0.238	0.25
217%	70%	4,550	4580	0.322	0.284	0.30
248%	80%	5,200	5200	0.366	0.324	0.35
279%	90%	5,850	5850	0.42	0.374	0.40
310%	100%	6,500	6530	0.522	0.458	0.49
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6980	1.172	1.022	1.10
0%	0%	0	0	0.734	0.628	0.68

Load at 1.0-in lateral deflection: **6,908** **lb**

Residual lateral deformation: **0.68** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-61A

Axial Uplift Load Test Results

PTL - **61** Designation: **A**

Section: W6x9

Latitude: 33.644706

Longitude: -114.71697

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 9.18

Pile Damaged? No

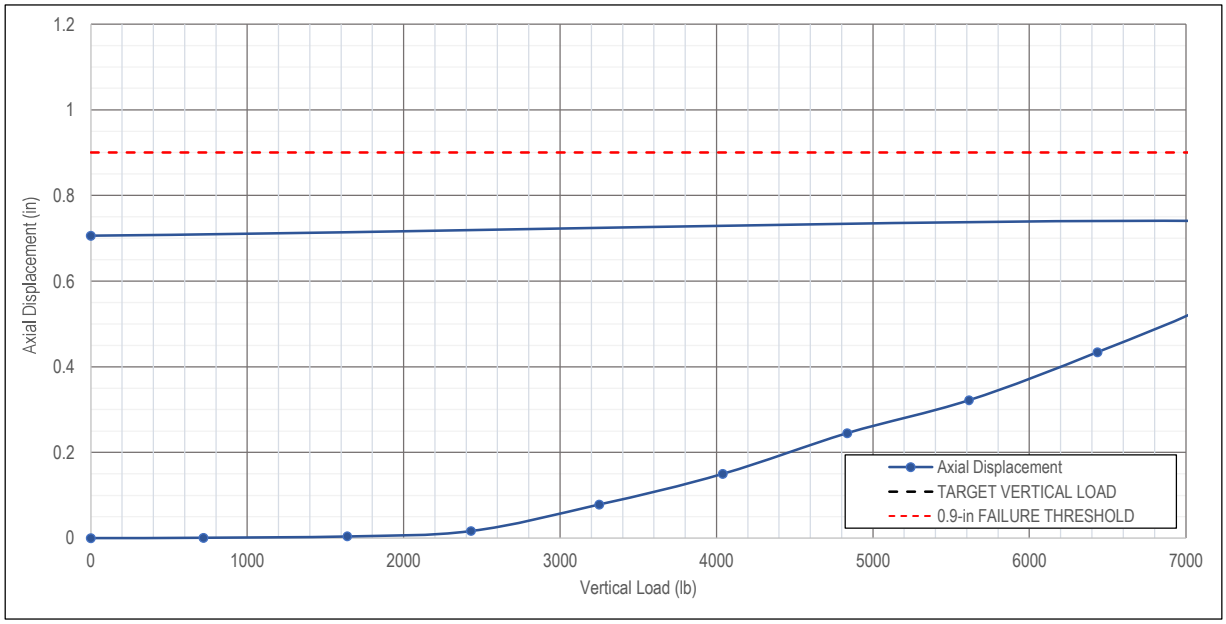
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	720	0.0005	0.001	0.00
13594%	20%	1,600	1640	0.0035	0.0045	0.00
20391%	30%	2,400	2430	0.0165	0.0165	0.02
27188%	40%	3,200	3250	0.0785	0.078	0.08
33985%	50%	4,000	4040	0.1505	0.1495	0.15
40782%	60%	4,800	4835	0.2455	0.2445	0.25
47579%	70%	5,600	5615	0.3225	0.3215	0.32
54376%	80%	6,400	6435	0.434	0.434	0.43
61173%	90%	7,200	7225	0.558	0.557	0.56
67970%	100%	8,000	8020	0.7345	0.732	0.73
0%	0%	0	0	0.706	0.706	0.71

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.71** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-61A

Lateral Load Test Results

PTL- **61** Designation: **A**

Section: W6x9

Latitude: 33.644706

Longitude: -114.71697

Tested By: ANS

Test Date: 10/29/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 9.18

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

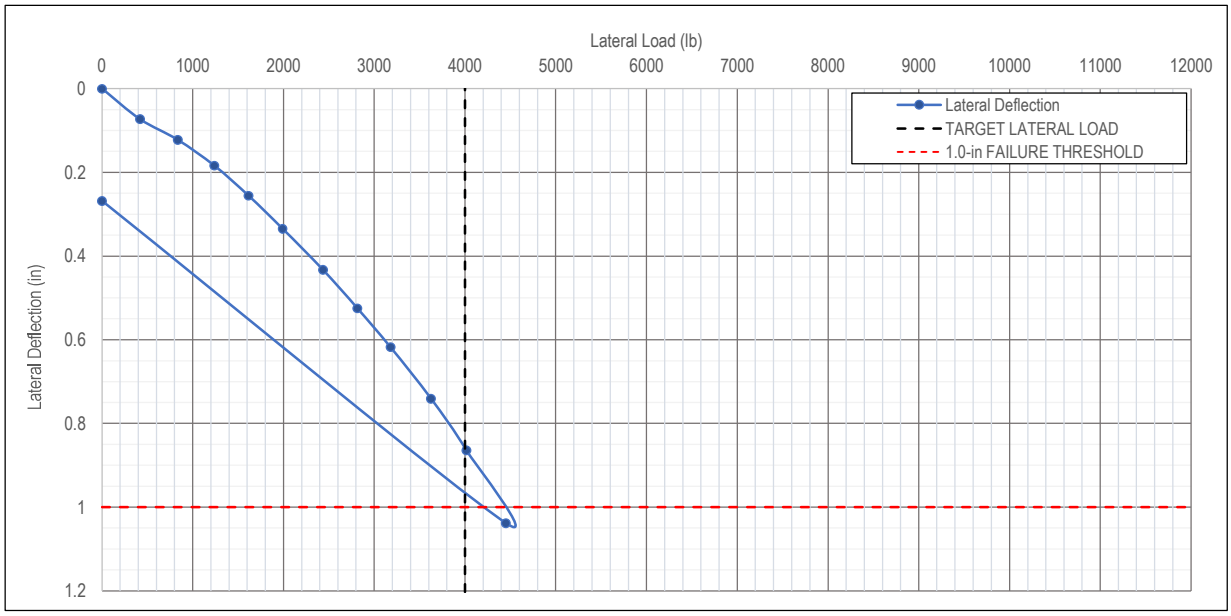
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	420	0.07	0.0755	0.07
62%	20%	800	835	0.115	0.129	0.12
93%	30%	1,200	1240	0.1745	0.193	0.18
124%	40%	1,600	1615	0.245	0.266	0.26
155%	50%	2,000	1990	0.322	0.347	0.33
186%	60%	2,400	2435	0.418	0.4475	0.43
217%	70%	2,800	2815	0.508	0.5415	0.52
248%	80%	3,200	3180	0.599	0.636	0.62
279%	90%	3,600	3625	0.72	0.761	0.74
310%	100%	4,000	4015	0.84	0.888	0.86
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			4450	1.0075	1.069	1.04
0%	0%	0	0	0.2695	0.267	0.27

Load at 1.0-in lateral deflection: **4,355** **lb**

Residual lateral deformation: **0.27** **in**



Notes:

Failure reached at 4,450 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-61B

Axial Uplift Load Test Results

PTL - **61** Designation: **B**

Section: W6x9

Latitude: 33.644706

Longitude: -114.71697

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 19.41

Pile Damaged? No

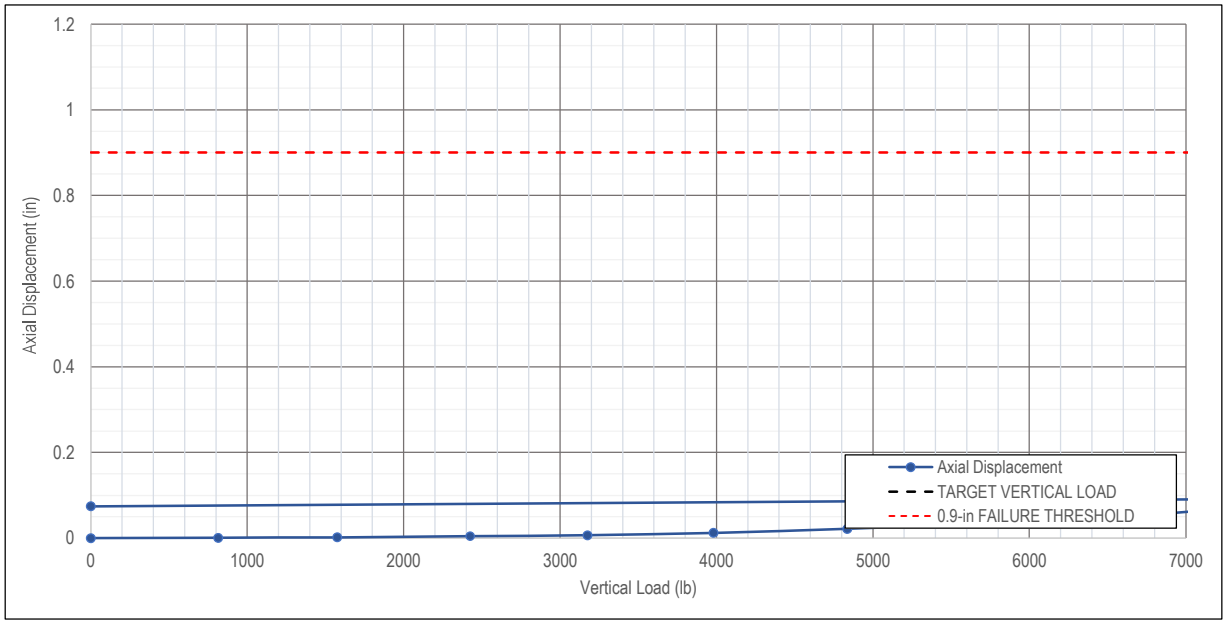
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	815	0.0015	0.0005	0.00
13594%	20%	1,600	1575	0.003	0.001	0.00
20391%	30%	2,400	2425	0.0065	0.003	0.00
27188%	40%	3,200	3175	0.0085	0.005	0.01
33985%	50%	4,000	3980	0.0145	0.0105	0.01
40782%	60%	4,800	4835	0.0235	0.0195	0.02
47579%	70%	5,600	5605	0.036	0.0305	0.03
54376%	80%	6,400	6380	0.047	0.0415	0.04
61173%	90%	7,200	7250	0.072	0.0655	0.07
67970%	100%	8,000	7975	0.0945	0.0885	0.09
0%	0%	0	0	0.0745	0.074	0.07

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.07** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-61B

Lateral Load Test Results

PTL- **61** Designation: **B**

Section: W6x9

Latitude: 33.644706

Longitude: -114.71697

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 19.41

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

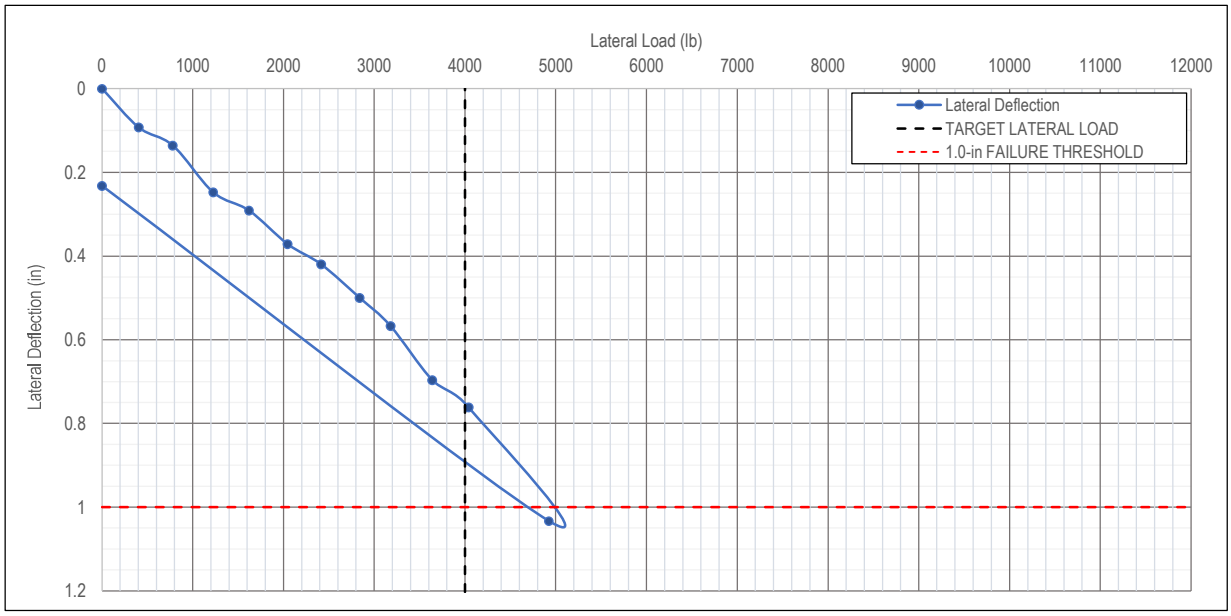
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	405	0.088	0.0965	0.09
62%	20%	800	780	0.1265	0.1445	0.14
93%	30%	1,200	1225	0.2345	0.26	0.25
124%	40%	1,600	1620	0.277	0.305	0.29
155%	50%	2,000	2045	0.3545	0.388	0.37
186%	60%	2,400	2415	0.401	0.4385	0.42
217%	70%	2,800	2840	0.478	0.5215	0.50
248%	80%	3,200	3180	0.542	0.5915	0.57
279%	90%	3,600	3640	0.6645	0.7285	0.70
310%	100%	4,000	4040	0.72335	0.7995	0.76
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			4925	0.949	1.1175	1.03
0%	0%	0	0	0.198	0.2665	0.23

Load at 1.0-in lateral deflection: **4,817** **lb**

Residual lateral deformation: **0.23** **in**



Notes:

Failure reached at 4,925 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-62A

Axial Uplift Load Test Results

PTL - **62** Designation: **A**

Section: W6x9

Latitude: 33.644035

Longitude: -114.71255

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 21.02

Pile Damaged? No

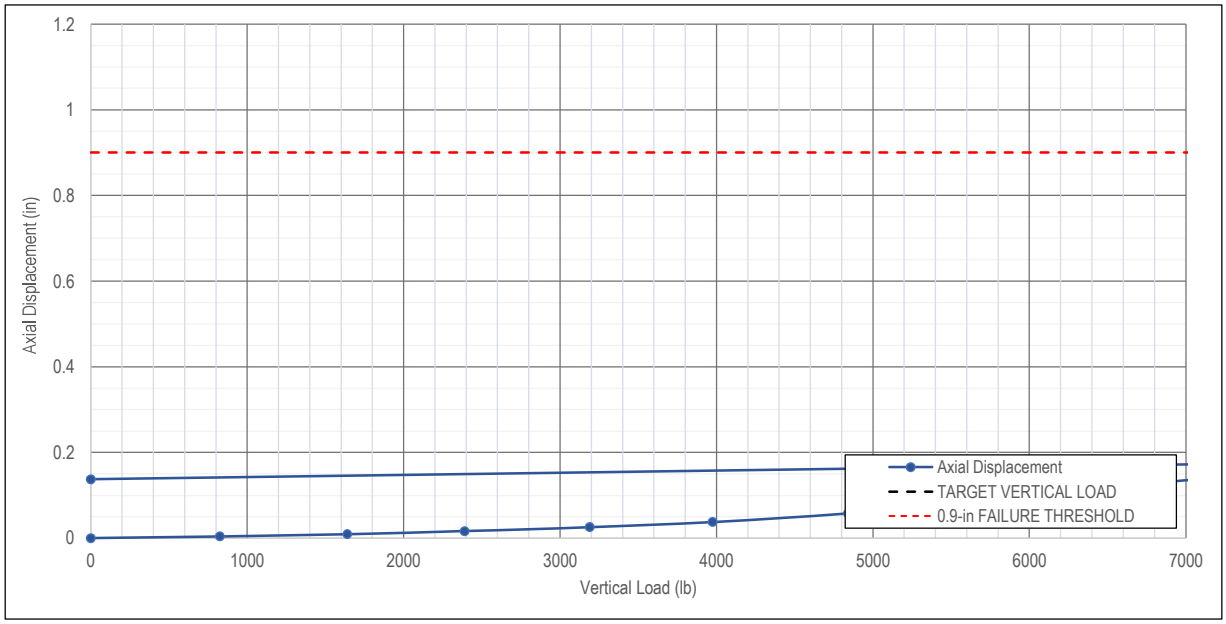
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	825	0.004	0.0045	0.00
436%	20%	1,600	1640	0.0085	0.01	0.01
654%	30%	2,400	2390	0.015	0.0175	0.02
872%	40%	3,200	3190	0.024	0.027	0.03
1090%	50%	4,000	3975	0.036	0.0395	0.04
1308%	60%	4,800	4840	0.056	0.0595	0.06
1526%	70%	5,600	5595	0.079	0.0835	0.08
1744%	80%	6,400	6430	0.1145	0.1195	0.12
1962%	90%	7,200	7230	0.1405	0.1455	0.14
2180%	100%	8,000	8025	0.172	0.178	0.18
0%	0%	0	0	0.137	0.138	0.14

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.14** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-62A

Lateral Load Test Results

PTL- **62** Designation: **A**

Section: W6x9

Latitude: 33.644035

Longitude: -114.71255

Tested By: ANS

Test Date: 10/29/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 21.02

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

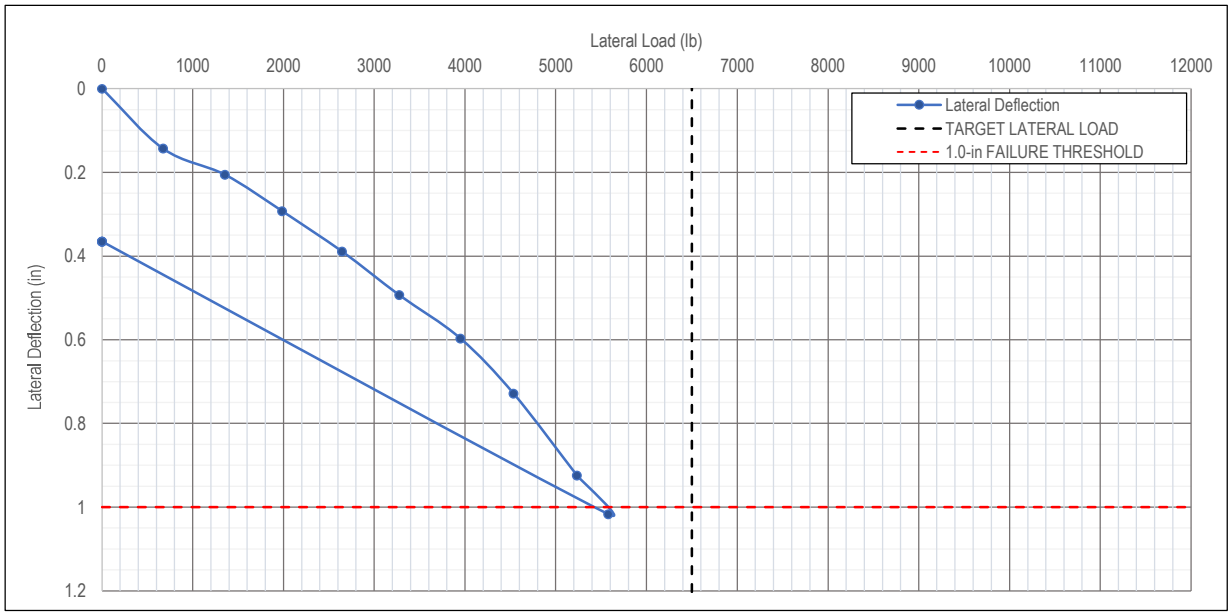
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	675	0.141	0.1455	0.14
62%	20%	1,300	1355	0.203	0.2085	0.21
93%	30%	1,950	1985	0.29	0.2955	0.29
124%	40%	2,600	2645	0.3865	0.392	0.39
155%	50%	3,250	3275	0.49	0.496	0.49
186%	60%	3,900	3950	0.593	0.6005	0.60
217%	70%	4,550	4535	0.719	0.738	0.73
248%	80%	5,200	5235	0.8945	0.954	0.92
279%	90%	5,850	5580	0.966	1.0675	1.02
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.3265	0.4045	0.37

Load at 1.0-in lateral deflection: **5,518** **lb**

Residual lateral deformation: **0.37** **in**



Notes:

Failure reached at 5,580 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-62B

Axial Uplift Load Test Results

PTL - **62** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.644035 - - Saturated Soil? No

Longitude: -114.71255 Target Embedment Depth (ft): 8.00 Drive Time (s): 22.43

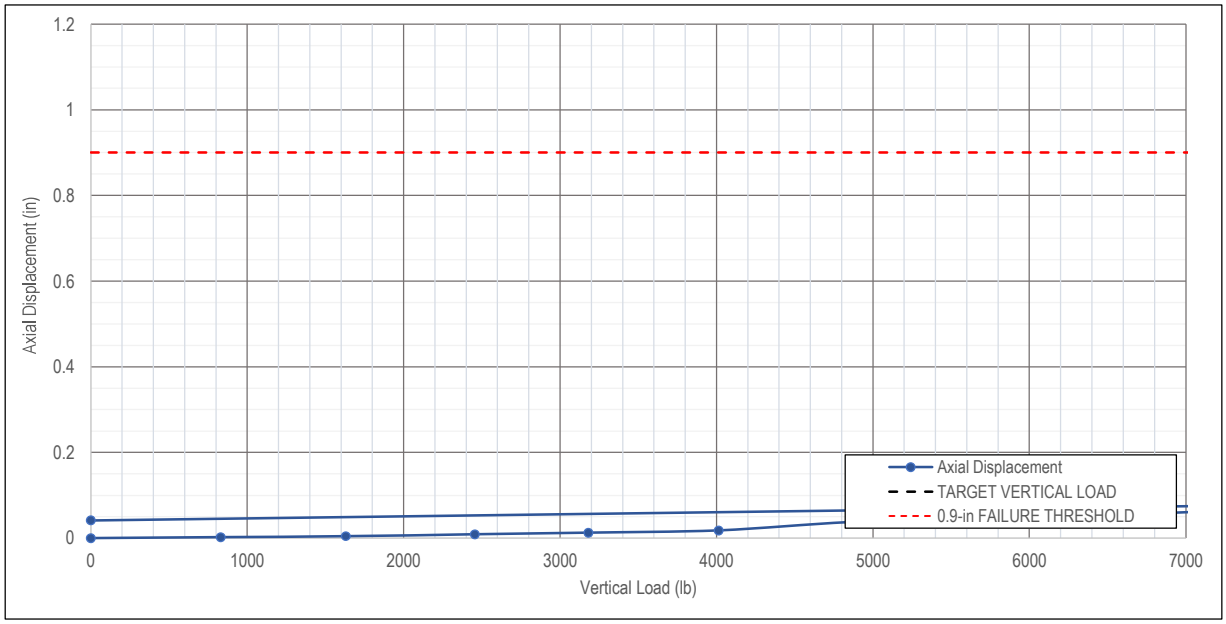
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/20/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	830	0.0025	0.002	0.00
436%	20%	1,600	1630	0.0045	0.0045	0.00
654%	30%	2,400	2455	0.0095	0.0085	0.01
872%	40%	3,200	3180	0.0135	0.012	0.01
1090%	50%	4,000	4015	0.0185	0.018	0.02
1308%	60%	4,800	4850	0.036	0.0385	0.04
1526%	70%	5,600	5610	0.0435	0.0465	0.05
1744%	80%	6,400	6360	0.0505	0.054	0.05
1962%	90%	7,200	7205	0.0615	0.066	0.06
2180%	100%	8,000	8020	0.0765	0.081	0.08
0%	0%	0	0	0.0405	0.042	0.04

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.04** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-62B

Lateral Load Test Results

PTL- **62** Designation: **B**

Section: W6x9

Latitude: 33.644035

Longitude: -114.71255

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 22.43

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

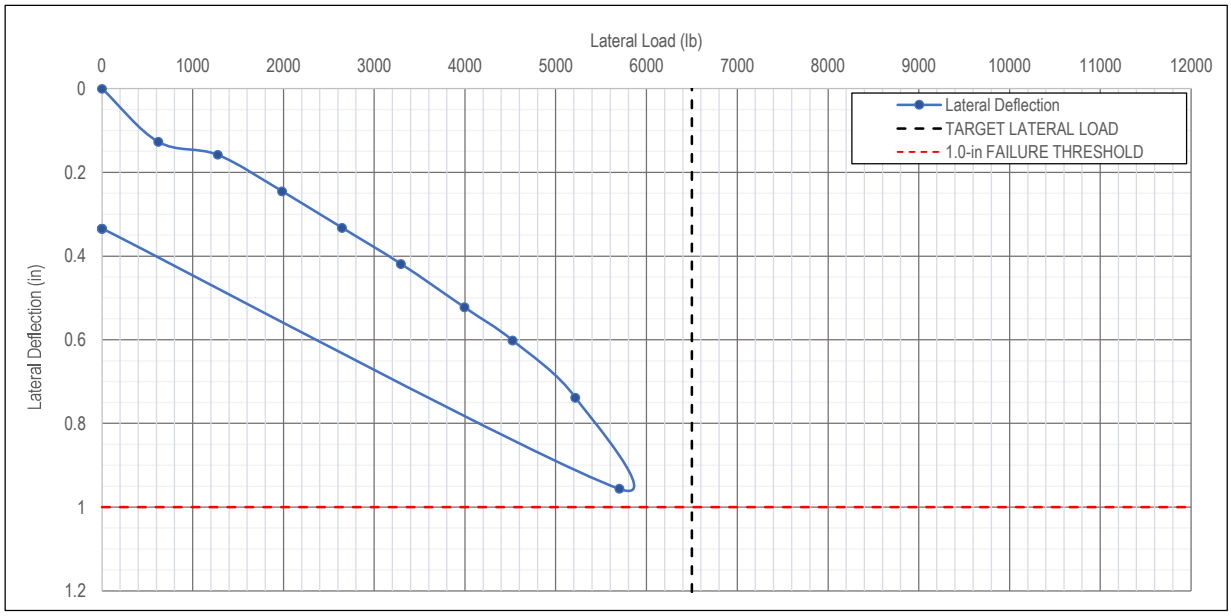
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	620	0.1255	0.1285	0.13
62%	20%	1,300	1275	0.1555	0.16	0.16
93%	30%	1,950	1985	0.2345	0.2565	0.25
124%	40%	2,600	2645	0.3195	0.3455	0.33
155%	50%	3,250	3295	0.403	0.434	0.42
186%	60%	3,900	3995	0.5025	0.5425	0.52
217%	70%	4,550	4525	0.575	0.6285	0.60
248%	80%	5,200	5215	0.6915	0.785	0.74
279%	90%	5,850	5700	0.7745	1.138	0.96
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.214	0.4545	0.33

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.33** **in**



Notes:

Pile twisting at 5,700 lbs. No reliable test results available after this point.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-62D

Axial Uplift Load Test Results

PTL - **62** Designation: **D**

Section: **W6x12**

Latitude: 33.644035

Longitude: -114.71255

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 11.44

Pile Damaged? No

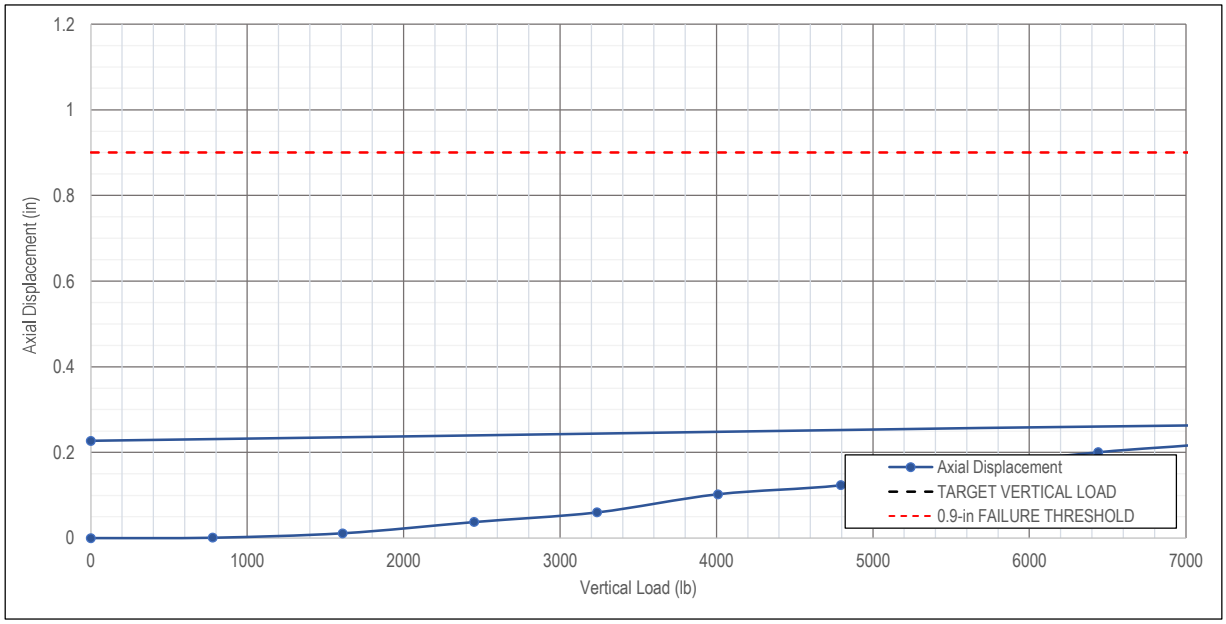
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	780	0.0015	0.001	0.00
436%	20%	1,600	1610	0.0115	0.0115	0.01
654%	30%	2,400	2450	0.0375	0.0375	0.04
872%	40%	3,200	3235	0.0605	0.06	0.06
1090%	50%	4,000	4010	0.1025	0.102	0.10
1308%	60%	4,800	4795	0.1235	0.1235	0.12
1526%	70%	5,600	5630	0.1665	0.1665	0.17
1744%	80%	6,400	6440	0.2005	0.2005	0.20
1962%	90%	7,200	7210	0.223	0.223	0.22
2180%	100%	8,000	8020	0.265	0.266	0.27
0%	0%	0	0	0.2265	0.2275	0.23

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.23** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-62D

Lateral Load Test Results

PTL- **62** Designation: **D**

Section: **W6x12**

Latitude: 33.644035

Longitude: -114.71255

Tested By: ANS

Test Date: 10/23/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.44

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

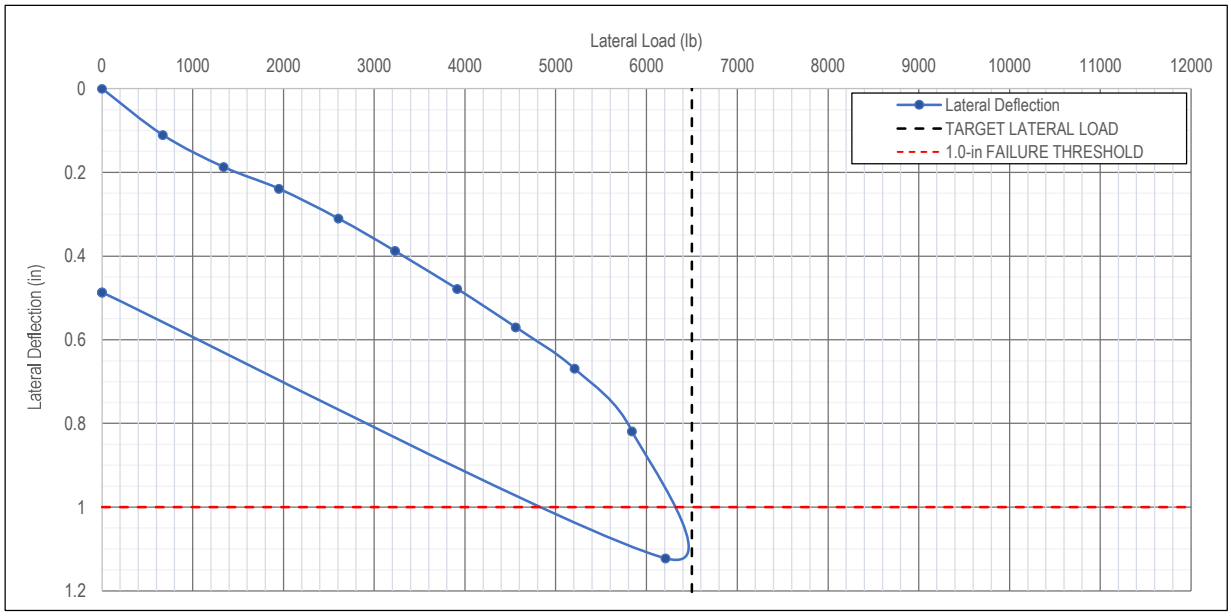
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	670	0.1085	0.113	0.11
62%	20%	1,300	1340	0.184	0.19	0.19
93%	30%	1,950	1950	0.236	0.242	0.24
124%	40%	2,600	2605	0.3065	0.3135	0.31
155%	50%	3,250	3230	0.3845	0.3915	0.39
186%	60%	3,900	3915	0.476	0.4815	0.48
217%	70%	4,550	4560	0.5675	0.5735	0.57
248%	80%	5,200	5210	0.6655	0.673	0.67
279%	90%	5,850	5840	0.8195	0.8185	0.82
310%	100%	6,500	6210	1.151	1.0935	1.12
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.457	0.5165	0.49

Load at 1.0-in lateral deflection: **6,061** **lb**

Residual lateral deformation: **0.49** **in**



Notes:

Failure reached at 6,210 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-63A

Axial Uplift Load Test Results

PTL - **63** Designation: **A**

Section: W6x9

Latitude: 33.644823

Longitude: -114.6777

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 11.42

Pile Damaged? No

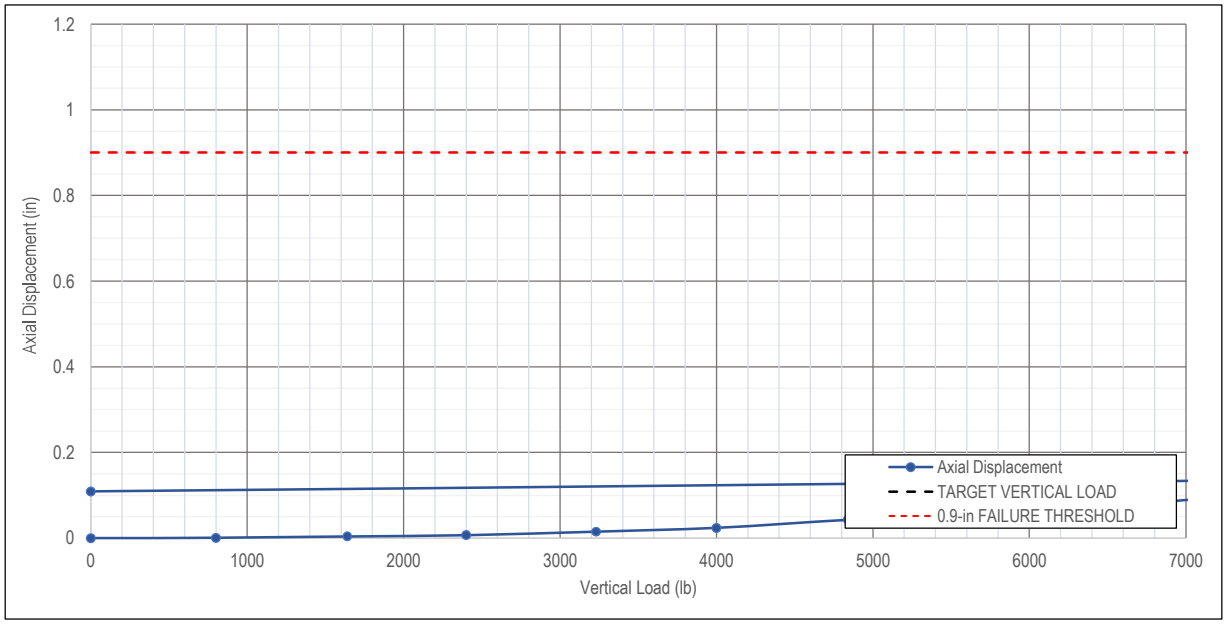
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	800	0.002	0	0.00
13594%	20%	1,600	1640	0.004	0.004	0.00
20391%	30%	2,400	2400	0.006	0.008	0.01
27188%	40%	3,200	3230	0.014	0.016	0.02
33985%	50%	4,000	4000	0.022	0.026	0.02
40782%	60%	4,800	4840	0.04	0.046	0.04
47579%	70%	5,600	5630	0.052	0.058	0.06
54376%	80%	6,400	6420	0.068	0.076	0.07
61173%	90%	7,200	7230	0.092	0.102	0.10
67970%	100%	8,000	8040	0.13	0.14	0.14
0%	0%	0	0	0.108	0.11	0.11

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.11** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-63A

Lateral Load Test Results

PTL- **63** Designation: **A**

Section: W6x9

Latitude: 33.644823

Longitude: -114.6777

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.42

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

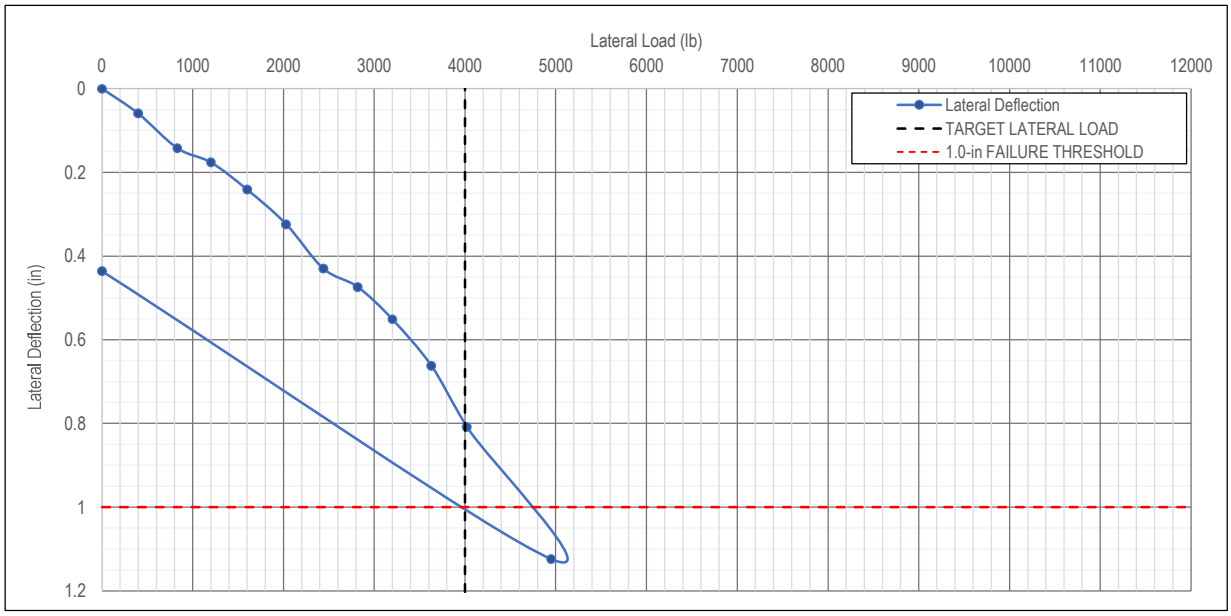
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.06	0.058	0.06
62%	20%	800	830	0.144	0.14	0.14
93%	30%	1,200	1200	0.178	0.174	0.18
124%	40%	1,600	1600	0.242	0.24	0.24
155%	50%	2,000	2030	0.326	0.322	0.32
186%	60%	2,400	2440	0.432	0.428	0.43
217%	70%	2,800	2820	0.476	0.472	0.47
248%	80%	3,200	3200	0.552	0.55	0.55
279%	90%	3,600	3630	0.662	0.662	0.66
310%	100%	4,000	4020	0.802	0.814	0.81
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			4950	1.092	1.156	1.12
0%	0%	0	0	0.404	0.468	0.44

Load at 1.0-in lateral deflection: **4,585** **lb**

Residual lateral deformation: **0.44** **in**



Notes:

Failure reached at 4,950 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-63B

Axial Uplift Load Test Results

PTL - **63** Designation: **B**

Section: W6x9

Latitude: 33.644823

Longitude: -114.6777

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 20.61

Pile Damaged? No

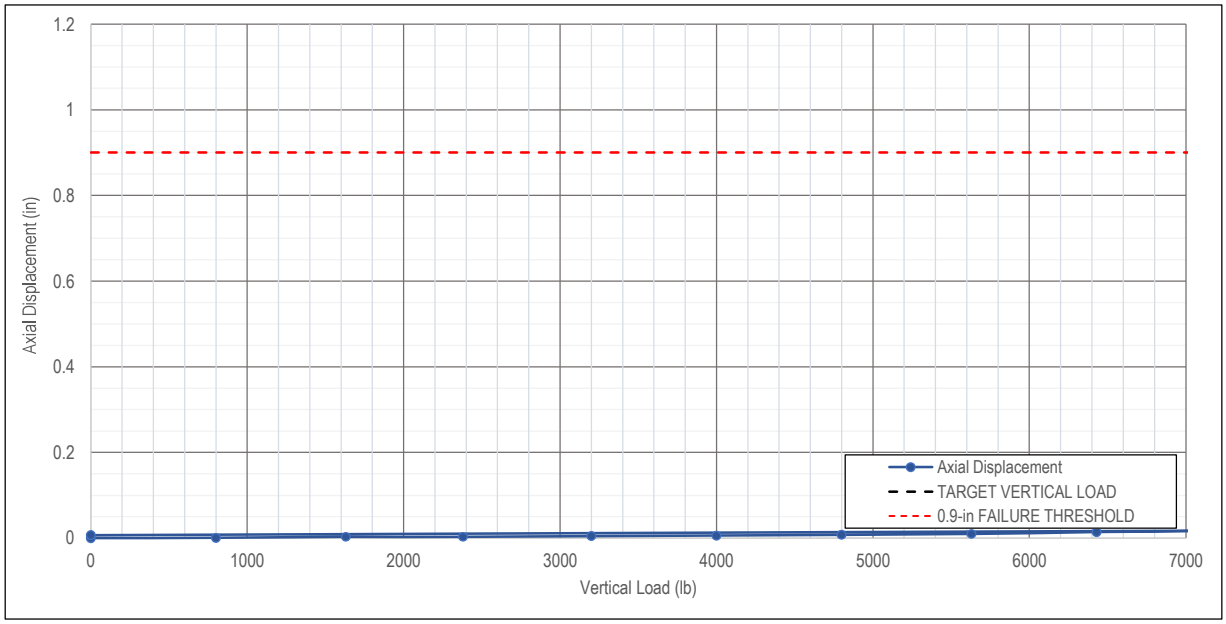
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	800	0.002	0	0.00
13594%	20%	1,600	1630	0.004	0.002	0.00
20391%	30%	2,400	2380	0.004	0.002	0.00
27188%	40%	3,200	3200	0.006	0.004	0.01
33985%	50%	4,000	4000	0.008	0.004	0.01
40782%	60%	4,800	4800	0.008	0.008	0.01
47579%	70%	5,600	5630	0.01	0.01	0.01
54376%	80%	6,400	6430	0.012	0.016	0.01
61173%	90%	7,200	7220	0.014	0.02	0.02
67970%	100%	8,000	8030	0.016	0.022	0.02
0%	0%	0	0	0.006	0.008	0.01

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-63B

Lateral Load Test Results

PTL- **63** Designation: **B**

Section: W6x9

Latitude: 33.644823

Longitude: -114.6777

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 20.61

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

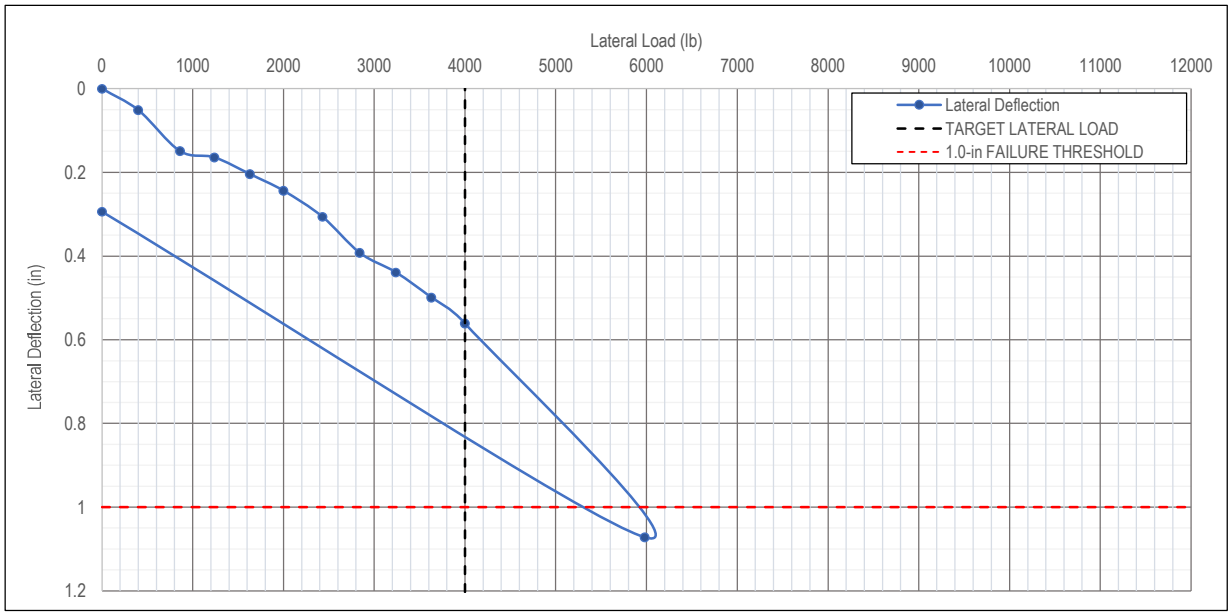
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.054	0.048	0.05
62%	20%	800	860	0.156	0.142	0.15
93%	30%	1,200	1240	0.172	0.156	0.16
124%	40%	1,600	1630	0.212	0.196	0.20
155%	50%	2,000	2000	0.254	0.234	0.24
186%	60%	2,400	2430	0.318	0.294	0.31
217%	70%	2,800	2840	0.406	0.378	0.39
248%	80%	3,200	3240	0.454	0.424	0.44
279%	90%	3,600	3630	0.516	0.482	0.50
310%	100%	4,000	4000	0.58	0.542	0.56
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5980	1.104	1.04	1.07
0%	0%	0	0	0.3	0.288	0.29

Load at 1.0-in lateral deflection: **5,701** **lb**

Residual lateral deformation: **0.29** **in**



Notes:

Failure reached at 5,980 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-64A

Axial Uplift Load Test Results

PTL - **64** Designation: **A**

Section: W6x9

Latitude: 33.644396

Longitude: -114.67304

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 18.9

Pile Damaged? No

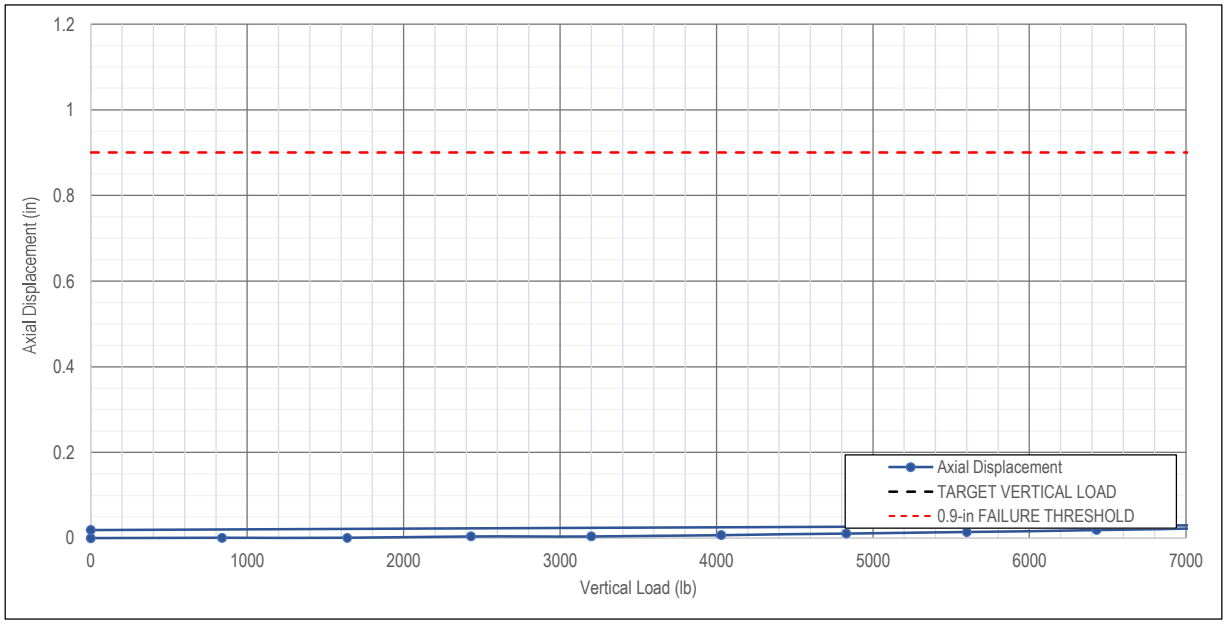
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	840	0	0.002	0.00
436%	20%	1,600	1640	0	0.002	0.00
654%	30%	2,400	2430	0.004	0.004	0.00
872%	40%	3,200	3200	0.004	0.004	0.00
1090%	50%	4,000	4030	0.008	0.006	0.01
1308%	60%	4,800	4830	0.012	0.01	0.01
1526%	70%	5,600	5600	0.016	0.012	0.01
1744%	80%	6,400	6430	0.022	0.016	0.02
1962%	90%	7,200	7240	0.028	0.02	0.02
2180%	100%	8,000	8000	0.038	0.024	0.03
0%	0%	0	0	0.02	0.018	0.02

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.02** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-64A

Lateral Load Test Results

PTL- **64** Designation: **A**

Section: W6x9

Latitude: 33.644396

Longitude: -114.67304

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 18.9

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

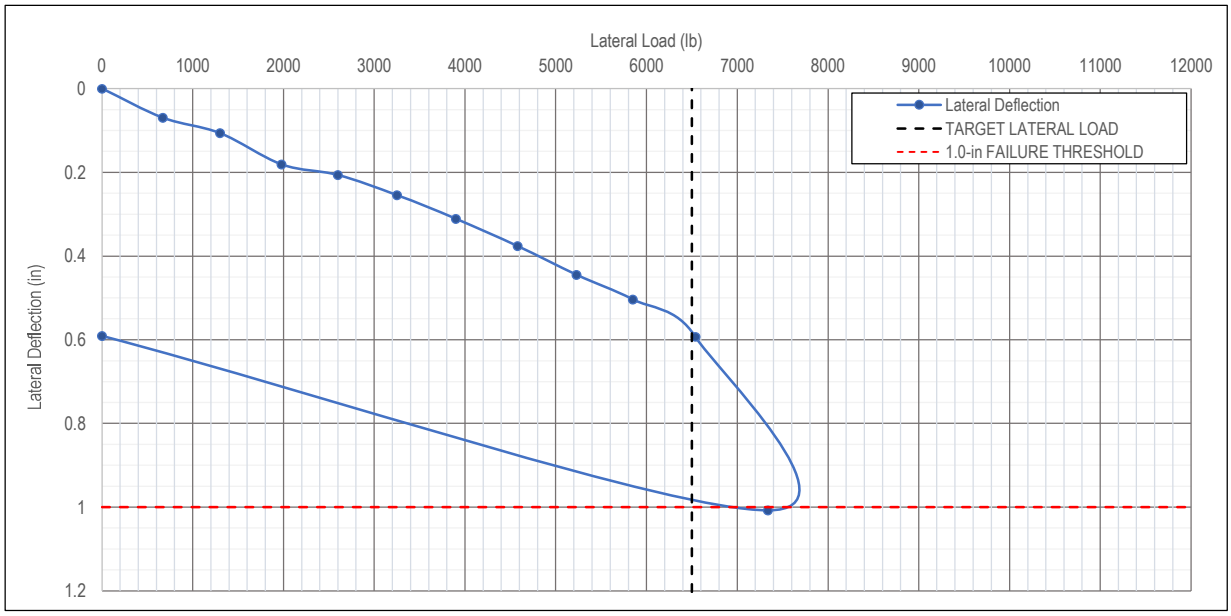
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	670	0.07	0.068	0.07
62%	20%	1,300	1300	0.108	0.104	0.11
93%	30%	1,950	1980	0.186	0.176	0.18
124%	40%	2,600	2600	0.212	0.2	0.21
155%	50%	3,250	3250	0.262	0.246	0.25
186%	60%	3,900	3900	0.322	0.3	0.31
217%	70%	4,550	4580	0.39	0.362	0.38
248%	80%	5,200	5230	0.464	0.426	0.45
279%	90%	5,850	5850	0.528	0.48	0.50
310%	100%	6,500	6540	0.628	0.558	0.59
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7340	1.116	0.9	1.01
0%	0%	0	0	0.598	0.584	0.59

Load at 1.0-in lateral deflection: **7,325** **lb**

Residual lateral deformation: **0.59** **in**



Notes:

Failure reached at 7,340 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-64B

Axial Uplift Load Test Results

PTL - **64** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.644396 - - Saturated Soil? No

Longitude: -114.67304 Target Embedment Depth (ft): 8.00 Drive Time (s): 24.4

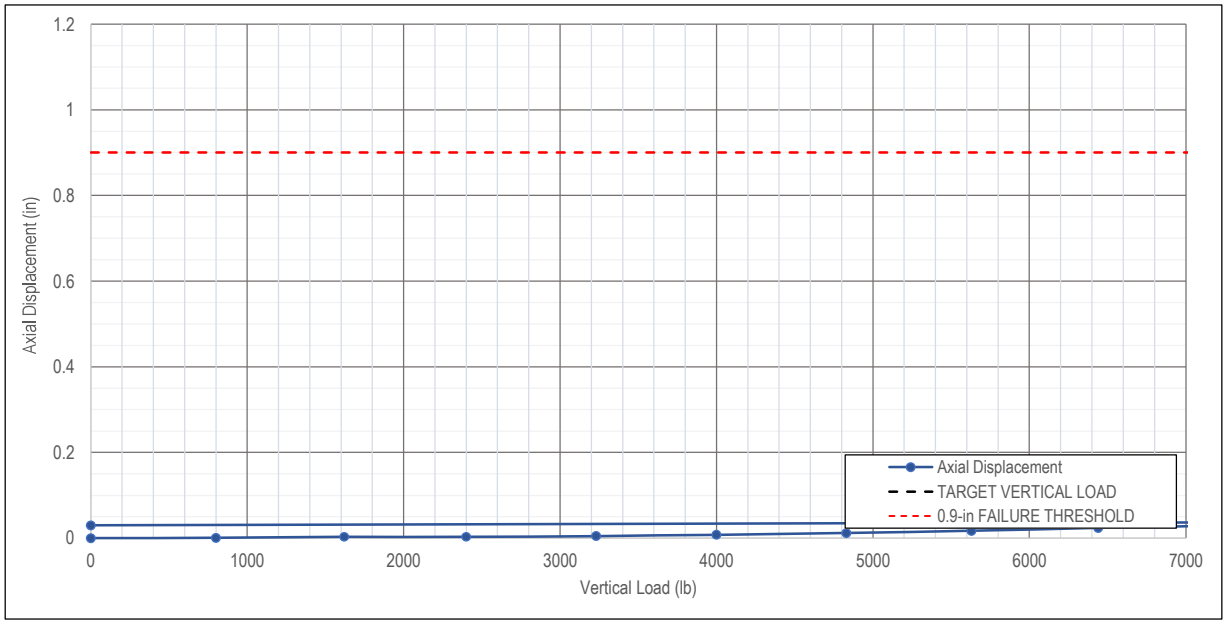
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/26/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0	0.002	0.00
436%	20%	1,600	1620	0.002	0.004	0.00
654%	30%	2,400	2400	0.002	0.004	0.00
872%	40%	3,200	3230	0.004	0.006	0.01
1090%	50%	4,000	4000	0.008	0.008	0.01
1308%	60%	4,800	4830	0.012	0.012	0.01
1526%	70%	5,600	5630	0.018	0.016	0.02
1744%	80%	6,400	6440	0.026	0.022	0.02
1962%	90%	7,200	7240	0.032	0.026	0.03
2180%	100%	8,000	8030	0.04	0.034	0.04
0%	0%	0	0	0.03	0.03	0.03

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-64B

Lateral Load Test Results

PTL- **64** Designation: **B**

Section: W6x9

Latitude: 33.644396

Longitude: -114.67304

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 24.4

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

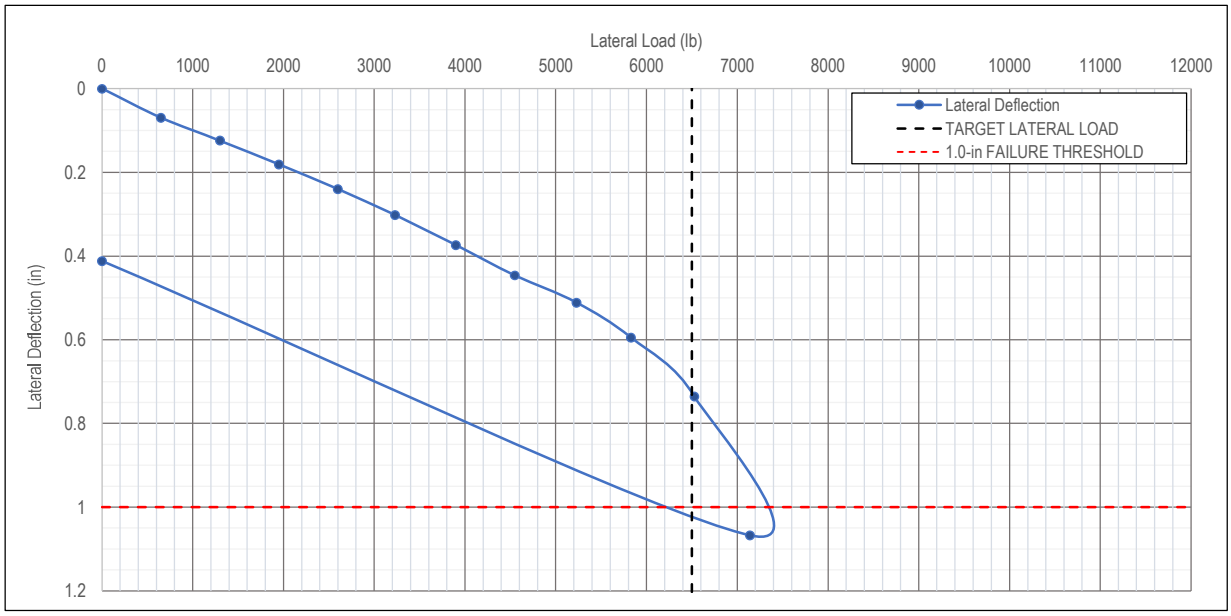
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.066	0.072	0.07
62%	20%	1,300	1300	0.122	0.126	0.12
93%	30%	1,950	1950	0.18	0.182	0.18
124%	40%	2,600	2600	0.24	0.24	0.24
155%	50%	3,250	3230	0.304	0.3	0.30
186%	60%	3,900	3900	0.378	0.37	0.37
217%	70%	4,550	4550	0.452	0.44	0.45
248%	80%	5,200	5230	0.518	0.504	0.51
279%	90%	5,850	5830	0.606	0.584	0.60
310%	100%	6,500	6530	0.752	0.72	0.74
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			7140	1.12	1.014	1.07
0%	0%	0	0	0.442	0.382	0.41

Load at 1.0-in lateral deflection: **7,017** **lb**

Residual lateral deformation: **0.41** **in**



Notes:

Failure reached at 7,140 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-65A

Axial Uplift Load Test Results

PTL - **65** Designation: **A**

Section: W6x9

Latitude: 33.644396

Longitude: -114.6701

Tested By: ANS

Test Date: 10/27/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 14.85

Pile Damaged? No

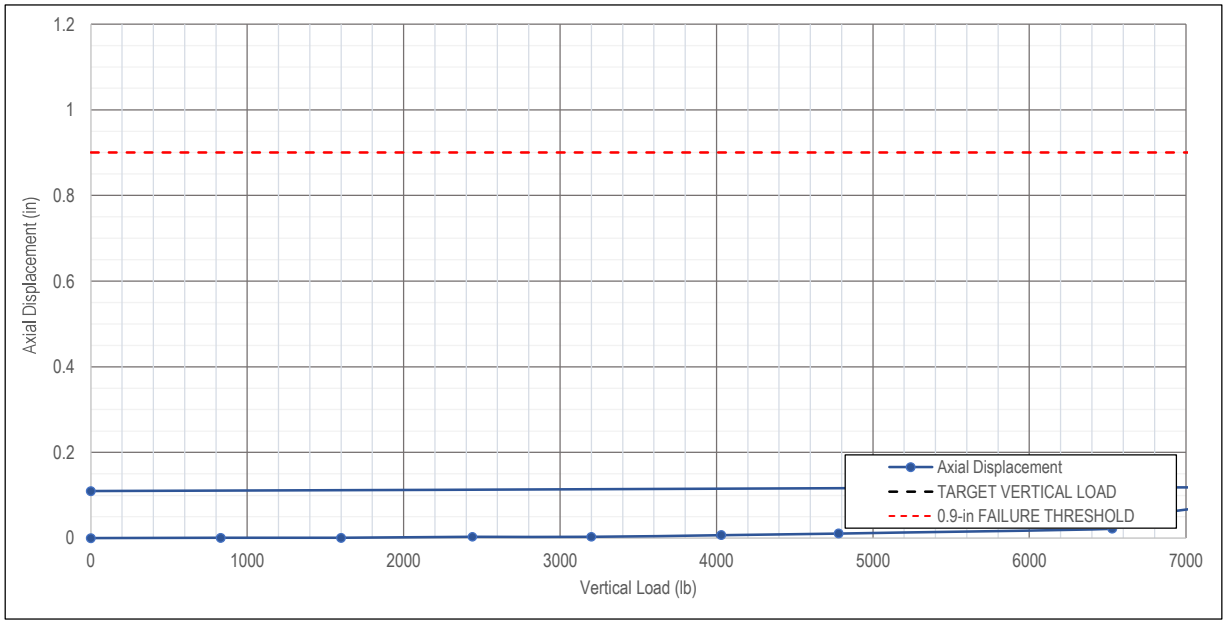
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	830	0	0.002	0.00
13594%	20%	1,600	1600	0	0.002	0.00
20391%	30%	2,400	2440	0.002	0.004	0.00
27188%	40%	3,200	3200	0.002	0.004	0.00
33985%	50%	4,000	4030	0.006	0.008	0.01
40782%	60%	4,800	4780	0.01	0.012	0.01
47579%	70%	5,600	6530	0.022	0.022	0.02
54376%	80%	6,400	6400	0.036	0.036	0.04
61173%	90%	7,200	7230	0.078	0.078	0.08
67970%	100%	8,000	8000	0.118	0.116	0.12
0%	0%	0	0	0.11	0.11	0.11

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.11** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-65A

Lateral Load Test Results

PTL- **65** Designation: **A**

Section: W6x9

Latitude: 33.644396

Longitude: -114.6701

Tested By: ANS

Test Date: 10/27/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 14.85

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

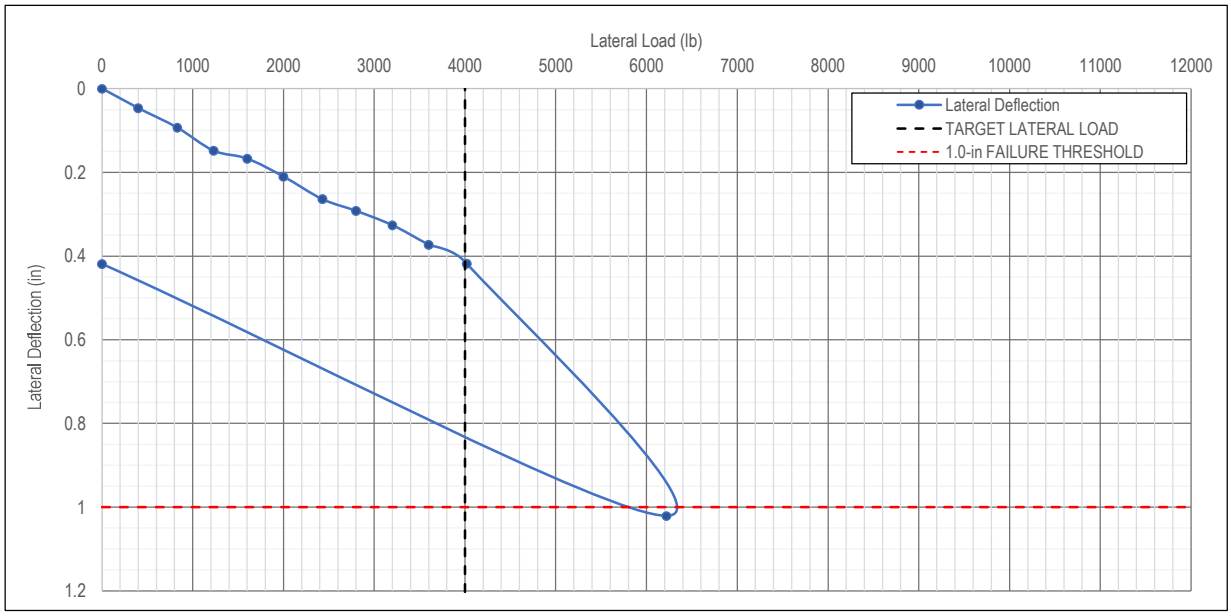
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.046	0.046	0.05
62%	20%	800	830	0.094	0.092	0.09
93%	30%	1,200	1230	0.148	0.148	0.15
124%	40%	1,600	1600	0.168	0.166	0.17
155%	50%	2,000	2000	0.212	0.208	0.21
186%	60%	2,400	2430	0.266	0.262	0.26
217%	70%	2,800	2800	0.294	0.29	0.29
248%	80%	3,200	3200	0.328	0.324	0.33
279%	90%	3,600	3600	0.374	0.372	0.37
310%	100%	4,000	4020	0.418	0.418	0.42
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6220	0.938	1.104	1.02
0%	0%	0	0	0.322	0.516	0.42

Load at 1.0-in lateral deflection: **6,143** **lb**

Residual lateral deformation: **0.42** **in**



Notes:

Failure reached at 6,220 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-65B

Axial Uplift Load Test Results

PTL - **65** Designation: **B**

Section: W6x9

Latitude: 33.644396

Longitude: -114.6701

Tested By: ANS

Test Date: 10/27/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 14.5

Pile Damaged? No

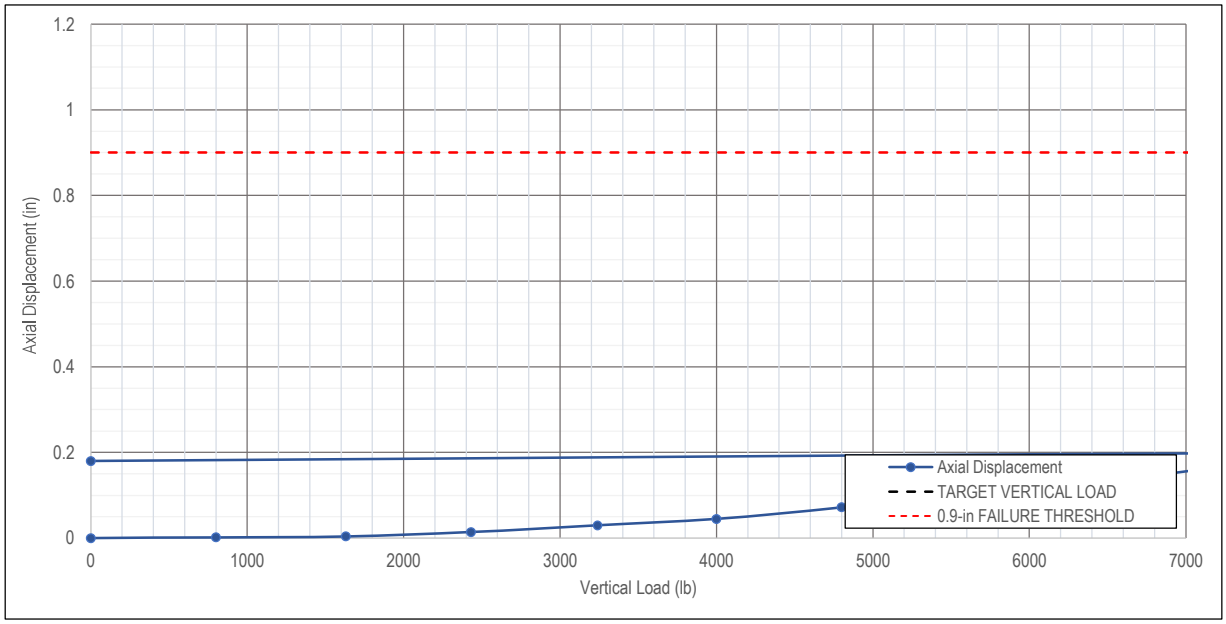
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	800	0.002	0.002	0.00
13594%	20%	1,600	1630	0.004	0.004	0.00
20391%	30%	2,400	2430	0.014	0.014	0.01
27188%	40%	3,200	3240	0.032	0.028	0.03
33985%	50%	4,000	4000	0.048	0.042	0.05
40782%	60%	4,800	4800	0.076	0.068	0.07
47579%	70%	5,600	5630	0.112	0.102	0.11
54376%	80%	6,400	6400	0.134	0.124	0.13
61173%	90%	7,200	7230	0.172	0.16	0.17
67970%	100%	8,000	8020	0.204	0.192	0.20
0%	0%	0	0	0.18	0.18	0.18

Load at 0.9-in axial displacement: N/A lb

Residual axial displacement: 0.18 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-65B

Lateral Load Test Results

PTL- **65** Designation: **B**

Section: W6x9

Latitude: 33.644396

Longitude: -114.6701

Tested By: ANS

Test Date: 10/27/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 14.5

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

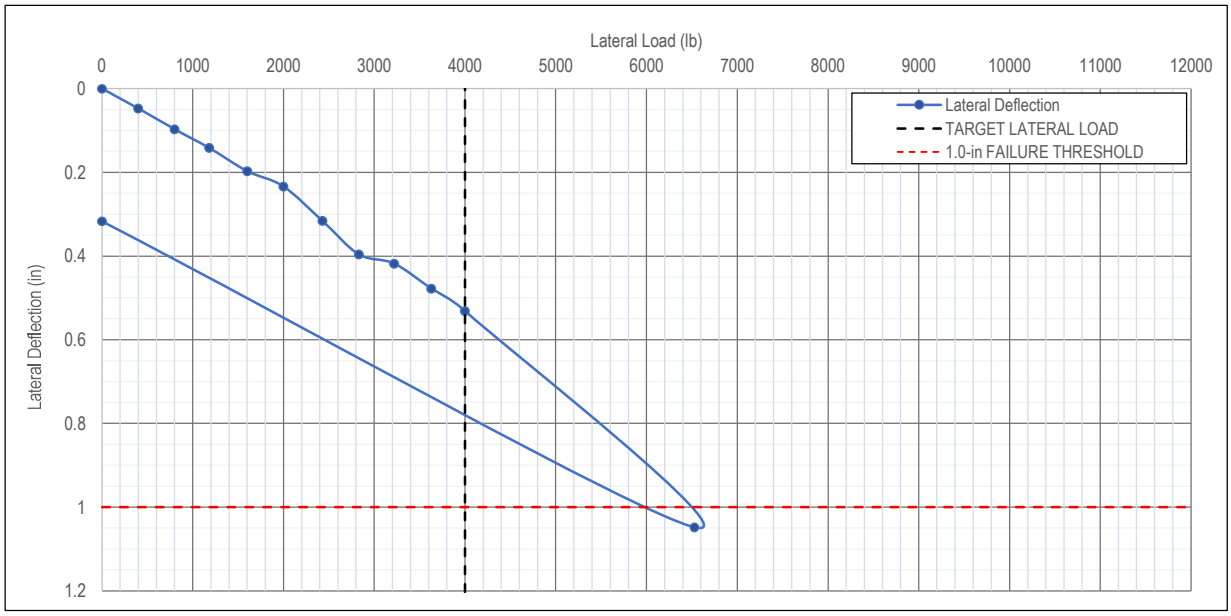
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.046	0.048	0.05
62%	20%	800	800	0.096	0.098	0.10
93%	30%	1,200	1180	0.142	0.14	0.14
124%	40%	1,600	1600	0.198	0.196	0.20
155%	50%	2,000	2000	0.236	0.232	0.23
186%	60%	2,400	2430	0.318	0.314	0.32
217%	70%	2,800	2830	0.4	0.392	0.40
248%	80%	3,200	3220	0.422	0.414	0.42
279%	90%	3,600	3630	0.482	0.474	0.48
310%	100%	4,000	4000	0.536	0.526	0.53
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6530	1.03	1.066	1.05
0%	0%	0	0	0.3	0.334	0.32

Load at 1.0-in lateral deflection: **6,295** **lb**

Residual lateral deformation: **0.32** **in**



Notes:

Failure reached at 6,530 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-66A

Axial Uplift Load Test Results

PTL - **66** Designation: **A**

Section: W6x9

Latitude: 33.644396

Longitude: -114.66715

Tested By: ANS

Test Date: 10/27/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 9.19

Pile Damaged? No

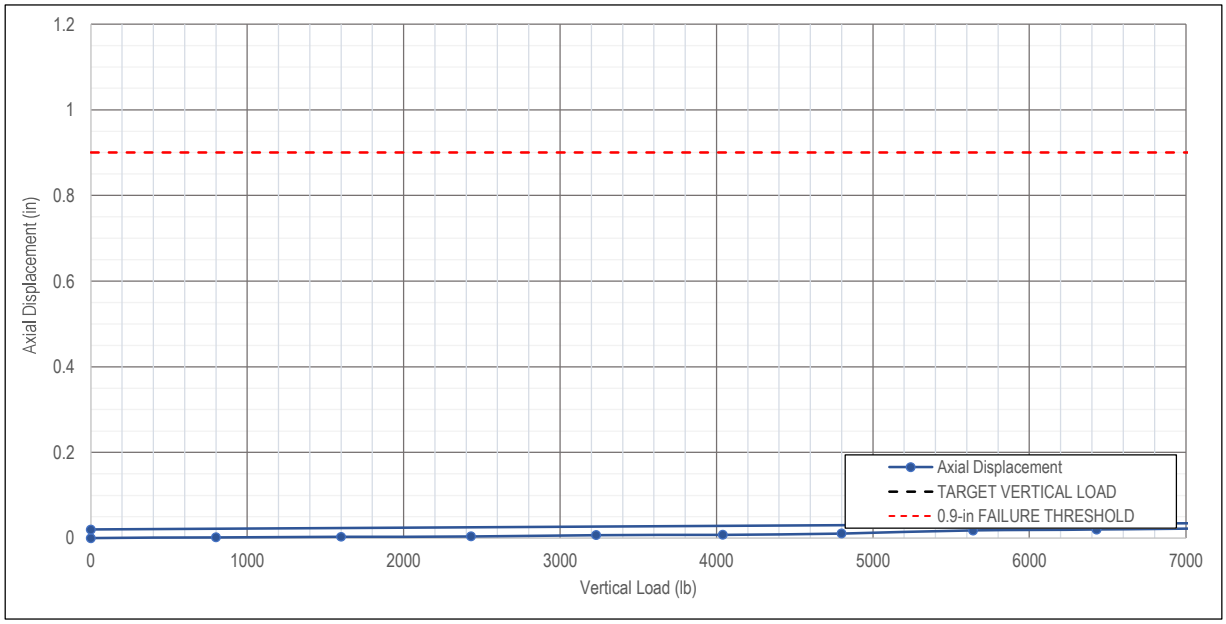
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0.002	0.002	0.00
436%	20%	1,600	1600	0.002	0.004	0.00
654%	30%	2,400	2430	0.004	0.004	0.00
872%	40%	3,200	3230	0.008	0.006	0.01
1090%	50%	4,000	4040	0.01	0.006	0.01
1308%	60%	4,800	4800	0.014	0.008	0.01
1526%	70%	5,600	5640	0.022	0.014	0.02
1744%	80%	6,400	6430	0.024	0.016	0.02
1962%	90%	7,200	7200	0.028	0.02	0.02
2180%	100%	8,000	8030	0.042	0.03	0.04
0%	0%	0	0	0.02	0.02	0.02

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.02** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-66A

Lateral Load Test Results

PTL- **66** Designation: **A**

Section: W6x9

Latitude: 33.644396

Longitude: -114.66715

Tested By: ANS

Test Date: 10/27/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 9.19

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

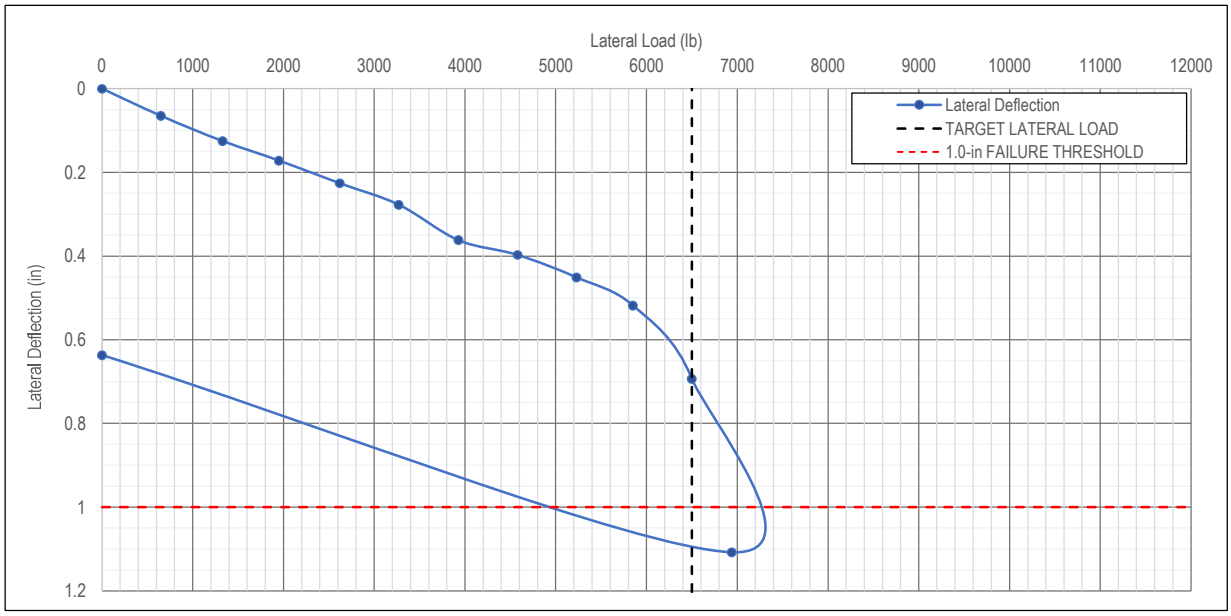
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.068	0.062	0.07
62%	20%	1,300	1330	0.13	0.12	0.13
93%	30%	1,950	1950	0.18	0.164	0.17
124%	40%	2,600	2620	0.236	0.216	0.23
155%	50%	3,250	3270	0.29	0.264	0.28
186%	60%	3,900	3930	0.378	0.346	0.36
217%	70%	4,550	4580	0.414	0.38	0.40
248%	80%	5,200	5230	0.47	0.432	0.45
279%	90%	5,850	5850	0.538	0.498	0.52
310%	100%	6,500	6500	0.704	0.682	0.69
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6940	1.04	1.176	1.11
0%	0%	0	0	0.578	0.696	0.64

Load at 1.0-in lateral deflection: **6,825** **lb**

Residual lateral deformation: **0.64** **in**



Notes:

Failure reached at 6,940 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-66B

Axial Uplift Load Test Results

PTL - **66** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.644396 - - Saturated Soil? No

Longitude: -114.66715 Target Embedment Depth (ft): 8.00 Drive Time (s): 17.31

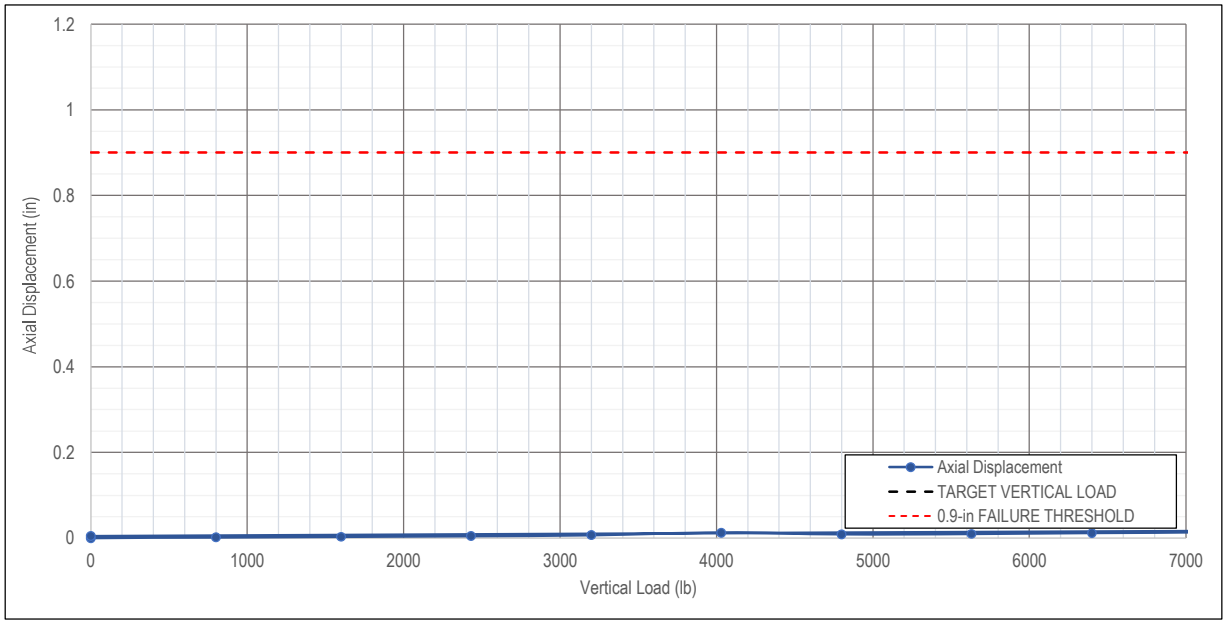
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/27/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0.002	0.002	0.00
436%	20%	1,600	1600	0.002	0.004	0.00
654%	30%	2,400	2430	0.004	0.006	0.01
872%	40%	3,200	3200	0.006	0.008	0.01
1090%	50%	4,000	4030	0.008	0.018	0.01
1308%	60%	4,800	4800	0.008	0.01	0.01
1526%	70%	5,600	5630	0.01	0.01	0.01
1744%	80%	6,400	6400	0.014	0.01	0.01
1962%	90%	7,200	7230	0.018	0.01	0.01
2180%	100%	8,000	8040	0.022	0.014	0.02
0%	0%	0	0	0.006	0.004	0.01

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-66B

Lateral Load Test Results

PTL- **66** Designation: **B**

Section: W6x9

Latitude: 33.644396

Longitude: -114.66715

Tested By: ANS

Test Date: 10/27/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 17.31

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

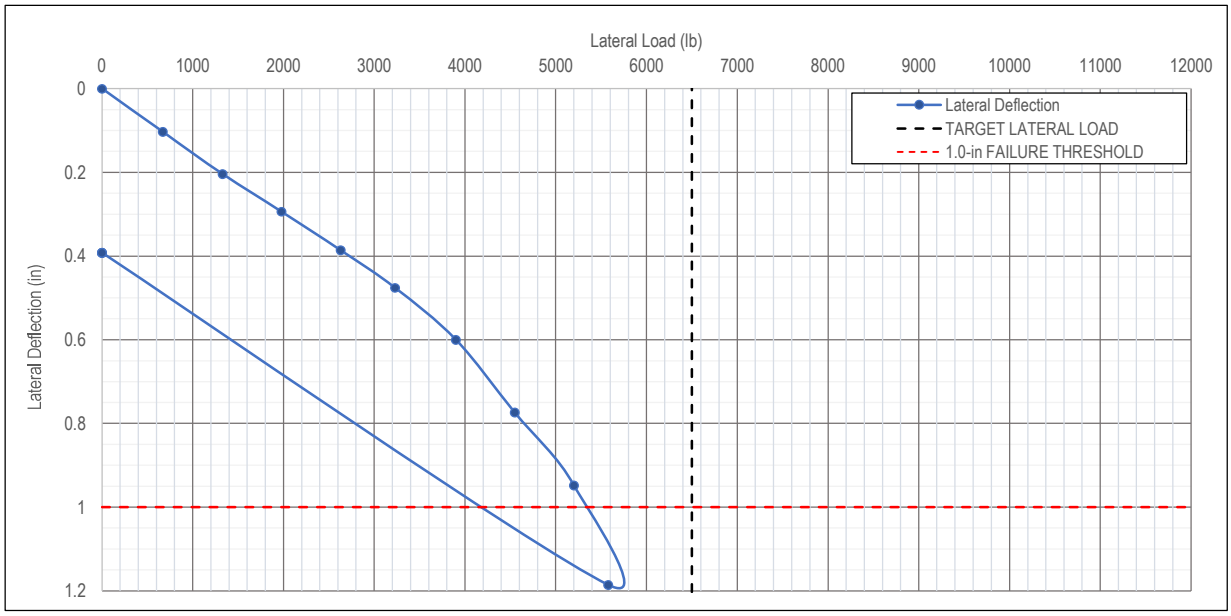
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	670	0.108	0.098	0.10
62%	20%	1,300	1330	0.214	0.194	0.20
93%	30%	1,950	1980	0.308	0.28	0.29
124%	40%	2,600	2630	0.404	0.368	0.39
155%	50%	3,250	3230	0.496	0.456	0.48
186%	60%	3,900	3900	0.626	0.574	0.60
217%	70%	4,550	4550	0.8	0.748	0.77
248%	80%	5,200	5200	0.966	0.93	0.95
279%	90%	5,850	5580	1.17	1.202	1.19
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.358	0.426	0.39

Load at 1.0-in lateral deflection: **5,283** **lb**

Residual lateral deformation: **0.39** **in**



Notes:

Failure reached at 5,580 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-67A

Axial Uplift Load Test Results

PTL - **67** Designation: **A**

Section: W6x9

Latitude: 33.644396

Longitude: -114.66421

Tested By: ANS

Test Date: 10/27/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 11.65

Pile Damaged? No

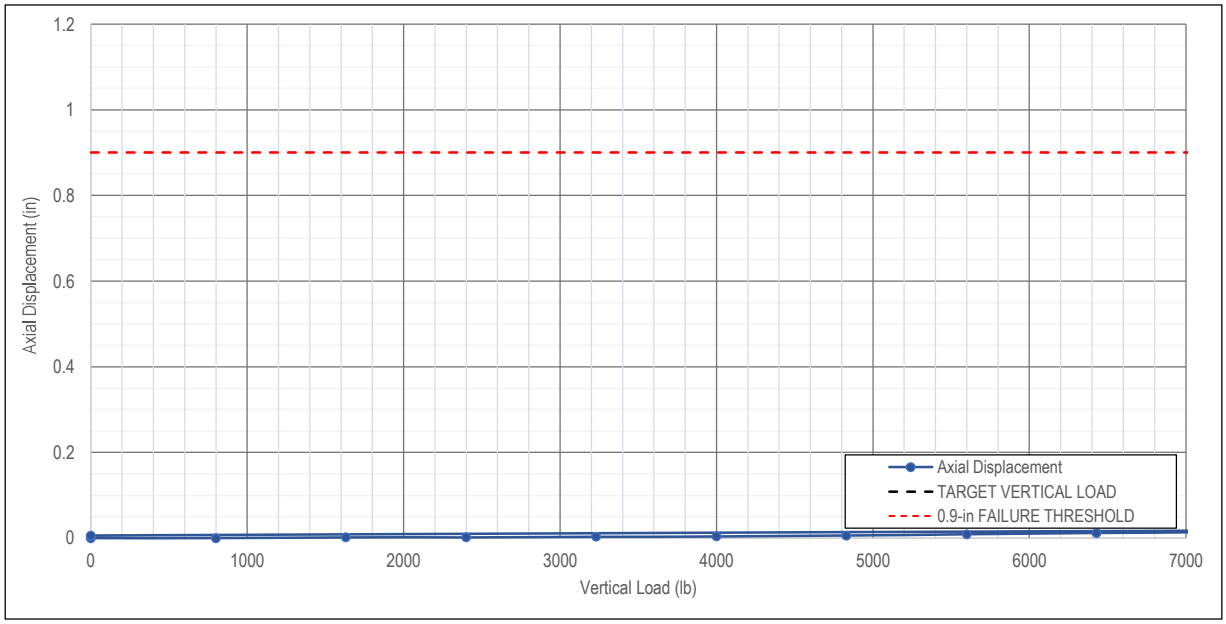
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	800	0	0	0.00
13594%	20%	1,600	1630	0.002	0.002	0.00
20391%	30%	2,400	2400	0.002	0.002	0.00
27188%	40%	3,200	3230	0.004	0.002	0.00
33985%	50%	4,000	4000	0.004	0.004	0.00
40782%	60%	4,800	4830	0.006	0.006	0.01
47579%	70%	5,600	5600	0.008	0.01	0.01
54376%	80%	6,400	6430	0.01	0.014	0.01
61173%	90%	7,200	7200	0.012	0.016	0.01
67970%	100%	8,000	8030	0.016	0.022	0.02
0%	0%	0	0	0.006	0.006	0.01

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-67A

Lateral Load Test Results

PTL- **67** Designation: **A**

Section: W6x9

Latitude: 33.644396

Longitude: -114.66421

Tested By: ANS

Test Date: 10/27/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.65

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

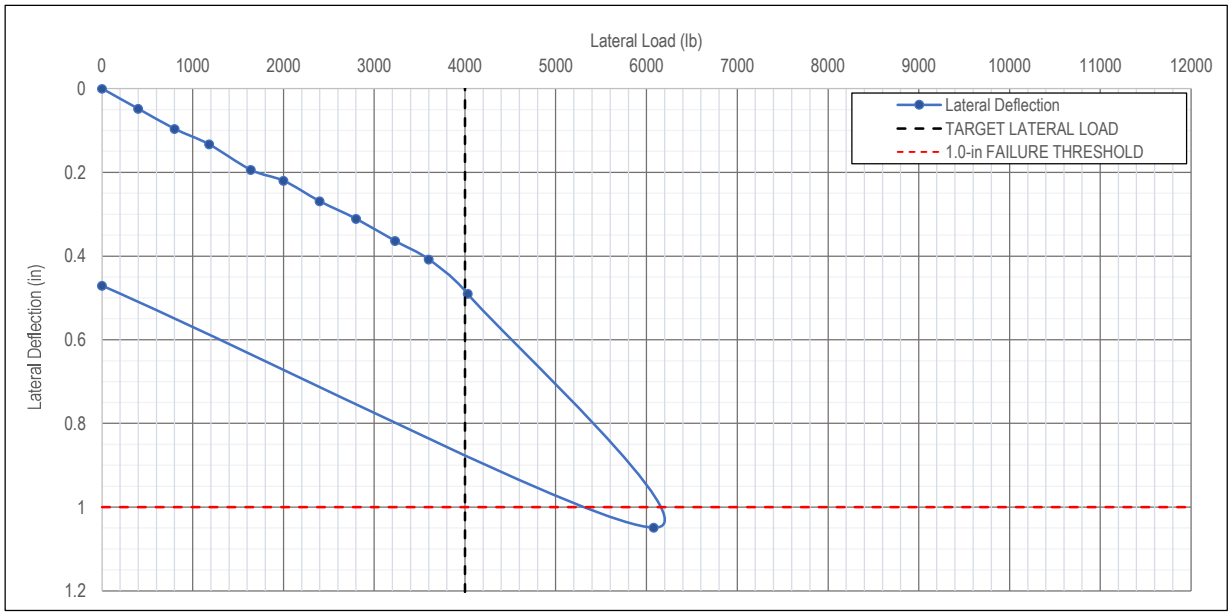
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.05	0.046	0.05
62%	20%	800	800	0.098	0.094	0.10
93%	30%	1,200	1180	0.136	0.13	0.13
124%	40%	1,600	1640	0.2	0.188	0.19
155%	50%	2,000	2000	0.226	0.214	0.22
186%	60%	2,400	2400	0.278	0.26	0.27
217%	70%	2,800	2800	0.322	0.3	0.31
248%	80%	3,200	3230	0.376	0.352	0.36
279%	90%	3,600	3600	0.422	0.394	0.41
310%	100%	4,000	4030	0.51	0.47	0.49
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6080	1.184	0.914	1.05
0%	0%	0	0	0.558	0.384	0.47

Load at 1.0-in lateral deflection: **5,900** **lb**

Residual lateral deformation: **0.47** **in**



Notes:

Failure reached at 6,080 lbs. Pile bending at 5,800 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-67B

Axial Uplift Load Test Results

PTL - **67** Designation: **B**

Section: W6x9

Latitude: 33.644396

Longitude: -114.66421

Tested By: ANS

Test Date: 10/27/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 18.11

Pile Damaged? No

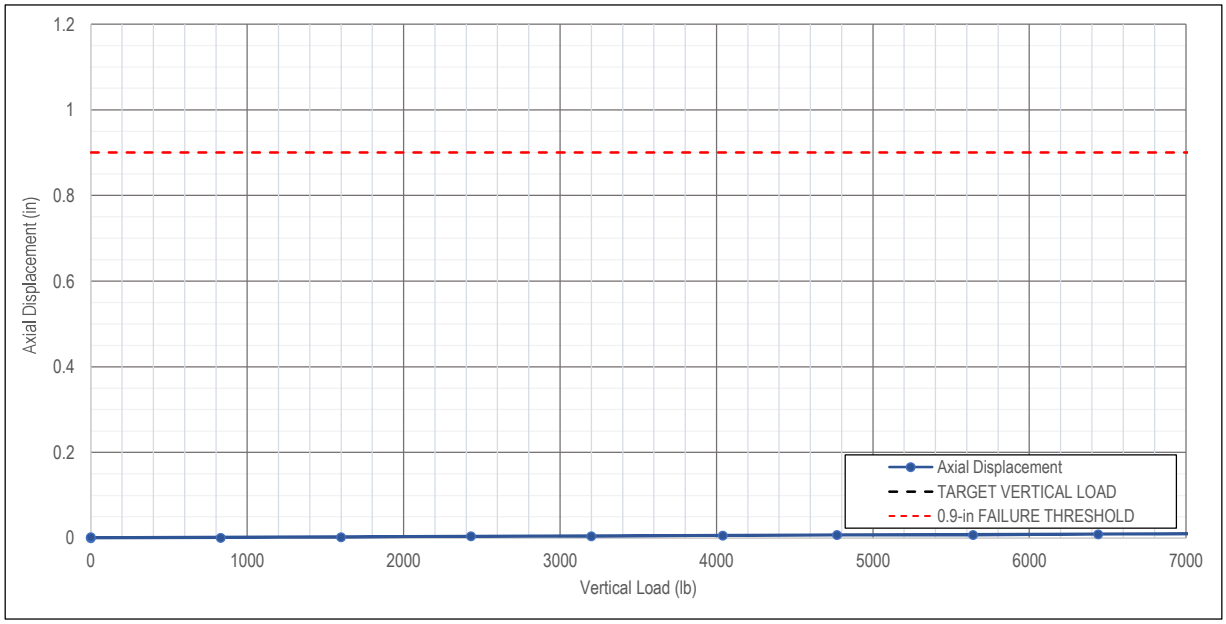
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	830	0	0.002	0.00
13594%	20%	1,600	1600	0.002	0.002	0.00
20391%	30%	2,400	2430	0.004	0.004	0.00
27188%	40%	3,200	3200	0.004	0.004	0.00
33985%	50%	4,000	4040	0.006	0.006	0.01
40782%	60%	4,800	4770	0.006	0.008	0.01
47579%	70%	5,600	5640	0.006	0.008	0.01
54376%	80%	6,400	6440	0.008	0.01	0.01
61173%	90%	7,200	7240	0.008	0.012	0.01
67970%	100%	8,000	8000	0.01	0.014	0.01
0%	0%	0	0	0.002	0.002	0.00

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.00** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-67B

Lateral Load Test Results

PTL- **67** Designation: **B**

Section: W6x9

Latitude: 33.644396

Longitude: -114.66421

Tested By: ANS

Test Date: 10/27/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 18.11

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

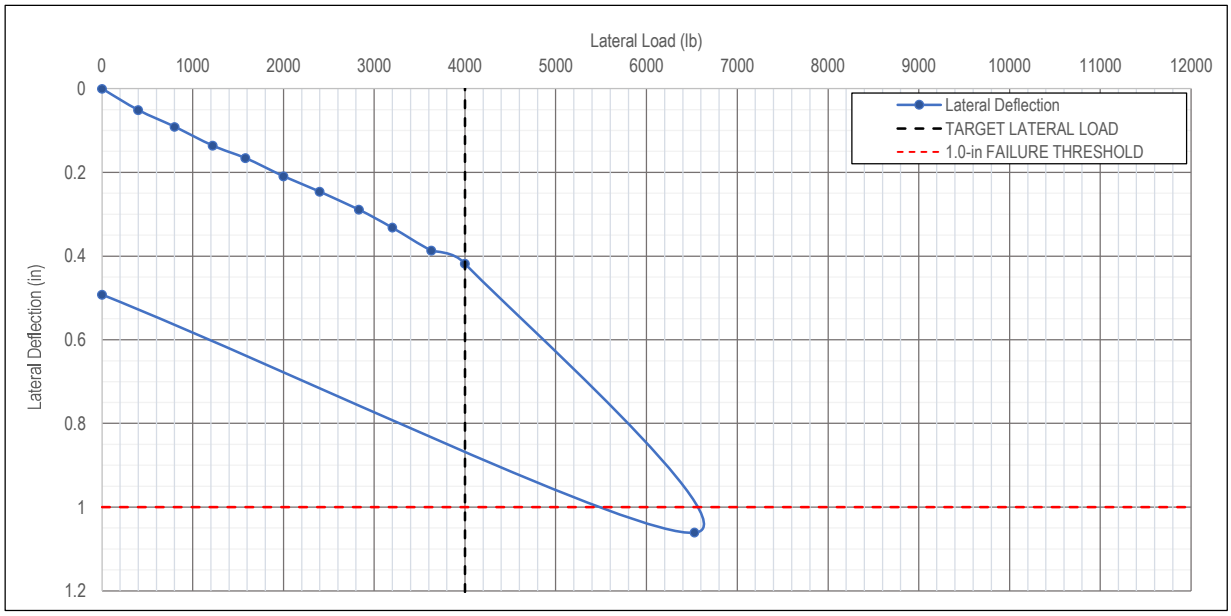
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.054	0.048	0.05
62%	20%	800	800	0.096	0.086	0.09
93%	30%	1,200	1220	0.142	0.13	0.14
124%	40%	1,600	1580	0.174	0.158	0.17
155%	50%	2,000	2000	0.218	0.2	0.21
186%	60%	2,400	2400	0.256	0.236	0.25
217%	70%	2,800	2830	0.302	0.276	0.29
248%	80%	3,200	3200	0.346	0.318	0.33
279%	90%	3,600	3630	0.404	0.37	0.39
310%	100%	4,000	4000	0.436	0.4	0.42
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6530	1.21	0.9118	1.06
0%	0%	0	0	0.586	0.398	0.49

Load at 1.0-in lateral deflection: **6,290** **lb**

Residual lateral deformation: **0.49** **in**



Notes:

Failure reached at 6,530 lbs. Pile bending at 6,000lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-67C

Axial Compression Load Test Results (supplemental pile)

PTL - **67**
Section: W6x9
Latitude: 33.644396
Longitude: -114.66421
Tested By: ANS
Test Date: 10/27/2025

Designation: **C**

Target Test Load (lb): **15,000** (≈ 1180% Design Load)
Installation Method: Direct Drive (DD)
- -
Target Embedment Depth (ft): 5.00
Actual Embedment Depth (ft): 5

Saturated Soil? No
Drive Time (s): 7.68
Pile Damaged? No

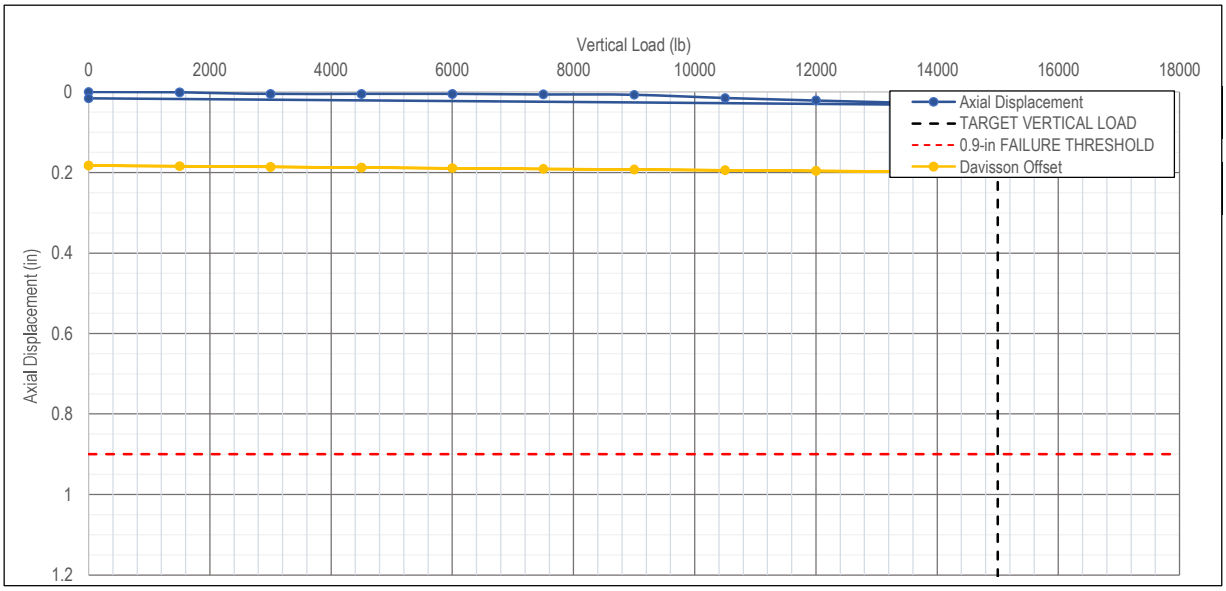
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL COMPRESSION** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
118%	10%	1,500	1500	0.002	0	0.00
236%	20%	3,000	3000	0.006	0.004	0.01
354%	30%	4,500	4500	0.006	0.004	0.01
472%	40%	6,000	6000	0.006	0.004	0.01
590%	50%	7,500	7500	0.006	0.006	0.01
708%	60%	9,000	9000	0.008	0.006	0.01
826%	70%	10,500	10500	0.014	0.016	0.02
944%	80%	12,000	12000	0.02	0.022	0.02
1062%	90%	13,500	13500	0.026	0.028	0.03
1180%	100%	15,000	15000	0.032	0.034	0.03
0%	0%	0	0	0.018	0.014	0.02

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.02** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-68A

Axial Uplift Load Test Results

PTL - **68** Designation: **A**

Section: W6x9

Latitude: 33.644396

Longitude: -114.66126

Tested By: ANS

Test Date: 10/27/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 10.68

Pile Damaged? No

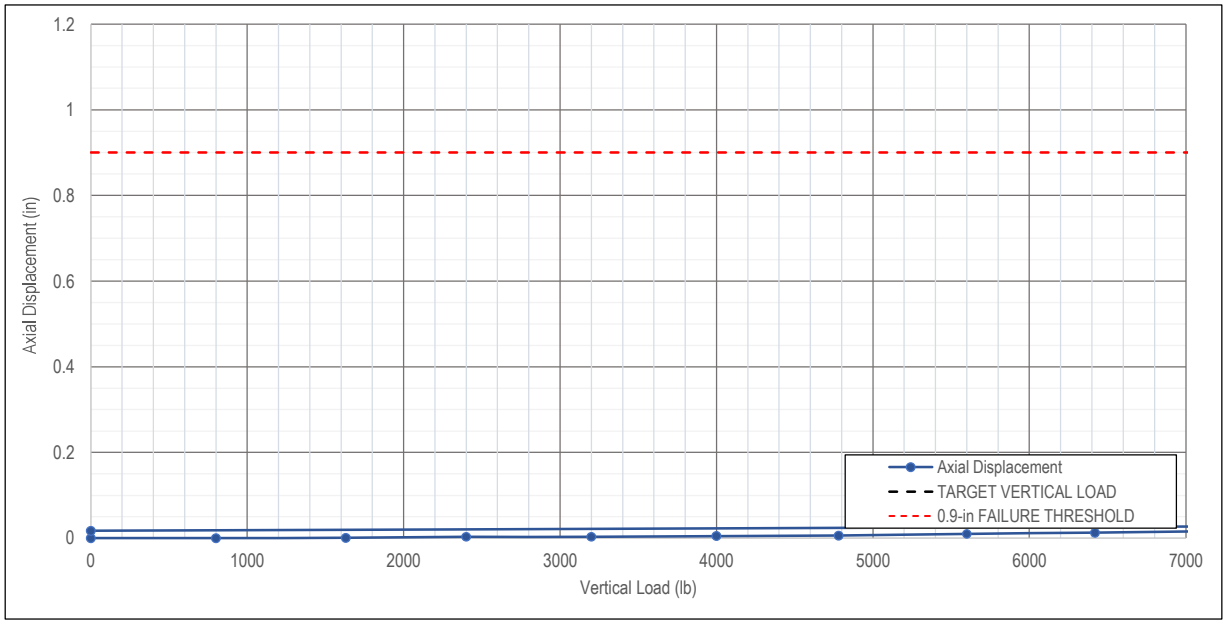
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0	0	0.00
436%	20%	1,600	1630	0	0.002	0.00
654%	30%	2,400	2400	0.002	0.004	0.00
872%	40%	3,200	3200	0.002	0.004	0.00
1090%	50%	4,000	4000	0.004	0.006	0.01
1308%	60%	4,800	4780	0.006	0.006	0.01
1526%	70%	5,600	5600	0.01	0.01	0.01
1744%	80%	6,400	6420	0.014	0.012	0.01
1962%	90%	7,200	7200	0.018	0.016	0.02
2180%	100%	8,000	8030	0.03	0.026	0.03
0%	0%	0	0	0.018	0.016	0.02

Load at 0.9-in axial displacement: N/A lb

Residual axial displacement: 0.02 in



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-68A

Lateral Load Test Results

PTL- **68** Designation: **A**

Section: W6x9

Latitude: 33.644396

Longitude: -114.66126

Tested By: ANS

Test Date: 10/27/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 10.68

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

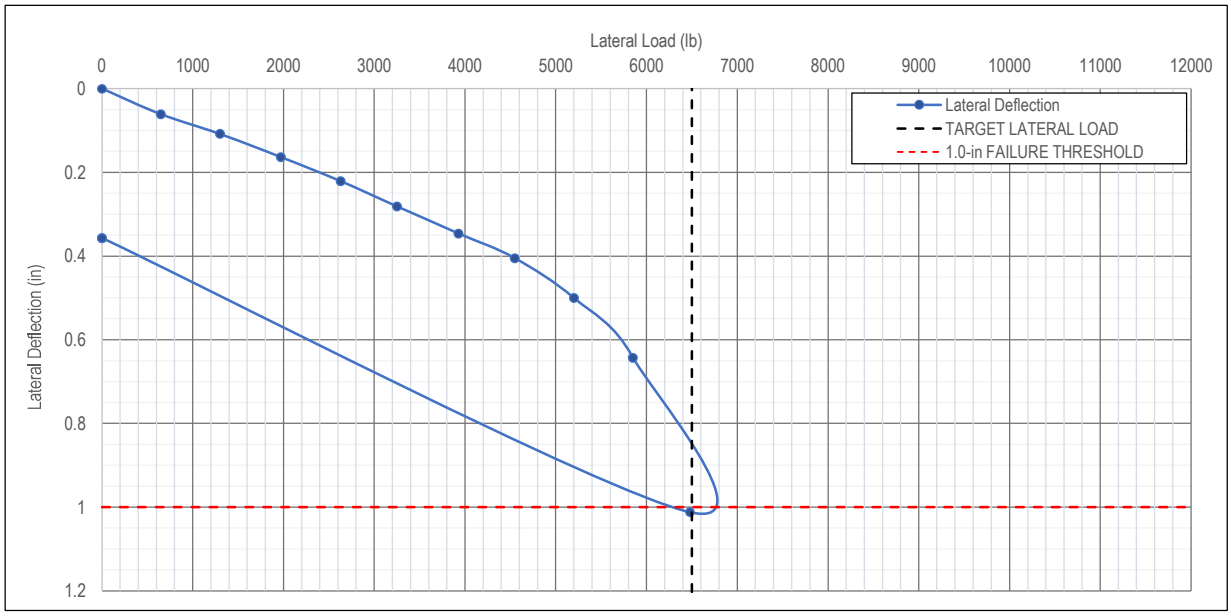
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.064	0.058	0.06
62%	20%	1,300	1300	0.112	0.104	0.11
93%	30%	1,950	1970	0.17	0.156	0.16
124%	40%	2,600	2630	0.23	0.212	0.22
155%	50%	3,250	3250	0.292	0.27	0.28
186%	60%	3,900	3930	0.356	0.336	0.35
217%	70%	4,550	4550	0.414	0.396	0.41
248%	80%	5,200	5200	0.504	0.496	0.50
279%	90%	5,850	5850	0.628	0.658	0.64
310%	100%	6,500	6480	1.014	1.01	1.01
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.366	0.348	0.36

Load at 1.0-in lateral deflection: **6,460** **lb**

Residual lateral deformation: **0.36** **in**



Notes:

Failure reached at 6,480 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-68B

Axial Uplift Load Test Results

PTL - **68** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.644396 - - Saturated Soil? No

Longitude: -114.66126 Target Embedment Depth (ft): 8.00 Drive Time (s): 15.76

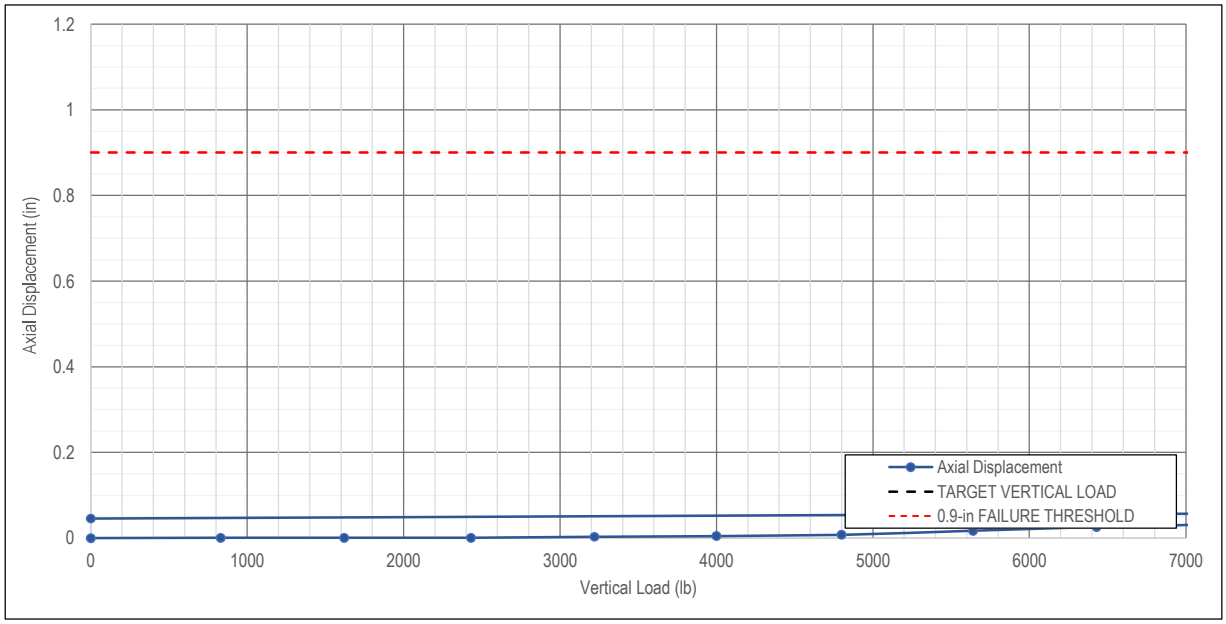
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/27/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	830	0	0.002	0.00
436%	20%	1,600	1620	0	0.002	0.00
654%	30%	2,400	2430	0	0.002	0.00
872%	40%	3,200	3220	0.002	0.004	0.00
1090%	50%	4,000	4000	0.004	0.006	0.01
1308%	60%	4,800	4800	0.008	0.008	0.01
1526%	70%	5,600	5640	0.02	0.014	0.02
1744%	80%	6,400	6430	0.03	0.022	0.03
1962%	90%	7,200	7200	0.038	0.03	0.03
2180%	100%	8,000	8020	0.064	0.05	0.06
0%	0%	0	0	0.046	0.046	0.05

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.05** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-68B

Lateral Load Test Results

PTL- **68** Designation: **B**

Section: W6x9

Latitude: 33.644396

Longitude: -114.66126

Tested By: ANS

Test Date: 10/27/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 15.76

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

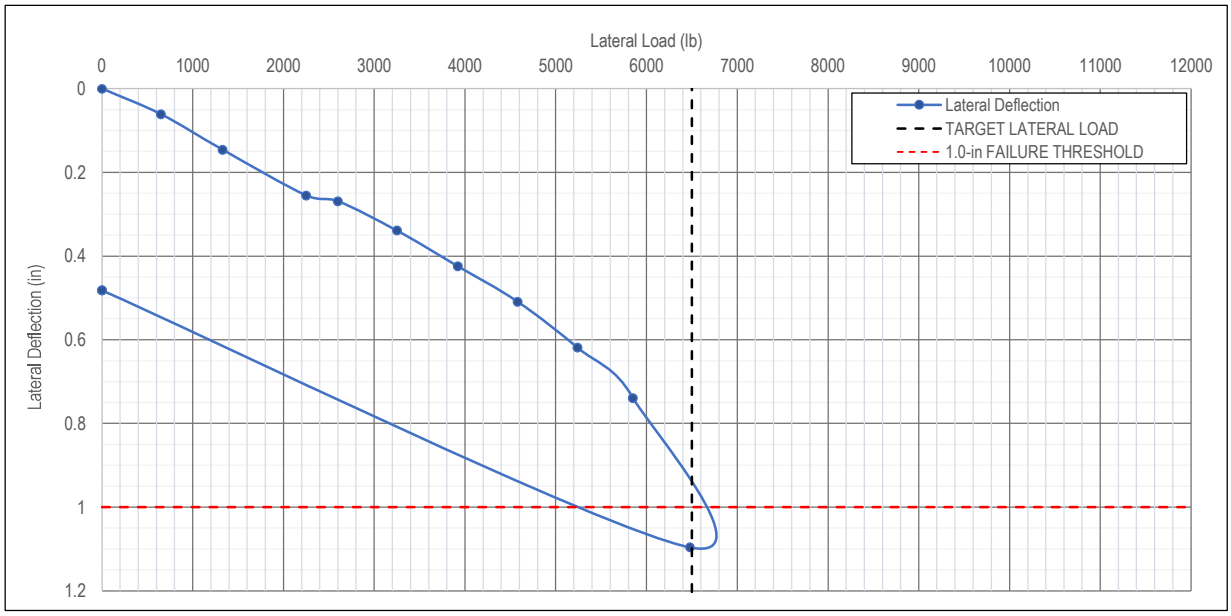
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.062	0.06	0.06
62%	20%	1,300	1330	0.152	0.14	0.15
93%	30%	1,950	2250	0.264	0.246	0.26
124%	40%	2,600	2600	0.28	0.258	0.27
155%	50%	3,250	3250	0.352	0.326	0.34
186%	60%	3,900	3920	0.44	0.408	0.42
217%	70%	4,550	4580	0.526	0.492	0.51
248%	80%	5,200	5240	0.634	0.604	0.62
279%	90%	5,850	5850	0.748	0.73	0.74
310%	100%	6,500	6480	1.026	1.166	1.10
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.44	0.524	0.48

Load at 1.0-in lateral deflection: **6,311** **lb**

Residual lateral deformation: **0.48** **in**



Notes:

Failure reached at 6,480 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-69A

Axial Uplift Load Test Results

PTL - **69** Designation: **A**

Section: W6x9

Latitude: 33.642303

Longitude: -114.71383

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 5.31

Pile Damaged? No

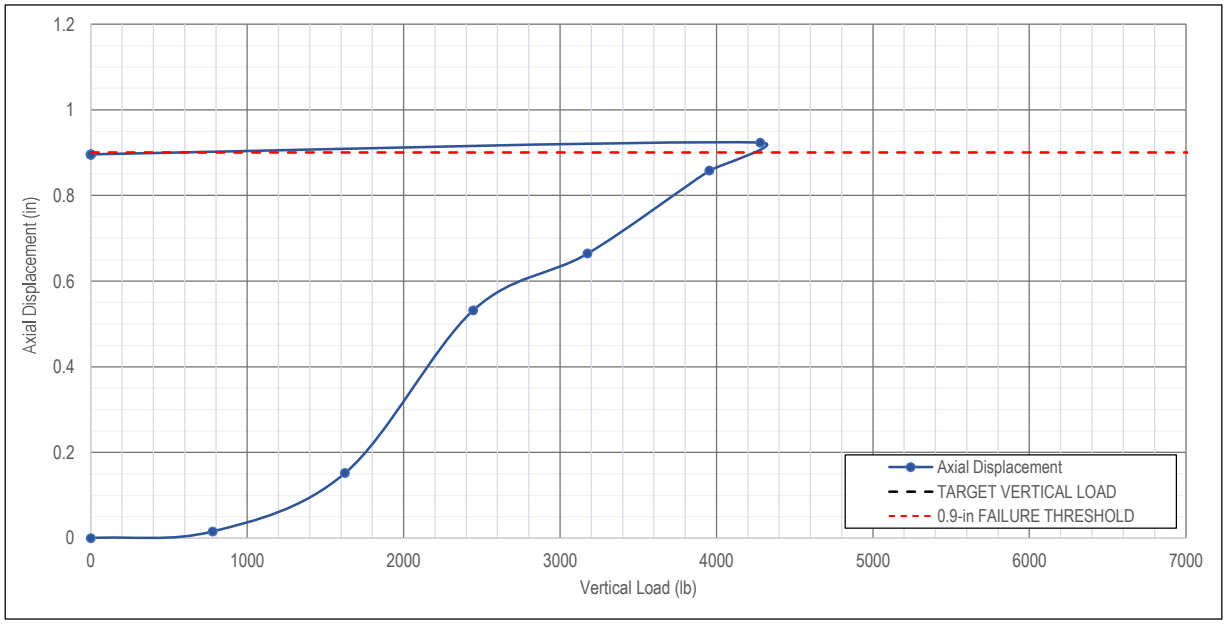
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	780	0.014	0.017	0.02
13594%	20%	1,600	1625	0.152	0.152	0.15
20391%	30%	2,400	2445	0.5335	0.5305	0.53
27188%	40%	3,200	3175	0.6675	0.6615	0.66
33985%	50%	4,000	3955	0.857	0.859	0.86
40782%	60%	4,800	4280	0.9195	0.927	0.92
47579%	70%	5,600				
54376%	80%	6,400				
61173%	90%	7,200				
67970%	100%	8,000				
0%	0%	0	0	0.897	0.894	0.90

Load at 0.9-in axial displacement: **4,164** **lb**

Residual axial displacement: **0.90** **in**



Notes:

Failure reached at 4,280 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-69A

Lateral Load Test Results

PTL- **69** Designation: **A**

Section: W6x9

Latitude: 33.642303

Longitude: -114.71383

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 5.31

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

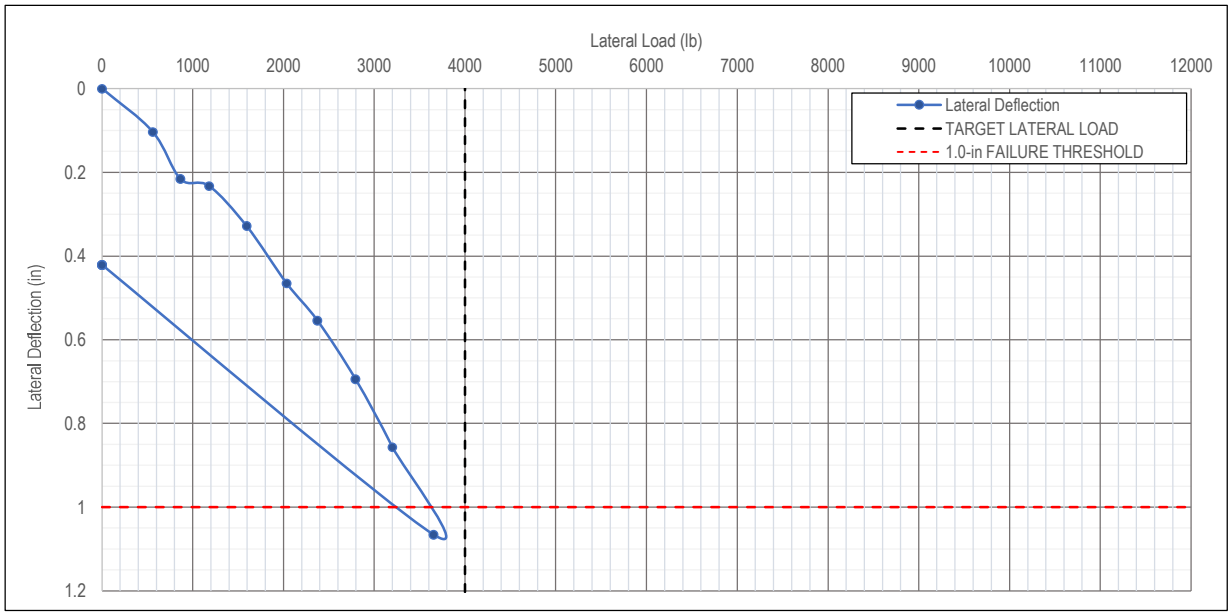
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	560	0.097	0.1095	0.10
62%	20%	800	865	0.2045	0.227	0.22
93%	30%	1,200	1180	0.221	0.244	0.23
124%	40%	1,600	1595	0.3155	0.341	0.33
155%	50%	2,000	2035	0.4505	0.48	0.47
186%	60%	2,400	2375	0.538	0.57	0.55
217%	70%	2,800	2795	0.6785	0.71	0.69
248%	80%	3,200	3200	0.8415	0.8715	0.86
279%	90%	3,600	3655	1.049	1.0825	1.07
310%	100%	4,000				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.423	0.4195	0.42

Load at 1.0-in lateral deflection: **3,512** **lb**

Residual lateral deformation: **0.42** **in**



Notes:

Failure reached at 3,655 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-69B

Axial Uplift Load Test Results

PTL - **69** Designation: **B**

Section: W6x9

Latitude: 33.642303

Longitude: -114.71383

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 21.11

Pile Damaged? No

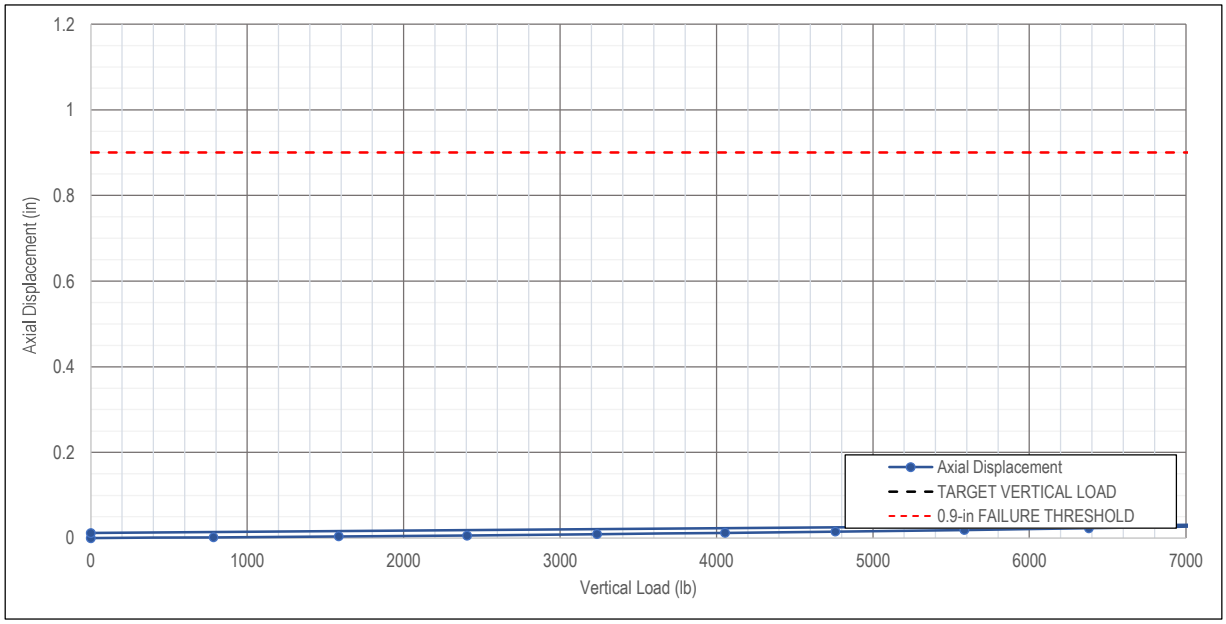
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	785	0.0025	0.0015	0.00
13594%	20%	1,600	1585	0.0045	0.0035	0.00
20391%	30%	2,400	2405	0.007	0.005	0.01
27188%	40%	3,200	3235	0.011	0.008	0.01
33985%	50%	4,000	4055	0.0145	0.01	0.01
40782%	60%	4,800	4760	0.0175	0.013	0.02
47579%	70%	5,600	5585	0.022	0.016	0.02
54376%	80%	6,400	6380	0.027	0.02	0.02
61173%	90%	7,200	7245	0.032	0.0245	0.03
67970%	100%	8,000	8025	0.038	0.0295	0.03
0%	0%	0	0	0.012	0.012	0.01

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-69B

Lateral Load Test Results

PTL- **69** Designation: **B**

Section: W6x9

Latitude: 33.642303

Longitude: -114.71383

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 21.11

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

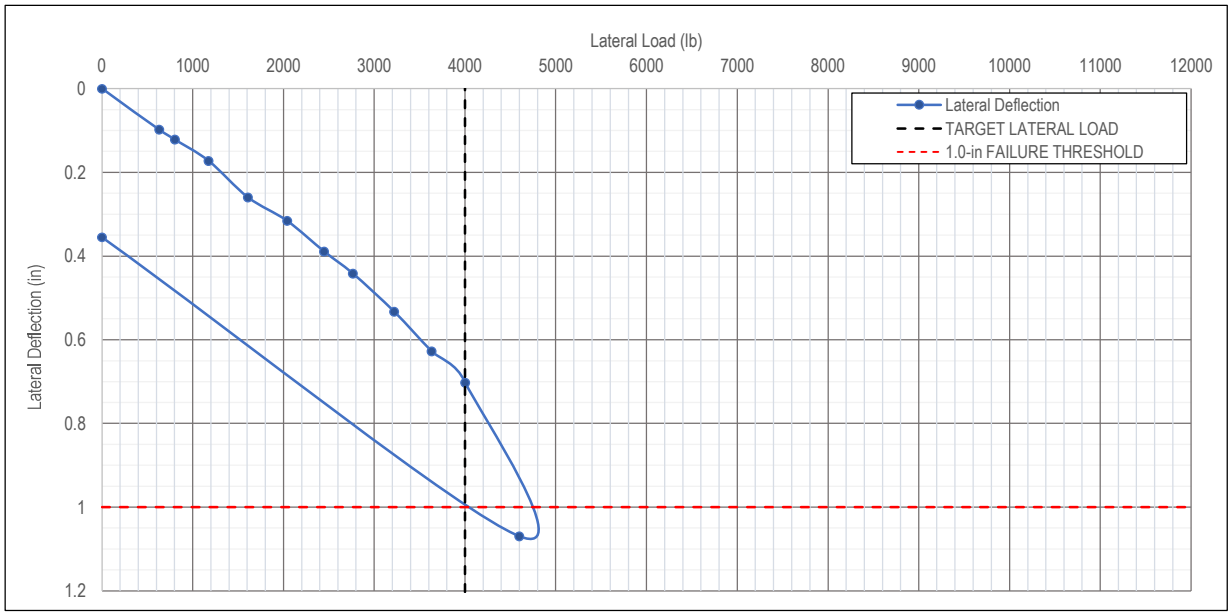
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	630	0.094	0.1015	0.10
62%	20%	800	805	0.1175	0.126	0.12
93%	30%	1,200	1175	0.1675	0.1775	0.17
124%	40%	1,600	1610	0.252	0.2675	0.26
155%	50%	2,000	2040	0.306	0.3255	0.32
186%	60%	2,400	2450	0.3765	0.402	0.39
217%	70%	2,800	2765	0.4265	0.457	0.44
248%	80%	3,200	3220	0.513	0.553	0.53
279%	90%	3,600	3635	0.5995	0.6555	0.63
310%	100%	4,000	4005	0.663	0.742	0.70
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			4600	0.927	1.212	1.07
0%	0%	0	0	0.1695	0.5405	0.36

Load at 1.0-in lateral deflection: **4,487** **lb**

Residual lateral deformation: **0.36** **in**



Notes:

Failure reached at 4,600 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-67C

Axial Compression Load Test Results (supplemental pile)

PTL - **67** Designation: **C**

Section: W6x9

Latitude: 33.642303

Longitude: -114.71383

Tested By: ANS

Test Date: 10/20/2025

Target Test Load (lb): **15,000** (≈ 1180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 11.28

Pile Damaged? No

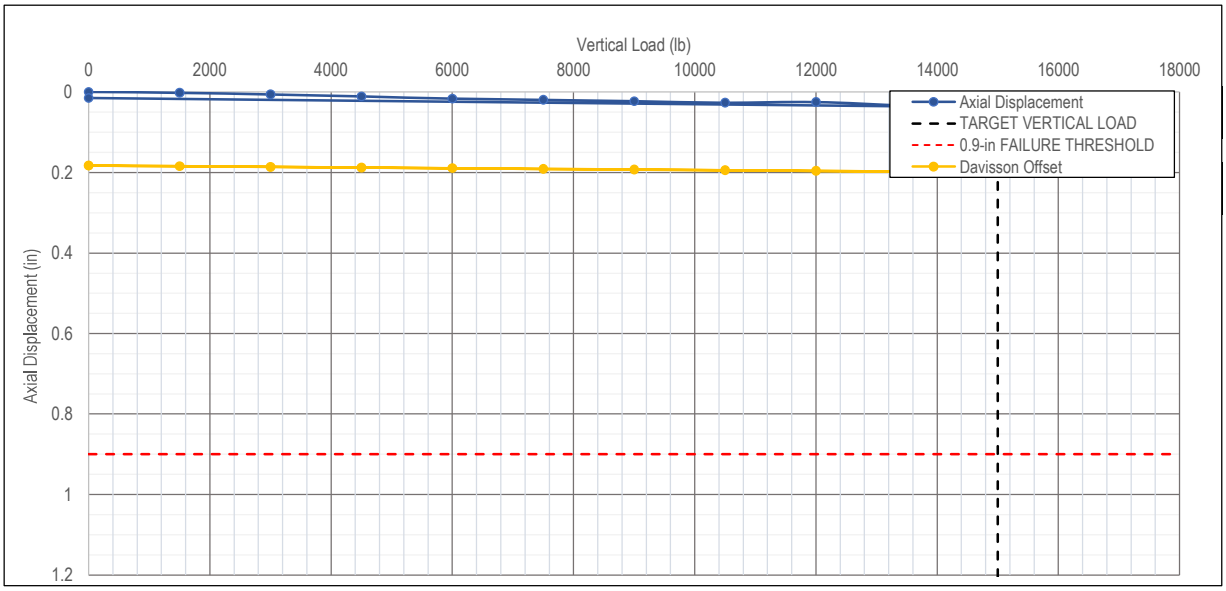
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL COMPRESSION** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
118%	10%	1,500	1500	0.003	0.001	0.00
236%	20%	3,000	3000	0.0075	0.0045	0.01
354%	30%	4,500	4500	0.0135	0.009	0.01
472%	40%	6,000	6000	0.019	0.014	0.02
590%	50%	7,500	7500	0.0225	0.0165	0.02
708%	60%	9,000	9000	0.026	0.02	0.02
826%	70%	10,500	10500	0.03	0.0235	0.03
944%	80%	12,000	12000	0.0235	0.0265	0.03
1062%	90%	13,500	13500	0.0375	0.03	0.03
1180%	100%	15,000	15000	0.042	0.034	0.04
0%	0%	0	0	0.0165	0.014	0.02

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.02** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-70A

Axial Uplift Load Test Results

PTL - **70** Designation: **A**

Section: W6x9

Latitude: 33.635169

Longitude: -114.69142

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 9.58

Pile Damaged? No

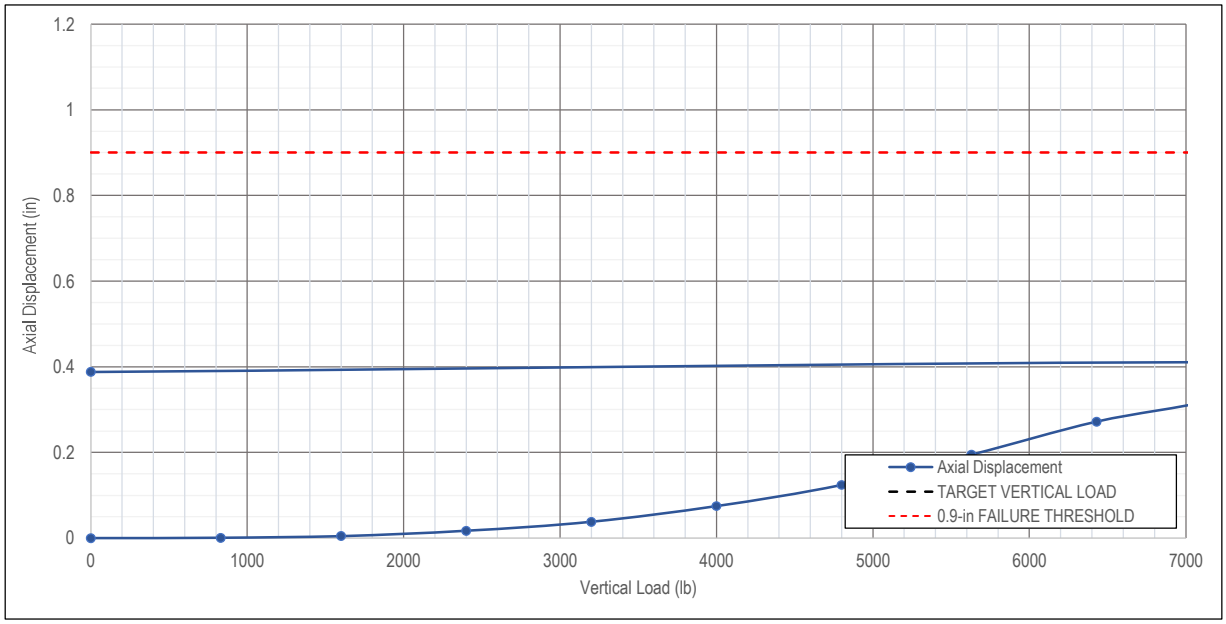
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	830	0.002	0	0.00
436%	20%	1,600	1600	0.006	0.004	0.01
654%	30%	2,400	2400	0.018	0.016	0.02
872%	40%	3,200	3200	0.04	0.036	0.04
1090%	50%	4,000	4000	0.078	0.072	0.08
1308%	60%	4,800	4800	0.128	0.12	0.12
1526%	70%	5,600	5630	0.2	0.19	0.20
1744%	80%	6,400	6430	0.278	0.266	0.27
1962%	90%	7,200	7200	0.33	0.318	0.32
2180%	100%	8,000	8030	0.416	0.4	0.41
0%	0%	0	0	0.39	0.386	0.39

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.39** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-70A

Lateral Load Test Results

PTL- **70** Designation: **A**

Section: W6x9

Latitude: 33.635169

Longitude: -114.69142

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 9.58

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

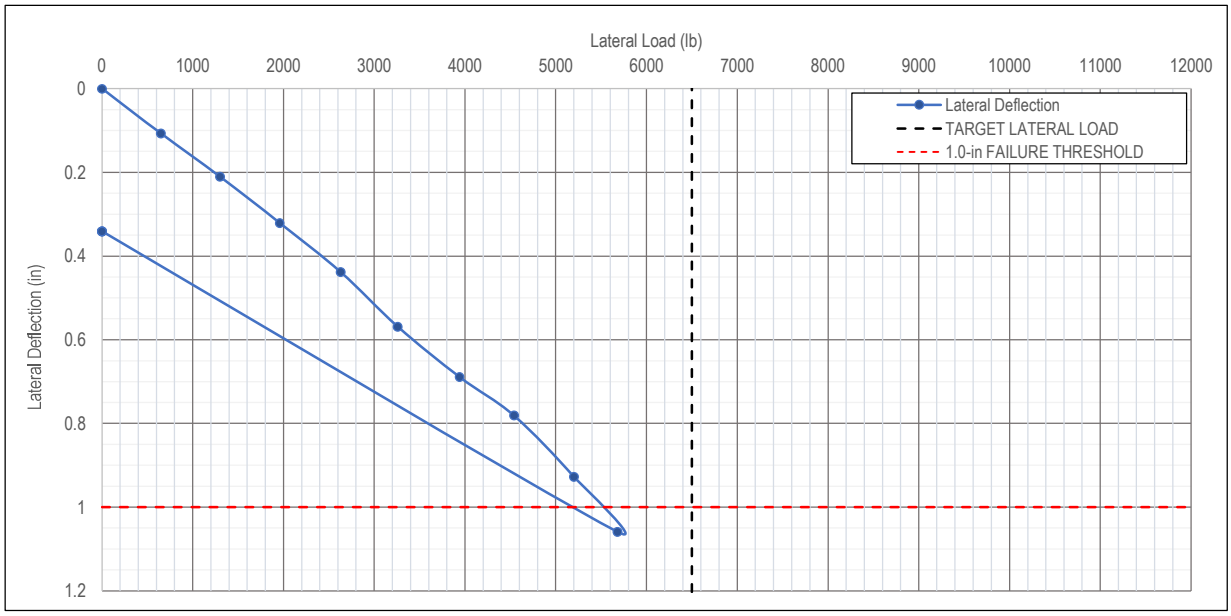
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.108	0.106	0.11
62%	20%	1,300	1300	0.212	0.208	0.21
93%	30%	1,950	1960	0.324	0.318	0.32
124%	40%	2,600	2630	0.442	0.434	0.44
155%	50%	3,250	3260	0.574	0.564	0.57
186%	60%	3,900	3940	0.696	0.682	0.69
217%	70%	4,550	4540	0.788	0.774	0.78
248%	80%	5,200	5200	0.932	0.922	0.93
279%	90%	5,850	5680	1.06	1.058	1.06
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.334	0.348	0.34

Load at 1.0-in lateral deflection: **5,465** **lb**

Residual lateral deformation: **0.34** **in**



Notes:

Failure reached at 5,680 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-70B

Axial Uplift Load Test Results

PTL - **70** Designation: **B**

Section: W6x9

Latitude: 33.635169

Longitude: -114.69142

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 16.27

Pile Damaged? No

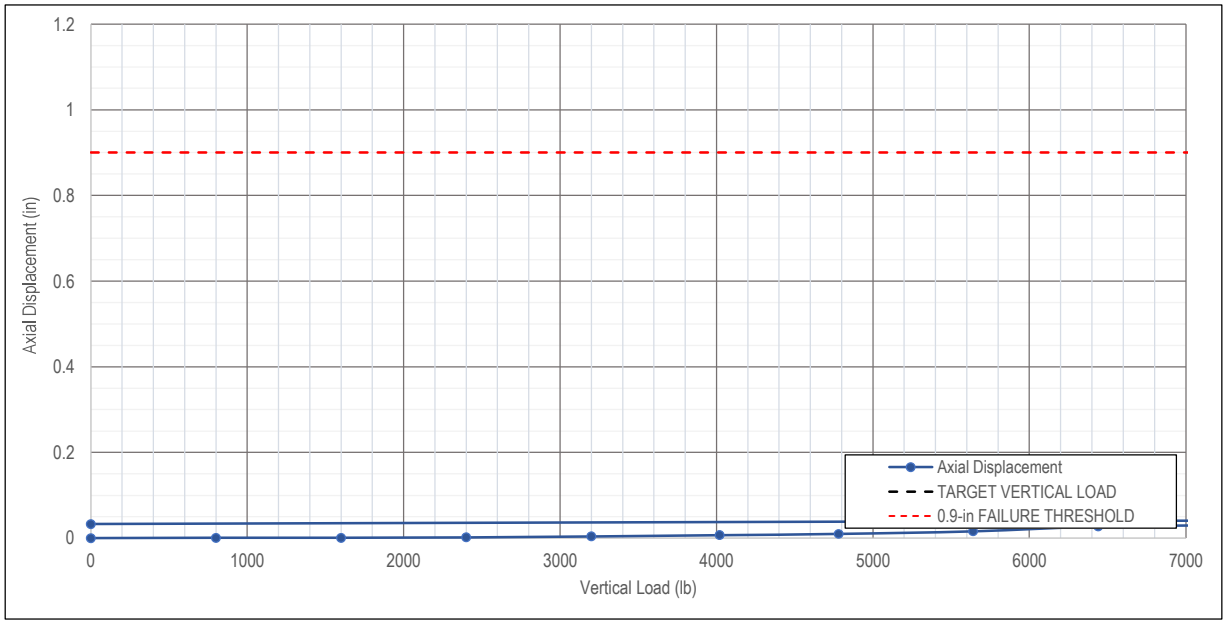
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0.002	0	0.00
436%	20%	1,600	1600	0.002	0	0.00
654%	30%	2,400	2400	0.002	0.002	0.00
872%	40%	3,200	3200	0.006	0.002	0.00
1090%	50%	4,000	4020	0.01	0.004	0.01
1308%	60%	4,800	4780	0.014	0.006	0.01
1526%	70%	5,600	5640	0.02	0.012	0.02
1744%	80%	6,400	6440	0.034	0.02	0.03
1962%	90%	7,200	7220	0.038	0.024	0.03
2180%	100%	8,000	8030	0.048	0.034	0.04
0%	0%	0	0	0.034	0.032	0.03

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-70B

Lateral Load Test Results

PTL- **70** Designation: **B**

Section: W6x9

Latitude: 33.635169

Longitude: -114.69142

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 16.27

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

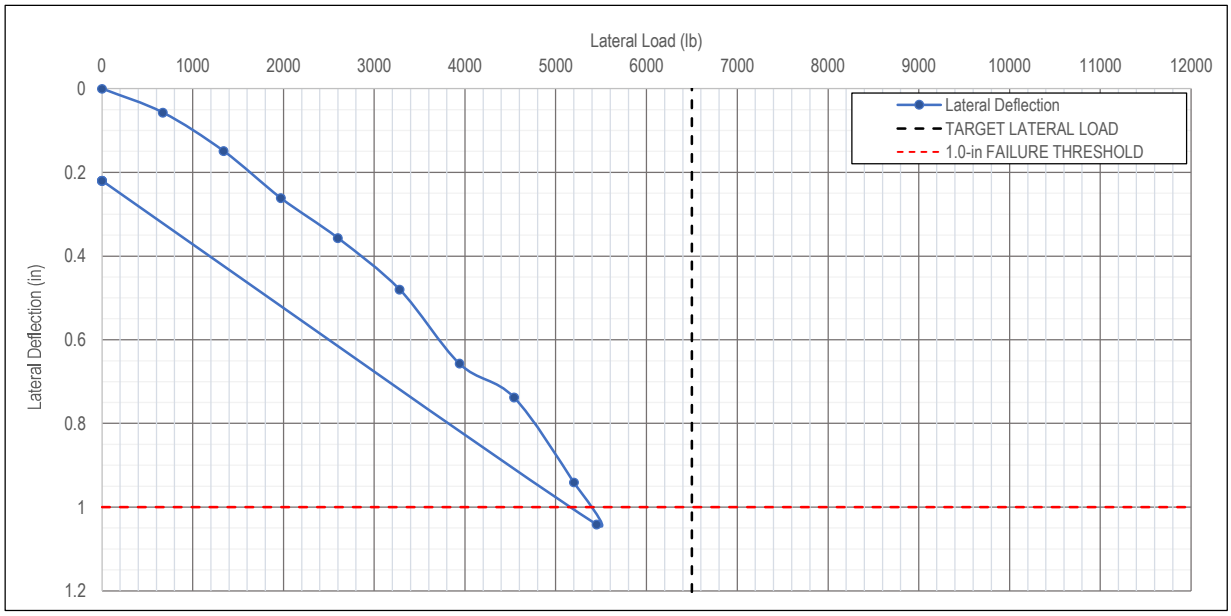
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	670	0.05	0.064	0.06
62%	20%	1,300	1340	0.144	0.154	0.15
93%	30%	1,950	1970	0.258	0.264	0.26
124%	40%	2,600	2600	0.356	0.358	0.36
155%	50%	3,250	3280	0.482	0.478	0.48
186%	60%	3,900	3940	0.662	0.652	0.66
217%	70%	4,550	4540	0.746	0.73	0.74
248%	80%	5,200	5200	0.958	0.924	0.94
279%	90%	5,850	5450	1.068	1.014	1.04
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.22	0.22	0.22

Load at 1.0-in lateral deflection: **5,348** **lb**

Residual lateral deformation: **0.22** **in**



Notes:

Failure reached at 5,450 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-70D

Axial Uplift Load Test Results

PTL - **70** Designation: **D**

Section: **W6x12**

Latitude: 33.635169

Longitude: -114.69142

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 18.05

Pile Damaged? No

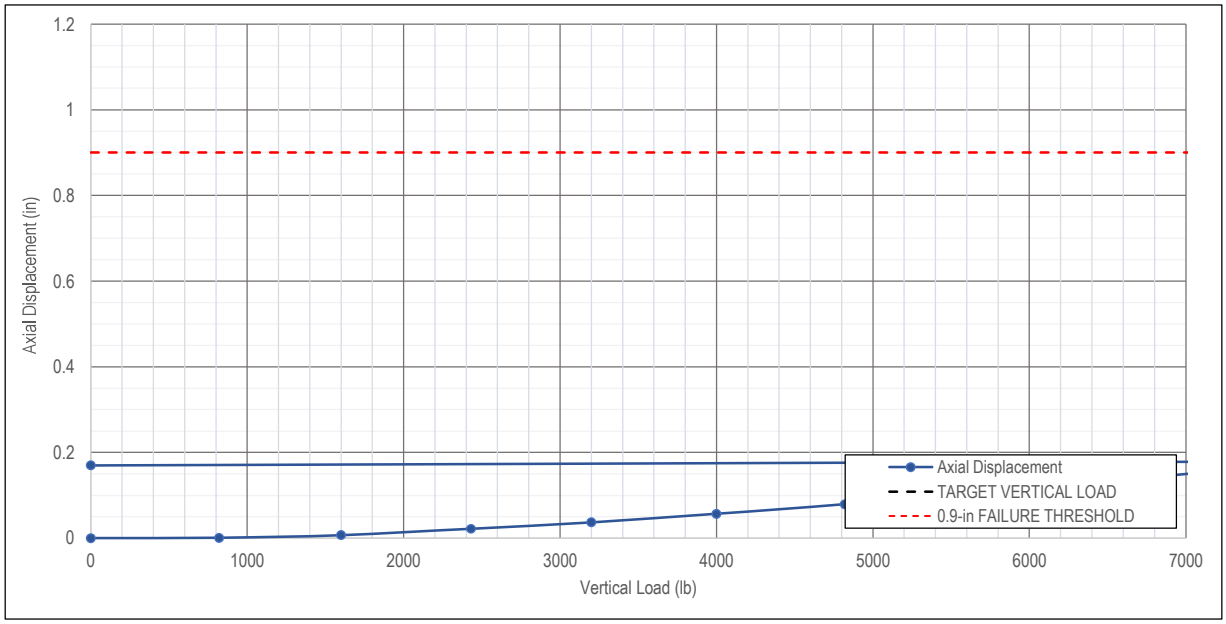
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	820	0	0.002	0.00
436%	20%	1,600	1600	0.006	0.008	0.01
654%	30%	2,400	2430	0.022	0.022	0.02
872%	40%	3,200	3200	0.038	0.036	0.04
1090%	50%	4,000	4000	0.06	0.054	0.06
1308%	60%	4,800	4820	0.084	0.074	0.08
1526%	70%	5,600	5630	0.11	0.102	0.11
1744%	80%	6,400	6430	0.136	0.126	0.13
1962%	90%	7,200	7200	0.162	0.15	0.16
2180%	100%	8,000	8030	0.18	0.176	0.18
0%	0%	0	0	0.17	0.17	0.17

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.17** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-70D

Lateral Load Test Results

PTL- **70** Designation: **D**

Section: **W6x12**

Latitude: 33.635169

Longitude: -114.69142

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 18.05

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

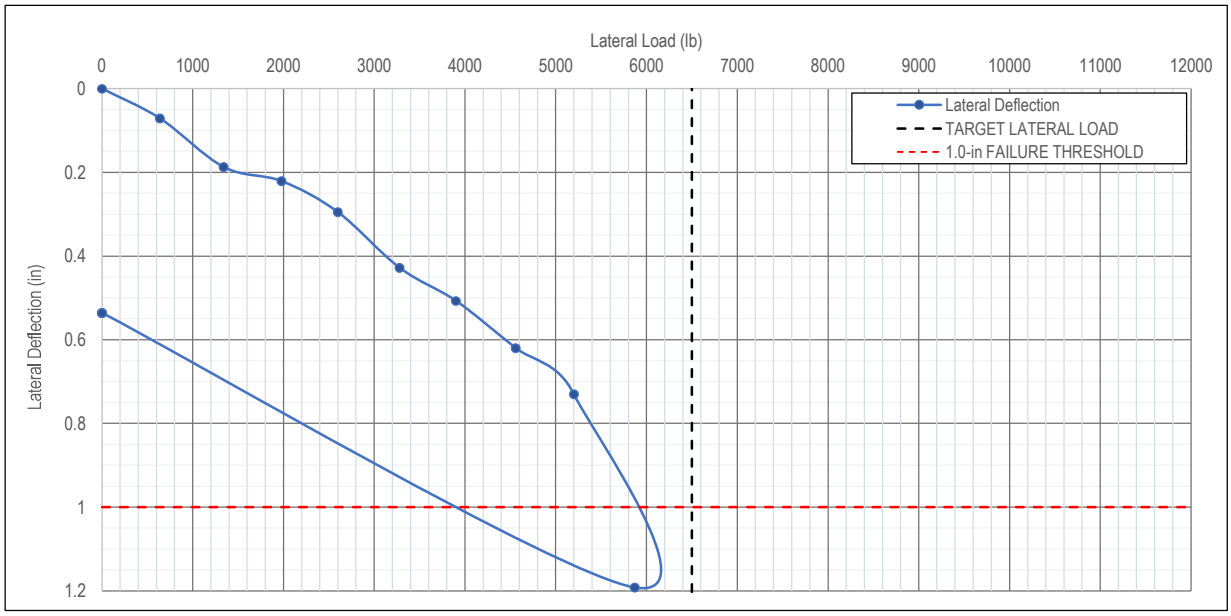
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	640	0.072	0.07	0.07
62%	20%	1,300	1340	0.192	0.182	0.19
93%	30%	1,950	1980	0.226	0.216	0.22
124%	40%	2,600	2600	0.302	0.288	0.30
155%	50%	3,250	3280	0.438	0.418	0.43
186%	60%	3,900	3900	0.52	0.494	0.51
217%	70%	4,550	4560	0.636	0.604	0.62
248%	80%	5,200	5200	0.75	0.71	0.73
279%	90%	5,850	5870	1.23	1.154	1.19
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.556	0.516	0.54

Load at 1.0-in lateral deflection: **5,592** **lb**

Residual lateral deformation: **0.54** **in**



Notes:

Failure reached at 5,870 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-71A

Axial Uplift Load Test Results

PTL - **71** Designation: **A**

Section: W6x9

Latitude: 33.639899

Longitude: -114.68149

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 13.14

Pile Damaged? No

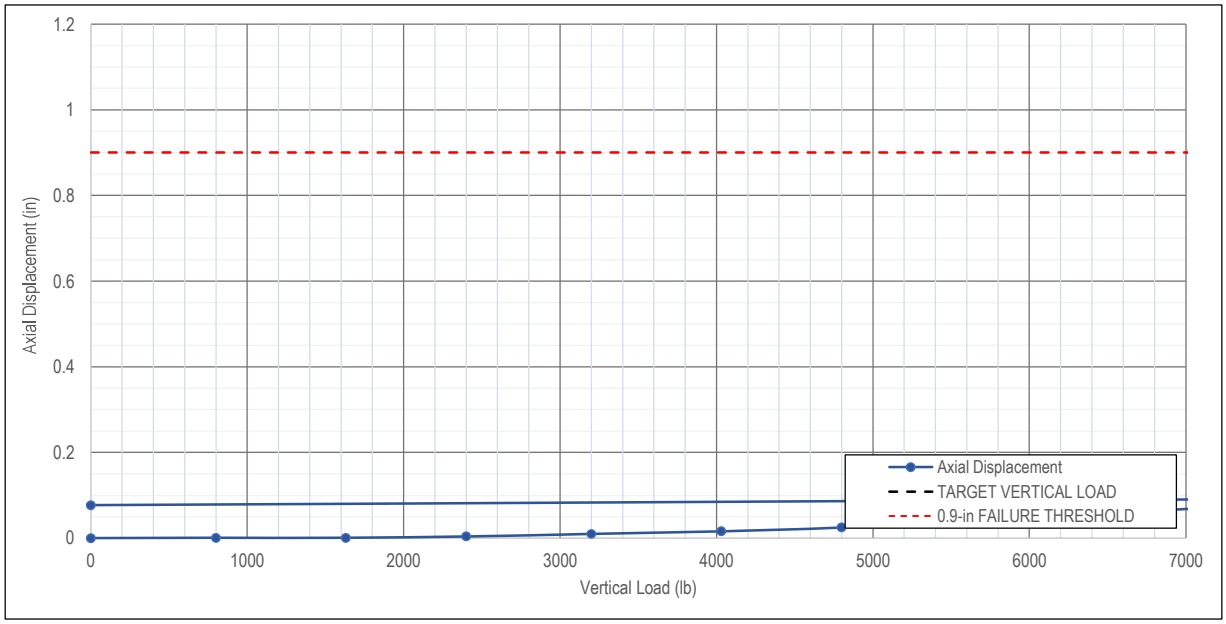
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	800	0.002	0	0.00
13594%	20%	1,600	1630	0.002	0	0.00
20391%	30%	2,400	2400	0.006	0.002	0.00
27188%	40%	3,200	3200	0.012	0.008	0.01
33985%	50%	4,000	4030	0.018	0.014	0.02
40782%	60%	4,800	4800	0.028	0.022	0.03
47579%	70%	5,600	5630	0.048	0.04	0.04
54376%	80%	6,400	6440	0.062	0.052	0.06
61173%	90%	7,200	7230	0.078	0.068	0.07
67970%	100%	8,000	8000	0.096	0.086	0.09
0%	0%	0	0	0.078	0.076	0.08

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.08** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-71A

Lateral Load Test Results

PTL- **71** Designation: **A**

Section: W6x9

Latitude: 33.639899

Longitude: -114.68149

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 13.14

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

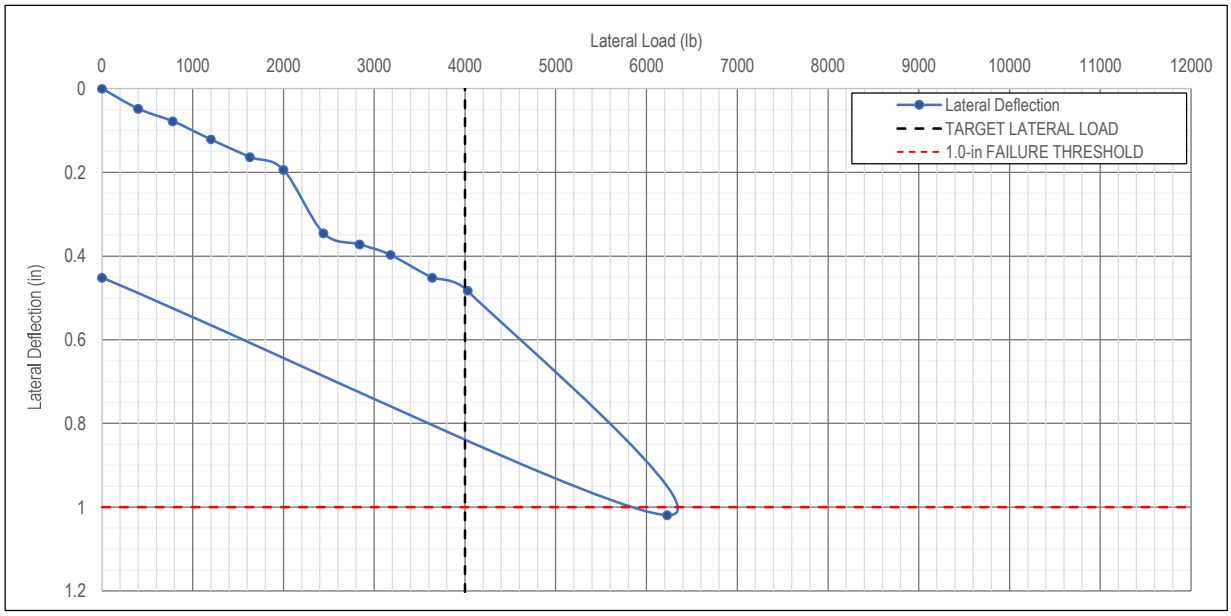
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.05	0.046	0.05
62%	20%	800	780	0.08	0.076	0.08
93%	30%	1,200	1200	0.124	0.118	0.12
124%	40%	1,600	1630	0.168	0.158	0.16
155%	50%	2,000	2000	0.2	0.188	0.19
186%	60%	2,400	2440	0.356	0.334	0.35
217%	70%	2,800	2840	0.384	0.36	0.37
248%	80%	3,200	3180	0.41	0.384	0.40
279%	90%	3,600	3640	0.466	0.438	0.45
310%	100%	4,000	4030	0.498	0.468	0.48
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6230	1.036	1.002	1.02
0%	0%	0	0	0.456	0.448	0.45

Load at 1.0-in lateral deflection: **6,152** **lb**

Residual lateral deformation: **0.45** **in**



Notes:

Failure reached at 6,230 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-71B

Axial Uplift Load Test Results

PTL - **71** Designation: **B**

Section: W6x9

Latitude: 33.639899

Longitude: -114.68149

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 18.75

Pile Damaged? No

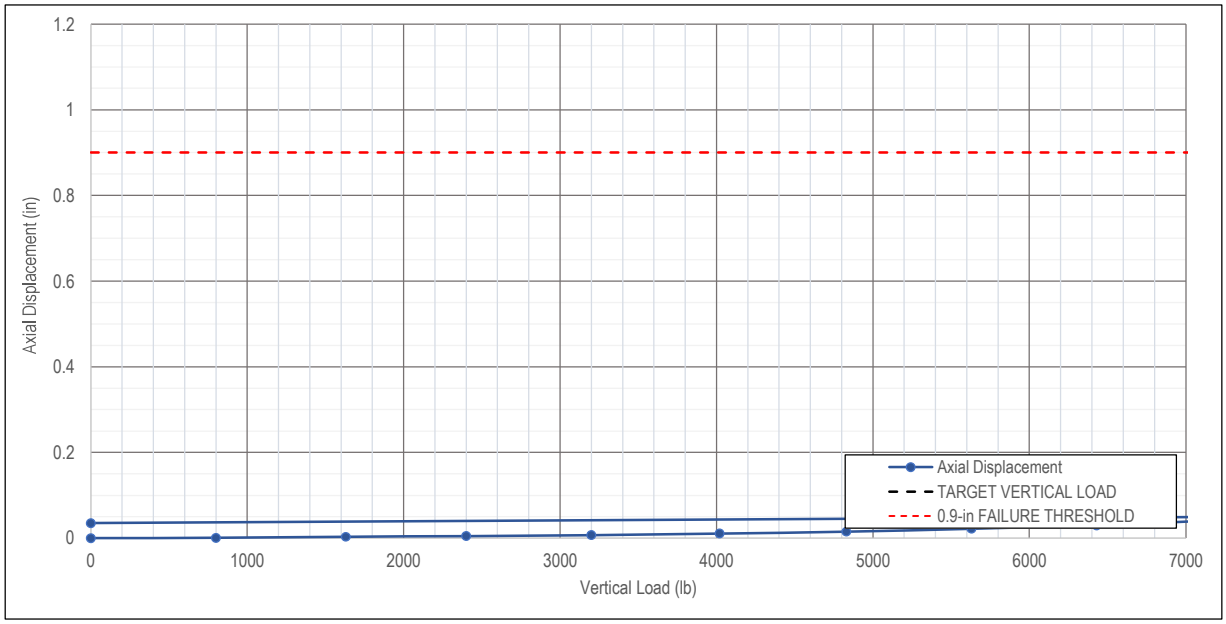
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	800	0.002	0	0.00
13594%	20%	1,600	1630	0.004	0.002	0.00
20391%	30%	2,400	2400	0.006	0.004	0.01
27188%	40%	3,200	3200	0.008	0.006	0.01
33985%	50%	4,000	4020	0.012	0.01	0.01
40782%	60%	4,800	4830	0.016	0.014	0.02
47579%	70%	5,600	5630	0.022	0.022	0.02
54376%	80%	6,400	6430	0.028	0.032	0.03
61173%	90%	7,200	7230	0.038	0.046	0.04
67970%	100%	8,000	8030	0.046	0.056	0.05
0%	0%	0	0	0.034	0.036	0.04

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.04** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-71B

Lateral Load Test Results

PTL- **71** Designation: **B**

Section: W6x9

Latitude: 33.639899

Longitude: -114.68149

Tested By: ANS

Test Date: 10/26/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 18.75

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

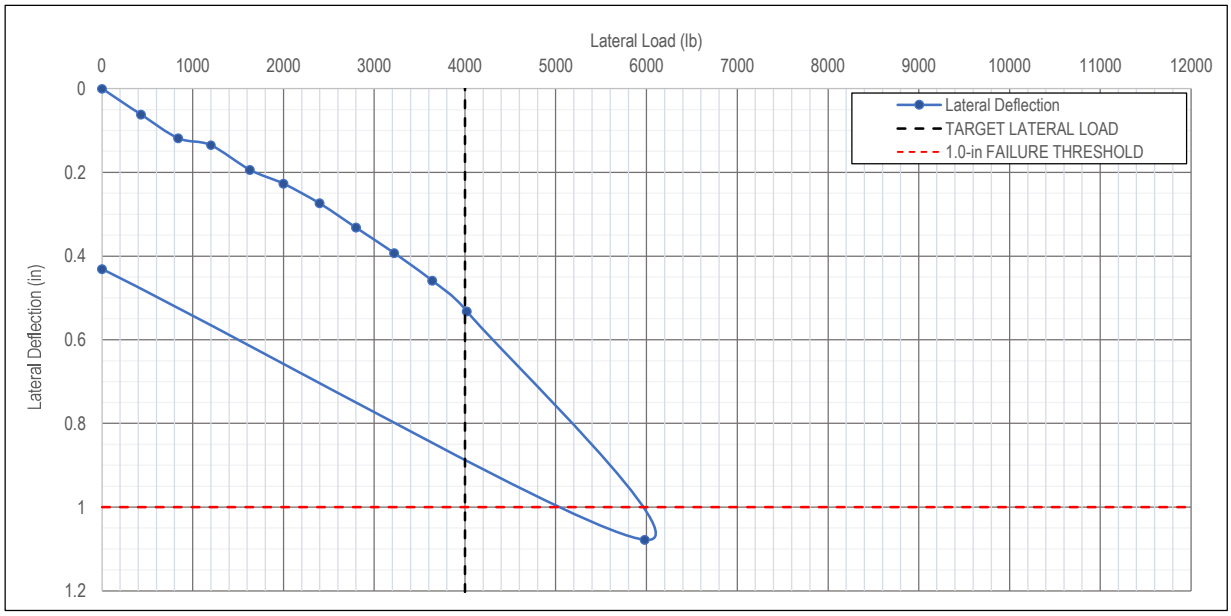
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	430	0.064	0.06	0.06
62%	20%	800	840	0.122	0.114	0.12
93%	30%	1,200	1200	0.14	0.13	0.14
124%	40%	1,600	1630	0.2	0.188	0.19
155%	50%	2,000	2000	0.234	0.22	0.23
186%	60%	2,400	2400	0.282	0.266	0.27
217%	70%	2,800	2800	0.342	0.322	0.33
248%	80%	3,200	3220	0.404	0.382	0.39
279%	90%	3,600	3640	0.47	0.448	0.46
310%	100%	4,000	4020	0.544	0.52	0.53
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			5980	1.02	1.136	1.08
0%	0%	0	0	0.376	0.486	0.43

Load at 1.0-in lateral deflection: **5,700** **lb**

Residual lateral deformation: **0.43** **in**



Notes:

Failure reached at 5,980 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-72A

Axial Uplift Load Test Results

PTL - **72** Designation: **A**

Section: W6x9

Latitude: 33.635789

Longitude: -114.6777

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 19.74

Pile Damaged? No

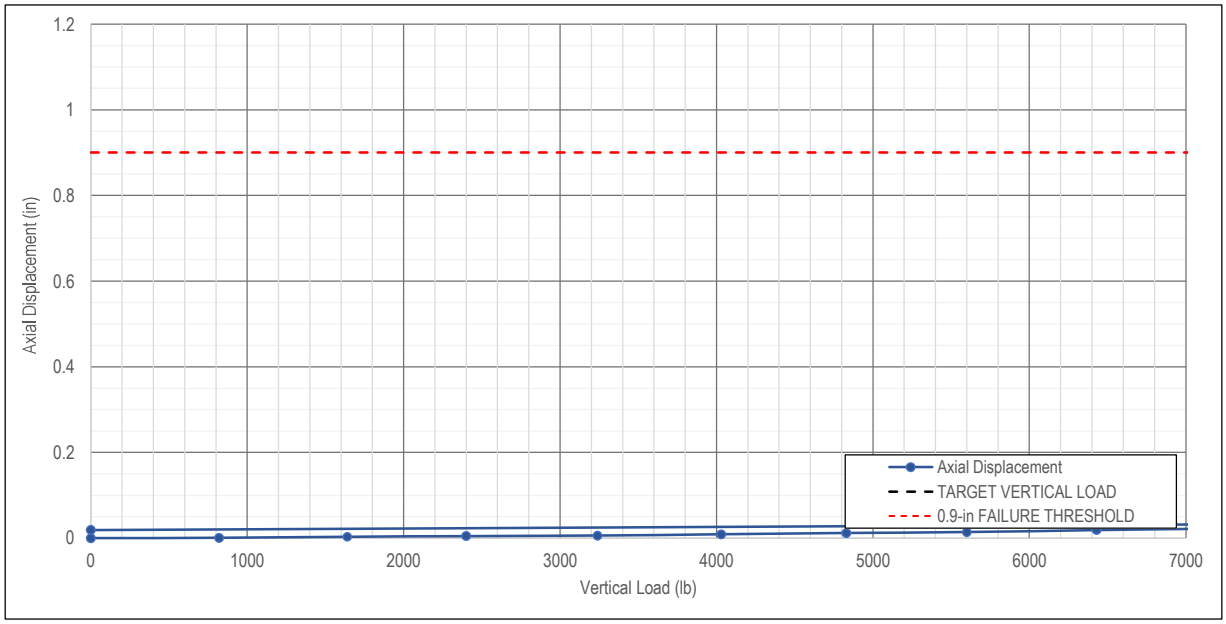
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	820	0.002	0	0.00
436%	20%	1,600	1640	0.004	0.002	0.00
654%	30%	2,400	2400	0.006	0.004	0.01
872%	40%	3,200	3240	0.006	0.006	0.01
1090%	50%	4,000	4030	0.008	0.01	0.01
1308%	60%	4,800	4830	0.012	0.012	0.01
1526%	70%	5,600	5600	0.014	0.014	0.01
1744%	80%	6,400	6430	0.018	0.02	0.02
1962%	90%	7,200	7200	0.022	0.024	0.02
2180%	100%	8,000	8030	0.032	0.034	0.03
0%	0%	0	0	0.018	0.02	0.02

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.02** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-72A

Lateral Load Test Results

PTL- **72** Designation: **A**

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Section: W6x9

Installation Method: Direct Drive (DD)

Saturated Soil? No

Latitude: 33.635789

- -

Drive Time (s): 19.74

Longitude: -114.6777

Target Embedment Depth (ft): 6.00

Pile Damaged? No

Tested By: ANS

Actual Embedment Depth (ft): 6

Gauge 1 Height (in): 6

Test Date: 10/25/2025

Lateral Load Application Height (ft): 5

Gauge 2 Height (in): 6

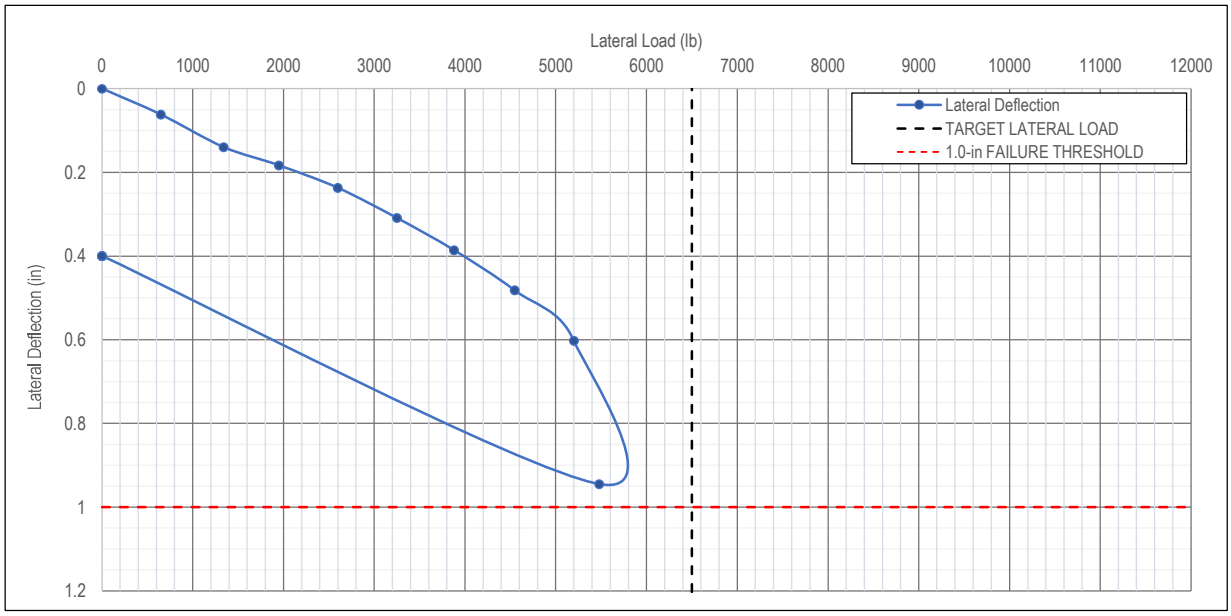
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.064	0.06	0.06
62%	20%	1,300	1340	0.144	0.135	0.14
93%	30%	1,950	1950	0.19	0.176	0.18
124%	40%	2,600	2600	0.246	0.228	0.24
155%	50%	3,250	3250	0.322	0.296	0.31
186%	60%	3,900	3880	0.402	0.37	0.39
217%	70%	4,550	4550	0.506	0.458	0.48
248%	80%	5,200	5200	0.644	0.56	0.60
279%	90%	5,850	5480	1.016	0.874	0.95
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.52	0.28	0.40

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.40** **in**



Notes:

Pile bending at 5,480 lbs. No reliable test results available after this point.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-72B

Axial Uplift Load Test Results

PTL - **72**
Section: W6x9
Latitude: 33.635789
Longitude: -114.6777
Tested By: ANS
Test Date: 10/25/2025

Designation: **B**

Target Test Load (lb): **8,000** (≈ 2180% Design Load)
Installation Method: Direct Drive (DD)
- -
Target Embedment Depth (ft): 8.00
Actual Embedment Depth (ft): 8

Saturated Soil? No
Drive Time (s): 33.27
Pile Damaged? No

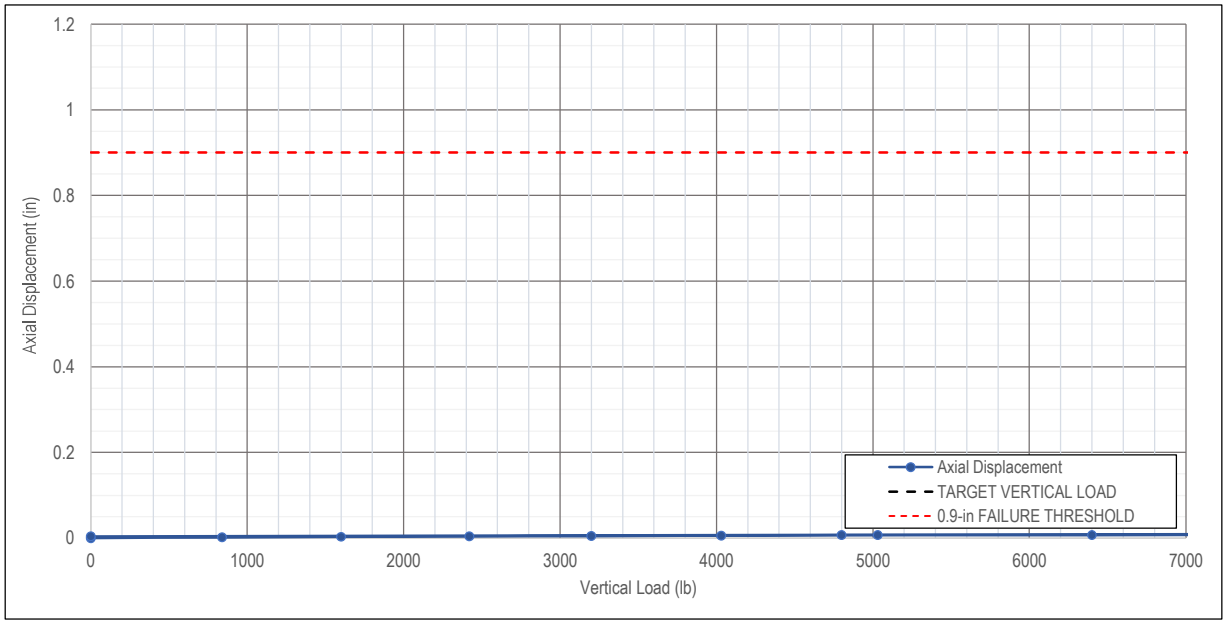
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	840	0.002	0.002	0.00
436%	20%	1,600	1600	0.004	0.002	0.00
654%	30%	2,400	2420	0.006	0.002	0.00
872%	40%	3,200	3200	0.008	0.002	0.01
1090%	50%	4,000	4030	0.01	0.002	0.01
1308%	60%	4,800	4800	0.012	0.002	0.01
1526%	70%	5,600	5030	0.01	0.004	0.01
1744%	80%	6,400	6400	0.008	0.006	0.01
1962%	90%	7,200	7200	0.008	0.008	0.01
2180%	100%	8,000	8020	0.008	0.012	0.01
0%	0%	0	0	0.004	0.004	0.00

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.00** **in**



Notes:

Pile twisting at 4,000 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-72B

Lateral Load Test Results

PTL- **72** Designation: **B**

Section: W6x9

Latitude: 33.635789

Longitude: -114.6777

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 33.27

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

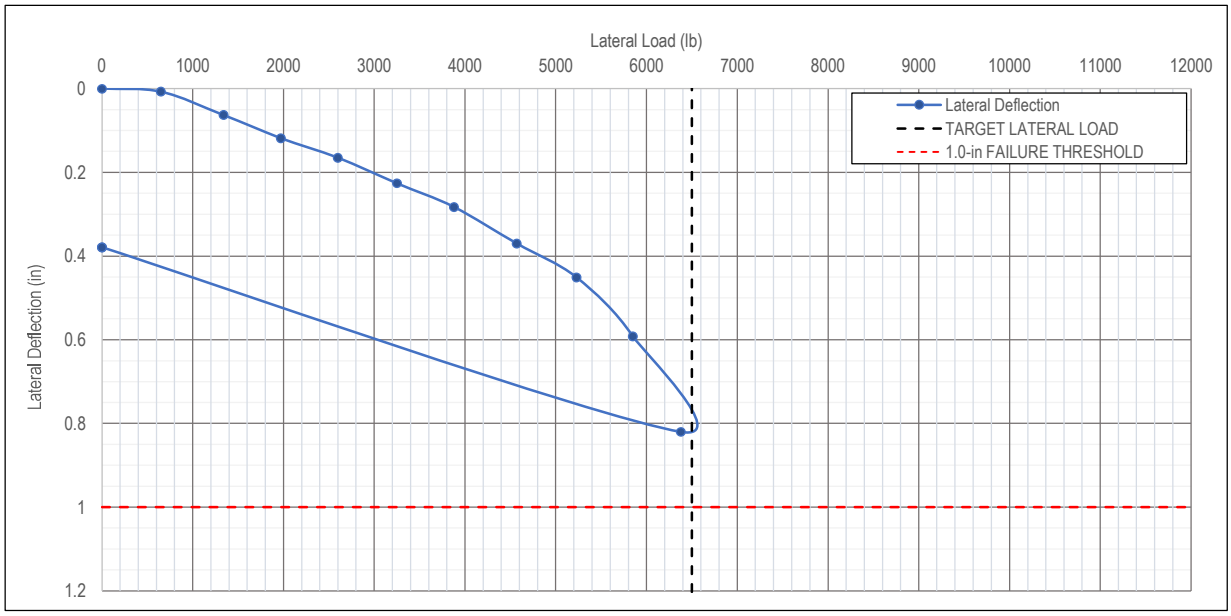
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.01	0.004	0.01
62%	20%	1,300	1340	0.068	0.058	0.06
93%	30%	1,950	1970	0.124	0.112	0.12
124%	40%	2,600	2600	0.174	0.156	0.17
155%	50%	3,250	3250	0.238	0.214	0.23
186%	60%	3,900	3880	0.298	0.268	0.28
217%	70%	4,550	4570	0.39	0.35	0.37
248%	80%	5,200	5230	0.478	0.424	0.45
279%	90%	5,850	5850	0.64	0.544	0.59
310%	100%	6,500	6380	0.884	0.756	0.82
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.426	0.332	0.38

Load at 1.0-in lateral deflection: **N/A** **lb**

Residual lateral deformation: **0.38** **in**



Notes:

Pile bending at 6,380 lbs. No reliable test results vailable after this point.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-72D

Axial Uplift Load Test Results

PTL - **72**
Section: **W6x12**
Latitude: 33.635789
Longitude: -114.6777
Tested By: ANS
Test Date: 10/25/2025

Designation: **D**

Target Test Load (lb): **8,000** (≈ 2180% Design Load)
Installation Method: Direct Drive (DD)
- -
Target Embedment Depth (ft): 5.00
Actual Embedment Depth (ft): 5

Saturated Soil? No
Drive Time (s): 33.06
Pile Damaged? No

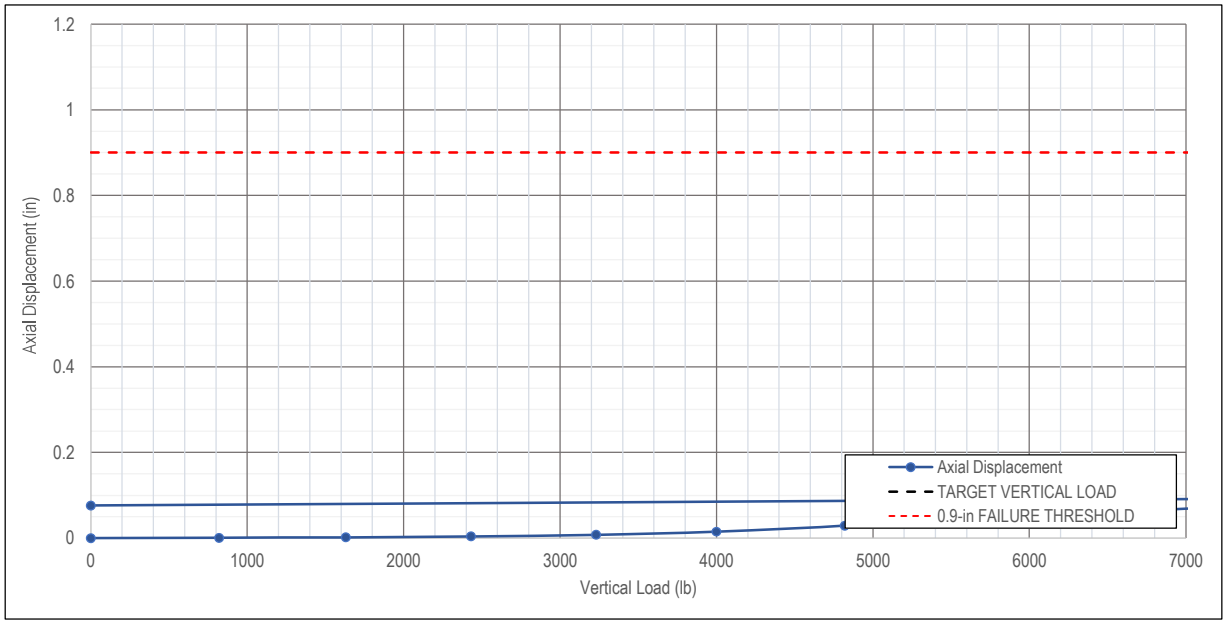
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	820	0	0.002	0.00
436%	20%	1,600	1630	0.002	0.002	0.00
654%	30%	2,400	2430	0.004	0.004	0.00
872%	40%	3,200	3230	0.008	0.008	0.01
1090%	50%	4,000	4000	0.014	0.016	0.02
1308%	60%	4,800	4820	0.028	0.03	0.03
1526%	70%	5,600	5030	0.04	0.044	0.04
1744%	80%	6,400	6400	0.056	0.062	0.06
1962%	90%	7,200	7200	0.07	0.076	0.07
2180%	100%	8,000	8000	0.088	0.096	0.09
0%	0%	0	0	0.076	0.076	0.08

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.08** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-72D

Lateral Load Test Results

PTL- **72** Designation: **D**

Section: **W6x12**

Latitude: 33.635789

Longitude: -114.6777

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 33.06

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

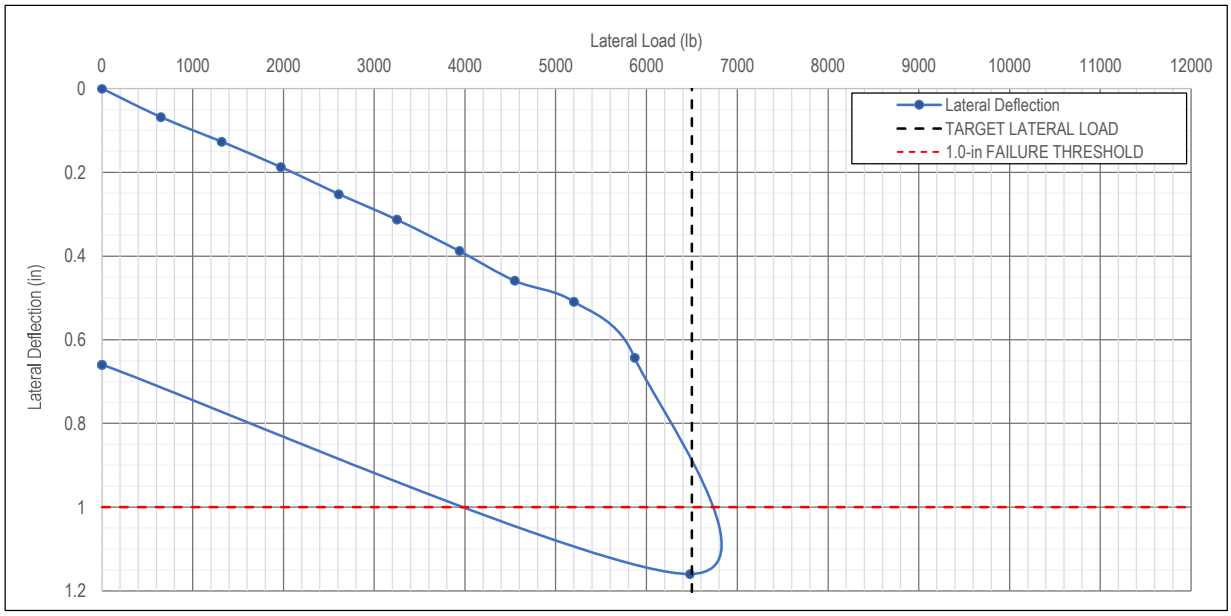
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.07	0.066	0.07
62%	20%	1,300	1320	0.13	0.124	0.13
93%	30%	1,950	1970	0.192	0.182	0.19
124%	40%	2,600	2610	0.26	0.244	0.25
155%	50%	3,250	3250	0.324	0.302	0.31
186%	60%	3,900	3940	0.402	0.374	0.39
217%	70%	4,550	4550	0.476	0.442	0.46
248%	80%	5,200	5200	0.528	0.49	0.51
279%	90%	5,850	5870	0.674	0.612	0.64
310%	100%	6,500	6480	1.236	1.084	1.16
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.724	0.596	0.66

Load at 1.0-in lateral deflection: **6,291** **lb**

Residual lateral deformation: **0.66** **in**



Notes:

Failure reached at 6,480 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-73A

Axial Uplift Load Test Results

PTL - **73** Designation: **A**

Section: W6x9

Latitude: 33.633618

Longitude: -114.68781

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 11.19

Pile Damaged? No

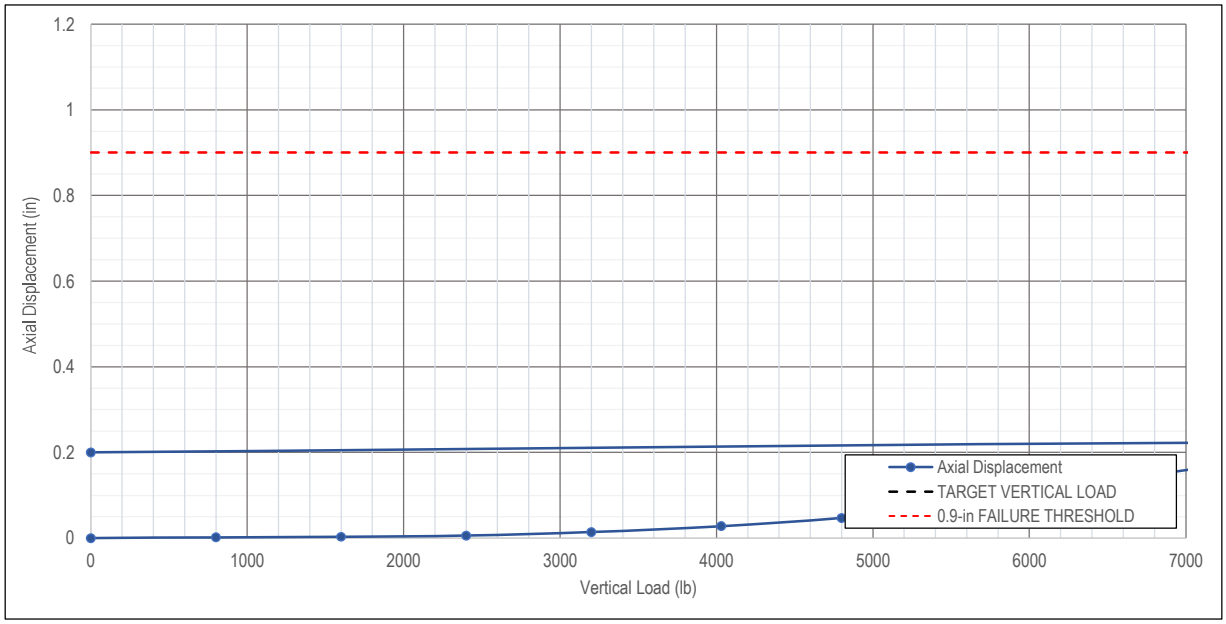
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	800	0.002	0.002	0.00
13594%	20%	1,600	1600	0.004	0.002	0.00
20391%	30%	2,400	2400	0.006	0.006	0.01
27188%	40%	3,200	3200	0.014	0.014	0.01
33985%	50%	4,000	4030	0.028	0.028	0.03
40782%	60%	4,800	4800	0.048	0.046	0.05
47579%	70%	5,600	5600	0.076	0.076	0.08
54376%	80%	6,400	6430	0.128	0.13	0.13
61173%	90%	7,200	7200	0.168	0.172	0.17
67970%	100%	8,000	8000	0.22	0.224	0.22
0%	0%	0	0	0.2	0.2	0.20

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.20** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-73A

Lateral Load Test Results

PTL- **73** Designation: **A**

Section: W6x9

Latitude: 33.633618

Longitude: -114.68781

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 11.19

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

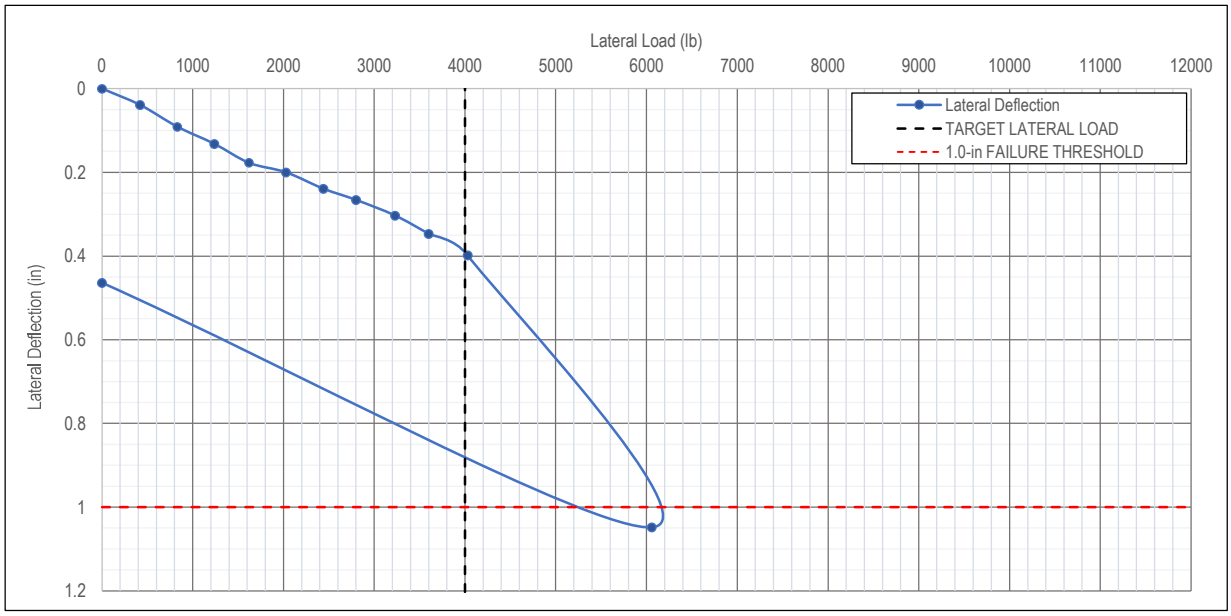
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	420	0.04	0.038	0.04
62%	20%	800	830	0.094	0.088	0.09
93%	30%	1,200	1240	0.136	0.128	0.13
124%	40%	1,600	1620	0.182	0.172	0.18
155%	50%	2,000	2030	0.206	0.194	0.20
186%	60%	2,400	2440	0.246	0.232	0.24
217%	70%	2,800	2800	0.274	0.258	0.27
248%	80%	3,200	3230	0.312	0.294	0.30
279%	90%	3,600	3600	0.358	0.336	0.35
310%	100%	4,000	4030	0.408	0.388	0.40
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			6060	0.962	1.134	1.05
0%	0%	0	0	0.394	0.534	0.46

Load at 1.0-in lateral deflection: **5,910** **lb**

Residual lateral deformation: **0.46** **in**



Notes:

Failure reached at 6,060 lbs

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-73B

Axial Uplift Load Test Results

PTL - **73** Designation: **B**

Section: W6x9

Latitude: 33.633618

Longitude: -114.68781

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 11.54

Pile Damaged? No

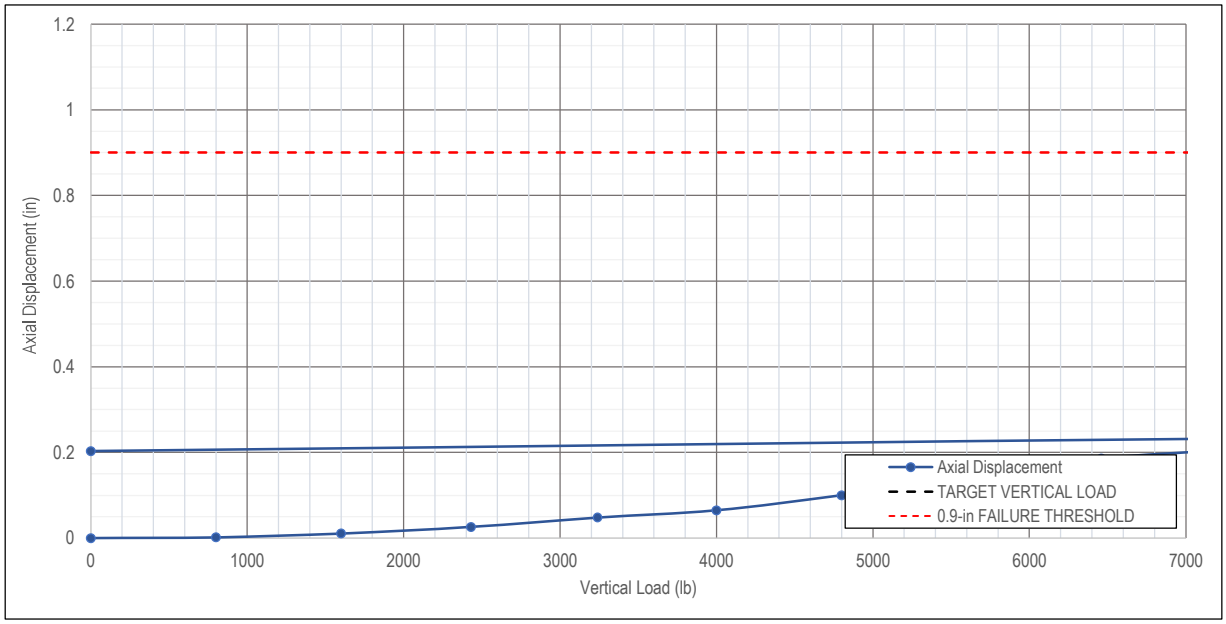
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	800	0.002	0.002	0.00
13594%	20%	1,600	1600	0.012	0.01	0.01
20391%	30%	2,400	2430	0.028	0.024	0.03
27188%	40%	3,200	3240	0.052	0.044	0.05
33985%	50%	4,000	4000	0.076	0.054	0.07
40782%	60%	4,800	4800	0.106	0.094	0.10
47579%	70%	5,600	5630	0.148	0.134	0.14
54376%	80%	6,400	6460	0.192	0.18	0.19
61173%	90%	7,200	7240	0.212	0.2	0.21
67970%	100%	8,000	8000	0.24	0.228	0.23
0%	0%	0	0	0.204	0.202	0.20

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.20** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-73B

Lateral Load Test Results

PTL- **73** Designation: **B**

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Section: W6x9

Installation Method: Direct Drive (DD)

Saturated Soil? No

Latitude: 33.633618

- -

Drive Time (s): 11.54

Longitude: -114.68781

Target Embedment Depth (ft): 7.00

Pile Damaged? No

Tested By: ANS

Actual Embedment Depth (ft): 7

Gauge 1 Height (in): 6

Test Date: 10/25/2025

Lateral Load Application Height (ft): 5

Gauge 2 Height (in): 6

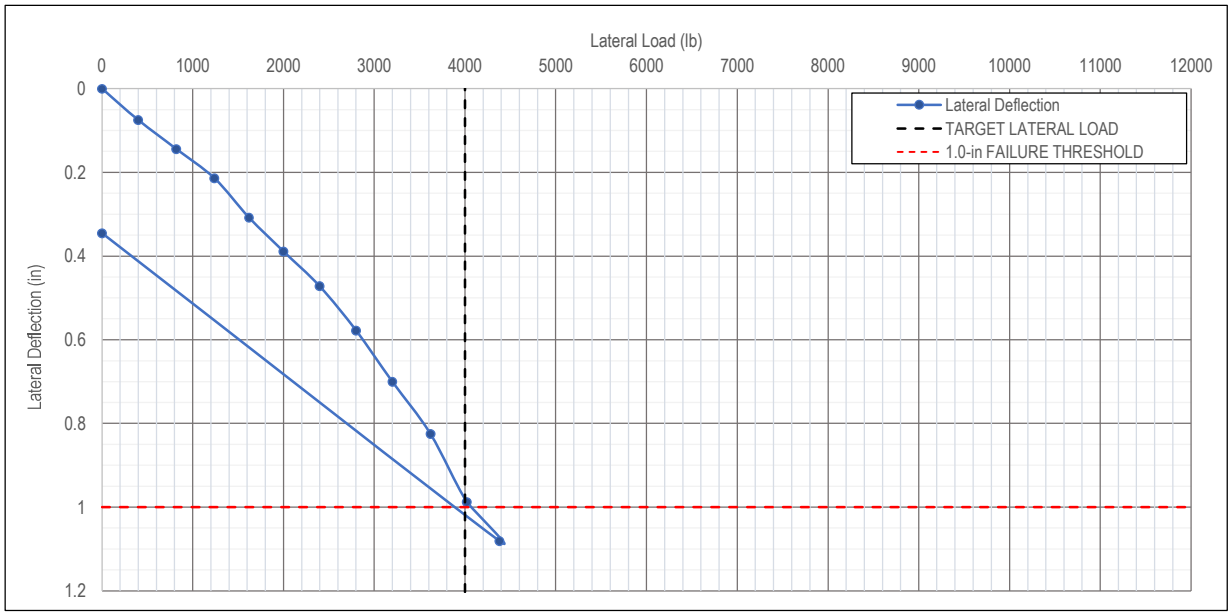
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.08	0.07	0.08
62%	20%	800	820	0.15	0.138	0.14
93%	30%	1,200	1240	0.272	0.156	0.21
124%	40%	1,600	1620	0.316	0.3	0.31
155%	50%	2,000	2000	0.398	0.38	0.39
186%	60%	2,400	2400	0.484	0.46	0.47
217%	70%	2,800	2800	0.592	0.564	0.58
248%	80%	3,200	3200	0.716	0.684	0.70
279%	90%	3,600	3620	0.844	0.806	0.83
310%	100%	4,000	4020	1.01	0.966	0.99
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->			4380	1.104	1.058	1.08
0%	0%	0	0	0.348	0.342	0.35

Load at 1.0-in lateral deflection: **4,066** **lb**

Residual lateral deformation: **0.35** **in**



Notes:

Failure reached at 4,380 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-74A

Axial Uplift Load Test Results

PTL - **74** Designation: **A**

Section: W6x9

Latitude: 33.633657

Longitude: -114.68153

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Saturated Soil? No

Drive Time (s): 27.07

Pile Damaged? No

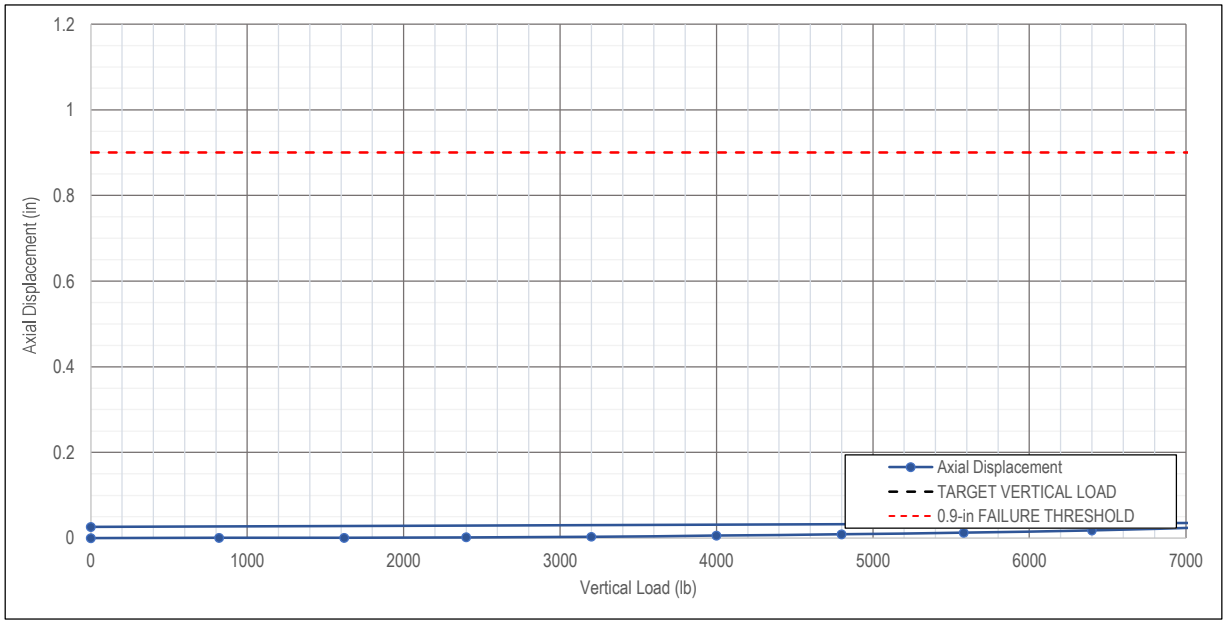
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	820	0	0.002	0.00
436%	20%	1,600	1620	0	0.002	0.00
654%	30%	2,400	2400	0	0.004	0.00
872%	40%	3,200	3200	0	0.006	0.00
1090%	50%	4,000	4000	0.002	0.01	0.01
1308%	60%	4,800	4800	0.004	0.014	0.01
1526%	70%	5,600	5580	0.006	0.02	0.01
1744%	80%	6,400	6400	0.01	0.026	0.02
1962%	90%	7,200	7230	0.016	0.036	0.03
2180%	100%	8,000	8030	0.026	0.046	0.04
0%	0%	0	0	0.026	0.026	0.03

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.03** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-74A

Lateral Load Test Results

PTL- **74** Designation: **A**

Section: W6x9

Latitude: 33.633657

Longitude: -114.68153

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 27.07

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

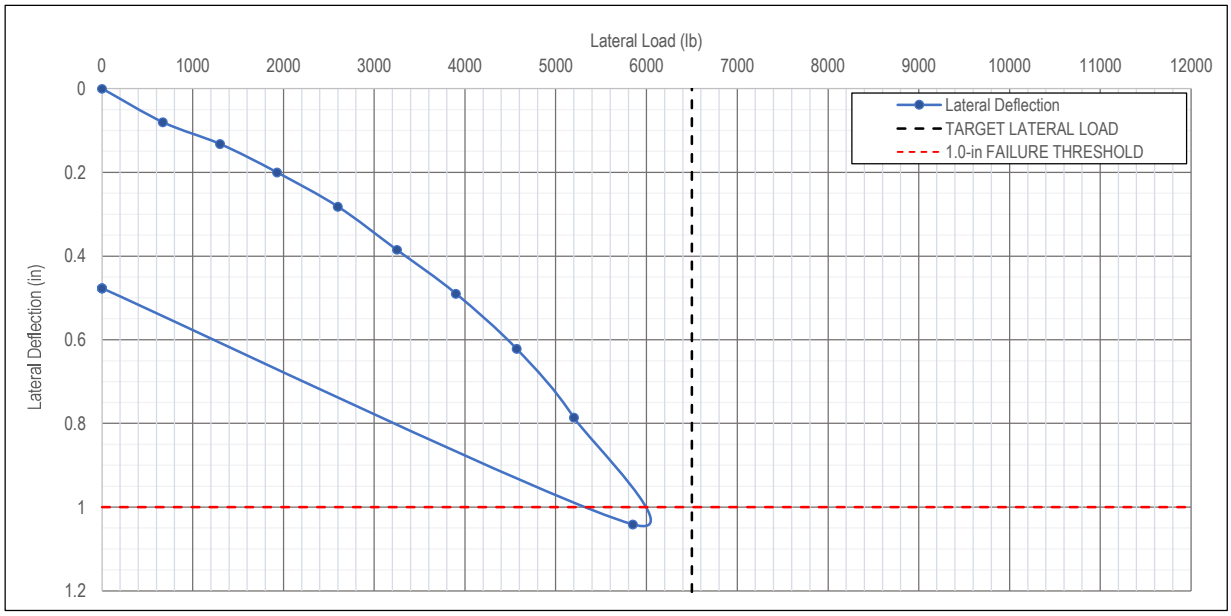
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	670	0.082	0.078	0.08
62%	20%	1,300	1300	0.136	0.128	0.13
93%	30%	1,950	1930	0.206	0.194	0.20
124%	40%	2,600	2600	0.29	0.274	0.28
155%	50%	3,250	3250	0.396	0.374	0.39
186%	60%	3,900	3900	0.504	0.476	0.49
217%	70%	4,550	4570	0.638	0.606	0.62
248%	80%	5,200	5200	0.802	0.77	0.79
279%	90%	5,850	5850	1.066	1.016	1.04
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.464	0.49	0.48

Load at 1.0-in lateral deflection: **5,745** **lb**

Residual lateral deformation: **0.48** **in**



Notes:

Failure reached at 5,850 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-74B

Axial Uplift Load Test Results

PTL - **74** Designation: **B** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.633657 - - Saturated Soil? No

Longitude: -114.68153 Target Embedment Depth (ft): 8.00 Drive Time (s): 17.17

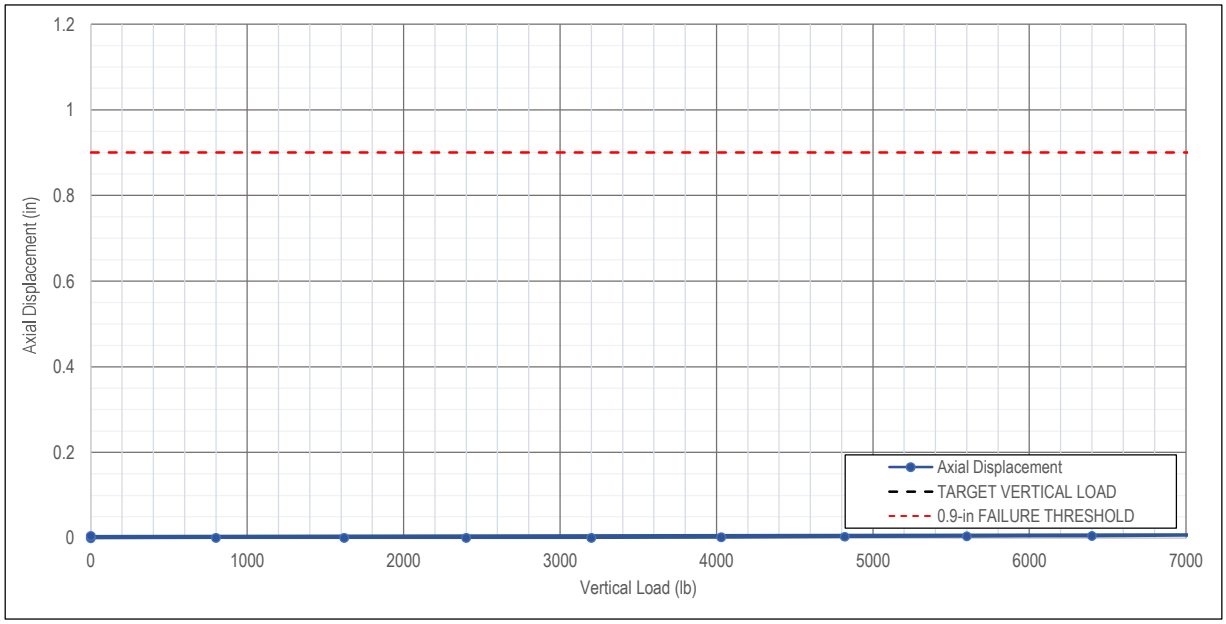
Tested By: ANS Actual Embedment Depth (ft): 8 Pile Damaged? No

Test Date: 10/25/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0.002	0	0.00
436%	20%	1,600	1620	0.002	0	0.00
654%	30%	2,400	2400	0.002	0	0.00
872%	40%	3,200	3200	0.002	0	0.00
1090%	50%	4,000	4030	0.004	0	0.00
1308%	60%	4,800	4820	0.004	0.002	0.00
1526%	70%	5,600	5600	0.006	0.002	0.00
1744%	80%	6,400	6400	0.008	0.002	0.01
1962%	90%	7,200	7230	0.01	0.004	0.01
2180%	100%	8,000	8030	0.012	0.006	0.01
0%	0%	0	0	0.006	0.004	0.01

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.01** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-74B

Lateral Load Test Results

PTL-	**74**	Designation:	**B**		
Section:	W6x9				
Latitude:	33.633657				
Longitude:	-114.68153				
Tested By:	ANS				
Test Date:	10/25/2025				

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 17.17

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

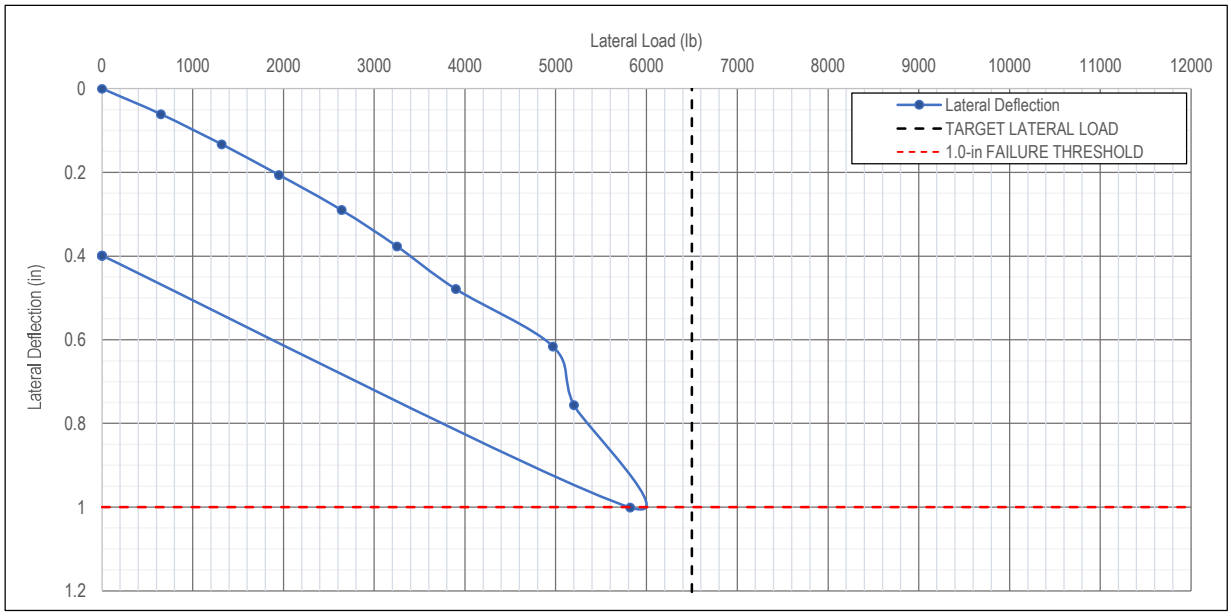
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.068	0.054	0.06
62%	20%	1,300	1320	0.144	0.122	0.13
93%	30%	1,950	1950	0.218	0.194	0.21
124%	40%	2,600	2640	0.306	0.274	0.29
155%	50%	3,250	3250	0.395	0.358	0.38
186%	60%	3,900	3900	0.5	0.458	0.48
217%	70%	4,550	4970	0.638	0.594	0.62
248%	80%	5,200	5200	0.776	0.736	0.76
279%	90%	5,850	5820	1.002	1	1.00
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.39	0.408	0.40

Load at 1.0-in lateral deflection: **5,817** **lb**

Residual lateral deformation: **0.40** **in**



Notes:

Failure reached at 5,820 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-75A

Axial Uplift Load Test Results

PTL - **75** Designation: **A**

Section: W6x9

Latitude: 33.636991

Longitude: -114.68677

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 7.17

Pile Damaged? No

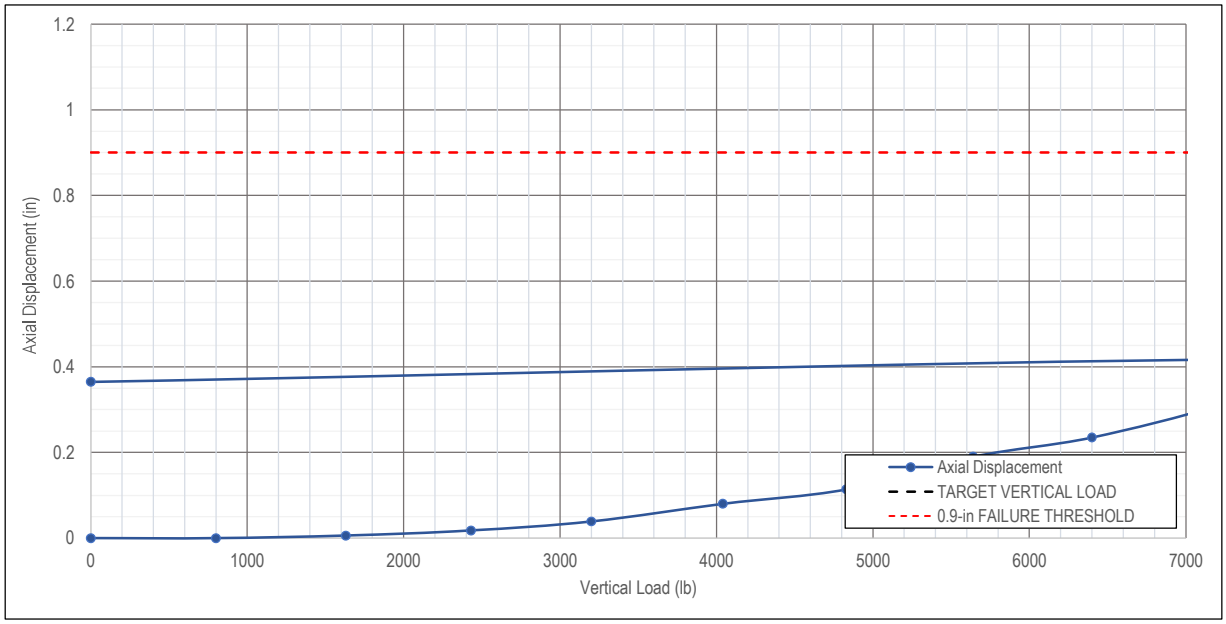
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	800	0	0	0.00
13594%	20%	1,600	1630	0.006	0.006	0.01
20391%	30%	2,400	2430	0.018	0.018	0.02
27188%	40%	3,200	3200	0.04	0.038	0.04
33985%	50%	4,000	4040	0.08	0.08	0.08
40782%	60%	4,800	4830	0.114	0.114	0.11
47579%	70%	5,600	5640	0.19	0.19	0.19
54376%	80%	6,400	6400	0.234	0.236	0.24
61173%	90%	7,200	7230	0.31	0.314	0.31
67970%	100%	8,000	8040	0.414	0.418	0.42
0%	0%	0	0	0.364	0.366	0.37

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.37** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-75A

Lateral Load Test Results

PTL- **75** Designation: **A**

Section: W6x9

Latitude: 33.636991

Longitude: -114.68677

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 7.17

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

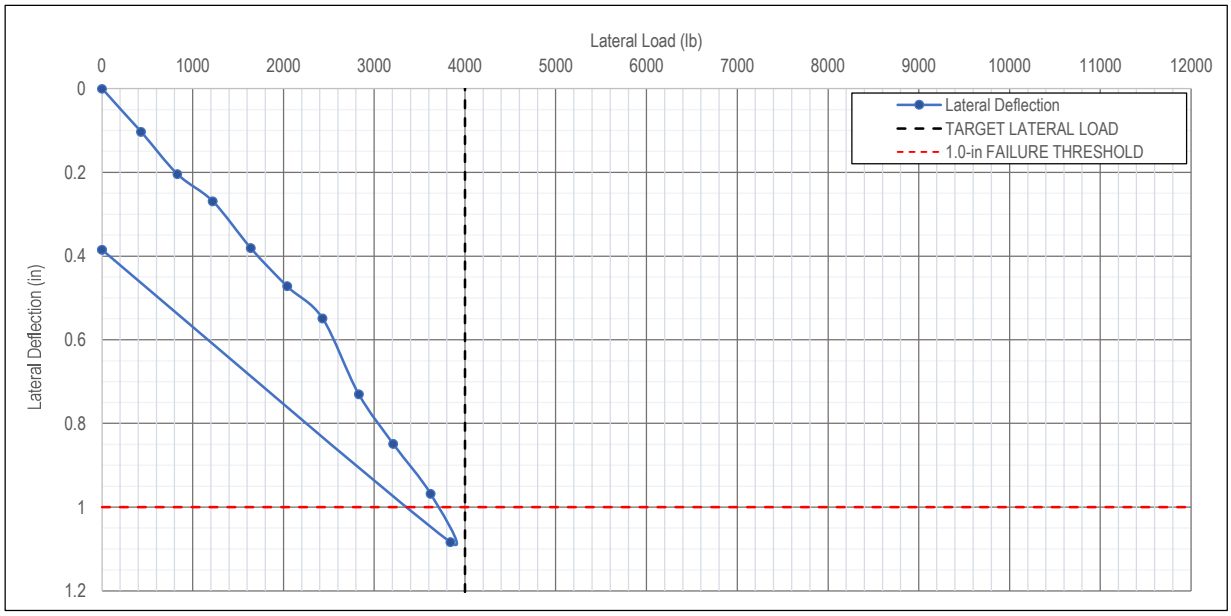
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	430	0.104	0.102	0.10
62%	20%	800	830	0.208	0.2	0.20
93%	30%	1,200	1220	0.274	0.264	0.27
124%	40%	1,600	1640	0.388	0.374	0.38
155%	50%	2,000	2040	0.482	0.462	0.47
186%	60%	2,400	2430	0.562	0.536	0.55
217%	70%	2,800	2830	0.746	0.714	0.73
248%	80%	3,200	3210	0.868	0.83	0.85
279%	90%	3,600	3620	0.99	0.946	0.97
310%	100%	4,000	3840	1.108	1.06	1.08
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.39	0.38	0.39

Load at 1.0-in lateral deflection: **3,681** **lb**

Residual lateral deformation: **0.39** **in**



Notes:

Failure reached at 3,840 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-75B

Axial Uplift Load Test Results

PTL - **75** Designation: **B**

Section: W6x9

Latitude: 33.636991

Longitude: -114.68677

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 67970% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 7.00

Actual Embedment Depth (ft): 7

Saturated Soil? No

Drive Time (s): 13.81

Pile Damaged? No

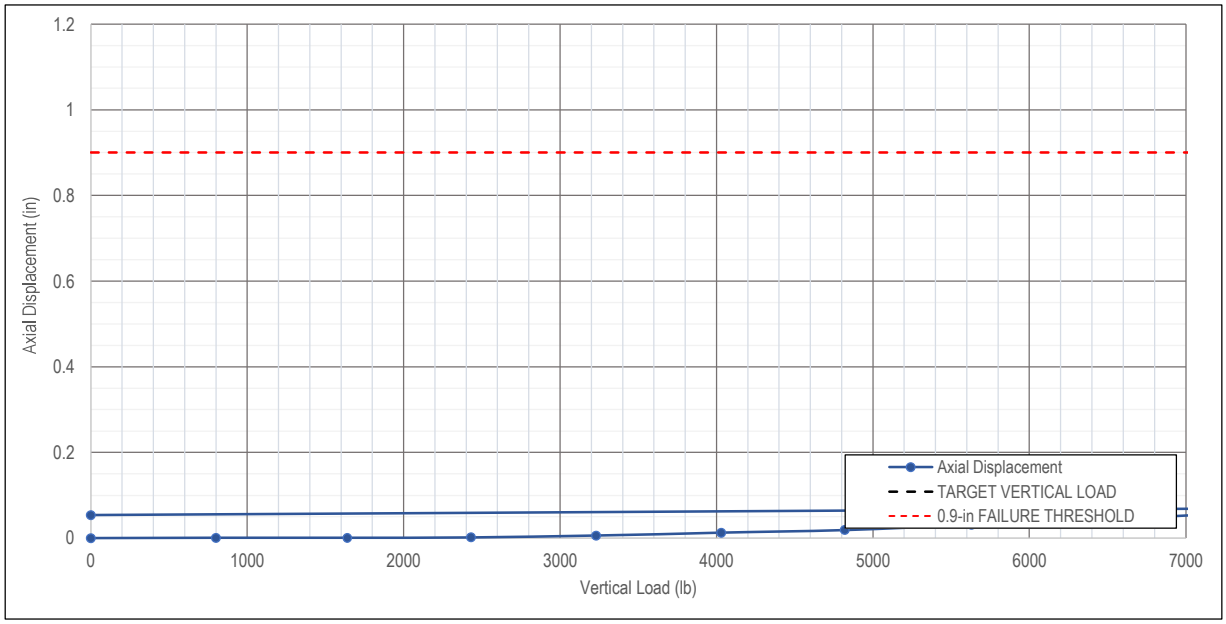
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
6797%	10%	800	800	0.002	0	0.00
13594%	20%	1,600	1640	0.002	0	0.00
20391%	30%	2,400	2430	0.002	0.002	0.00
27188%	40%	3,200	3230	0.004	0.008	0.01
33985%	50%	4,000	4030	0.01	0.016	0.01
40782%	60%	4,800	4820	0.016	0.022	0.02
47579%	70%	5,600	5630	0.026	0.036	0.03
54376%	80%	6,400	6440	0.038	0.048	0.04
61173%	90%	7,200	7230	0.05	0.064	0.06
67970%	100%	8,000	8000	0.062	0.078	0.07
0%	0%	0	0	0.05	0.058	0.05

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.05** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-75B

Lateral Load Test Results

PTL- **75** Designation: **B**

Target Test Load (lb): **4,000** (≈ 310% Design Load)

Section: W6x9

Installation Method: Direct Drive (DD)

Saturated Soil? No

Latitude: 33.636991

- -

Drive Time (s): 13.81

Longitude: -114.68677

Target Embedment Depth (ft): 7.00

Pile Damaged? No

Tested By: ANS

Actual Embedment Depth (ft): 7

Gauge 1 Height (in): 6

Test Date: 10/25/2025

Lateral Load Application Height (ft): 5

Gauge 2 Height (in): 6

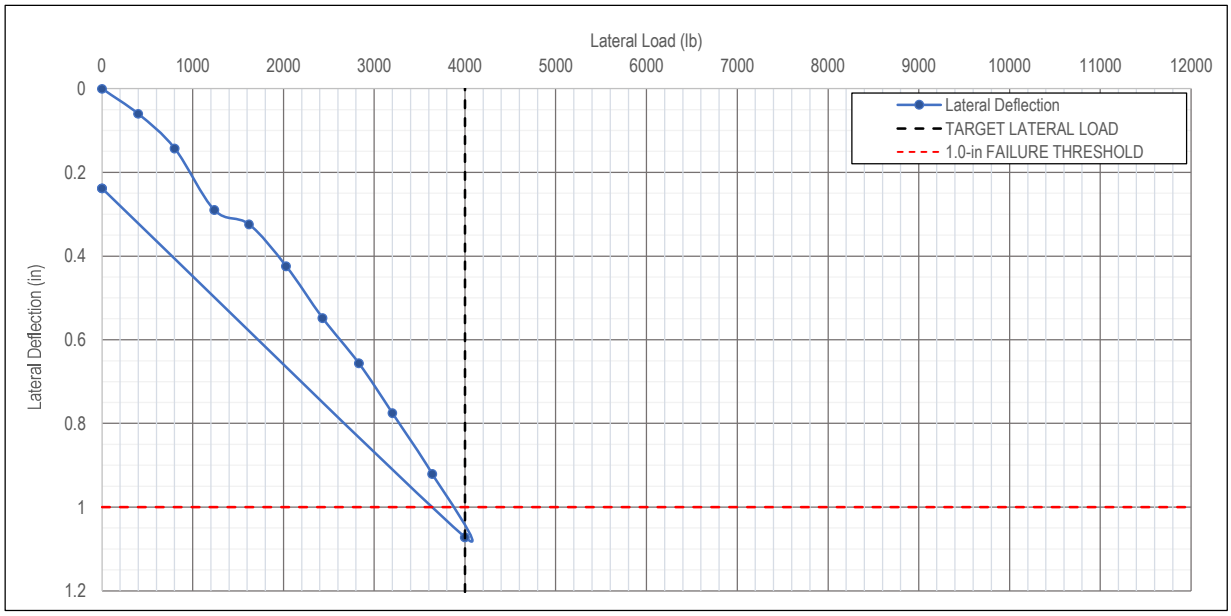
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	400	400	0.062	0.058	0.06
62%	20%	800	800	0.149	0.136	0.14
93%	30%	1,200	1240	0.3	0.28	0.29
124%	40%	1,600	1620	0.338	0.31	0.32
155%	50%	2,000	2030	0.436	0.412	0.42
186%	60%	2,400	2430	0.566	0.53	0.55
217%	70%	2,800	2830	0.676	0.636	0.66
248%	80%	3,200	3200	0.8	0.75	0.78
279%	90%	3,600	3640	0.952	0.89	0.92
310%	100%	4,000	4000	1.112	1.032	1.07
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.246	0.23	0.24

Load at 1.0-in lateral deflection: **3,828** **lb**

Residual lateral deformation: **0.24** **in**



Notes:

Failure reached at 4,000 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

PT-76A

Axial Uplift Load Test Results

PTL - **76** Designation: **A** Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Section: W6x9 Installation Method: Direct Drive (DD)

Latitude: 33.637262 - - Saturated Soil? No

Longitude: -114.68165 Target Embedment Depth (ft): 6.00 Drive Time (s): 17.81

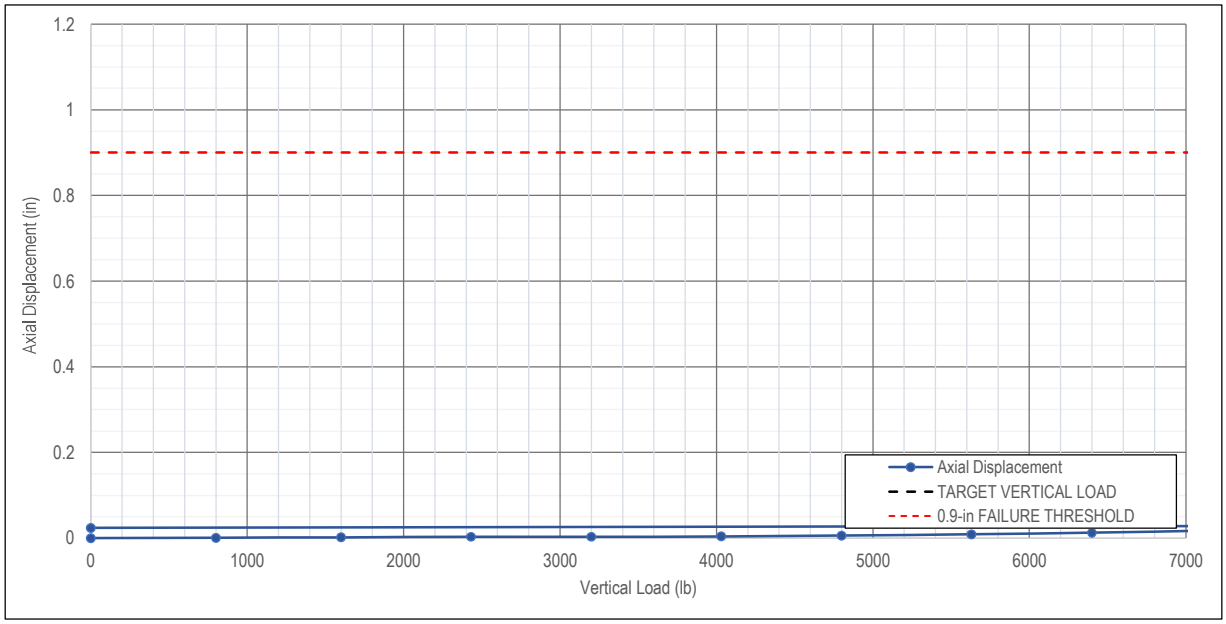
Tested By: ANS Actual Embedment Depth (ft): 6 Pile Damaged? No

Test Date: 10/25/2025

Strong Axis Plumb. (deg): Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0.002	0	0.00
436%	20%	1,600	1600	0.002	0.002	0.00
654%	30%	2,400	2430	0.004	0.002	0.00
872%	40%	3,200	3200	0.004	0.002	0.00
1090%	50%	4,000	4030	0.004	0.004	0.00
1308%	60%	4,800	4800	0.006	0.006	0.01
1526%	70%	5,600	5630	0.008	0.01	0.01
1744%	80%	6,400	6400	0.012	0.014	0.01
1962%	90%	7,200	7200	0.016	0.02	0.02
2180%	100%	8,000	8000	0.024	0.032	0.03
0%	0%	0	0	0.024	0.024	0.02

Load at 0.9-in axial displacement: **N/A** **lb** **Residual axial displacement:** **0.02** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-76A

Lateral Load Test Results

PTL- **76** Designation: **A**

Section: W6x9

Latitude: 33.637262

Longitude: -114.68165

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 6.00

Actual Embedment Depth (ft): 6

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 17.81

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

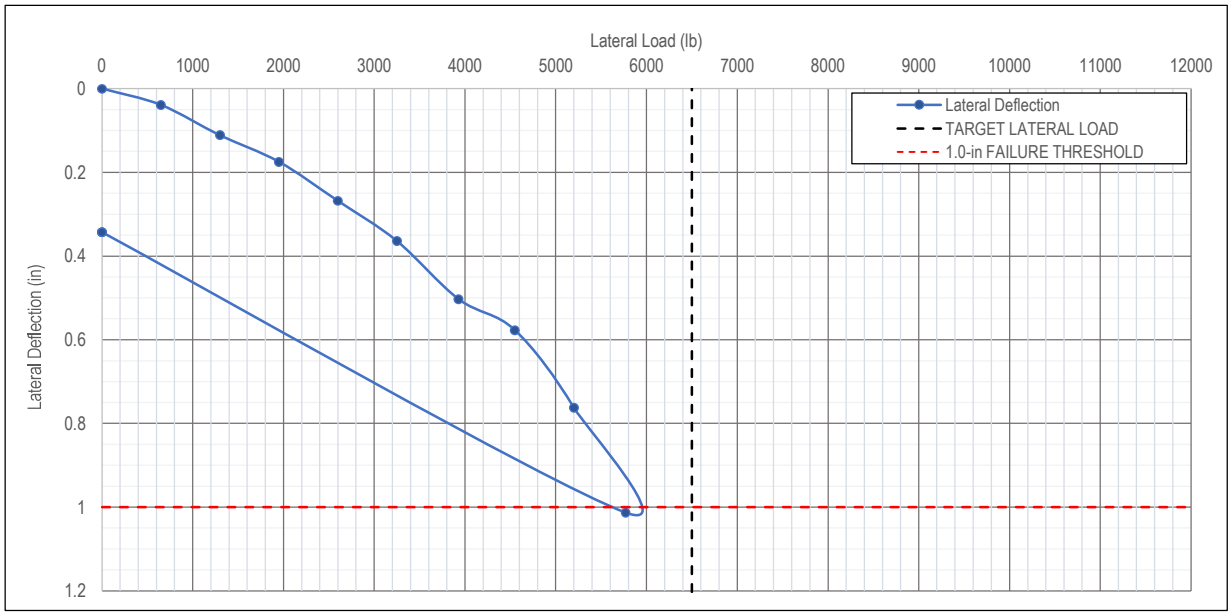
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	650	0.038	0.04	0.04
62%	20%	1,300	1300	0.11	0.112	0.11
93%	30%	1,950	1950	0.176	0.174	0.18
124%	40%	2,600	2600	0.272	0.264	0.27
155%	50%	3,250	3250	0.37	0.358	0.36
186%	60%	3,900	3930	0.508	0.498	0.50
217%	70%	4,550	4550	0.59	0.564	0.58
248%	80%	5,200	5200	0.792	0.732	0.76
279%	90%	5,850	5770	1.106	0.92	1.01
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.392	0.294	0.34

Load at 1.0-in lateral deflection: **5,740** **lb**

Residual lateral deformation: **0.34** **in**



Notes:

Failure reached at 5,770 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-76B

Axial Uplift Load Test Results

PTL - **76** Designation: **B**

Section: W6x9

Latitude: 33.637262

Longitude: -114.68165

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Saturated Soil? No

Drive Time (s): 33.67

Pile Damaged? No

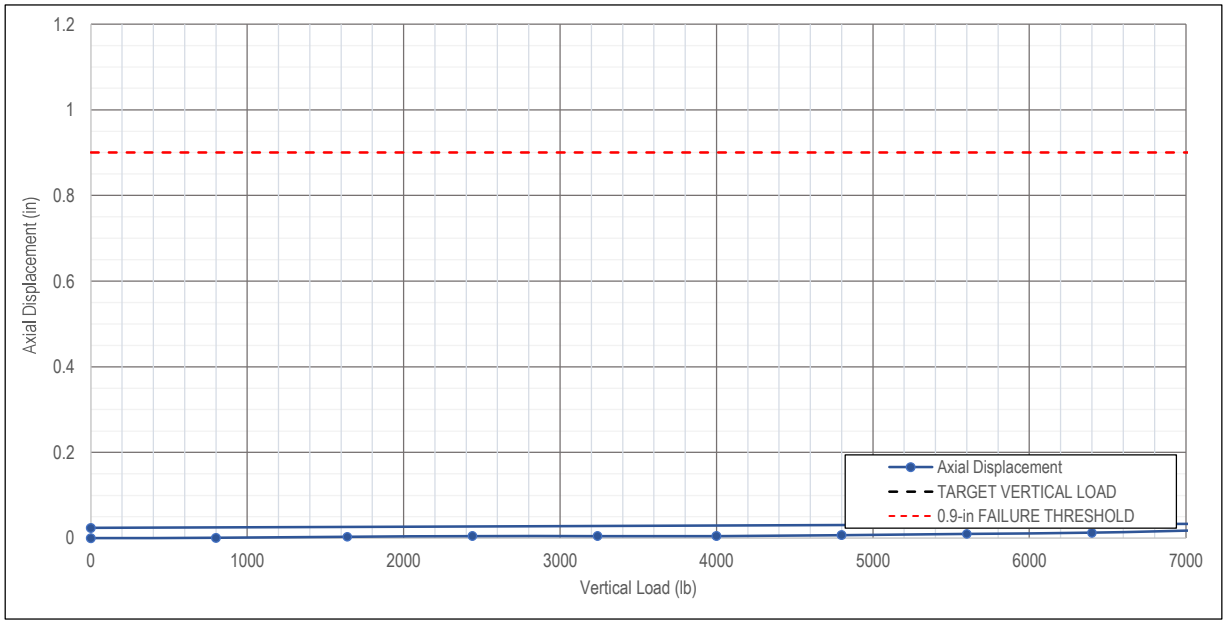
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0.002	0	0.00
436%	20%	1,600	1640	0.002	0.004	0.00
654%	30%	2,400	2440	0.004	0.006	0.01
872%	40%	3,200	3240	0.004	0.006	0.01
1090%	50%	4,000	4000	0.004	0.006	0.01
1308%	60%	4,800	4800	0.006	0.008	0.01
1526%	70%	5,600	5600	0.008	0.012	0.01
1744%	80%	6,400	6400	0.012	0.014	0.01
1962%	90%	7,200	7230	0.018	0.022	0.02
2180%	100%	8,000	8030	0.032	0.036	0.03
0%	0%	0	0	0.024	0.024	0.02

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.02** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-76B

Lateral Load Test Results

PTL- **76** Designation: **B**

Section: W6x9

Latitude: 33.637262

Longitude: -114.68165

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 8.00

Actual Embedment Depth (ft): 8

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 33.67

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

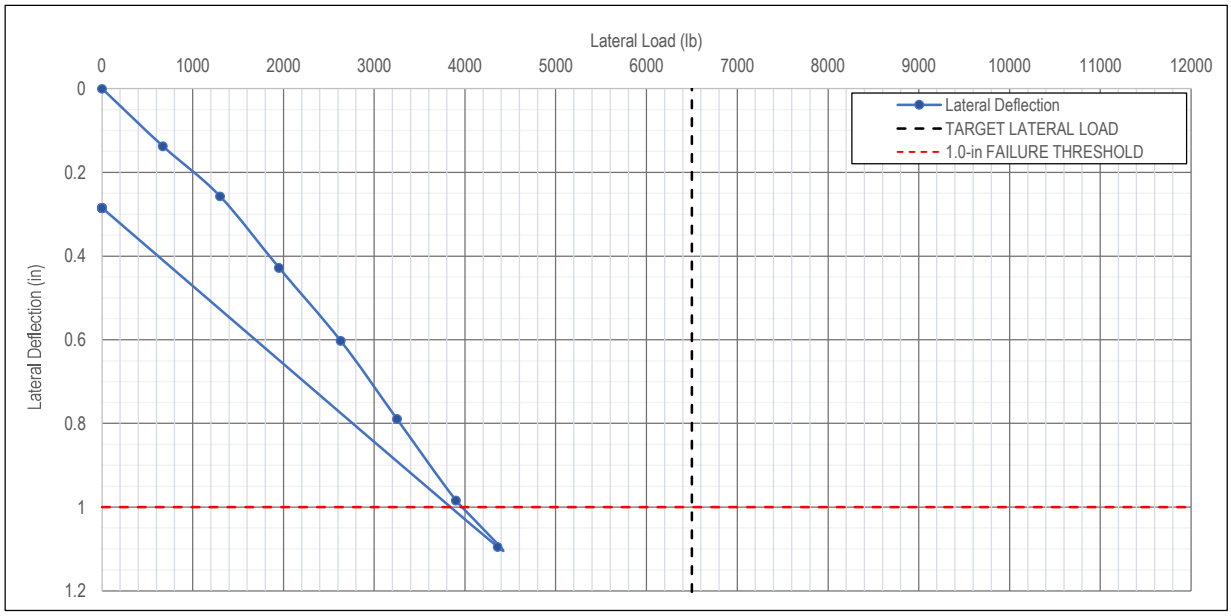
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	670	0.14	0.134	0.14
62%	20%	1,300	1300	0.262	0.252	0.26
93%	30%	1,950	1950	0.438	0.418	0.43
124%	40%	2,600	2630	0.614	0.59	0.60
155%	50%	3,250	3250	0.806	0.772	0.79
186%	60%	3,900	3900	1.006	0.962	0.98
217%	70%	4,550	4360	1.12	1.07	1.10
248%	80%	5,200				
279%	90%	5,850				
310%	100%	6,500				
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.288	0.282	0.29

Load at 1.0-in lateral deflection: **3,966** **lb**

Residual lateral deformation: **0.29** **in**



Notes:

Failure reached at 4,360 lbs.

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-76D

Axial Uplift Load Test Results

PTL - **76** Designation: **D**

Section: **W6x12**

Latitude: 33.637262

Longitude: -114.68165

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **8,000** (≈ 2180% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Saturated Soil? No

Drive Time (s): 34.43

Pile Damaged? No

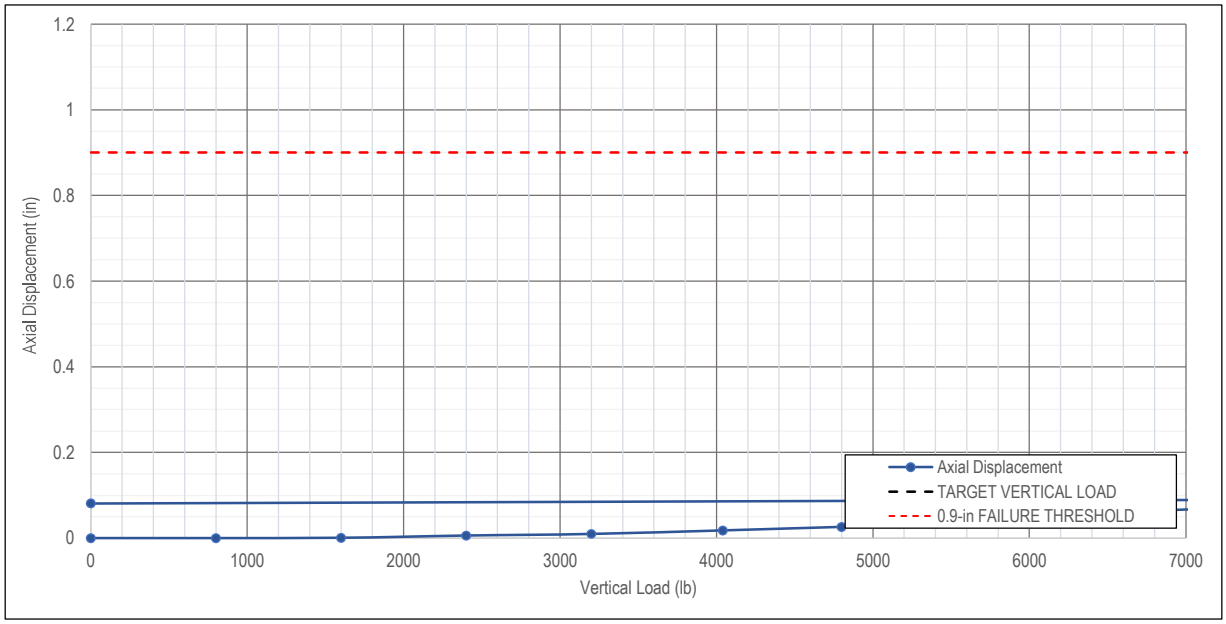
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **AXIAL UPLIFT** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DISPLACEMENT GAUGE 1 (in)	DISPLACEMENT GAUGE 2 (in)	AVERAGE DISPLACEMENT (in)
0%	0%	0	0	0	0	0.00
218%	10%	800	800	0	0	0.00
436%	20%	1,600	1600	0.002	0	0.00
654%	30%	2,400	2400	0.008	0.004	0.01
872%	40%	3,200	3200	0.012	0.008	0.01
1090%	50%	4,000	4040	0.02	0.016	0.02
1308%	60%	4,800	4800	0.028	0.024	0.03
1526%	70%	5,600	6530	0.042	0.038	0.04
1744%	80%	6,400	6420	0.06	0.056	0.06
1962%	90%	7,200	7200	0.072	0.068	0.07
2180%	100%	8,000	8030	0.09	0.088	0.09
0%	0%	0	0	0.08	0.082	0.08

Load at 0.9-in axial displacement: **N/A** **lb**

Residual axial displacement: **0.08** **in**



Notes:

Cupertino - Grace

Blythe, CA [Lat: 33.647270° ; Long: -114.677151°]

GAMECHANGE SOLAR
REPOWERING THE PLANET

PT-76D

Lateral Load Test Results

PTL- **76** Designation: **D**

Section: **W6x12**

Latitude: 33.637262

Longitude: -114.68165

Tested By: ANS

Test Date: 10/25/2025

Target Test Load (lb): **6,500** (≈ 310% Design Load)

Installation Method: Direct Drive (DD)

- -

Target Embedment Depth (ft): 5.00

Actual Embedment Depth (ft): 5

Lateral Load Application Height (ft): 5

Saturated Soil? No

Drive Time (s): 34.43

Pile Damaged? No

Gauge 1 Height (in): 6

Gauge 2 Height (in): 6

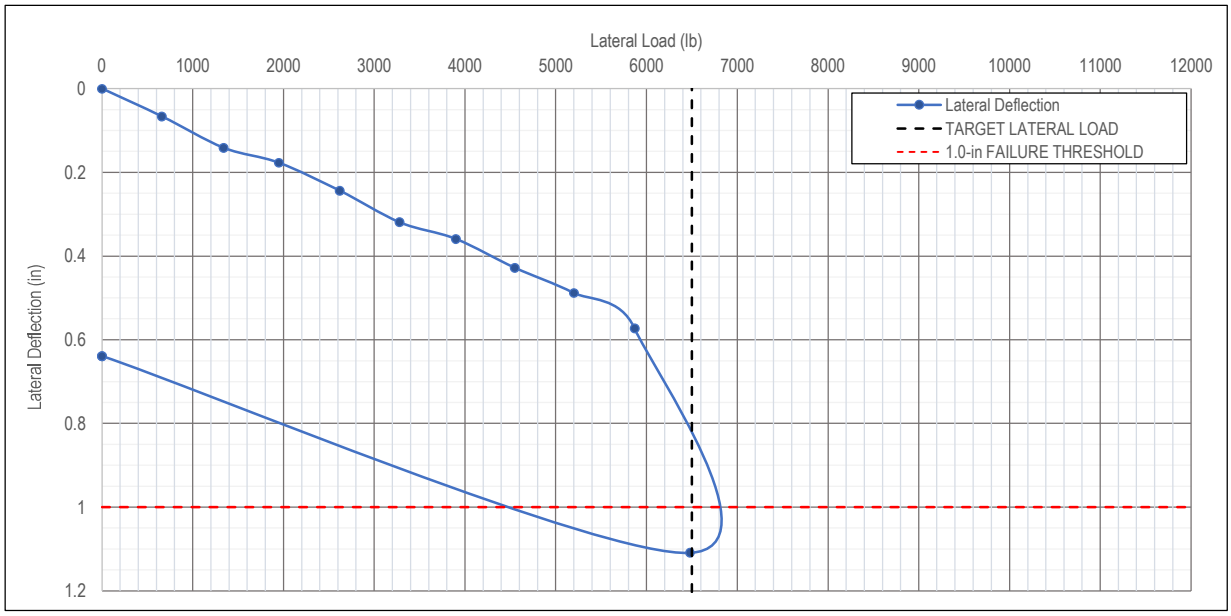
Strong Axis Plumb. (deg):

Weak Axis Plumb. (deg):

TESTING UNDER **LATERAL** LOAD						
PERCENT OF DESIGN LOAD	PERCENT OF TARGET LOAD	TARGET TEST LOAD (lb)	APPLIED LOAD (lb)	DEFLECTION GAUGE 1 (in)	DEFLECTION GAUGE 2 (in)	AVERAGE DEFLECTION (in)
0%	0%	0	0	0	0	0.00
31%	10%	650	660	0.07	0.062	0.07
62%	20%	1,300	1340	0.15	0.132	0.14
93%	30%	1,950	1950	0.188	0.166	0.18
124%	40%	2,600	2620	0.258	0.23	0.24
155%	50%	3,250	3280	0.336	0.302	0.32
186%	60%	3,900	3900	0.378	0.34	0.36
217%	70%	4,550	4550	0.45	0.406	0.43
248%	80%	5,200	5200	0.512	0.464	0.49
279%	90%	5,850	5870	0.602	0.544	0.57
310%	100%	6,500	6480	1.164	1.054	1.11
If not reached before, increase the load until **1.0-in** deflection is reached, and enter the corresponding data here ------->						
0%	0%	0	0	0.672	0.606	0.64

Load at 1.0-in lateral deflection: **6,356** **lb**

Residual lateral deformation: **0.64** **in**



Notes:

Failure reached at 6,480 lbs.

Attachment C

Photo Logs



Media 1: PT-01A Installation Set-up

Weather: Sunny

Lat/Lng: 33.6652,-114.708
Bearing: N
Date Taken: 10/15/2025

Taken By: Milo Capps
Tags:



Media 2: PT-05A Installation Set-up

Weather: Sunny

Lat/Lng: 33.6659,-114.690
Bearing: S
Date Taken: 10/13/2025

Taken By: Milo Capps
Tags:



Media 3: PT-13B Installation Set-up

Weather: Clear

Lat/Lng: 33.6630,-114.690
Bearing: N
Date Taken: 10/13/2025

Taken By: Milo Capps
Tags:



Media 4: PT-20B Installation Set-up

Weather: Sunny

Lat/Lng: 33.6584,-114.687
Bearing: S
Date Taken: 10/14/2025

Taken By: Milo Capps
Tags:

ANS

Project Name: Gamechange - Cupertino Grace

Project Location: Blythe, CA, USA

Client: Gamechange

Project Code:

Preparer: Sam Supan

Reviewer: Kristen Kakascik

Report Date: 2025-11-04

Powered By Filio



Media 5: PT-27B Installation Set-up

Weather: Sunny

Lat/Lng: 33.6553,-114.691
Bearing: E
Date Taken: 10/14/2025

Taken By: Caleb Ross
Tags:



Media 6: PT-38A Installation Set-up

Weather: Clear

Lat/Lng: 33.6511,-114.711
Bearing: NE
Date Taken: 10/15/2025

Taken By: Milo Capps
Tags:



Media 7: PT-45A Installation Set-up

Weather: Sunny

Lat/Lng: 33.6496,-114.685
Bearing: NW
Date Taken: 10/14/2025

Taken By: Milo Capps
Tags:



Media 8: PT-50A Installation Set-up

Weather: Clear

Lat/Lng: 33.6507,-114.670
Bearing: E
Date Taken: 10/16/2025

Taken By: Milo Capps
Tags:

ANS

Project Name: Gamechange - Cupertino Grace

Project Location: Blythe, CA, USA

Client: Gamechange

Project Code:

Preparer: Sam Supan

Reviewer: Kristen Kakascik

Report Date: 2025-11-04

Powered By Filio



Media 9: PT-69B Installation Set-up

Weather: Sunny

Lat/Lng: 33.6419,-114.715
Bearing: N
Date Taken: 10/16/2025

Taken By: Milo Capps
Tags:



Media 10: PT-72B Installation Set-up

Weather: Sunny

Lat/Lng: 33.6398,-114.680
Bearing: N
Date Taken: 10/18/2025

Taken By: Milo Capps
Tags:



Media 11: PT-01A Uplift Set-up

Weather: Sunny

Lat/Lng: 33.6653,-114.708
Bearing: SE
Date Taken: 10/19/2025

Taken By: Austin Denison
Tags: UPLIFT



Media 12: PT-14A Uplift Set-up

Weather: Clear

Lat/Lng: 33.6667,-114.695
Bearing: S
Date Taken: 10/18/2025

Taken By: Austin Denison
Tags: UPLIFT

ANS

Project Name: Gamechange - Cupertino Grace

Project Location: Blythe, CA, USA

Client: Gamechange

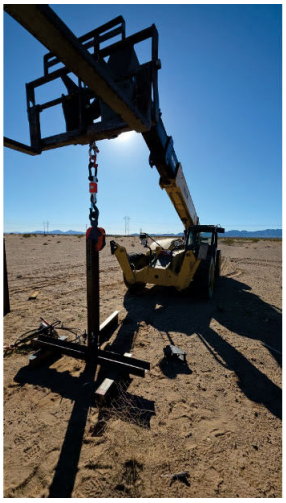
Project Code:

Preparer: Sam Supan

Reviewer: Kristen Kakascik

Report Date: 2025-11-04

Powered By Filio



Media 13: PT-21A Uplift Set-up

Weather: Clear

Lat/Lng: 33.6621,-114.682
Bearing: SW
Date Taken: 10/23/2025

Taken By: Austin Denison
Tags: LATERAL, UPLIFT



Media 14: PT-25A Uplift Set-up

Weather: Sunny

Lat/Lng: 33.6569,-114.697
Bearing: NE
Date Taken: 10/24/2025

Taken By: Toby Leyba
Tags: uplift



Media 15: PT-36A Uplift Set-up

Weather: Clear

Lat/Lng: 33.6561,-114.679
Bearing: SW
Date Taken: 10/28/2025

Taken By: Toby Leyba
Tags: uplift



Media 16: PT-38A Uplift Set-up

Weather: Clear

Lat/Lng: 33.6511,-114.711
Bearing: W
Date Taken: 10/24/2025

Taken By: Austin Denison
Tags: LATERAL, UPLIFT

ANS

Project Name: Gamechange - Cupertino Grace

Project Location: Blythe, CA, USA

Client: Gamechange

Project Code:

Preparer: Sam Supan

Reviewer: Kristen Kakascik

Report Date: 2025-11-04

Powered By Filio



Media 17: PT-44A Uplift Set-up

Weather: Clear

Lat/Lng: 33.6479,-114.689
Bearing: SW
Date Taken: 10/24/2025

Taken By: Toby Leyba
Tags: uplift



Media 18: PT-50B Uplift Set-up

Weather: Sunny

Lat/Lng: 33.6507,-114.670
Bearing: NE
Date Taken: 10/28/2025

Taken By: Toby Leyba
Tags: uplift



Media 19: PT-67A Uplift Set-up

Weather: Sunny

Lat/Lng: 33.6444,-114.664
Bearing: NW
Date Taken: 10/27/2025

Taken By: Toby Leyba
Tags: uplift



Media 20: PT-74B Uplift Set-up

Weather: Sunny

Lat/Lng: 33.6368,-114.681
Bearing: S
Date Taken: 10/26/2025

Taken By: Toby Leyba
Tags: uplift

ANS

Project Name: Gamechange - Cupertino Grace

Project Location: Blythe, CA, USA

Client: Gamechange

Project Code:

Preparer: Sam Supan

Reviewer: Kristen Kakascik

Report Date: 2025-11-04

Powered By Filio



Media 21: PT-04A Lateral Set-up

Weather: Clear

Lat/Lng: 33.6666,-114.695
Bearing: N
Date Taken: 10/18/2025

Taken By: Milo Capps
Tags:

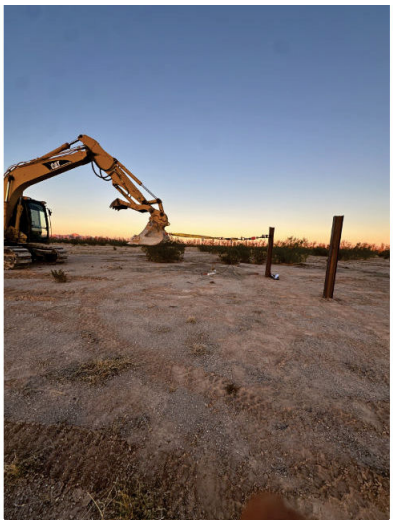


Media 22: PT-17B Lateral Set-up

Weather: Sunny

Lat/Lng: 33.6589,-114.699
Bearing: SW
Date Taken: 10/23/2025

Taken By: Toby Leyba
Tags: Lateral



Media 23: PT-23B Lateral Set-up

Weather: Clear

Lat/Lng: 33.6550,-114.704
Bearing: E
Date Taken: 10/19/2025

Taken By: Milo Capps
Tags:



Media 24: PT-26D Lateral Set-up

Weather: Clear

Lat/Lng: 33.6550,-114.694
Bearing: NW
Date Taken: 10/23/2025

Taken By: Toby Leyba
Tags: Lateral

ANS

Project Name: Gamechange - Cupertino Grace

Project Location: Blythe, CA, USA

Client: Gamechange

Project Code:

Preparer: Sam Supan

Reviewer: Kristen Kakascik

Report Date: 2025-11-04

Powered By Filio



Media 25: PT-27A Lateral Set-up

Weather: Sunny

Lat/Lng: 33.6553,-114.691
Bearing: NE
Date Taken: 10/22/2025

Taken By: Toby Leyba
Tags: Lateral



Media 26: PT-41A Lateral Set-up

Weather: Sunny

Lat/Lng: 33.6498,-114.697
Bearing: NW
Date Taken: 10/24/2025

Taken By: Toby Leyba
Tags: Lateral



Media 27: PT-59B Lateral Set-up

Weather: Sunny

Lat/Lng: 33.6490,-114.666
Bearing: N
Date Taken: 10/28/2025

Taken By: Toby Leyba
Tags: Lateral



Media 28: PT-67A Lateral Set-up

Weather: Sunny

Lat/Lng: 33.6447,-114.664
Bearing: N
Date Taken: 10/27/2025

Taken By: Toby Leyba
Tags: Lateral

ANS

Project Name: Gamechange - Cupertino Grace

Project Location: Blythe, CA, USA

Client: Gamechange

Project Code:

Preparer: Sam Supan

Reviewer: Kristen Kakascik

Report Date: 2025-11-04

Powered By Filio



Media 29: PT-72D Lateral Set-up

Weather: Clear

Lat/Lng: 33.6399,-114.680
Bearing: SE
Date Taken: 10/26/2025

Taken By: Toby Leyba
Tags: Lateral



Media 30: PT-74B Lateral Set-up

Weather: Sunny

Lat/Lng: 33.6367,-114.681
Bearing: SE
Date Taken: 10/26/2025

Taken By: Toby Leyba
Tags: Lateral

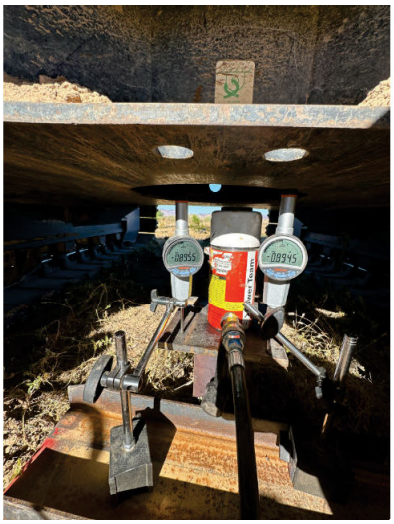


Media 31: PT-05C Compression Set-up

Weather: Sunny

Lat/Lng: 33.6659,-114.691
Bearing: NE
Date Taken: 10/19/2025

Taken By: Austin Denison
Tags: COMPRESSION



Media 32: PT-07C Compression Set-up

Weather: Clear

Lat/Lng: 33.6626,-114.708
Bearing: NE
Date Taken: 10/19/2025

Taken By: Milo Capps
Tags:

ANS

Project Name: Gamechange - Cupertino Grace

Project Location: Blythe, CA, USA

Client: Gamechange

Project Code:

Preparer: Sam Supan

Reviewer: Kristen Kakascik

Report Date: 2025-11-04

Powered By Filio



Media 33: PT-15C Compression Set-up

Weather: Partly cloudy

Lat/Lng: 33.6579,-114.707
Bearing: E
Date Taken: 10/25/2025

Taken By: Austin Denison
Tags: COMPRESSION



Media 34: PT-17C Compression Set-up

Weather: Sunny

Lat/Lng: 33.6589,-114.699
Bearing: W
Date Taken: 10/23/2025

Taken By: Toby Leyba
Tags: Compression



Media 35: PT-21C Compression Set-up

Weather: Sunny

Lat/Lng: 33.6620,-114.682
Bearing: N
Date Taken: 10/26/2025

Taken By: Austin Denison
Tags: COMPRESSION



Media 36: PT-31C Compression Set-up

Weather: Clear

Lat/Lng: 33.6553,-114.681
Bearing: NE
Date Taken: 10/26/2025

Taken By: Austin Denison
Tags: COMPRESSION

ANS

Project Name: Gamechange - Cupertino Grace

Project Location: Blythe, CA, USA

Client: Gamechange

Project Code:

Preparer: Sam Supan

Reviewer: Kristen Kakascik

Report Date: 2025-11-04

Powered By Filio



Media 37: PT-51C Compression Set-up

Weather: Clear

Lat/Lng: 33.6480,-114.714
Bearing: N
Date Taken: 10/20/2025

Taken By: Toby Leyba
Tags: compression



Media 38: PT-55C Compression Set-up

Weather: Clear

Lat/Lng: 33.6480,-114.694
Bearing: S
Date Taken: 10/23/2025

Taken By: Toby Leyba
Tags: Compression

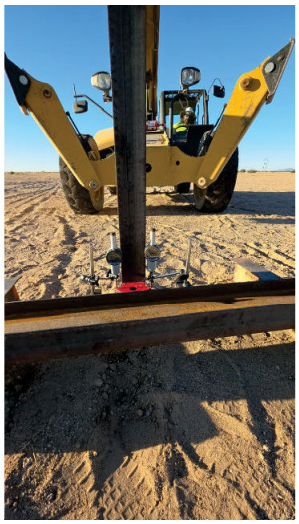


Media 39: PT-59C Compression Set-up

Weather: Sunny

Lat/Lng: 33.6490,-114.666
Bearing: NE
Date Taken: 10/28/2025

Taken By: Toby Leyba
Tags: Compression



Media 40: PT-67C Compression Set-up

Weather: Sunny

Lat/Lng: 33.6500,-114.681
Bearing: S
Date Taken: 10/27/2025

Taken By: Austin Denison
Tags: COMPRESSION

ANS

Project Name: Gamechange - Cupertino Grace

Project Location: Blythe, CA, USA

Client: Gamechange

Project Code:

Preparer: Sam Supan

Reviewer: Kristen Kakascik

Report Date: 2025-11-04

Powered By Filio

Attachment D

Calibration Sheets

JACKING SOLUTIONS LLC

Rentals • Rebuilt Equipment • New Sales • Testing

Certificate of Calibration Report

Certificate Number 359830242

Information

Customer:	ANS Geo		Calibrated Equipment	
Contact:		RAM	Manufacturer:	Power Team
			Model:	C256C
Address:	6128 E. Carolina Dr.		S/N	ANS2502
	Scottsdale, AZ. 85254	PUMP	Manufacturer:	n/a
			Model:	
Phone:			S/N	
Fax:		GAUGE	Manufacturer:	Wika
			Model:	213.53
			S/N	607

Calibration Statistics

Date:	7/24/2025
Time:	3:15 pm
Temperature:	78 degrees
Humidity:	49 %

Results

Pass?:	Yes
Seals OK?:	Yes
Action Taken:	n/a

Standards used for Certification

Manufacturer:	National Scale	Date of Calibration	2/06/2025
Model:	1100083-660	Next Calibration Due	2/06/2026
S/N	39451		
Description:	compression load cell		

JACKING SOLUTIONS LLC

Rentals · Rebuilt Equipment · New Sales · Testing

7/24/2025

Calibration Results

CYLINDER
Manufacturer: **Power Team**
Model: **C256C**
S/N: **ANS2502**

Effective Area
5.15

Certificate No: 359830242

Date: 7/24/2025

PUMP
Manufacturer: **n/a**
Model:
S/N:

GAUGE
Manufacturer: **Wika**
Model: **213.53**
S/N: **607**

Test Data

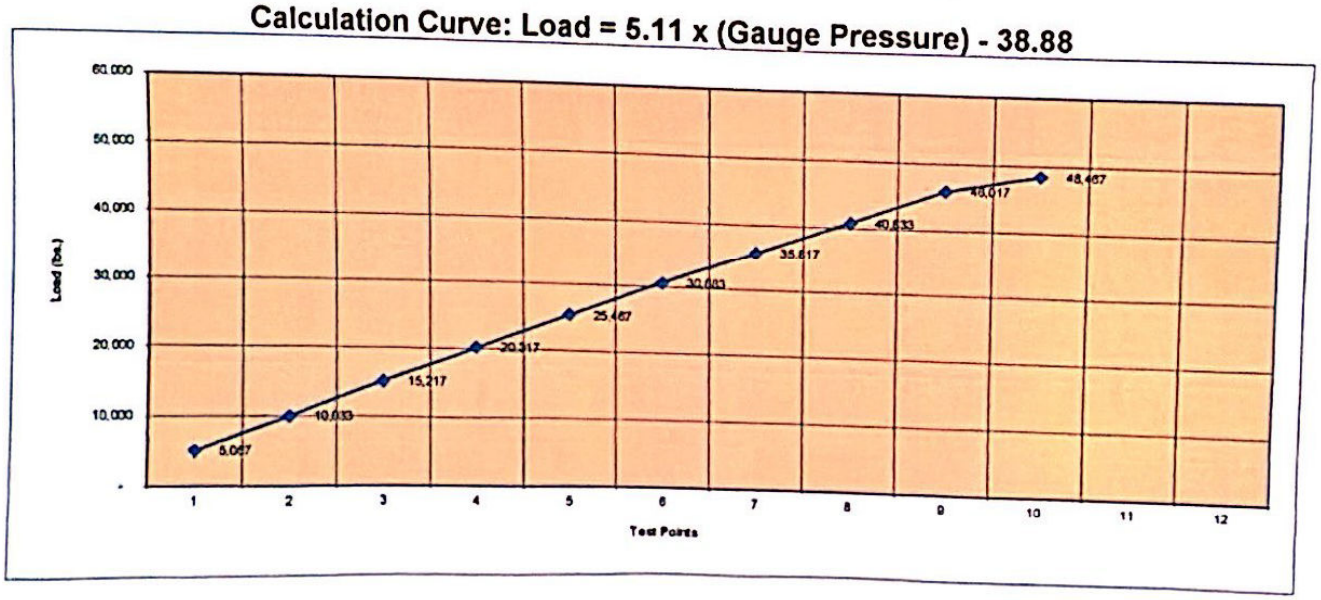
Gauge Pressure	Verified Actual Average Load	Calculated Load	Percent of Error
1000	5,067	5,150	-1.64%
2000	10,033	10,300	-2.66%
3000	15,217	15,450	-1.53%
4000	20,317	20,600	-1.39%
5000	25,467	25,750	-1.11%
6000	30,683	30,900	-0.71%
7000	35,817	36,050	-0.65%
8000	40,833	41,200	-0.90%
9000	46,017	46,350	-0.72%
9500	48,467	48,925	-0.94%

Jacking Solutions LLC hereby certifies that the above described instrument(s) met or exceeded all published specifications at the time of calibration specified on attached Calibration Report. The instrument(s) has been calibrated using standards whose accuracies are traceable to the national Institute of Standards and Technology (NIST) within the limitations of the Institute's calibration services.

Tested by: David Lilyblade

David Lilyblade
Jacking Solutions LLC

Calculation Curve: Load = 5.11 x (Gauge Pressure) - 38.88



19210 Auburn-Black Dia. Rd • Auburn, WA. 98092 • 206.595.2847 • Fax 253.737.4192

Certification #: 359830242 7/24/2025

Mfg.:	Power Team

Model:	C256C

Variance #:	-38.88
Slope Calc.:	5.110

PSI Reading	Force (lbs.)
100	472.12
200	983.12
300	1,494.12
400	2,005.12
500	2,516.12
600	3,027.12
700	3,538.12
800	4,049.12
900	4,560.12
1000	5,071.12
1100	5,582.12
1200	6,093.12
1300	6,604.12
1400	7,115.12
1500	7,626.12
1600	8,137.12
1700	8,648.12
1800	9,159.12
1900	9,670.12
2000	10,181.12
2100	10,692.12
2200	11,203.12
2300	11,714.12
2400	12,225.12
2500	12,736.12
2600	13,247.12
2700	13,758.12
2800	14,269.12
2900	14,780.12
3000	15,291.12
3100	15,802.12
3200	16,313.12
3300	16,824.12
3400	17,335.12
3500	17,846.12
3600	18,357.12
3700	18,868.12
3800	19,379.12
3900	19,890.12
4000	20,401.12
4100	20,912.12
4200	21,423.12
4300	21,934.12
4400	22,445.12
4500	22,956.12
4600	23,467.12
4700	23,978.12
4800	24,489.12
4900	25,000.12
5000	25,511.12

PSI Reading	Force (lbs.)
5050	25,766.62
5100	26,022.12
5150	26,277.62
5200	26,533.12
5250	26,788.62
5300	27,044.12
5350	27,299.62
5400	27,555.12
5450	27,810.62
5500	28,066.12
5550	28,321.62
5600	28,577.12
5650	28,832.62
5700	29,088.12
5750	29,343.62
5800	29,599.12
5850	29,854.62
5900	30,110.12
5950	30,365.62
6000	30,621.12
6050	30,876.62
6100	31,132.12
6150	31,387.62
6200	31,643.12
6250	31,898.62
6300	32,154.12
6350	32,409.62
6400	32,665.12
6450	32,920.62
6500	33,176.12
6550	33,431.62
6600	33,687.12
6650	33,942.62
6700	34,198.12
6750	34,453.62
6800	34,709.12
6850	34,964.62
6900	35,220.12
6950	35,475.62
7000	35,731.12
7050	35,986.62
7100	36,242.12
7150	36,497.62
7200	36,753.12
7250	37,008.62
7300	37,264.12
7350	37,519.62
7400	37,775.12
7450	38,030.62
7500	38,286.12

PSI Reading	Force (lbs.)
7550	38,541.62
7600	38,797.12
7650	39,052.62
7700	39,308.12
7750	39,563.62
7800	39,819.12
7850	40,074.62
7900	40,330.12
7950	40,585.62
8000	40,841.12
8050	41,096.62
8100	41,352.12
8150	41,607.62
8200	41,863.12
8250	42,118.62
8300	42,374.12
8350	42,629.62
8400	42,885.12
8450	43,140.62
8500	43,396.12
8550	43,651.62
8600	43,907.12
8650	44,162.62
8700	44,418.12
8750	44,673.62
8800	44,929.12
8850	45,184.62
8900	45,440.12
8950	45,695.62
9000	45,951.12
9050	46,206.62
9100	46,462.12
9150	46,717.62
9200	46,973.12
9250	47,228.62
9300	47,484.12
9350	47,739.62
9400	47,995.12
9450	48,250.62
9500	48,506.12
9550	48,761.62
9600	49,017.12
9650	49,272.62
9700	49,528.12
9750	49,783.62
9800	50,039.12
9850	50,294.62
9900	50,550.12
9950	50,805.62
10000	51,061.12

Appendix B2

Grace Solar Transmission Line Geotechnical Engineering Report

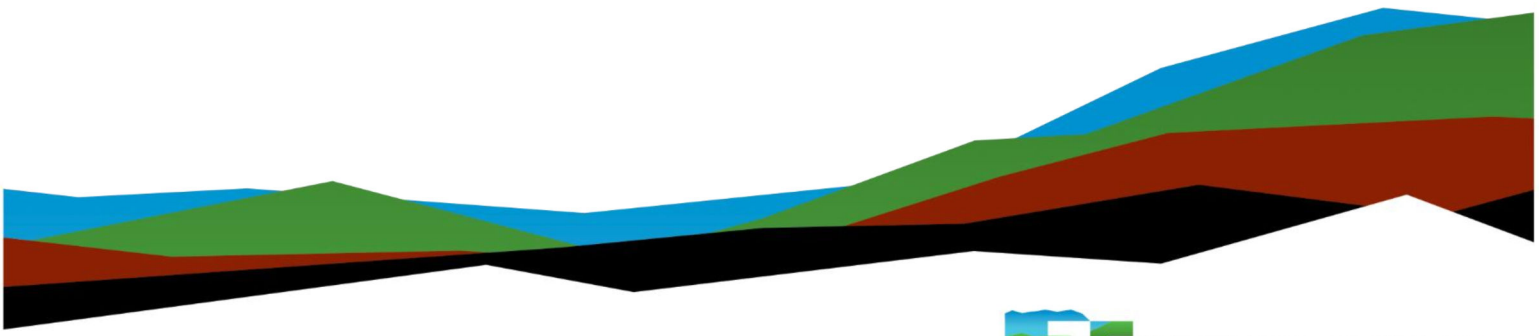
Grace Solar Transmission Line

Geotechnical Engineering Report

May 1, 2025 | Terracon Project No. 60225164

Prepared for:

NextEra Energy Resources, LLC
700 Universe Boulevard
Juno Beach, Florida 33408



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Laguna Hills, CA 92653
P (949) 261-0051
Terracon.com

May 1, 2025

NextEra Energy Resources, LLC
700 Universe Boulevard
Juno Beach, Florida 33408

Attn: Ms. Yesenia Diaz
P: (772) 284-3446
E: Yesenia.Diaz@nexteraenergy.com

Re: Geotechnical Engineering Report
Grace Solar Transmission Line
Blythe, Riverside County, CA
Terracon Project No. 60225164

Dear Ms. Diaz:

We have completed the scope of Geotechnical Engineering services for the above referenced project in general accordance with Project Change Request (PCR) 002 to Purchase Order Number 200489416. This report presents the findings of Terracon's subsurface exploration and provides geotechnical recommendations concerning earthwork and the design and construction of the proposed transmission line and access roads.

We appreciate the opportunity to be of service to you on this project. If you have any questions concerning this report or if we may be of further service, please contact us.

Sincerely,

Terracon

Toni Aguilar
Field Engineer



Victor V. Nguyen, P.E.
Project Engineer

John B. Mancini, P.E. (UT)
Senior National Accounts Manager

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Exploration and Testing Procedures
Site Location and Exploration Plans
Exploration, Laboratory, and Field Electrical Resistivity Results
Supporting Information

Note: This report was originally delivered in a web-based format. **Blue Bold** text in the report indicates a referenced section heading. The PDF version also includes hyperlinks which direct the reader to that section and clicking on the logo will bring you back to this page. For more interactive features, please view your project online at **client.terracon.com**.

Refer to each individual Attachment for a listing of contents.

Introduction

This report presents the results of our subsurface exploration and Geotechnical Engineering services performed for the proposed transmission line located near Blythe, Riverside County, CA. The purpose of these services was to provide information and geotechnical engineering recommendations relative to:

- Subsurface soil conditions
- Groundwater conditions
- Seismic site classification per CBC
- Site preparation and earthwork
- Trenching considerations
- Foundation design and construction
- Gravel surface road design and construction

The geotechnical engineering Scope of Services for this project included the advancement of test borings, laboratory testing, engineering analysis, and preparation of this report. The geotechnical engineering Scope of Services for our current scope of work included the following:

- Four (4) soil test boring to an approximate depth of 51.5 feet below ground surface (bgs)
- One (1) field electrical resistivity test
- Corrosion testing on soil samples obtained from four (4) locations
- California bearing ratio test on soil samples obtained from one (1) location
- Lab thermal resistivity testing on soil samples obtained from one (1) location

Drawings showing the site and boring locations are shown on the **Site Location** and **Exploration Location Plan**, respectively. The results of the laboratory testing performed on soil samples obtained from the site during our field exploration are included on the boring logs and/or as separate graphs in the **Exploration and Laboratory Results** section.

Project Description

Our initial understanding of the project was provided in our proposal and was discussed during project planning. A period of collaboration has transpired since the project was initiated, and our final understanding of the project conditions is as follows:

Item	Description
Information Provided	The following documents were provided by NextEra: ■ Excel files titled *C4.003332-GEO01* and *Grace Soil Boring Locations* which provided the planned boring locations, boring depths, and requested laboratory testing at each boring. Borings were advanced at the approximate locations provided in the excel. ■ KMZ titled *Attachment 2 – Overhead Gen-Tie Bore Locations* which provided the planned boring locations ■ PDF titled *Specification for Grace Solar, LLC, Geotechnical Engineering Services for he Grace Solar Overhead Transmission Line Scope, Revision A*, prepared by Electrical Consultants, Inc. dated October 2024.
Project Description	The project involves the installation of two 230 kV transmission lines, extending between the following start and end coordinates. Transmission Line 1 (Near Boring TR-UG1, approximately 2,230 linear feet). Portions are anticipated to be underground and portions are above ground. The exact limits of the underground and above ground segments were not provided to Terracon at the time of preparation of this report. ■ Starting Coordinates: 33.5822°N, 114.8174°W. ■ End Coordinates: 33.5896°N, 114.8160°W. Transmission Line 2 (Near Borings B-1, MV2, and MV7, approximately 10,580 linear feet). Anticipated to be entirely above ground. ■ Starting Coordinates: 33.6496°N, 114.7206°W. ■ End Coordinates: 33.6501°N, 114.7533°W. In addition, an aggregate surface access road is planned along the Transmission Line 2 location.
Proposed Construction	The proposed construction consists of overhead transmission line structures consisting of deadend, 3-pole dead end, tangent, and riser poles.

Item	Description
Maximum Loads	According to project specifications document, maximum loads for the structures are as follows: Deadend: ■ Moment: 18,000 kip-ft ■ Shear: 200 kips ■ Axial: 200 kips Guyed Deadend: ■ Moment: 600 kip-ft ■ Shear: 10 kips ■ Axial: 275 kips Tangent: ■ Moment: 2,000 kip-ft ■ Shear: 25 kips ■ Axial: 60 kips Typical loads were not provided for the riser structure. The loads are estimated to be within the range of loads provided for guyed dead end and tangent poles.
Grading/Slopes	Grading is not anticipated to occur at the locations of the transmission lines. Grading within access roads are anticipated to be on the order of 1 to 2 feet.
Access Roads	We anticipate low-volume, aggregate-surfaced access roads are planned along the Transmission Line 2 alignment. Traffic loading was not provided to Terracon; however we anticipate access roads will have a maximum vehicle load of 30,000 lbs. and will travel over the access roads only once per week for a period of 35 years. As such, an ESAL of 2,000 is estimated for the design of the access roads. Furthermore, we understand that the fire access roads shall be able to support a maximum firetruck vehicle load of 80,000 lbs.

Terracon should be notified if any of the above information is inconsistent with the planned construction, especially the grading limits, as modifications to our recommendations may be necessary.

Site Conditions

The following description of site conditions is derived from our site visit in association with the field exploration and our review of publicly available geologic and topographic maps.

Item	Description
Parcel Information	The project transmission lines are located South (Transmission Line 1) and North (Transmission Line 10) of the I-10 Freeway, in Blythe, Riverside County, CA. The transmission lines extend between the following start and end coordinates: Transmission Line 1 (Near Boring TR-UG1) ■ Starting Coordinates: 33.5822°N, 114.8174°W. ■ End Coordinates: 33.5896°N, 114.8160°W. Transmission Line 2 (Near Borings B-1, MV2, and MV7) ■ Starting Coordinates: 33.6496°N, 114.7206°W. ■ End Coordinates: 33.6501°N, 114.7533°W.
Existing Improvements	Existing improvements nearby Transmission Line 1 include unpaved access roads and the Colorado River Substation to the south. Existing improvements near Transmission Line 2 include solar array fields to the north.
Current Ground Cover	Ground cover consists of exposed soil with light to moderate vegetation
Existing Topography	Elevations along the proposed transmission lines are as follows: Transmission Line 1(Near Boring TR-UG1): ■ Relatively flat with approximate elevations ranging from 457 to 470 Transmission Line 2 (Near Borings B-1, MV2, and MV7) ■ Relatively flat with approximate elevations ranging from 408 to 470 feet.

Geotechnical Characterization

Subsurface Conditions

We have developed a general characterization of the subsurface conditions based upon our review of the subsurface exploration, laboratory data, geologic setting and our understanding of the project. Subsurface soils at the four borings drilled for the transmission line consist of silty sands and poorly graded sands that are typically medium dense to depths of about 10 to 20 feet, becoming dense to very dense to the maximum depth explored of 51.5 feet bgs. Occasional layers of hard fat clay were also encountered. Cementation was observed within on-site soils from depths ranging from near the surface to a maximum depth of 46½ feet. Cemented materials ranged from medium dense to very dense sandy materials and hard fat clays. Conditions observed at each exploration point

are indicated on the individual logs which can be found in the **Exploration and Laboratory Results** section of this report.

The geotechnical characterization forms the basis of our geotechnical calculations and evaluation of site preparation and trench design. As noted in **General Comments**, the characterization is based upon widely spaced exploration points across the site, and variations are likely.

Lab Results

Laboratory tests were conducted on selected soil samples and the test results are presented in the **Exploration and Laboratory Results** section and on the boring logs. Atterberg limits testing indicates that on-site soils are generally non-plastic. Direct shear test performed on a sample taken at a depth of 7½ to 10 feet feet bgs indicate that the surface soils have a peak cohesion values ranging from 10 to 140 psf and effective friction angles of 30 to 32.

Groundwater

The borings were advanced using a hollow-stem-auger technique that allow short term groundwater observations to be made while drilling. Groundwater was not observed in the borings while drilling to a maximum depth of 51.5 feet (bgs). These observations represent groundwater conditions at the time of the field exploration and may not be indicative of other times, or at other locations. Groundwater conditions may be different at the time of construction.

Groundwater level fluctuations occur due to seasonal variations in the amount of rainfall, runoff and other factors not evident at the time the borings were performed. Therefore, groundwater levels during construction or at other times in the life of the structure may be higher or lower than the levels indicated on the boring logs. The possibility of groundwater level fluctuations should be considered when developing the design and construction plans for the project.

According to data collected from the California Department of Water Resources Water Data Library from nearby wells located approximately 1 mile north of Transmission Line 1 (Well ID 07S21E05C002S) and 1 mile south of Transmission Line 2 (Well ID 06S21E24K001S),

, historic groundwater levels at both transmission lines are recorded as deeper than 100 feet bgs.[1]

Seismic Site Class

The 2022 California Building Code (CBC) Seismic Design Parameters have been generated using the ASCE Seismic Hazard Tool. This web-based software application calculates seismic design parameters in accordance with ASCE 7-16, and 2022 CBC. The 2022 CBC requires that a site-specific ground motion study be performed in accordance with Section 11.4.8 of ASCE 7-16 for Site Class D sites with a mapped Ss value greater than or equal 0.2.

An exception to this requirement is included in Supplement 3 to ASCE 7-16 that states: "A ground motion hazard analysis is not required where the value of the parameter S_{M1} determined by Eq. (11.4-2) is increased by 50% for all applications of S_{M1} in this standard. The resulting value of the parameter S_{D1} determined by Eq. (11.4-4) shall be used for all applications of S_{D1} in the Standard." **The structural engineer should verify the applicability of this exception and apply the modifications to the applied values of S_{M1} and S_{D1} as per Supplement No. *3,* if warranted.**

Based on this exception, the spectral response accelerations presented below were determined using the site coefficients (F_a and F_v) from Tables 1613.2.3(1) and 1613.2.3(2) presented in Section 16.4.4 of the 2022 CBC.

Description	Value	
	Transmission Line 1	Transmission Line 2
2022 California Building Code Site Classification (CBC)[1]	D[2]	D[2]
Site Latitude (°N)	33.5874	33.6496
Site Longitude (°W)	114.8180	114.7321
S_s Spectral Acceleration for a 0.2-Second Period	0.373	0.329
S_1 Spectral Acceleration for a 1-Second Period	0.18	0.166
F_a Site Coefficient for a 0.2-Second Period	1.50	1.53

[1] https://wdl.water.ca.gov/2.24

Description	Value	
	Transmission Line 1	Transmission Line 2
F_v Site Coefficient for a 1-Second Period	2.24	2.26

1. Seismic site classification in general accordance with the *2022 California Building Code*.
2. The 2022 California Building Code (CBC) requires a site soil profile determination extending to a depth of 100 feet for seismic site classification. The current scope does not include the 100-foot soil profile determination. Borings were extended to a maximum depth of 51½ feet, and this seismic site class definition considers that similar or denser soils continue below the maximum depth of the subsurface exploration. Additional exploration to deeper depths would be required to confirm the conditions below the current depth of exploration.

Typically, a site-specific ground motion study may generate less conservative coefficients and acceleration values which may reduce construction costs. We recommend consulting with a structural engineer to evaluate the need for such study and its potential impact on construction costs. Terracon should be contacted if a site-specific ground motion study is desired

Faulting and Estimated Ground Motions

The site is located in southern California, which is a seismically active area. The type and magnitude of seismic hazards affecting the site are dependent on the distance to causative faults, the intensity, and the magnitude of the seismic event. Nearby faults and seismicity were determined utilizing the USGS Unified Hazard Tool and is summarized in the following table:

Description	Value	
	Transmission Line 1	Transmission Line 2
Nearby Faults	San Andreas (Coachella) and Brawley Seismic Zone	San Andreas (Coachella)
Distance to Fault (km)	86.7 (Brawley) and 89.6 (San Andreas)	98.6
Site Mean Magnitude	6.79	6.83
Site Modified Peak Ground Acceleration, PGA_M	0.243	0.221

Furthermore, the site is not located within an Alquist-Priolo Earthquake Fault Zone based on our review of the State Fault Hazard Maps.[2]

Liquefaction

Liquefaction is a mode of ground failure that results from the generation of high pore water pressures during earthquake ground shaking, causing loss of shear strength. Liquefaction is typically a hazard where loose sandy soils or low plasticity fine grained soils exist below groundwater. The California Geologic Survey (CGS) has designated certain areas within California as potential liquefaction hazard zones. These are areas considered at a risk of liquefaction-related ground failure during a seismic event, based upon mapped surficial deposits and the presence of a relatively shallow water table.

The project site has not yet been mapped by CGS for liquefaction hazards. Based on the review of the Riverside GIS Mapping Portal, both transmission line sites are located within a "Moderate" severity liquefaction hazard zone designated by the County of Riverside. However, based on the anticipated depth to the historic groundwater, the liquefaction hazard potential at the site is considered to be low. Other geologic hazards related to liquefaction, such as lateral spreading, are therefore also considered low.

Corrosivity

The table below lists the results of laboratory pH, soluble sulfate, sulfides, soluble chloride, total salts, oxidation-reduction potential (redox), and electrical resistivity testing. The values may be used to estimate potential corrosive characteristics of the on-site soils with respect to contact with the various underground materials which will be used for project construction. The individual laboratory corrosion testing results are provided in the **Exploration and Laboratory Results** section of this report and are summarized below.

Corrosivity Test Results Summary

Parameter	Number of Tests	Min.	Max.	Average
pH	4	7.47	9.14	8.56

[2] California Geological Survey. https://maps.conservation.ca.gov/cgs/informationwarehouse.

Corrosivity Test Results Summary

Parameter	Number of Tests	Min.	Max.	Average
Soluble Sulfate (% by weight)	4	<0.01	0.05	0.02
Sulfides (mg/kg)	4	Nil	0.57	0.14
Soluble Chloride (mg/kg)	4	27	211.5	85.1
Total Salts (mg/kg)	4	182	460	325
Redox (mV)	4	+169	+735	591.5
Saturated Minimum Electrical Resistivity (Ω-cm)	4	540	2,144	1,559

Results of soluble sulfate testing can be classified in accordance with ACI 318 – Building Code Requirements for Structural Concrete. Based on review of ACI 318, soluble sulfate testing indicates samples of the on-site soils tested have sulfate concentrations ranging from exposure class S0 when classified in accordance with Table 19.3.1.1 of the ACI Design Manual. Concrete should be designed in accordance with the exposure class provisions of the ACI Design Manual, Section 318, Chapter 19.

Numerous sources are available to characterize corrosion potential to buried metals using the parameters above. Section 10.7.5 of the AASHTO LRFD Bridge Manual, 9th Edition, 2020, states the following soil or site conditions should be considered as indicative of potential deterioration or corrosion situation for steel piles:

- Soil electrical resistivity less than 2,000 ohm-cm
- Ph value less than 5.5
- Ph value between 5.5 and 8.5 with high organic content
- Sulfate concentration greater than 1,000 ppm (mg/kg)

These test results are provided to assist in determining the type and degree of corrosion protection that may be required. We recommend that a NACE certified corrosion professional be retained to analyze the need for corrosion protection and to design appropriate protective measures, if required.

Imported fill materials may have significantly different properties than the site materials noted above and should be evaluated if expected to be in contact with metals used for construction.

Electrical Resistivity Testing

Terracon performed field measurements of soil electrical resistivity for the support of grounding design. Soil resistivity data was obtained from two perpendicular arrays at one (1) locations in the proposed gen-tie line. The approximate location of the test is shown in the **Exploration Plan**. The testing was performed in general accordance with Wenner Array (4-pin) method per ASTM G57. This method was performed in with IEEE Standard 81, IEEE Guide for Measuring Earth Resistivity, Ground Impedance and Earth Surface Potentials of a Ground System. The test in the proposed location included perpendicular arrays with "a" spacings 2, 5, 10, 20, and 40 feet. The "a" spacing is generally considered to be the depth of influence of the test. The electrical resistivity test results are presented in **Exploration and Laboratory Results**.

Thermal Resistivity Testing

Thermal resistivity tests were analyzed by Geotherm USA on bulk samples taken at approximate depths of 1 to 4 and 4 to 6 feet bgs. Testing on bulk samples from a depth of 1 to 4 feet targeted 85 and 95 percent relative compaction as determined by the Modified Proctor (ASTM D1557). Testing on bulk samples from a depth of 4 to 6 feet targeted 90 percent relative compaction as determined by the Modified Proctor (ASTM D1557). All tests targeted the optimum moisture content, totally dry conditions, and two intermediate moisture percentages. The individual laboratory thermal resistivity dry-out curves are provided in the **Exploration and Laboratory Results** section of this report

Geotechnical Overview

The site appears suitable for the proposed construction based upon geotechnical conditions encountered in the test borings, provided that the recommendations and considerations provided in this report are implemented in the design and construction phases of this project.

We anticipate that the proposed transmission structures will be supported by drilled concrete pier foundations.

Geotechnical engineering recommendations for trenching and other earth-connected phases of the project are outlined below. The recommendations contained in this report are based upon the results of field and laboratory testing (presented in the **Exploration Results**), engineering analyses, and our current understanding of the proposed project. The **General Comments** section provides an understanding of the report limitations.

Earthwork

The following presents recommendations for site preparation, excavation, subgrade preparation, and placement of engineered fills on the project. The recommendations presented for the design and construction of trenches are contingent upon following the recommendations outlined in this section.

Earthwork on the project should be observed and evaluated by Terracon. The evaluation of earthwork should include observation and testing of engineered fill, subgrade preparation, and other geotechnical conditions exposed during construction of the project.

Site Preparation

Exposed surfaces should be free of mounds and depressions which could prevent uniform compaction.

Stripped materials consisting of vegetation and organic materials should be wasted from the site or used to revegetate non-structural areas or exposed slopes after completion of grading operations. If it is necessary to dispose of organic materials on site, they should be placed in non-structural areas, and in fill sections not exceeding 5 feet in height.

Although no evidence of fills, utilities, or underground facilities such as septic tanks, cesspools, basements, and utilities was observed during the site reconnaissance, such features could be encountered during construction. If unexpected fills, utilities, or underground facilities are encountered, such features should be removed, and the excavation thoroughly cleaned prior to utility placement and/or backfill placement and/or construction of proposed towers and underground transmission lines.

If unexpected fills, utilities, or underground facilities are encountered, such features should be removed, and the excavation thoroughly cleaned prior to utility placement and/or backfill placement.

Subgrade Preparation

Subgrade soils beneath roadways should be scarified to a minimum depth of 12 inches, moisture conditioned, and compacted. The moisture content and compaction of subgrade soils should be maintained until pavement construction.

Structures supported on either drilled shafts or driven piles may be constructed without the above recommended remedial grading.

Based upon the subsurface conditions determined from the geotechnical exploration, subgrade soils exposed during construction are anticipated to be relatively workable; however, the workability of the subgrade may be affected by precipitation, repetitive

construction traffic or other factors. If unworkable conditions develop, workability may be improved by scarifying and drying.

Excavation

It is anticipated that excavations for the proposed construction can be accomplished with conventional earthmoving equipment.

Terracon did not encounter auger refusal during drilling and sampling with a track-mounted GtechDrill GT-16 drill rig. However, cemented soils were encountered on site at depths ranging from near the surface to a maximum depth of 46½ feet bgs. Cemented soils ranged from medium dense to very dense sands and hard fat clays. Although auger refusal was not encountered during subsurface exploration and testing, difficulty drilling may be encountered during foundation installation. Consideration should be given to obtaining a unit price for difficult drilling and excavations in the contract documents for the project.

The bottom of excavations should be thoroughly cleaned of loose soils and disturbed materials prior to backfill placement and/or construction.

Onsite soils consist of cohesionless sandy soils. Therefore, OSHA Soil Type C should be utilized for sloping considerations and on-site soils shall be sloped at a minimum of 1.5H:1V. Furthermore, benching is not permitted.

The earthwork contractor is solely responsible for designing and constructing stable, temporary excavations as required to maintain stability of both the excavation sides and bottoms. Excavations should be sloped or shored in the interest of safety following local and federal regulations, including current OSHA excavation and trench safety standards. As a minimum, excavations should be performed in accordance with OSHA 29 CFR, Part 1926, Subpart P, "Excavations" and its appendices, and in accordance with any applicable local, and/or state regulations. However, based on the currently available information, we have assumed that excavations deeper than 4 feet will not be required.

Fill Material and Placement

All fill materials should be inorganic soils free of vegetation, debris, and fragments larger than six inches in size. Pea gravel or other open-graded materials should not be used as fill or backfill without the prior approval of the geotechnical engineer.

Clean on-site soils or approved imported materials may be used as fill material for the following:

- general site grading
- utility trench backfill

- Roadways

Imported soils for use as fill material within proposed structure areas should conform to low volume change materials as indicated in the following specifications:

Gradation	Percent Finer by Weight (ASTM C 136)
3"	100
No. 4 Sieve	50-100
No. 200 Sieve	20-50

- Liquid Limit ..... 30 (max)
- Plasticity Index ..... 10 (max)
- Maximum Expansion Index* ..... 20 (max)

*ASTM D4829

The contractor shall notify the Geotechnical Engineer of import sources sufficiently ahead of their use so that the sources can be observed and approved as to the physical characteristic of the import material. For all import material, the contractor shall also submit current verified reports from a recognized analytical laboratory indicating that the import has a "not applicable" (Class S0) potential for sulfate attack based upon current ACI criteria and is "mildly corrosive" to ferrous metal and copper. The reports shall be accompanied by a written statement from the contractor that the laboratory test results are representative of all import material that will be brought to the job.

Engineered fill should be placed and compacted in horizontal lifts, using equipment and procedures that will produce recommended moisture contents and densities throughout the lift. Fill lifts should not exceed 10 inches loose thickness.

Compaction Requirements

Recommended compaction and moisture content criteria for engineered fill materials are as follows:

Material Type and Location	Per the Modified Proctor Test (ASTM D 1557)		
	Minimum Compaction Requirement	Range of Moisture Contents for Compaction Above Optimum	
		Minimum	Maximum
On-site soils and low-volume change approved imported fill soils:			
Utility trenches*	90%	-2%	+2%

Material Type and Location	Per the Modified Proctor Test (ASTM D 1557)		
	Minimum Compaction Requirement	Range of Moisture Contents for Compaction Above Optimum	
		Minimum	Maximum
Beneath gravel access roads:	95%	-2%	+2%
Aggregate Base	95%	-2%	+2%
Miscellaneous backfill:	90%	-2%	+2%

*Upper 12 inches should be compacted to 95% within roadway areas. Low-volume change imported soils should be used in structural areas. Minimum compaction may be modified by the electrical engineer based on thermal resistivity test requirements.

Utility Trench Backfill

We anticipate that the on-site soils will provide suitable support for underground utilities and piping that may be installed. Any soft and/or unsuitable material encountered at the bottom of excavations should be removed and be replaced with an adequate bedding material.

Onsite soils consist of cohesionless sandy soils. Therefore, OSHA Soil Type C should be utilized for sloping considerations and on-site soils shall be sloped at a minimum of 1.5H:1V. Furthermore, benching is not permitted.

A non-expansive granular material with a sand equivalent greater than 30 should be used for bedding and shading of utilities, unless allowed or specified otherwise by the utility manufacturer.

On-site materials are considered suitable for backfill of utility and pipe trenches from 1 foot above the top of the pipe to the final ground surface, provided the material is free of organic matter and deleterious substances.

Trench backfill should be mechanically placed and compacted as discussed earlier in this report. Compaction of initial lifts should be accomplished with hand-operated tampers or other lightweight compactors. Where trenches are placed beneath footings, the backfill should satisfy the gradation and expansion index requirements of engineered fill discussed in this report. Flooding or jetting for placement and compaction of backfill is not recommended.

Grading and Drainage

All grades must provide effective drainage away from the proposed structures during and after construction and should be maintained throughout the life of the structure. Water

retained next to the structures results in soil movements greater than those discussed in this report.

Earthwork Construction Considerations

Upon completion of filling and grading, care should be taken to maintain the subgrade water content prior to construction. The site should also be graded to prevent ponding of surface water on the prepared subgrades or in excavations. Water collecting over or adjacent to construction areas should be removed.

Construction site safety is the sole responsibility of the contractor who controls the means, methods, and sequencing of construction operations. Under no circumstances shall the information provided herein be interpreted to mean Terracon is assuming responsibility for construction site safety, or the contractor's activities; such responsibility shall neither be implied nor inferred.

The individual contractor(s) is responsible for designing and constructing stable, temporary excavations as required to maintain stability of both the excavation sides and bottom. Excavations should be sloped or shored in the interest of safety following local, and federal regulations, including current Occupational Safety and Health Administration (OSHA) excavation and trench safety standards.

Construction Observation and Testing

The geotechnical engineer should be retained during the construction phase of the project to observe earthwork and to perform necessary tests and observations during subgrade preparation, placement and compaction of controlled compacted fills, backfilling of excavations to the completed subgrade.

The exposed subgrade and each lift of compacted fill should be tested, evaluated, and reworked, as necessary, as recommended by the Geotechnical Engineer prior to placement of additional lifts. Where not specified by local ordinance, one density and water content test should be performed for every 150 linear feet of compacted utility trench backfill. This testing frequency criteria may be adjusted during construction as specified by the geotechnical engineer of record.

In addition to the documentation of the essential parameters necessary for construction, the continuation of the Geotechnical Engineer into the construction phase of the project provides the continuity to maintain the Geotechnical Engineer’s evaluation of subsurface conditions, including assessing variations and associated design changes.

Deep Foundations

Drilled Shaft Design Recommendations

Proposed transmission line structures may be supported on drilled shafts. Total required embedment of the drilled shaft should be determined by the structural engineer based on structural loading and parameters provided in this report.

Drilled Shaft Design Parameters

Allowable skin friction and total capacity charts are attached to our **Supporting Information** section at the end of this report. The values presented for allowable side friction and end bearing include a factor of safety of 2.5 of 3.0, respectively. Due to potential for disturbance within the upper soils around the shaft, lateral and axial capacity of soils within the upper 2 feet should be neglected.

The allowable uplift capacities shall be taken as 66 percent of the side friction of the shaft in compression; however, the weight of the foundation should be added to these values to obtain the actual allowable uplift capacities for drilled shafts. Tensile reinforcement should extend to the bottom of shafts subjected to uplift loading.

Recommended geotechnical parameters for lateral load analyses of drilled shaft foundations have been developed for use in the L-PILE computer program. In addition, MFAD deformation modulus are provided for the transmission line foundations. We recommend that Terracon review the final drilled shaft design to confirm that the geotechnical parameters and design considerations provided in our report have been appropriately incorporated.

The load capacities provided herein are based on the stresses induced in the supporting soil strata. The structural capacity of the shafts/piles should be checked to assure they can safely accommodate the combined stresses induced by axial and lateral forces. Lateral deflections of shafts/piles should be evaluated using an appropriate analysis method, and will depend upon the pile's diameter, length, configuration, stiffness and "fixed head" or "free head" condition. We can provide additional analyses and estimates of lateral deflections for specific loading conditions upon request. The load-carrying capacity of shafts/piles may be increased by increasing the diameter and/or length.

Drilled Shaft Design Parameters – Transmission Line Alignment 1 (Near Boring TR-UG7)							
Depth Interval (feet)	**LPile Soil Type**	**Effective Unit Weight (pcf)**	**Cohesion (psf)**	**Friction Angle (degrees)**	**Allowable Skin Friction [1] (psf)**	**Allowable End Bearing [1] (psf)**	**Deformation Modulus (ksi)**
0 to 2[2]	Sand	110	---	---	---	---	---
2 to 7.5	Sand	110	---	31	65	2,200	1.26
7.5 to 30	Sand	115	---	32	270	12,000	1.89
30 to 48	Sand	120	---	38	650	12,000	4.05

1. The allowable skin friction and end bearing include a factor of safety of 2.5 and 3.0, respectively.
2. The upper 2-foot should be neglected due to the potential for soil disturbance.
3. Default LPILE k_h and E_{50} values are considered acceptable.

Drilled Shaft Design Parameters – Transmission Line Alignment 2 (Near Borings B-1, MV-2, MV-7)							
Depth Interval (feet)	**LPile Soil Type**	**Effective Unit Weight (pcf)**	**Cohesion (psf)**	**Friction Angle (degrees)**	**Allowable Skin Friction [1] (psf)**	**Allowable End Bearing [1] (psf)**	**Deformation Modulus (ksi)**
0 to 2[2]	Sand	110	---	---	---	---	---
2 to 7.5	Sand	110	---	35	75	4,500	2.25
7.5 to 20	Sand	115	---	32	190	7,000	1.80
20 to 48	Sand	120	---	38	600	12,000	4.50

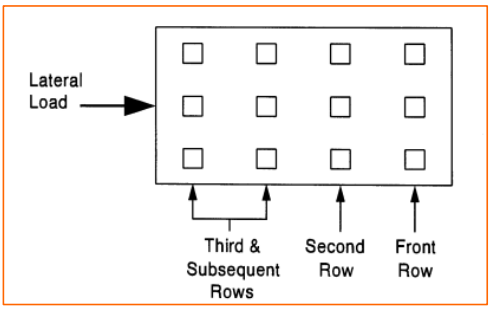
1. The allowable skin friction and end bearing include a factor of safety of 2.5 and 3.0, respectively.
2. The upper 2-foot should be neglected due to the potential for soil disturbance.
3. Default LPILE k_h and E_{50} values are considered acceptable.

Drilled piers should have a minimum (center-to-center) spacing of three diameters. Closer spacing may require a reduction in axial load capacity. Axial capacity reduction can be determined by comparing the allowable axial capacity determined from the sum of individual piers in a group versus the capacity calculated using the perimeter and base of the pier group acting as a unit. The lesser of the two capacities should be used in design.

Drilled shafts with a center-to-center spacing of less than 5D (where D is the diameter of the shaft) are considered to work in a group. Piles in closely spaced groups behave

differently than single isolated piles because of pile-soil-pile interactions that take place in the group. It is generally recognized that deflections of a pile in a closely spaced group are greater than the deflections of an individual pile at the same load because of these interaction effects. This unequal distribution of load among piles is caused by "shadowing". When piles are in a closely-spaced group with substantial interaction, the shear-failure planes resulting from the movement of each pile will overlap, and the ultimate resistance for piles in a group may be less than that of a single pile. Thus, a modification factor, with a value less than one, is needed to reduce the ultimate soil resistance, as shown in the figure below. This reduces the ultimate soil resistance and softens the shape of the p-y curve.

Guidance for applying p-multiplier factors to the p values in the p-y curves for each row of pier foundations within a pier group are as follows:



Pile Center to Center Spacing (in the direction of loading)	P-Multipliers, P_M		
	Row 1	Row 2	Row 3 and Subsequent Rows
5.0B	1.0	0.85	0.70
3.0B	0.7	0.50	0.35

Drilled Shaft Construction Considerations

Drilling for the proposed drilled shafts to design depths should be possible with conventional single flight power augers. However, due to the presence of gravels and very dense materials onsite, consideration should be given to obtaining a unit price for difficult excavation in the contract documents for the project. For drilled shaft depths above the depth of groundwater, temporary steel casing will likely be required to properly drill and clean shafts prior to concrete placement. Drilled pier bearing surfaces must be thoroughly cleaned and all loose materials removed from the bottom of the pier excavation prior to concrete placement.

If foundation concrete cannot be placed in dry conditions, a tremie should be used for concrete placement.

In the event drilled hole walls slough during drilling, temporary steel casing may be required to properly drilled shafts prior to concrete placement. We recommend the use of slurry drilling methods with polymers method to keep the solids in suspension during the drilling. Drilled shaft foundation concrete should be placed within 6 inches of the shaft base of the slurry-filled excavation immediately after completion of drilling and cleaning. The tremie should remain inserted several feet into the fresh concrete as it displaces the slurry upward and until placement is complete. The slurry should have a sand content no greater than 1% at the time concrete placement commences. The maximum unit weight of the slurry should be established in consultation with Terracon.

If temporary casing is used for drilled shaft construction, it should be withdrawn in a slow continuous manner maintaining a sufficient head of concrete to prevent infiltration of water or the creation of voids in shaft concrete. Shaft concrete should have a relatively high fluidity when placed in cased shaft holes or through a tremie. Shaft concrete with slump in the range of 6 to 8 inches is recommended, unless otherwise specified by the design engineer.

Foundation concrete should be placed immediately after completion of drilling and cleaning. All excavations should be filled with concrete as soon after drilling as possible. In no event should shaft holes be left open overnight.

Formation of mushrooms or enlargements at the tops of shafts should be avoided during shaft drilling. If mushrooms develop at the tops of the shafts during drilling, sono-tubes should be placed at the shaft tops to help isolate the shafts.

Free-fall concrete placement in drilled piers will only be acceptable if provisions are taken to avoid striking the concrete on the sides of the hole or reinforcing steel. The use of a bottom-dump hopper, or an "elephant's trunk" discharging near the bottom of the hole where concrete segregation will be minimized, is recommended.

We recommend that all drilled shaft installations be observed on a full-time basis by an experienced geotechnical engineer in order to evaluate that the soils encountered are consistent with the recommended design parameters. If the subsurface soil conditions encountered differ significantly from those presented in this report, supplemental recommendations will be required. The Geotechnical Engineer should observe the installation of drilled piers to verify the soil conditions and the diameter and depth of piers. Drilled piers should be constructed true and plumb.

Drilled pier end bearing surfaces must be thoroughly cleaned prior to concrete placement. A representative of the Geotechnical Engineer should inspect the bearing surface and foundation pier configuration. If the subsurface soil conditions encountered differ significantly from those presented in this report, supplemental recommendations will be required.

Access Road Recommendations

General Roadway Comments

We understand that a new aggregate surface roadway is planned along the Transmission Line 2 Alignment.

Roadway designs are provided for the traffic conditions and roadway life conditions as noted in the following sections of this report. A critical aspect of roadway performance is site preparation. Roadway sections noted in this section are contingent upon the site being adequately prepared.

Roadway Subgrades

Results of CBR testing indicated that on-site soils have a CBR of 13 corresponding to 95% of the maximum dry density as determined using the modified proctor method, ASTM D1557.

Design Parameters

The unpaved and paved road sections for post-construction use have been developed under the following assumptions:

Roadway Design Parameters		
Parameter	**Design Value**	**Comments**
Traffic Loading	2,000 ESALs	Assumed
Design Life	30 years	Assumed
Design CBR	13 (based on previous experience with similar geology)	Estimated
Resilient Modulus	14,400 psi (all-weather)	Based on CBR of 13
Aggregate Base Elastic Modulus	36,000 psi	Assumed
Allowable Rut Depth	2.0 inches	Assumed
Design Serviceability Loss	2.0 inches	Assumed

Roadway Design Parameters		
Parameter	**Design Value**	**Comments**
Vehicle Tire Pressure	80 psi	Assumed

1. ESAL = 18 kips Equivalent Single Axle Load

Access Road Sections

As a minimum, we recommend the following options for unpaved access roads:

Typical Unpaved Road Section – Post Construction Traffic	
Base Course Thickness (inches)	**Traffic (ESALs)**
4[1, 2]	2,000

1. Minimum section thickness is anticipated to support fire trucks and pick-up trucks associated with on-going maintenance. Trucks containing heavy equipment may require localized repairs.
2. Base materials shall consist of Class II Base meeting requirements of the Caltrans Standard Specifications.

Roadway sections should be constructed over a minimum of 12 inches of scarified, moisture conditioned, and compacted native soils to 95% of the maximum dry density using ASTM D1557. The recommended thicknesses should be measured after full compaction. To provide edge support, width of the roadway should extend a minimum distance of 1 foot on each side of the desired travel width.

Aggregate materials should conform to the specifications of Class II aggregate base in accordance with the requirements and specifications of the State of California Department of Transportation (Caltrans), or other approved local governing specifications.

Positive drainage should be provided during construction and maintained throughout the life of the roadways. Proposed roadway design should maintain the integrity of the road and eliminate ponding.

Roadway Design and Construction Considerations

Regardless of the design, un-surfaced roadways will display varying levels of wear and deterioration. We recommend an implementation of a site inspection program at a frequency

of at least once per year to verify the adequacy of the roadways. Preventative measures should be applied as needed for erosion control and re-grading. An initial site inspection should be completed approximately three months following construction.

Surfacing materials should not be placed when the surface is wet. Surface drainage should be provided away from the edge of roadways to reduce lateral moisture transmission into the subgrade.

If rut depths become excessive as construction work progresses, re-grading and re-compaction should be performed as necessary. Care should be taken to reduce or eliminate trafficking of the unpaved access road when the subgrade is wet as this will result in accelerated rutting conditions. Scarification, moisture treatment as necessary, and re-compaction of the roadways will likely be necessary as the roadways deteriorate.

Materials and construction of roadways for the project should be in accordance with the requirements and specifications of the California Department of Transportation or the applicable local governing body.

General Comments

Our analysis and opinions are based upon our understanding of the project, the geotechnical conditions in the area, and the data obtained from our site exploration. Variations will occur between exploration point locations or due to the modifying effects of construction or weather. The nature and extent of such variations may not become evident until during or after construction. Terracon should be retained as the Geotechnical Engineer, where noted in this report, to provide observation and testing services during pertinent construction phases. If variations appear, we can provide further evaluation and supplemental recommendations. If variations are noted in the absence of our observation and testing services on-site, we should be immediately notified so that we can provide evaluation and supplemental recommendations.

Our Scope of Services does not include either specifically or by implication any environmental or biological (e.g., mold, fungi, bacteria) assessment of the site or identification or prevention of pollutants, hazardous materials or conditions. If the owner is concerned about the potential for such contamination or pollution, other studies should be undertaken.

Our services and any correspondence are intended for the sole benefit and exclusive use of our client for specific application to the project discussed and are accomplished in accordance with generally accepted geotechnical engineering practices with no third-party beneficiaries intended. Any third-party access to services or correspondence is solely for information purposes to support the services provided by Terracon to our client. Reliance upon the services and any work product is limited to our client and is not intended for

third parties. Any use or reliance of the provided information by third parties is done solely at their own risk. No warranties, either express or implied, are intended or made.

Site characteristics as provided are for design purposes and not to estimate excavation cost. Any use of our report in that regard is done at the sole risk of the excavating cost estimator as there may be variations on the site that are not apparent in the data that could significantly effect excavation cost. Any parties charged with estimating excavation costs should seek their own site characterization for specific purposes to obtain the specific level of detail necessary for costing. Site safety and cost estimating including excavation support and dewatering requirements/design are the responsibility of others. Construction and site development have the potential to affect adjacent properties. Such impacts can include damages due to vibration, modification of groundwater/surface water flow during construction, foundation movement due to undermining or subsidence from excavation, as well as noise or air quality concerns. Evaluation of these items on nearby properties are commonly associated with contractor means and methods and are not addressed in this report. The owner and contractor should consider a preconstruction/precondition survey of surrounding development. If changes in the nature, design, or location of the project are planned, our conclusions and recommendations shall not be considered valid unless we review the changes and either verify or modify our conclusions in writing.

Attachments

Exploration and Testing Procedures

Field Exploration

Number of Borings	Approximate Boring Depth	Location
4	51.5 feet	Proposed Transmission Line Areas/Access Roadways

Boring Layout and Elevations: Terracon personnel provided the boring layout using handheld GPS equipment (estimated horizontal accuracy of about ±15 feet) and referencing existing site features. Approximate ground surface elevations were estimated using Google Earth. If elevations and a more precise boring layout are desired, we recommend borings be surveyed.

Subsurface Exploration Procedures: We advanced the borings with a track-mounted drill rig using continuous hollow stem flight. Four samples were obtained in the upper 10 feet of each boring and at intervals of 5 feet thereafter. Test samples were collected during drilling in general accordance with the appropriate ASTM methods using Standard Penetration Testing (SPT) and sampling using either standard split-spoon or Modified California samplers. A sampling spoon was driven into the ground by a 140 pound automatic hammer falling a distance of 30 inches. The number of blows required to advance the sampling spoon the last 12 inches of a normal 18 inch penetration was recorded as the Standard Penetration Test (SPT) resistance value, also referred to as N-values. The N-values are indicated on the boring logs at the test depths. The samples were placed in appropriate containers, taken to our soil laboratory for testing, and classified by a geotechnical engineer.

For safety purposes, all borings were backfilled with auger cuttings after their completion.

We also observed the boreholes while drilling and at the completion of drilling for the presence of groundwater. Groundwater was not encountered at the time of drilling.

The sampling depths, penetration distances, and other sampling information was recorded on the field boring logs. The samples were placed in appropriate containers and taken to our soil laboratory for testing and classification by a Geotechnical Engineer. Our exploration team prepared field boring logs as part of the drilling operations. These field logs included visual classifications of the materials observed during drilling and our interpretation of the subsurface conditions between samples. Final boring logs were prepared from the field logs. The final boring logs represent the Geotechnical Engineer's interpretation of the field logs and include modifications based on observations and tests of the samples in our laboratory.

Electrical Resistivity Testing: Soil electrical resistivity data was be obtained in accordance with ASTM G57 Standard Test Method for Field Measurement of Soil Resistivity Using the Wenner Four-Electrode Method. At each test location, two near perpendicular lines were tested. Electrode "a" spacings are summarized in the following table. Electrode spacing was adjusted to conform to site conditions.

No. of Test Locations	Electrode "a" Spacing (feet)
1	2, 5, 10, 20, and 40 feet

Laboratory Testing

The project engineer reviewed the field data and assigned laboratory tests. The laboratory testing program included the following types of tests:

- Moisture Content
- Dry Unit Weight
- Atterberg Limits
- Modified Proctor Compaction
- Direct Shear
- Corrosivity
- Thermal Resistivity

The laboratory testing program often included examination of soil samples by an engineer. Based on the results of our field and laboratory programs, we described and classified the soil samples in accordance with the Unified Soil Classification System.

Site Location and Exploration Plans

Contents:

Site Location
Exploration Location Plan
Electrical Resistivity Test Location Plan

Site Location



DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Exploration Location Plan

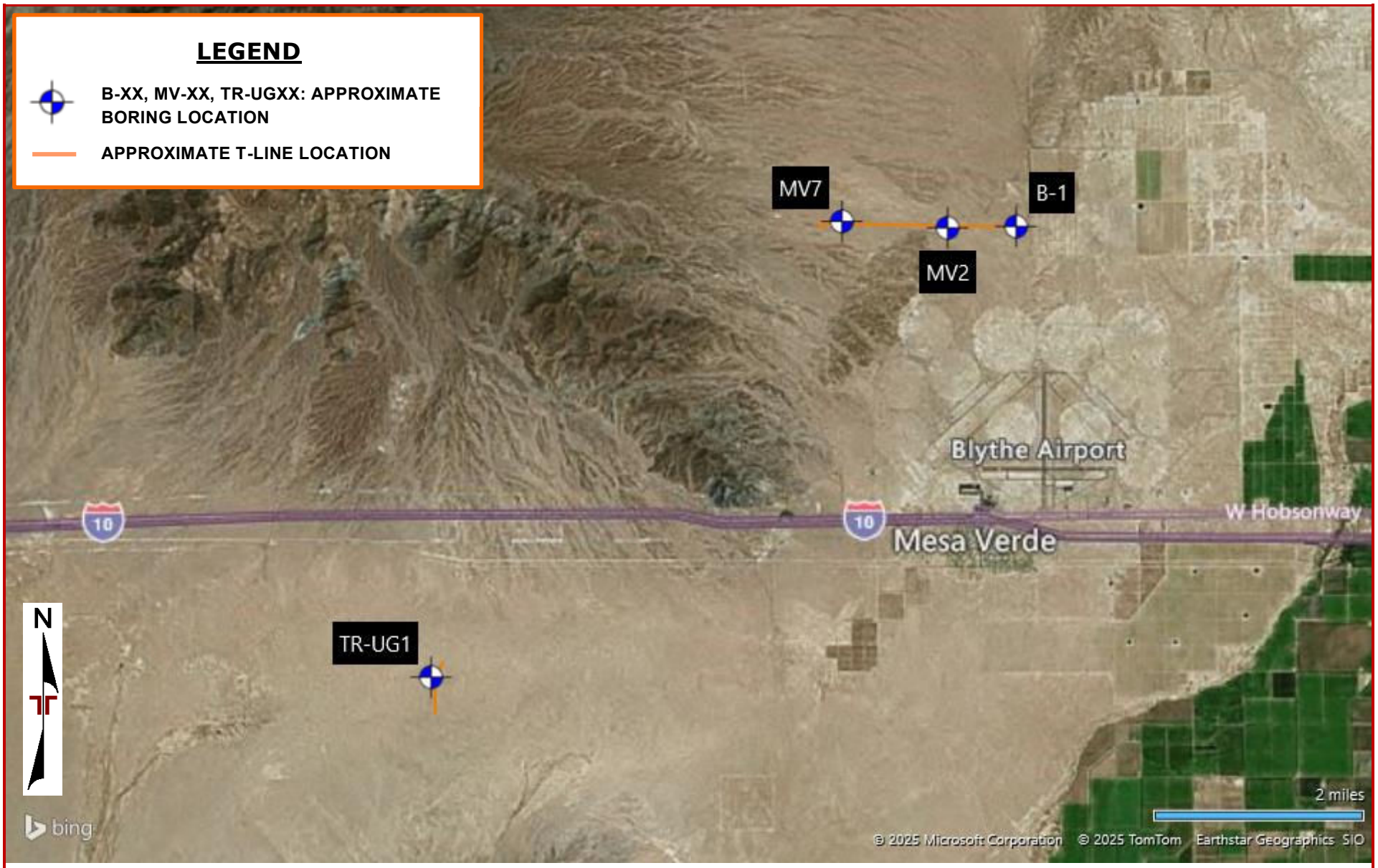


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Electrical Resistivity Test Location Plan

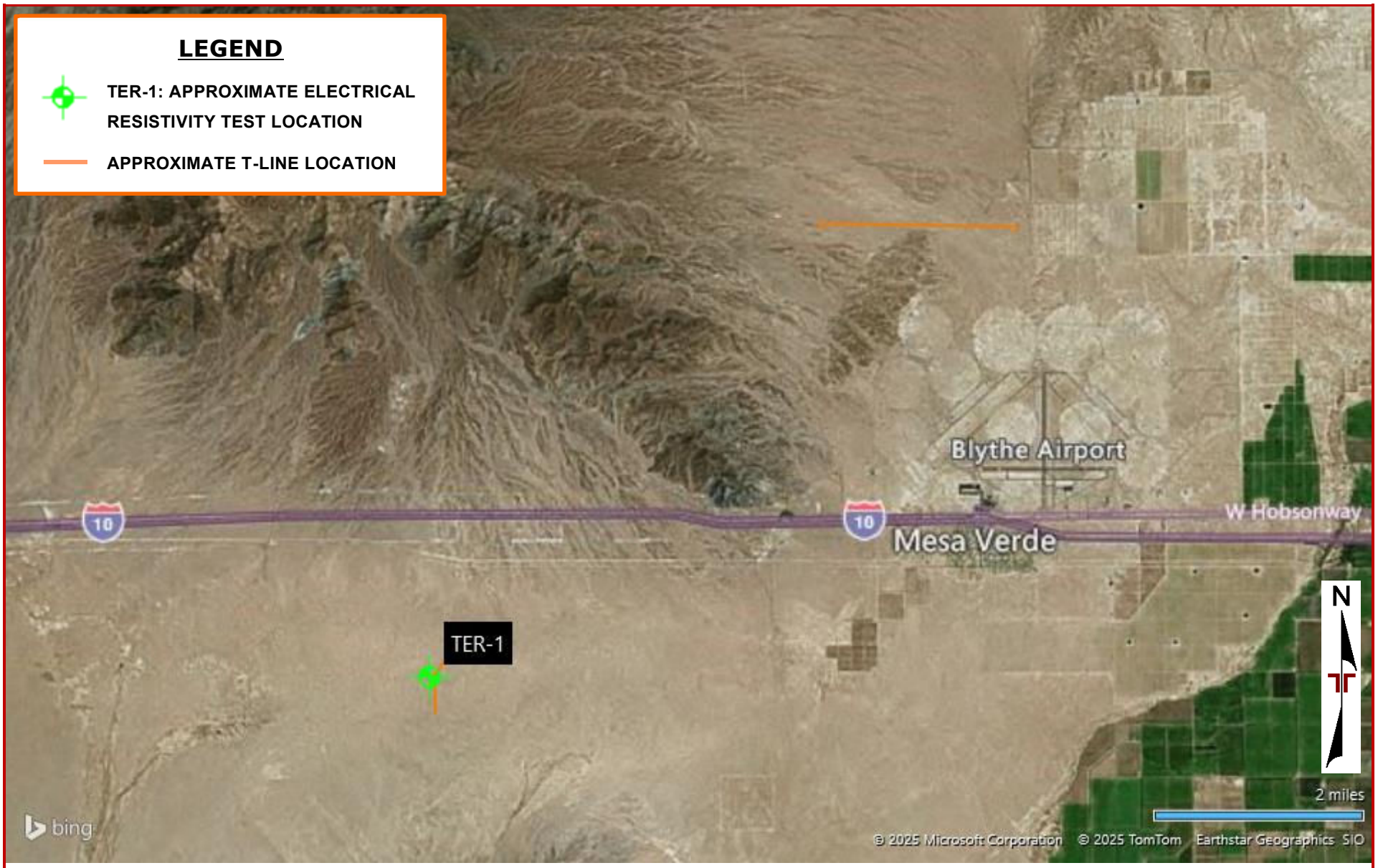


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Exploration and Laboratory Results

Contents:

Boring Logs
Atterberg Limits
Sieve + Hydrometer
Moisture-Density Relationship
Direct Shear
Corrosion Results
Thermal Resistivity Test Results
Field Electrical Resistivity Results

Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. B-1

Location: See Exploration Plan

Latitude: 33.6408° Longitude: -114.6990°

Depth (Ft.)	Graphic Log	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0.1		**TOPSOIL**, silty sand with gravel									
		SILTY SAND (SM), brown, moderate cementation									
		medium dense, sample disturbed				20-28-30	C-5	7.1		NP	29.5
5.0		**POORLY GRADED SAND (SP)**, brown to light brown, medium dense	5			5-10-16 N=26					
7.5		**FAT CLAY (CH)**, brown to light brown, hard, moderate cementation, sample disturbed				9-22-30	E-8	16.4			
10.0		**POORLY GRADED SAND (SP)**, brown to light brown, medium dense	10			6-6-7 N=13					
		sample disturbed	15			8-12-24	C-5	1.3			
20.0		**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown, very dense	20			16-26-36 N=62					
25.0		**POORLY GRADED SAND (SP)**, light brown, very dense, moderate cementation, sample disturbed	25			50/6"		2.3			

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-13-2025

Boring Completed
02-13-2025

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. B-1

Location: See Exploration Plan

Latitude: 33.6408° Longitude: -114.6990°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0	**POORLY GRADED SAND (SP)**, light brown, very dense, moderate cementation, sample disturbed *(continued)*								
30.0	**SILTY SAND (SM)**, trace gravel, light brown, very dense			22-42-37 N=79	C-3				
35	brown, moderate cementation, sample disturbed			45-50/1"					
40.0	**SILTY SAND (SM)**, light brown, dense, moderate cementation			18-20-26 N=46	C-4				
45.0	**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown, very dense			24-44-50/4"	C-3	2.3	106		
50				12-30-40 N=70					
51.5	***Boring Terminated at 51.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-13-2025

Boring Completed
02-13-2025

Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. MV-2

Location: See Exploration Plan

Latitude: 33.6497° Longitude: -114.7321°

Depth (Ft.)	Material Description	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0.1	**TOPSOIL**, poorly graded sand with gravel						
	SILTY SAND (SM), trace gravel, light brown, moderate cementation; medium dense	17-16-23	C-5	2.3	107	NP	18.6
5.0	**POORLY GRADED SAND WITH GRAVEL (SP)**, light brown, medium dense	7-8-9 N=17					
7.5	**POORLY GRADED SAND (SP)**, light brown, medium dense, sample disturbed	10-22-24		0.7			
10.0	**POORLY GRADED SAND WITH GRAVEL (SP)**, light brown, medium dense	6-11-11 N=22					
	dense, sample disturbed	14-30-42	C-4	0.7			
20.0	**POORLY GRADED GRAVEL WITH SILT AND SAND (GP-GM)**, light brown, very dense	33-50/5"	B-3				
25.0	**POORLY GRADED SAND (SP)**, light brown, very dense	21-50/6"		7.2	101		

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-19-2025

Boring Completed
02-19-2025

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. MV-2

Location: See Exploration Plan

Latitude: 33.6497° Longitude: -114.7321°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	POORLY GRADED SAND (SP), light brown, very dense *(continued)*								
30.0									
30	**POORLY GRADED SAND WITH GRAVEL (SP)**, light brown, very dense			7-25-31 N=56					
35.0									
35	**POORLY GRADED SAND (SP)**, light brown, very dense			24-50/5"	C-3	1.6	105		
40				14-50/6"					
45				33-50/6"		1.8	100		
50	dense			7-16-22 N=38	C-4				
51.5									
	Boring Terminated at 51.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-19-2025

Boring Completed
02-19-2025

Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. MV-7

Location: See Exploration Plan

Latitude: 33.6506° Longitude: -114.7497°

Depth (Ft.)	Graphic Log	Material Description	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0		**SILTY SAND (SM)**, light brown dense			13-18-17 N=35	C-4			NP	
5		medium dense			16-26-30	C-5	3.4	104		
7.5		**POORLY GRADED SAND (SP)**, light brown dense			8-14-16 N=30					
10					18-28-36	C-4	1.0	101		
15		medium dense			7-11-15 N=26	C-5				
20		very dense, moderate cementation			22-50/5"	C-3	0.7	97		
25		dense			7-21-27 N=48					

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-20-2025

Boring Completed
02-20-2025

Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. MV-7

Location: See Exploration Plan

Latitude: 33.6506° Longitude: -114.7497°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
					C-4				
30				21-38-50/4"		0.8	98		
35				12-25-28 N=53	C-3				
40				31-50/4"		1.4	94		
45				8-20-30 N=50	C-4				
50				21-50/5"	C-3	2.0			

POORLY GRADED SAND (SP), light brown *(continued)*

very dense

dense

50.0
POORLY GRADED SAND WITH GRAVEL (SP), light brown, very
50.9 dense, sample disturbed

Boring Terminated at 50.92 Feet

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-20-2025

Boring Completed
02-20-2025

Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. TR-UG1

Location: See Exploration Plan

Latitude: 33.5874° Longitude: -114.8181°

Depth (Ft.)	Graphic Log	Material Description	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
0.1		**TOPSOIL**, poorly graded sand								
		SILTY SAND (SM), light brown								
		medium dense, sample disturbed			9-11-13		1.1			15.5
5					7-13-18		0.9	95		21.3
					6-7-11 N=18					
10					11-18-22		4.5	89		
15		trace clay, moderate cementation			6-9-13 N=22	C-5				
20		sample disturbed			9-17-26		1.0			
25					6-11-14 N=25					

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-20-2025

Boring Completed
02-20-2025

Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. TR-UG1

Location: See Exploration Plan

Latitude: 33.5874° Longitude: -114.8181°

Depth (Ft.)	Graphic Log	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	SILTY SAND (SM), light brown *(continued)*								
30	very dense			16-38-50/5"	C-3	1.0	105		
35	dense			10-20-24 N=44					
40				18-30-50/5"	C-4	0.7	94		
45	medium dense, moderate cementation			7-16-25 N=41					
50	very dense			25-50/4"	C-3	1.8	98		
50.8	***Boring Terminated at 50.8 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

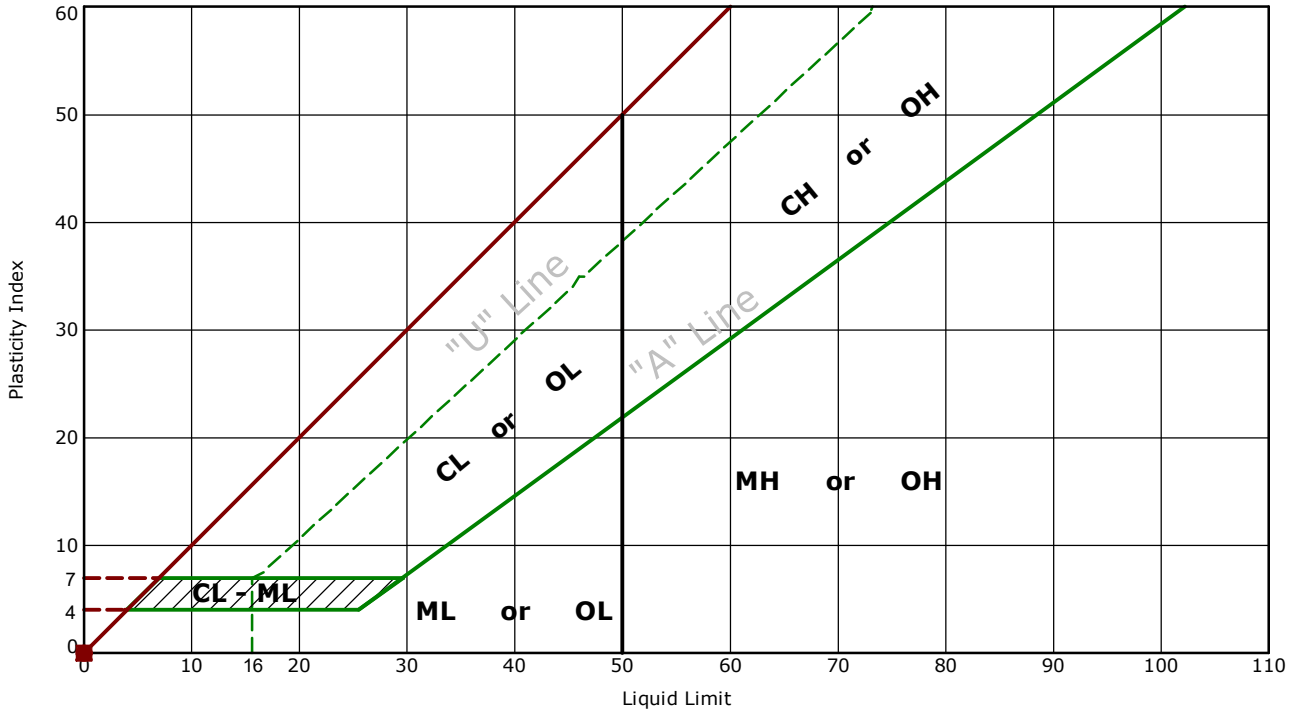
Boring Started
02-20-2025

Boring Completed
02-20-2025

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Atterberg Limit Results

ASTM D4318



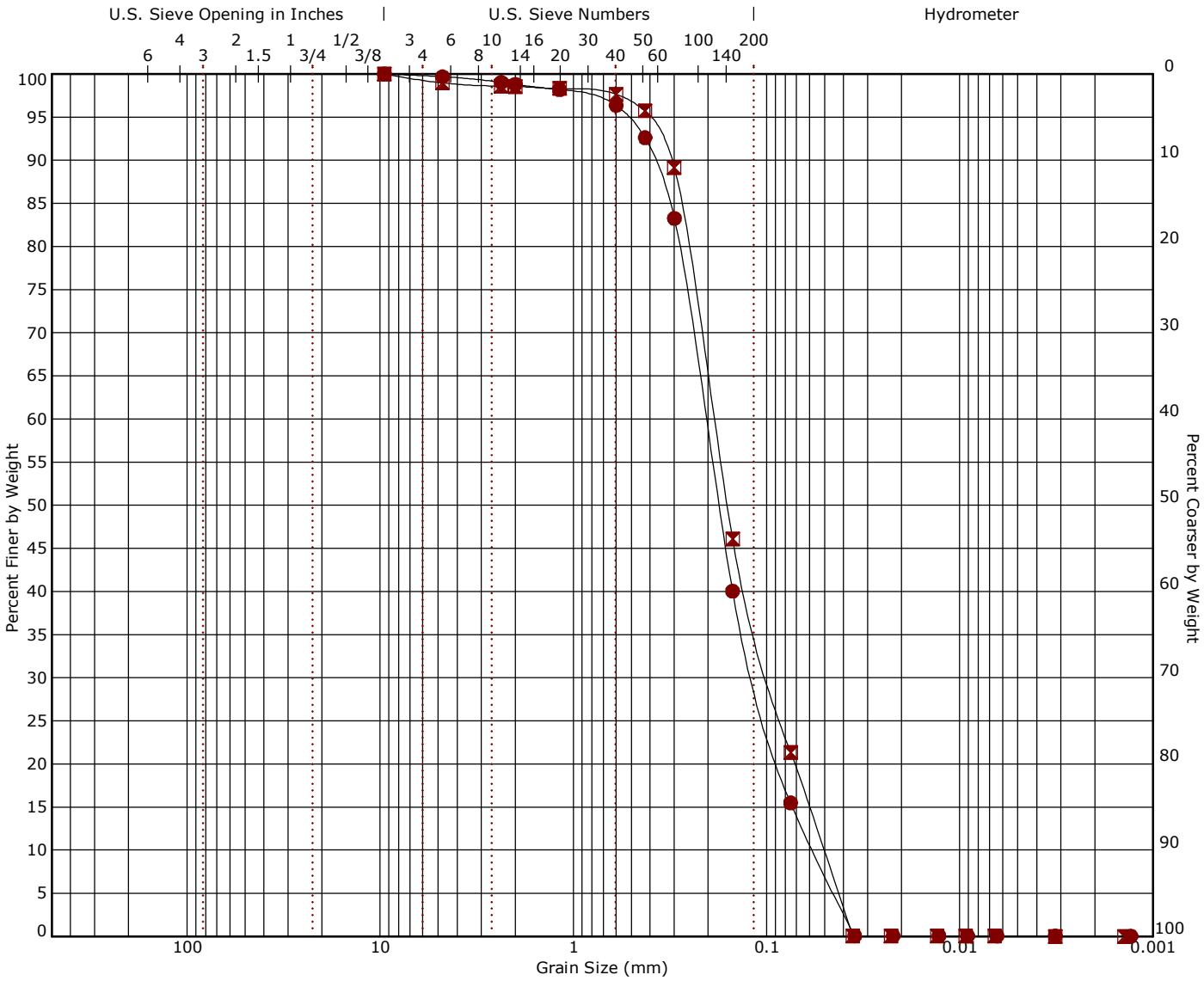
	Boring ID	Depth (Ft)	LL	PL	PI	Fines	USCS	Description
●	B-1	1 - 4	NP	NP	NP	29.5	SM	SILTY SAND
⊠	MV-2	1 - 4	NP	NP	NP	18.6	SM	SILTY SAND
▲	MV-7	1 - 4	NP	NP	NP	--	SM	SILTY SAND

Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

Grain Size Distribution

ASTM D422 / ASTM C136



Cobbles	Gravel		Sand			Silt or Clay
	coarse	fine	coarse	medium	fine	

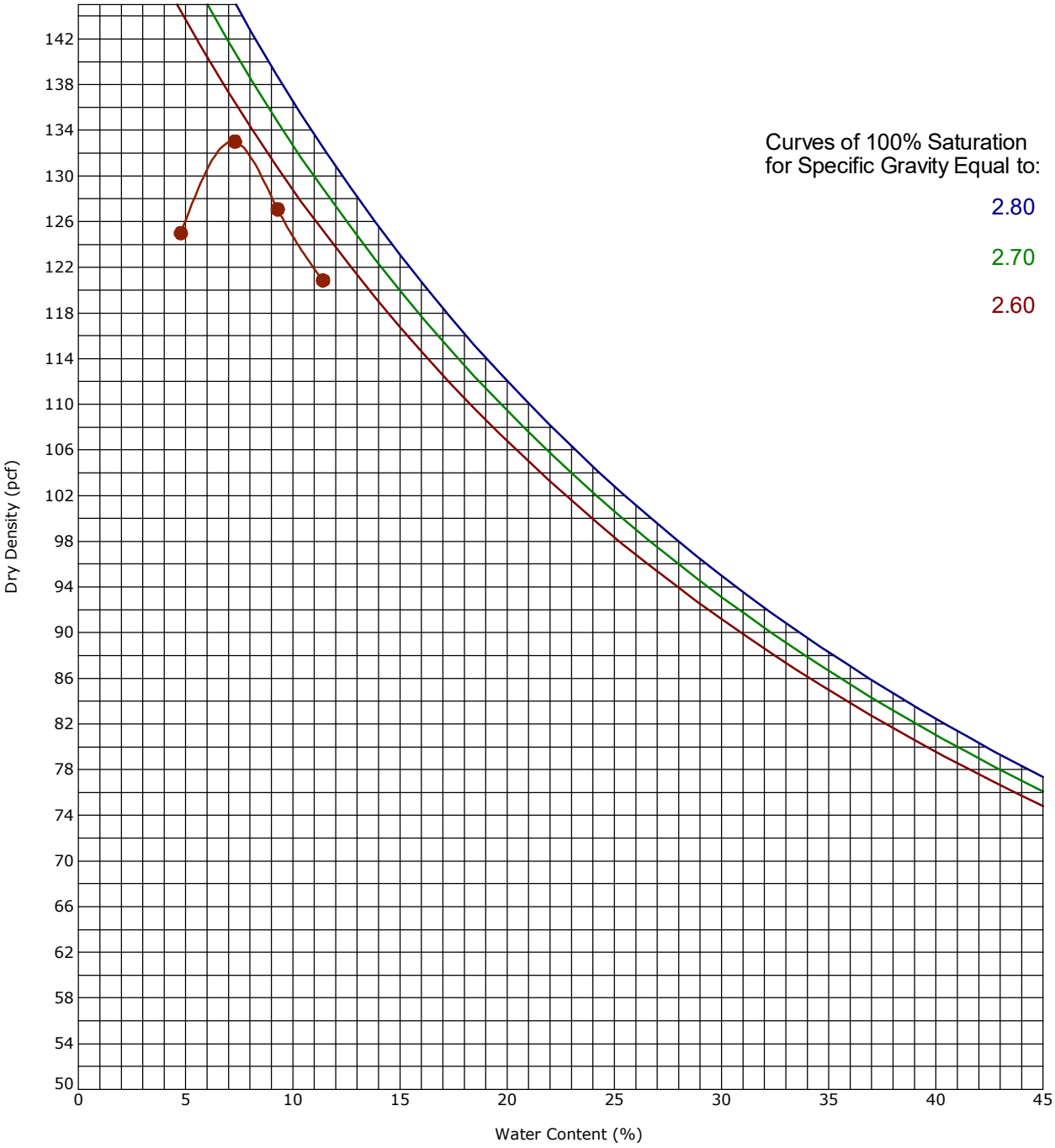
Boring ID	Depth (Ft)	Description	USCS	LL	PL	PI	Cc	Cu
TR-UG1	1 - 4	SILTY SAND	SM	--	--	--	1.08	3.61
TR-UG1	4 - 6	SILTY SAND	SM	--	--	--	0.96	3.71

Boring ID	Depth (Ft)	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
TR-UG1	1 - 4	9.5	0.207	0.113	0.057	0.0	0.3	84.2	15.5	15.4	0.1
TR-UG1	4 - 6	9.5	0.188	0.096	0.051	0.0	1.0	77.6	21.3	21.3	0.0

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Moisture-Density Relationship

ASTM D1557-Method C



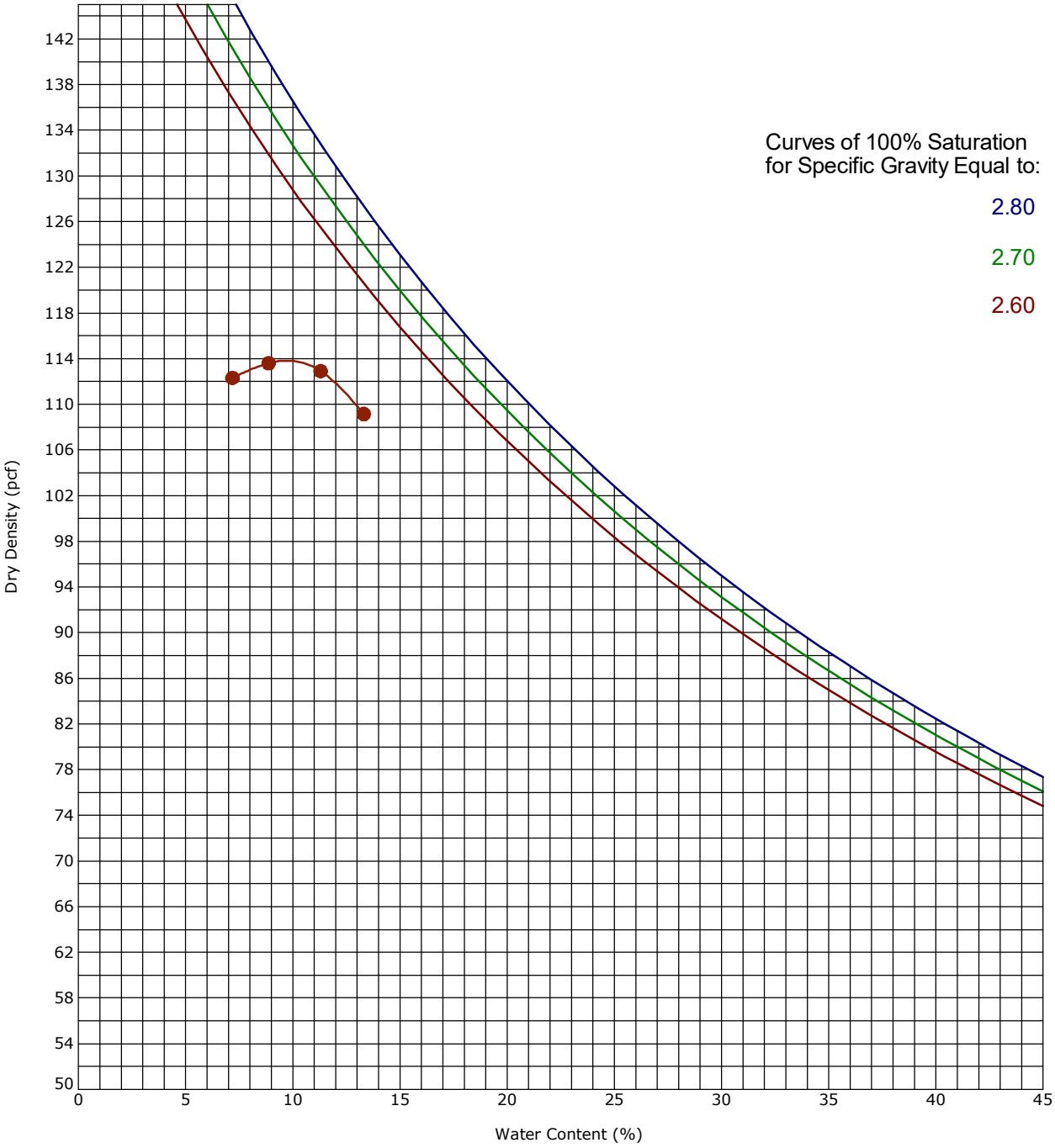
Boring ID	Depth (Ft)	Description of Materials
MV-2	1 - 4	SILTY SAND

Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
19	0.074	NP	NP	NP	ASTM D1557-Method C	133.0	7.2

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Moisture-Density Relationship

ASTM D1557-Method A



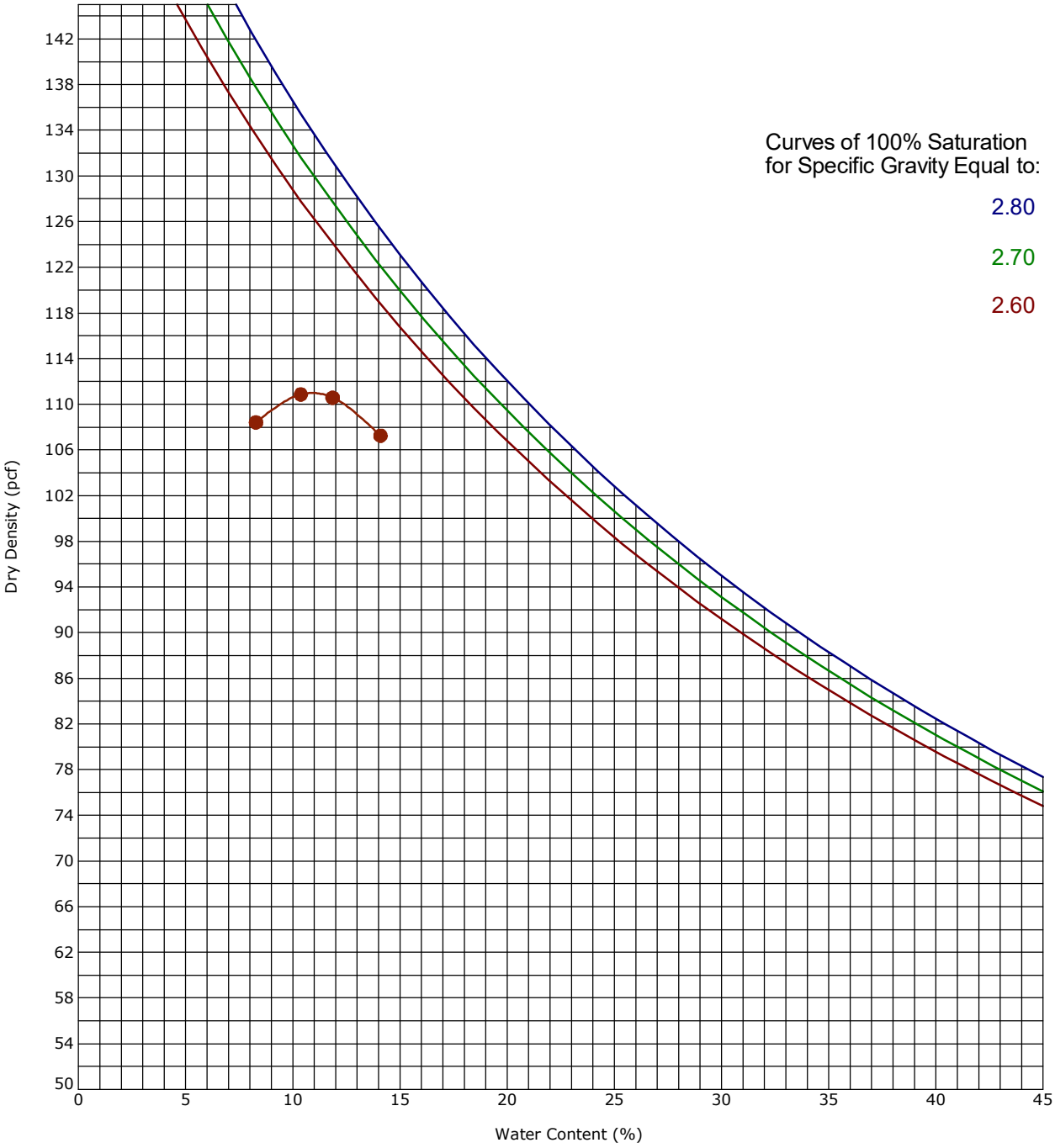
Boring ID	Depth (Ft)	Description of Materials
TR-UG1	1 - 4	SILTY SAND

Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
15	0.074	--	--	--	ASTM D1557-Method A	113.8	9.8

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Moisture-Density Relationship

ASTM D1557-Method A



Boring ID	Depth (Ft)	Description of Materials
TR-UG1	4 - 6	SILTY SAND

Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
21	0.074	--	--	--	ASTM D1557-Method A	111.0	10.9

Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No.: 60225164

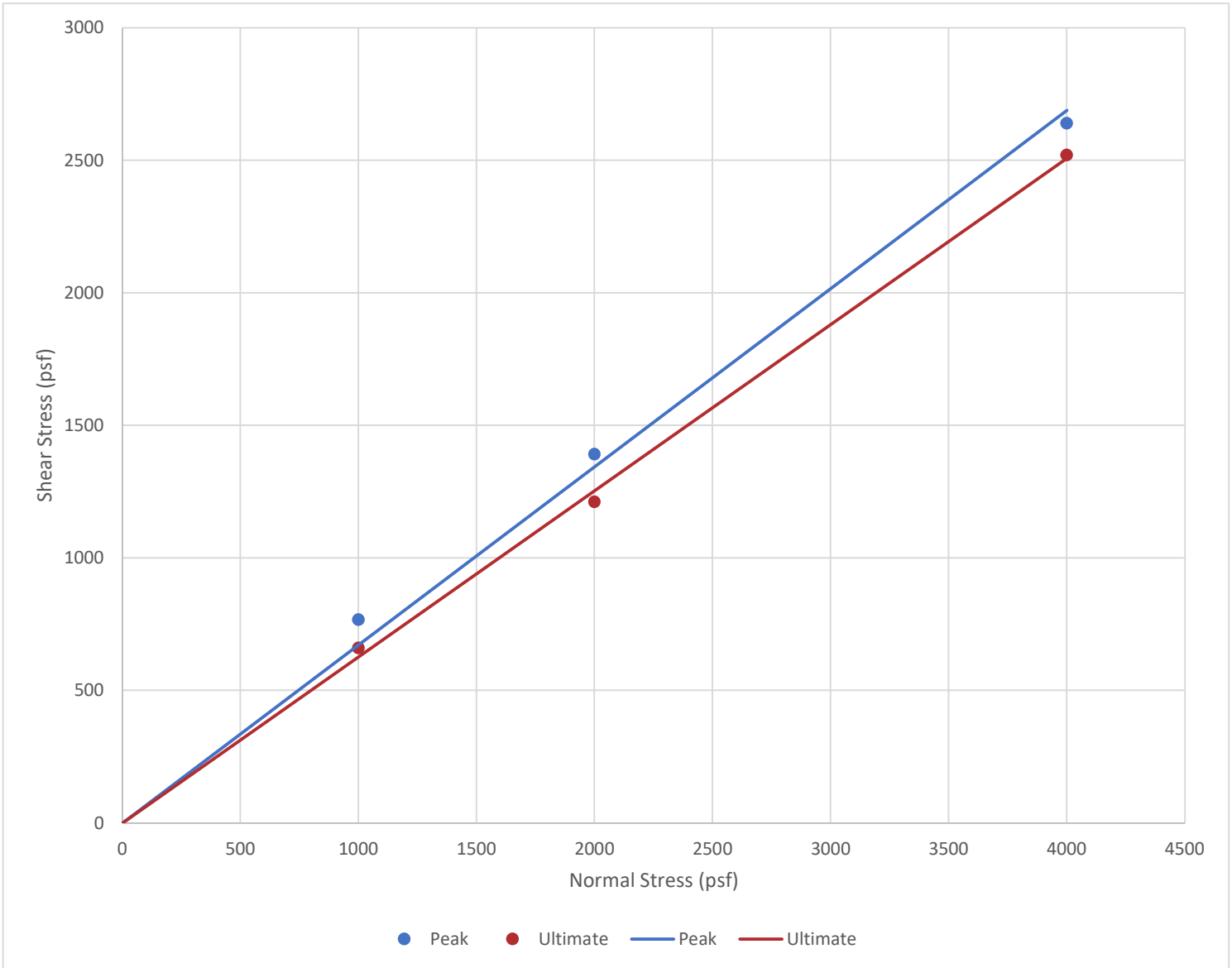
145 W Walnut St.
Gardena, CA

Direct Shear Test

ASTM D3080

Boring ID	Depth (ft)	Description	USCS	γ_d (pcf)	W(%)
MV-2	7.5	Poorly Graded Sand	SP		0.7

Normal Stress (psf)	Peak Shear Stress (psf)	Ultimate Shear Stress (psf)	Peak		Ultimate	
			φ°	C (psf)	φ°	C (psf)
1000	768	660	**32.0**	**140**	**32.0**	**10**
2000	1392	1212				
4000	2640	2520				



Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No.: 60225164

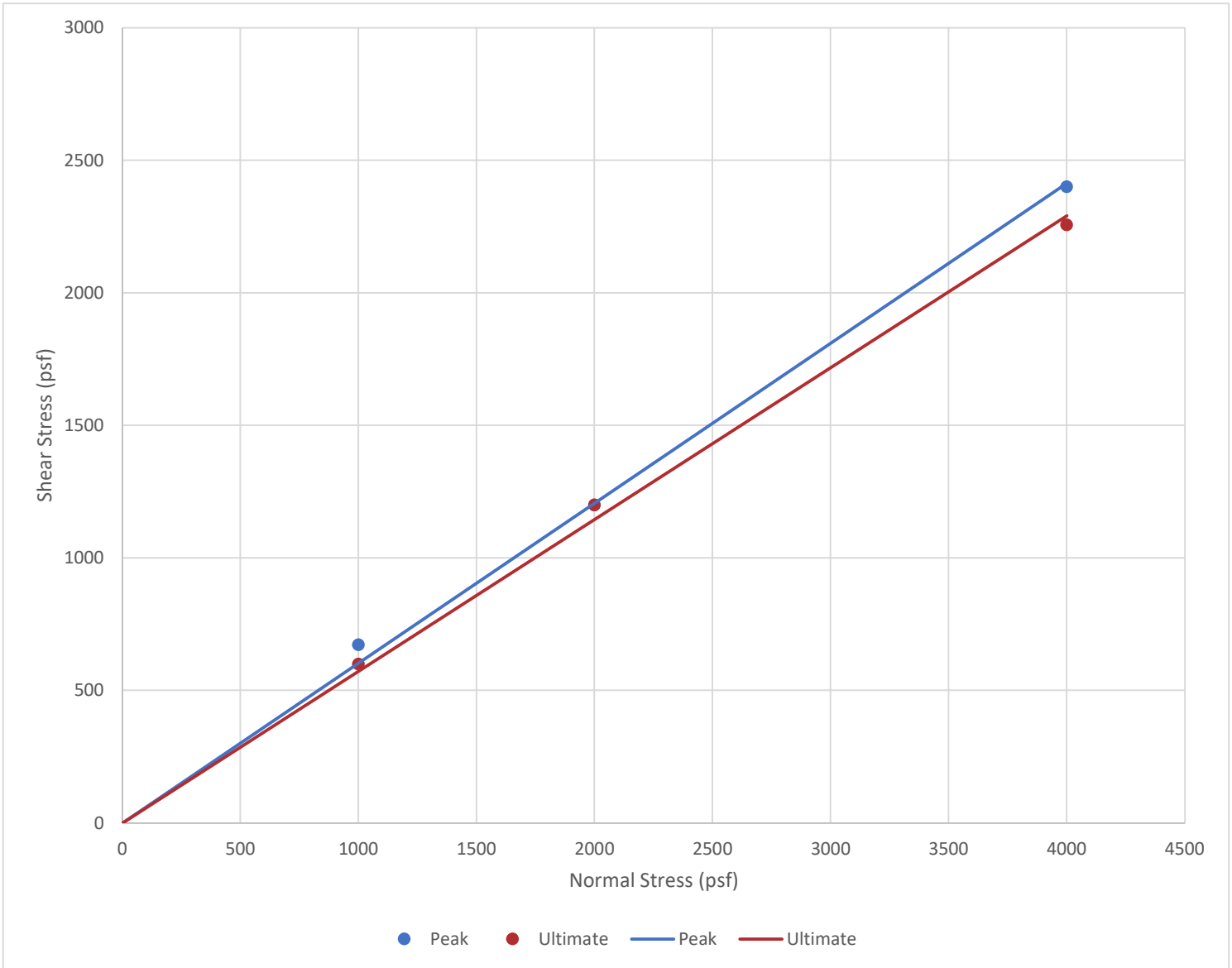
145 W Walnut St.
Gardena, CA

Direct Shear Test

ASTM D3080

Boring ID	Depth (ft)	Description	USCS	γ_d (pcf)	W(%)
MV-7	10	Poorly Graded Sand	SP	101	1.0

Normal Stress (psf)	Peak Shear Stress (psf)	Ultimate Shear Stress (psf)	Peak		Ultimate	
			φ°	C (psf)	φ°	C (psf)
1000	672	600	**30.0**	**70**	**29.0**	**70**
2000	1200	1200				
4000	2400	2256				



Grace Solar - Transmission Line
Blythe | Riverside County, CA
Terracon Project No.: 60225164

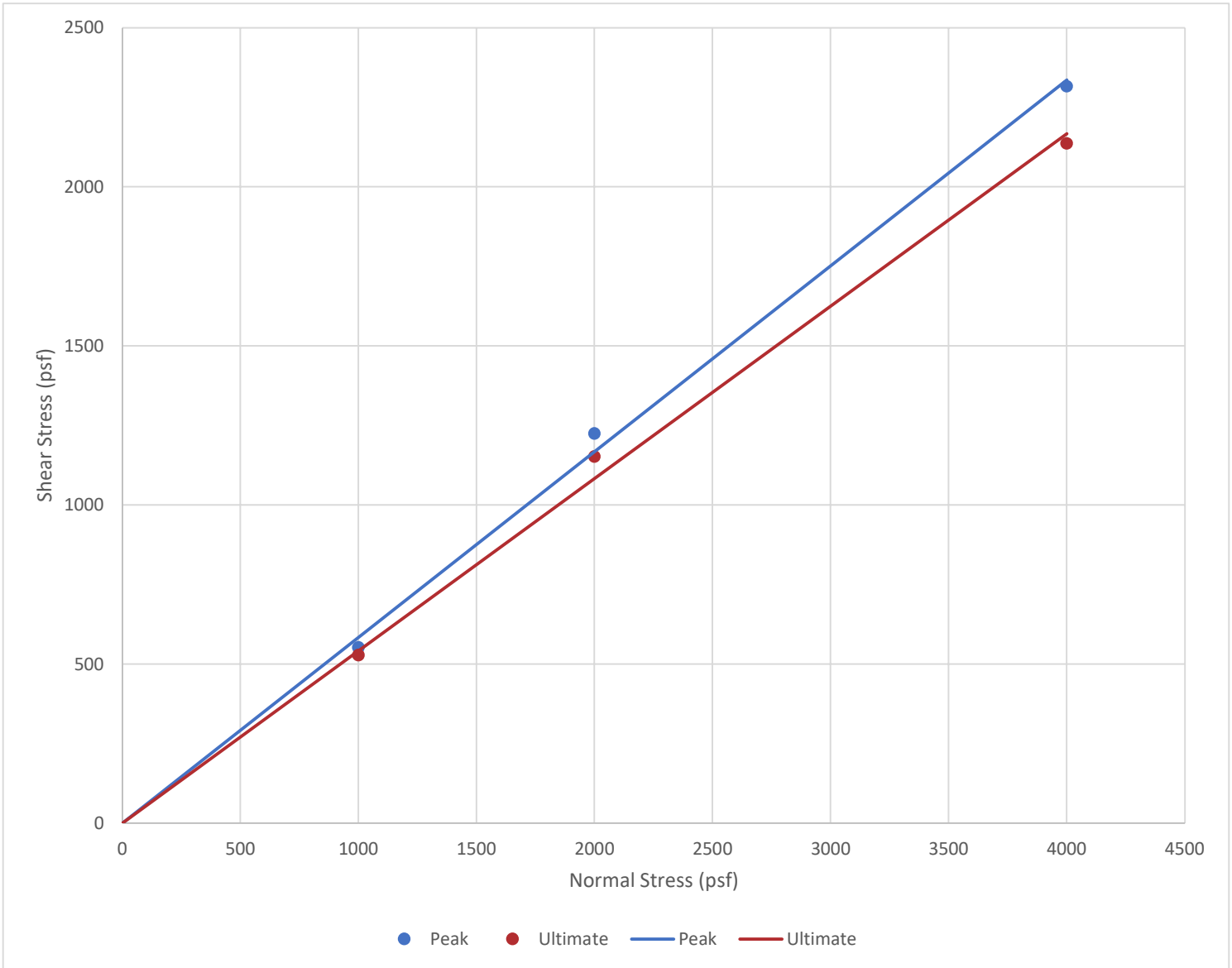
145 W Walnut St.
Gardena, CA

Direct Shear Test

ASTM D3080

Boring ID	Depth (ft)	Description	USCS	γ_d (pcf)	W(%)
TR-UG1	10	Silty Sand	SM	89	4.5

Normal Stress (psf)	Peak Shear Stress (psf)	Ultimate Shear Stress (psf)	Peak		Ultimate	
			φ°	C (psf)	φ°	C (psf)
1000	552	528	**30.0**	**10**	**28.0**	**40**
2000	1224	1152				
4000	2316	2136				



9123 Chesapeake Dr
San Diego, CA 92123
(619) 821-3630

Client
NextEra Energy Resources LLC

Project
Grace Solar - Transmission Line

Sample Submitted By: V. Nguyen **Date Tested:** 3/18/2025 **Project Number:** 60225164

Sample Type	Auger
Sample Location	B-1
Sample Depth (ft.)	1.0' - 4.0'
pH Analysis, ASTM G 51	7.47
Water Soluble Sulfate (SO4), ASTM D516 (%)	0.05
Sulfides, AWWA 4500-S^{2-} D, (mg/kg)	0.57
Chlorides, ASTM D512, (mg/kg)	211.5
Red-Ox, ASTM G 200, (mV)	+169
Total Salts, AWWA 2520 B, (mg/kg)	460.0
Saturated Minimum Resistivity, ASTM G 57, (ohm-cm)	540

Reviewed By:

Tom Remmel
Laboratory Manager

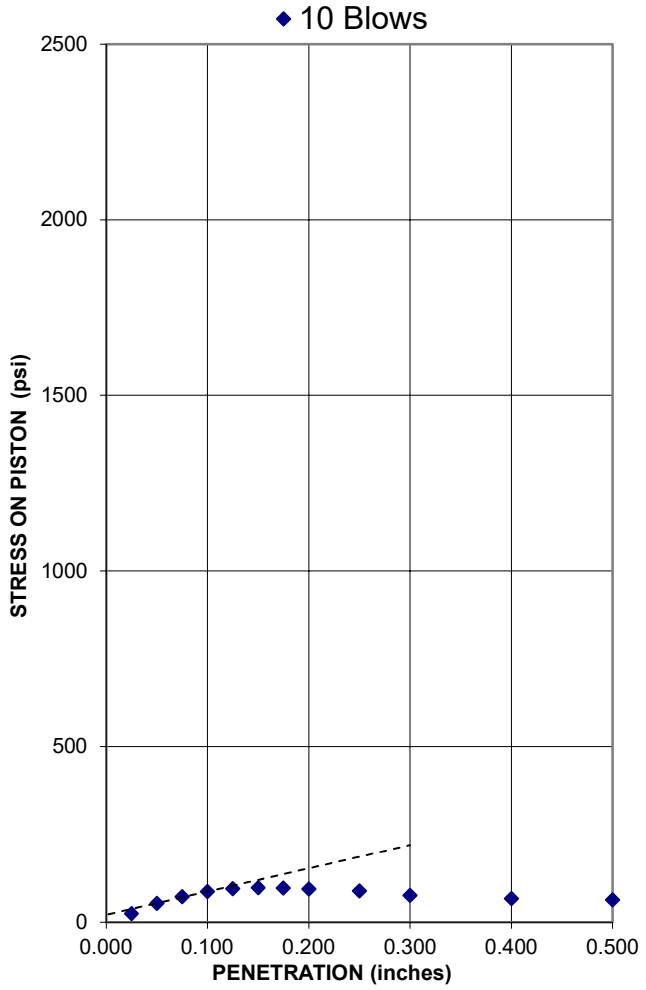
The tests were performed in general accordance with applicable ASTM and AWWA test methods. This report is exclusively for the use of the client indicated above and shall not be reproduced except in full without the written consent. Test results transmitted herein are only applicable to the actual samples tested at the location(s) referenced and are not necessarily indicative of the properties of other apparently similar or identical materials.

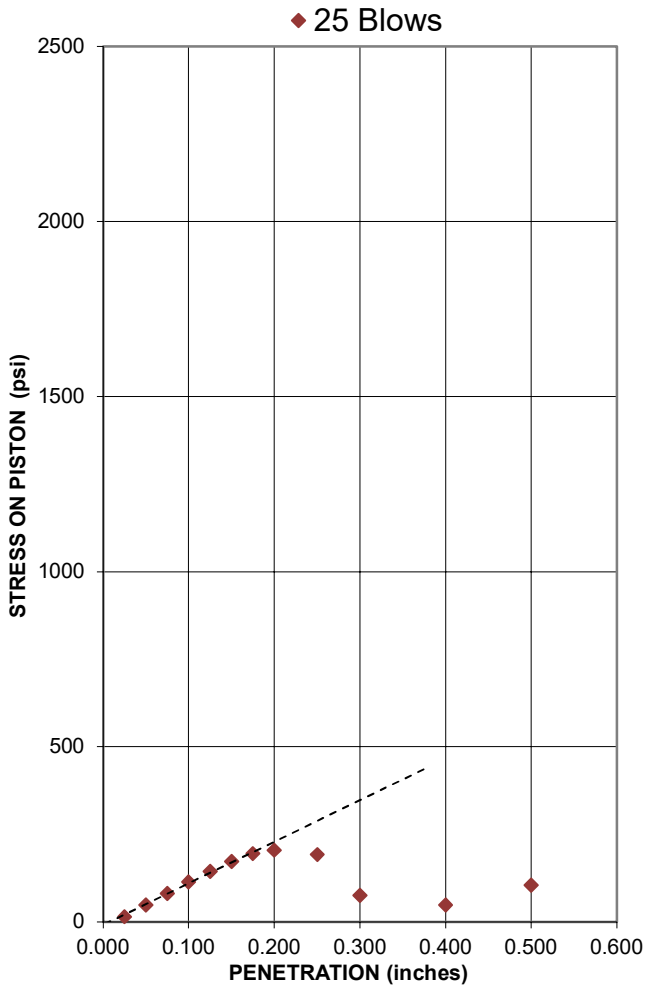
CBR(CALIFORNIA BEARING RATIO) OF LABORATORY-COMPACTED SOILS
ASTM D1883 (SOAKED)

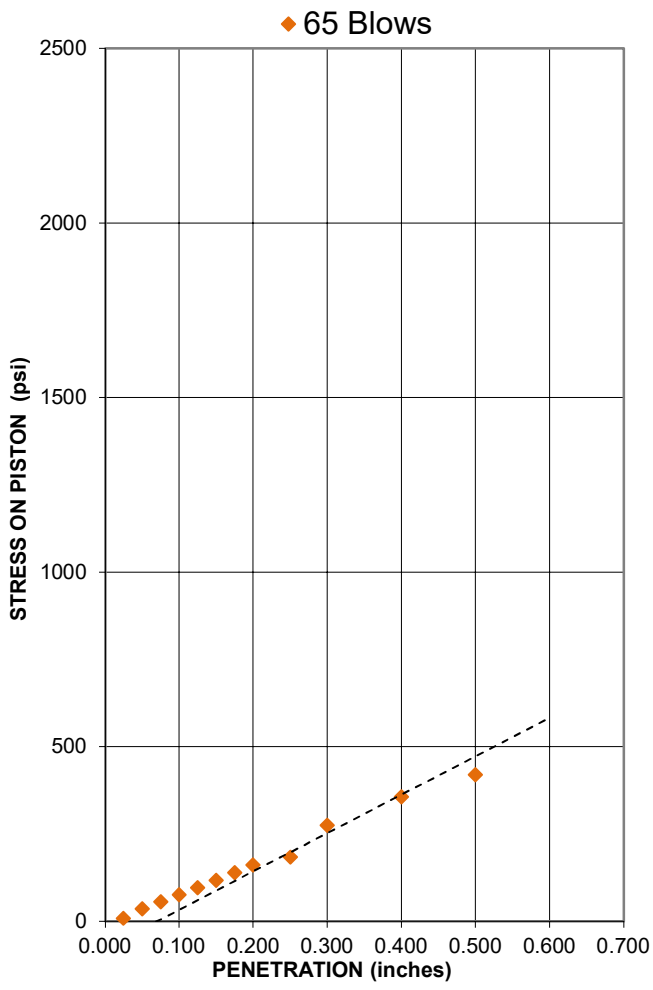
Terracon

PROJECT:	Grace Solar Basis of Bid	**JOB NO:**	60225164
LOCATION:	Blythe, CA	**WORK ORDER NO:**	60225164.0004
MATERIAL:	Auger	**LAB NO:**	MV-2 @ 0'-4'
SAMPLE SOURCE:	MV-2 @ 0'-4'	**DATE SAMPLED:**	4/22/25

	Blows per Lift	Compaction %	Compaction Method	Surcharge Weight (Lb.)	Sample Condition	Percent Swell	Before Soak		After Soak	
							Dry Density	Moisture %	Dry Density	Moisture %
Point 1	10	85.3	ASTM D1557	10	Soak	0.00	113.4	7.0	111.1	9.3
Point 2	25	90.1	ASTM D1557	10	Soak	0.00	119.9	7.0	117.0	9.6
Point 3	65	95.1	ASTM D1557	10	Soak	0.00	126.5	7.0	123.8	9.3







REVIEWED BY _____

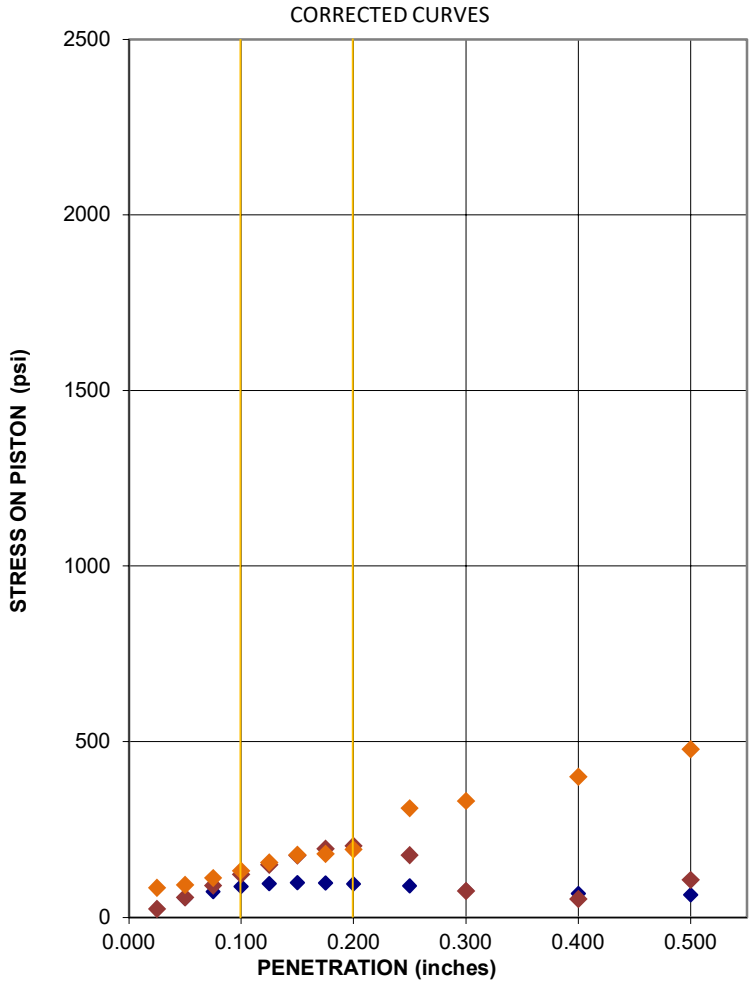
CBR(CALIFORNIA BEARING RATIO) OF LABORATORY-COMPACTED SOILS
ASTM D1883 (SOAKED)

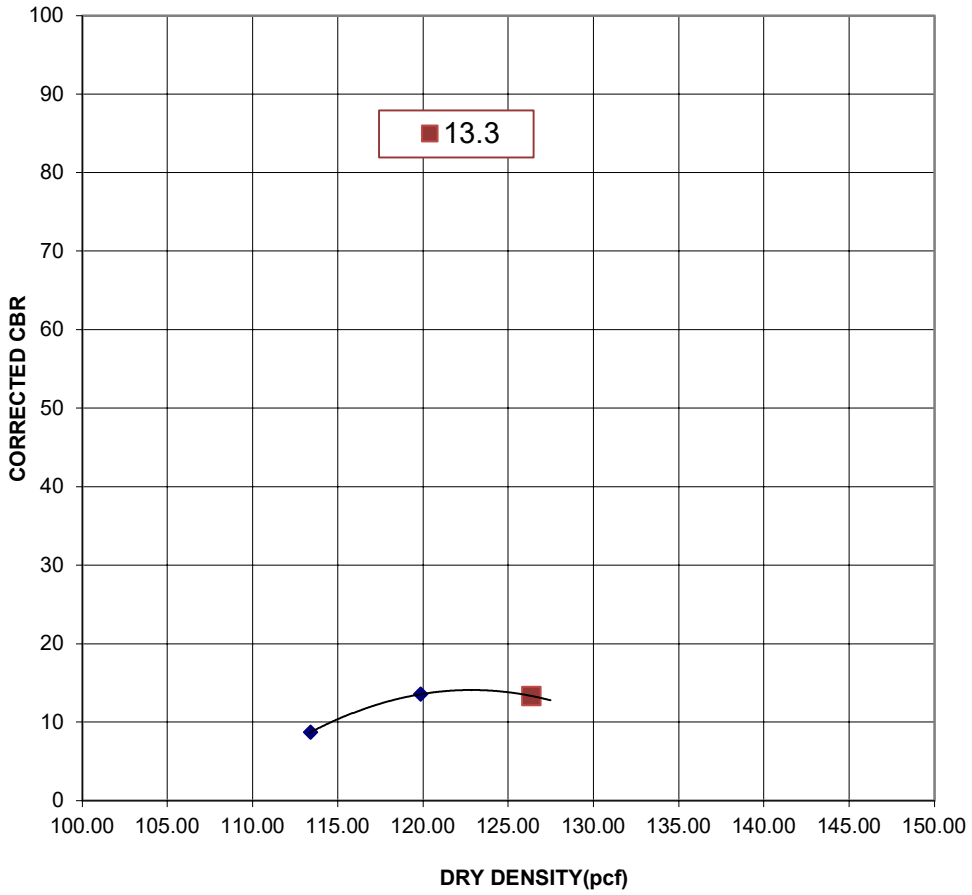
Terracon

PROJECT:	Grace Solar Basis of Bid	**JOB NO:**	60225164
LOCATION:	Blythe, CA	**WORK ORDER NO:**	60225164.0004
MATERIAL:	Auger	**LAB NO:**	MV-2 @ 0'-4'
SAMPLE SOURCE:	MV-2 @ 0'-4'	**DATE SAMPLED:**	4/22/25

	Correction Factor		Corrected CBR		
	0.1"	0.2"	0.1"	0.2"	CBR
Point 1	0.00	0.00	8.7	6.3	8.7
Point 2	0.01	0.01	12.2	13.6	13.6
Point 3	0.07	0.07	13.3	12.9	13.3

ASTM D1557				95% COMPACTION CBR
Moisture	Dry Density (pcf)			
%	100%	95%	90%	13.3
7.0	133.0	126.4	119.7	





750 Pilot Road, Suite F
Las Vegas, Nevada 89119
(702) 597-9393

Client
NextEra Energy Constructors LLC

Project
Grace Solar - Transmission Line

Sample Submitted By: Terracon (60) **Date Received:** 4/3/2025 **Lab No.: 25-0099**

Results of Corrosion Analysis

Sample Number	--	--	--	--
Sample Location	MV-2	MV-7	TR-UG1	BESS-02
Sample Depth (ft.)	1.0-4.0	1.0-4.0	1.0-4.0	1.0-4.0
pH Analysis, ASTM G51	8.75	9.14	8.89	9.14
Water Soluble Sulfate (SO4), ASTM C 15806 (Percent %)	0.01	<0.01	0.01	0.01
Sulfides, AWWA 4500-S D, (mg/Kg)	Nil	Nil	Nil	Nil
Chlorides, ASTM D512, (mg/kg)	50	52	27	325
Red-Ox, ASTM G200, (mV)	+728	+734	+735	+721
Total Salts, AWWA 2540, (mg/Kg)	420	238	182	1232
Saturated Minimum Resistivity, ASTM G-187, (ohm-cm)	1675	1876	2144	3149

Analyzed By N. Campo

Nathan Campo
Laboratory Coordinator

The tests were performed in general accordance with applicable ASTM and AWWA test methods. This report is exclusively for the use of the client indicated above and shall not be reproduced except in full without the written consent of our company. Test results transmitted herein are only applicable to the actual samples tested at the location(s) referenced and are not necessarily indicative of the properties of other apparently similar or identical materials.

21239 FM529 Rd., Bldg F
Cypress, TX 77433
Office: 291-985-9344
info@geothermusa.com
http://www.geothermusa.com

April 29, 2025

Terracon
23041 Avenida de la Carlota, Suite 350
Laguna Hills, CA 92653
Attn: Victor Nguyen

Thermal Resistivity Report
Grace Solar – Blythe, CA (Project No. 60225164)

The following is the report of thermal dryout characterization tests conducted on seven (7) native soil samples from the referenced project sent to our laboratory.

Thermal Resistivity Tests: The samples were tested at either the 'optimum' or 'as received' moisture content and at the 'specified' Proctor dry density ***provided by Terracon***. The tests were conducted in accordance with the **IEEE standard 442-2017**. The results are tabulated below and the thermal dry out curves are presented in **Figures 1 to 4**.

Sample ID, Description, Thermal Resistivity, Moisture Content and Density

Sample ID	Depth (ft)	Effort (%)	Description (Terracon)	Thermal Resistivity (°C-cm/W)		Moisture Content (%)	Dry Density (lb/ft^3)
				Wet	Dry		
BESS-02	1 - 4	85	---	54	150	7	114
BESS-02	1 - 4	95	---	49	114	7	127
BESS-02	4 - 6	90	---	51	135	8	117
BESS-08	1 - 4	85	---	57	158	8	112
BESS-08	1 - 4	95	---	51	120	8	125
BESS-08	4 - 6	90	---	53	131	7	118
BESS-09	1 - 4	85	---	60	190	9	109
BESS-09	1 - 4	95	---	54	144	9	122

Geotherm

Sample ID, Description, Thermal Resistivity, Moisture Content and Density

Sample ID	Depth (ft)	Effort (%)	Description (Terracon)	Thermal Resistivity (°C-cm/W)		Moisture Content (%)	Dry Density (lb/ft^3)
				Wet	Dry		
TR-UG1	1 - 4	85	---	69	254	11	98
TR-UG1	1 - 4	95	---	62	192	11	109
TR-UG1	4 - 6	90	---	64	196	12	101

Comments: The thermal characteristic depicted in the dryout curves apply for the soils at their respective test dry density.

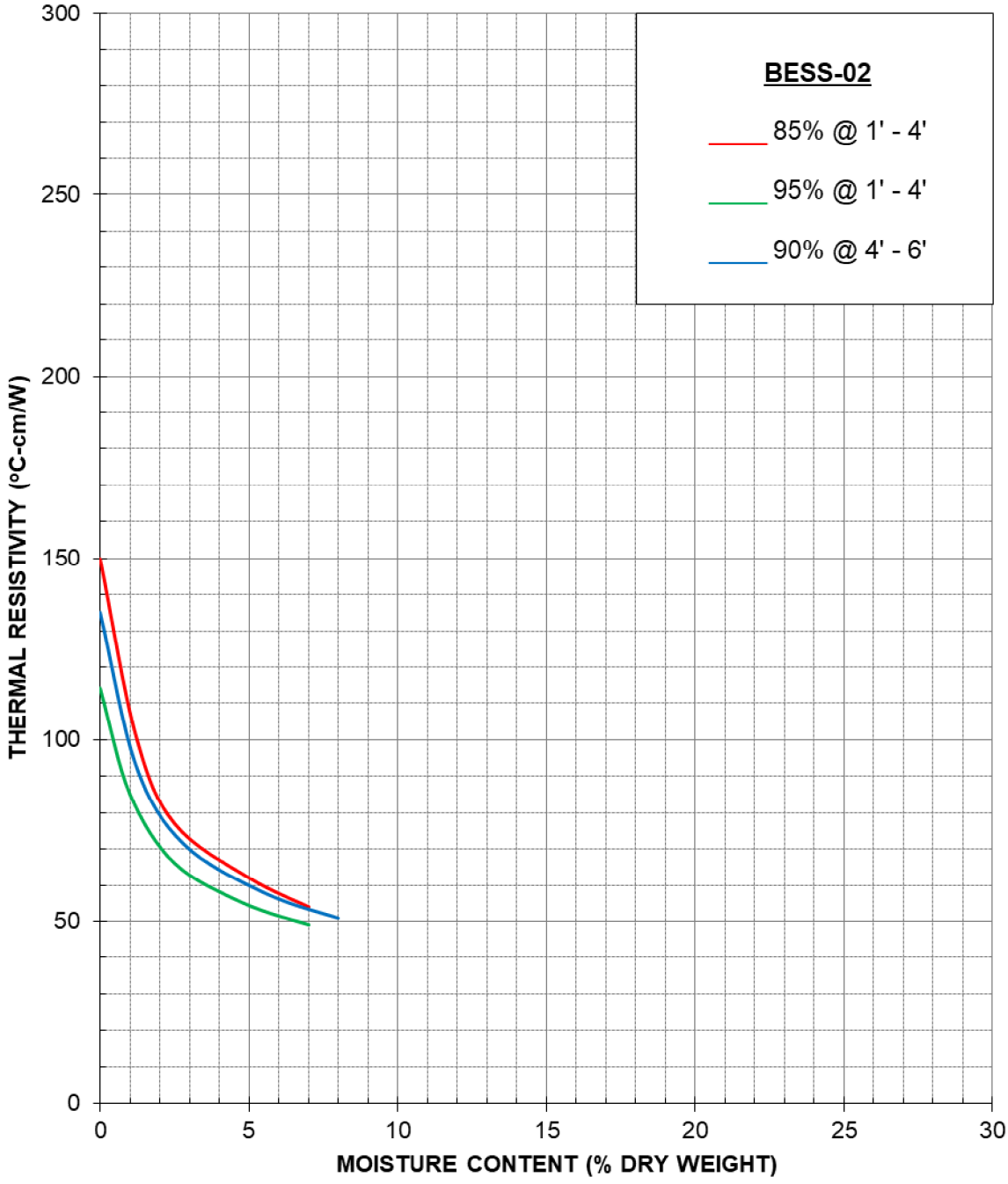
Please contact us if you have any questions or if we can be of further assistance.

Geotherm USA, LLC

Nimesh Patel

Geotherm

THERMAL DRYOUT CURVES



Terracon (Project No. 60225164)

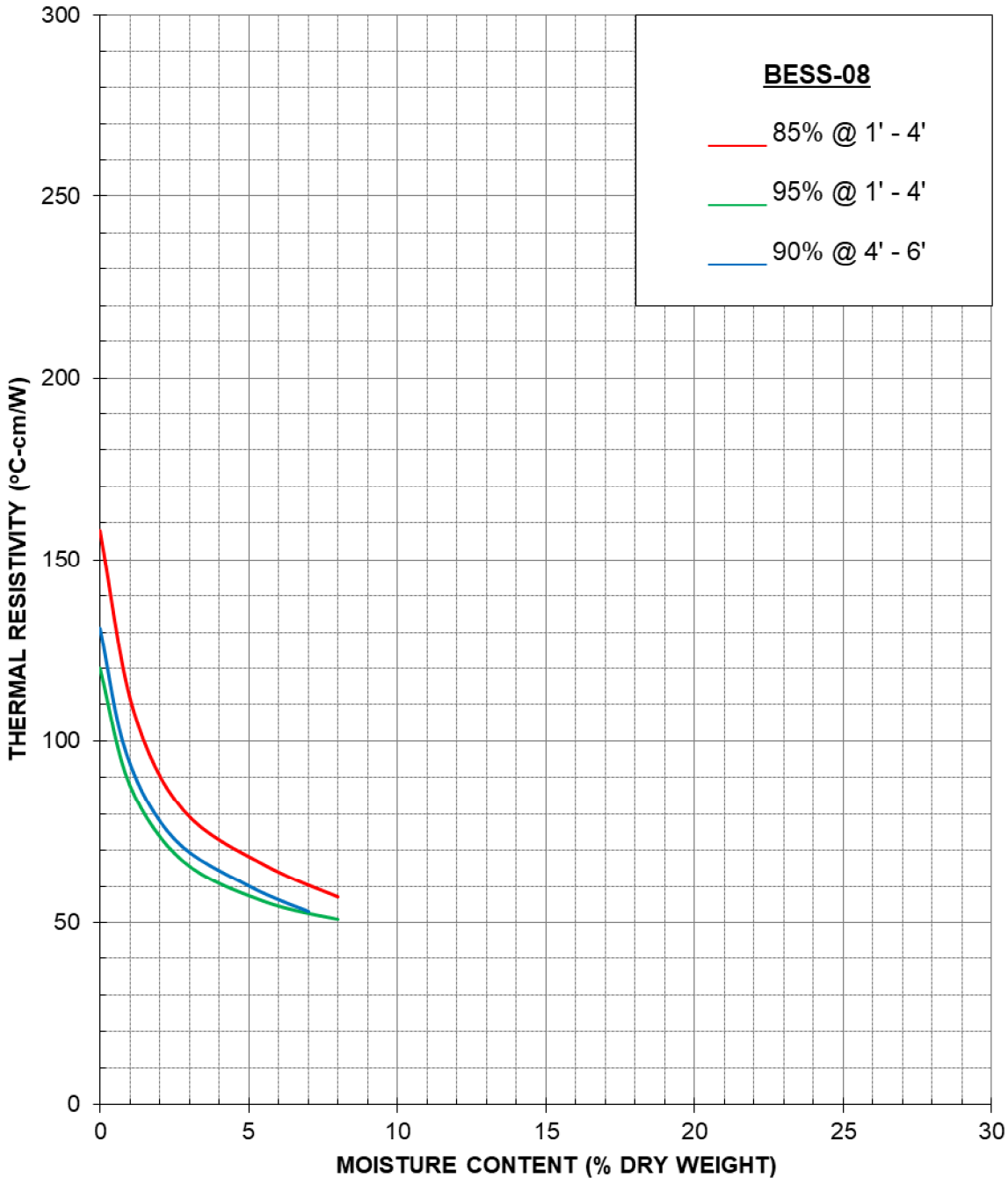
Grace Solar – Blythe, CA

Thermal Resistivity Report

April 2025 **Figure 1**

Geotherm

THERMAL DRYOUT CURVES



Terracon (Project No. 60225164)

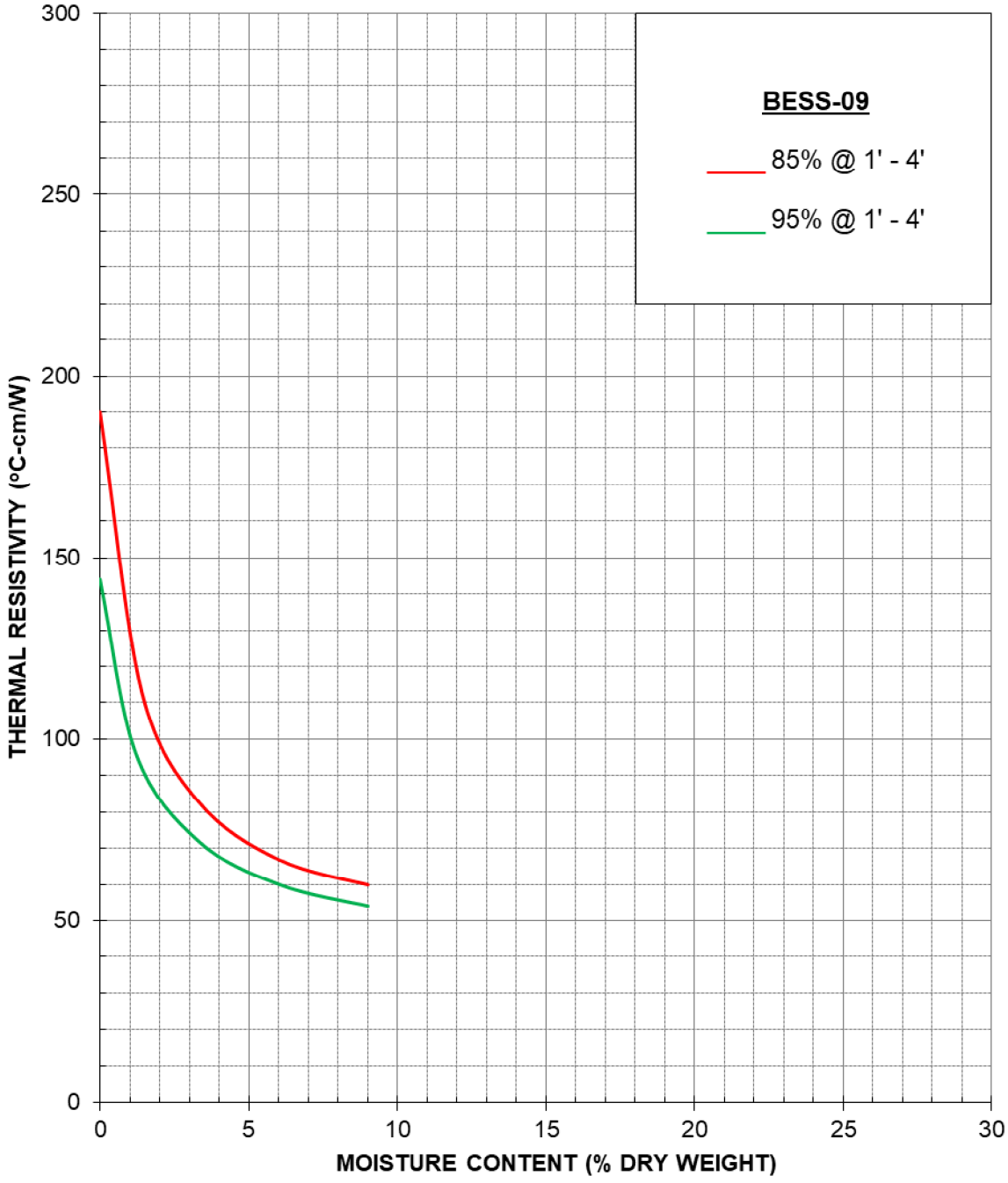
Grace Solar – Blythe, CA

Thermal Resistivity Report

April 2025 **Figure 2**

Geotherm

THERMAL DRYOUT CURVES



Terracon (Project No. 60225164)

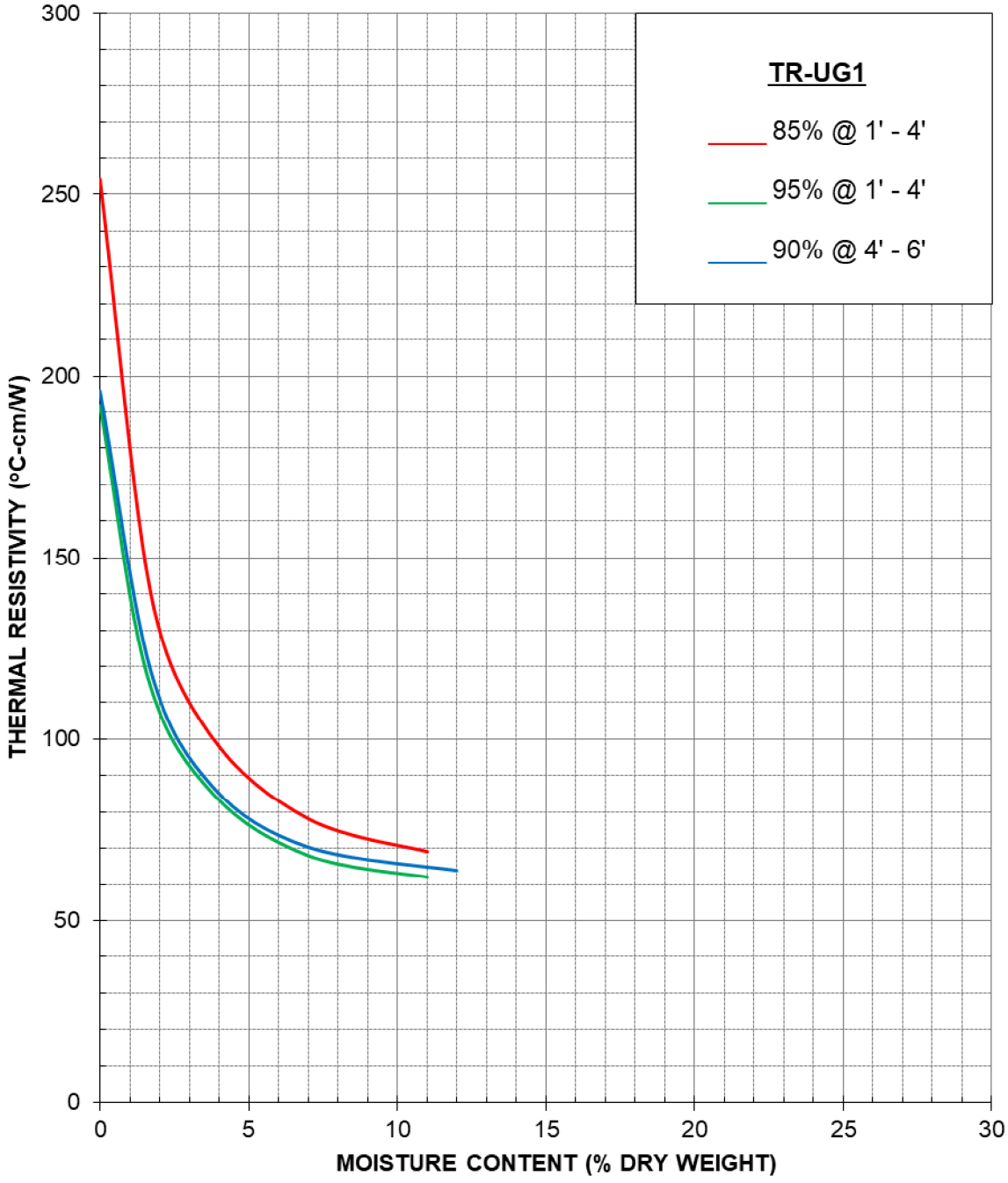
Grace Solar – Blythe, CA

Thermal Resistivity Report

April 2025 **Figure 3**

Geotherm

THERMAL DRYOUT CURVES



Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Resistivity Report

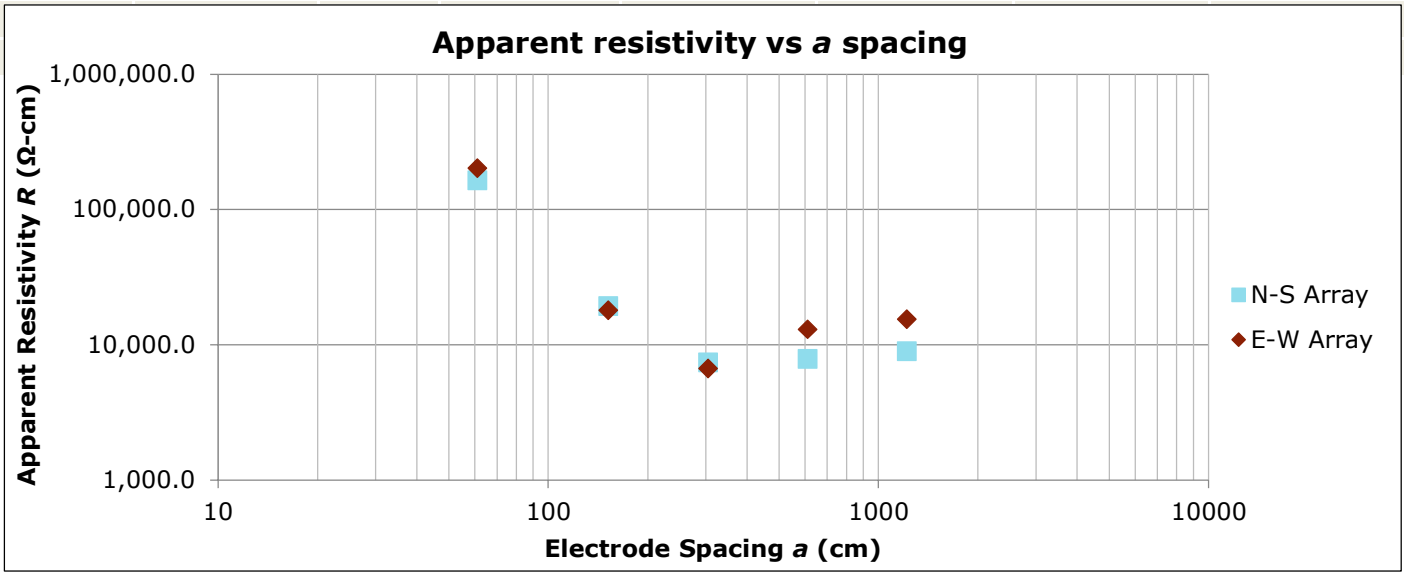
April 2025 **Figure 4**

FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace Solar - Transmission Line | Blythe, Riverside County, California
Terracon Project No. 60225164

Array Loc.	TER-1 (33.5874, -114.8181)		
Instrument	TerraMeter	**Weather**	Sunny
Serial #	27545004	**Ground Cond.**	Soil
Calib. Due	August 28, 2025	**Tested By**	AT/EA
Test Date	March 5, 2025	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Electrode depths were increased at shorter spacings to achieve more conductivity in order to obtain higher quality data.		

Apparent resistivity ρ is calculated as : $$\rho = \frac{4\pi a R}{1 + \frac{2a}{\sqrt{a^2 + 4b^2}} - \frac{a}{\sqrt{a^2 + b^2}}}$$

Electrode Spacing *a*		Electrode Depth *b*		N-S Test		E-W Test	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
2	61	2	5	424	164,400	521	202,010
5	152	3	8	20	19,390	19	18,040
10	305	6	15	3.9	7,410	3.5	6,680
20	610	12	30	2.1	7,890	3.4	13,050
40	1219	12	30	1.2	8,970	2.0	15,490



Supporting Information

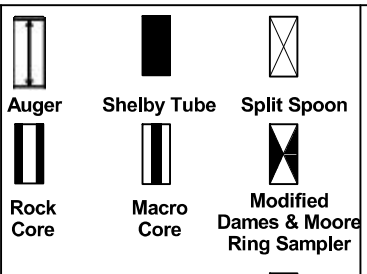
Contents:

General Notes
Unified Soil Classification System

GENERAL NOTES

DESCRIPTION OF SYMBOLS AND ABBREVIATIONS

SAMPLING



WATER LEVEL



Water levels indicated on the soil boring logs are the levels measured in the borehole at the times indicated. Groundwater level variations will occur over time. In low permeability soils, accurate determination of groundwater levels is not possible with short term water level observations.

FIELD TESTS

(HP)	Hand Penetrometer
(T)	Torvane
(b/f)	Standard Penetration Test (blows per foot)
N	N value
(PID)	Photo-Ionization Detector
(OVA)	Organic Vapor Analyzer
(WOH)	Weight of Hammer

DESCRIPTIVE SOIL CLASSIFICATION

Soil classification is based on the Unified Soil Classification System. Coarse Grained Soils have more than 50% of their dry weight retained on a #200 sieve; their principal descriptors are: boulders, cobbles, gravel or sand. Fine Grained Soils have less than 50% of their dry weight retained on a #200 sieve; they are principally described as clays if they are plastic, and silts if they are slightly plastic or non-plastic. Major constituents may be added as modifiers and minor constituents may be added according to the relative proportions based on grain size. In addition to gradation, coarse-grained soils are defined on the basis of their in-place relative density and fine-grained soils on the basis of their consistency.

LOCATION AND ELEVATION NOTES

Unless otherwise noted, Latitude and Longitude are approximately determined using a hand-held GPS device. The accuracy of such devices is variable. Surface elevation data annotated with +/- indicates that no actual topographical survey was conducted to confirm the surface elevation. Instead, the surface elevation was approximately determined from topographic maps of the area.

STRENGTH TERMS

RELATIVE DENSITY OF COARSE-GRAINED SOILS (More than 50% retained on No. 200 sieve.) Density determined by Standard Penetration Resistance Includes gravels, sands and silts.			CONSISTENCY OF FINE-GRAINED SOILS (50% or more passing the No. 200 sieve.) Consistency determined by laboratory shear strength testing, field visual-manual procedures or standard penetration resistance			
Descriptive Term (Density)	**Standard Penetration or N-Value Blows/Ft.**	**Ring Sampler Blows/Ft.**	**Descriptive Term (Consistency)**	**Unconfined Compressive Strength, Qu, psf**	**Standard Penetration or N-Value Blows/Ft.**	**Ring Sampler Blows/Ft.**
Very Loose	0 - 3	0 - 6	Very Soft	less than 500	0 - 1	< 3
Loose	4 - 9	7 - 18	Soft	500 to 1,000	2 - 4	3 - 4
Medium Dense	10 - 29	19 - 58	Medium-Stiff	1,000 to 2,000	4 - 8	5 - 9
Dense	30 - 50	59 - 98	Stiff	2,000 to 4,000	8 - 15	10 - 18
Very Dense	> 50	≥ 99	Very Stiff	4,000 to 8,000	15 - 30	19 - 42
			Hard	> 8,000	> 30	> 42

RELATIVE PROPORTIONS OF SAND AND GRAVEL

Descriptive Term(s) of other constituents	Percent of Dry Weight
Trace	< 15
With	15 - 29
Modifier	> 30

GRAIN SIZE TERMINOLOGY

Major Component of Sample	Particle Size
Boulders	Over 12 in. (300 mm)
Cobbles	12 in. to 3 in. (300mm to 75mm)
Gravel	3 in. to #4 sieve (75mm to 4.75 mm)
Sand	#4 to #200 sieve (4.75mm to 0.075mm
Silt or Clay	Passing #200 sieve (0.075mm)

RELATIVE PROPORTIONS OF FINES

Descriptive Term(s) of other constituents	Percent of Dry Weight
Trace	< 5
With	5 - 12
Modifier	> 12

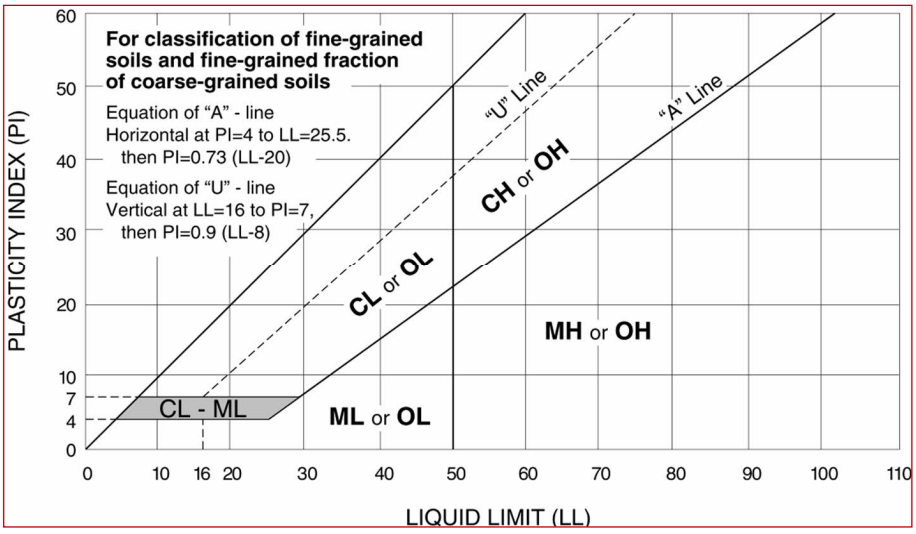
PLASTICITY DESCRIPTION

Term	Plasticity Index
Non-plastic	0
Low	1 - 10
Medium	11 - 30
High	> 30

Unified Soil Classification System

Criteria for Assigning Group Symbols and Group Names Using Laboratory Tests [A]				Soil Classification	
				Group Symbol	Group Name [B]
Coarse-Grained Soils: More than 50% retained on No. 4 sieve	**Gravels:** More than 50% of coarse fraction retained on No. 4 sieve	**Clean Gravels:** Less than 5% fines [C]	Cu≥4 and 1≤Cc≤3 [E]	GW	Well-graded gravel [F]
			Cu<4 and/or [Cc<1 or Cc>3.0] [E]	GP	Poorly graded gravel [F]
		Gravels with Fines: More than 12% fines [C]	Fines classify as ML or MH	GM	Silty gravel [F, G, H]
			Fines classify as CL or CH	GC	Clayey gravel [F, G, H]
	Sands: 50% or more of coarse fraction passes No. 4 sieve	**Clean Sands:** Less than 5% fines [D]	Cu≥6 and 1≤Cc≤3 [E]	SW	Well-graded sand [I]
			Cu<6 and/or [Cc<1 or Cc>3.0] [E]	SP	Poorly graded sand [I]
		Sands with Fines: More than 12% fines [D]	Fines classify as ML or MH	SM	Silty sand [G, H, I]
			Fines classify as CL or CH	SC	Clayey sand [G, H, I]
Fine-Grained Soils: 50% or more passes the No. 200 sieve	**Silts and Clays:** Liquid limit less than 50	**Inorganic:**	PI > 7 and plots above "A" line [J]	CL	Lean clay [K, L, M]
			PI < 4 or plots below "A" line [J]	ML	Silt [K, L, M]
		Organic:	$\frac{LL\ oven\ dried}{LL\ not\ dried} < 0.75$	OL	Organic clay [K, L, M, N]
					Organic silt [K, L, M, O]
	Silts and Clays: Liquid limit 50 or more	**Inorganic:**	PI plots on or above "A" line	CH	Fat clay [K, L, M]
			PI plots below "A" line	MH	Elastic silt [K, L, M]
		Organic:	$\frac{LL\ oven\ dried}{LL\ not\ dried} < 0.75$	OH	Organic clay [K, L, M, P]
					Organic silt [K, L, M, Q]
Highly organic soils:	Primarily organic matter, dark in color, and organic odor			PT	Peat

[A] Based on the material passing the 3-inch (75-mm) sieve.
[B] If field sample contained cobbles or boulders, or both, add "with cobbles or boulders, or both" to group name.
[C] Gravels with 5 to 12% fines require dual symbols: GW-GM well-graded gravel with silt, GW-GC well-graded gravel with clay, GP-GM poorly graded gravel with silt, GP-GC poorly graded gravel with clay.
[D] Sands with 5 to 12% fines require dual symbols: SW-SM well-graded sand with silt, SW-SC well-graded sand with clay, SP-SM poorly graded sand with silt, SP-SC poorly graded sand with clay.
[E] $Cu = D_{60}/D_{10}$ $\quad Cc = \frac{(D_{30})^2}{D_{10} \times D_{60}}$
[F] If soil contains ≥ 15% sand, add "with sand" to group name.
[G] If fines classify as CL-ML, use dual symbol GC-GM, or SC-SM.
[H] If fines are organic, add "with organic fines" to group name.
[I] If soil contains ≥ 15% gravel, add "with gravel" to group name.
[J] If Atterberg limits plot in shaded area, soil is a CL-ML, silty clay.
[K] If soil contains 15 to 29% plus No. 200, add "with sand" or "with gravel," whichever is predominant.
[L] If soil contains ≥ 30% plus No. 200 predominantly sand, add "sandy" to group name.
[M] If soil contains ≥ 30% plus No. 200, predominantly gravel, add "gravelly" to group name.
[N] PI ≥ 4 and plots on or above "A" line.
[O] PI < 4 or plots below "A" line.
[P] PI plots on or above "A" line.
[Q] PI plots below "A" line.



Appendix B-3

Grace Solar BESS and Substation Geotechnical Engineering and Pile Load Testing Report - Rev. 1

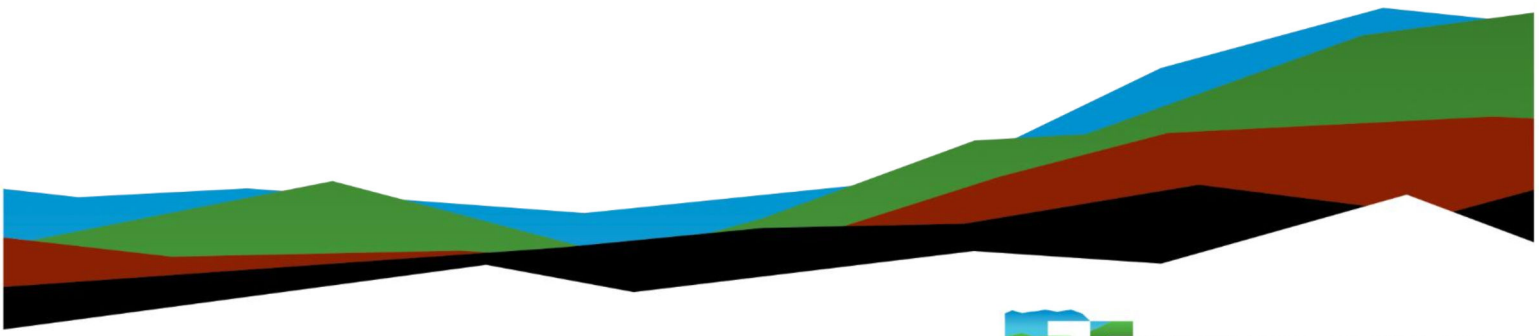
Grace Solar BESS and Substation

Geotechnical Engineering and Pile Load Testing Report – Rev. 1

October 8, 2025 | Terracon Project No. 60225164

Prepared for:

NextEra Energy Resources, LLC
700 Universe Boulevard
Juno Beach, Florida 33408



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October 8, 2025

NextEra Energy Resources, LLC
700 Universe Boulevard
Juno Beach, Florida 33408

Attn: Mr. Alejandro Velasco
P: (954) 540-7513
E: Alejandro.velasco@nexteraenergy.com

Re: Final Geotechnical Engineering and Pile Load Testing Report – Rev. 1
Grace Solar BESS and Substation
Blythe, Riverside County, CA
Terracon Project No. 60225164

Dear Mr. Velasco:

We have completed the scope of services for the above-referenced project in general accordance with *Exhibit A: Project Information, Scope of Work, and Specifications, Battery Energy Storage System (BESS), Final Geotechnical Engineering Services (Stand Alone BESS with or w/o Substation), for Grace Energy Storage, Riverside County, CA, dated November 5, 2024*. This report presents the findings of the subsurface exploration and provides geotechnical recommendations concerning earthwork and the design and construction of foundations, unpaved roads, and ancillary structures for the proposed BESS and Substation project.

We appreciate the opportunity to be of service to you on this project. If you have any questions concerning this report or if we may be of further service, please contact us.

Sincerely,

Terracon

Phillip Penrose, P.E.
Project Engineer

Victor V. Nguyen, P.E.
Project Engineer



SME Review by: Jimmy Jackson, P.E. (FL)



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Attachments

Field Exploration Results

- Exploration and Testing Procedures
- Site Location Plan
- Exploration Location Plan
- General Notes
- Unified Soil Classification System
- Geomodel
- Boring Logs (BESS-01 through BESS-15, PERC-01 through PERC-11 and SUB-1 through SUB-7)
- Test Pit Logs (BTP-1 to BTP-4, STP-2 to STP-3)

Laboratory Test Results

- Laboratory Testing Procedures
- Grain Size Distribution
- Atterberg Limits
- Swell/Collapse Potential
- Moisture Density Relationship
- CBR Test Results
- Corrosion Test Results
- Thermal Resistivity Test Results

Field Soil Electrical Resistivity Results

- Electrical Resistivity Test Procedures
- Electrical Resistivity Test Locations Plan
- Electrical Resistivity Test Results

Test Pile Driving Data

- Test Pile Installation Details
- Pile Load Test Locations Plan
- Test Pile Driving Records

Pile Load Test Results

- Load Test Procedure Details
- Axial Tension Test Results
- Lateral Test Results
- Axial Compression Test Results

Note: This report was originally delivered in a web-based format. **Blue Bold** text in the report indicates a referenced section heading. The PDF version also includes hyperlinks which direct the reader to that section and clicking on the Terracon logo will bring you back to this page. For more interactive features, please view your project online at **client.terracon.com**.

Geohazards

Item	Overview Statement [1]
Pile Drivability	Pile driving refusal was not encountered during our pile driving program. However, based on the near surface dense to very dense sands and gravels encountered at most of the explored locations, pile drivability may be difficult for portions of the site. Pile refusal may occur prior to planned embedment depths.
Shallow Bedrock	Shallow bedrock was not encountered at the site.
Frost Potential	The frost potential is considered negligible in this area.
Shallow Groundwater	Shallow groundwater was not during our field exploration.
Liquefaction and Related Hazards	Based on review of the Riverside County GIS, the site is located within "Moderate" severity liquefaction hazard zone as designated by the County of Riverside. However, based on the anticipated depth to historic groundwater and encountered subsurface conditions, liquefaction hazard potential at the site is considered to be low. Other hazards related to liquefaction, such as lateral spreading, are also considered low.
Karst	Karst is not a concern at this site.
Expansive Soils	Expansive soils were not observed at the site.
Collapsible Soils	Near-surface collapsible soils were encountered at the site within the BESS and substation areas. Mitigative measures including over-excavation and re-compaction have been provided in the text of this report to support shallow concrete foundations. Collapsible soils are not anticipated to adversely affect performance of driven steel piles or drilled piers.

1. This summary is for convenience only. It should be used in conjunction with the entire report for design purposes.

Preliminary Embedment Depth Analysis

The following top-of-pile load was considered to determine the embedment depth:

- Compressive load: 15,000 pounds

The actual top-of-pile structural loads will vary based on the BESS container and the manufacturer's load information as determined in accordance with requirements by the applicable building codes and local municipality.

Our analyses have been performed using preliminary information and are intended to assist you in roughly evaluating construction costs and viability for the proposed project. Ultimately, the design of foundations for support of BESS containers will depend on a number of factors including the actual structural loading conditions, anticipated corrosion losses, and other factors where complete and final information is not available at this time.

PEA Results	
Zone	**Embedment Depth of W6x15 Pile (feet-bgs)**
Entire Site	9.5

Based on the results of the PEA, we expect that a typical GAYK HRE 4000 pile driver will be suitable for pile installation at this site.

Introduction

This report presents the results of our subsurface exploration and services performed for the proposed battery energy storage system (BESS) and substation facility to be located near Blythe, Riverside County, CA. The purpose of these services was to provide information and geotechnical engineering recommendations relative to the proposed BESS and substation development.

The geotechnical engineering Scope of Services for this project included the advancement of test borings, field electrical resistivity, laboratory thermal resistivity testing, laboratory corrosion testing, soil index property and strength laboratory testing, engineering analysis, and preparation of this report.

The following hyperlinks can be used to quickly navigate to the location plans, field data, and laboratory data completed for this report:

- **Field Exploration Results** (includes exploration plans, boring logs, and test pit logs)
- **Laboratory Test Results** (includes the classification, Proctor, strength test, corrosion series, and thermal resistivity tests)
- **Field Soil Electrical Resistivity Data** (includes test procedures, locations, and individual electrical resistivity test data)
- **Test Pile Driving Data** (includes test procedures, locations, and test pile driving records
- **Pile Load Test Results** (includes test procedures and pile load test results)

Project Description

Our initial understanding of the project was provided in our proposal and was discussed during project planning. A period of collaboration has transpired since the project was initiated, and our final understanding of the project conditions is as follows:

Item	Description
Information Provided	▪ Exhibit A: Project Information, Scope of Work, and Specifications, dated 11/5/2024 ▪ Grace SS with Geotech Testing 2024-11-01.kmz
Project Description	It is our understanding that the Client intends to develop a 500Mw/4hr BESS with an approximate footprint of 25 acres. A substation and stormwater management area are also planned as part of the BESS development.

Item	Description
Proposed Structures	We understand the proposed BESS and substation structures will be supported by driven steel piles, drilled shafts, mat foundations, or grade beams.
Maximum Loads	We have been provided the following foundation loads for the project: BESS Containers: ■ Total weight: 100 kips ■ Maximum axial compressive load on an individual pile: 15 kips Inverters: ■ 40 kips Generator Step-Up (GSU): ■ 600 kips
Grading/Slopes	Grading and/or site plans were not provided at this stage of the project. However, it is anticipated that the site work involves cuts and fills within +/- 2 feet of existing grade.
Access Roads	Unpaved access roads are planned for the site as described below: ■ Access roads are to support post-construction traffic which we understand will be primarily light maintenance vehicles. The roads will be required to support a maximum vehicle load of 80,000 pounds for fire truck access. Additionally, the substation access road should be able to support heavy vehicle delivery (HS-20 loading) up to two times per year throughout the design life. ■ We understand it is acceptable for the access roads to require ongoing maintenance throughout their design life.

Terracon should be notified if any of the above information is inconsistent with the planned construction, as modifications to our recommendations may be necessary.

Site Conditions

The following description of site conditions is derived from our site visit in association with the field exploration and our review of publicly available geologic and topographic maps.

Item	Description
Parcel Information	The project site is located near Blythe, Riverside County, California and is approximately 50 acres in total size. Coordinates near the approximate center of the substation are: 33.6486° N, 114.7208° W. See **Site Location**
Existing Improvements	The site is primarily undeveloped. Nearby features include unpaved access roads, solar array fields to the northwest and east of the site, and the Blythe Airport south of the site.
Current Ground Cover	The majority of the surface of the site is covered by exposed soils with sparse desert vegetation.
Existing Topography	Site-specific topographic map was not available at the time of this report. Based on review of publicly available topographic data from Google Earth Pro™, elevations within the project area range approximately from 402 to 415 feet MSL and generally decreases in elevation towards the southeast direction.

Geotechnical Characterization

Site Geology

The site is located in the eastern portion of the Mojave Desert geomorphic province, a broad interior region characterized by isolated mountains separated by expanses of desert plains that include playa lakes. The site region is arid, and topography includes hills with dissected older alluvial aprons and younger sediments as alluvial plains. Alluvium in the site area is derived from adjacent highlands (McCoy Mountains) and includes Pleistocene age sediments on Palo Verde Mesa and younger recent wash deposits. The Colorado River is approximately 11 miles to the east.

Surficial geologic materials are mapped as recent alluvium of valley plains and wash areas and Pleistocene age alluvium of Palo Verde Mesa that underlie the majority of the site (Stone, 2006)[1]. Materials encountered in site explorations include gravel layers and caliche horizons (an accumulation of carbonate derived from precipitation) typical of the

[1] Stone, Paul, 2006, Geologic map of the west half of the Blythe 30' by 60' quadrangle, Riverside County, California and La Paz County, Arizona, U.S. Geological Survey, Scientific Investigations Map SIM-2922, 1:100,000

arid Mojave environment. Caliche-rich and reddish soils typically reflect pre-Holocene age sediments consistent with published geologic mapping.

As encountered in site explorations, soils materials include silty sand, gravelly sands, poorly- to well-graded sands and locally silt layers. These sediments are interpreted as locally less dense, disturbed surficial materials derived from and mantling older, more dense materials, consistent with the mapped older alluvial deposits.

Soil Survey

The Soil Survey for the Palo Verde Area of Riverside County, California, as prepared by the United States Department of Agriculture (USDA), Natural Resource Conservation Service - NRCS, identifies almost the entire project area (>95%) being Typic Haplocambids-Snaggletooth complex 0% to 2% slopes. The soil survey presents shallow (typically upper 80 inches) soil stratification information.

The Typic Haplocambids-Snaggletooth complex is classified as moderately well to well drained, with depth to restrictive feature more than 80 inches, and calcium carbonate content up to 15%. These soils types are typical of the arid desert environment.

It should be noted that the NRCS Soil Survey is not intended as a substitute for site-specific geotechnical exploration; rather it is a useful tool in planning a project scope in that it provides information relative to the soil types likely to be encountered. Boundaries between adjacent soil types on the NRSC Soil Survey maps are approximate. In general, the shallow subsurface conditions identified in the borings conducted for this project generally agree with the NRCS Soil Survey.

Exploration Results

We have developed a general characterization of the subsurface conditions based upon our review of the subsurface exploration, laboratory data, geologic setting, and our understanding of the project.

As part of our analyses, we identified the following model layers within the subsurface profile. For a more detailed view of the model layer depths at each boring location, refer to the GeoModel.

Model Layer	Layer Name	General Description
1	**Sand**	Medium dense to very dense sand with varying amounts of silt, clay, and gravel
2	**Gravel**	Medium dense to very dense gravel with varying amounts of silt and sand

Conditions observed at each exploration point are indicated on the individual logs. The individual logs can be found in the **Exploration Results** attachment of this report.

Groundwater

The site is located within the Palo Verde Mesa Groundwater Basin. The basin is bounded by mountains to the north and west and is drained by McCoy Wash that trends near the northeast portion of the site and drains to the Colorado River. According to the State of California Water Data Library (WDL) – Groundwater Data website, groundwater is deeper than 150 feet bgs in the site area.

Groundwater was not observed in the borings while drilling, or for the short duration the borings remained open. Groundwater is not anticipated to occur within the depth of excavations or foundation installations at the site.

These observations represent groundwater conditions at the time of the field exploration and may not be indicative of other times, or at other locations. Groundwater conditions may change because of seasonal variations in rainfall, runoff, and other conditions not apparent at the time of exploration. Therefore, groundwater levels during construction or at other times may be higher or lower than expected.

Field Electrical Resistivity

Field measurements of soil electrical resistivity were performed by Terracon between March 3 to March 5, 2025. The location and results of the field electrical resistivity testing are included in the **Field Soil Electrical Resistivity Test Results** section of this report.

Laboratory Thermal Resistivity

Nineteen laboratory thermal resistivity tests were completed on remolded soil samples. Undisturbed thermal tests were not conducted due to the sandy conditions encountered and the high potential for disturbance during long distance transport to the GeoTherm testing lab. In lieu of this, an additional remolded thermal test was conducted on bulk samples at approximate depths of 4 to 6 feet bgs. The individual laboratory thermal resistivity dry-out curves are provided in the **Laboratory Test Results** section of this report and are summarized below. The compaction percentages shown in the table are relative to the soil's Modified Proctor (ASTM D1557) maximum dry density.

Thermal Resistivity Test Results Summary					
Test Condition	**Thermal Resistivity (°C-cm/W)**				**Number of Tests**
	Parameter[1, 2]	**Min.**	**Max.**	**Average**	
Remolded to 85%	Remolded Wet	53	72	61.3	7
	Remolded Dry	140	190	165.3	
Remolded to 90%	Remolded Wet	45	60	53.2	6
	Remolded Dry	110	278	170	
Remolded to 95%	Remolded Wet	49	65	54.8	6
	Remolded Dry	111	144	126.8	

1. The "Remolded Wet" samples were tested near the higher of either the optimum or in-situ moisture content.
2. The "Dry" samples were tested at a moisture content of near 0%.

Laboratory Corrosion Testing

The table below lists a summary of the results of laboratory pH, soluble sulfate, sulfides, soluble chloride, total salts, oxidation-reduction potential (redox), and electrical resistivity testing. The values may be used to estimate potential corrosive characteristics of the on-site soils with respect to contact with the various underground materials which will be used for project construction.

Corrosivity Test Results Summary				
Parameter	**Number of Tests**	**Min.**	**Max.**	**Average**
pH	5	7.45	9.38	8.42
Soluble Sulfate (percent by weight)	5	0.01	0.05	0.034

Corrosivity Test Results Summary				
Parameter	**Number of Tests**	**Min.**	**Max.**	**Average**
Sulfides (mg/kg)	5	Nil	1.02	0.34
Soluble Chloride (mg/kg)	5	19	325	185
Total Salts (mg/kg)	5	374	1232	711
Redox (mV)	5	+174	+727	+504
Electrical Resistivity (Ω-cm)	5	332	3149	1146

Results of soluble sulfate testing can be classified in accordance with ACI 318 – Building Code Requirements for Structural Concrete. Based on ACI 318 the water-soluble sulfates in the soils tested is negligible. Numerous sources are available to characterize corrosion potential to buried metals using the parameters above. Section 10.7.5 of the AASHTO LRFD Bridge Manual, 8th Edition, 2017, states the following soil or site conditions should be considered as indicative of potential deterioration or corrosion situation for steel piles:

- Soil electrical resistivity less than 2,000 ohm-cm
- pH less than 5.5
- pH between 5.5 and 8.5 with high organic content
- Sulfate concentration greater than 1,000 ppm mg/kg (0.1 percent)

These test results are provided to assist in determining the type and degree of corrosion protection that may be required. We recommend that a NACE certified corrosion professional be retained to analyze the need for corrosion protection and to design appropriate protective measures, if required.

Imported fill materials may have significantly different properties than the site materials noted above and should be evaluated if expected to be in contact with metals used for construction.

Seismic Site Class

The 2022 California Building Code (CBC) Seismic Design Parameters have been generated using the SEAOC/OSHPD Seismic Design Maps Tool. This web-based software

application calculates seismic design parameters in accordance with ASCE 7-16 and 2022 CBC. The 2022 CBC requires that a site-specific ground motion study be performed in accordance with Section 11.4.8 of ASCE 7-16 for Site Class D sites with a mapped S_1 value greater than or equal 0.2.

However, Section 11.4.8 of ASCE 7-16 includes an exception from such analysis for specific structures on Site Class D sites. The commentary for Section 11 of ASCE 7-16 (Page 534 of Section C11 of ASCE 7-16) states that "In general, this exception effectively limits the requirements for site-specific hazard analysis to very tall and or flexible structures at Site Class D sites." Based on our understanding of the proposed structures, it is our assumption that the exception in Section 11.4.8 applies to the proposed structure. However, the structural engineer should verify the applicability of this exception.

Based on this exception, the spectral response accelerations presented below were calculated using the site coefficients (F_a and F_v) from Tables 1613.2.3(1) and 1613.2.3(2) presented in Section 16.4.4 of 2022 CBC.

Description	Value
2022 California Building Code Site Classification (CBC) [1]	D [2]
Site Latitude (°N) [3]	33.6486
Site Longitude (°W) [3]	114.7208
S_s Spectral Acceleration for a 0.2-Second Period	0.325
S_1 Spectral Acceleration for a 1-Second Period	0.165
F_a Site Coefficient for a 0.2-Second Period	1.54
F_v Site Coefficient for a 1-Second Period	2.27
S_{DS} Spectral Response Acceleration Parameter at Short Periods	0.334
S_{D1} Spectral Response Acceleration Parameter at 1-Second Period	0.250
Site Modified Peak Ground Acceleration PGA_M	0.22 g

Description	Value

1. Seismic site classification in general accordance with the *2022 California Building Code*.
2. The 2022 California Building Code (CBC) requires a site soil profile determination extending to a depth of 100 feet for seismic site classification. The current scope does not include the required 100-foot soil profile determination. Borings were extended to a maximum depth of 51½ feet, and this seismic site class definition considers that similar soils continue below the maximum depth of the subsurface exploration. Additional exploration to deeper depths would be required to confirm the conditions below the current depth of exploration.
3. Coordinates were centered around the proposed substation facility.

Typically, a site-specific ground motion study may generate less conservative coefficients and acceleration values which may reduce construction costs.

Faulting and Estimated Ground Motions

The site is located in eastern California, which is a region of moderate seismic activity. The type and magnitude of seismic hazards affecting the site are dependent on the distance to causative faults, the intensity, and the magnitude of the seismic event. As estimated using the USGS Unified Hazard Tool, the San Andreas fault and Brawley Seismic zone are the closest major faults to the site at a distance of approximately 60 miles. However, the majority of seismic hazard is 'grid based' background seismicity associated with non-fault sources in the seismic hazard model.

Based on the USGS Design Maps Summary Report, using the American Society of Civil Engineers (ASCE 7-16) standard, the peak ground acceleration (PGA_M) for design at the project site is 0.22 g. Based on the USGS Earthquake Hazard Toolbox Disaggregation Tool, the project site has a mean magnitude of 7.06. Furthermore, the site is not located within an Alquist-Priolo Earthquake Fault Zone based on our review of the State Fault Hazard Maps and County of Riverside Fault Hazard Maps.[2]

[2] California Geological Survey (CGS), https://maps.conservation.ca.gov/cgs/informationwarehouse/regulatorymaps/

Liquefaction

Liquefaction is a mode of ground failure that results from the generation of high porewater pressures during earthquake ground shaking, causing loss of shear strength. Liquefaction is typically a hazard where loose sandy soils exist below groundwater. The California Geological Survey (CGS) has designated certain areas as potential liquefaction hazard zones. These are areas considered at a risk of liquefaction-related ground failure during a seismic event, based upon mapped surficial deposits and the presence of a relatively shallow water table.

The site has not been mapped for liquefaction hazard by the California Geologic Survey. Based on review of the Riverside County GIS, the site is located within "Moderate" severity liquefaction hazard zone as designated by the County of Riverside. However, based on the anticipated depth to historic groundwater, liquefaction hazard potential at the site is considered to be low. Other geologic hazards related to liquefaction, such as lateral spreading, are therefore also considered low.

Pile Load Testing

We completed a full-scale pile load testing program that included:

- Directing the installation of a group of three test piles at six locations within the BESS Area.
- Performing full-scale testing under axial compressive loads on two (2) piles at each location (12 tests)
- Performing full-scale testing under axial tensile loads for one test pile at each location (6 tests)
- Performing full-scale testing under lateral loads on two test piles at each location (12 tests).

The piles were installed on May 15, 2025, with a track-mounted GAYK Model HRE 4000 pile driver equipped with a hydraulic hammer to embedment depths ranging from approximately 8 to 12 feet below the ground surface (bgs). The time rate of installation was recorded with a stopwatch. A summary of the total drive times and load test results is provided below. Additional installation details and pile drive time curves are included in the **Test Pile Driving Data** section.

Summary of Cumulative Pile Installation Times (W6x9)			
Embedment Depth (feet-bgs)	Drive Time (seconds)		
	Min.	Max.	Average
1	1.6	5.2	3.1
2	4.0	42.1	9.8
3	8.6	81.0	18.2
4	15.0	93.8	27.4
5	17.2	109.3	38.6
6	21.4	130.2	51.1
7	25.1	166.4	65.2
8	30.9	249.5	82.8
9	38.8	364.5	103.1
10	46.5	478.5	124.7
11	65.3	193.1	133.4
12	74.0	217.8	150.5

Summary of Pile Load Test Results

The individual pile load test results are provided in the **Pile Load Test Results** section. The following table provides a summary of each test pile location, embedment depth, total drive time, uplift load at ¼-inch of vertical displacement, lateral load at ½-inch of lateral displacement, and compressive load at ¼-inch of vertical displacement:

Pile Load Test Results Summary					
Location	Pile Emb. Depth (ft-bgs)	Total Drive Time (seconds)	Uplift Load at ¼" Displacement [1] (lbs)	Lateral Load near ½" Displacement (lbs)	Compression Load at ¼" Displacement [1] (lbs)
BPLT-1A	8	59	---	5,050	>30,000
BPLT-1B	12	110	---	6,050	>30,000
BPLT-1C	8	57	14,620	---	---

Pile Load Test Results Summary					
Location	**Pile Emb. Depth (ft-bgs)**	**Total Drive Time (seconds)**	**Uplift Load at ¼" Displacement [1] (lbs)**	**Lateral Load near ½" Displacement (lbs)**	**Compression Load at ¼" Displacement [1] (lbs)**
BPLT-2A	8	83	---	5,000	>30,000
BPLT-2B	12	124	---	>7,000	>30,000
BPLT-2C	8	62	>15,000	---	---
BPLT-3A	8	51	---	5,000	29,270
BPLT-3B	12	101	---	6,250	>30,000
BPLT-3C	8	46	>15,000	---	---
BPLT-4A	8	36	---	1,820	17,000
BPLT-4B	12	62	---	4,500	>30,000
BPLT-4C	8	37	6,250	---	---
BPLT-5A	8	61	---	5,200	>30,000
BPLT-5B	12	263	---	6,190	>30,000
BPLT-5C	8	53	>15,000	---	---
BPLT-6A	8	30	---	4,910	>30,000
BPLT-6B	12	48	---	5,000	23,500
BPLT-6C	8	32	12,710	---	---

1. > indicates that the target displacement is anticipated to occur at load greater than the maximum load that was applied during the pile load test.

Geotechnical Overview

The site appears suitable for the proposed construction based upon geotechnical conditions encountered in the test borings, provided that the findings and recommendations presented in this report are incorporated into project design and construction.

We understand the main foundation components for the BESS and substation will be supported on driven piles or drilled shafts; however, some lightly loaded inverter structures are typically required for the substation and across the site. In general, small, lightly loaded, inverter structures may be supported on driven piles or isolated mat/slab foundation systems.

Potentially compressible soils, which show significant tendency for hydro-compaction when elevated in moisture content, were encountered on site. Such soils will require particular attention in the design and construction. Hydro-compactive soils, sometimes referred to as collapsible soils, are capable of supporting typical building loads at natural moisture contents, these same materials however, undergo volume decrease, including settlement and consolidation, when subjected to increases in moisture content under constant load.

Estimated movements described in this report are based on effective drainage for the life of the structure and cannot be relied upon if effective drainage is not maintained. Exposed ground, extending at least 10 feet from the perimeter, should be sloped a minimum of 5% away from the structure to provide positive drainage away from the structure. Grades around the structure should be periodically inspected and adjusted as part of the structure's maintenance program.

Geotechnical engineering recommendations for foundation systems and other earth connected phases of the project are outlined below. The recommendations contained in this report are based upon the results of test borings, laboratory testing, engineering analyses, and our current understanding of the proposed project. The General Comments section provides an understanding of the report limitations.

Recommendations for drilled shafts and shallow foundations are described in the following sections.

Driven Steel Pile Foundations

Geotechnical Considerations

We understand driven steel piles may be used for support of battery containers and inverters. The geotechnical design parameters provided in this section assume up to about 2 feet of cut or fill earthwork will be required to grade the site. If earthwork mass grading exceeds about 2 feet, please contact us so we may modify or append our design recommendations.

Axial Capacity Recommendations

The axial uplift and compression capacity of driven piles in the BESS area may be estimated based on skin friction developed along the perimeter of the pile. When determining embedment depths, the perimeter of a wide flange beam should be taken as twice the sum of the flange width and section depth. The upper 1 foot of soil for each pile should be neglected in the axial capacity analyses. Below is a tables with recommended ultimate skin friction values:

Axial Uplift Design Parameters				Axial Compression Design Parameters		
Zone	**Pile Embedment Depth (feet-bgs)**	**Ultimate Uplift Skin Friction, $q_{s(up)}$ (psf)[1, 2]**	**Recommended Factor of Safety**	**Pile Embedment Depth (feet-bgs)**	**Ultimate Compression Skin Friction, $q_{s(comp)}$ (psf)[1, 2]**	**Recommended Factor of Safety**
BESS	1 to 8	475	1.5	1 to 8	1,300	1.5
				8 to 12	2,000	1.5
	8 to 20	475	2.0	12 to 20	2,000	2.0

1. The upper 1 foot of soil or the scour depth, whichever is deeper, should be neglected when determining the uplift or compression skin friction capacity of the pile.
2. The uplift and compression skin friction values are only applicable for piles installed to a minimum depth of 8 feet below existing grade.

The above values are to be used in the following equations to obtain the ultimate uplift or compression load capacity of a pile:

$$Q_{ult\ (compressive)} = \Sigma H \times P \times q_{s\ (comp)}$$
$$Q_{ult\ (uplift)} = \Sigma H \times P \times q_{s\ (up)}$$

Q_{ult} = Ultimate uplift or compression capacity of pile (lbs.)
H = Depth of embedment of pile (ft)
P = Box perimeter area/ft. of pile. (i.e. W6x15 = 2.00 ft.)
q_s = Skin friction per table above (psf).

An example calculation to determine the allowable compression capacity for a 15-foot embedded W6x15 pile in the BESS area is shown below:

$Q_{allowable\ (compressive)} = (8\text{-}1) \times 2.00 \times \frac{1,300}{1.5} + (12\text{-}8) \times 2.00 \times \frac{2,000}{1.5} + (15\text{-}12) \times 2.00 \times \frac{2,000}{2.0} =$ 28,800 pounds

An example calculation to determine the allowable tension capacity for a 15-foot embedded W6x15 pile in the BESS area is shown below:

$Q_{allowable\ (uplift)}$ = (8-1) x 2.00 x $\frac{475}{1.5}$ + (15-8) x 2.00 x $\frac{475}{2.0}$ = 7,758 pounds

The provided skin friction values are applicable for piles that are driven using equipment similar to a Gayk HRE 4000 pile driver with a hydraulically operated hammer.

Piles should have a minimum center-to-center spacing of at least three times their largest cross-sectional dimension to prevent reduction in the axial capacities due to group effects.

Final pile design to be completed by an engineering licensed in the State of California based upon information contained in this geotechnical report.

Lateral Capacity Recommendations

Lateral load response of pile foundations was calculated using the computer program *LPILE 2022*, by Ensoft, Inc. The stiffness of the pile and the stress-strain properties of the surrounding soils determine the lateral resistance of the foundation. We modeled the lateral response of the tested piles to evaluate *LPILE* input parameters that can be used for design of the production piles. Recommended *LPIL*E input parameters for lateral load analysis for driven pile foundations are shown in the following tables:

LPILE Parameters				
Depth Interval (feet-bgs)	**LPILE Soil Type**	**Effective Unit Weight (pcf)**	**Friction Angle (degree)**	**Soil Modulus (k) / Strain Factor (€50)**
0 to 20	Sand (Reese)	105	29	Default

The lateral load test results were varied between the different embedment depths at the site. Therefore, we are providing the following table of p-multiplier values that should be used for the corresponding embedment depth:

Minimum Pile Embedment Depth (feet-bgs)	P-Multiplier [1, 2, 3]
8	1.1
12	5.0

Minimum Pile Embedment Depth (feet-bgs)	P-Multiplier [1, 2, 3]

1. Linear interpolation should be used to determine the p-multiplier for embedment depths between those shown in this table. The calculated value should then be applied to the entire pile depth. For example, a pile installed to an embedment depth of 10 feet would have a p-multiplier of 3.05 applied from 0 to 10 feet bgs.
2. The p-multiplier values provided in this table are only applicable to piles installed to a depth of at least 8 feet.
3. The p-multiplier value for the 15-foot-deep piles may be used for piles installed deeper than 15 feet.

Lateral analyses were performed by using *LPILE* to generate a load vs. deflection curve that was generally consistent with the field load test results. The lateral load was applied at approximately 18 inches above the ground surface. The effective unit weight and friction angle were based on the results of the borings. The p-multiplier was then adjusted (by trial-and-error method) such that the applied load resulted in a deflection value that matched the load test results. Please note that this procedure was based on only one discrete set of data determined at about six inches from the ground surface during the field load testing. These results should be used for *LPILE* analysis only using the 2022 version of *LPILE*. These parameters are only applicable to piles installed a minimum of 8 feet bgs. In our evaluation, the piles were modeled as a Steel AISC Section Strong Axis.

The structural engineer should evaluate the moment capacity of the pile as part of their structural evaluation. Piles should have a minimum center-to-center spacing of at least five times their largest cross-sectional dimension in the direction of the lateral loads, or the lateral capacities should be reduced due to group effects. If piles will be spaced closer than five times their largest cross-sectional dimension, we should be notified to provide supplemental recommendations regarding resistance to lateral loads.

Construction Considerations

Pile driving refusal was not encountered during our pile driving program. However, based on the near surface dense to very dense sands and gravels encountered at most of explored locations, pile drivability may be difficult for portions of the site. Pile refusal may occur prior to planned embedment depths.

The pile driving contractor should perform production pile testing as part of a Quality Control plan. If unforeseen conditions are found during the pile installation and testing, the Geotechnical Engineer should be contacted to provide additional recommendations.

Earthwork

General

The following presents recommendations for site preparation, excavation, subgrade preparation and placement of engineered fills on the project. The recommendations presented for design and construction of earth supported elements including foundations, slabs and pavements are contingent upon following the recommendations outlined in this section. All grading for the structures should incorporate the limits of the proposed structure plus a minimum pad extension of 8 feet beyond proposed perimeter building walls, batteries, inverters, transformers, any exterior columns and appurtenant slabs, or any near grade supported structures. (i.e., building envelope).

Earthwork on the project should be observed and evaluated by Terracon. The evaluation of earthwork should include observation and testing of engineered fill, subgrade preparation, foundation bearing soils, and other geotechnical conditions exposed during the construction of the project.

Site Preparation

Although no physical evidence of fills, utilities, or underground facilities such as septic tanks, cesspools, basements, and utilities was observed during the site reconnaissance, such features could be encountered during construction. If unexpected fills, utilities, or underground facilities are encountered, such features should be removed, and the excavation thoroughly cleaned prior to backfill placement and/or construction.

We recommend the surficial topsoil or deleterious material encountered during construction be removed from within the proposed on-site structure and access road limits to remove any organic material that may be present at the surface. Based on subsurface exploration, topsoil depth is anticipated to be less than 6 inches bgs.

The exposed subgrade should then be proof-rolled utilizing vibratory roller to delineate any loose areas. Following proof-rolling, the subgrade preparation for structures can begin.

Subgrade Preparation

Due to the collapse potential of near surface soils, BESS structures supported on a shallow foundation system should bear on structural fill extending to a minimum depth of 3 feet below the bottom of foundation, or 6 feet below existing site grades, whichever is greater.

Substation structures supported on a shallow foundation system should bear on structural fill extending to a minimum depth of 2 feet below the bottom of foundation, or 4 feet below existing site grades, whichever is greater.

To provide lateral support, grading for the proposed foundations should incorporate the footprint of foundations plus a lateral distance of 8 inches beyond the outside perimeter of foundations of every foot of vertical over-excavation, where space is available.

Subgrade soils beneath exterior slabs and pavements should be scarified to a minimum depth of 12 inches, moisture conditioned, and compacted. The moisture content and compaction of subgrade soils should be maintained until non-structural slab or pavement construction.

Exposed areas which will receive compacted fill, once properly cleared and benched where necessary, should be scarified to a minimum depth of 10 inches, moisture conditioned, and compacted per the compaction requirements in this report.

Based upon the subsurface conditions determined from the geotechnical exploration, subgrade soils exposed during construction are anticipated to be relatively workable. However, the workability of the subgrade may be affected by precipitation, repetitive construction traffic or other factors. If unworkable conditions develop, workability may be improved by scarifying and moisture conditioning process.

Structural Fill Material Types

Structural fill should meet the following material property requirements:

Fill Type[1]	USCS Classification	Acceptable Areas for Placement
Structural Fill	Sands with greater than 15% and less than 40% passing No. 200 sieve and Organic Content (OC) less than 3% and maximum particle size of 3 inches (SP, SP-SM, SM, SC)	▪ All locations and elevations
Non-Structural Fill	Soils with OC less than 10% and maximum particle size of 3 inches (Any USCS Classification)	▪ Green (non-structural) areas

Fill Type[1]	USCS Classification	Acceptable Areas for Placement

1. Structural fill should consist of approved materials that are free of organic matter and debris. A sample of each material type should be submitted to the Geotechnical Engineer for evaluation prior to use on this site.

Based on the borings completed at the site, on-site soils are generally suitable to re-use as Structural Fill at the site.

Structural Fill Placement and Compaction Requirements

Structural fill should meet the following compaction requirements.

Description	Maximum Density per the Modified Proctor Test (ASTM D 1557) [1]			Recommended Test Frequency [1, 2]
	Minimum Compaction Requirement (%)	Range of Moisture Contents for Compaction [2] (% over optimum)		
		Minimum	Maximum	
Fill soils placed beneath grade supported structure areas and other BESS/substation structures supported on piles	90	-2 [3]	+2 [3]	One test per 2,500 SF, per lift, or one per foundation, whichever is more frequent
Base Coarse Aggregate for BESS Pad	95	-2 [3]	+2 [3]	One test per lift per 5,000 SF
Access roads - subgrade composed of new fill (below the upper 1 foot of subgrade)	90	-2 [3]	+2 [3]	1 test per 1,000 LF per lift until 3 consecutive tests pass, then one test per 5,000 LF per lift thereafter
Access roads - subgrade (upper 1 foot of subgrade)	95	-2 [3]	+2 [3]	One test per 500 LF per lift
Access roads – top of subgrade (native soil subgrade)	Proof-roll	Maximum 1 inch rut		2 passes (out and back). Proof-roll with a 25-ton fully loaded tandem axle dump truck or equivalent water truck.
Access roads -aggregate surfacing	95	-2 [3]	+2 [3]	2 passes (out and back). Proof-roll with a 25-ton fully loaded tandem axle dump truck or equivalent water truck.

Description	Maximum Density per the Modified Proctor Test (ASTM D 1557) [1]			Recommended Test Frequency [1, 2]
	Minimum Compaction Requirement (%)	Range of Moisture Contents for Compaction [2] (% over optimum)		
		Minimum	Maximum	
Utility and cable trench backfill (non-load bearing areas) [4]	90	-2%	+2%	1 test per 500 LF per lift until 3 consecutive tests pass, then 1 test per 1,000 LF per lift thereafter
Utility and cable trench backfill (load bearing areas) [4]	95	-2%	+2%	1 test per lift

1. Frequency of tests may be increased or decreased at the discretion of the Geotechnical Engineer of Record.
2. Moisture ranges are for informational purposes only. At minimum, Contractor shall achieve the target minimum density specified and the surface should be non-yielding.
3. If the target density is achieved but the moisture is outside of the moisture tolerance, the test is deemed acceptable.
4. Compaction requirements may be increased by the electrical engineer based on thermal resistivity analyses. Compaction should be reviewed and approved by the Electrical Engineer of Record.

Utility Trenches

It is anticipated that the on-site soils will provide suitable support for underground utilities and piping that may be installed. Any soft and/or unsuitable material encountered at the bottom of excavations should be removed and be replaced with an adequate bedding material. A non-expansive granular material with a sand equivalent greater than 30 should be used for bedding and shading of utilities, unless allowed or specified otherwise by the utility manufacturer.

On-site materials are considered suitable for backfill of utility and pipe trenches from one foot above the top of the pipe to the final ground surface, provided the material is free of organic matter and deleterious substances.

Trench backfill should be mechanically placed and compacted as discussed earlier in this report. Compaction of initial lifts should be accomplished with hand-operated tampers or other lightweight compactors. If trenches are placed beneath footings, the backfill should satisfy the gradation and expansion index requirements of engineered fill discussed in this report. Flooding or jetting for placement and compaction of backfill is not recommended.

Shrinkage

For balancing grading on-site, estimated shrink factors of the on-site soils, when used as compacted fill, are provided in this section. Shrinkage factors are based on converting materials in its natural state before disturbance to materials after compaction. The earthwork factors are based on a range of target densities for varying compaction levels compared to the maximum dry density as determined by ASTM D1557. The estimated shrinkage of the near surface site soils when used as compacted fill is presented in the table below:

Percent Compaction (%)	Shrinkage (%)[1]
85	0 to 10
90	5 to 15
95	10 to 20

1. The degree of shrinkage is expected to vary across the site, however it our opinion that the midpoint of each range can be used to provide an estimated volume for import/export considerations.

These estimates are general in nature, and are based on our experience, limited data from our field exploration, results of laboratory tests, and the soil conditions we encountered at the site. Earthwork factors may vary dependent upon the actual subsurface conditions, which may include variations in soil gradations as well as gravel and fines content.

Excavations

It is anticipated that excavations for the proposed construction can be generally accomplished with conventional earthmoving equipment.

Some additional effort may be necessary to extract cobble sized materials, particularly in deep narrow excavations such as utility trenches. Consideration should be given to obtaining a unit price for difficult excavation in the contract documents for the project.

The bottom of excavations should be thoroughly cleaned of loose soils and disturbed materials prior to backfill placement and/or construction.

Onsite soils consist of cohesionless sandy soils. Such soils have the tendency to cave and slough during excavations. Therefore, formwork may be needed for foundation excavations.

The earthwork contractor is solely responsible for designing and constructing stable, temporary excavations as required to maintain stability of both the excavation sides and

bottoms. Excavations should be sloped or shored in the interest of safety following local and federal regulations, including current OSHA excavation and trench safety standards. As a minimum, excavations should be performed in accordance with OSHA 29 CFR, Part 1926, Subpart P, "Excavations" and its appendices, and in accordance with any applicable local, and/or state regulations. However, based on the currently available information, we have assumed that excavations deeper than 3 feet will not be required.

Earthwork Construction Considerations

We recommend that a Terracon geotechnical engineer or qualified representative be retained during the construction phase of the project to observe earthwork and to perform necessary tests and observations during stripping of topsoil, subgrade preparation, placement and compaction of controlled compacted fills, backfilling of excavations, and just prior to construction of any foundations, slabs, or roadways.

Care should be taken to avoid disturbance of prepared subgrade soils. The near surface soils can be easily disturbed, especially by construction traffic. Construction traffic should not operate directly on saturated or low strength soils. If the subgrade becomes saturated, desiccated, or disturbed, the affected materials should either be scarified and compacted, or be removed and replaced as previously discussed. Subgrades should be observed and tested by Terracon prior to construction.

Excavations for utility installations or shallow foundations are not expected to encounter shallow groundwater near-surface. The contractor is responsible for employing appropriate dewatering methods to control seepage and facilitate construction, if needed.

As a minimum, excavations should be performed in accordance with OSHA 29 CFR, Part 1926, Subpart P, "Excavations" and its appendices as well as other applicable codes, and in accordance with any applicable local, state, and federal safety regulations. The contractor should be aware that slope height, slope inclination, and excavation depth should in no instance exceed those specified by these safety regulations. Flatter slopes than those dictated by these regulations may be required depending upon the soil conditions encountered and other external factors. These regulations are strictly enforced and if they are not followed, the owner, the contractor, and/or earthwork and utility subcontractor could be liable and subject to substantial penalties. Under no circumstances should the information provided in this report be interpreted to mean that Terracon is responsible for construction site safety or the contractor's activities. Construction site safety is the sole responsibility of the contractor who shall also be solely responsible for the means, methods, and sequencing of the construction operations.

Construction Observation and Testing

Terracon should be retained to observe the earthwork efforts. Observation and testing should include documentation of removal of vegetation and topsoil, proofrolling, and mitigation of soft/unstable areas delineated by the proofroll.

The earthwork efforts should be monitored under the direction of Terracon Geotechnical Engineer. Field density tests should be conducted during placement and compaction of engineered fill. The testing frequency should be in accordance with **Fill Placement and Compaction Requirements.**

Terracon may require additional tests as considered necessary to check on the uniformity of compaction. No additional layers of fill should be placed until the field density test results indicate that the specified density has been obtained.

In areas of foundation excavations, the bearing subgrade should be evaluated by Terracon. In the event unanticipated conditions are encountered, Terracon may prescribe mitigation options.

In addition to the documentation of the essential parameters necessary for construction, the continuation of including Terracon into the construction phase of the project provides the continuity to maintain the Geotechnical Engineer's evaluation of subsurface conditions, including assessing variations and associated design changes.

Spread Footing and Isolated Slab Foundations

General

Based on the anticipated magnitude of loading for these structures, the site is suitable to support the anticipated contact pressure assumed in the project description section of this report and site preparation as discussed in the **Earthwork** section of this report should be followed for shallow foundation support of these structures.

The following sections present design recommendations and construction considerations for the shallow foundations for proposed substation structures.

Shallow Foundation Design Recommendations

Item	Description
Foundation System	Spread footings, mat foundation, or support slab with thickened edges bearing on engineered fill

Item	Description
Maximum net allowable bearing pressure (settlement controlled)[1]	Square Spread Footings: ▪ 3,000 psf up to 5 feet wide Mat foundations or support slabs with thickened edges: ▪ 2,500 psf up to 10 feet by 20 feet ▪ 1,750 up to 15 by 30 feet ▪ 1,500 up to 20 by 40 feet Grade Beams: ▪ 2,500 psf up to 2 feet wide ▪ 2,000 psf up to 5 feet wide
Modulus of subgrade reaction for slab-on-grade design (K_{v1})	200 pounds per square inch per in (psi/in) for point loading conditions
Bearing material	Compacted structural fill in accordance with **Earthwork** section of this report.
Minimum width dimensions	2 feet
Maximum width dimensions	20 feet
Ultimate Passive Resistance (equivalent fluid pressures) [5]	400 psf/ft
Minimum embedment below finished grade [2]	18 inches
Approximate total settlement [3]	1 inch
Estimated differential settlement	½ inch over 50 feet
Ultimate coefficient of sliding friction [4, 5]	0.35

1. The recommended net allowable bearing pressure is the pressure in excess of the minimum surrounding overburden pressure at the footing base elevation. It assumes any unsuitable soils, if encountered, will be replaced with compacted structural fill.
2. Required for the allowable bearing pressure, erosion protection and to reduce the effects of seasonal moisture variations in the subgrade soils.
3. The foundation settlement will depend upon the variations within the subsurface soil profile, the structural loading conditions, the embedment depth of the footings, the thickness of compacted fill, and the quality of the earthwork operations. Footings should be proportioned to relatively constant dead-load pressure in order to reduce differential movement between adjacent footings.
4. Sliding friction along the base of the footing will not develop where net uplift conditions exist.
5. Passive and frictional resistance may be considered together however a reduction of 50 percent should be applied to one of the resistances.

Settlement calculations were performed utilizing Boussinesq stress distribution methods to estimate the static settlement and allowable bearing pressure for various foundation widths.

The allowable foundation bearing pressures apply to dead loads plus design live load conditions. The weight of the foundation concrete below grade may be neglected in dead load computations.

The subgrade modulus (K_v) for the mat is affected by the size of the mat foundation and would vary according to the following equation:

$$K_v = K_{v1}*((B+1)/2B)^2$$

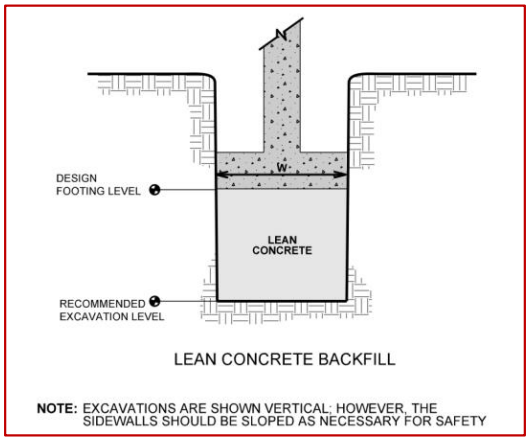
Where: K_v is the modulus for the size footing being analyzed
B is the width of the mat foundation.

Foundation excavations should be observed by the Geotechnical Engineer. If the soil conditions encountered differ significantly from those presented in this report, Terracon should be contacted to provide additional evaluation and supplemental recommendations.

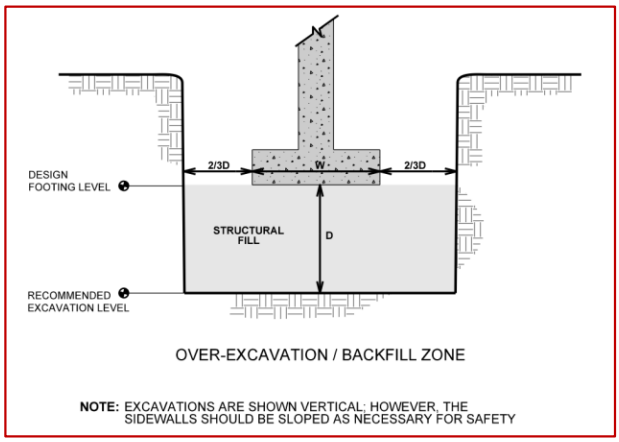
Spread Footing Construction Considerations

As noted in **Earthwork**, the footing excavations should be evaluated under the observation of the Geotechnical Engineer. The base of all foundation excavations should be free of water and loose soil, prior to placing concrete. Concrete should be placed soon after excavating to reduce bearing soil disturbance. Care should be taken to prevent wetting or drying of the bearing materials during construction. Excessively wet or dry material or any loose/disturbed material in the bottom of the footing excavations should be removed/reconditioned before foundation concrete is placed.

If unsuitable bearing soils are observed at the base of the planned footing excavation, the excavation should be extended deeper to suitable soils, and the footings could bear directly on these soils at the lower level or on lean concrete backfill placed in the excavations. The lean concrete replacement zone is illustrated on the sketch below.



Over-excavation for engineered fill placement below footings should be conducted as shown below. The over-excavation should be backfilled up to the footing base elevation, with suitable engineered fill placed, as recommended in the **Earthwork** section.



Drilled Shaft Foundation Design

Deep foundations, including drilled shaft foundations and/or direct embedment foundations with concrete backfill, may be utilized for the support of substation structures for the project. Soil parameters are presented in the following section which may be used for design of drilled shafts or direct embedment foundations with concrete backfill.

Drilled piers should have a minimum (center-to-center) spacing of three diameters. Closer spacing may require a reduction in axial load capacity. Axial capacity reduction can be determined by comparing the allowable axial capacity determined from the sum of individual piers in a group versus the capacity calculated using the perimeter and base of the pier group acting as a unit. The lesser of the two capacities should be used in design.

The allowable uplift capacities may be taken as 66 percent of the allowable compressive skin friction capacity provided in the table in the following section. The weight of the foundation should be added to these values to obtain the actual allowable uplift capacities for drilled shafts. Tensile reinforcement should extend to the bottom of shafts subjected to uplift loading.

Design Parameters

The soil effective unit weight and cohesion shown in the following table are based on an empirical correlation with SPT blow counts (N-Values) from borings that were located within the planned substation area. Static settlement of the drilled shaft foundations designed on the basis of the following table are anticipated to be less than 1 inch (based on a maximum shaft diameter of 6 feet).

Substation Deep Foundation Recommendations							
Depth Interval (feet)	**LPile Soil Type[3]**	**Effective Unit Weight (pcf)**	**Cohesion (psf)**	**Friction Angle (degrees)**	**Allowable Skin Friction [1] (psf)**	**Allowable End Bearing [1] (psf)**	**Deformation Modulus (ksi)**
0 to 2[2]	Sand	100	--	5	--	--	0.01
2 to 5	Sand	110	--	38	70	16,000	3.15
5 to 20	Sand	100	--	33	250	20,000	1.80
20 to 30	Sand	105	--	36	600	20,000	2.70
30 to 41½	Sand	105	--	35	700	20,000	2.25

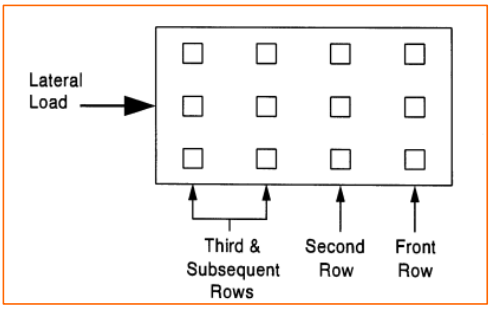
1. The allowable skin friction and end bearing include a factor of safety of 2.5 and 3.0, respectively.
2. Depth of neglect may be modeled as very low strength soil in LPile.
3. Default LPILE k_h values are considered acceptable.

Design of the deep foundations should be completed by the structural engineer using the geotechnical engineering design criteria provided herein. The required foundation size and depth should be determined based upon analyses for vertical loads, lateral loads, and overturning moments.

The load capacities provided herein are based on the stresses induced in the supporting soil strata. The structural capacity of the shafts/piles should be checked to assure they can safely accommodate the combined stresses induced by axial and lateral forces. Lateral deflections of shafts/piles should be evaluated using an appropriate analysis method, and will depend upon the pile's diameter, length, configuration, stiffness and "fixed head" or "free head" condition. We can provide additional analyses and estimates of lateral deflections for specific loading conditions upon request. The load-carrying capacity of shafts/piles may be increased by increasing the diameter and/or length.

Drilled shafts with a center-to-center spacing of less than 5D (where D is the diameter of the shaft) are considered to work in a group. Piles in closely spaced groups behave differently than single isolated piles because of pile-soil-pile interactions that take place in the group. It is generally recognized that deflections of a pile in a closely spaced group are greater than the deflections of an individual pile at the same load because of these interaction effects. This unequal distribution of load among piles is caused by "shadowing". When piles are in a closely-spaced group with substantial interaction, the shear-failure planes resulting from the movement of each pile will overlap, and the ultimate resistance for piles in a group may be less than that of a single pile. Thus, a modification factor, with a value less than one, is needed to reduce the ultimate soil resistance, as shown in the figure below. This reduces the ultimate soil resistance and softens the shape of the p-y curve.

Guidance for applying p-multiplier factors to the p values in the p-y curves for each row of pier foundations within a pier group are as follows:



Pile Center to Center Spacing (in the direction of loading)	P-Multipliers, P_M		
	Row 1	Row 2	Row 3 and Subsequent Rows
5.0B	1.0	0.85	0.70
3.0B	0.7	0.50	0.35

Drilled Shaft Construction Considerations

Drilling for the proposed drilled shafts to design depths should be possible with conventional single flight power augers. However, due to the presence of gravels and very dense materials onsite, consideration should be given to obtaining a unit price for difficult excavation in the contract documents for the project. For drilled shaft depths above the depth of groundwater, temporary steel casing will likely be required to properly drill and clean shafts prior to concrete placement. Drilled pier bearing surfaces must be thoroughly cleaned and all loose materials removed from the bottom of the pier excavation prior to concrete placement.

If foundation concrete cannot be placed in dry conditions, a tremie should be used for concrete placement.

In the event drilled hole walls slough during drilling, temporary steel casing may be required to properly drilled shafts prior to concrete placement. We recommend the use of slurry drilling methods with polymers method to keep the solids in suspension during the drilling. Drilled shaft foundation concrete should be placed within 6 inches of the shaft base of the slurry-filled excavation immediately after completion of drilling and cleaning. The tremie should remain inserted several feet into the fresh concrete as it displaces the slurry upward and until placement is complete. The slurry should have a sand content no greater than 1% at the time concrete placement commences. The maximum unit weight of the slurry should be established in consultation with Terracon.

If casing is used for drilled shaft construction, it should be withdrawn in a slow continuous manner maintaining a sufficient head of concrete to prevent infiltration of water or the creation of voids in shaft concrete. Shaft concrete should have a relatively high fluidity when placed in cased shaft holes or through a tremie. Shaft concrete with slump in the range of 6 to 8 inches is recommended.

Foundation concrete should be placed immediately after completion of drilling and cleaning. All excavations should be filled with concrete as soon after drilling as possible. In no event should shaft holes be left open overnight.

Formation of mushrooms or enlargements at the tops of shafts should be avoided during shaft drilling. If mushrooms develop at the tops of the shafts during drilling, sono-tubes should be placed at the shaft tops to help isolate the shafts.

Free-fall concrete placement in drilled piers will only be acceptable if provisions are taken to avoid striking the concrete on the sides of the hole or reinforcing steel. The use of a bottom-dump hopper, or an “elephant's trunk” discharging near the bottom of the hole where concrete segregation will be minimized, is recommended.

We recommend that all drilled shaft installations be observed on a full-time basis by an experienced geotechnical engineer in order to evaluate that the soils encountered are consistent with the recommended design parameters. If the subsurface soil conditions encountered differ significantly from those presented in this report, supplemental recommendations will be required. The Geotechnical Engineer should observe the installation of drilled piers to verify the soil conditions and the diameter and depth of piers. Drilled piers should be constructed true and plumb.

Drilled pier end bearing surfaces must be thoroughly cleaned prior to concrete placement. A representative of the Geotechnical Engineer should inspect the bearing surface and foundation pier configuration. If the subsurface soil conditions encountered differ significantly from those presented in this report, supplemental recommendations will be required.

Gravel-Surfaced Access Roads

General Pavement Comments

Roadway designs are provided for the traffic conditions and pavement life conditions as noted in the **Project Description** and in the following sections of this report. A critical aspect of pavement performance is site preparation. Roadway designs noted in this section are contingent upon the site being prepared as recommended in the **Earthwork** section. Additionally, our recommendations are based on *Chapter 4 Low-Volume Road Design, Part II, of the AASHTO Guide for Design of Pavement Structures, 1993.*

Design Parameters

We understand gravel-surfaced access roads are planned throughout the site. The gravel-surfaced road sections for post-construction use have been developed under the following assumptions:

Gravel-Surfaced Roadway Design Parameters		
Parameter	**Design Value**	**Comments**
Traffic Loading	1,500 ESALs[1]	Assumed
Design Life	30 years	Assumed
Design CBR	12	Based on lab testing
Resilient Modulus	12,000	Based on AASHTO 2002 Design Guide
Aggregate Base Elastic Modulus	30,000 psi	Assumed
Allowable Rut Depth	2 inches	Assumed
Design Serviceability Loss	2.0	Assumed
Vehicle Tire Pressure	80 psi	Assumed

1. ESAL = 18 kips Equivalent Single Axle Load

Access Road Section

As a minimum, we recommend the following for gravel-surfaced access roads:

Typical Gravel-Surfaced Road Section – Post Construction Traffic		
Base Course Thickness	**Subbase Type**	**Geogrid Stabilization**
4 inches	12 inches of compacted on-site soil	No

1. Base materials should meet the current Caltrans Highway Design Manual specifications.
   Base – Caltrans Class 2 aggregate base

Roadway section should be constructed over a minimum of 12 inches of scarified, moisture conditioned, and compacted native soils to 95% of the maximum dry density using ASTM D1557. The recommended thicknesses should be measured after full compaction. The width of the roadway should extend a minimum distance of 1 foot on each side of the desired surface width.

Aggregate materials should conform to the specifications of Class II aggregate base in accordance with the requirements and specifications of the State of California Department of Transportation (Caltrans), or other approved local governing specifications.

Positive drainage should be provided during construction and maintained throughout the life of the roadways. Proposed roadway design should maintain the integrity of the road and eliminate ponding.

Roadway Design and Construction Considerations

Regardless of the design, un-surfaced roadways will display varying levels of wear and deterioration. We recommend an implementation of a site inspection program at a frequency of at least once per year to verify the adequacy of the roadways. Preventative measures should be applied as needed for erosion control and re-grading. An initial site inspection should be completed approximately three months following construction.

Surfacing materials should not be placed when the surface is wet. Surface drainage should be provided away from the edge of roadways to reduce lateral moisture transmission into the subgrade.

If rut depths become excessive as construction work progresses, re-grading and re-compaction should be performed as necessary. Care should be taken to reduce or eliminate trafficking of the unpaved access road when the subgrade is wet as this will result in accelerated rutting conditions. Scarification, moisture treatment as necessary, and re-compaction of the roadways will likely be necessary as the roadways deteriorate.

Materials and construction of roadways for the project should be in accordance with the requirements and specifications of the California Department of Transportation or the applicable local governing body.

Stormwater Management

Eleven in-situ percolation tests were performed to approximate depths of 3 to 5 feet bgs. A 2-inch-thick layer of gravel was placed in the bottom of each boring after the borings were drilled to investigate the soil profile. A 3¼ -inch diameter perforated pipe was installed on top of the gravel layer in each boring. Gravel was used to backfill between the perforated pipes and the boring sidewall. The borings were then filled with water for a pre-soak period of a minimum of 1 hour or 16 hours (depending on the drainage characteristics of the percolation test). Testing began after the pre-soak period. At the beginning of the test, the pipes were refilled with water and readings were taken at standardized time intervals. Percolation rates are provided in the following table:

Percolation Test Results Summary

Test Location (depth, feet bgs)	Average of the Final Three Percolation Rates (in/hr.)	Correlated Infiltration Rate[1] (in/hr.)
PERC-1 (0 to 3 feet)	254.7	>20
PERC-2 (0 to 5 feet)	>300	>20
PERC-3 (0 to 3 feet)	>300	>20
PERC-4 (0 to 5 feet)	272.0	13.6
PERC-5 (0 to 3 feet)	154.0	10.6
PERC-6 (0 to 5 feet)	252.0	13.7
PERC-7 (0 to 3 feet)	138.0	8.4
PERC-8 (0 to 5 feet)	218.0	10.7
PERC-9 (0 to 3 feet)	130.0	8.4
PERC-10 (0 to 5 feet)	208.2	9.9
PERC-11 (0 to 3 feet)	146.0	8.5

1. If proposed infiltration system will mainly rely on vertical downward seepage, the correlated infiltration rates should be used. The infiltration rates were correlated using the Porchet method.

With time, the bottoms of infiltration systems tend to plug with organics, sediments, and other debris. Long term maintenance will likely be required to remove these deleterious materials to help reduce decreases in actual percolation rates.

The percolation tests were performed with clear water, whereas the storm water will likely not be clear, but may contain organics, fines, and grease/oil. The presence of these deleterious materials will tend to decrease the rate that water percolates from the infiltration systems. Design of the stormwater infiltration systems should account for the presence of these materials and should incorporate structures/devices to remove these deleterious materials. A safety factor should be applied to these measured rates.

Based on the soils encountered in our borings, we expect the percolation rates of the soils could be different than measured in the field due to variations in fines and gravel content. The design elevation and size of the proposed infiltration system should account for this expected variability in infiltration rates.

Infiltration testing should be performed after construction of the infiltration system to verify the design infiltration rates. It should be noted that siltation and vegetation growth along with other factors may affect the infiltration rates of the infiltration areas. The actual infiltration rate may vary from the values reported here. Infiltration systems should be located a minimum of 10 feet from any existing or proposed foundation system.

General Comments

Our analysis and opinions are based upon our understanding of the project, the geotechnical conditions in the area, and the data obtained from our site exploration. Variations will occur between exploration point locations or due to the modifying effects of construction or weather. The nature and extent of such variations may not become evident until during or after construction. Terracon should be retained as the Geotechnical Engineer, where noted in this report, to provide observation and testing services during pertinent construction phases. If variations appear, we can provide further evaluation and supplemental recommendations. If variations are noted in the absence of our observation and testing services on-site, we should be immediately notified so that we can provide evaluation and supplemental recommendations.

Our Scope of Services does not include either specifically or by implication any environmental or biological (e.g., mold, fungi, bacteria) assessment of the site or identification or prevention of pollutants, hazardous materials, or conditions. If the owner is concerned about the potential for such contamination or pollution, other studies should be undertaken.

Our services and any correspondence are intended for the sole benefit and exclusive use of our client for specific application to the project discussed and are accomplished in accordance with generally accepted geotechnical engineering practices with no third-party beneficiaries intended. Any third-party access to services or correspondence is solely for information purposes to support the services provided by Terracon to our

client. Reliance upon the services and any work product is limited to our client and is not intended for third parties. Any use or reliance of the provided information by third parties is done solely at their own risk. No warranties, either express or implied, are intended or made.

Site characteristics as provided are for design purposes and not to estimate excavation cost. Any use of our report in that regard is done at the sole risk of the excavating cost estimator as there may be variations on the site that are not apparent in the data that could significantly affect excavation cost. Any parties charged with estimating excavation costs should seek their own site characterization for specific purposes to obtain the specific level of detail necessary for costing. Site safety and cost estimating including excavation support and dewatering requirements/design are the responsibility of others. Construction and site development have the potential to affect adjacent properties. Such impacts can include damages due to vibration, modification of groundwater/surface water flow during construction, foundation movement due to undermining or subsidence from excavation, as well as noise or air quality concerns. Evaluation of these items on nearby properties is commonly associated with contractor means and methods and is not addressed in this report. The owner and contractor should consider a preconstruction/precondition survey of the surrounding development. If changes in the nature, design, or location of the project are planned, our conclusions and recommendations shall not be considered valid unless we review the changes and either verify or modify our conclusions in writing.

Attachments

Field Exploration Results

Contents:

Exploration and Testing Procedures
Site Location
Exploration Plan
General Notes
Unified Soil Classification System
Geomodel
Boring Logs (BESS-01 through BESS-15, PERC-01 through PERC-11 and SUB-1 through SUB-7)
Test Pit Logs (BTP-1 to BTP-4, STP-2 to STP-3)

Exploration and Testing Procedures

Field Exploration

The following table provides a summary of our geotechnical exploration completed at the site.

Number of Explorations	Type of Exploration	Depth (feet)	Exploration ID	Location
15	Soil Boring	21½ to 41½	BESS-01 through BESS-15	Planned BESS
4	Test Pit	10	BTP-1 to BTP-4	
11	Soil Boring	3 to 5	PERC-01 to PERC-11	Planned Infiltration Basin
7	Soil Boring	41½	SUB-1 to SUB-7	Planned Substation
2	Test Pit	10	STP-2 to STP-3	

Boring Layout and Elevations: Terracon personnel provided the boring layout using handheld GPS equipment (estimated horizontal accuracy of about ±10 feet) and referencing existing site features. Approximate ground surface elevations were estimated using Google Earth Pro. If more precise elevations or boring layout are desired, we recommend the borings be surveyed.

Subsurface Exploration Procedures: We advanced the borings with a track-mounted, drill rig using continuous flight augers (hollow stem). Four samples were generally obtained in the upper 10 feet of each boring and at intervals of 5 feet thereafter. Test samples were collected during drilling in general accordance with the appropriate ASTM methods using Standard Penetration Testing (SPT) and sampling using either standard split-spoon or Modified California samplers. A sampling spoon was driven into the ground by a 140-pound automatic hammer falling a distance of 30 inches. The number of blows required to advance the sampling spoon the last 12 inches of a normal 18-inch penetration was recorded as the Standard Penetration Test (SPT) resistance value, also referred to as N-values. The N-values are indicated on the boring logs at the test depths.

We also observed the boreholes while drilling and at the completion of drilling for the presence of groundwater. The groundwater levels are shown on the attached boring logs.

The sampling depths, penetration distances, and other sampling information was recorded on the field boring logs. The samples were placed in appropriate containers and taken to our soil laboratory for testing and classification by a Geotechnical Engineer. Our exploration team prepared field boring logs as part of the drilling operations. These field

logs included visual classifications of the materials observed during drilling and our interpretation of the subsurface conditions between samples. Final boring logs were prepared from the field logs. The final boring logs represent the Geotechnical Engineer's interpretation of the field logs and include modifications based on observations and tests of the samples in our laboratory.

Test Pits: Test pits were performed using a metal track-mounted excavator and observed by a Terracon field engineer. Samples and intact sidewalls of the pits were classified in the field by the field engineer with photos at each location. In addition, we observed and recorded groundwater levels (if present) during excavations. Test pits were backfilled with the excavated materials upon completion.

Site Location

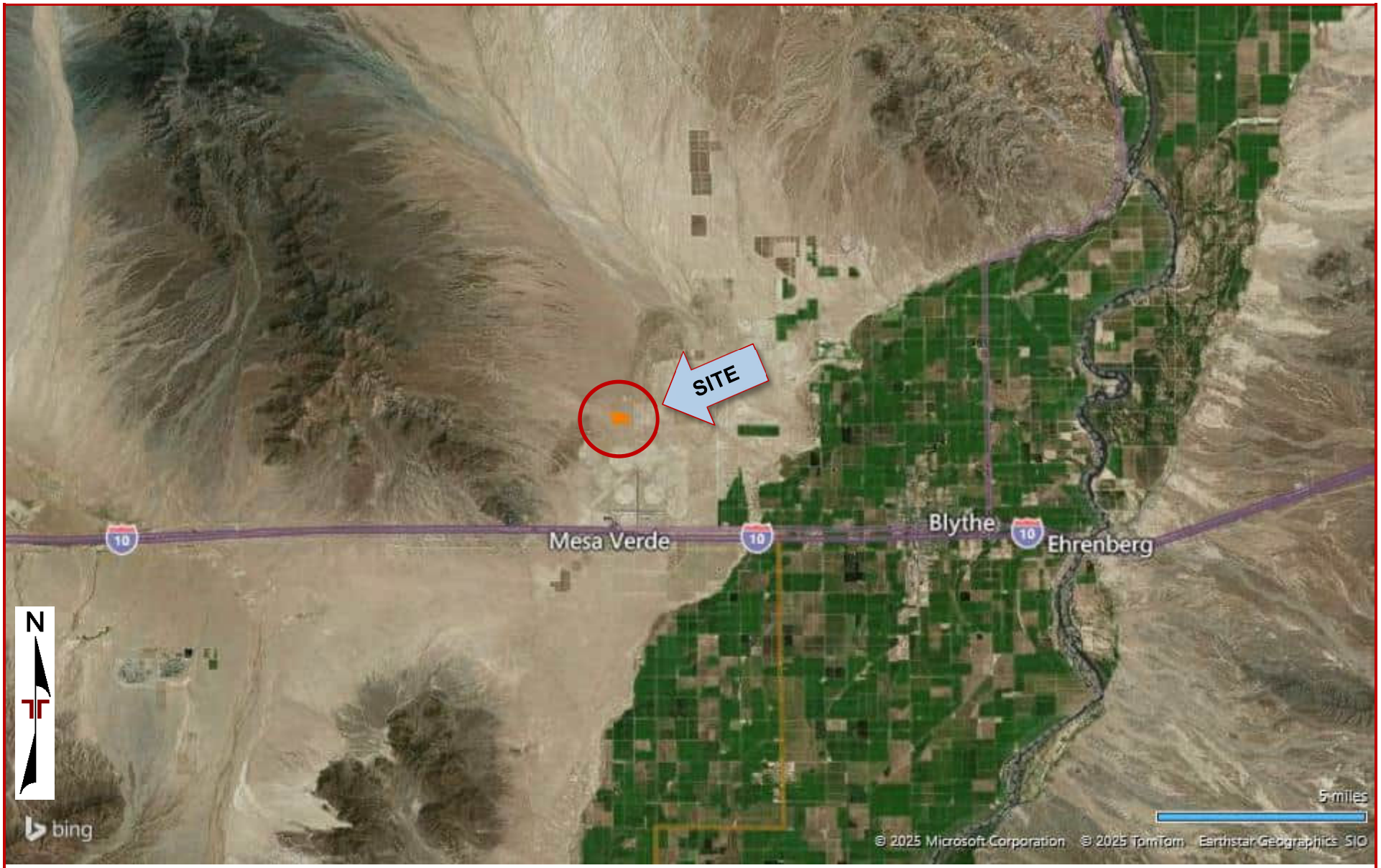


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Exploration Location Plan

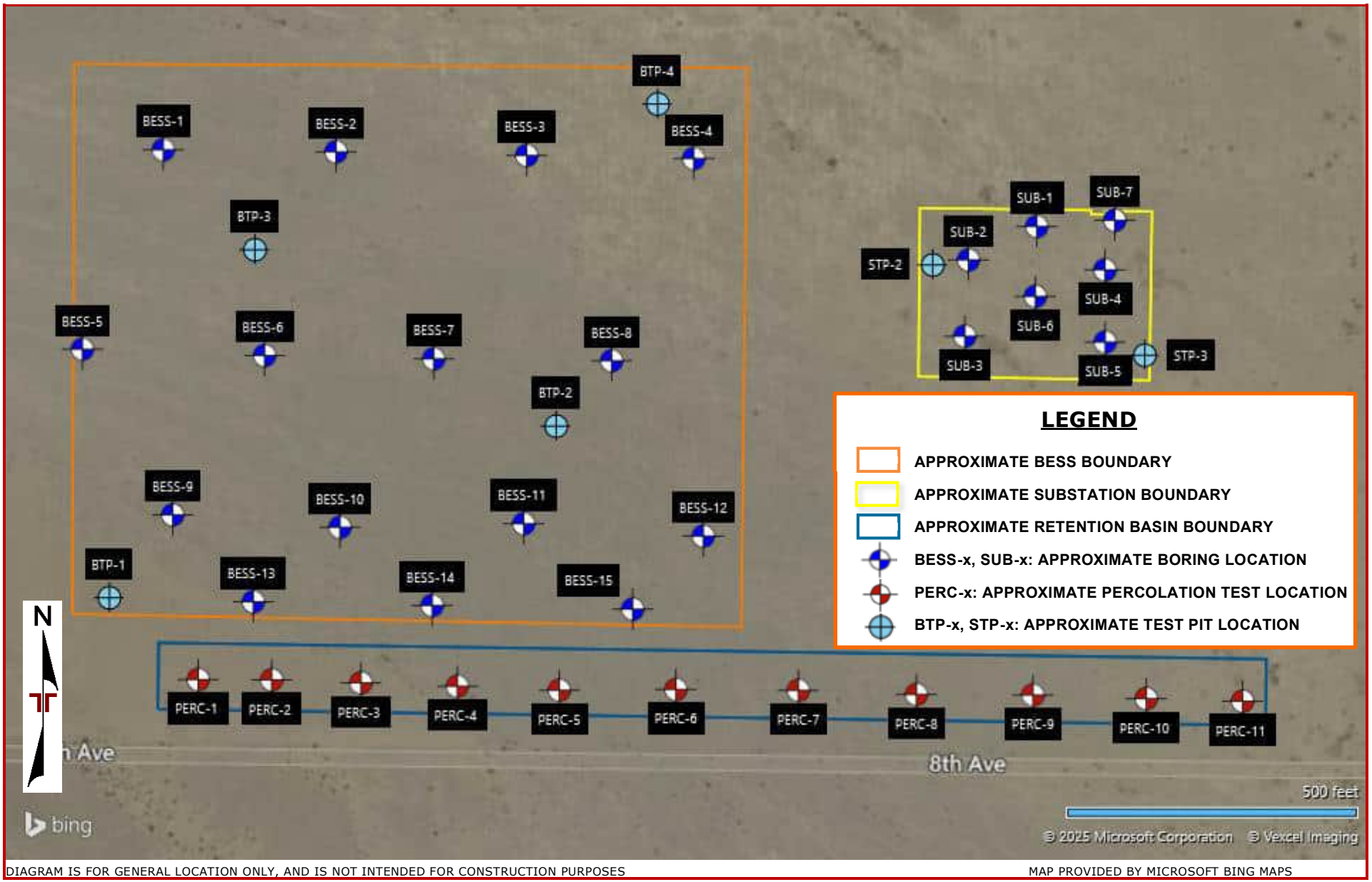
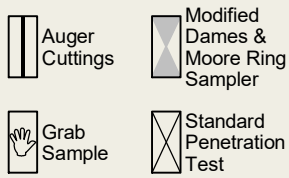


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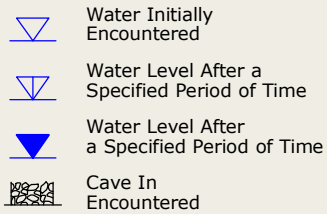
MAP PROVIDED BY MICROSOFT BING MAPS

General Notes

Sampling



Water Level



Water levels indicated on the soil boring logs are the levels measured in the borehole at the times indicated. Groundwater level variations will occur over time. In low permeability soils, accurate determination of groundwater levels is not possible with short term water level observations.

Field Tests

N	Standard Penetration Test Resistance (Blows/Ft.)
(HP)	Hand Penetrometer
(T)	Torvane
(DCP)	Dynamic Cone Penetrometer
UC	Unconfined Compressive Strength
(PID)	Photo-Ionization Detector
(OVA)	Organic Vapor Analyzer

Descriptive Soil Classification

Soil classification as noted on the soil boring logs is based Unified Soil Classification System. Where sufficient laboratory data exist to classify the soils consistent with ASTM D2487 "Classification of Soils for Engineering Purposes" this procedure is used. ASTM D2488 "Description and Identification of Soils (Visual-Manual Procedure)" is also used to classify the soils, particularly where insufficient laboratory data exist to classify the soils in accordance with ASTM D2487. In addition to USCS classification, coarse grained soils are classified on the basis of their in-place relative density, and fine-grained soils are classified on the basis of their consistency. See "Strength Terms" table below for details. The ASTM standards noted above are for reference to methodology in general. In some cases, variations to methods are applied as a result of local practice or professional judgment.

Location And Elevation Notes

Exploration point locations as shown on the Exploration Plan and as noted on the soil boring logs in the form of Latitude and Longitude are approximate. See Exploration and Testing Procedures in the report for the methods used to locate the exploration points for this project. Surface elevation data annotated with +/- indicates that no actual topographical survey was conducted to confirm the surface elevation. Instead, the surface elevation was approximately determined from topographic maps of the area.

Strength Terms

Relative Density of Coarse-Grained Soils
(More than 50% retained on No. 200 sieve.)
Density determined by Standard Penetration Resistance

Relative Density	Standard Penetration or N-Value (Blows/Ft.)	Ring Sampler (Blows/Ft.)
Very Loose	0 - 3	0 - 6
Loose	4 - 9	7 - 18
Medium Dense	10 - 29	19 - 58
Dense	30 - 50	59 - 98
Very Dense	> 50	> 99

Consistency of Fine-Grained Soils
(50% or more passing the No. 200 sieve.)
Consistency determined by laboratory shear strength testing, field visual-manual procedures or standard penetration resistance

Consistency	Unconfined Compressive Strength Qu (tsf)	Standard Penetration or N-Value (Blows/Ft.)	Ring Sampler (Blows/Ft.)
Very Soft	less than 0.25	0 - 1	< 3
Soft	0.25 to 0.50	2 - 4	3 - 4
Medium Stiff	0.50 to 1.00	5 - 8	5 - 9
Stiff	1.00 to 2.00	9 - 15	10 - 18
Very Stiff	2.00 to 4.00	16 - 30	19 - 42
Hard	> 4.00	> 30	> 42

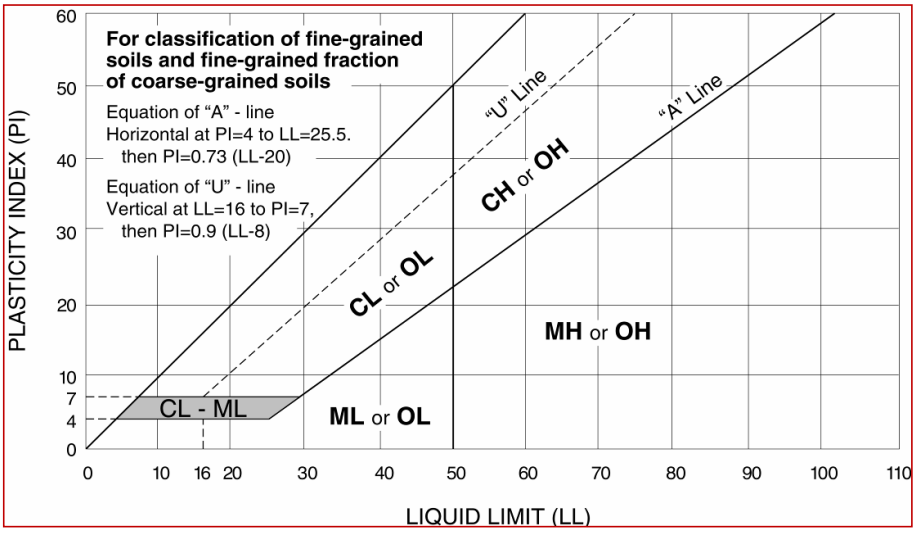
Relevance of Exploration and Laboratory Test Results

Exploration/field results and/or laboratory test data contained within this document are intended for application to the project as described in this document. Use of such exploration/field results and/or laboratory test data should not be used independently of this document.

Unified Soil Classification System

Criteria for Assigning Group Symbols and Group Names Using Laboratory Tests [A]				Soil Classification	
				Group Symbol	Group Name [B]
Coarse-Grained Soils: More than 50% retained on No. 4 sieve	**Gravels:** More than 50% of coarse fraction retained on No. 4 sieve	**Clean Gravels:** Less than 5% fines [C]	Cu≥4 and 1≤Cc≤3 [E]	GW	Well-graded gravel [F]
			Cu<4 and/or [Cc<1 or Cc>3.0] [E]	GP	Poorly graded gravel [F]
		Gravels with Fines: More than 12% fines [C]	Fines classify as ML or MH	GM	Silty gravel [F, G, H]
			Fines classify as CL or CH	GC	Clayey gravel [F, G, H]
	Sands: 50% or more of coarse fraction passes No. 4 sieve	**Clean Sands:** Less than 5% fines [D]	Cu≥6 and 1≤Cc≤3 [E]	SW	Well-graded sand [I]
			Cu<6 and/or [Cc<1 or Cc>3.0] [E]	SP	Poorly graded sand [I]
		Sands with Fines: More than 12% fines [D]	Fines classify as ML or MH	SM	Silty sand [G, H, I]
			Fines classify as CL or CH	SC	Clayey sand [G, H, I]
Fine-Grained Soils: 50% or more passes the No. 200 sieve	**Silts and Clays:** Liquid limit less than 50	**Inorganic:**	PI > 7 and plots above "A" line [J]	CL	Lean clay [K, L, M]
			PI < 4 or plots below "A" line [J]	ML	Silt [K, L, M]
		Organic:	$\frac{LL\ oven\ dried}{LL\ not\ dried} < 0.75$	OL	Organic clay [K, L, M, N]
					Organic silt [K, L, M, O]
	Silts and Clays: Liquid limit 50 or more	**Inorganic:**	PI plots on or above "A" line	CH	Fat clay [K, L, M]
			PI plots below "A" line	MH	Elastic silt [K, L, M]
		Organic:	$\frac{LL\ oven\ dried}{LL\ not\ dried} < 0.75$	OH	Organic clay [K, L, M, P]
					Organic silt [K, L, M, Q]
Highly organic soils:	Primarily organic matter, dark in color, and organic odor			PT	Peat

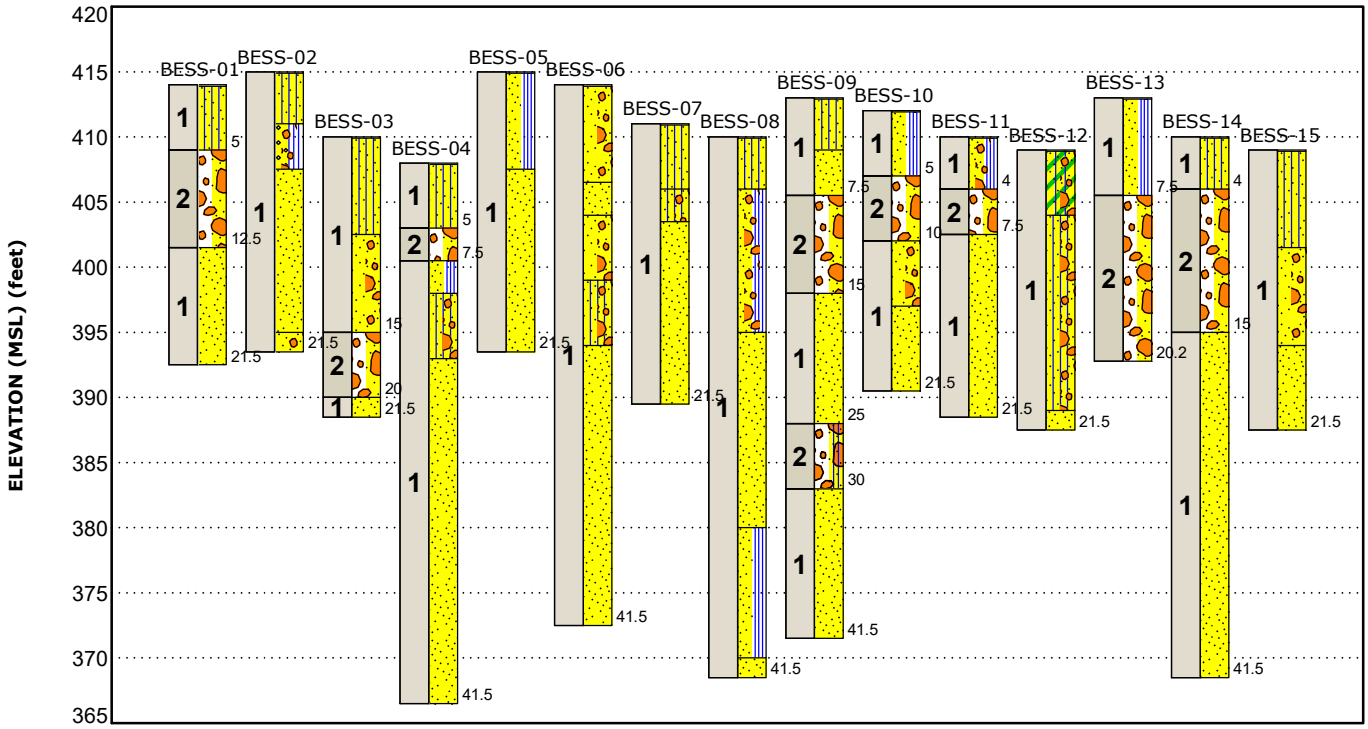
[A] Based on the material passing the 3-inch (75-mm) sieve.
[B] If field sample contained cobbles or boulders, or both, add "with cobbles or boulders, or both" to group name.
[C] Gravels with 5 to 12% fines require dual symbols: GW-GM well-graded gravel with silt, GW-GC well-graded gravel with clay, GP-GM poorly graded gravel with silt, GP-GC poorly graded gravel with clay.
[D] Sands with 5 to 12% fines require dual symbols: SW-SM well-graded sand with silt, SW-SC well-graded sand with clay, SP-SM poorly graded sand with silt, SP-SC poorly graded sand with clay.
[E] $Cu = D_{60}/D_{10}$ $\quad Cc = \frac{(D_{30})^2}{D_{10} \times D_{60}}$
[F] If soil contains ≥ 15% sand, add "with sand" to group name.
[G] If fines classify as CL-ML, use dual symbol GC-GM, or SC-SM.
[H] If fines are organic, add "with organic fines" to group name.
[I] If soil contains ≥ 15% gravel, add "with gravel" to group name.
[J] If Atterberg limits plot in shaded area, soil is a CL-ML, silty clay.
[K] If soil contains 15 to 29% plus No. 200, add "with sand" or "with gravel," whichever is predominant.
[L] If soil contains ≥ 30% plus No. 200 predominantly sand, add "sandy" to group name.
[M] If soil contains ≥ 30% plus No. 200, predominantly gravel, add "gravelly" to group name.
[N] PI ≥ 4 and plots on or above "A" line.
[O] PI < 4 or plots below "A" line.
[P] PI plots on or above "A" line.
[Q] PI plots below "A" line.



Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

GeoModel



This is not a cross section. This is intended to display the Geotechnical Model only. See individual logs for more detailed conditions.

Model Layer	Layer Name	General Description
1	Sand	Medium dense to very dense sand with varying amounts of silt, clay, and gravel
2	Gravel	Medium dense to very dense gravel with varying amounts of silt and sand

Legend

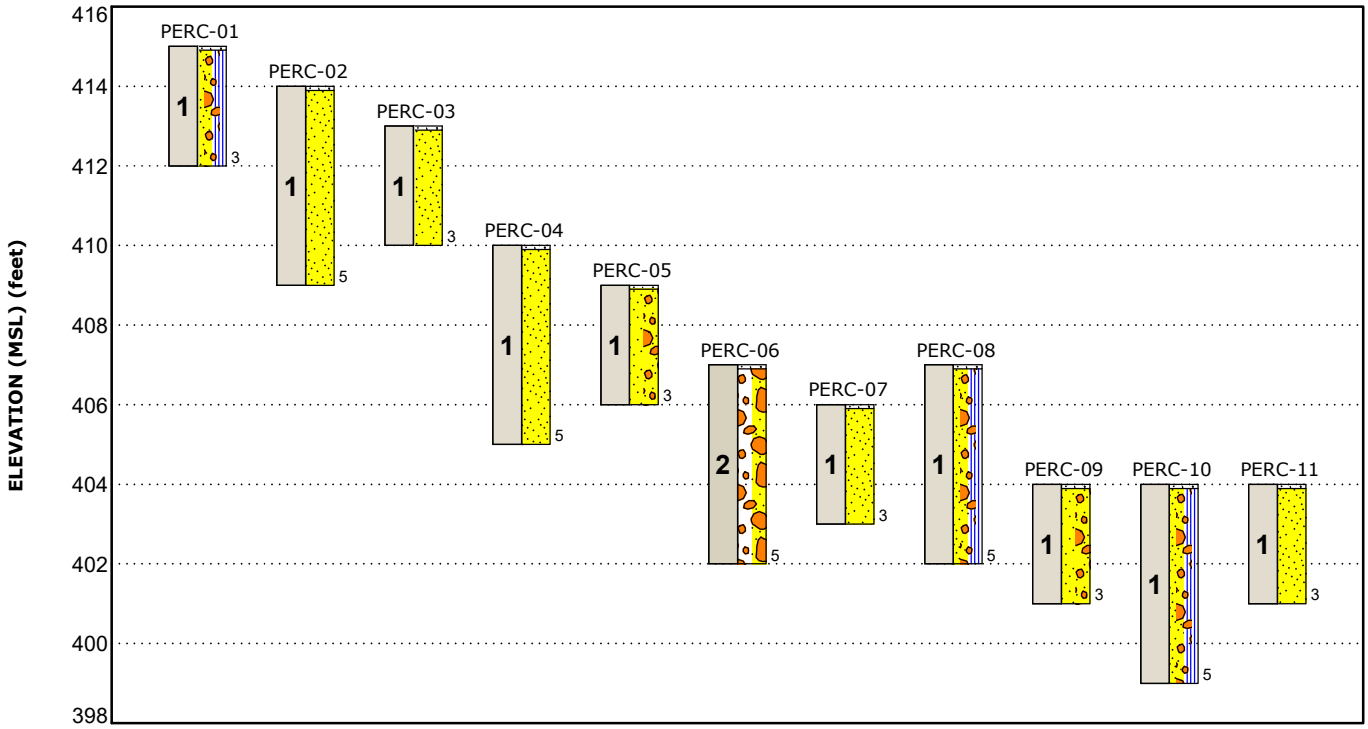
- Topsoil
- Poorly-graded Gravel with Sand
- Well-graded Sand with Silt and Gravel
- Poorly-graded Sand with Silt
- Poorly-graded Sand with Silt and Gravel
- Silty Clayey Sand with Gravel
- Silty Sand
- Poorly-graded Sand
- Poorly-graded Sand with Gravel
- Silty Sand with Gravel
- Poorly-graded Gravel with Silt and Sand

NOTES:
Layering shown on this figure has been developed by the geotechnical engineer for purposes of modeling the subsurface conditions as required for the subsequent geotechnical engineering for this project. Numbers adjacent to soil column indicate depth below ground surface.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

GeoModel



This is not a cross section. This is intended to display the Geotechnical Model only. See individual logs for more detailed conditions.

Model Layer	Layer Name	General Description
1	Sand	Medium dense to very dense sand with varying amounts of silt, clay, and gravel
2	Gravel	Medium dense to very dense gravel with varying amounts of silt and sand

Legend

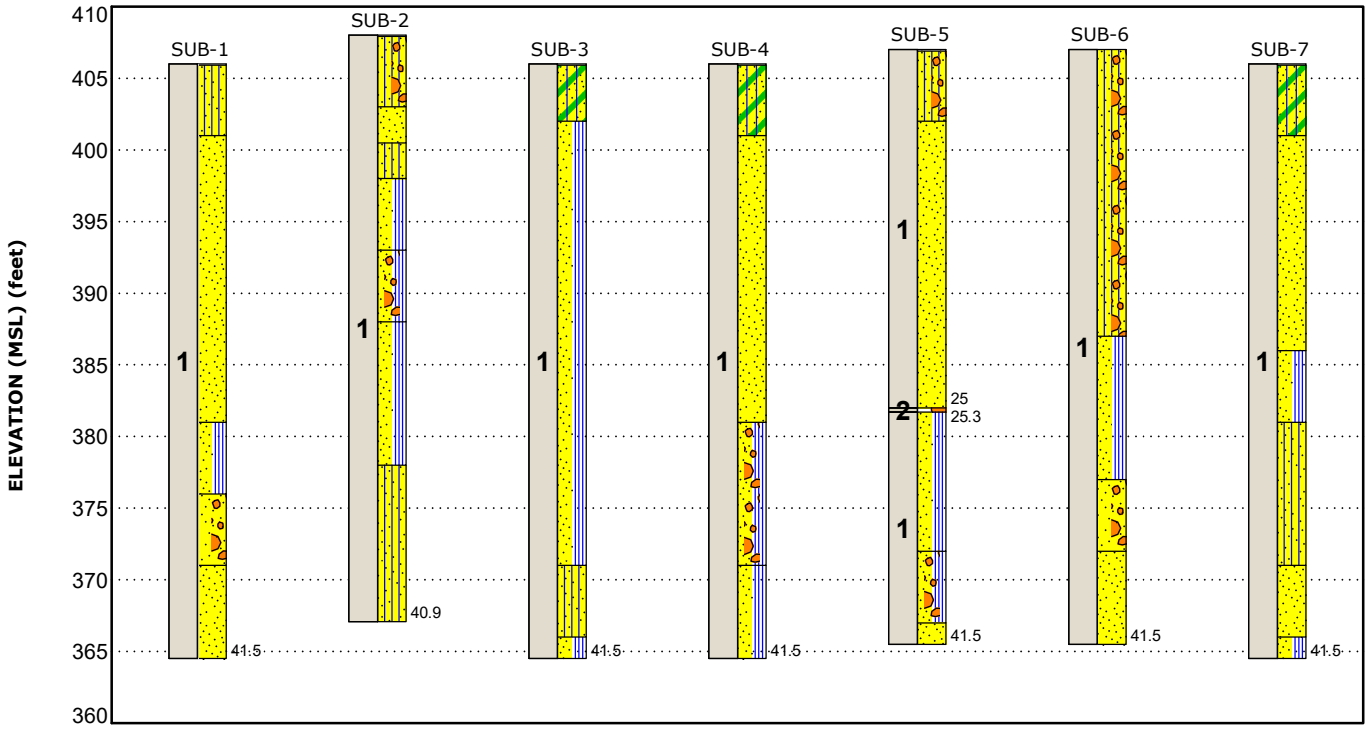
- Topsoil
- Poorly-graded Sand
- Poorly-graded Gravel with Sand
- Poorly-graded Sand with Silt and Gravel
- Poorly-graded Sand with Gravel

NOTES:
Layering shown on this figure has been developed by the geotechnical engineer for purposes of modeling the subsurface conditions as required for the subsequent geotechnical engineering for this project. Numbers adjacent to soil column indicate depth below ground surface.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

GeoModel



This is not a cross section. This is intended to display the Geotechnical Model only. See individual logs for more detailed conditions.

Model Layer	Layer Name	General Description
1	Sand	Medium dense to very dense sand with varying amounts of silt, clay, and gravel
2	Gravel	Medium dense to very dense gravel with varying amounts of silt and sand

Legend

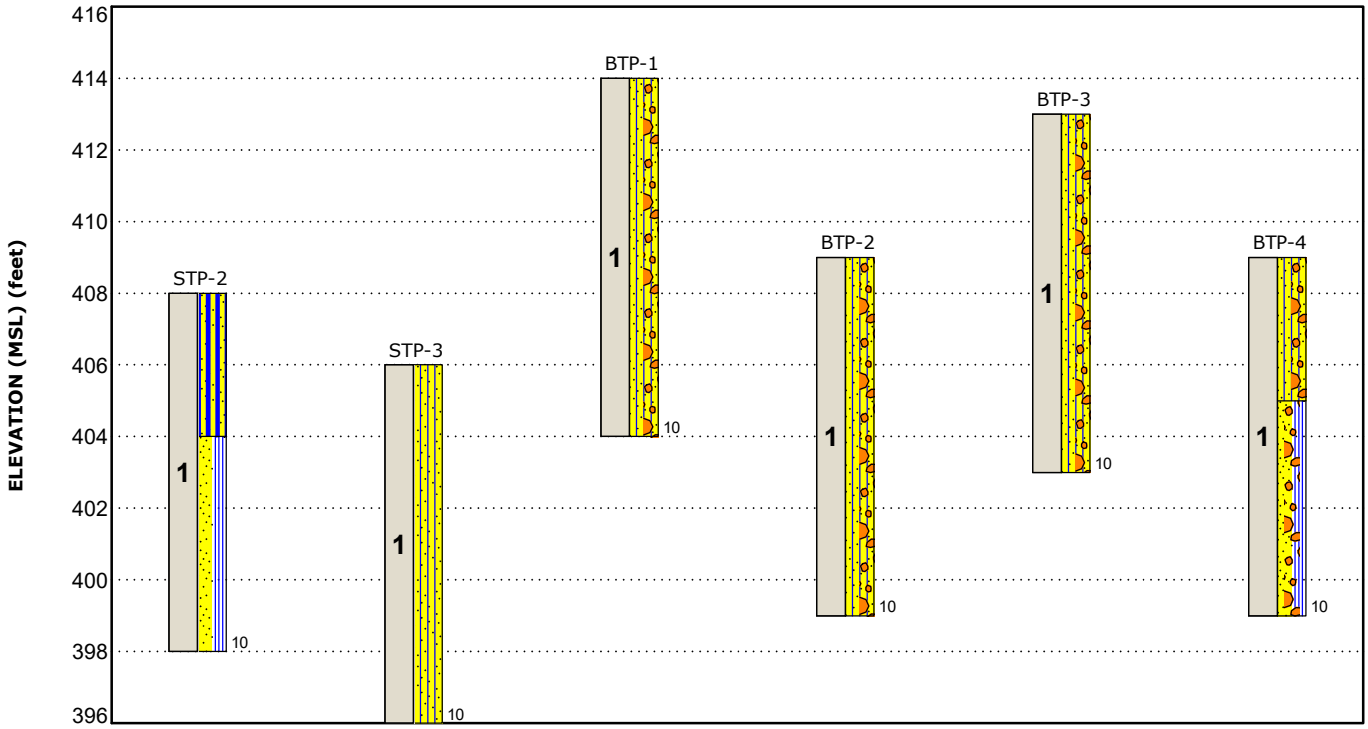
- Topsoil
- Poorly-graded Sand
- Poorly-graded Sand with Gravel
- Poorly-graded Sand with Silt and Gravel
- Poorly-graded Gravel with Sand
- Silty Sand
- Poorly-graded Sand with Silt
- Silty Sand with Gravel
- Silty Clayey Sand

NOTES:
Layering shown on this figure has been developed by the geotechnical engineer for purposes of modeling the subsurface conditions as required for the subsequent geotechnical engineering for this project. Numbers adjacent to soil column indicate depth below ground surface.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota
Ste 350
Laguna Hills, CA

GeoModel



This is not a cross section. This is intended to display the Geotechnical Model only. See individual logs for more detailed conditions.

Model Layer	Layer Name	General Description
1	Sand	Medium dense to very dense sand with varying amounts of silt, clay, and gravel
2	Gravel	Medium dense to very dense gravel with varying amounts of silt and sand

Legend

- Sandy Silt
- Poorly-graded Sand with Silt
- Silty Sand
- Silty Sand with Gravel
- Poorly-graded Sand with Silt and Gravel

NOTES:
Layering shown on this figure has been developed by the geotechnical engineer for purposes of modeling the subsurface conditions as required for the subsequent geotechnical engineering for this project. Numbers adjacent to soil column indicate depth below ground surface.

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-01

Location: See Exploration Plan

Latitude: 33.6492° Longitude: -114.7257°

Model Layer	Graphic Log	Depth (Ft.)	Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		0.1	**TOPSOIL**, poorly graded sand with gravel and caliche									
			SILTY SAND (SM), trace gravel, light brown, moderate cementation, gravel up to 1/2"									
			dense				28-38-38	C-4	1.3	102	NP	19.7
		5.0		5								
2			**POORLY GRADED GRAVEL WITH SAND (GP)**, light brown, medium dense, sample disturbed				12-23-24	B-5	0.8			
			dense, no recovery				12-28-42	B-4				
			very dense, sample disturbed	10			7-50/4"	B-3				
		12.5										
1			**POORLY GRADED SAND (SP)**, light brown, dense				14-18-23 N=41	C-4				
				15			8-14-18 N=32					
			medium dense	20			8-12-12 N=24	C-5				
		21.5	***Boring Terminated at 21.5 Feet***									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-19-2025

Boring Completed
02-19-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-02

Location: See Exploration Plan

Latitude: 33.6492° Longitude: -114.7247°

Model Layer	Depth (Ft.)	Graphic Log	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	0.1		**TOPSOIL**, poorly graded sand with gravel									
			SILTY SAND (SM), trace gravel and caliche, light brown, dense				22-32-44	C-4	1.4	103	NP	20.4
	4.0		**WELL GRADED SAND WITH SILT AND GRAVEL (SW-SM)**, light brown, dense, sample disturbed	5			12-26-36		1.1			12.0
	7.5		**POORLY GRADED SAND (SP)**, light brown, medium dense				12-24-34	C-5	0.5	96		
1			dense, no recovery	10			14-29-45	C-4				
				15			4-14-28 N=42					
	20.0		**POORLY GRADED SAND WITH GRAVEL (SP)**, light brown, very dense, gravel up to 1/2"	20			10-25-50 N=75	C-3				
	21.5		***Boring Terminated at 21.5 Feet***									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-19-2025

Boring Completed
02-19-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-03

Location: See Exploration Plan

Latitude: 33.6492° Longitude: -114.7236°

Model Layer	Depth (Ft.)	Graphic Log	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	0.1		**TOPSOIL**, poorly graded sand with gravel									
			SILTY SAND (SM), light brown, moderate cementation									
			very dense				12-42-50/5"	C-3	1.7	107	NP	21.7
			brown, sample disturbed	5			50/5"					
1	7.5		**POORLY GRADED SAND WITH GRAVEL (SP)**, light brown, medium dense				17-7-9 N=16					
			sample disturbed	10			14-24-24	C-5				
	15.0		**POORLY GRADED GRAVEL WITH SAND (GP)**, light brown, very dense	15			28-50/6"					
2								B-3				
	20.0		**POORLY GRADED SAND (SP)**, light brown, dense	20								
1	21.5						7-18-18 N=36	C-4				
			Boring Terminated at 21.5 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-18-2025

Boring Completed
02-18-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-04

Location: See Exploration Plan

Latitude: 33.6492° Longitude: -114.7227°

Model Layer	Depth (Ft.)	Material Description	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1	0.1	**TOPSOIL**, poorly graded sand with gravel							
		SILTY SAND (SM), brown, moderate cementation							
		very dense		30-50/5"	C-3	2.3	88	NP	26.9
	5.0								
2		**POORLY GRADED GRAVEL WITH SAND (GP)**, light brown, very dense, gravel up to 2 1/2", sample disturbed		31-50/5"	B-3				
	7.5								
		POORLY GRADED SAND WITH SILT (SP-SM), light brown, medium dense		7-19-24	C-5	0.8	94		
	10.0								
		SILTY SAND WITH GRAVEL (SM), light brown, very dense, moderate cementation, gravel up to 1"		21-50/5"	C-3	1.1	108		
	15.0								
1		**POORLY GRADED SAND (SP)**, light brown dense		9-17-26 N=43	C-4				
				14-15-17 N=32					
		very dense		13-30-40 N=70					

Depth (Ft.) markers: 5, 10, 15, 20, 25

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-19-2025

Boring Completed
02-19-2025

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-04

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6492° Longitude: -114.7227° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		**POORLY GRADED SAND (SP)**, light brown *(continued)*					C-3				
		trace silt, dense	30			12-17-22 N=39	C-4				
		very dense, moderate cementation	35			15-24-36 N=60	C-3				
		41.5	40			16-26-32 N=58					
		Boring Terminated at 41.5 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-19-2025

Boring Completed
02-19-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-05

Location: See Exploration Plan

Latitude: 33.6483° Longitude: -114.7262°

Model Layer	Depth (Ft.)	Graphic Log	Material Description	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	0.1		**TOPSOIL**, poorly graded sand with silt and gravel								
			POORLY GRADED SAND WITH SILT (SP-SM), light brown, moderate cementation								
			dense			30-34-44	C-4	2.5	105	NP	11.3
	5		medium dense			12-19-25		1.0	109		
	7.5		**POORLY GRADED SAND (SP)**, light brown, medium dense, sample disturbed			12-17-22		0.7			
1	10					9-18-28		0.3	100		
	15					9-11-13 N=24	C-5				
	20		dense			8-14-16 N=30	C-4				
	21.5		***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-18-2025

Boring Completed
02-18-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-06

Location: See Exploration Plan

Latitude: 33.6483° Longitude: -114.7251°

Model Layer	Depth (Ft.)	Graphic Log	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1	0.1		**TOPSOIL**, poorly graded gravel with sand									
			POORLY GRADED SAND WITH GRAVEL (SP), light brown, moderate cementation									
			dense				12-40-44		1.3	103	NP	
			brown, medium dense, sample disturbed	5			14-24-30		0.6			
	7.5		**POORLY GRADED SAND (SP)**, light brown, dense, sample disturbed				14-24-36		0.4			
	10.0		**POORLY GRADED SAND WITH GRAVEL (SP)**, light brown, dense, gravel up to 1 1/2", sample disturbed	10			12-32-44	C-4	0.9			
	15.0		**SILTY SAND WITH GRAVEL (SM)**, light brown to brown, dense, gravel up to 1/2"	15			14-24-22 N=46					
	20.0		**POORLY GRADED SAND (SP)**, trace silt, light brown dense	20			9-13-23 N=36					
			medium dense	25			8-10-12 N=22					

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-18-2025

Boring Completed
02-18-2025

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-06

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6483° Longitude: -114.7251° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		**POORLY GRADED SAND (SP)**, trace silt, light brown *(continued)*					C-5				
		very dense	30			14-24-30 N=54	C-3				
		dense	35			13-21-26 N=47	C-4				
			40			11-17-19 N=36					
		41.5 ***Boring Terminated at 41.5 Feet***									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-18-2025

Boring Completed
02-18-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-07

Location: See Exploration Plan

Latitude: 33.6483° Longitude: -114.7242°

Model Layer	Depth (Ft.)	Graphic Log	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	0.1		**TOPSOIL**, poorly graded gravel with sand									
			SILTY SAND (SM), light brown, moderate cementation									
			medium dense				4-16-30	C-5	1.2	105	NP	18.6
	5.0		**SILTY SAND WITH GRAVEL (SM)**, light brown, very dense, moderate cementation, gravel up to 3/4", sample disturbed	5			26-50/5"	C-3	1.9			
	7.5		**POORLY GRADED SAND (SP)**, light brown, medium dense				14-24-30	C-5	0.2	104		
1			dense	10			12-24-34	C-4	0.5	104		
			very dense	15			18-24-30 N=54	C-3				
			dense	20			9-17-22 N=39	C-4				
	21.5		***Boring Terminated at 21.5 Feet***									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-18-2025

Boring Completed
02-18-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-08

Location: See Exploration Plan

Latitude: 33.6482° Longitude: -114.7232°

Model Layer	Depth (Ft.)	Description	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1	0.1	**TOPSOIL**, poorly graded sand with gravel								
		SILTY SAND (SM), trace gravel, light brown, moderate cementation				C-4				
		dense			16-26-44		0.7	99	NP	16.8
	4.0	**POORLY GRADED SAND WITH SILT AND GRAVEL (SP-SM)**, light brown, medium dense, sample disturbed, gravel up to 3/4"			10-20-24		0.4			10.4
		sample disturbed			16-22-30		1.0			
		sample disturbed			8-16-22	C-5	0.5			
	15.0	**POORLY GRADED SAND (SP)**, trace silt, light brown loose, sample disturbed			4-7-9		2.6			
		medium dense			9-11-14 N=25	C-6				
					7-11-15 N=26	C-5				

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-18-2025

Boring Completed
02-18-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-08

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6482° Longitude: -114.7232° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		**POORLY GRADED SAND (SP)**, trace silt, light brown *(continued)* 30.0	30			14-30-42 N=72	C-3				
		POORLY GRADED SAND WITH SILT (SP-SM), light brown, very dense									
		dense	35			7-17-25 N=42	C-4				
		40.0									
		POORLY GRADED SAND (SP), light brown, very dense 41.5	40			14-24-38 N=62	C-3				
		Boring Terminated at 41.5 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-18-2025

Boring Completed
02-18-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-09

Location: See Exploration Plan

Latitude: 33.6475° Longitude: -114.7256°

Model Layer	Depth (Ft.)	Graphic Log	Material Description	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	0.1		**TOPSOIL**, poorly graded sand with silt and gravel								
1			**SILTY SAND (SM)**, trace gravel, light brown, moderate cementation medium dense			15-21-25	C-4	1.0	102	NP	13.5
	4.0		**POORLY GRADED SAND (SP)**, trace silt, light brown, medium dense, sample disturbed			11-17-25		0.5			2.7
	7.5		**POORLY GRADED GRAVEL WITH SAND (GP)**, dense, gravel up to 1 1/2"			19-30-49	B-4	0.2	105		
2			very dense, sample disturbed			18-36-50/5"	B-3	1.5			
	15.0		**POORLY GRADED SAND (SP)**, light brown, dense			10-15-20 N=35	C-4				
1			medium dense			10-11-13 N=24	C-5				
	25.0		**POORLY GRADED GRAVEL WITH SILT AND SAND (GP-GM)**, light brown, gravel up to 1 1/4"			20-50/2"					
2			very dense								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-17-2025

Boring Completed
02-17-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-09

Location: See Exploration Plan

Latitude: 33.6475° Longitude: -114.7256°

Model Layer	Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
2		**POORLY GRADED GRAVEL WITH SILT AND SAND (GP-GM)**, light brown, gravel up to 1 1/4" *(continued)*				B-3				
		30.0								
1		**POORLY GRADED SAND (SP)**, light brown, very dense	30		16-30-34 N=64	C-3				
		dense	35		9-19-26 N=45	C-4				
		very dense	40		11-24-36 N=60	C-3				
		41.5								
		Boring Terminated at 41.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-17-2025

Boring Completed
02-17-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-10

Location: See Exploration Plan

Latitude: 33.6475° Longitude: -114.7247°

Model Layer	Graphic Log	Depth (Ft.)	Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
		0.1	**TOPSOIL**, poorly graded sand with silt and gravel									
1			**POORLY GRADED SAND WITH SILT (SP-SM)**, brown, moderate cementation									
			medium dense, sample disturbed				18-22-30	C-5	1.7		NP	8.6
		5.0		5								
2			**POORLY GRADED GRAVEL WITH SAND (GP)**, light brown, medium dense, gravel to 1 1/2", sample disturbed				18-18-20		0.3			
			no recovery				10-20-35	B-5				
		10.0		10								
			POORLY GRADED SAND WITH GRAVEL (SP), light brown, medium dense, gravel up to 1"				3-8-11 N=19	C-5				
		15.0		15								
1			**POORLY GRADED SAND (SP)**, light brown, very dense, moderate cementation				12-28-30 N=58	C-3				
			dense	20			9-15-20 N=35	C-4				
		21.5	***Boring Terminated at 21.5 Feet***									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-17-2025

Boring Completed
02-17-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-11

Location: See Exploration Plan

Latitude: 33.6475° Longitude: -114.7237°

Model Layer	Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		0.1 **TOPSOIL**, poorly graded sand with silt and gravel								
		POORLY GRADED SAND WITH SILT AND GRAVEL (SP-SM), light brown, gravel up to 1"				C-4			NP	9.7
		dense			22-28-36		1.4	84		
2		4.0 **POORLY GRADED GRAVEL WITH SAND (GP)**, light brown, gravel up to 1 1/2", sample disturbed								
		dense			16-26-35	B-4	0.4			
		7.5 **POORLY GRADED SAND (SP)**, light brown, dense, moderate cementation			32-40-44		3.3	103		
1					14-26-36	C-4	1.5	101		
		medium dense			6-12-16 N=28					
						C-5				
		dense			7-16-18 N=34	C-4				
		21.5 ***Boring Terminated at 21.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-17-2025

Boring Completed
02-17-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-12

Location: See Exploration Plan

Latitude: 33.6474° Longitude: -114.7226°

Model Layer	Depth (Ft.)	Graphic Log	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1	0.1		**TOPSOIL**, poorly graded sand with gravel									
			SILTY CLAYEY SAND WITH GRAVEL (SC-SM), light brown									
			very dense, no recovery				50/2"				21-15-6	14.8
	5.0		**SILTY SAND WITH GRAVEL (SM)**, light brown, very dense, no recovery	5			50/4"					
							35-20-45 N=65					
			no recovery				50/3"					
							38-50/6"					
				10			16-36-38 N=74	C-3				
				15			28-50/5"					
	20.0		**POORLY GRADED SAND (SP)**, light brown, very dense	20			11-28-32 N=60					
	21.5		***Boring Terminated at 21.5 Feet***									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-14-2025

Boring Completed
02-14-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-13

Location: See Exploration Plan

Latitude: 33.6471° Longitude: -114.7252°

Model Layer	Depth (Ft.)	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1	0.1	**TOPSOIL**, poorly graded gravel with silt and sand									
		POORLY GRADED SAND WITH SILT (SP-SM), trace gravel, light brown, moderate cementation									
		dense				24-36-31	C-4	1.9	103	NP	11.1
		medium dense, trace gravel up to 3/4", sample disturbed	5			6-7-12	C-5	1.5			
	7.5										
2		**POORLY GRADED GRAVEL WITH SAND (GP)**, light brown, medium dense, sample disturbed				14-16-26	B-5	0.7			
		sample disturbed	10			12-20-24		0.7			
		trace cobbles, very dense, no recovery	15			50/2"					
							B-3				
	20.2	no recovery	20			50/2"					
		Boring Terminated at 20.2 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-14-2025

Boring Completed
02-14-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-14

Location: See Exploration Plan

Latitude: 33.6471° Longitude: -114.7242°

Model Layer	Depth (Ft.)	Material Description	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1	0.1	**TOPSOIL**, poorly graded sand with gravel **SILTY SAND (SM)**, light brown							
		dense, sample disturbed		14-29-31	C-4	0.9		NP	16.1
	4.0								
2		**POORLY GRADED GRAVEL WITH SAND (GP)**, light brown							
	5	medium dense, sample disturbed		6-20-20		1.4			
					B-5				
		sample disturbed		10-20-26		1.2			
	10	very dense, sample disturbed		22-50/2"		0.9			
					B-3				
	15.0								
1		**POORLY GRADED SAND (SP)**, light brown dense, moderate cementation		22-38-48		1.8	104		
	20	trace silt		10-20-25 N=45	C-4				
	25			9-18-21 N=39					

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-14-2025

Boring Completed
02-14-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-14

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6471° Longitude: -114.7242° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		**POORLY GRADED SAND (SP)**, light brown *(continued)* very dense	30			20-32-40 N=72	C-3				
			35			10-24-27 N=51					
			40			10-25-28 N=53					
		41.5 ***Boring Terminated at 41.5 Feet***									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-14-2025

Boring Completed
02-14-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. BESS-15

Location: See Exploration Plan

Latitude: 33.6471° Longitude: -114.7230°

Model Layer	Depth (Ft.)	Graphic Log	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
	0.1		**TOPSOIL**, poorly graded sand with gravel									
			SILTY SAND (SM), trace gravel, light brown					C-3				
			very dense, sample disturbed				22-50/4"		2.4			25.3
			dense, sample disturbed	5			20-35-45	C-4	1.2			13.4
	7.5		**POORLY GRADED SAND WITH GRAVEL (SP)**, light brown, very dense, moderate cementation, sample disturbed				22-50/3"		2.5			
1			sample disturbed	10			50/5"	C-3	3.1			
	15.0		**POORLY GRADED SAND (SP)**, light brown, medium dense	15			8-12-16 N=28					
			dense	20			8-18-28 N=46	C-5 C-4				
	21.5		***Boring Terminated at 21.5 Feet***									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-14-2025

Boring Completed
02-14-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. PERC-01

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6467° Longitude: -114.7255° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		0.1 **TOPSOIL**, poorly graded sand with gravel **POORLY GRADED SAND WITH SILT AND GRAVEL (SP-SM)**, light brown, moderate cementation, gravel up to 1/2" medium dense, sample disturbed 3.0				13-20-29	C-5	1.1			
		Boring Terminated at 3 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-11-2025

Boring Completed
02-11-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. PERC-02

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6467° Longitude: -114.7251° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		0.1 **TOPSOIL**, poorly graded sand with gravel **POORLY GRADED SAND (SP)**, trace gravel, light brown, gravel up to 3/4" medium dense, sample disturbed 5.0	5			16-25-33	C-5	1.3			
		Boring Terminated at 5 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-11-2025

Boring Completed
02-11-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. PERC-03

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6467° Longitude: -114.7246° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		0.1 **TOPSOIL**, poorly graded sand with gravel **POORLY GRADED SAND (SP)**, trace gravel, light brown, gravel up to 1/2" dense, sample disturbed 3.0				29-40-42	C-4	1.1			
		Boring Terminated at 3 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-12-2025

Boring Completed
02-12-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. PERC-04

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6467° Longitude: -114.7240° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		0.1 **TOPSOIL**, poorly graded sand with gravel **POORLY GRADED SAND (SP)**, trace gravel, light brown, gravel up to 1/2" medium dense, sample disturbed 5.0	5			8-15-21	C-5	0.6			
		Boring Terminated at 5 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-11-2025

Boring Completed
02-11-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. PERC-05

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6467° Longitude: -114.7235° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		0.1 **TOPSOIL**, poorly graded sand with gravel **POORLY GRADED SAND WITH GRAVEL (SP)**, light brown, gravel up to 1 1/2" medium dense, sample disturbed 3.0				14-20-24	C-5	0.9			
		Boring Terminated at 3 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-11-2025

Boring Completed
02-11-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. PERC-06

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6467° Longitude: -114.7228° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
2		0.1 **TOPSOIL**, poorly graded sand with gravel **POORLY GRADED GRAVEL WITH SAND (GP)**, light brown very dense, no recovery 5.0	5			34-50/5"	B-3				
		Boring Terminated at 5 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-11-2025

Boring Completed
02-11-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. PERC-07

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6467° Longitude: -114.7221° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		0.1 **TOPSOIL**, poorly graded sand with gravel **POORLY GRADED SAND (SP)**, light brown medium dense 3.0				5-8-11	C-5	3.4	114		
		Boring Terminated at 3 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-11-2025

Boring Completed
02-11-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. PERC-08

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6467° Longitude: -114.7214° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		0.1 **TOPSOIL**, poorly graded sand with gravel **POORLY GRADED SAND WITH SILT AND GRAVEL (SP-SM)**, light brown, gravel up to 1" dense, sample disturbed 5.0	5			18-38-48	C-4	0.6			
		Boring Terminated at 5 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-11-2025

Boring Completed
02-11-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. PERC-09

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6467° Longitude: -114.7208° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		0.1 **TOPSOIL**, poorly graded sand with gravel **POORLY GRADED SAND WITH GRAVEL (SP)**, brown, gravel up to 3/4" medium dense, sample disturbed 3.0				9-13-22	C-5	1.0			
		Boring Terminated at 3 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-11-2025

Boring Completed
02-11-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. PERC-10

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6467° Longitude: -114.7201° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		0.1 **TOPSOIL**, poorly graded sand with silt and gravel **POORLY GRADED SAND WITH SILT AND GRAVEL (SP-SM)**, brown, moderate cementation, gravel up to 1/2" medium dense, sample disturbed 5.0 ***Boring Terminated at 5 Feet***	5			14-18-20	C-5	1.3			

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-11-2025

Boring Completed
02-11-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. PERC-11

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6466° Longitude: -114.7196° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		0.1 **TOPSOIL**, poorly graded sand with silt and gravel **POORLY GRADED SAND (SP)**, trace gravel, light brown, gravel up to 1" medium dense, sample disturbed 3.0				12-18-22	C-5	1.2			
		Boring Terminated at 3 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Logged by
DD

Boring Started
02-11-2025

Boring Completed
02-11-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. SUB-1

Location: See Exploration Plan

Latitude: 33.6489° Longitude: -114.7207°

Model Layer	Depth (Ft.)	Graphic Log	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1	0.1		**TOPSOIL**, silty sand with gravel									
			SILTY SAND (SM), trace gravel, brown, gravel up to 1/2"									
			dense, sample disturbed				16-29-30	C-4	2.2		NP	19.1
	5.0		**POORLY GRADED SAND (SP)**, light brown, medium dense, sample disturbed	5			16-18-20		1.2			
			sample disturbed				9-13-18		0.9			
			sample disturbed	10			12-18-26		0.5			
				15			9-12-14 N=26	C-5				
				20			4-8-11 N=19					
	25.0		**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown, gravel up to 1 1/2" very dense	25			18-50/4"					

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-13-2025

Boring Completed
02-13-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. SUB-1

Location: See Exploration Plan

Latitude: 33.6489° Longitude: -114.7207°

Model Layer	Graphic Log	Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown, gravel up to 1 1/2" *(continued)*					C-3				
		30.0	30			15-22-28 N=50					
		POORLY GRADED SAND WITH GRAVEL (SP), light brown, dense, gravel up to 1/2"									
		35.0	35			11-16-24 N=40	C-4				
		POORLY GRADED SAND (SP), trace gravel, light brown, dense, gravel up to 1/2"									
		very dense	40			12-24-42 N=66	C-3				
		41.5									
		Boring Terminated at 41.5 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-13-2025

Boring Completed
02-13-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. SUB-2

Location: See Exploration Plan

Latitude: 33.6487° Longitude: -114.7211°

Model Layer	Depth (Ft.)	Graphic Log	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1	0.1		**TOPSOIL**, silty sand with gravel									
			SILTY SAND WITH GRAVEL (SM), light brown, gravel up to 3/4", dense, sample disturbed				30-36-40	C-4	2.3		NP	18.3
	5.0		**POORLY GRADED SAND (SP)**, trace gravel, light brown, medium dense, sample disturbed	5			15-20-25					
	7.5		**SILTY SAND (SM)**, traces of caliche, light brown to white, medium dense, moderate cementation, sample disturbed				7-15-20		2.9			
	10.0		**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown, medium dense, sample disturbed	10			7-16-22	C-5	1.0			
	15.0		**POORLY GRADED SAND WITH SILT AND GRAVEL (SP-SM)**, light brown, very dense, gravel up to 1"	15			22-50/5"					
	20.0		**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown, moderate cementation, very dense	20			14-32-40 N=72	C-3				
			dense	25			10-13-22 N=35					

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-13-2025

Boring Completed
02-13-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. SUB-2

Model Layer	Graphic Log	Location: See Exploration Plan Latitude: 33.6487° Longitude: -114.7211° Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown, moderate cementation *(continued)* 30.0									
		SILTY SAND (SM), light brown, dense	30			6-18-20 N=38	C-4				
		trace gravel, very dense, moderate cementation	35			10-22-30 N=52	C-3				
		40.9	40			30-50/5"					
		Boring Terminated at 40.9 Feet									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).
See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-13-2025

Boring Completed
02-13-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. SUB-3

Location: See Exploration Plan

Latitude: 33.6484° Longitude: -114.7212°

Model Layer	Depth (Ft.)	Graphic Log	Material Description	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1	0.1		**TOPSOIL**, silty sand with gravel								
			SILTY CLAYEY SAND (SC-SM), with caliche, brown medium dense			20-28-25		3.4	106	19-13-6	24.6
	4.0		**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown medium dense sample disturbed			10-13-18		0.9			
	5		sample disturbed			12-16-20		1.6			
	10		sample disturbed			8-16-18		0.6			
	15					5-9-10 N=19	C-5				
	20		no recovery			4-6-9 N=15					
	25					6-8-17 N=25					

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-12-2025

Boring Completed
02-12-2025

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. SUB-3

Location: See Exploration Plan

Latitude: 33.6484° Longitude: -114.7212°

Model Layer	Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown *(continued)*								
		30			6-7-7 N=14					
		35.0								
		SILTY SAND (SM), light brown, very dense, moderate cementation								
		35			18-50/5"					27.6
		40.0				C-3				
		POORLY GRADED SAND WITH SILT (SP-SM), light brown, dense								
		40			13-18-21 N=39	C-4				
		41.5								
		Boring Terminated at 41.5 Feet								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-12-2025

Boring Completed
02-12-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. SUB-4

Location: See Exploration Plan

Latitude: 33.6487° Longitude: -114.7204°

Model Layer	Depth (Ft.)	Graphic Log	Material Description	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1	0.1		**TOPSOIL**, silty sand with gravel								
			SILTY CLAYEY SAND (SC-SM), brown								
			medium dense, sample disturbed			9-14-16		4.0		19-15-4	20.7
	5.0		**POORLY GRADED SAND (SP)**, trace silt, brown, medium dense			8-13-15		3.5	101		
			sample disturbed			10-13-15		1.0			
	10		sample disturbed			7-9-15	C-5	0.7			
	15					5-8-10 N=18					
	20		dense, trace gravel up to 1/2"			7-10-20 N=30					
	25.0		**POORLY GRADED SAND WITH SILT AND GRAVEL (SP-SM)**, light brown, trace gravel up to 1" dense			12-20-28 N=48	C-4				

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-13-2025

Boring Completed
02-13-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. SUB-4

Location: See Exploration Plan

Latitude: 33.6487° Longitude: -114.7204°

Model Layer	Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		**POORLY GRADED SAND WITH SILT AND GRAVEL (SP-SM)**, light brown, trace gravel up to 1" *(continued)*								
		very dense			30-38-30 N=68	C-3				
		35.0 **POORLY GRADED SAND WITH SILT (SP-SM)**, light brown, very dense			12-24-28 N=52					
		dense			9-20-21 N=41	C-4				
		41.5 ***Boring Terminated at 41.5 Feet***								

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-13-2025

Boring Completed
02-13-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. SUB-5

Location: See Exploration Plan

Latitude: 33.6483° Longitude: -114.7204°

Model Layer	Depth (Ft.)	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1	0.1	**TOPSOIL**, silty sand with gravel									
		SILTY SAND WITH GRAVEL (SM), light brown, gravel up to 3/4" dense				24-38-40	C-4			NP	19.4
	5.0	**POORLY GRADED SAND (SP)**, light brown, medium dense, sample disturbed	5			16-16-25		1.2			
		no recovery				11-20-32					
		sample disturbed	10			8-18-28		0.5			
		trace cobbles	15			8-12-15 N=27	C-5				
			20			7-10-11 N=21					
2	25.0		25								
1	25.3	**POORLY GRADED GRAVEL WITH SAND (GP)**, brown, very dense **POORLY GRADED SAND WITH SILT (SP-SM)**, light brown very dense				28-31-50/6"	B-3				

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-12-2025

Boring Completed
02-12-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. SUB-5

Location: See Exploration Plan

Latitude: 33.6483° Longitude: -114.7204°

Model Layer	Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown *(continued)*								
		very dense			28-50/5"					
		35.0				C-3				
		POORLY GRADED SAND WITH SILT AND GRAVEL (SP-SM), light brown, very dense, gravel up to 1"			20-25-43 N=68					
		40.0								
		POORLY GRADED SAND (SP), trace silt, light brown, very dense			12-24-28 N=52					
		41.5								
		Boring Terminated at 41.5 Feet								

30 35 40

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Boring Started
02-12-2025

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Completed
02-12-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. SUB-6

Location: See Exploration Plan

Latitude: 33.6486° Longitude: -114.7208°

Model Layer	Depth (Ft.)	Material Description	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1	0	**SILTY SAND WITH GRAVEL (SM)**, light brown very dense, sample disturbed			32-50/5"	C-3	2.1		NP	16.6
	5	medium dense, sample disturbed			10-11-13	C-5	1.3			
		sample disturbed			9-15-18		0.8			
	10	gravel up to 3/4", sample disturbed			9-17-22		0.5			
	15	very dense, moderate cementation			50/5"	C-3				
	20	20.0 **POORLY GRADED SAND WITH SILT (SP-SM)**, light brown dense, moderate cementation			15-22-28 N=50	C-4				
	25	very dense, weak cementation, trace gravel up to 1/2"			12-24-32 N=56					

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-12-2025

Boring Completed
02-12-2025

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. SUB-6

Location: See Exploration Plan

Latitude: 33.6486° Longitude: -114.7208°

Model Layer	Graphic Log	Depth (Ft.)	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1			**POORLY GRADED SAND WITH SILT (SP-SM)**, light brown *(continued)*					C-3				
		30.0		30			12-25-25 N=50					
			POORLY GRADED SAND WITH GRAVEL (SP), light brown to gray, dense, gravel up to 1"									
		35.0		35			9-14-20 N=34	C-4				
			POORLY GRADED SAND (SP), light brown, dense									
			very dense, trace gravel up to 3/4"	40			24-32-32 N=64	C-3				
		41.5	***Boring Terminated at 41.5 Feet***									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-12-2025

Boring Completed
02-12-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. SUB-7

Location: See Exploration Plan

Latitude: 33.6489° Longitude: -114.7203°

Model Layer	Depth (Ft.)	Description	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1	0.1	**TOPSOIL**, silty sand with gravel								
		SILTY CLAYEY SAND (SC-SM), trace gravel, brown, gravel up to 1/2"								
		dense, sample disturbed			24-40-40	C-4	2.4		19-13-6	21.0
	5.0									
		POORLY GRADED SAND (SP), light brown, medium dense, sample disturbed			9-14-17		1.2			
		trace gravel up to 3/4", sample disturbed			7-13-15		0.6			
		sample disturbed			5-12-18		0.5			
						C-5				
	20.0				6-9-10 N=19					
		POORLY GRADED SAND WITH SILT (SP-SM), light brown, very dense, moderate cementation			6-30-48 N=78					
	25.0									
		SILTY SAND (SM), trace gravel, light brown, gravel up to 1" very dense			24-50/5"					

Depth (Ft.) ticks: 5, 10, 15, 20, 25

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-13-2025

Boring Completed
02-13-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Boring Log No. SUB-7

Location: See Exploration Plan

Latitude: 33.6489° Longitude: -114.7203°

Model Layer	Graphic Log	Depth (Ft.)	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		**SILTY SAND (SM)**, trace gravel, light brown, gravel up to 1" *(continued)*									
		light brown to brown, very dense, weak cementation, gravel up to 1/2"	30			20-40-40 N=80	C-3				
		35.0 **POORLY GRADED SAND (SP)**, light brown, very dense	35			14-36-48 N=84					
		40.0 **POORLY GRADED SAND WITH SILT (SP-SM)**, light brown, dense	40			12-16-16 N=32	C-4				
		41.5 ***Boring Terminated at 41.5 Feet***									

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Drill Rig
GTech Drill GT-16

Hammer Type
Automatic

Driller
2R Drilling

Notes

Advancement Method
8" Hollow Stem Auger

Logged by
DD

Abandonment Method
Boring backfilled with auger cuttings upon completion.

Boring Started
02-13-2025

Boring Completed
02-13-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Test Pit Log No. STP-2

Location: See Exploration Plan

Latitude: 33.6487° Longitude: -114.7213°

Model Layer	Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
		SANDY SILT (ML), with caliche, light brown and white								
		4.0								56.3
1		**POORLY GRADED SAND WITH SILT (SP-SM)**, brown to light brown								7.2
		10.0								
		Test Pit Terminated at 10 Feet								



See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Logged by
AS

Test Pit Started
05-19-2025

Test Pit Completed
05-19-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

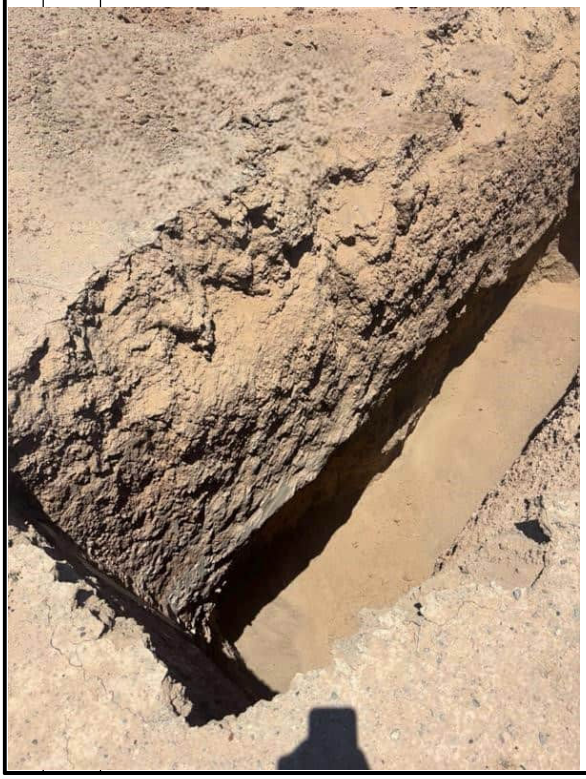
Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Test Pit Log No. STP-3

Location: See Exploration Plan

Latitude: 33.6483° Longitude: -114.7201°

Model Layer	Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		**SILTY SAND (SM)**, with caliche, trace gravel, light brown								13.3
										17.9
		10.0								
		Test Pit Terminated at 10 Feet								



See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Logged by
AS

Test Pit Started
05-19-2025

Test Pit Completed
05-19-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

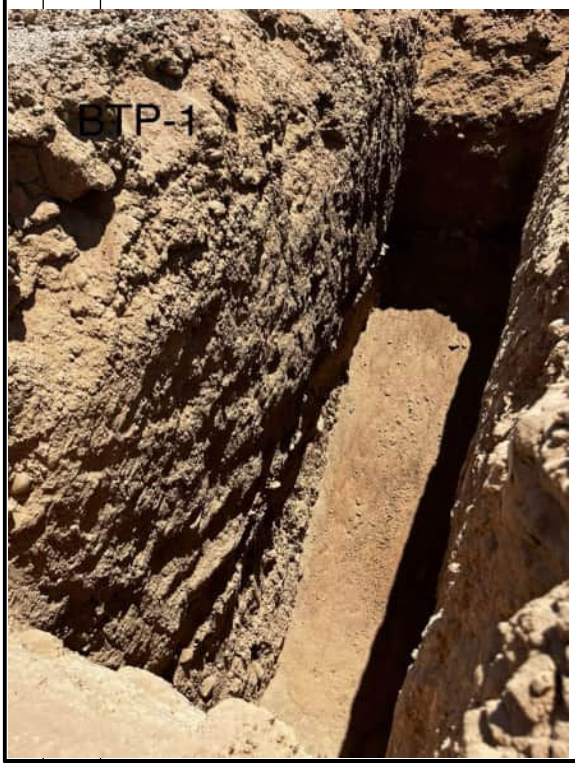
Terracon
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Laguna Hills, CA

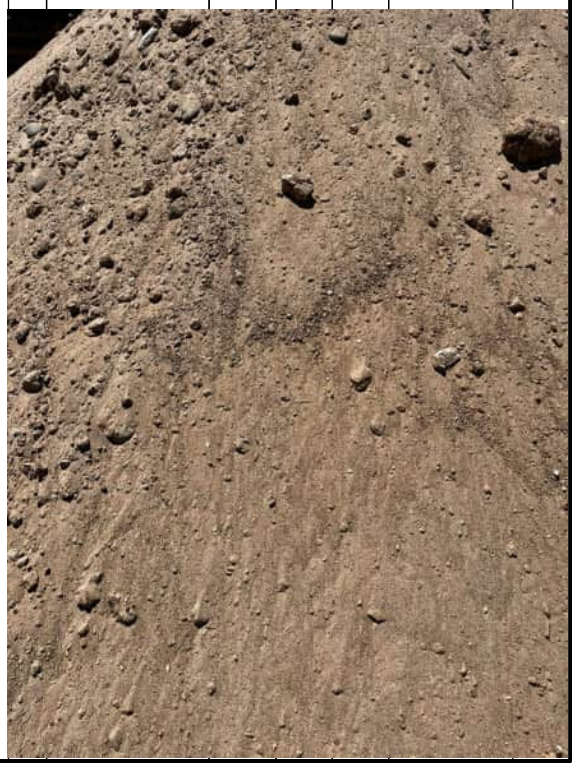
Test Pit Log No. BTP-1

Location: See Exploration Plan

Latitude: 33.6471° Longitude: -114.7260°

Model Layer	Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		**SILTY SAND WITH GRAVEL (SM)**, light brown trace cobbles 10.0								
		Test Pit Terminated at 10 Feet								





See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Notes

Advancement Method
Bucket

Logged by
EO

Test Pit Started
05-21-2025

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Test Pit Completed
05-21-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

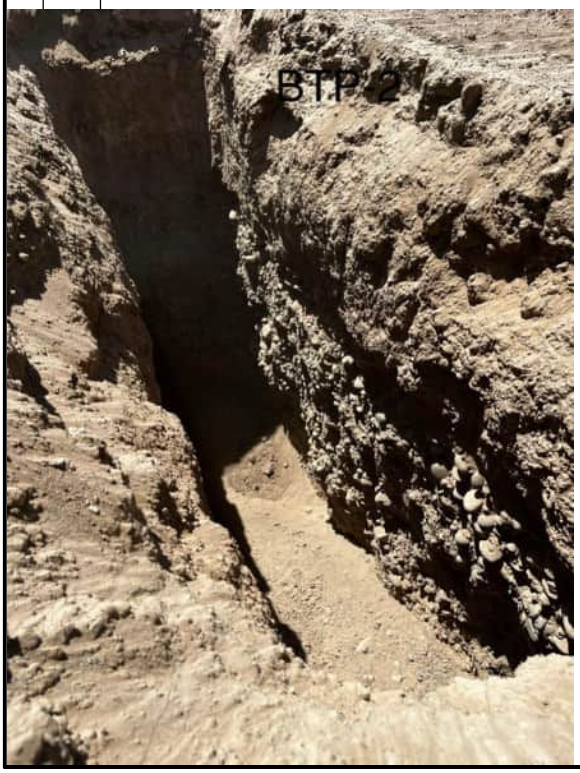
Test Pit Log No. BTP-2

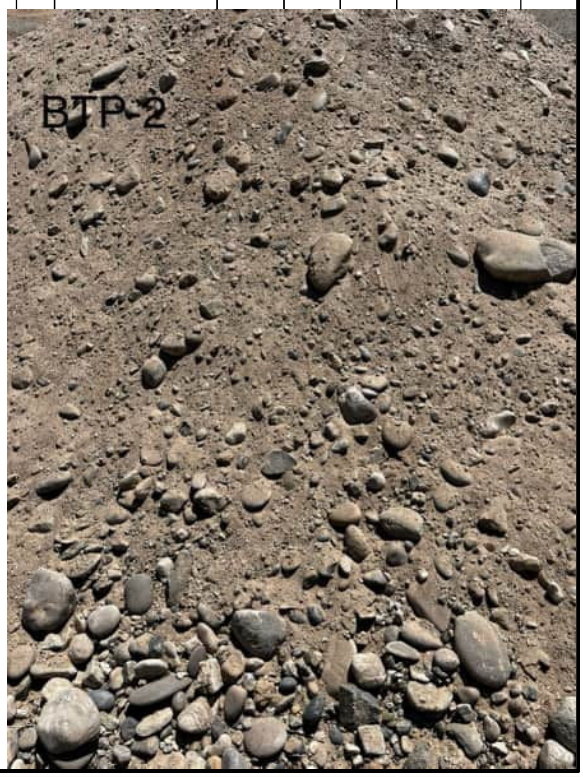
Location: See Exploration Plan

Latitude: 33.6479° Longitude: -114.7235°

Depth (Ft.)

Model Layer	Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		**SILTY SAND WITH GRAVEL (SM)**, light brown								
		10.0								
		Test Pit Terminated at 10 Feet								





See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Notes

Advancement Method
Bucket

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Logged by
EO

Test Pit Started
05-21-2025

Test Pit Completed
05-21-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

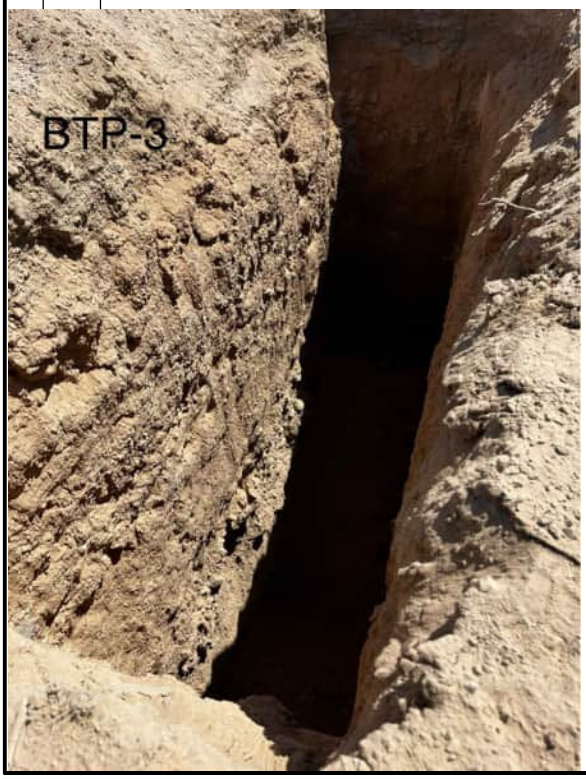
Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

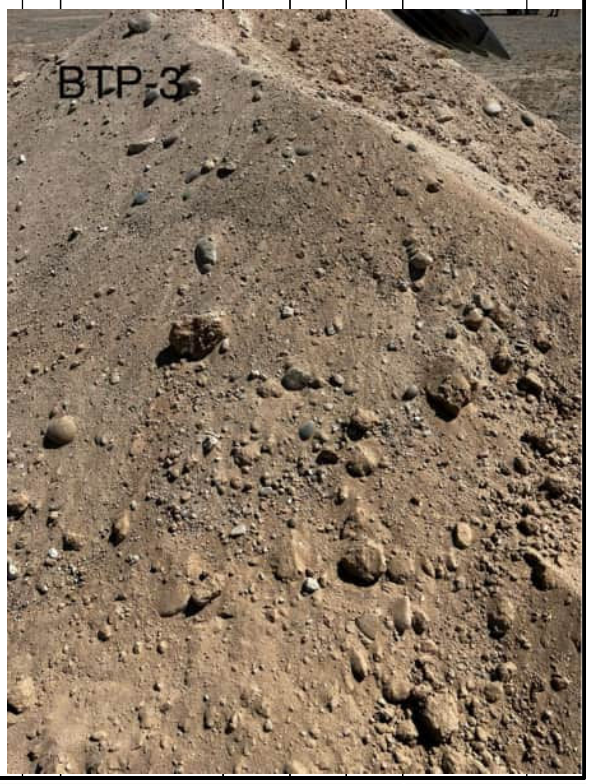
Test Pit Log No. BTP-3

Location: See Exploration Plan

Latitude: 33.6488° Longitude: -114.7252°

Model Layer	Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		**SILTY SAND WITH GRAVEL (SM)**, trace cobbles, light brown								
		10.0								
		Test Pit Terminated at 10 Feet								





See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Notes

Advancement Method
Bucket

Logged by
EO

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Test Pit Started
05-21-2025

Test Pit Completed
05-21-2025

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

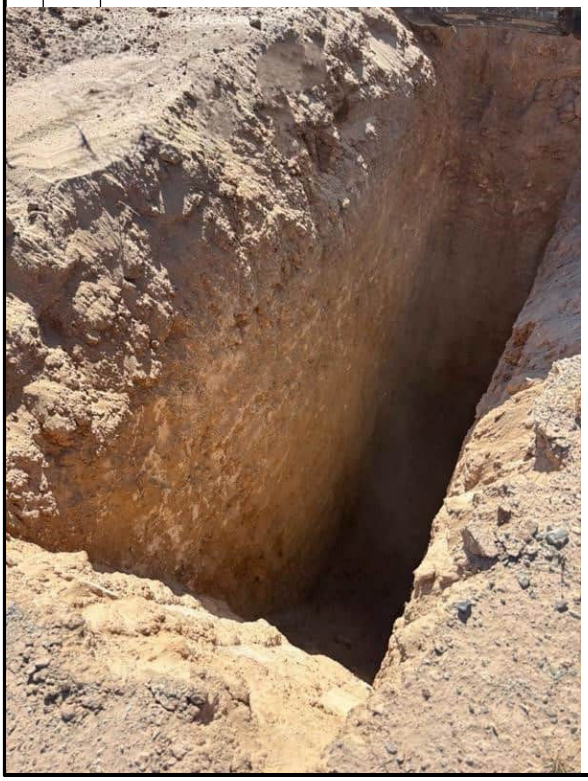
Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Test Pit Log No. BTP-4

Location: See Exploration Plan

Latitude: 33.6495° Longitude: -114.7229°

Model Layer	Graphic Log	Depth (Ft.)	Description	Depth (Ft.)	Water Level Observations	Sample Type	Field Test Results	SBC	Water Content (%)	Dry Unit Weight (pcf)	Atterberg Limits LL-PL-PI	Percent Fines
1		4.0	**SILTY SAND WITH GRAVEL (SM)**, with cobbles, light brown	5								
		10.0	**POORLY GRADED SAND WITH SILT AND GRAVEL (SP-SM)**, brown	10								
			Test Pit Terminated at 10 Feet									



See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered

Excavator
Backhoe

Hammer Type
N/A

Operator
Lourenco

Notes

Advancement Method
Bucket

Logged by
EO

Test Pit Started
05-19-2025

Abandonment Method
Test pit backfilled with soil spoils upon completion.

Test Pit Completed
05-19-2025

Laboratory Test Results

Contents:

Laboratory Testing Procedures
Grain Size Distribution
Atterberg Limits
Swell/Collapse Potential
Moisture Density Relationship
CBR Test Results
Corrosion Test Results
Thermal Resistivity Test Results

Laboratory Testing Procedures

The project engineer reviewed the field data and assigned laboratory tests. The laboratory testing program included the following types of tests:

- Moisture Content
- Grain Size Analysis
- Atterberg Limits
- Swell/Collapse Potential
- California Bearing Ratio
- Moisture Density Relationship
- Thermal Resistivity
- Corrosivity

Based on the results of our field and laboratory programs, we described and classified the soil samples in accordance with the Unified Soil Classification System.

Thermal Resistivity: Thermal resistivity tests were analyzed by Geotherm USA. Terracon collected bulk samples of subsurface materials obtained within the substation/BESS areas. Each bulk sample had a Modified Proctor test performed, and each bulk sample was tested for thermal resistivity tests on samples remolded to 85% and 95% in the upper 1 to 4 feet samples, and 90% in the 4 to 6 feet samples. The percent compaction is based on comparing to the material's maximum dry density as determined by test method ASTM D1557 (Modified Proctor). Tests included a minimum of 4 readings, including optimum moisture content or as-received moisture (whichever is higher), totally dry condition, and 2 intermediate moisture contents.

Corrosivity Testing: Bulk samples of near surface soils were tested in the laboratory for the following properties in general accordance with the corresponding standards:

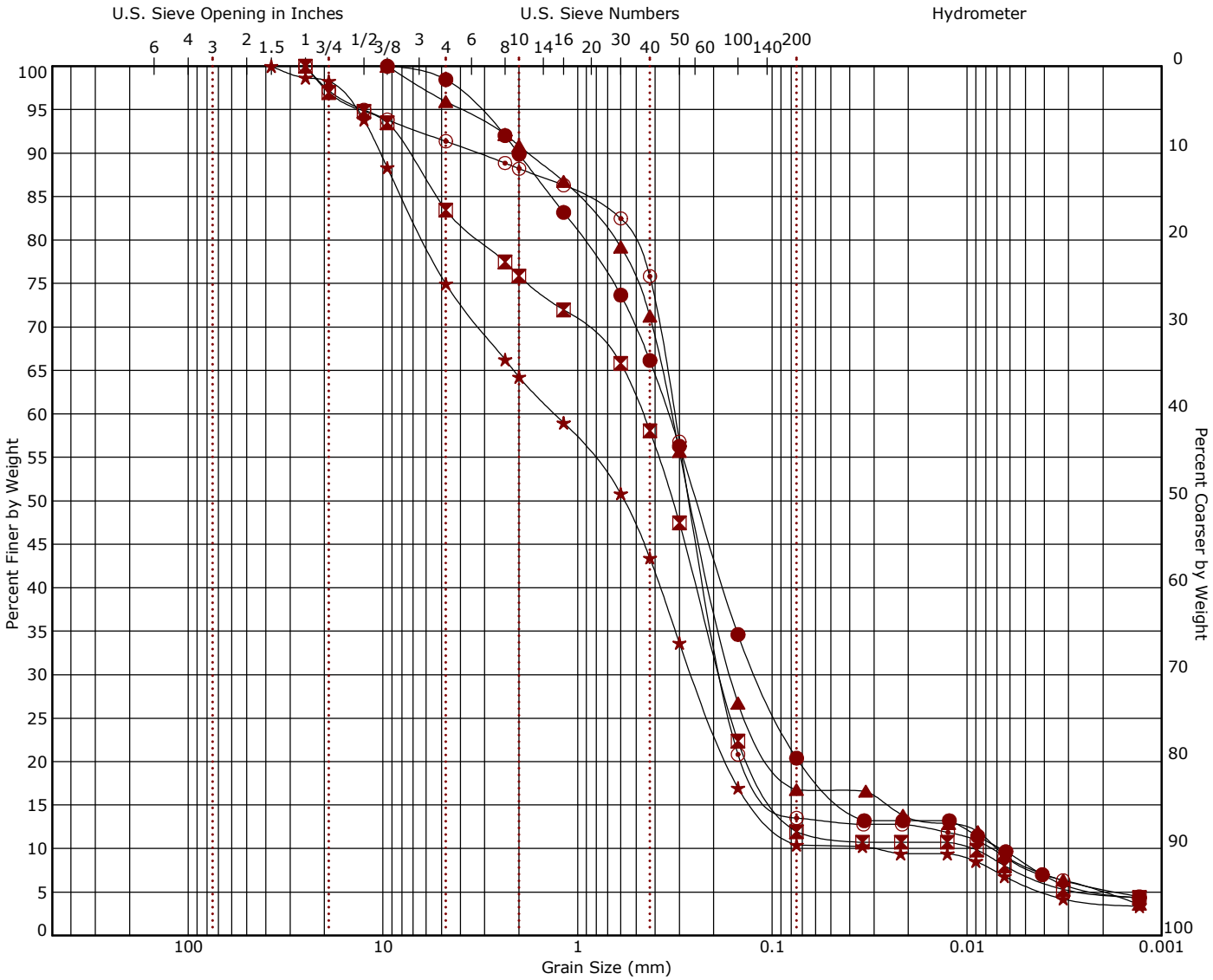
- pH Analysis (ASTM G51)
- Chloride (ASTM D512)
- Sulfate (ASTM C1580)
- Sulfide Content (AWWA 4500-S D)
- Oxidation-Reduction Potential (ASTM G200)
- Electrical Resistivity Testing (ASTM G187)
- Total Salts (AWWA 2520 B)

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Terracon
23041 Avenida De La Carlota Ste 350
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Grain Size Distribution

ASTM D422 / ASTM C136 / AASHTO T27



Cobbles	Gravel		Sand			Silt or Clay
	coarse	fine	coarse	medium	fine	

Boring ID		Depth (Ft)	Description	USCS	LL	PL	PI	Cc	Cu
●	BESS-02	1 - 4	SILTY SAND	SM	NP	NP	NP	6.23	50.81
⊠	BESS-02	4 - 6	WELL GRADED SAND with SILT AND GRAVEL	SW-SM				7.76	48.55
▲	BESS-08	1 - 4	SILTY SAND	SM	NP	NP	NP	11.31	46.92
★	BESS-08	4 - 6	POORLY GRADED SAND with SILT AND GRAVEL	SP-SM				1.69	43.31
⊙	BESS-09	1 - 4	SILTY SAND	SM	NP	NP	NP	13.39	42.26

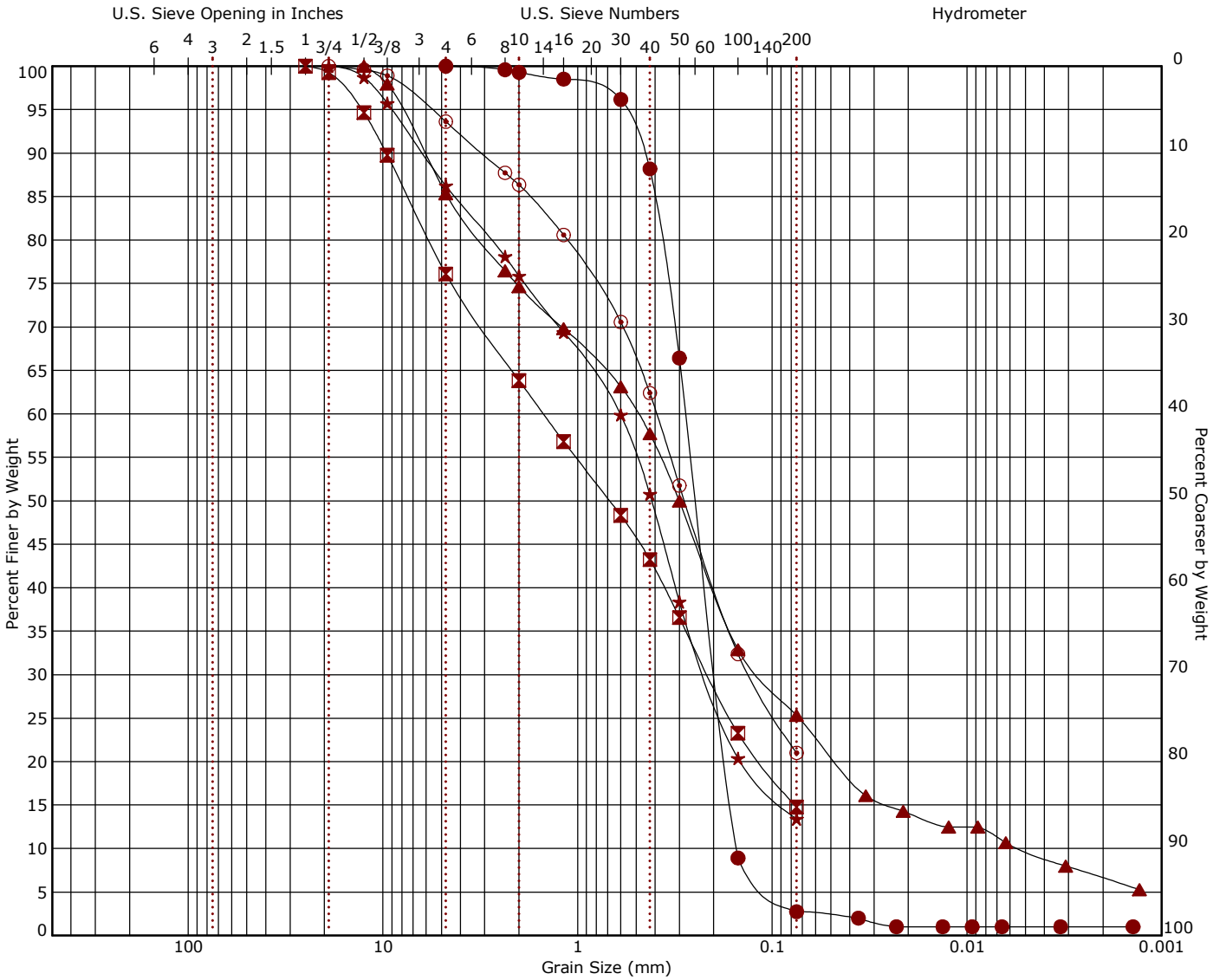
Boring ID		Depth (Ft)	D_{100}	D_{60}	D_{50}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
●	BESS-02	1 - 4	9.5	0.342	0.245	0.12	0.007	0.0	1.5	78.1		12.2	8.2
⊠	BESS-02	4 - 6	25	0.463	0.326	0.185	0.01	0.0	16.5	71.5		4.9	7.0
▲	BESS-08	1 - 4	9.5	0.33	0.262	0.162	0.007	0.0	4.1	79.2		8.6	8.2
★	BESS-08	4 - 6	37.5	1.306	0.577	0.258	0.03	0.0	25.0	64.5		4.5	5.9
⊙	BESS-09	1 - 4	25	0.318	0.263	0.179	0.008	0.0	8.6	77.9		5.4	8.1

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Terracon Project No. 60225164

Terracon
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Laguna Hills, CA

Grain Size Distribution

ASTM D422 / ASTM C136 / AASHTO T27



Cobbles	Gravel		Sand			Silt or Clay
	coarse	fine	coarse	medium	fine	

Boring ID	Depth (Ft)	Description	USCS	LL	PL	PI	Cc	Cu
● BESS-09	4 - 6	POORLY GRADED SAND	SP				0.89	1.83
⊠ BESS-12	1 - 4	SILTY, CLAYEY SAND with GRAVEL	SC-SM	21	15	6		
▲ BESS-15	1 - 4	SILTY SAND	SM				5.12	93.11
★ BESS-15	4 - 6	SILTY SAND	SM					
⊙ SUB-7	1 - 4	SILTY, CLAYEY SAND	SC-SM	19	13	6		

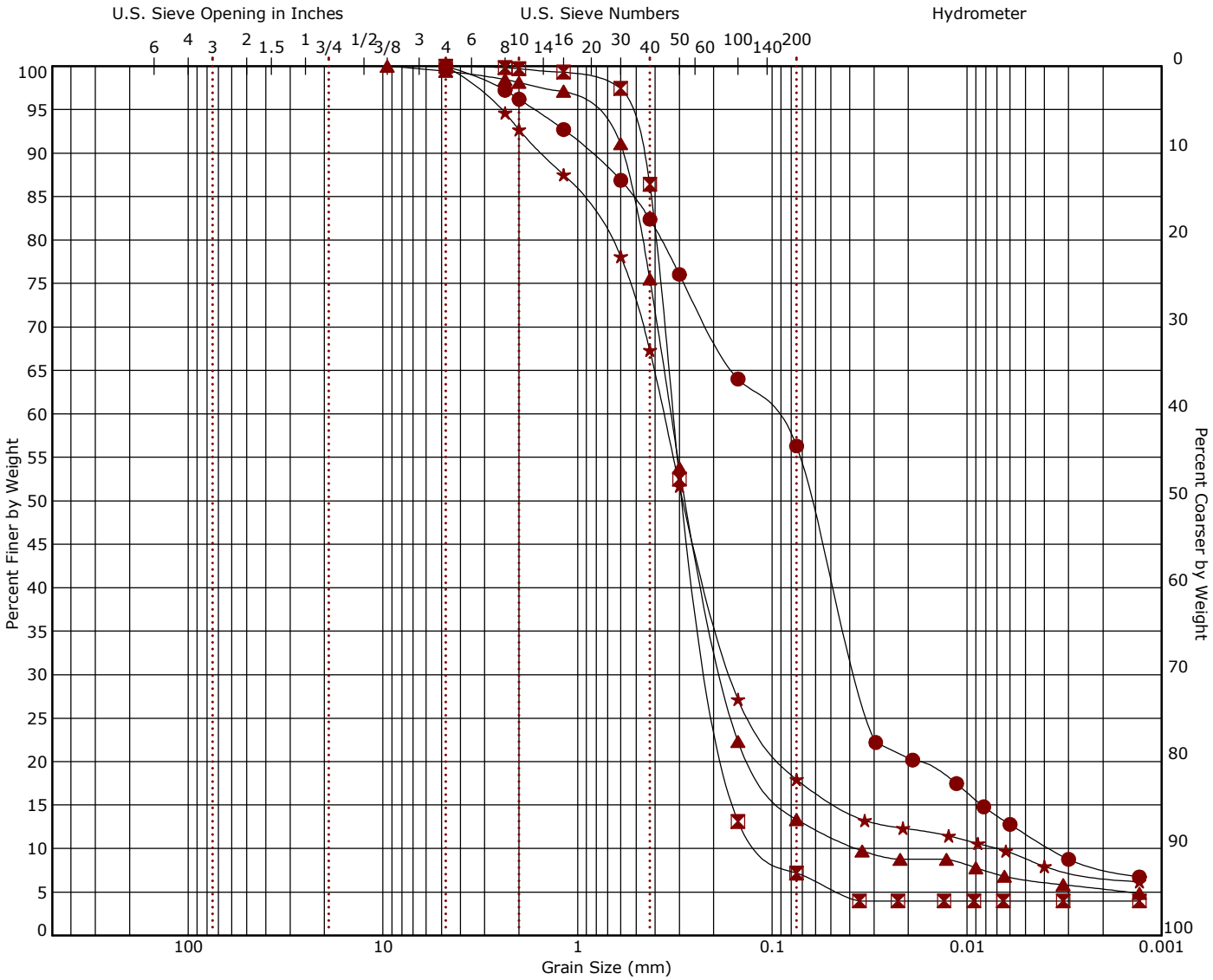
Boring ID	Depth (Ft)	D_{100}	D_{60}	D_{50}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
● BESS-09	4 - 6	4.75	0.278	0.246	0.193	0.152	0.0	0.0	97.3		1.7	1.0
⊠ BESS-12	1 - 4	25	1.498	0.686	0.213		0.0	23.9	61.3	14.8		
▲ BESS-15	1 - 4	12.5	0.491	0.301	0.115	0.005	0.0	14.7	60.0		15.5	9.8
★ BESS-15	4 - 6	19	0.605	0.416	0.217		0.0	13.8	72.8	13.4		
⊙ SUB-7	1 - 4	19	0.393	0.282	0.13		0.0	6.4	72.6	21.0		

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Terracon
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Laguna Hills, CA

Grain Size Distribution

ASTM D422 / ASTM C136 / AASHTO T27



Cobbles	Gravel		Sand			Silt or Clay
	coarse	fine	coarse	medium	fine	

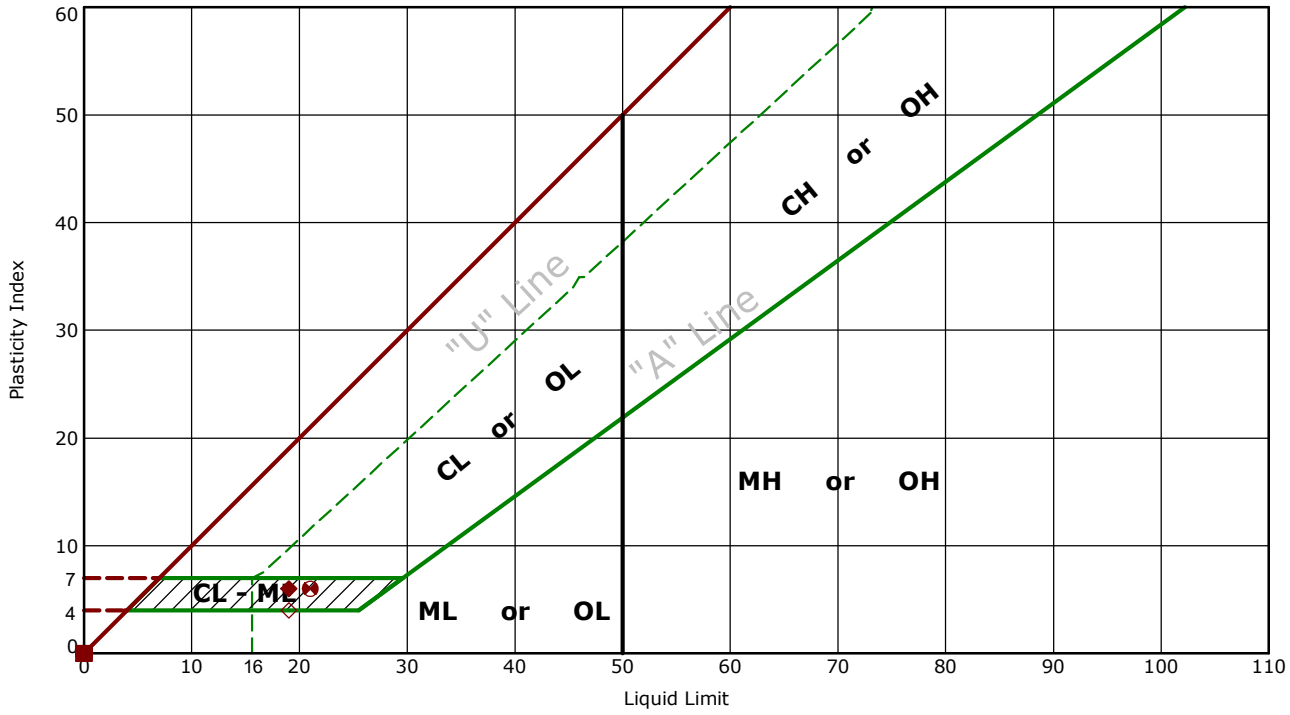
Boring ID	Depth (Ft)	Description	USCS	LL	PL	PI	Cc	Cu
● STP-2	1 - 4	SANDY SILT	ML				3.41	28.05
⊠ STP-2	4 - 6	POORLY GRADED SAND with SILT	SP-SM				1.21	3.11
▲ STP-3	1 - 4	SILTY SAND	SM				2.60	9.05
★ STP-3	4 - 6	SILTY SAND	SM				10.37	51.13

Boring ID	Depth (Ft)	D_{100}	D_{60}	D_{50}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
● STP-2	1 - 4	4.75	0.104	0.063	0.036	0.004	0.0	0.0	43.7		44.6	11.7
⊠ STP-2	4 - 6	4.75	0.324	0.287	0.202	0.104	0.0	0.0	92.8		3.2	4.0
▲ STP-3	1 - 4	9.5	0.331	0.276	0.178	0.037	0.0	0.6	86.1		6.9	6.5
★ STP-3	4 - 6	4.75	0.361	0.286	0.163	0.007	0.0	0.0	82.1		9.1	8.8

Terracon
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Laguna Hills, CA

Atterberg Limit Results

ASTM D4318



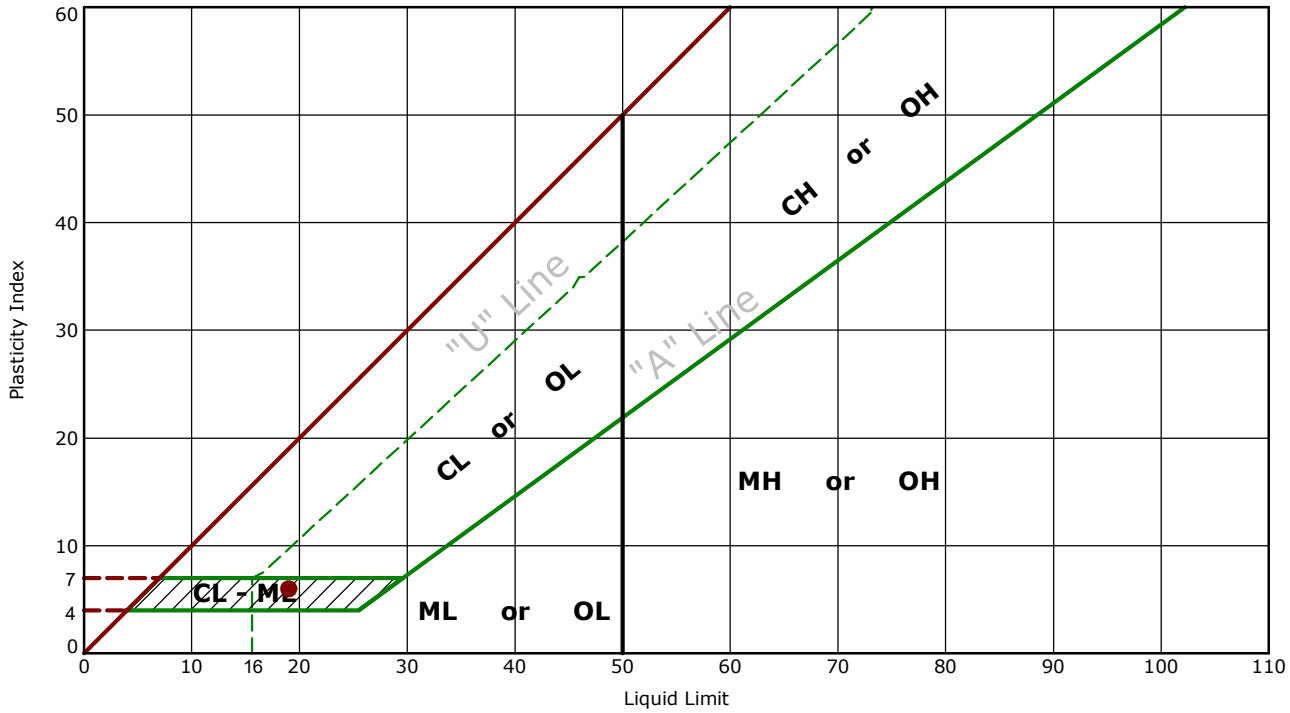
	Boring ID	Depth (Ft)	LL	PL	PI	Fines	USCS	Description
●	BESS-01	1 - 5	NP	NP	NP	19.7	SM	SILTY SAND
⊠	BESS-02	1 - 4	NP	NP	NP	20.4	SM	SILTY SAND
▲	BESS-03	1 - 4	NP	NP	NP	21.7	SM	SILTY SAND
★	BESS-04	1 - 4	NP	NP	NP	26.9	SM	SILTY SAND
⊙	BESS-05	1 - 4	NP	NP	NP	11.3	SP-SM	POORLY GRADED SAND with SILT
✿	BESS-06	1 - 4	NP	NP	NP		SP	POORLY GRADED SAND with GRAVEL
○	BESS-07	1 - 4	NP	NP	NP	18.6	SM	SILTY SAND
△	BESS-08	1 - 4	NP	NP	NP	16.8	SM	SILTY SAND
⊗	BESS-09	1 - 4	NP	NP	NP	13.5	SM	SILTY SAND
⊕	BESS-10	1 - 4	NP	NP	NP	8.6	SP-SM	POORLY GRADED SAND with SILT
□	BESS-11	1 - 4	NP	NP	NP	9.7	SP-SM	POORLY GRADED SAND with SILT AND GRAVEL
⊛	BESS-12	1 - 4	21	15	6	14.8	SC-SM	SILTY, CLAYEY SAND with GRAVEL
◍	BESS-13	1 - 4	NP	NP	NP	11.1	SP-SM	POORLY GRADED SAND with SILT
☆	BESS-14	1 - 4	NP	NP	NP	16.1	SM	SILTY SAND
⊠	SUB-1	1 - 4	NP	NP	NP	19.1	SM	SILTY SAND
■	SUB-2	1 - 4	NP	NP	NP	18.3	SM	SILTY SAND with GRAVEL
◆	SUB-3	1 - 4	19	13	6	24.6	SC-SM	SILTY, CLAYEY SAND
◇	SUB-4	1 - 4	19	15	4	20.7	SC-SM	SILTY, CLAYEY SAND
✕	SUB-5	1 - 4	NP	NP	NP	19.4	SM	SILTY SAND with GRAVEL
✖	SUB-6	1 - 4	NP	NP	NP	16.6	SM	SILTY SAND with GRAVEL

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Atterberg Limit Results

ASTM D4318

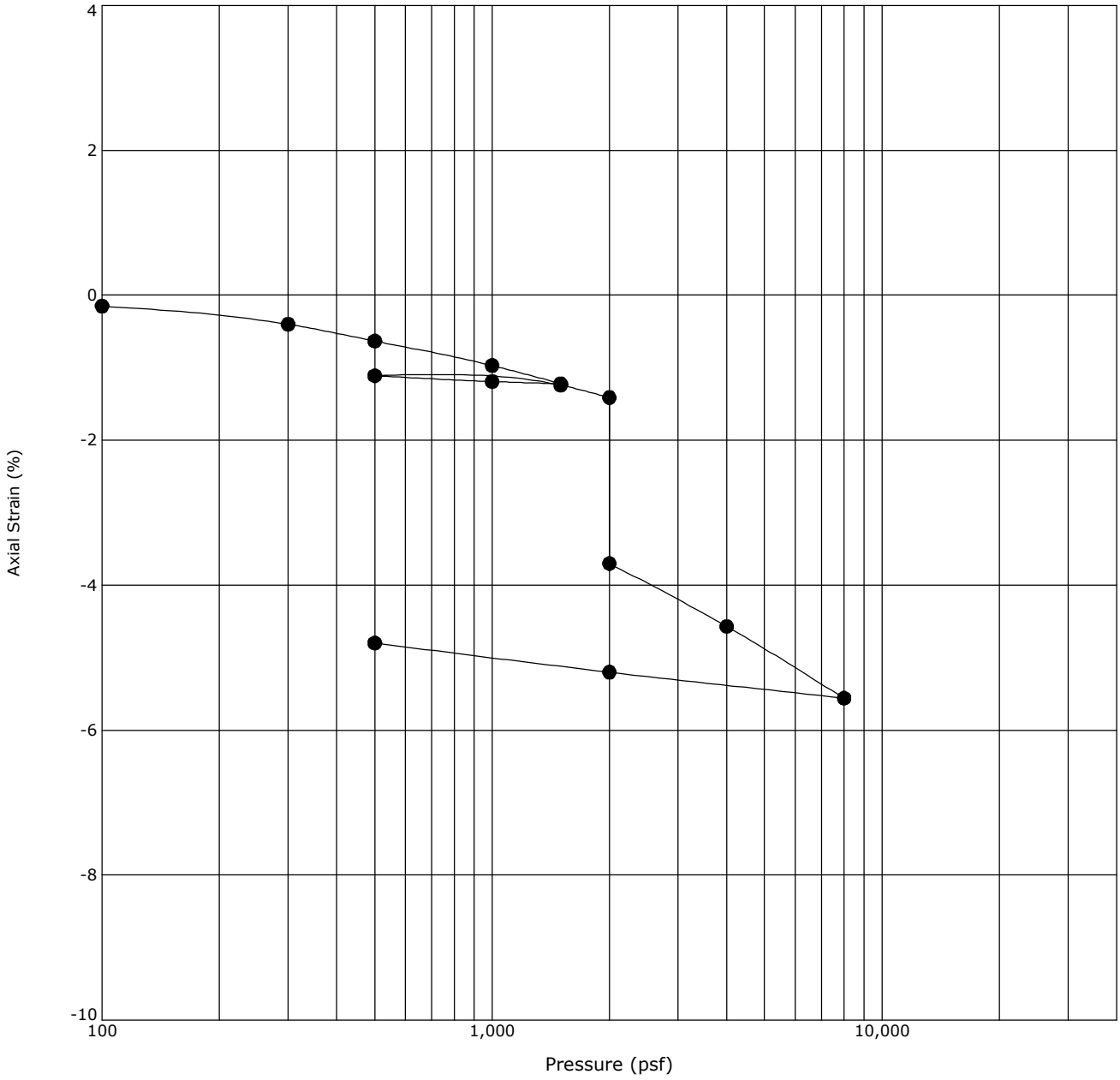


	Boring ID	Depth (Ft)	LL	PL	PI	Fines	USCS	Description
●	SUB-7	1 - 4	19	13	6	21.0	SC-SM	SILTY, CLAYEY SAND

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

One-Dimensional Swell or Collapse

ASTM D2435



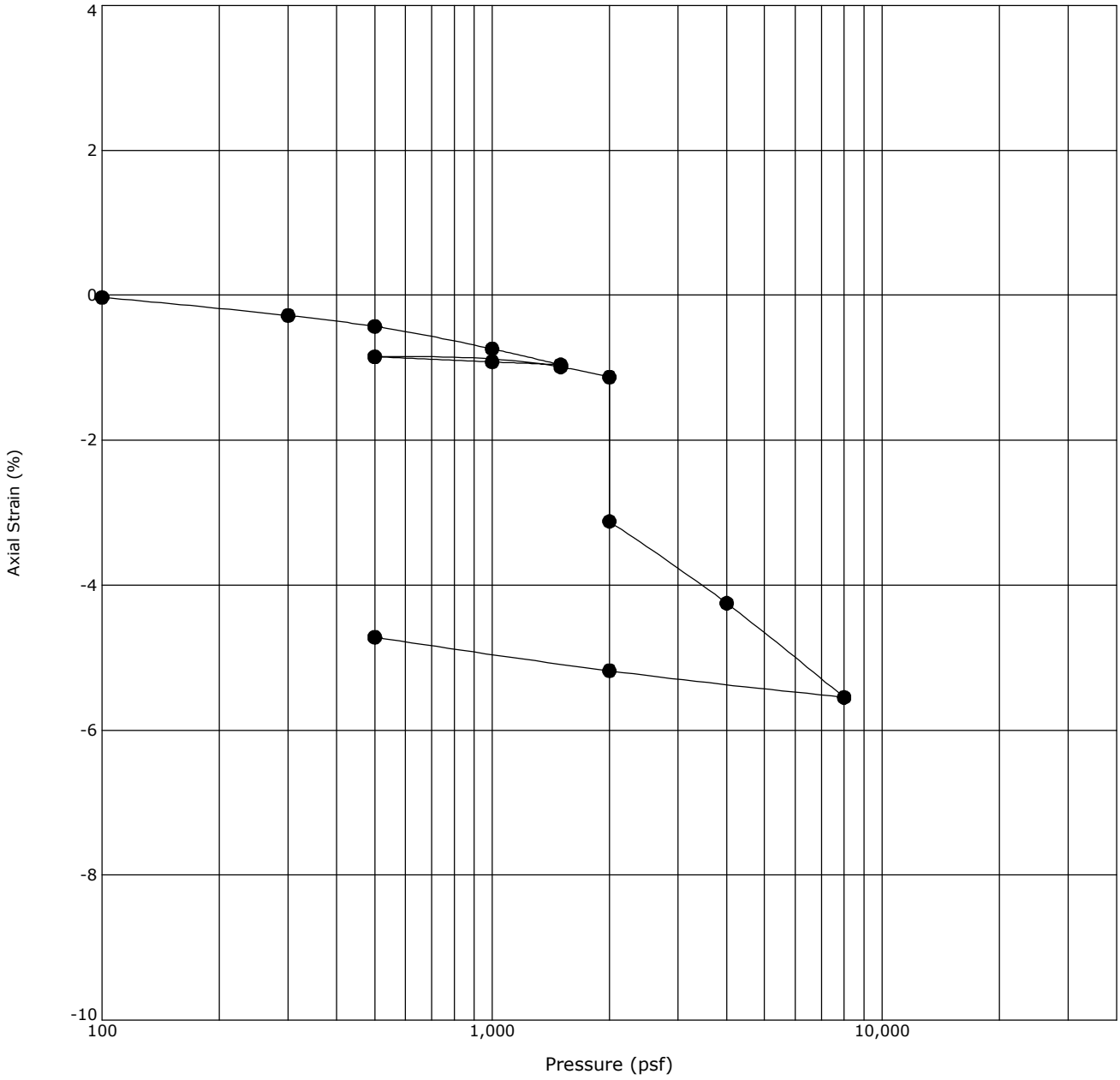
Boring ID		Depth (Ft)	Description	USCS	γ_d(pcf)	WC (%)
●	BESS-05	5 - 6.5	POORLY GRADED SAND with SILT	SP-SM	109	1.0

Notes: Water added at 2,000 psf

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

One-Dimensional Swell or Collapse

ASTM D2435



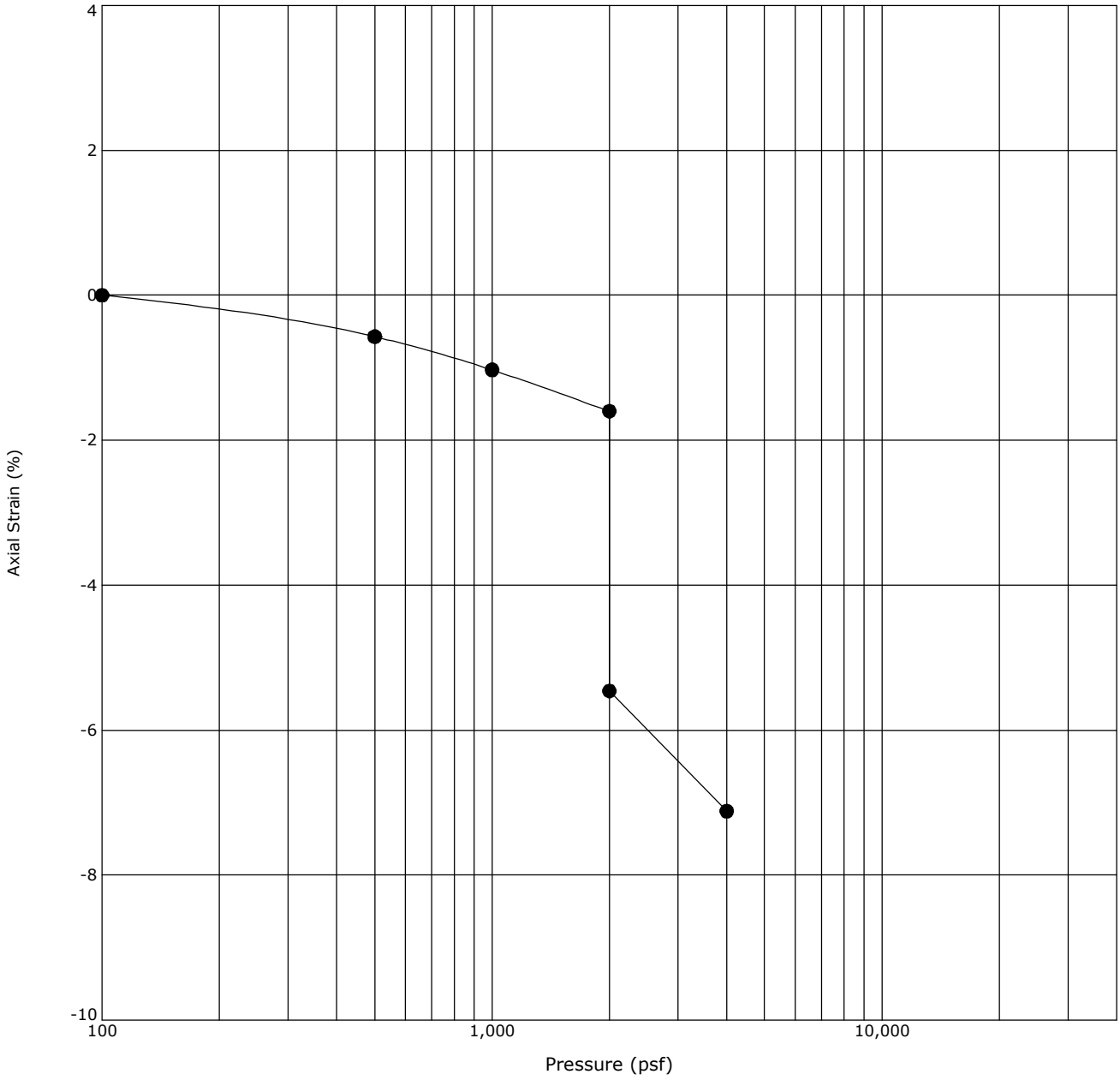
Boring ID		Depth (Ft)	Description	USCS	γ_d(pcf)	WC (%)
●	BESS-07	2.5 - 4	SILTY SAND	SM	105	1.2

Notes: Water added at 2,000 psf

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

One-Dimensional Swell or Collapse

ASTM D2435



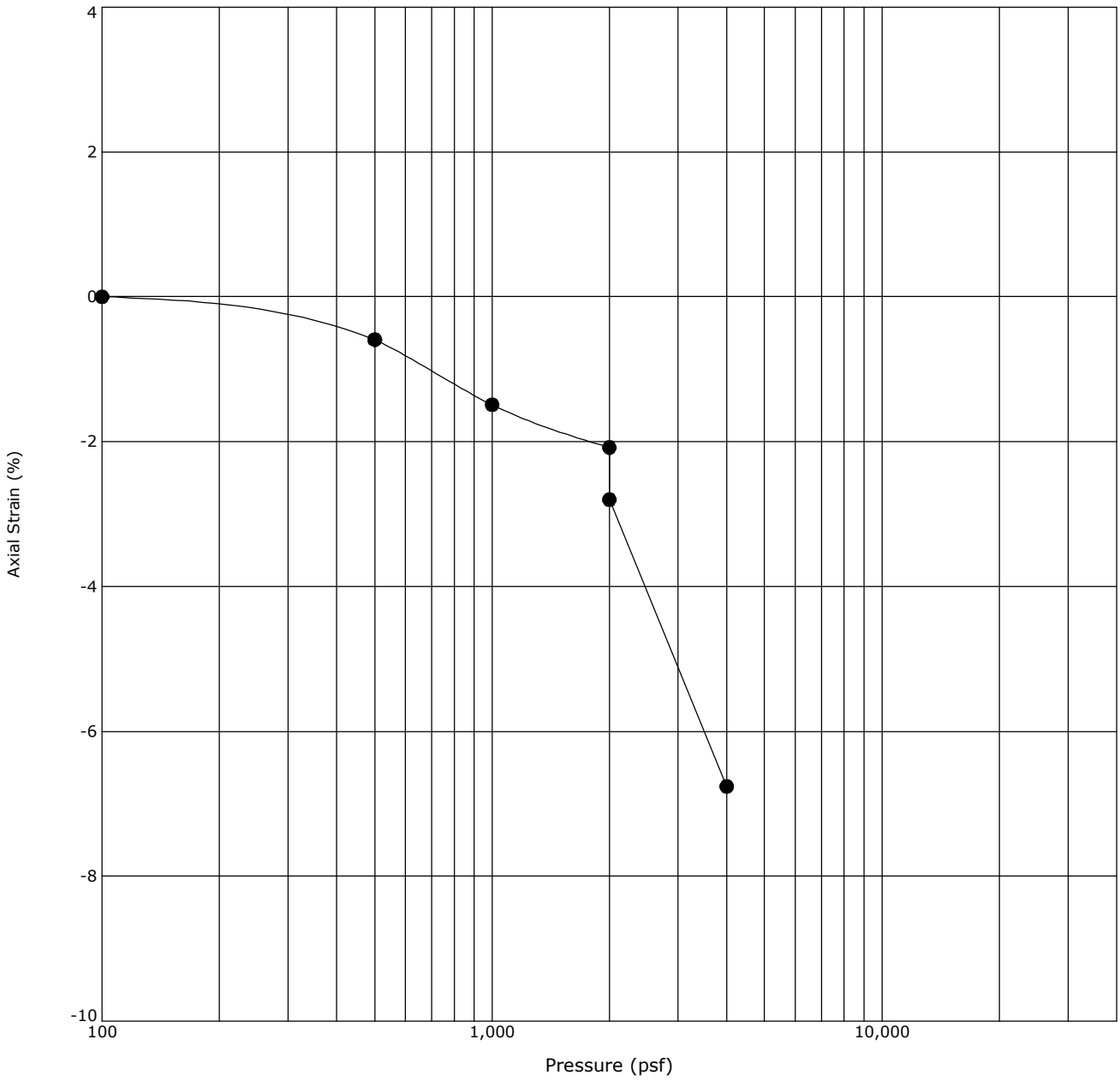
Boring ID		Depth (Ft)	Description	USCS	γ_d(pcf)	WC (%)
●	SUB-3	2.5 - 4	SILTY CLAYEY SAND	SC-SM	106	3.4

Notes: Water added at 2,000 psf

Terracon
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Laguna Hills, CA

One-Dimensional Swell or Collapse

ASTM D2435



Boring ID		Depth (Ft)	Description	USCS	γ_d(pcf)	WC (%)
●	SUB-4	5 - 6.5	POORLY GRADED SAND	SP	101	3.5

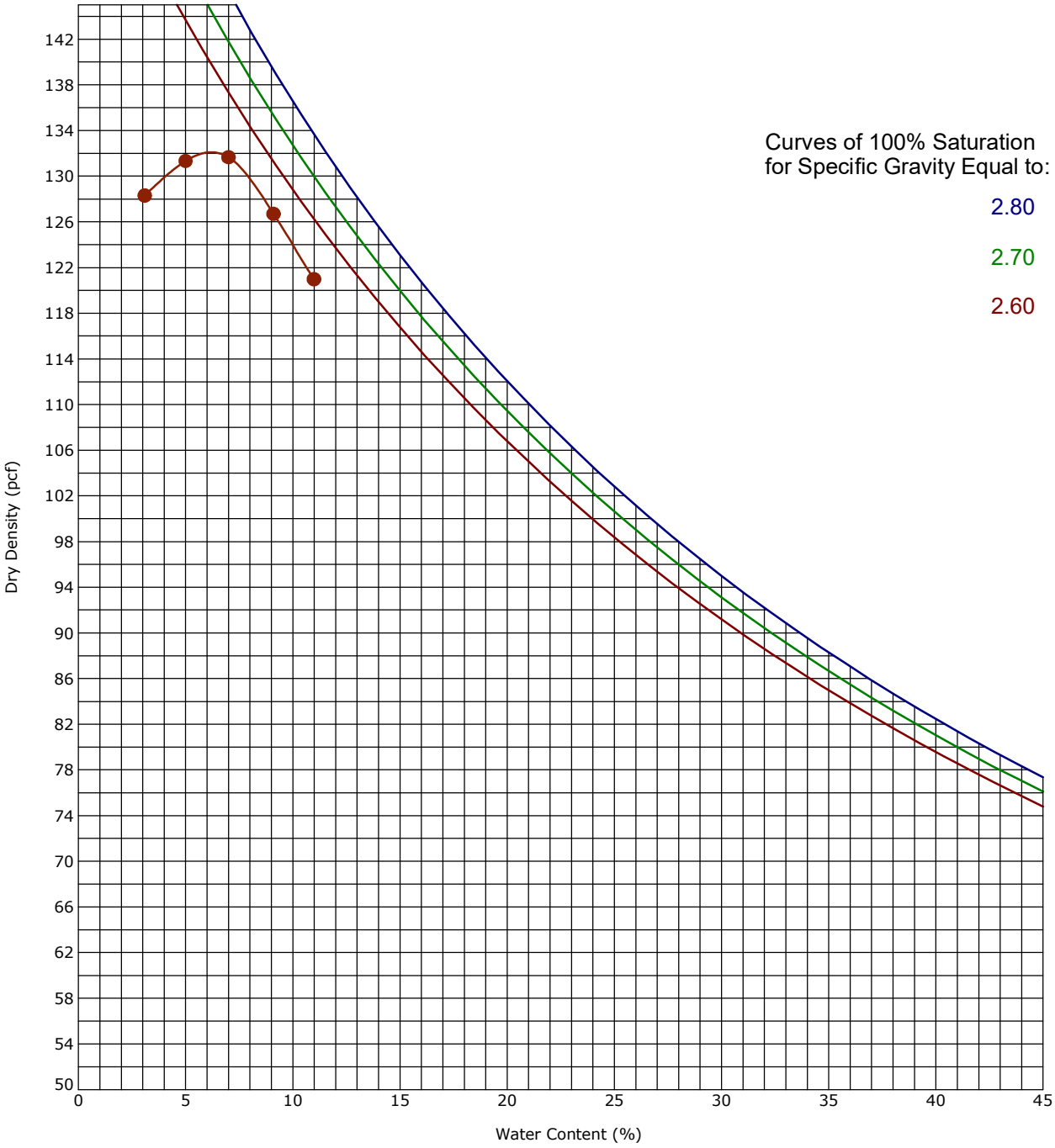
Notes: Water added at 2,000 psf

Grace Solar
Blythe | Riverside County, CA
Terracon Project No. 60225164

Terracon
23041 Avenida De La Carlota Ste 350
Laguna Hills, CA

Moisture-Density Relationship

ASTM D1557-Method B



Boring ID	Depth (Ft)	Description of Materials
BESS-02	1 - 4	SILTY SAND

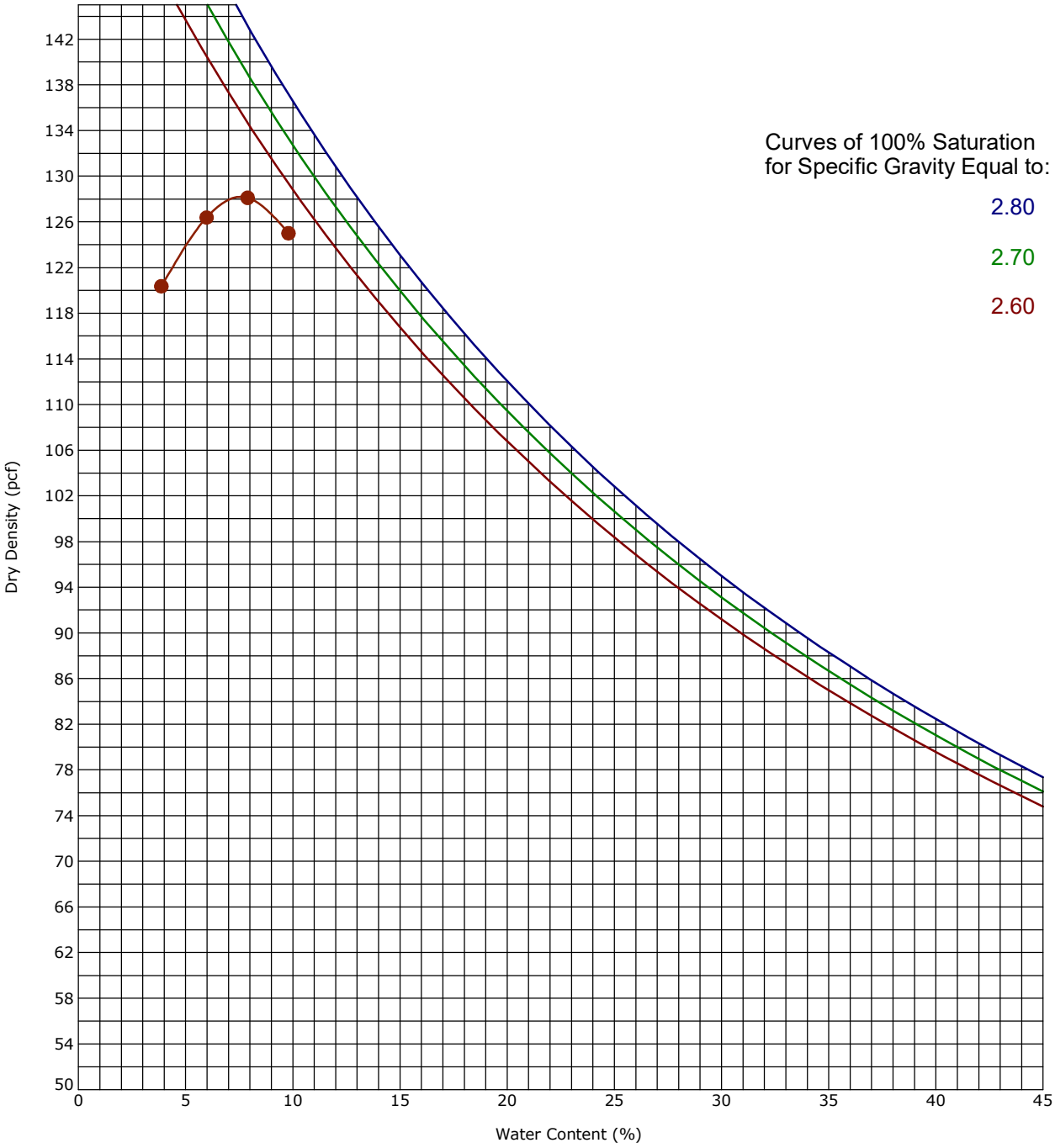
Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
20	0.0	NP	NP	NP	ASTM D1557-Method B	132.1	6.2

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Moisture-Density Relationship

ASTM D1557-Method B



Boring ID	Depth (Ft)	Description of Materials
BESS-02	4 - 6	WELL GRADED SAND with SILT AND GRAVEL

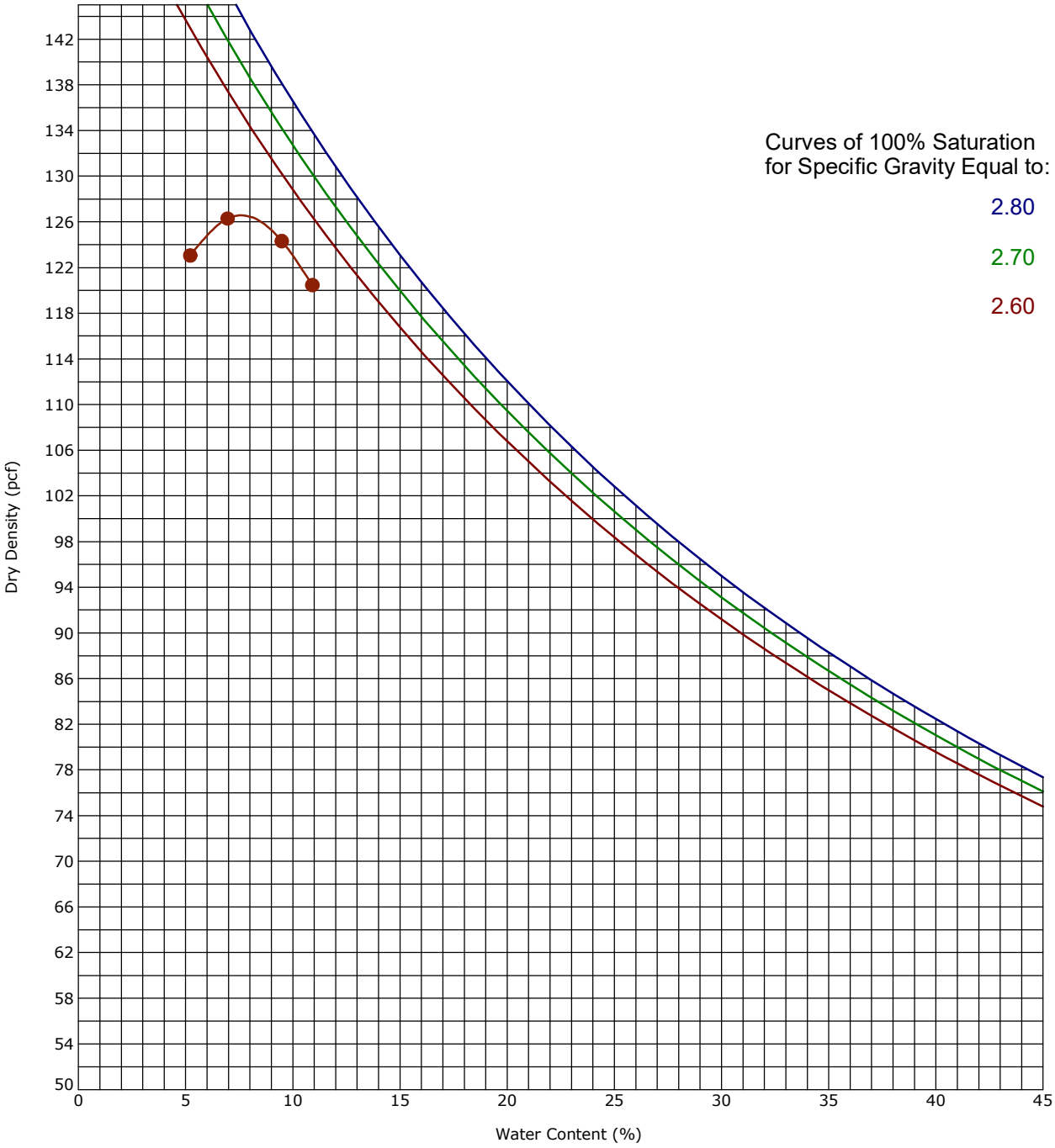
Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
12	0.0				ASTM D1557-Method B	128.2	7.6

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Terracon Project No. 60225164

Terracon
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Moisture-Density Relationship

ASTM D1557-Method C



Boring ID	Depth (Ft)	Description of Materials
BESS-07	1 - 4	SILTY SAND

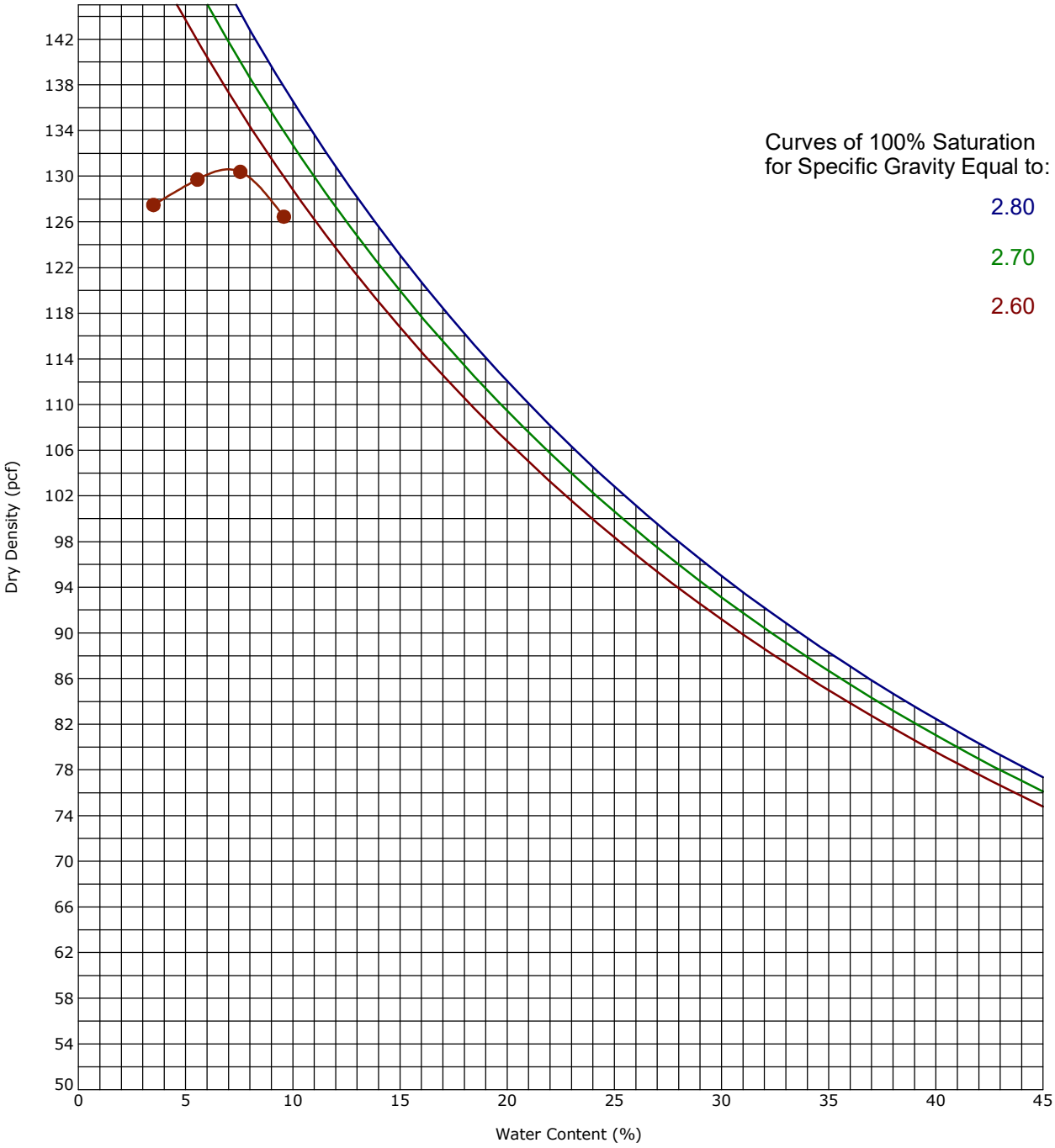
Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
19	0.0	NP	NP	NP	ASTM D1557-Method C	126.6	7.6

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Terracon Project No. 60225164

Terracon
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Laguna Hills, CA

Moisture-Density Relationship

ASTM D1557-Method B



Boring ID	Depth (Ft)	Description of Materials
BESS-08	1 - 4	SILTY SAND

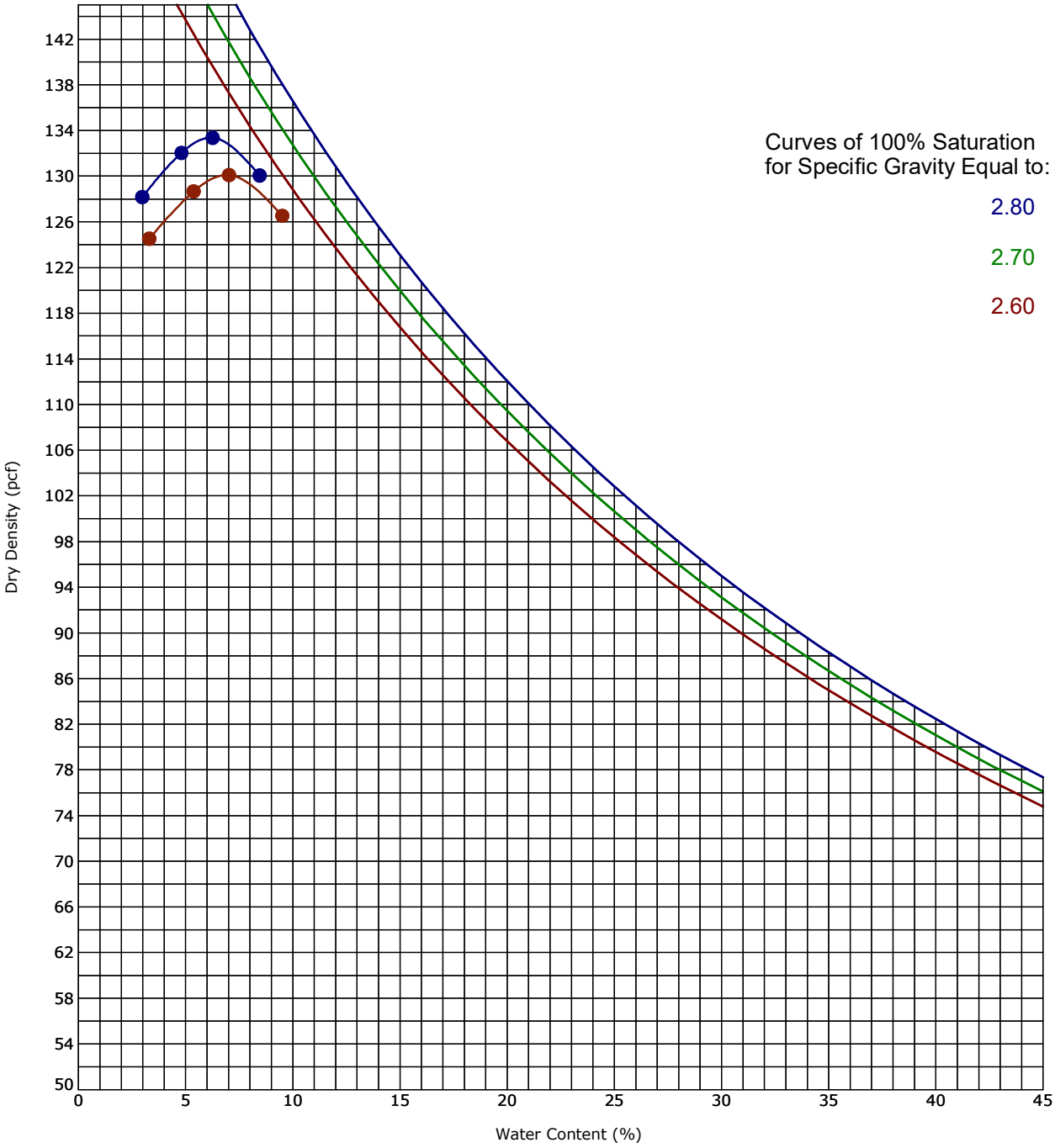
Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
17	0.0	NP	NP	NP	ASTM D1557-Method B	130.6	7.0

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Terracon
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Moisture-Density Relationship

ASTM D1557-Method B



Boring ID	Depth (Ft)	Description of Materials
BESS-08	4 - 6	POORLY GRADED SAND with SILT AND GRAVEL

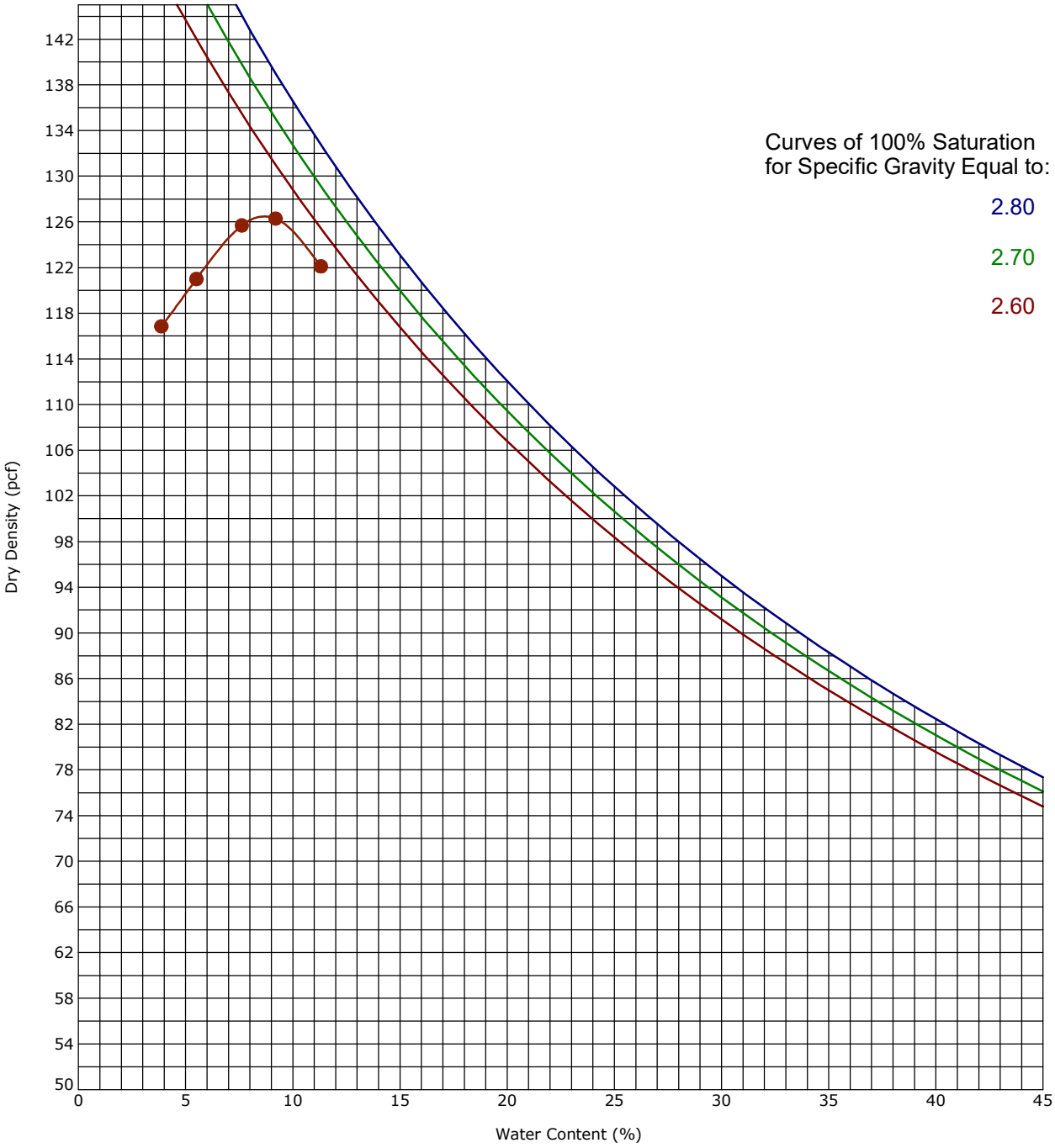
						Uncorrected		Corrected	
Fines (%)	Fraction >9.5 mm size	LL	PL	PI	Test Method	Max DD (pcf)	Optimum WC (%)	Max DD (pcf)	Optimum WC (%)
10	11.7				ASTM D1557-Method B	130.1	6.9	133.4	6.2

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Laguna Hills, CA

Moisture-Density Relationship

ASTM D1557-Method A



Boring ID	Depth (Ft)	Description of Materials
BESS-09	1 - 4	SILTY SAND

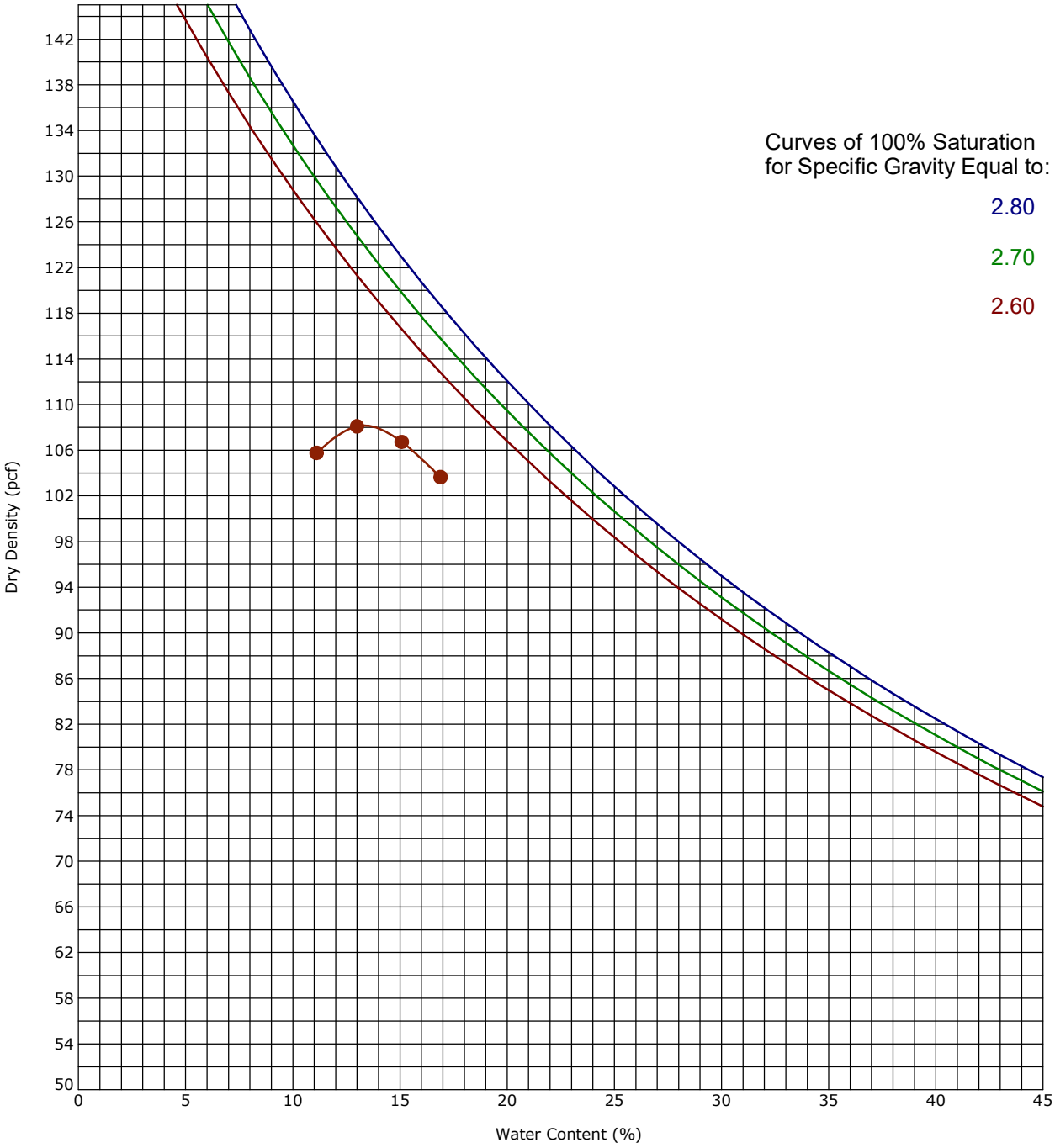
Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
14	0.0	NP	NP	NP	ASTM D1557-Method A	126.5	8.7

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Moisture-Density Relationship

ASTM D1557-Method A



Boring ID	Depth (Ft)	Description of Materials
BESS-09	4 - 6	POORLY GRADED SAND

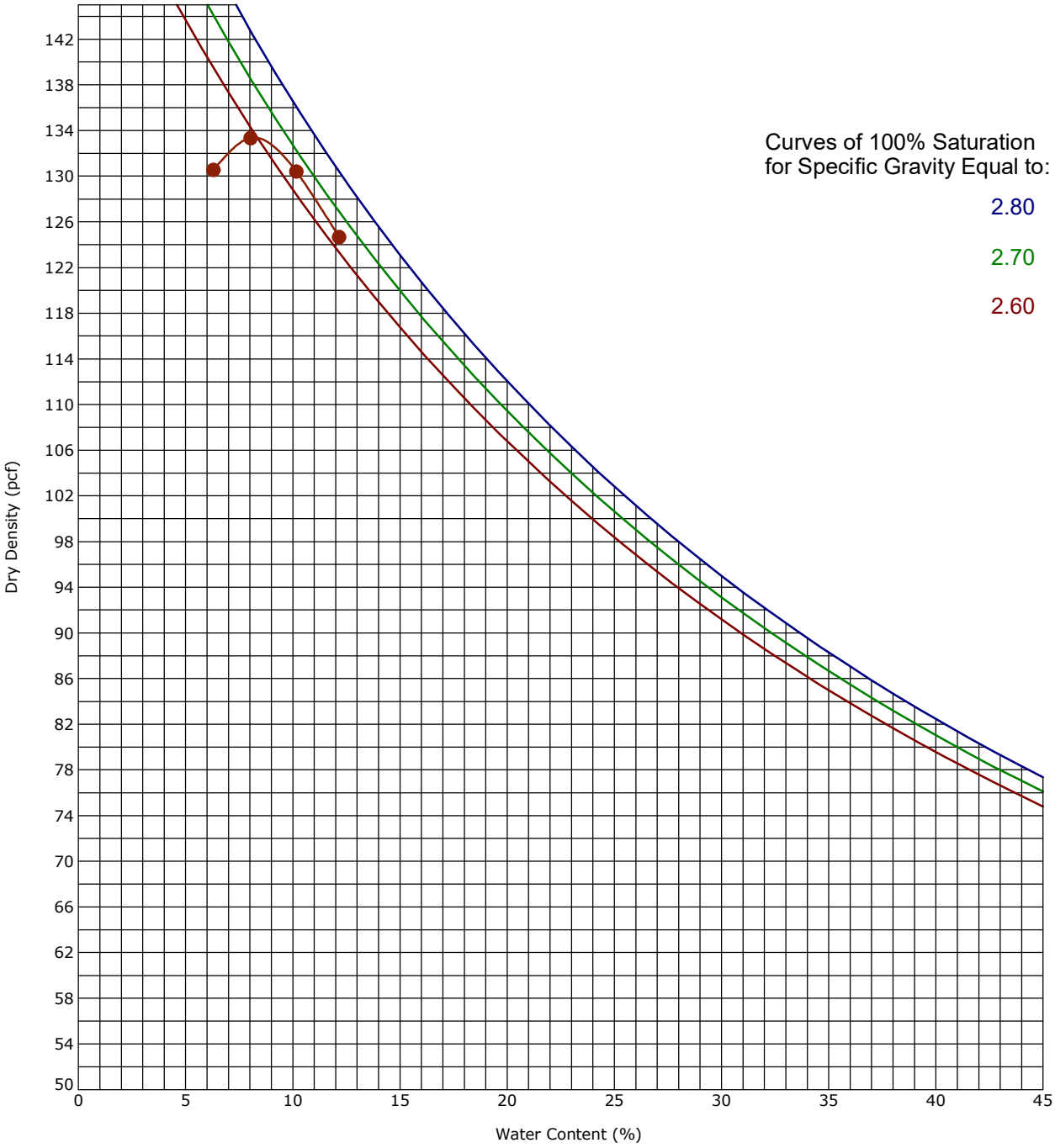
Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
3	0.0				ASTM D1557-Method A	108.2	13.3

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Moisture-Density Relationship

ASTM D1557-Method B



Boring ID	Depth (Ft)	Description of Materials
BESS-15	1 - 4	SILTY SAND

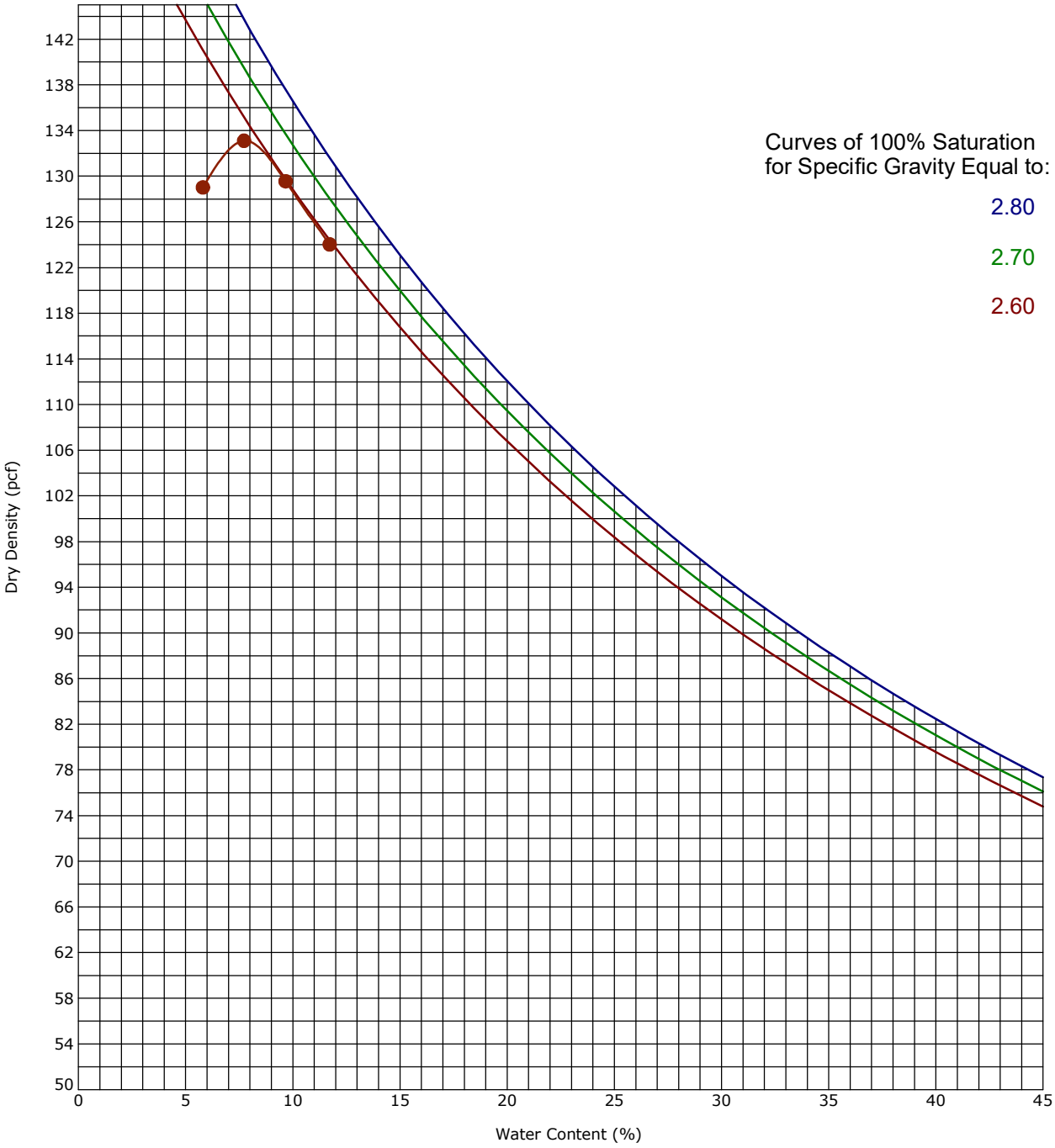
Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
25	0.0				ASTM D1557-Method B	133.4	8.1

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Moisture-Density Relationship

ASTM D1557-Method B



Boring ID	Depth (Ft)	Description of Materials
BESS-15	4 - 6	SILTY SAND

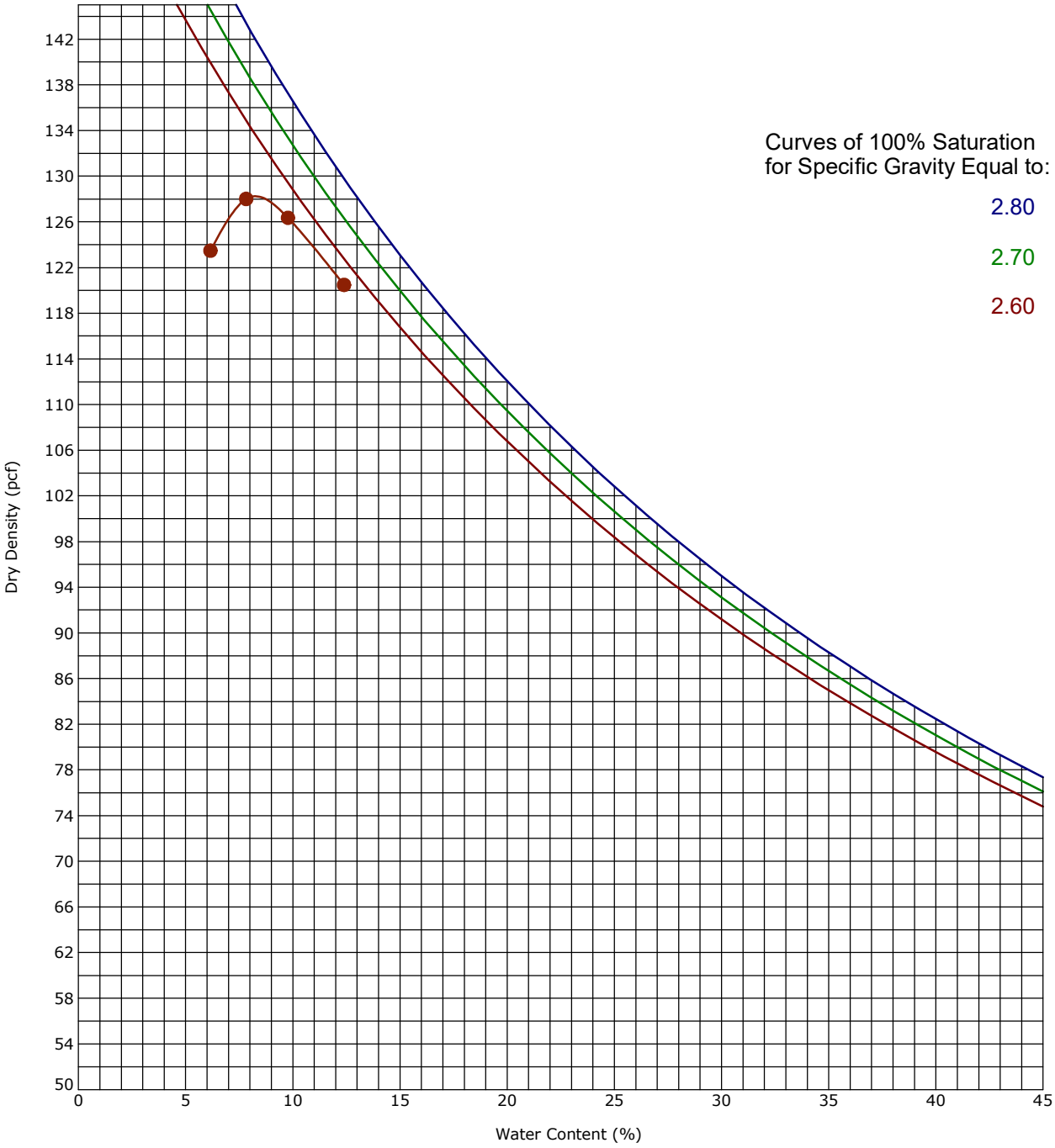
Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
13	0.0				ASTM D1557-Method B	133.1	7.7

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Moisture-Density Relationship

ASTM D1557-Method B



Boring ID	Depth (Ft)	Description of Materials
STP-2	1 - 4	SANDY SILT

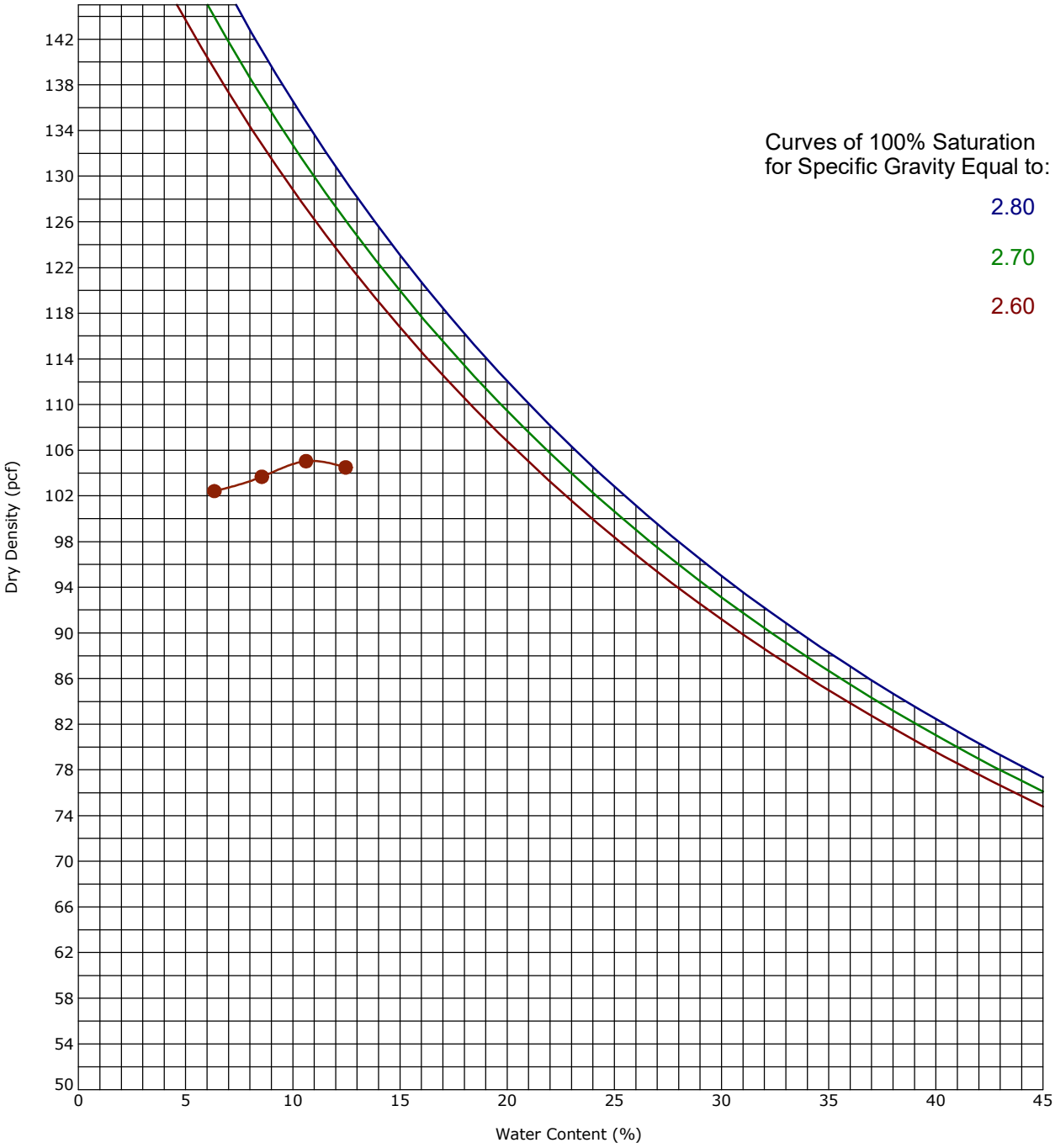
Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
56	0.0				ASTM D1557-Method B	128.2	8.2

Grace Solar
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Terracon Project No. 60225164

Terracon
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Moisture-Density Relationship

ASTM D1557-Method B



Boring ID	Depth (Ft)	Description of Materials
STP-2	4 - 6	POORLY GRADED SAND with SILT

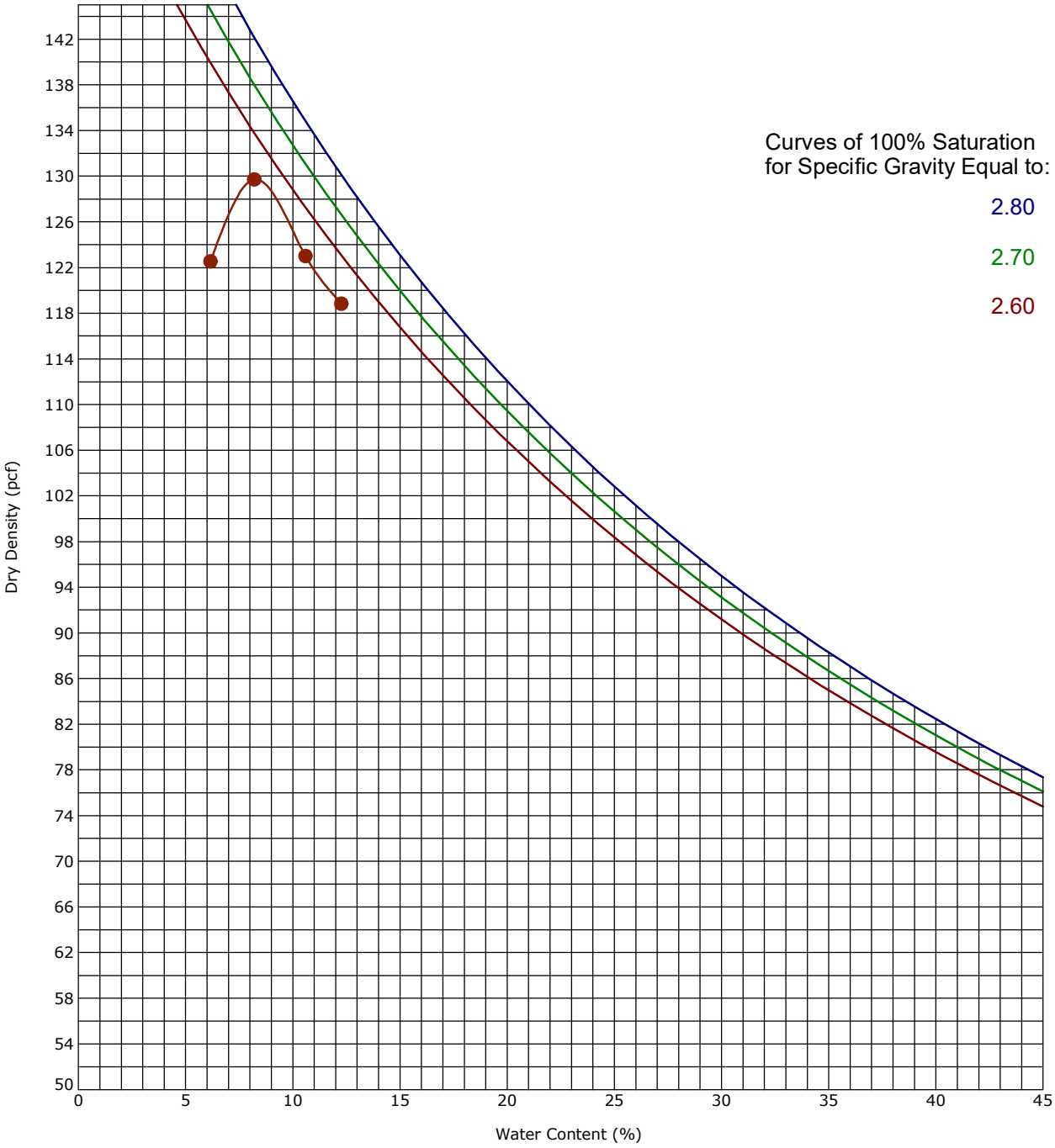
Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
7	0.0				ASTM D1557-Method B	105.1	10.9

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Moisture-Density Relationship

ASTM D1557-Method B



Boring ID	Depth (Ft)	Description of Materials
STP-3	1 - 4	SILTY SAND

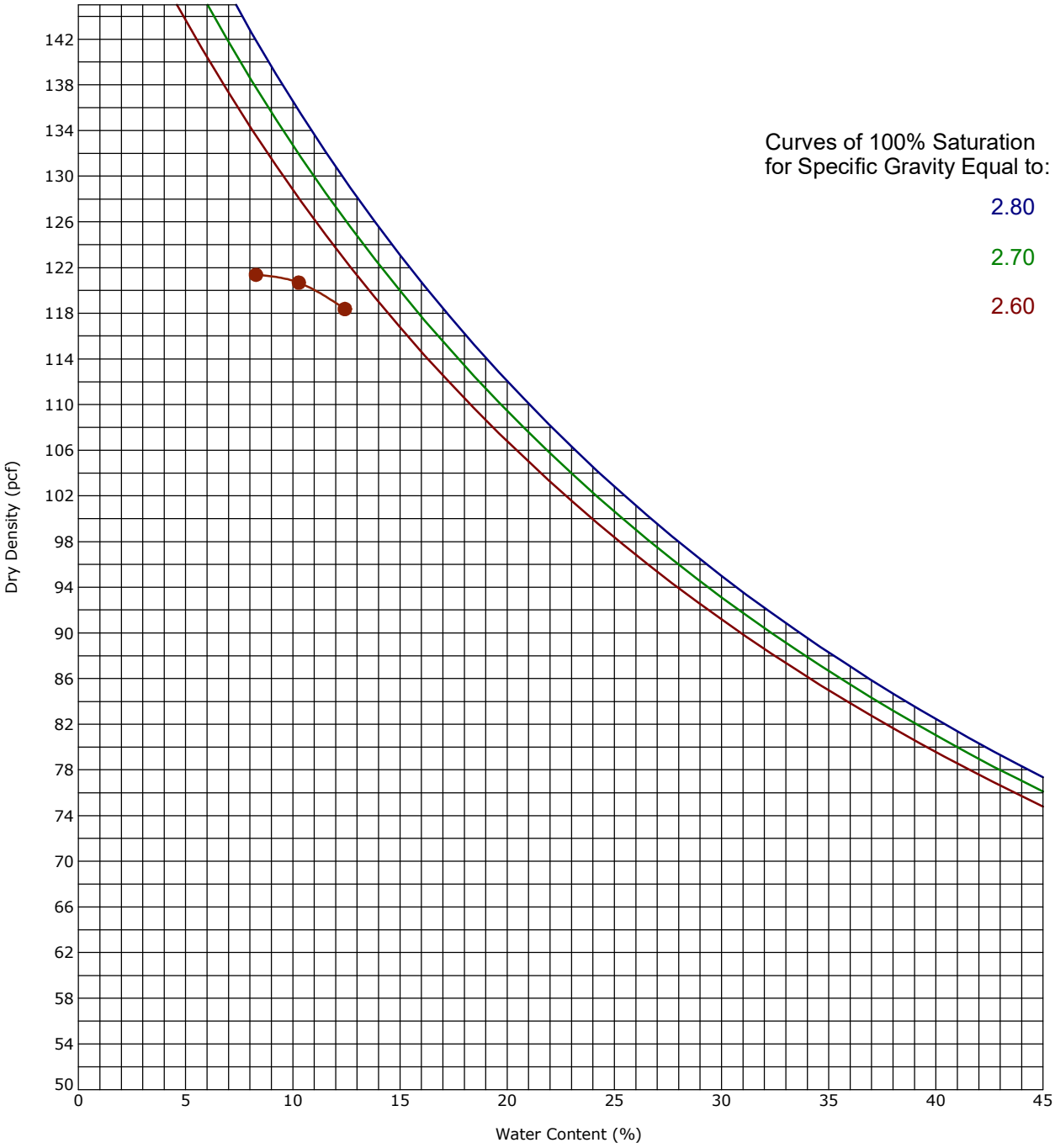
Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
13	0.0				ASTM D1557-Method B	129.7	8.3

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Terracon Project No. 60225164

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Laguna Hills, CA

Moisture-Density Relationship

ASTM D1557-Method B



Boring ID	Depth (Ft)	Description of Materials
STP-3	4 - 6	SILTY SAND

Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
18	0.0				ASTM D1557-Method B	121.4	8.3

California Bearing Ratio

PROJECT:	Grace Solar Basis of Bid	**JOB NO:**	60225164
LOCATION:	Blythe, CA	**WORK ORDER NO:**	60225164.0004
MATERIAL:	Auger	**LAB NO:**	BESS-7 S1 @1-4
SAMPLE SOURCE:	BESS-7 S1 @1-4	**DATE SAMPLED:**	4/22/2025
REVISED (Y/N)?	N	**# OF REVISIONS**	1
Hammer Weight Lb.		**Compaction Method**	ASTM D1557
Sample Condition	Soak		

			POINT 1				POINT 2				POINT 3			
	PENETRATION	PROVING RING READING	TOTAL LOAD	UNIT LOAD	CORRECTED LOAD	Corrected CBR	TOTAL LOAD	UNIT LOAD	CORRECTED LOAD	Corrected CBR	TOTAL LOAD	UNIT LOAD	CORRECTED LOAD	Corrected CBR
1	0.025	0.0010	9	9	9		12	12	12		4	4	14	
2	0.050	0.0023	22	22	22		33	33	33		9	9	32	
3	0.075	0.0034	32	32	32		45	45	45		20	20	55	
4	0.100	0.0042	39	39	39	3.9	61	61	61	6.1	37	37	95	9.5
5	0.125	0.0050	42	42	42		75	75	75		62	62	136	
6	0.150	0.0058	42	42	42		85	85	85		93	93	171	
7	0.175	0.0066	39	39	39		100	100	100		129	129	184	
8	0.200	0.0070	31	31	31	2.1	124	124	124	8.3	168	168	211	14.0
9	0.250	0.0084	26	26	26		145	145	145		209	209	361	
10	0.300	0.0098	29	29	29		195	195	195		355	355	421	
11	0.400	0.0119	43	43	43		228	228	228		481	481	548	
12	0.500	0.0140	43	43	43		311	311	311		610	610	293	
	0.600													

	POINT 1	POINT 2	POINT 3
CORRECTION	0.0000	0.0000	0.0520
# OF LAYERS	3	3	3
# OF BLOWS/LAYER	10	25	65
Surcharge Wgt.(lbs.)	10	10	10

COVER PAGE INPUT INFORMATION		
Compaction(%)		**Corrected CBR**
Percent Swell		
Surcharge Wgt.	10	
CBR		

	10 Blows		25 Blows		65 Blows	
Point to use on Correction Line 13-24	Min	Max	Min	Max	Min	Max
	2	6	2	7	4	9

	POINT 1		POINT 2		POINT 3	
	MOIST BEFORE	MOIST AFTER	MOIST BEFORE	MOIST AFTER	MOIST BEFORE	MOIST AFTER
Wgt Sample+Mold(g)	11377.8	11327.8	11553	11553	11615.9	11615.9
Wgt Mold(g)	7322.7	7322.7	7315	7715	7121.4	7121.4
Wgt Wet Sample(g)	4055.1	4005.1	4238	3838	4494.5	4494.5
Wgt Wet Sample(lbs.)	8.94	8.83	9.34	8.46	9.91	9.91
Wet Density(lbs./cu.ft)	119.2	117.7	124.5	112.8	132.1	132.1
Moist Sample Wet(g)	380	437.5	238	425	378.4	297.7
Moist Sample Dry(g)	351.9	403.3	220.4	390.3	350.4	272.9
% Moisture	8.0%	8.5%	8.0%	8.9%	8.0%	9.1%
Dry Density(lbs./cu.ft)	110.4	108.5	115.3	103.6	122.3	121.1

PROCTOR INFO.

ASTM 698C DENSITY	128.0
ASTM 698C MOISTURE	8.0%

SWELL CALCULATIONS

POINT 1			POINT 2			POINT 3		
DATE	READING	% SWELL	DATE	READING	% SWELL	DATE	READING	% SWELL
1/27/2023	0		1/27/2023	0		1/27/2023	0	
1/31/2023	0.008	0.2%	1/31/2023	0	0.0%	1/31/2023	0.015	0.3%

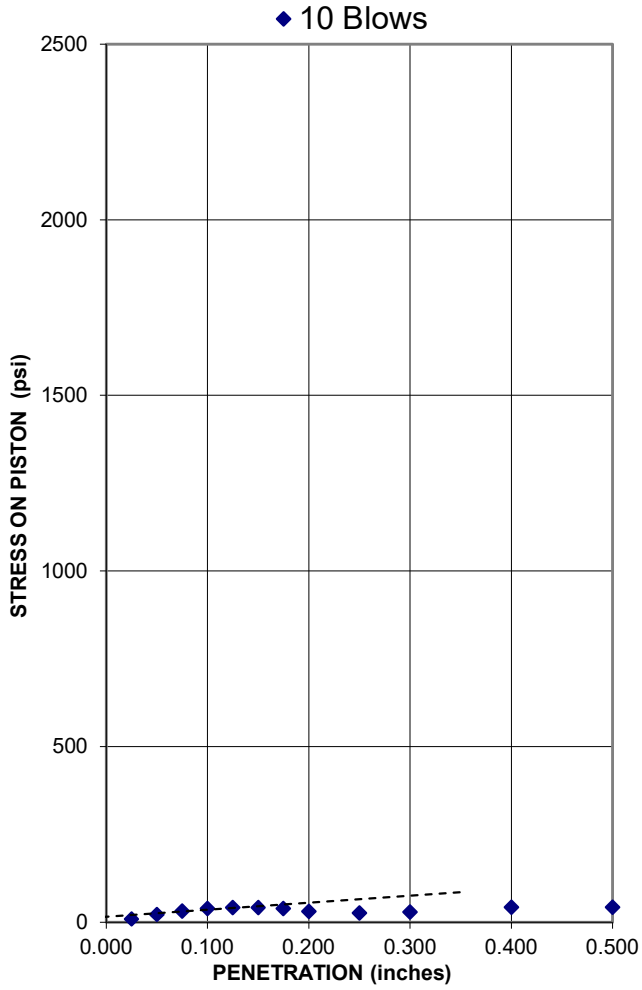
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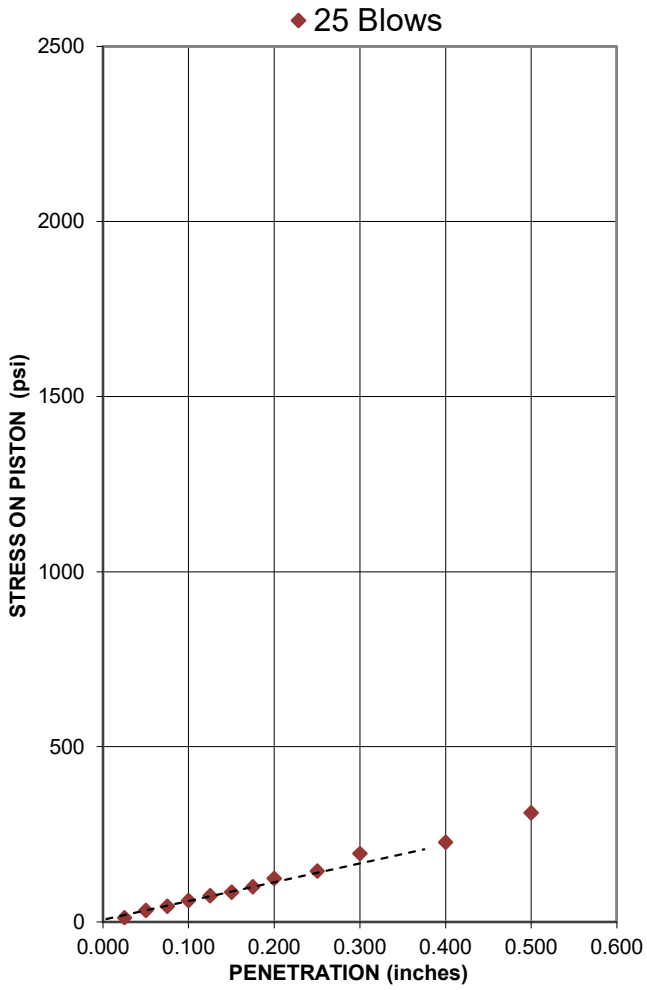
CBR(CALIFORNIA BEARING RATIO) OF LABORATORY-COMPACTED SOILS
ASTM D1883 (SOAKED)

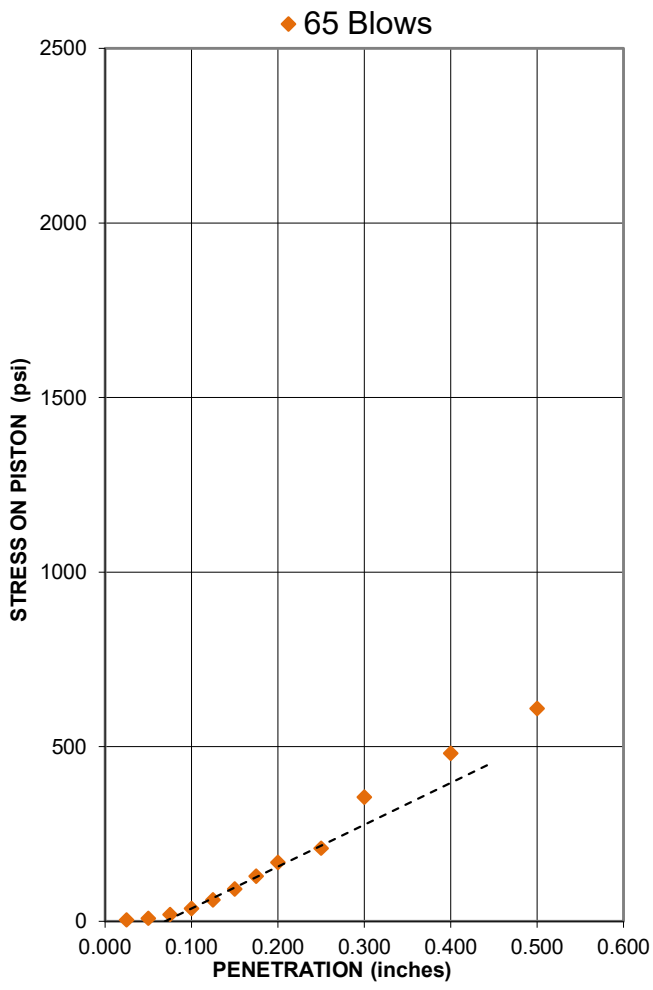
Terracon

PROJECT:	Grace Solar Basis of Bid	**JOB NO:**	60225164
LOCATION:	Blythe, CA	**WORK ORDER NO:**	60225164.0004
MATERIAL:	Auger	**LAB NO:**	BESS-7 S1 @1-4
SAMPLE SOURCE:	BESS-7 S1 @1-4	**DATE SAMPLED:**	4/22/25

	Blows per Lift	Compaction %	Compaction Method	Surcharge Weight (Lb.)	Sample Condition	Percent Swell	Before Soak		After Soak	
							Dry Density	Moisture %	Dry Density	Moisture %
Point 1	10	86.2	ASTM D1557	10	Soak	0.17	110.4	8.0	108.5	8.5
Point 2	25	90.1	ASTM D1557	10	Soak	0.00	115.3	8.0	103.6	8.9
Point 3	65	95.6	ASTM D1557	10	Soak	0.33	122.3	8.0	121.1	9.1







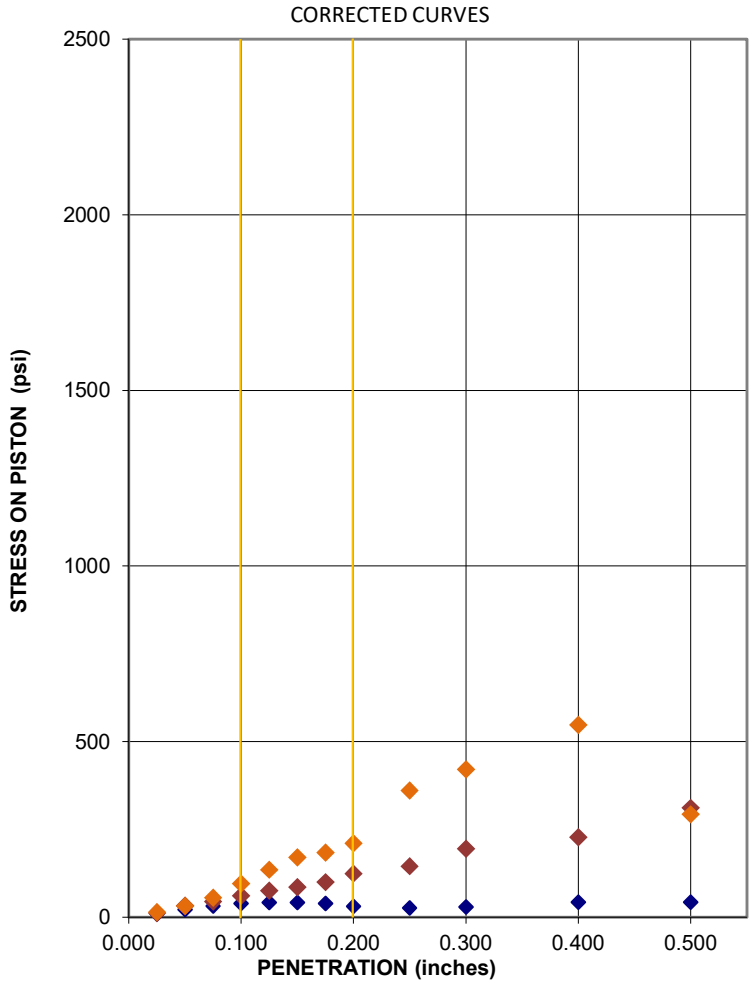
REVIEWED BY _____

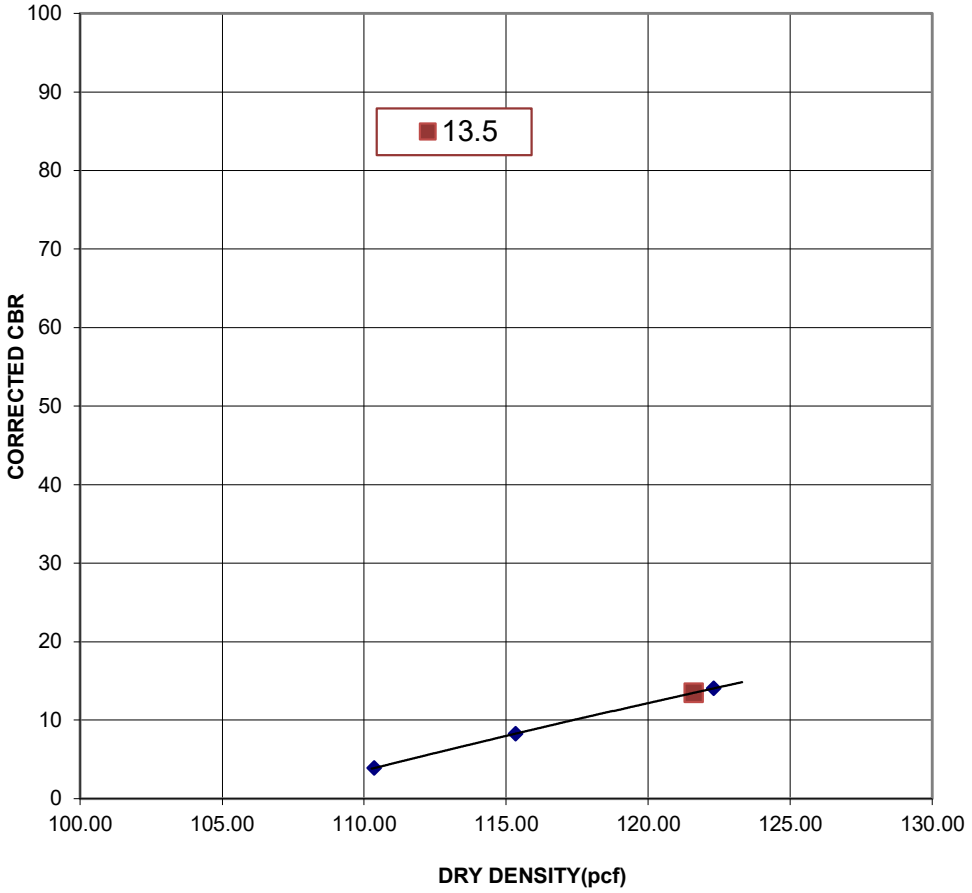
CBR(CALIFORNIA BEARING RATIO) OF LABORATORY-COMPACTED SOILS
ASTM D1883 (SOAKED)

PROJECT:	Grace Solar Basis of Bid	**JOB NO:**	60225164
LOCATION:	Blythe, CA	**WORK ORDER NO:**	60225164.0004
MATERIAL:	Auger	**LAB NO:**	BESS-7 S1 @1-4
SAMPLE SOURCE:	BESS-7 S1 @1-4	**DATE SAMPLED:**	4/22/25

	Correction Factor		Corrected CBR		
	0.1"	0.2"	0.1"	0.2"	CBR
Point 1	0.00	0.00	3.9	2.1	3.9
Point 2	0.00	0.00	6.1	8.3	8.3
Point 3	0.05	0.05	9.5	14.0	14.0

ASTM D1557				95% COMPACTION CBR
Moisture	Dry Density (pcf)			
%	100%	95%	90%	13.5
8.0	128.0	121.6	115.2	





CBR(CALIFORNIA BEARING RATIO) OF LABORATORY-COMPACTED SOILS(ASTM D1883)

ENTER INTO CBR.XLS

PROJECT:	Grace Solar Basis of Bid	**JOB NO:**	60225164
LOCATION:	Blythe, CA	**WORK ORDER NO:**	60225164.0004
MATERIAL:	Auger	**LAB NO:**	BESS-7 S1 @1-4
SAMPLE SOURCE:	BESS-7 S1 @1-4	**DATE SAMPLED:**	04/22/25

			MOIST BEFORE	MOIST AFTER
Diameter of Mold(in)	6"	Wgt Sample+Mold(g)		
Height of Mold(in)		Wgt Mold(g)		
Number of Layers	5	Wgt Wet Sample(g)		
Blows per Layer	**10**	Wgt Wet Sample(lbs)		
Weight of Hammer(lb)	5	Wet Density(lbs/cu.ft)		
Height of Drop(in)	12	Moist Sample Wet(g)		
Materials Used	-3/4"	Moist Sample Dry(g)		
Spacer(in)	Volume(cu.ft)	% Moisture		
Surcharge Weights:	10 Lbs	Dry Density(lbs/cu.ft)		
Soaking(lbs)				
Penetration(lbs)				

Max. Dry Density(lbs/cu.ft) **TECHNICIAN:**

Optimum Moisture Content(%) **DATE:**

PENETRATION DATA

PENETRATION	STANDARD UNIT LOAD	PROVING RING READING	TOTAL UNIT LOAD	UNIT LOAD	CORRECTED UNIT LOAD	C B R
0.025	250	0.0010				
0.050	500	0.0023				
0.075	750	0.0034				
0.100	1000	0.0042				
0.125	1125	0.0050				
0.150	1250	0.0058				
0.175	1375	0.0066				
0.200	1500	0.0070				
0.250	1700	0.0084				
0.300	1900	0.0098				
0.400	2300	0.0119				
0.500	2600	0.0140				

CONDITION OF MATERIAL AS RECEIVED:
ACCEPTABLE_____
NOT ACCEPTABLE_____

SWELL CALCULATIONS

DATE	TIME	DAYS	READING	SWELL (in)	% SWELL
		0			
		1			
		2			
		3			
		4			

750 Pilot Road, Suite F
Las Vegas, Nevada 89119
(702) 597-9393

Client
NextEra Energy Constructors LLC

Project
Grace Solar Basis of Bid

Sample Submitted By: Terracon (60) **Date Received:** 4/3/2025 **Lab No.: 25-0099**

Results of Corrosion Analysis

Sample Number	--	--	--	--
Sample Location	MV-2	MV-7	TR-UG1	BESS-02
Sample Depth (ft.)	1.0-4.0	1.0-4.0	1.0-4.0	1.0-4.0
pH Analysis, ASTM G51	8.75	9.14	8.89	9.14
Water Soluble Sulfate (SO4), ASTM C 15806 (Percent %)	0.01	<0.01	0.01	0.01
Sulfides, AWWA 4500-S D, (mg/Kg)	Nil	Nil	Nil	Nil
Chlorides, ASTM D512, (mg/kg)	50	52	27	325
Red-Ox, ASTM G200, (mV)	+728	+734	+735	+721
Total Salts, AWWA 2540, (mg/Kg)	420	238	182	1232
Saturated Minimum Resistivity, ASTM G-187, (ohm-cm)	1675	1876	2144	3149

Analyzed By N. Campo

Nathan Campo
Laboratory Coordinator

The tests were performed in general accordance with applicable ASTM and AWWA test methods. This report is exclusively for the use of the client indicated above and shall not be reproduced except in full without the written consent of our company. Test results transmitted herein are only applicable to the actual samples tested at the location(s) referenced and are not necessarily indicative of the properties of other apparently similar or identical materials.

750 Pilot Road, Suite F
Las Vegas, Nevada 89119
(702) 597-9393

Client
NextEra Energy Constructors LLC

Project
Grace Solar Basis of Bid

Sample Submitted By: Terracon (60) **Date Received:** 4/3/2025 **Lab No.: 25-0099**

Results of Corrosion Analysis

Sample Number	--	--
Sample Location	BESS-08	BESS-09
Sample Depth (ft.)	1.0-4.0	1.0-4.0
pH Analysis, ASTM G51	8.58	9.38
Water Soluble Sulfate (SO4), ASTM C 15806 (Percent %)	0.01	0.02
Sulfides, AWWA 4500-S D, (mg/Kg)	Nil	Nil
Chlorides, ASTM D512, (mg/kg)	130	190
Red-Ox, ASTM G200, (mV)	+727	+724
Total Salts, AWWA 2540, (mg/Kg)	704	794
Saturated Minimum Resistivity, ASTM G-187, (ohm-cm)	1072	737

Analyzed By N. Campo

Nathan Campo
Laboratory Coordinator

The tests were performed in general accordance with applicable ASTM and AWWA test methods. This report is exclusively for the use of the client indicated above and shall not be reproduced except in full without the written consent of our company. Test results transmitted herein are only applicable to the actual samples tested at the location(s) referenced and are not necessarily indicative of the properties of other apparently similar or identical materials.

9123 Chesapeake Dr
San Diego, CA 92123
(619) 821-3630

Client
NextEra Energy Resources LLC

Project
Grace Solar

Sample Submitted By: V. Nguyen **Date Tested:** 3/18/2025 **Project Number:** 60225164

Sample Type	Auger	Auger
Sample Location	BESS-15	BESS-15
Sample Depth (ft.)	1.0' - 4.0'	4.0' - 6.0'
pH Analysis, ASTM G 51	7.18	7.77
Water Soluble Sulfate (SO4), ASTM D516 (%)	0.11	0.09
Sulfides, AWWA 4500-S^{2-} D, (mg/kg)	0.86	1.23
Chlorides, ASTM D512, (mg/kg)	41.0	29.0
Red-Ox, ASTM G 200, (mV)	+157	+175
Total Salts, AWWA 2520 B, (mg/kg)	275.0	169.0
Saturated Minimum Resistivity, ASTM G 57, (ohm-cm)	470	1,020

Reviewed By:
Tom Remmel
Laboratory Manager

The tests were performed in general accordance with applicable ASTM and AWWA test methods. This report is exclusively for the use of the client indicated above and shall not be reproduced except in full without the written consent. Test results transmitted herein are only applicable to the actual samples tested at the location(s) referenced and are not necessarily indicative of the properties of other apparently similar or identical materials.

9123 Chesapeake Dr
San Diego, CA 92123
(619) 821-3630

Client
NextEra Energy Resources LLC

Project
Grace Solar

Sample Submitted By: V. Nguyen **Date Tested:** 3/18/2025 **Project Number:** 60225164

Sample Type	Auger	Auger
Sample Location	SUB-3	SUB-7
Sample Depth (ft.)	1.0' - 4.0'	1.0' - 4.0'
pH Analysis, ASTM G 51	7.45	7.53
Water Soluble Sulfate (SO4), ASTM D516 (%)	0.05	0.04
Sulfides, AWWA 4500-S^{2-} D, (mg/kg)	1.02	0.69
Chlorides, ASTM D512, (mg/kg)	18.8	259.3
Red-Ox, ASTM G 200, (mV)	+174	+176
Total Salts, AWWA 2520 B, (mg/kg)	374.0	450.0
Saturated Minimum Resistivity, ASTM G 57, (ohm-cm)	440	332

Reviewed By:
Tom Remmel
Laboratory Manager

The tests were performed in general accordance with applicable ASTM and AWWA test methods. This report is exclusively for the use of the client indicated above and shall not be reproduced except in full without the written consent. Test results transmitted herein are only applicable to the actual samples tested at the location(s) referenced and are not necessarily indicative of the properties of other apparently similar or identical materials.

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http://www.geothermusa.com

April 29, 2025

Terracon
23041 Avenida de la Carlota, Suite 350
Laguna Hills, CA 92653
Attn: Victor Nguyen

Thermal Resistivity Report
Grace Solar – Blythe, CA (Project No. 60225164)

The following is the report of thermal dryout characterization tests conducted on seven (7) native soil samples from the referenced project sent to our laboratory.

Thermal Resistivity Tests: The samples were tested at either the 'optimum' or 'as received' moisture content and at the 'specified' Proctor dry density ***provided by Terracon***. The tests were conducted in accordance with the **IEEE standard 442-2017**. The results are tabulated below and the thermal dry out curves are presented in **Figures 1 to 4**.

Sample ID, Description, Thermal Resistivity, Moisture Content and Density

Sample ID	Depth (ft)	Effort (%)	Description (Terracon)	Thermal Resistivity (°C-cm/W)		Moisture Content (%)	Dry Density (lb/ft^3)
				Wet	Dry		
BESS-02	1 - 4	85	---	54	150	7	114
BESS-02	1 - 4	95	---	49	114	7	127
BESS-02	4 - 6	90	---	51	135	8	117
BESS-08	1 - 4	85	---	57	158	8	112
BESS-08	1 - 4	95	---	51	120	8	125
BESS-08	4 - 6	90	---	53	131	7	118
BESS-09	1 - 4	85	---	60	190	9	109
BESS-09	1 - 4	95	---	54	144	9	122

Geotherm

Sample ID, Description, Thermal Resistivity, Moisture Content and Density

Sample ID	Depth (ft)	Effort (%)	Description (Terracon)	Thermal Resistivity (°C-cm/W)		Moisture Content (%)	Dry Density (lb/ft^3)
				Wet	Dry		
TR-UG1	1 - 4	85	---	69	254	11	98
TR-UG1	1 - 4	95	---	62	192	11	109
TR-UG1	4 - 6	90	---	64	196	12	101

Comments: The thermal characteristic depicted in the dryout curves apply for the soils at their respective test dry density.

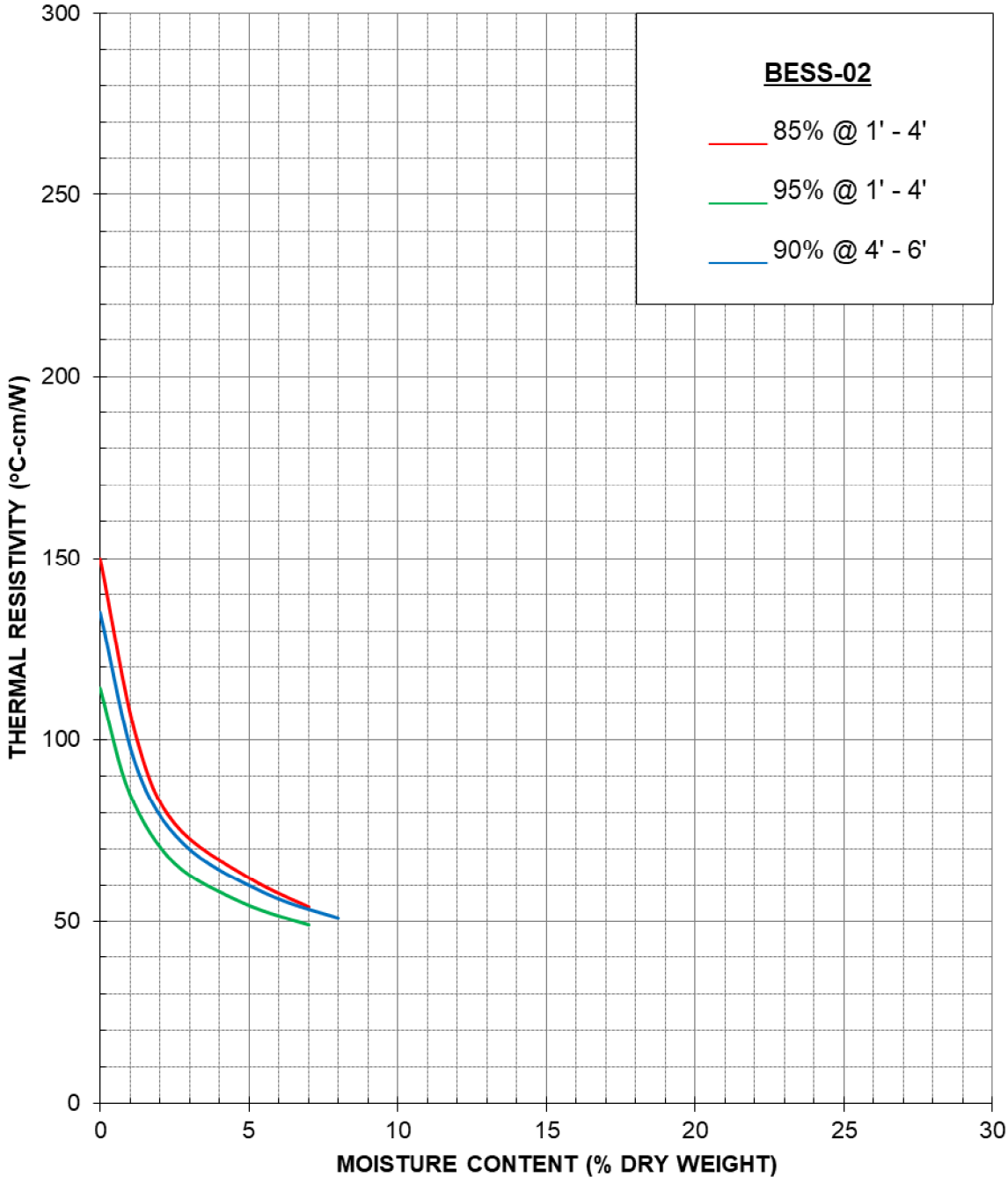
Please contact us if you have any questions or if we can be of further assistance.

Geotherm USA, LLC

Nimesh Patel

Geotherm

THERMAL DRYOUT CURVES



Terracon (Project No. 60225164)

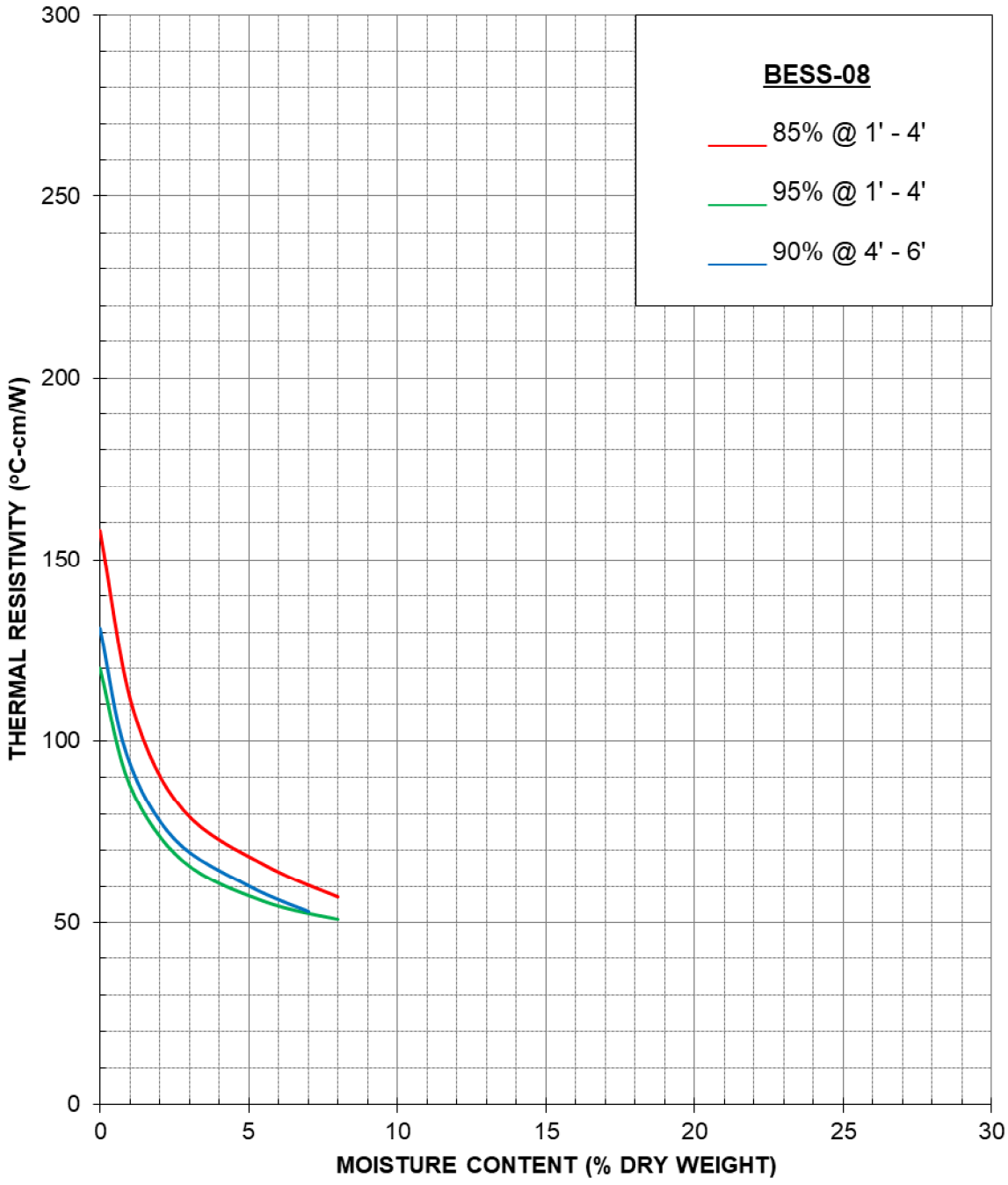
Grace Solar – Blythe, CA

Thermal Resistivity Report

April 2025 **Figure 1**

Geotherm

THERMAL DRYOUT CURVES



Terracon (Project No. 60225164)

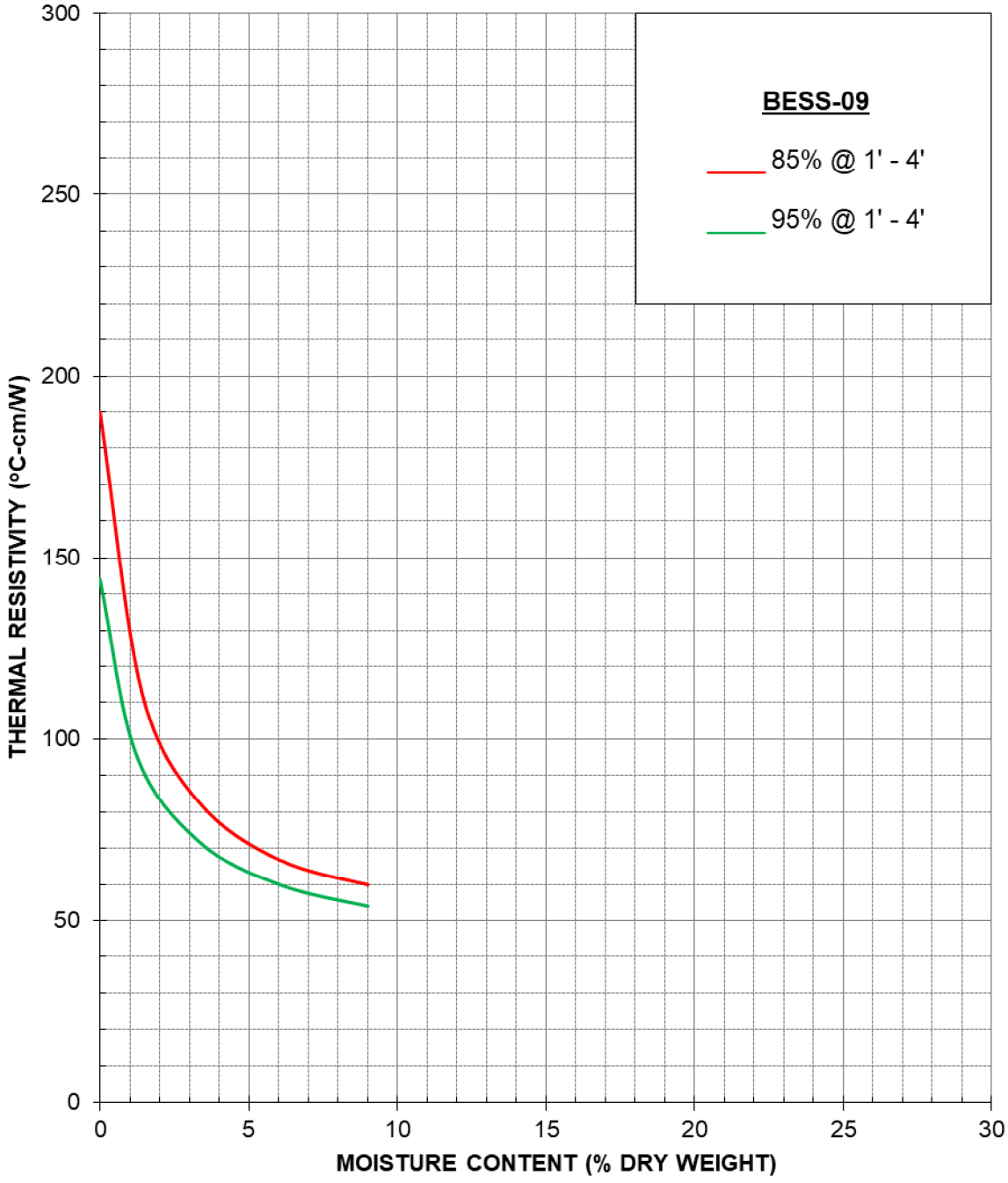
Grace Solar – Blythe, CA

Thermal Resistivity Report

April 2025 **Figure 2**

Geotherm

THERMAL DRYOUT CURVES



Terracon (Project No. 60225164)

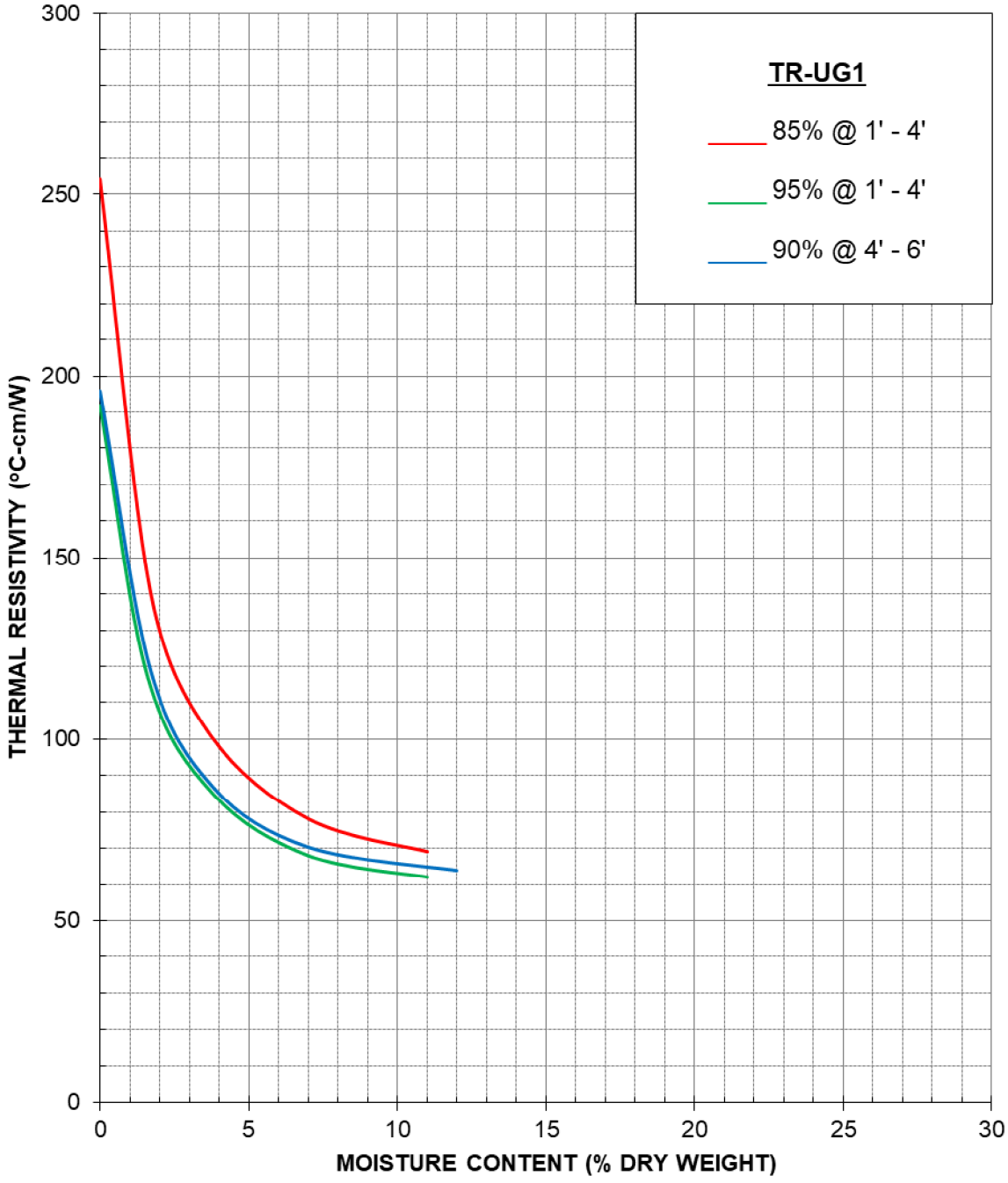
Grace Solar – Blythe, CA

Thermal Resistivity Report

April 2025 **Figure 3**

Geotherm

THERMAL DRYOUT CURVES



Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Resistivity Report

April 2025 **Figure 4**

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April 4, 2025

Terracon
23041 Avenida de la Carlota, Suite 350
Laguna Hills, California 92653
Attn: Victor Nguyen

Thermal Resistivity Report
Grace Solar – Blythe, CA (Project No. 60225164)

The following is the report of thermal dryout characterization tests conducted on two (2) bulk samples of native soil from the referenced project sent to our laboratory.

Thermal Resistivity Tests: The samples were tested at the 'optimum' moisture content and at 85% and 95% of the modified Proctor dry density ***provided by Terracon***. The tests were conducted in accordance with the **IEEE standard 442-2017**. The results are tabulated below and the thermal dryout curves are presented in **Figure 1.**

Sample ID, Description, Thermal Resistivity, Moisture Content and Density

Sample ID	Depth (ft)	Effort (%)	Description (Terracon)	Thermal Resistivity (°C-cm/W)		Moisture Content (%)	Dry Density (lb/ft^3)
				Wet	Dry		
BESS-15	1 – 4	85	---	72	184	8	113
BESS-15	1 – 4	95	---	65	140	8	127
BESS-15	4 – 6	85	---	67	175	8	113
BESS-15	4 – 6	95	---	61	132	8	126

Comments: The thermal characteristic depicted in the dryout curves apply for the soils at their respective test dry density.

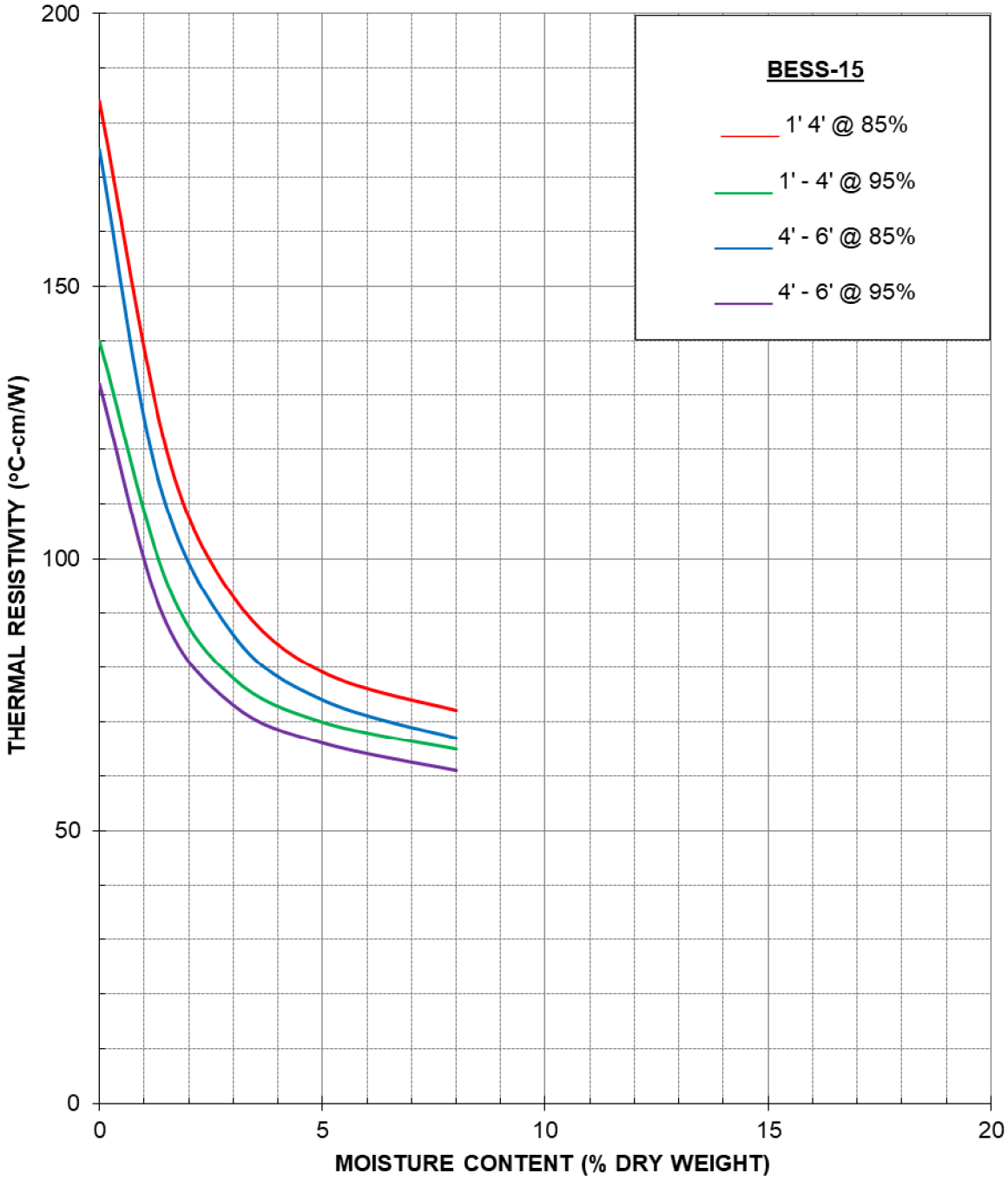
Please contact us if you have any questions or if we can be of further assistance.

Geotherm USA, LLC

Nimesh Patel

Geotherm

THERMAL DRYOUT CURVES



Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Resistivity Report

April 2025 **Figure 1**

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June 30, 2025

Terracon
23041 Avenida de la Carlota, Suite 350
Laguna Hills, CA 92653
Attn: Victor Nguyen

Thermal Resistivity Report
Grace Solar – Blythe, CA (Project No. 60225164)

The following is the report of thermal dryout characterization tests conducted on five (5) bulk samples of native soil from the referenced project sent to our laboratory.

Thermal Resistivity Tests: The samples were tested at the 'optimum' moisture content and at 85%, 90% and/or 95% of the modified Proctor dry density ***provided by Terracon***. The tests were conducted in accordance with the **IEEE standard 442-2017**. The results are tabulated below and the thermal dry out curves are presented in **Figures 1 to 3**.

Sample ID, Description, Thermal Resistivity, Moisture Content and Density

Sample ID	Depth (ft)	Effort (%)	Description (Terracon)	Thermal Resistivity (°C-cm/W)		Moisture Content (%)	Dry Density (lb/ft^3)
				Wet	Dry		
BESS-09	4 - 6	90	---	58	278	13	97
STP-2	1 - 4	85	---	66	160	8	109
STP-2	1 - 4	95	---	49	111	8	122
STP-2	4 - 6	90	---	60	214	11	95
STP-3	1 - 4	85	---	53	140	8	110
STP-3	1 - 4	90	---	45	110	8	117
STP-3	4 - 6	90	---	52	152	8	109

Geotherm

Comments: The thermal characteristic depicted in the dryout curves apply for the soils at their respective test dry density.

Please contact us if you have any questions or if we can be of further assistance.

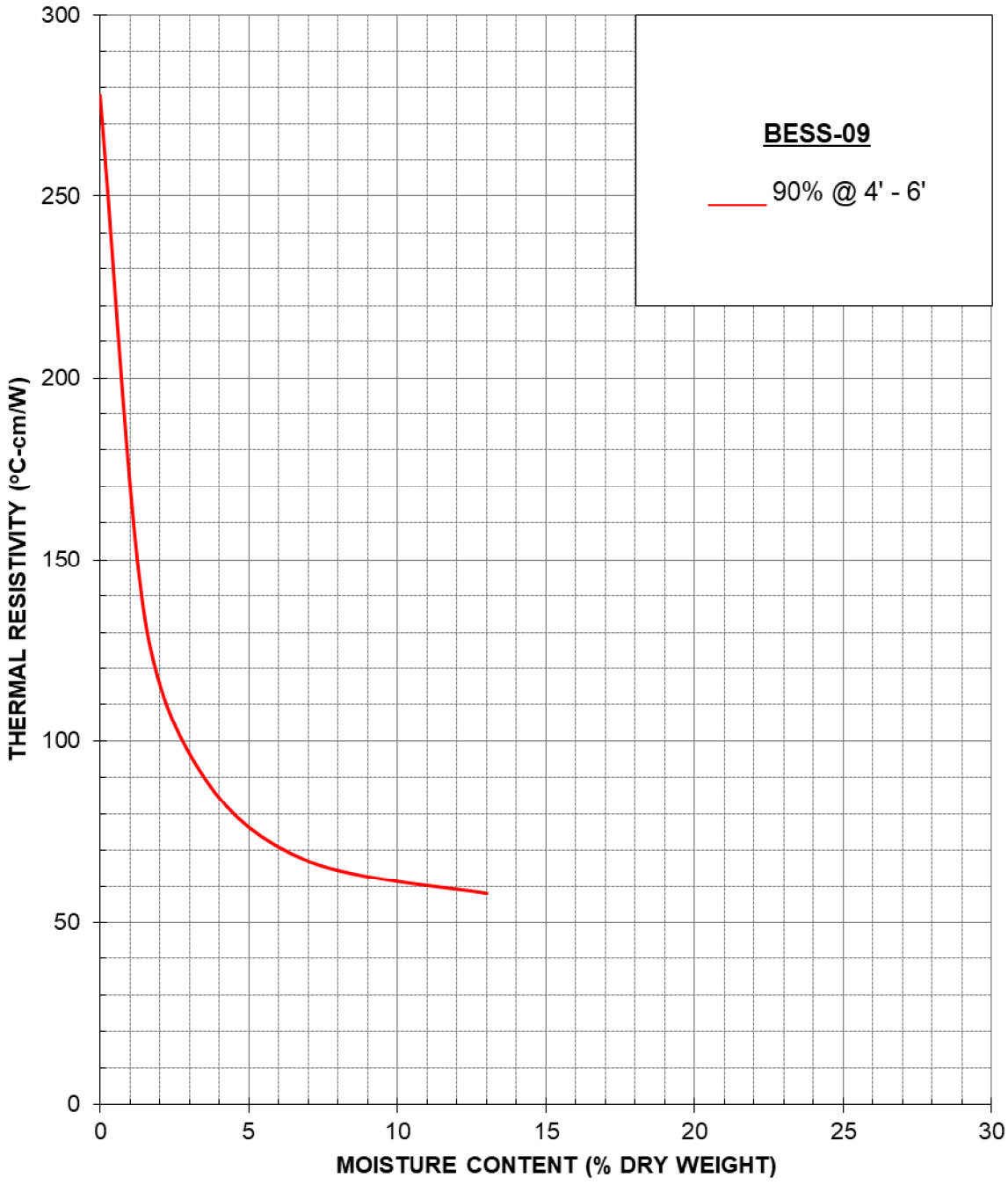
Geotherm USA, LLC



Nimesh Patel

Geotherm

THERMAL DRYOUT CURVE



Terracon (Project No. 60225164)

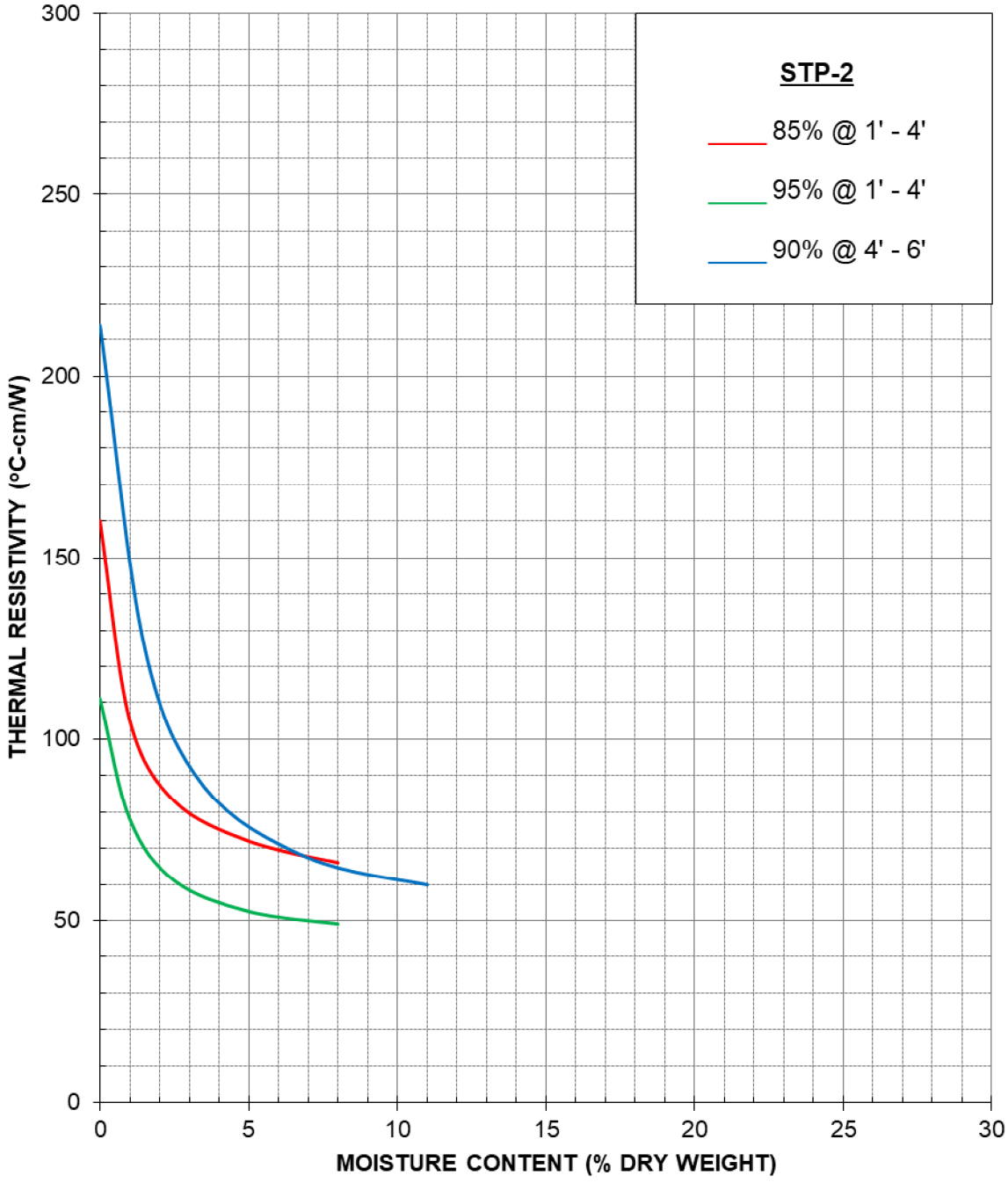
Grace Solar – Blythe, CA

Thermal Resistivity Report

June 2025 **Figure 1**

Geotherm

THERMAL DRYOUT CURVES



Terracon (Project No. 60225164)

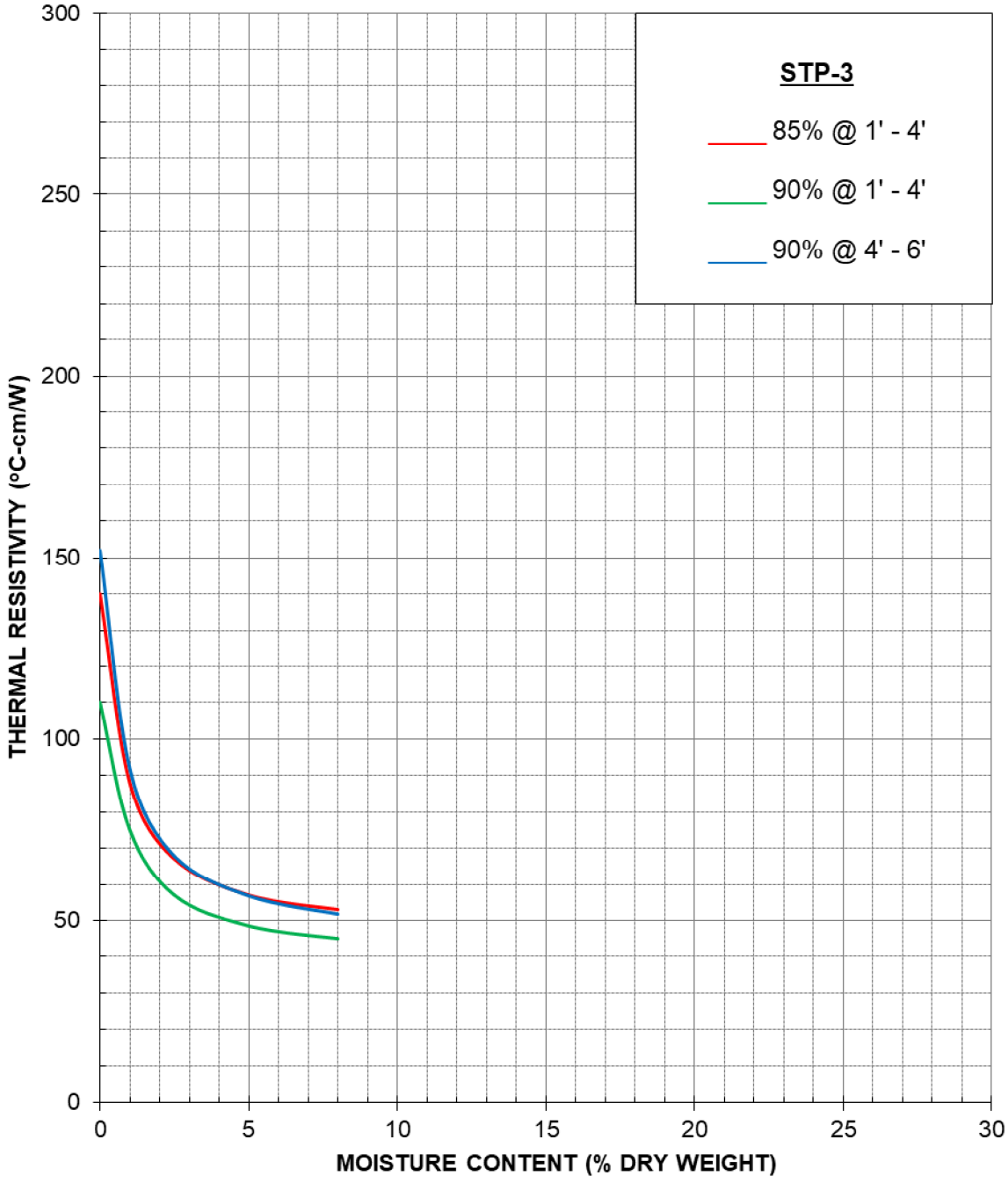
Grace Solar – Blythe, CA

Thermal Resistivity Report

June 2025 **Figure 2**

Geotherm

THERMAL DRYOUT CURVES



Terracon (Project No. 60225164)

Grace Solar – Blythe, CA

Thermal Resistivity Report

June 2025 **Figure 3**

Field Soil Electrical Resistivity Test Results

Contents:

Electrical Resistivity Test Procedures
Electrical Resistivity Test Locations Plan
Electrical Resistivity Results

Field Electrical Resistivity Test Procedures

Field measurements of soil electrical resistivity were performed in general accordance with ASTM Test Method G 57, and IEEE Standard 81, using the Wenner Four-Electrode. The Wenner arrangement (equal electrode spacing) was used with the following "a" spacings:

- 0.5, 1, 1.5, 2, 3, 5, 7, 10, 15, 20, 30, 45, 70, 100, 150, 200, 250, 350, and 450 feet within the planned substation and BESS area

The "a" spacing is generally considered to be the depth of influence of the test. The testing was performed in both a north-south and an east-west orientation.

Electrical Resistivity Test Location Plan

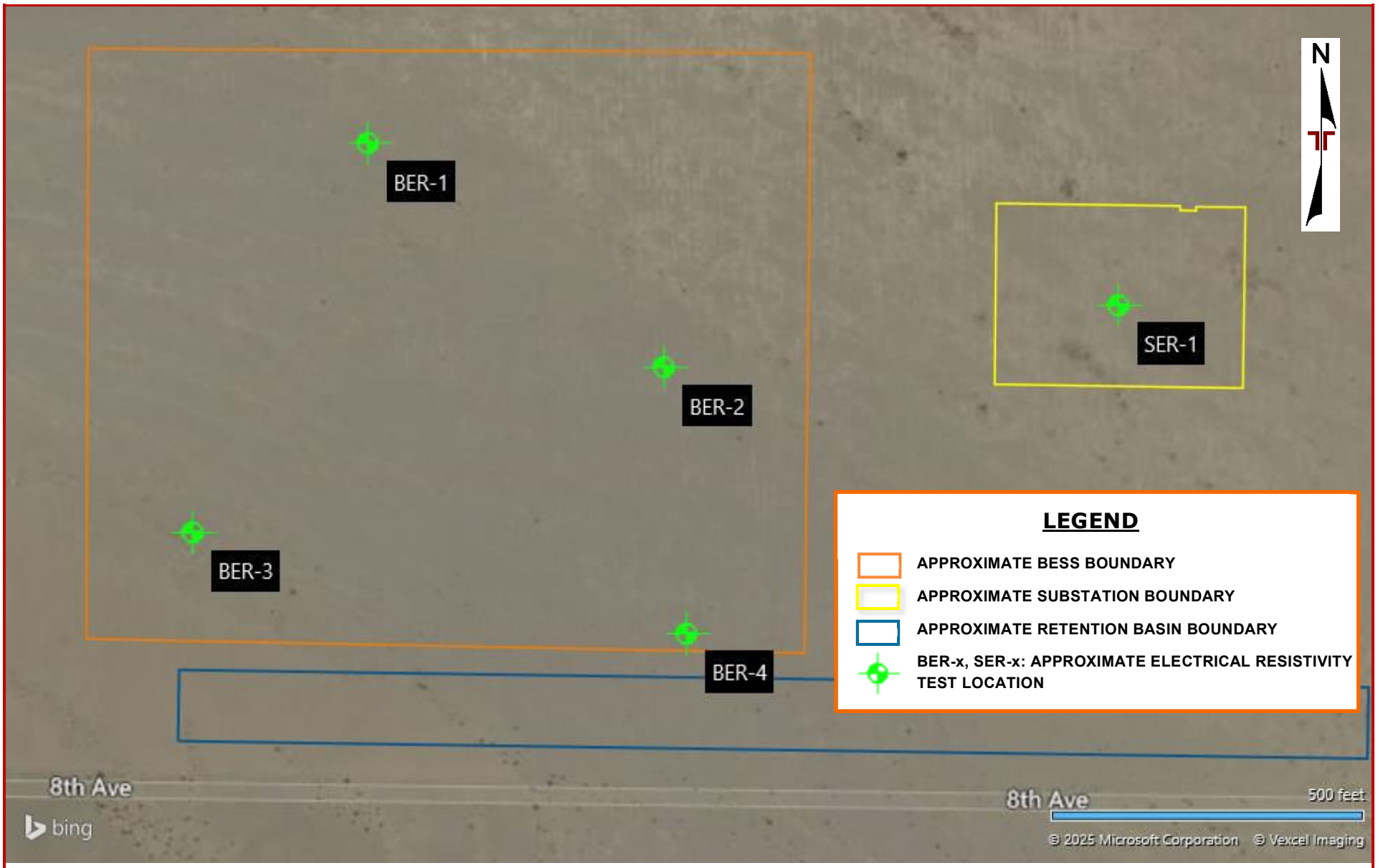


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

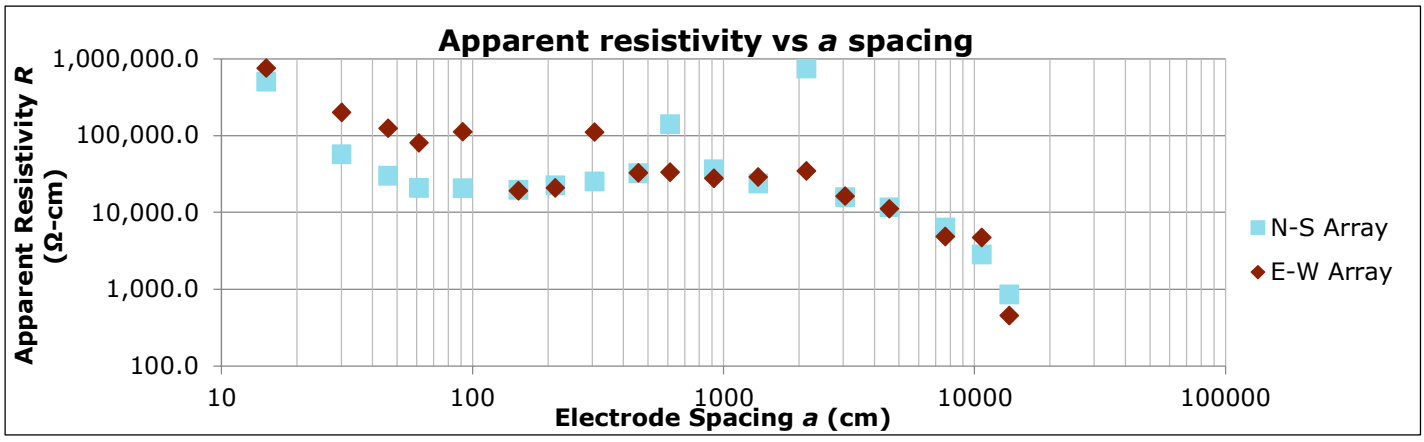
MAP PROVIDED BY MICROSOFT BING MAPS

FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace Solar|Blythe, Riverside County, California
Terracon Project No. 60225164

Array Loc.	SER-1 (33.6485, -114.7208)		
Instrument	TerraMeter	**Weather**	Sunny
Serial #	27545004	**Ground Cond.**	Soil
Calib. Due	August 28, 2025	**Tested By**	AT/EA
Test Date	March 3, 2025	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Electrode depths were increased at shorter spacings to achieve more conductivity in order to obtain higher quality data. Interference from unknown source observed in the highlighted cells. Highlighted readings are considered irregular.		

Apparent resistivity ρ is calculated as : $$\rho = \frac{4\pi a R}{1 + \frac{2a}{\sqrt{a^2 + 4b^2}} - \frac{a}{\sqrt{a^2 + b^2}}}$$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
0.5	15	2	5	4,600	505,460	6,940	762,590
1	30	2	5	292	57,600	1,020	201,220
1.5	46	2	5	103	30,370	425	125,330
2	61	2	5	54	20,980	208	80,650
3	91	2	5	36	20,810	197	113,230
5	152	3	8	21	19,960	20	19,390
7	213	5	13	17	22,630	16	21,150
10	305	6	15	13	25,600	58	112,200
15	457	9	23	11	32,590	11	32,880
20	610	12	30	37	142,410	8.7	33,490
30	914	12	30	6.3	36,480	4.9	28,140
45	1,372	12	30	2.8	23,990	3.4	29,250
70	2,134	12	30	56	744,420	2.6	35,010
100	3,048	12	30	0.83	15,970	0.86	16,490
150	4,572	12	30	0.41	11,750	0.39	11,230
250	7,620	12	30	0.13	6,420	0.10	4,880
350	10,668	12	30	0.043	2,870	0.071	4,780
450	13,716	12	30	0.010	860	0.0053	460

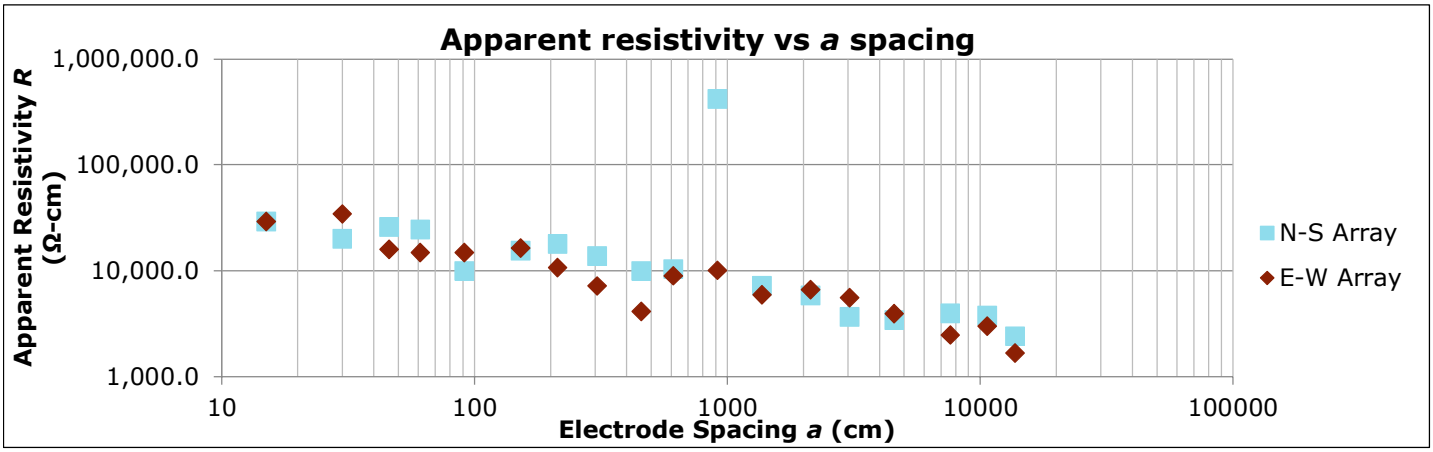


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace Solar|Blythe, Riverside County, California
Terracon Project No. 60225164

Array Loc.	BER-1 (33.6492, -114.7247)		
Instrument	TerraMeter	**Weather**	Sunny
Serial #	27545004	**Ground Cond.**	Soil
Calib. Due	August 28, 2025	**Tested By**	AT/EA
Test Date	March 3, 2025	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Electrode depths were increased at shorter spacings to achieve more conductivity in order to obtain higher quality data. Interference from unknown source observed in the highlighted cells. Highlighted readings are considered irregular.		

Apparent resistivity ρ is calculated as : $$\rho = \frac{4\pi a R}{1 + \frac{2a}{\sqrt{a^2 + 4b^2}} - \frac{a}{\sqrt{a^2 + b^2}}}$$

Electrode Spacing *a*		Electrode Depth *b*		N-S Test		E-W Test	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
0.5	15	2	5	264	29,010	264	29,010
1	30	2	5	101	19,920	174	34,330
1.5	46	2	5	88	25,800	54	15,840
2	61	2	5	63	24,430	38	14,850
3	91	2	5	17	9,890	26	14,830
5	152	3	8	16	15,450	17	16,310
7	213	5	13	13	17,910	7.9	10,670
10	305	6	15	7.1	13,740	3.7	7,160
15	457	9	23	3.4	9,890	1.4	4,120
20	610	12	30	2.7	10,350	2.3	8,970
30	914	12	30	72	416,560	1.8	10,070
45	1,372	12	30	0.84	7,230	0.69	5,920
70	2,134	12	30	0.44	5,830	0.49	6,600
100	3,048	12	30	0.19	3,660	0.29	5,550
150	4,572	12	30	0.12	3,420	0.14	3,910
250	7,620	12	30	0.083	3,960	0.051	2,460
350	10,668	12	30	0.056	3,750	0.045	3,000
450	13,716	12	30	0.028	2,390	0.019	1,660

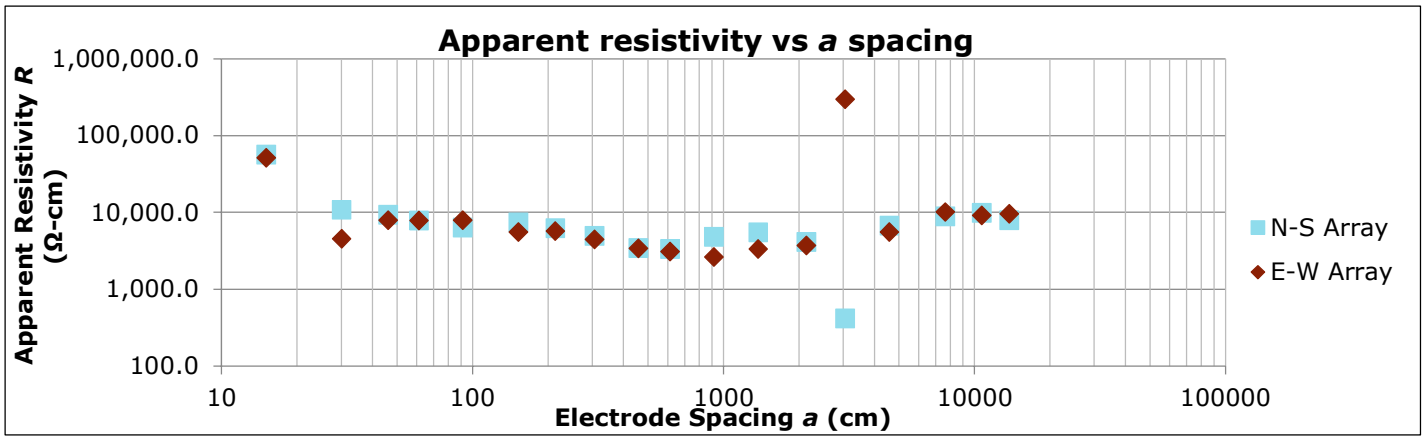


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace Solar|Blythe, Riverside County, California
Terracon Project No. 60225164

Array Loc.	BER-2 (33.6494, -114.7213)		
Instrument	TerraMeter	**Weather**	Sunny
Serial #	27545004	**Ground Cond.**	Soil
Calib. Due	August 28, 2025	**Tested By**	AT/EA
Test Date	March 4, 2025	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Electrode depths were increased at shorter spacings to achieve more conductivity in order to obtain higher quality data. Interference from unknown source observed in the highlighted cells. Highlighted readings are considered irregular.		

Apparent resistivity ρ is calculated as : $$\rho = \frac{4\pi a R}{1 + \frac{2a}{\sqrt{a^2 + 4b^2}} - \frac{a}{\sqrt{a^2 + b^2}}}$$

Electrode Spacing *a*		Electrode Depth *b*		N-S Test		E-W Test	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
0.5	15	2	5	516	56,700	468	51,430
1	30	2	5	55	10,930	23	4,600
1.5	46	2	5	32	9,380	27	7,960
2	61	2	5	20	7,870	20	7,910
3	91	2	5	11	6,380	14	7,930
5	152	3	8	7.8	7,460	5.8	5,590
7	213	5	13	4.7	6,280	4.3	5,750
10	305	6	15	2.6	5,000	2.3	4,480
15	457	9	23	1.2	3,460	1.2	3,430
20	610	12	30	0.88	3,390	0.81	3,130
30	914	12	30	0.85	4,870	0.46	2,660
45	1,372	12	30	0.64	5,520	0.39	3,360
70	2,134	12	30	0.31	4,180	0.28	3,760
100	3,048	12	30	0.022	420	16	298,810
150	4,572	12	30	0.24	6,750	0.20	5,600
250	7,620	12	30	0.19	8,950	0.21	10,250
350	10,668	12	30	0.15	9,920	0.14	9,180
450	13,716	12	30	0.092	7,940	0.11	9,650

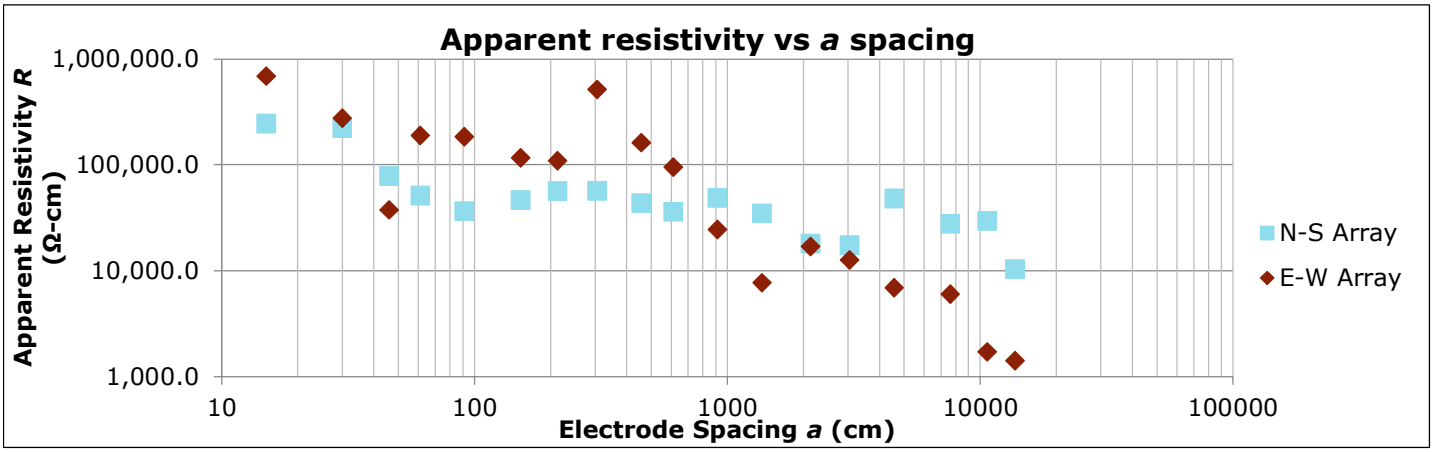


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace Solar|Blythe, Riverside County, California
Terracon Project No. 60225164

Array Loc.	BER-3 (33.6475, -114.7256)		
Instrument	TerraMeter	**Weather**	Sunny
Serial #	27545004	**Ground Cond.**	Soil
Calib. Due	August 28, 2025	**Tested By**	AT/EA
Test Date	March 4, 2025	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Electrode depths were increased at shorter spacings to achieve more conductivity in order to obtain higher quality data. Interference from unknown source observed in the highlighted cells. Highlighted readings are considered irregular.		

Apparent resistivity ρ is calculated as : $$\rho = \frac{4\pi a R}{1 + \frac{2a}{\sqrt{a^2 + 4b^2}} - \frac{a}{\sqrt{a^2 + b^2}}}$$

Electrode Spacing *a*		Electrode Depth *b*		N-S Test		E-W Test	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
0.5	15	2	5	2,230	245,040	6,270	688,970
1	30	2	5	1,130	222,920	1,400	276,190
1.5	46	2	5	265	78,150	127	37,450
2	61	2	5	132	51,180	489	189,600
3	91	2	5	64	36,500	322	185,080
5	152	3	8	49	46,640	121	116,120
7	213	5	13	42	56,300	81	109,380
10	305	6	15	30	56,960	268	515,750
15	457	9	23	15	43,260	56	162,090
20	610	12	30	9.4	36,100	25	94,680
30	914	12	30	8.5	48,910	4.3	24,450
45	1,372	12	30	4.0	34,680	0.89	7,690
70	2,134	12	30	1.3	17,970	1.3	16,900
100	3,048	12	30	0.91	17,410	0.66	12,680
150	4,572	12	30	1.7	47,980	0.24	6,870
250	7,620	12	30	0.58	27,670	0.13	5,980
350	10,668	12	30	0.44	29,490	0.026	1,710
450	13,716	12	30	0.12	10,340	0.016	1,410

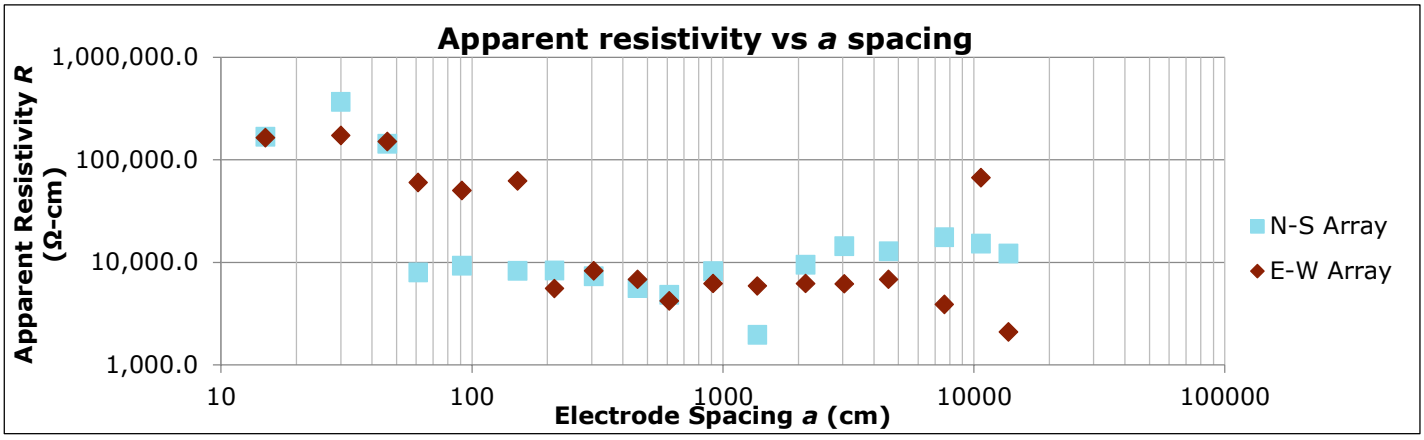


FIELD ELECTRICAL RESISTIVITY TEST DATA
Grace Solar|Blythe, Riverside County, California
Terracon Project No. 60225164

Array Loc.	BER-4 (33.6471, -114.7230)		
Instrument	TerraMeter	**Weather**	Sunny
Serial #	27545004	**Ground Cond.**	Soil
Calib. Due	August 28, 2025	**Tested By**	AT/EA
Test Date	March 4, 2025	**Method**	Wenner 4-pin (ASTM G57-06 (2012); IEEE 81-2012)
Notes & Conflicts	Electrode depths were increased at shorter spacings to achieve more conductivity in order to obtain higher quality data. Interference from unknown source observed in the highlighted cells. Highlighted readings are considered irregular.		

Apparent resistivity ρ is calculated as : $$\rho = \frac{4\pi a R}{1 + \frac{2a}{\sqrt{a^2 + 4b^2}} - \frac{a}{\sqrt{a^2 + b^2}}}$$

Electrode Spacing *a*		**Electrode Depth *b***		**N-S Test**		**E-W Test**	
(feet)	(centimeters)	(inches)	(centimeters)	Measured Resistance *R*	Apparent Resistivity ρ	Measured Resistance *R*	Apparent Resistivity ρ
				Ω	(Ω-cm)	Ω	(Ω-cm)
0.5	15	2	5	1,530	168,120	1,500	164,820
1	30	2	5	1,860	366,940	875	172,620
1.5	46	2	5	488	143,910	513	151,280
2	61	2	5	21	7,990	155	60,100
3	91	2	5	16	9,310	87	50,060
5	152	3	8	8.6	8,260	65	62,190
7	213	5	13	6.2	8,360	4.1	5,550
10	305	6	15	3.8	7,330	4.3	8,280
15	457	9	23	1.9	5,600	2.4	6,840
20	610	12	30	1.3	4,810	1.1	4,200
30	914	12	30	1.4	8,230	1.1	6,210
45	1,372	12	30	0.23	1,960	0.68	5,890
70	2,134	12	30	0.71	9,470	0.46	6,200
100	3,048	12	30	0.75	14,400	0.32	6,170
150	4,572	12	30	0.45	12,780	0.24	6,840
250	7,620	12	30	0.37	17,570	0.081	3,870
350	10,668	12	30	0.23	15,150	1.0	66,960
450	13,716	12	30	0.14	12,150	0.024	2,090



Test Pile Driving Data

Contents:

Test Pile Installation Details
Test Pile Installation Locations Plan
Test Pile Driving Records

Test Pile Installation Details

Pile Location Procedures

The test locations were established in the field by using a hand-held GPS (accurate to about 15 feet) and existing site features as reference points. The mapped test locations should be considered accurate only to the degree implied by the means and methods used to define them.

Test Pile Installation

The test piles consisted of wide-flange, bare steel W6x9 sections. A group of three test piles were installed at each of the test locations. The pile identification system for each location begins with "BPLT" and is followed by the number corresponding to the test pile group location and a letter identifying the pile. The "A" piles were installed to 8 feet below grade and the "B" piles were installed to 12 feet below grade. The "C" piles were installed to 12 feet.

The piles were advanced on with a track-mounted GAYK HRE 4000 equipped with a hydraulic hammer. The time rate of installation was recorded with a stopwatch. The total time required to advance each pile to its specified embedment depth was recorded and is summarized in the following graphs.

Pile Load Test Location Plan

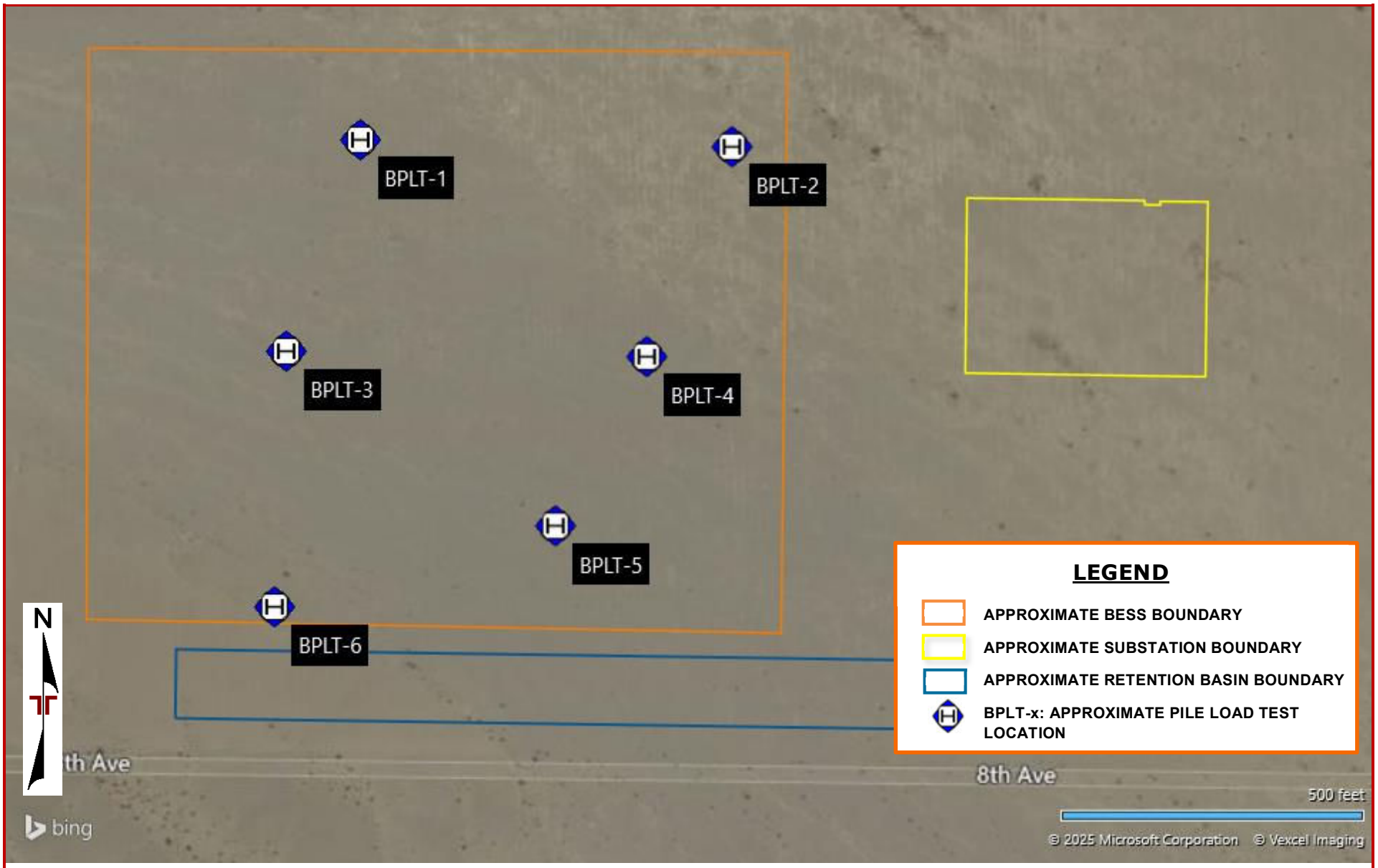


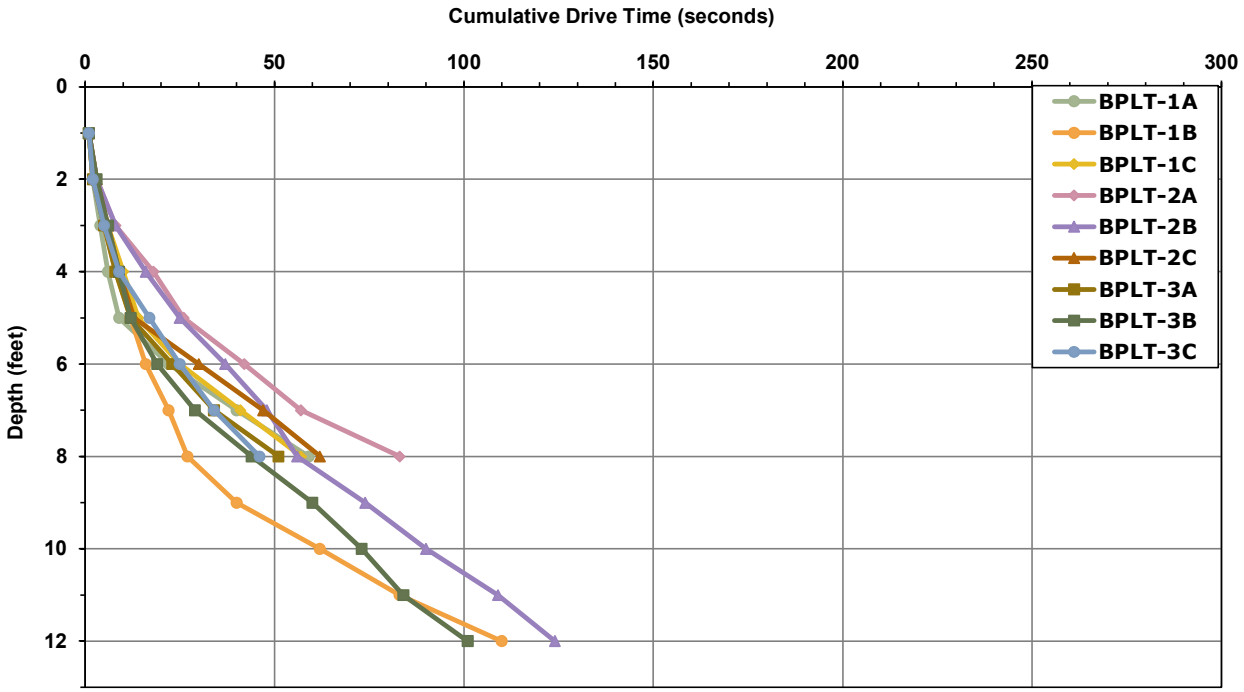
DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

TEST PILE DRIVING RECORDS

Grace Solar

Terracon Project No. 60225164



Pile Designation	Cumulative Driving Time, seconds (See Note 1)								
	BPLT-1A	BPLT-1B	BPLT-1C	BPLT-2A	BPLT-2B	BPLT-2C	BPLT-3A	BPLT-3B	BPLT-3C
Pile Push (in.) (See Note 2)	**8**	**9**	**8**	**11**	**9**	**10**	**7**	**9**	**10**
0 to 1	1	1	1	1	1	1	1	1	1
1 to 2	2	2	2	3	3	3	2	3	2
2 to 3	4	5	6	8	8	6	5	6	5
3 to 4	6	8	10	18	16	9	8	9	9
4 to 5	9	12	14	26	25	13	12	12	17
5 to 6	22	16	25	42	37	30	23	19	25
6 to 7	40	22	41	57	48	47	34	29	34
7 to 8	59	27	57	83	56	62	51	44	46
8 to 9		40			74			60	
9 to 10		62			90			73	
10 to 11		83			109			84	
11 to 12		110			124			101	
Install Date	**5/15/25**	**5/15/25**	**5/15/25**	**5/15/25**	**5/15/25**	**5/15/25**	**5/15/25**	**5/15/25**	**5/15/25**
Pile Size	**W6x9**	**W6x9**	**W6x9**	**W6x9**	**W6x9**	**W6x9**	**W6x9**	**W6x9**	**W6x9**
Refusal	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**	**No**
Embedment Depth, ft.	**8.0**	**12.0**	**8.0**	**8.0**	**12.0**	**8.0**	**8.0**	**12.0**	**8.0**
Total Drive Time, sec.	**59**	**110**	**57**	**83**	**124**	**62**	**51**	**101**	**46**
Average, sec./ft.	**7.4**	**9.2**	**7.1**	**10.4**	**10.3**	**7.8**	**6.4**	**8.4**	**5.8**

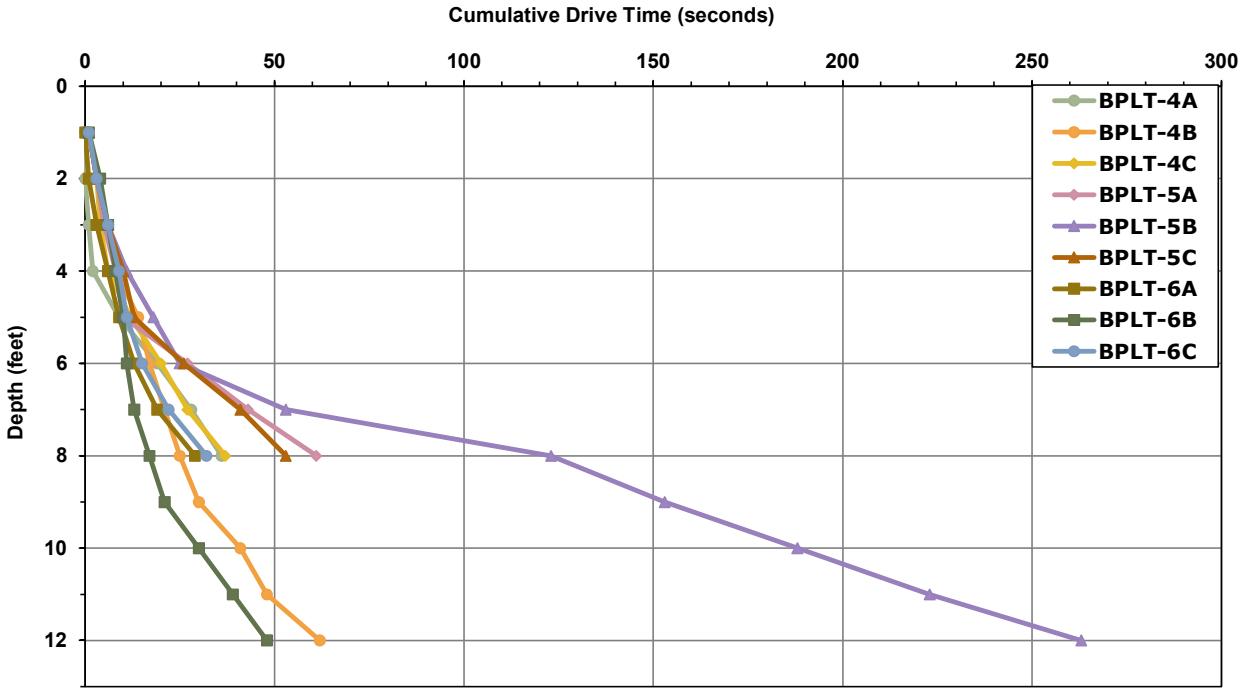
NOTES:

1. Piles advanced with a Track Mounted GAYK Model HRE 4000 pile driver.
2. Pile driving started at the end of pile push depth which is reported in feet
3. Refusal is defined when installation time is greater than 120 second per foot

TEST PILE DRIVING RECORDS

Grace Solar

Terracon Project No. 60225164



Pile Designation	Cumulative Driving Time, seconds (See Note 1)								
	BPLT-4A	BPLT-4B	BPLT-4C	BPLT-5A	BPLT-5B	BPLT-5C	BPLT-6A	BPLT-6B	BPLT-6C
Pile Push (in.) (See Note 2)	34	10	9	9	9	9	22	10	8
0 to 1	0	1	1	1	1	1	0	1	1
1 to 2	0	3	3	3	3	3	1	4	3
2 to 3	1	6	4	5	6	6	3	6	6
3 to 4	2	8	6	8	11	10	6	8	9
4 to 5	9	14	12	10	18	13	9	10	11
5 to 6	19	17	20	27	25	26	13	11	15
6 to 7	28	21	27	43	53	41	19	13	22
7 to 8	36	25	37	61	123	53	29	17	32
8 to 9		30			153			21	
9 to 10		41			188			30	
10 to 11		48			223			39	
11 to 12		62			263			48	
Install Date	5/15/25	5/15/25	5/15/25	5/15/25	5/15/25	5/15/25	5/15/25	5/15/25	5/15/25
Pile Size	W6x9	W6x9	W6x9	W6x9	W6x9	W6x9	W6x9	W6x9	W6x9
Refusal	No	No	No	No	No	No	No	No	No
Embedment Depth, ft.	8.0	12.0	8.0	8.0	12.0	8.0	8.0	12.0	8.0
Total Drive Time, sec.	36	62	37	61	263	53	29	48	32
Average, sec./ft.	4.5	5.2	4.6	7.6	21.9	6.6	3.6	4.0	4.0

NOTES:

1. Piles advanced with a Track Mounted GAYK Model HRE 4000 pile driver.
2. Pile driving started at the end of pile push depth which is reported in feet
3. Refusal is defined when installation time is greater than 120 second per foot

Pile Load Test Results

Contents:

Load Test Procedure Details
Axial Tension Test Results
Lateral Test Results
Axial Compression Test Results

Load Test Procedure Details

Testing Under Axial Compressive Load (ASTM D1143)

Axial compression load tests were performed on piles designated as "A" and "C" at each pile load test location. For compression testing, the compression load cell and hydraulic ram jack were placed on the top of the test pile. The loads were applied by jacking against a hydraulic excavator. The loads were applied in 500 lb. increments to a maximum of 30,000 lbs. or until the pile reached ¾-inch deflection. Each load increment was held for at least 1 minute. Deflections were measured with digital gauges and loads were measured with a 35-kip electronic load cell. The gauges and load cell were read, and the data was recorded manually by Terracon field personnel.

Testing Under Lateral Load (ASTM D3966)

After testing under axial compression load, the piles at each location were then tested under lateral load as described below.

As the test piles were installed in-line with each other, the piles were connected together to provide a reaction for the opposite pile and tested simultaneously in the strong axis direction. The load for the lateral tests was applied at about 1.5 feet above the ground surface against the strong axis of the piles. The loads were applied in 500-pound increments in 5 cycles from 0 pounds to a maximum load of 7,000 pounds or a deflection of 1 inch was achieved. Each load increment was held for at least 1 minute and the stabilized deflection reading of both indicator gauges was recorded. Deflections were measured with digital gauges and loads were measured with a 25-kip electronic load cell. The gauges and load cell were read, and the data was recorded manually by Terracon field personnel.

Testing Under Axial Tensile ("pull-out") Load (ASTM D3689)

One pile at each PLT location was tested under axial tensile ("pull-out") load. The test piles with the designation "C" were tested under axial tensile load and were embedded 8 feet below the ground surface.

The "pull-out" load reaction was supported using the arm of the excavator with the bucket braced on the ground at an appropriate lateral distance from the post. A hydraulic jack and pump were used to apply the test loads using chains and other accessories all rated for at least a 10-ton safe working capacity. The axial tension load was applied in load increments of 500 lbs. to a maximum of 15,000 lbs or until the pile reached ¾-inch deflection. Each load increment was held for at least 1 minute. Deflections were measured with digital gauges and loads were measured with a 25-kip electronic load cell. The gauges and load cell were read, and the data was recorded manually by Terracon field personnel.

Axial Tension Test Results

Tension Load Test Result for BPLT-1C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Dillon Ed Junior

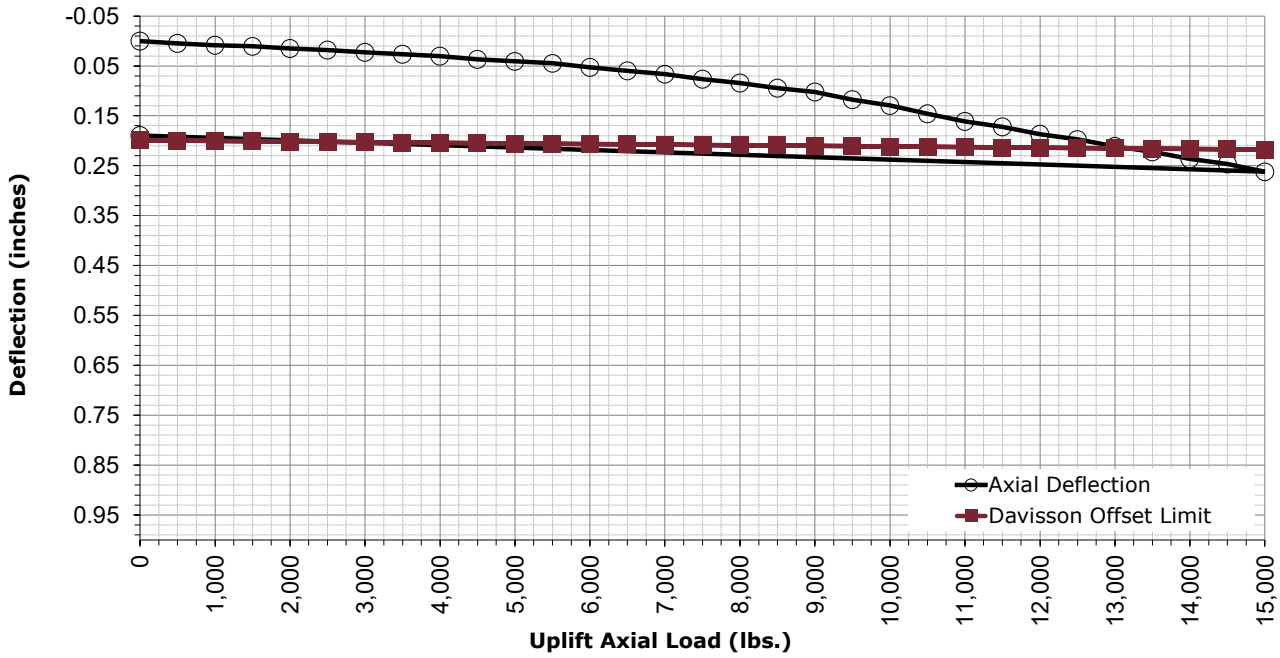
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-1C
Latitude [deg.]:	33.64923
Longitude[deg.]:	-114.72472
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	15,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	57

Tension Test Results			Davisson Offset Limit Lines		
% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offest Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.0000	0.000	0.199	
3%	500	0.0048	0.001	0.200	
7%	1,000	0.0085	0.001	0.200	
10%	1,500	0.0108	0.002	0.201	
13%	2,000	0.0153	0.002	0.202	
17%	2,500	0.0185	0.003	0.202	
20%	3,000	0.0228	0.004	0.203	
23%	3,500	0.0265	0.004	0.203	
27%	4,000	0.0303	0.005	0.204	
30%	4,500	0.0368	0.006	0.205	
33%	5,000	0.0410	0.006	0.205	
37%	5,500	0.0448	0.007	0.206	
40%	6,000	0.0525	0.007	0.207	
43%	6,500	0.0598	0.008	0.207	
47%	7,000	0.0665	0.009	0.208	
50%	7,500	0.0765	0.009	0.208	
53%	8,000	0.0843	0.010	0.209	
57%	8,500	0.0945	0.010	0.210	
60%	9,000	0.1019	0.011	0.210	
63%	9,500	0.1173	0.012	0.211	
67%	10,000	0.1295	0.012	0.212	
70%	10,500	0.1460	0.013	0.212	
73%	11,000	0.1610	0.014	0.213	
77%	11,500	0.1720	0.014	0.213	
80%	12,000	0.1870	0.015	0.214	
83%	12,500	0.1975	0.015	0.215	
87%	13,000	0.2115	0.016	0.215	
90%	13,500	0.2223	0.017	0.216	
93%	14,000	0.2365	0.017	0.216	
97%	14,500	0.2463	0.018	0.217	
100%	15,000	0.2620	0.019	0.218	
0%	0	0.1897	0.000	0.199	



Tension Load Test Result for BPLT-2C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Dillon Ed Junior

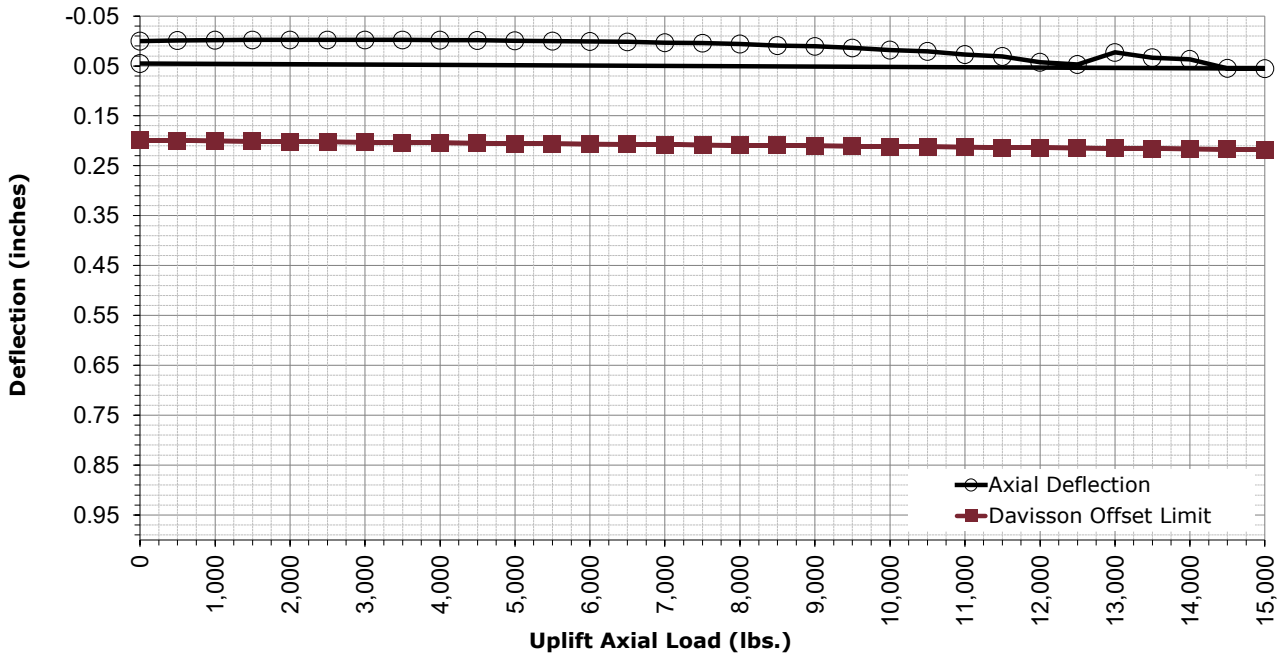
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-2C
Latitude [deg.]:	33.64920
Longitude[deg.]:	-114.72269
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	15,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	62

Tension Test Results			Davisson Offset Limit Lines		
% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offest Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.0000	0.000	0.199	
3%	500	-0.0010	0.001	0.200	
7%	1,000	-0.0018	0.001	0.200	
10%	1,500	-0.0020	0.002	0.201	
13%	2,000	-0.0020	0.002	0.202	
17%	2,500	-0.0020	0.003	0.202	
20%	3,000	-0.0020	0.004	0.203	
23%	3,500	-0.0020	0.004	0.203	
27%	4,000	-0.0015	0.005	0.204	
30%	4,500	-0.0013	0.006	0.205	
33%	5,000	-0.0005	0.006	0.205	
37%	5,500	0.0003	0.007	0.206	
40%	6,000	0.0010	0.007	0.207	
43%	6,500	0.0020	0.008	0.207	
47%	7,000	0.0035	0.009	0.208	
50%	7,500	0.0045	0.009	0.208	
53%	8,000	0.0063	0.010	0.209	
57%	8,500	0.0090	0.010	0.210	
60%	9,000	0.0110	0.011	0.210	
63%	9,500	0.0140	0.012	0.211	
67%	10,000	0.0180	0.012	0.212	
70%	10,500	0.0210	0.013	0.212	
73%	11,000	0.0270	0.014	0.213	
77%	11,500	0.0310	0.014	0.213	
80%	12,000	0.0425	0.015	0.214	
83%	12,500	0.0470	0.015	0.215	
87%	13,000	0.0230	0.016	0.215	chain link moved
90%	13,500	0.0335	0.017	0.216	
93%	14,000	0.0370	0.017	0.216	
97%	14,500	0.0548	0.018	0.217	
100%	15,000	0.0555	0.019	0.218	
0%	0	0.0450	0.000	0.199	



Tension Load Test Result for BPLT-3C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Dillon Ed Junior

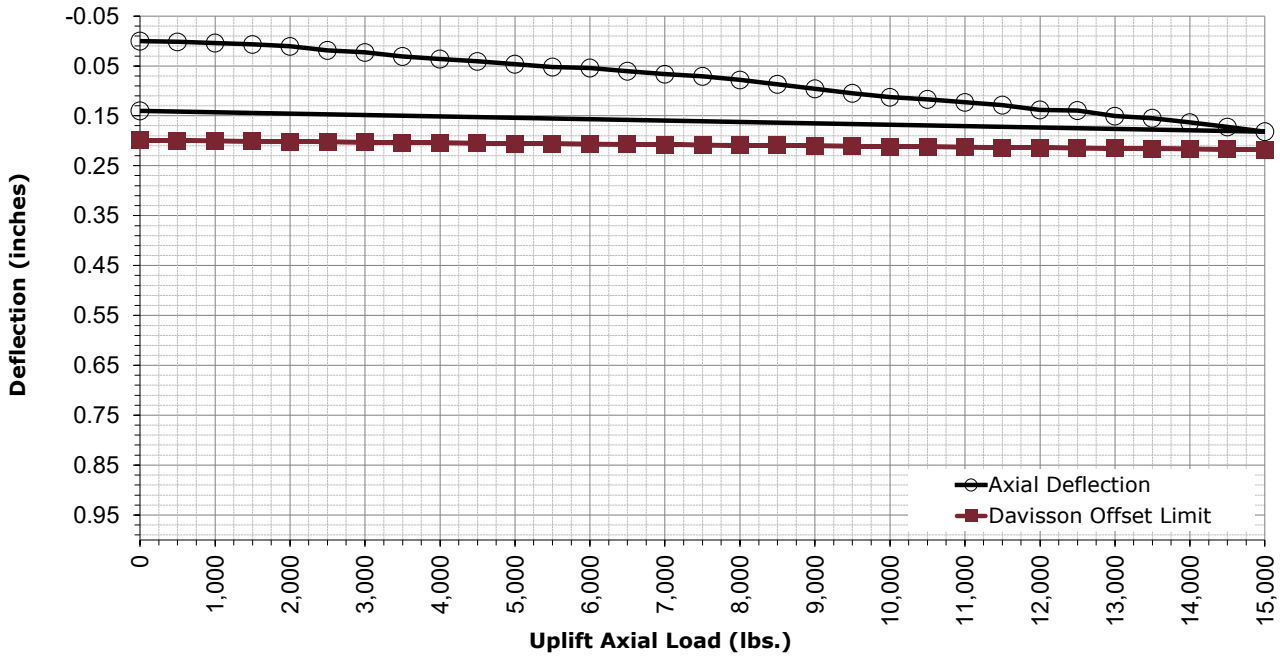
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-3C
Latitude [deg.]:	33.64827
Longitude[deg.]:	-114.72512
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	15,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	46

Tension Test Results			Davisson Offset Limit Lines		
% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offest Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.0000	0.000	0.199	
3%	500	0.0020	0.001	0.200	
7%	1,000	0.0040	0.001	0.200	
10%	1,500	0.0068	0.002	0.201	
13%	2,000	0.0108	0.002	0.202	
17%	2,500	0.0188	0.003	0.202	
20%	3,000	0.0230	0.004	0.203	
23%	3,500	0.0310	0.004	0.203	
27%	4,000	0.0363	0.005	0.204	
30%	4,500	0.0410	0.006	0.205	
33%	5,000	0.0465	0.006	0.205	
37%	5,500	0.0523	0.007	0.206	
40%	6,000	0.0540	0.007	0.207	
43%	6,500	0.0605	0.008	0.207	
47%	7,000	0.0668	0.009	0.208	
50%	7,500	0.0710	0.009	0.208	
53%	8,000	0.0780	0.010	0.209	
57%	8,500	0.0865	0.010	0.210	
60%	9,000	0.0960	0.011	0.210	
63%	9,500	0.1048	0.012	0.211	
67%	10,000	0.1128	0.012	0.212	
70%	10,500	0.1170	0.013	0.212	
73%	11,000	0.1230	0.014	0.213	
77%	11,500	0.1285	0.014	0.213	
80%	12,000	0.1380	0.015	0.214	
83%	12,500	0.1393	0.015	0.215	
87%	13,000	0.1505	0.016	0.215	
90%	13,500	0.1550	0.017	0.216	
93%	14,000	0.1635	0.017	0.216	
97%	14,500	0.1723	0.018	0.217	
100%	15,000	0.1815	0.019	0.218	
0%	0	0.1400	0.000	0.199	



Tension Load Test Result for BPLT-4C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Dillon Ed Junior

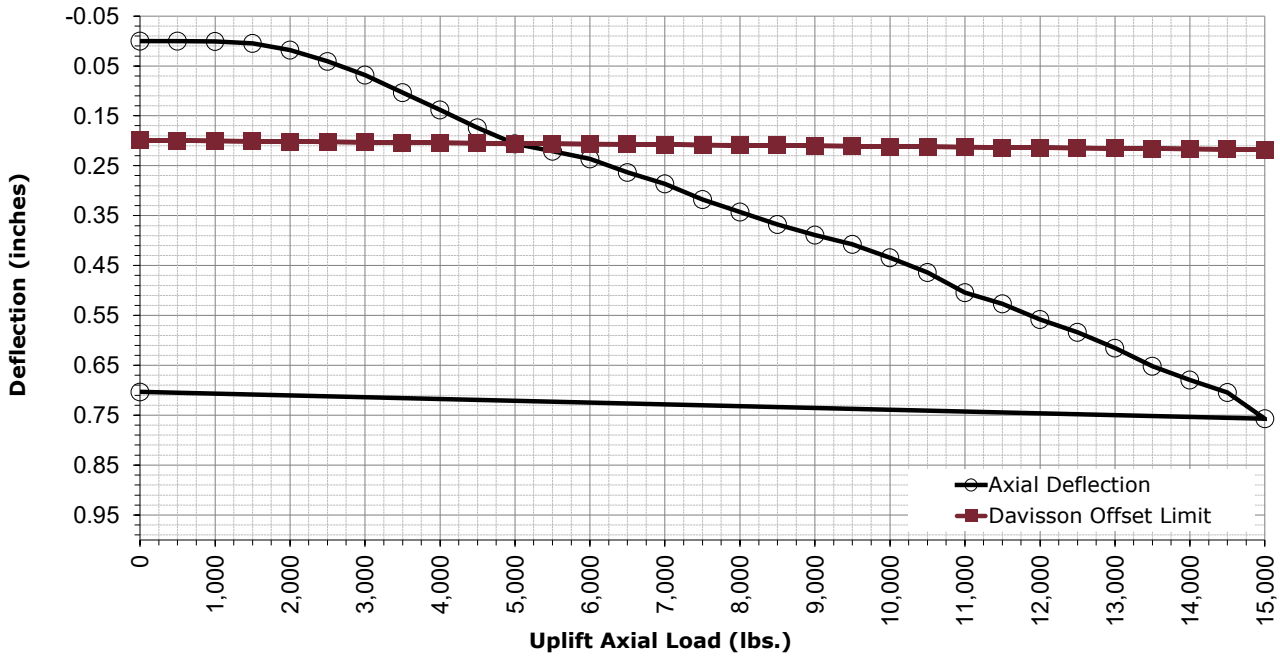
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-4C
Latitude [deg.]:	33.64825
Longitude[deg.]:	-114.72316
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	15,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	37

Tension Test Results			Davisson Offset Limit Lines		
% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offest Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.0000	0.000	0.199	
3%	500	0.0000	0.001	0.200	
7%	1,000	0.0008	0.001	0.200	
10%	1,500	0.0048	0.002	0.201	
13%	2,000	0.0180	0.002	0.202	
17%	2,500	0.0410	0.003	0.202	
20%	3,000	0.0685	0.004	0.203	
23%	3,500	0.1033	0.004	0.203	
27%	4,000	0.1380	0.005	0.204	
30%	4,500	0.1740	0.006	0.205	
33%	5,000	0.2060	0.006	0.205	
37%	5,500	0.2205	0.007	0.206	
40%	6,000	0.2365	0.007	0.207	
43%	6,500	0.2635	0.008	0.207	
47%	7,000	0.2865	0.009	0.208	
50%	7,500	0.3178	0.009	0.208	
53%	8,000	0.3430	0.010	0.209	
57%	8,500	0.3675	0.010	0.210	
60%	9,000	0.3890	0.011	0.210	
63%	9,500	0.4075	0.012	0.211	
67%	10,000	0.4345	0.012	0.212	
70%	10,500	0.4640	0.013	0.212	
73%	11,000	0.5045	0.014	0.213	
77%	11,500	0.5265	0.014	0.213	
80%	12,000	0.5580	0.015	0.214	
83%	12,500	0.5838	0.015	0.215	
87%	13,000	0.6155	0.016	0.215	
90%	13,500	0.6515	0.017	0.216	
93%	14,000	0.6795	0.017	0.216	
97%	14,500	0.7045	0.018	0.217	
100%	15,000	0.7570	0.019	0.218	
0%	0	0.7035	0.000	0.199	



Tension Load Test Result for BPLT-5C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Dillon Ed Junior

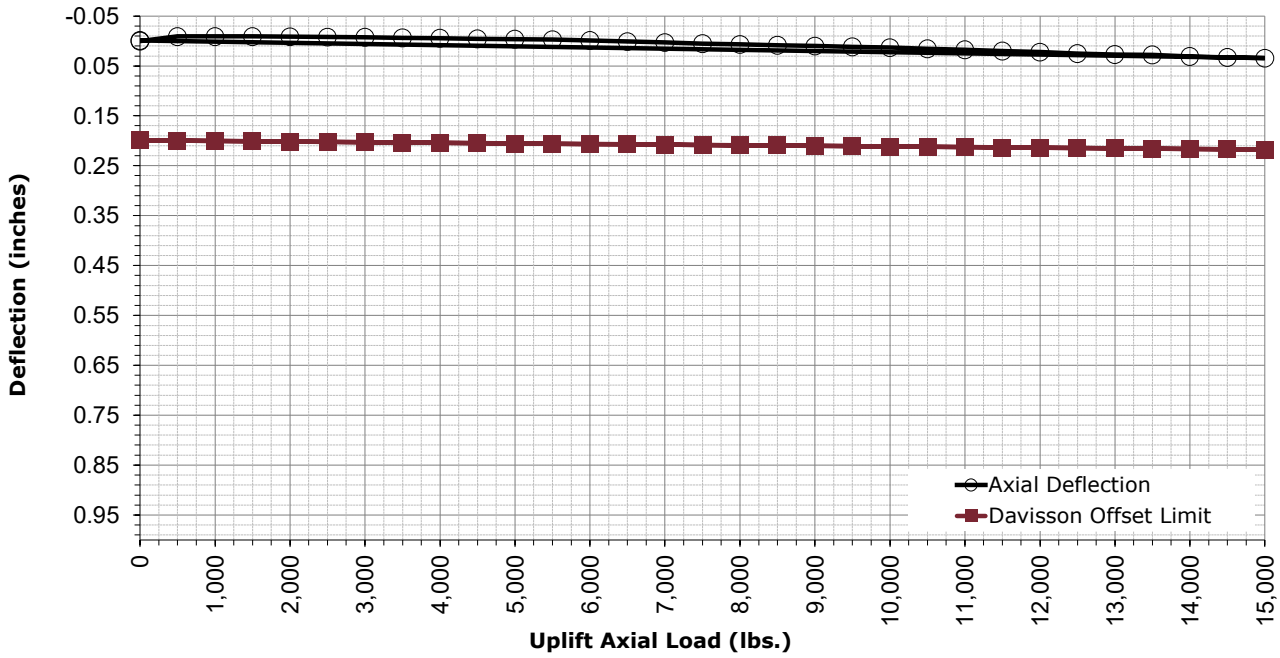
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-5C
Latitude [deg.]:	33.64748
Longitude[deg.]:	-114.72365
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	15,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	53

Tension Test Results			Davisson Offset Limit Lines		
% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offest Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.0000	0.000	0.199	
3%	500	-0.0090	0.001	0.200	
7%	1,000	-0.0090	0.001	0.200	
10%	1,500	-0.0090	0.002	0.201	
13%	2,000	-0.0085	0.002	0.202	
17%	2,500	-0.0078	0.003	0.202	
20%	3,000	-0.0073	0.004	0.203	
23%	3,500	-0.0063	0.004	0.203	
27%	4,000	-0.0058	0.005	0.204	
30%	4,500	-0.0043	0.006	0.205	
33%	5,000	-0.0035	0.006	0.205	
37%	5,500	-0.0028	0.007	0.206	
40%	6,000	-0.0010	0.007	0.207	
43%	6,500	0.0013	0.008	0.207	
47%	7,000	0.0030	0.009	0.208	
50%	7,500	0.0053	0.009	0.208	
53%	8,000	0.0069	0.010	0.209	
57%	8,500	0.0085	0.010	0.210	
60%	9,000	0.0100	0.011	0.210	
63%	9,500	0.0115	0.012	0.211	
67%	10,000	0.0133	0.012	0.212	
70%	10,500	0.0160	0.013	0.212	
73%	11,000	0.0175	0.014	0.213	
77%	11,500	0.0200	0.014	0.213	
80%	12,000	0.0223	0.015	0.214	
83%	12,500	0.0253	0.015	0.215	
87%	13,000	0.0268	0.016	0.215	
90%	13,500	0.0280	0.017	0.216	
93%	14,000	0.0310	0.017	0.216	
97%	14,500	0.0330	0.018	0.217	
100%	15,000	0.0343	0.019	0.218	
0%	0	-0.0010	0.000	0.199	



Tension Load Test Result for BPLT-6C

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in.]:	6
Load Cell:	Dillon Ed Junior

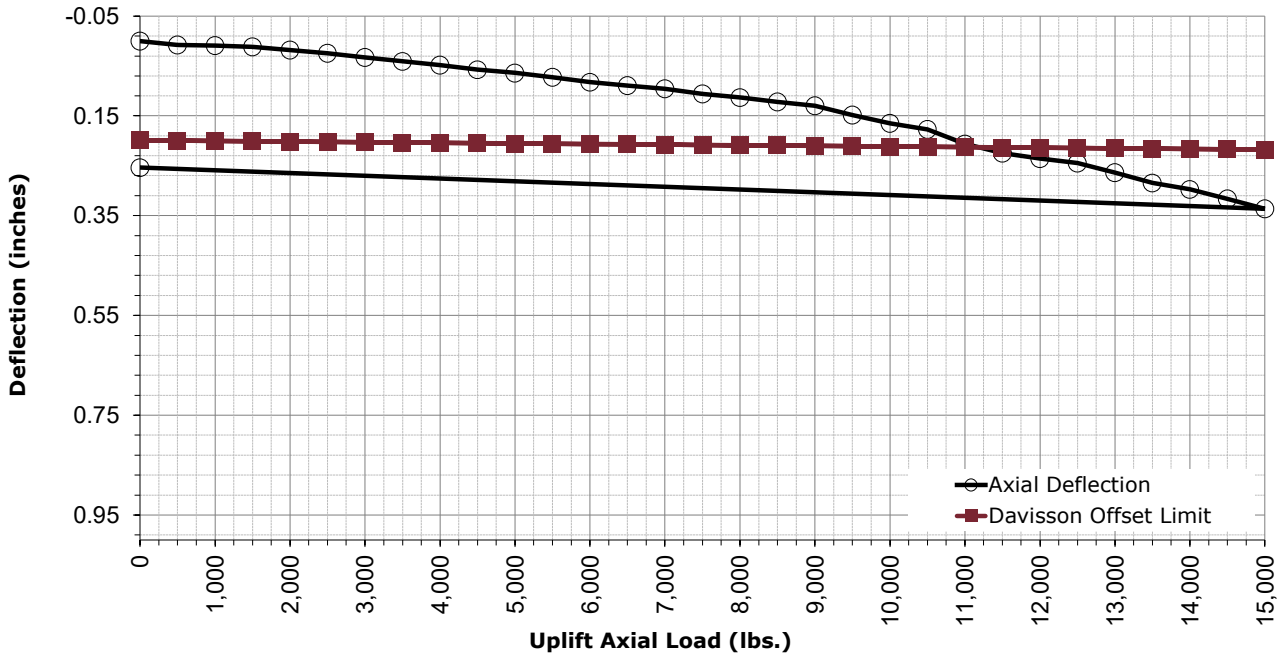
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-6C
Latitude [deg.]:	33.64711
Longitude[deg.]:	-114.72519
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	15,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	32

Tension Test Results			Davisson Offset Limit Lines		
% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Elastic Data (in.) (PL/AE)	Davisson Offest Limit (in.) (0.15+D/120+(PL/AE))	Comments
0%	0	0.0000	0.000	0.199	
3%	500	0.0080	0.001	0.200	
7%	1,000	0.0090	0.001	0.200	
10%	1,500	0.0115	0.002	0.201	
13%	2,000	0.0180	0.002	0.202	
17%	2,500	0.0245	0.003	0.202	
20%	3,000	0.0330	0.004	0.203	
23%	3,500	0.0410	0.004	0.203	
27%	4,000	0.0485	0.005	0.204	
30%	4,500	0.0575	0.006	0.205	
33%	5,000	0.0640	0.006	0.205	
37%	5,500	0.0725	0.007	0.206	
40%	6,000	0.0825	0.007	0.207	
43%	6,500	0.0895	0.008	0.207	
47%	7,000	0.0955	0.009	0.208	
50%	7,500	0.1060	0.009	0.208	
53%	8,000	0.1135	0.010	0.209	
57%	8,500	0.1220	0.010	0.210	
60%	9,000	0.1300	0.011	0.210	
63%	9,500	0.1485	0.012	0.211	
67%	10,000	0.1648	0.012	0.212	
70%	10,500	0.1770	0.013	0.212	
73%	11,000	0.2065	0.014	0.213	
77%	11,500	0.2250	0.014	0.213	
80%	12,000	0.2355	0.015	0.214	
83%	12,500	0.2450	0.015	0.215	
87%	13,000	0.2640	0.016	0.215	
90%	13,500	0.2845	0.017	0.216	
93%	14,000	0.2975	0.017	0.216	
97%	14,500	0.3165	0.018	0.217	
100%	15,000	0.3365	0.019	0.218	
0%	0	0.2540	0.000	0.199	



Lateral Test Results

Lateral Load Test Results for BPLT-1A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Lateral Load Test Set Up

Number of Top Gauges:	N/A
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	N/A
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	18
Load Cell:	Dillon Ed Junior

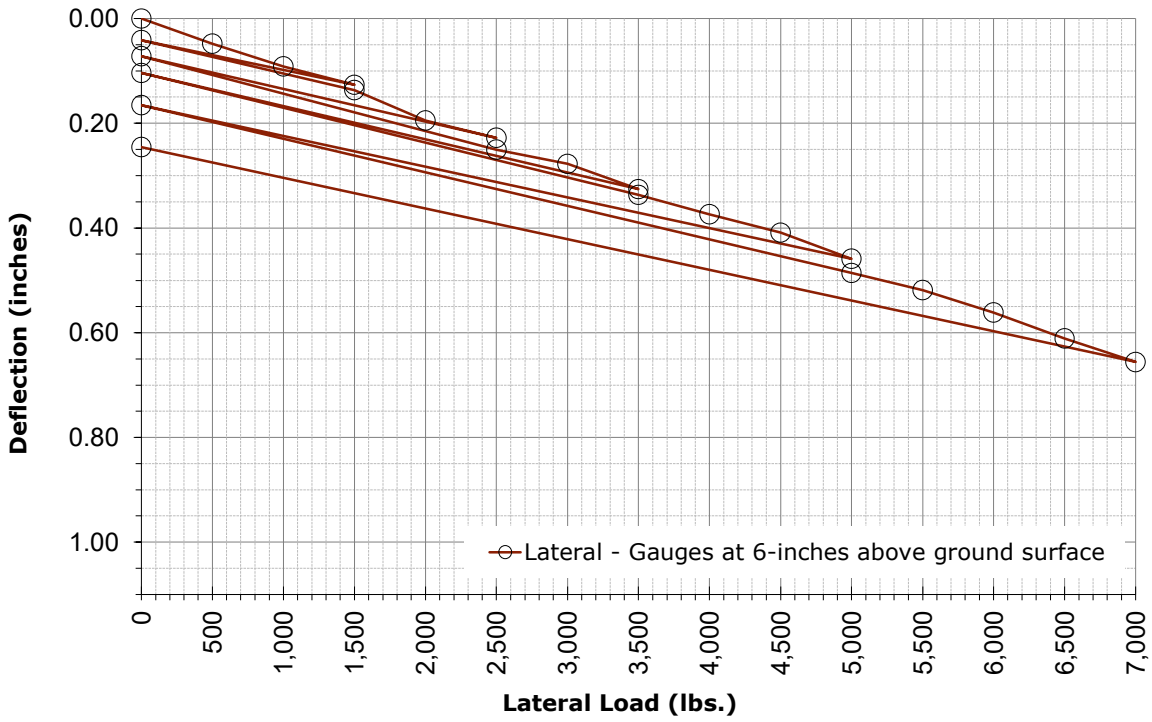
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-1A
Latitude [deg.]:	33.64923
Longitude [deg.]:	-114.72472
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Stick-Up [in.]:	24
Lateral Design Load [lbs.]:	7,000
Drive Time [sec.]:	59

% of Design Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.048	
14%	1,000	0.091	
21%	1,500	0.127	
0%	0	0.041	
21%	1,500	0.137	
29%	2,000	0.195	
36%	2,500	0.228	
0%	0	0.072	
36%	2,500	0.251	
43%	3,000	0.278	
50%	3,500	0.326	
0%	0	0.104	
50%	3,500	0.337	
57%	4,000	0.374	
64%	4,500	0.409	
71%	5,000	0.459	
0%	0	0.166	
71%	5,000	0.486	
79%	5,500	0.519	
86%	6,000	0.562	
93%	6,500	0.611	
100%	7,000	0.656	
0%	0	0.246	



Lateral Load Test Results for BPLT-1B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Lateral Load Test Set Up

Number of Top Gauges:	N/A
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	N/A
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	18
Load Cell:	Dillon Ed Junior

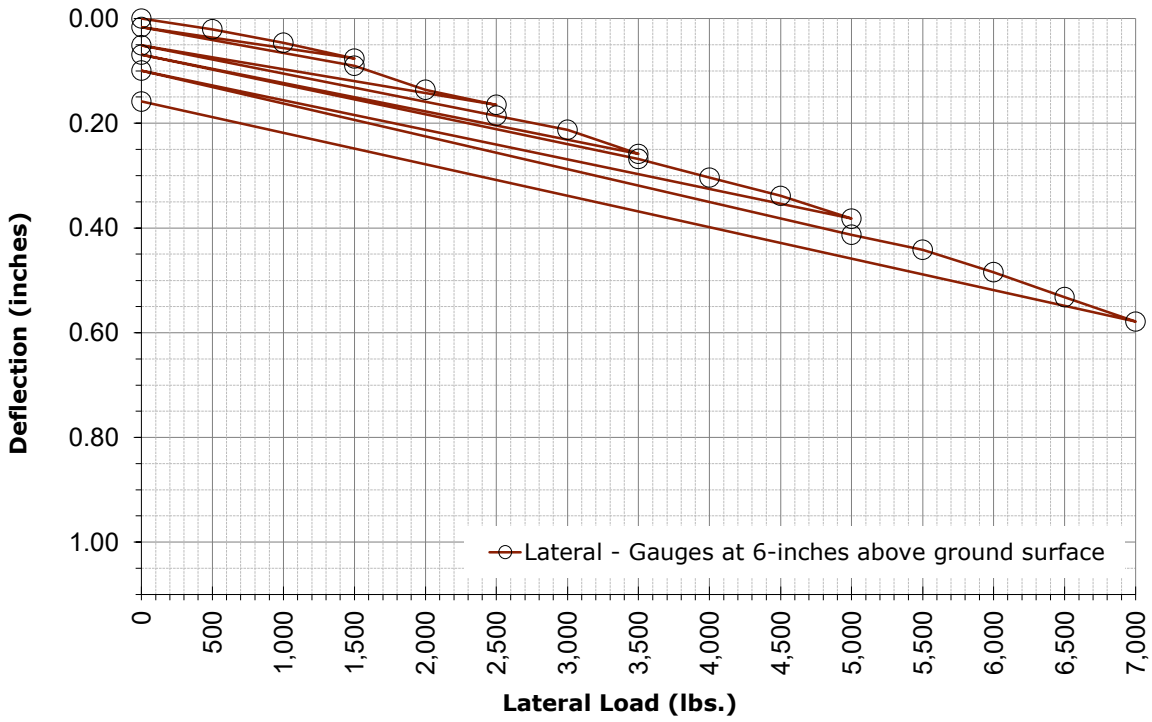
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-1B
Latitude [deg.]:	33.64923
Longitude [deg.]:	-114.72472
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Stick-Up [in.]:	24
Lateral Design Load [lbs.]:	7,000
Drive Time [sec.]:	110

% of Design Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.020	
14%	1,000	0.046	
21%	1,500	0.077	
0%	0	0.017	
21%	1,500	0.090	
29%	2,000	0.136	
36%	2,500	0.165	
0%	0	0.051	
36%	2,500	0.186	
43%	3,000	0.213	
50%	3,500	0.259	
0%	0	0.069	
50%	3,500	0.268	
57%	4,000	0.304	
64%	4,500	0.339	
71%	5,000	0.382	
0%	0	0.100	
71%	5,000	0.413	
79%	5,500	0.442	
86%	6,000	0.485	
93%	6,500	0.532	
100%	7,000	0.579	
0%	0	0.159	



Lateral Load Test Results for BPLT-2A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Lateral Load Test Set Up

Number of Top Gauges:	N/A
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	N/A
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	18
Load Cell:	Dillon Ed Junior

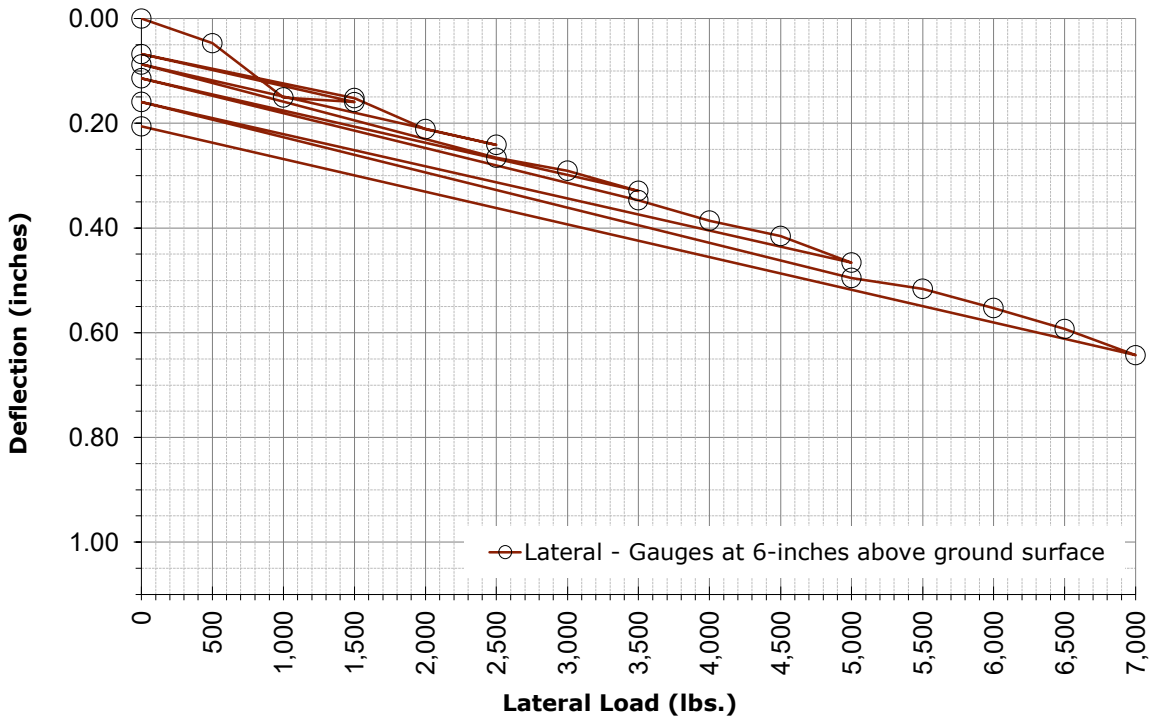
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-2A
Latitude [deg.]:	33.64920
Longitude [deg.]:	-114.72269
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Stick-Up [in.]:	24
Lateral Design Load [lbs.]:	7,000
Drive Time [sec.]:	83

% of Design Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.047	
14%	1,000	0.151	
21%	1,500	0.160	
0%	0	0.068	
21%	1,500	0.152	
29%	2,000	0.211	
36%	2,500	0.241	
0%	0	0.088	
36%	2,500	0.266	
43%	3,000	0.291	
50%	3,500	0.329	
0%	0	0.114	
50%	3,500	0.347	
57%	4,000	0.386	
64%	4,500	0.416	
71%	5,000	0.466	
0%	0	0.159	
71%	5,000	0.496	
79%	5,500	0.516	
86%	6,000	0.553	
93%	6,500	0.593	
100%	7,000	0.643	
0%	0	0.206	



Lateral Load Test Results for BPLT-2B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Lateral Load Test Set Up

Number of Top Gauges:	N/A
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	N/A
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	18
Load Cell:	Dillon Ed Junior

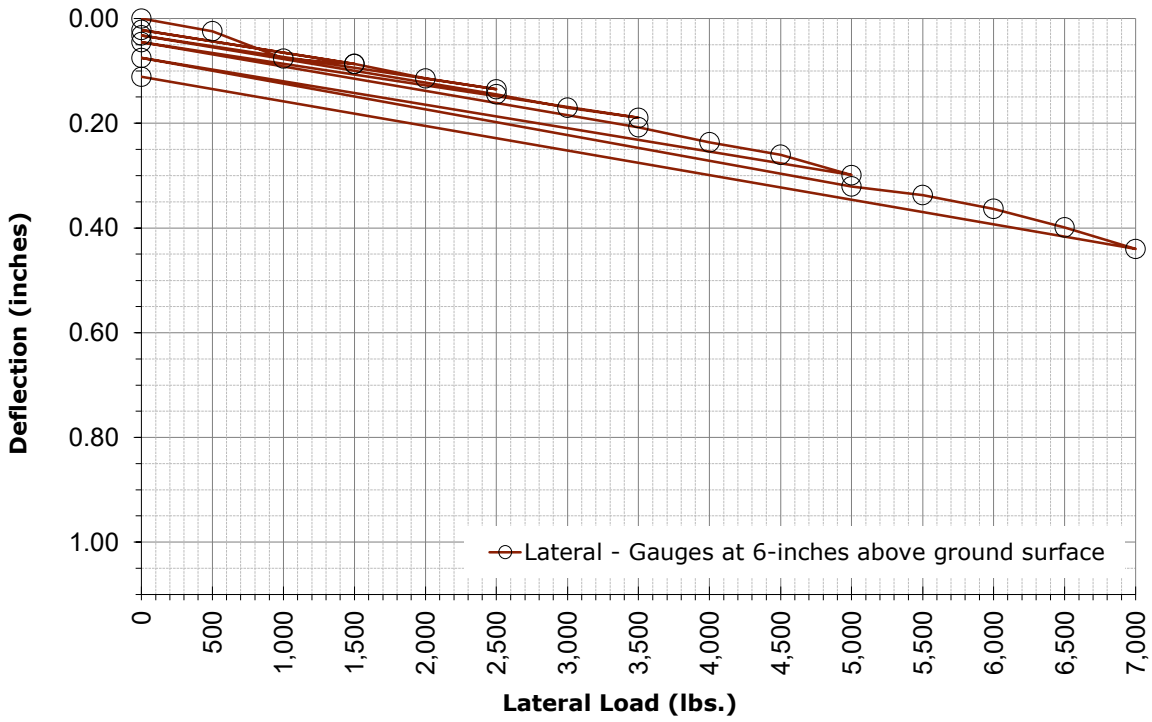
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-2B
Latitude [deg.]:	33.64920
Longitude [deg.]:	-114.72269
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Stick-Up [in.]:	24
Lateral Design Load [lbs.]:	7,000
Drive Time [sec.]:	124

% of Design Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.024	
14%	1,000	0.077	
21%	1,500	0.087	
0%	0	0.022	
21%	1,500	0.087	
29%	2,000	0.114	
36%	2,500	0.135	
0%	0	0.032	
36%	2,500	0.145	
43%	3,000	0.170	
50%	3,500	0.190	
0%	0	0.045	
50%	3,500	0.207	
57%	4,000	0.237	
64%	4,500	0.260	
71%	5,000	0.299	
0%	0	0.076	
71%	5,000	0.321	
79%	5,500	0.337	
86%	6,000	0.363	
93%	6,500	0.399	
100%	7,000	0.440	
0%	0	0.111	



Lateral Load Test Results for BPLT-3A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Lateral Load Test Set Up

Number of Top Gauges:	N/A
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	N/A
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	18
Load Cell:	Dillon Ed Junior

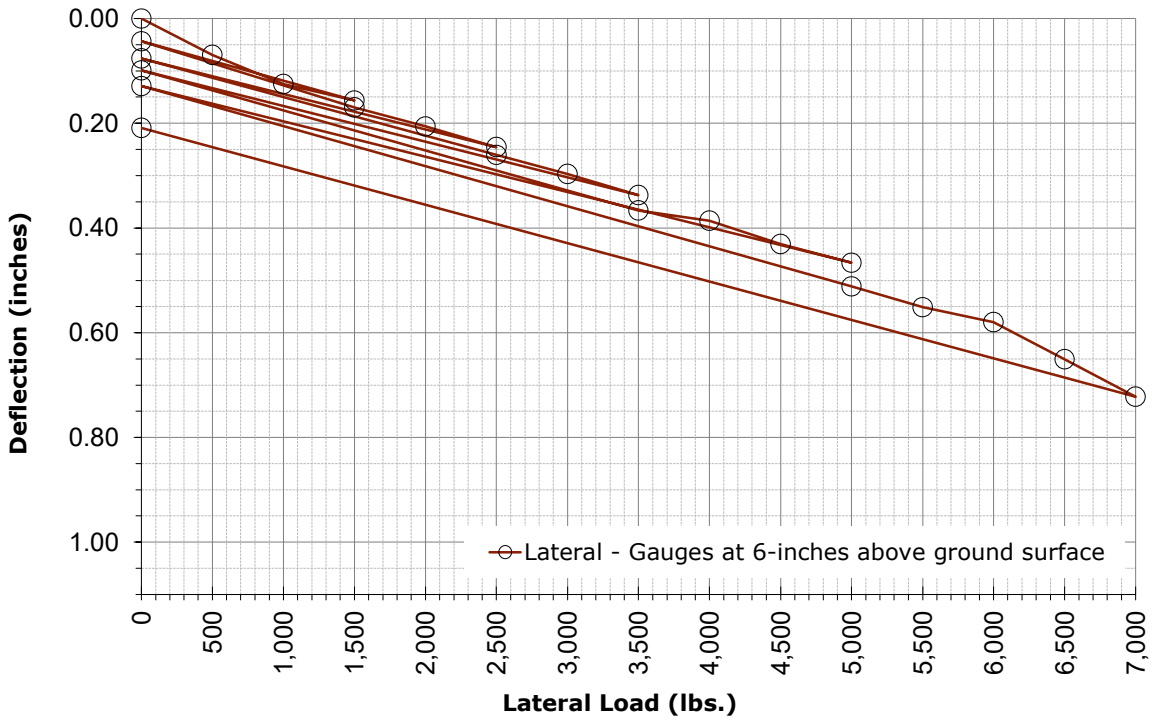
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-3A
Latitude [deg.]:	33.64827
Longitude [deg.]:	-114.72512
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Stick-Up [in.]:	24
Lateral Design Load [lbs.]:	7,000
Drive Time [sec.]:	51

% of Design Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.069	
14%	1,000	0.125	
21%	1,500	0.157	
0%	0	0.044	
21%	1,500	0.170	
29%	2,000	0.206	
36%	2,500	0.246	
0%	0	0.076	
36%	2,500	0.261	
43%	3,000	0.297	
50%	3,500	0.337	
0%	0	0.099	
50%	3,500	0.367	
57%	4,000	0.386	
64%	4,500	0.431	
71%	5,000	0.467	
0%	0	0.129	
71%	5,000	0.512	
79%	5,500	0.552	
86%	6,000	0.580	
93%	6,500	0.651	
100%	7,000	0.723	
0%	0	0.209	



Lateral Load Test Results for BPLT-3B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Lateral Load Test Set Up

Number of Top Gauges:	N/A
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	N/A
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	18
Load Cell:	Dillon Ed Junior

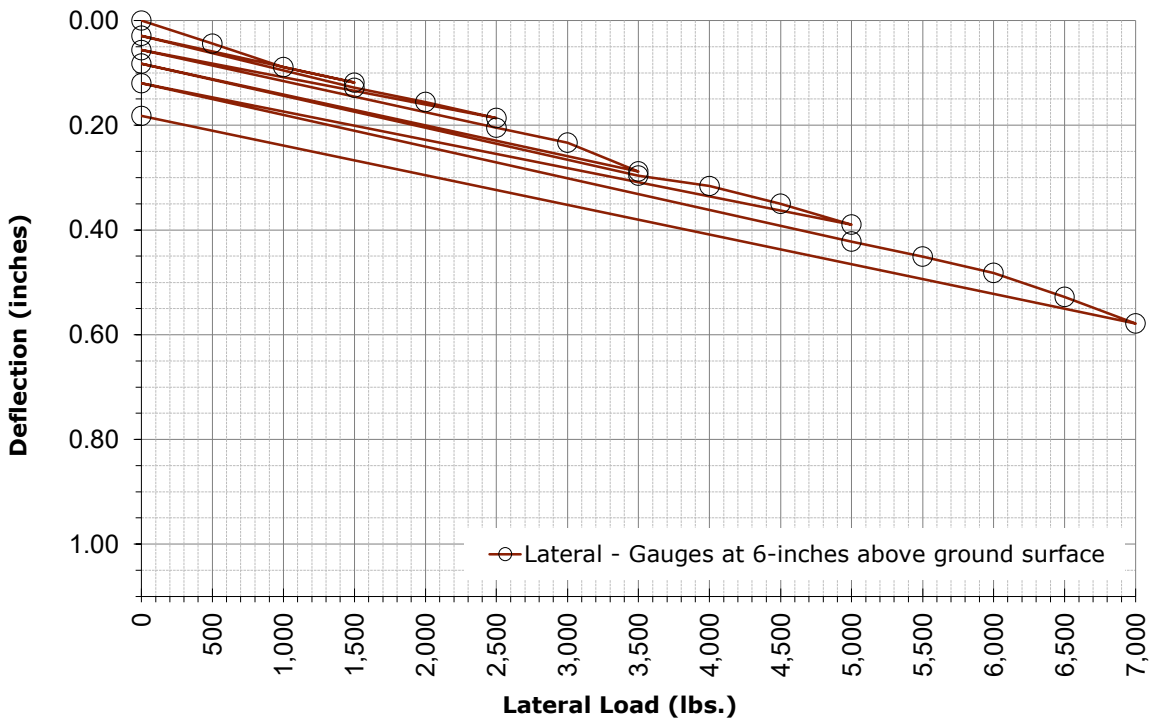
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-3B
Latitude [deg.]:	33.64827
Longitude [deg.]:	-114.72512
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Stick-Up [in.]:	24
Lateral Design Load [lbs.]:	7,000
Drive Time [sec.]:	101

% of Design Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.044	
14%	1,000	0.089	
21%	1,500	0.119	
0%	0	0.029	
21%	1,500	0.128	
29%	2,000	0.156	
36%	2,500	0.186	
0%	0	0.056	
36%	2,500	0.205	
43%	3,000	0.233	
50%	3,500	0.288	
0%	0	0.083	
50%	3,500	0.296	
57%	4,000	0.316	
64%	4,500	0.350	
71%	5,000	0.390	
0%	0	0.120	
71%	5,000	0.422	
79%	5,500	0.451	
86%	6,000	0.482	
93%	6,500	0.528	
100%	7,000	0.579	
0%	0	0.182	



Lateral Load Test Results for BPLT-4A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Lateral Load Test Set Up

Number of Top Gauges:	N/A
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	N/A
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	18
Load Cell:	Dillon Ed Junior

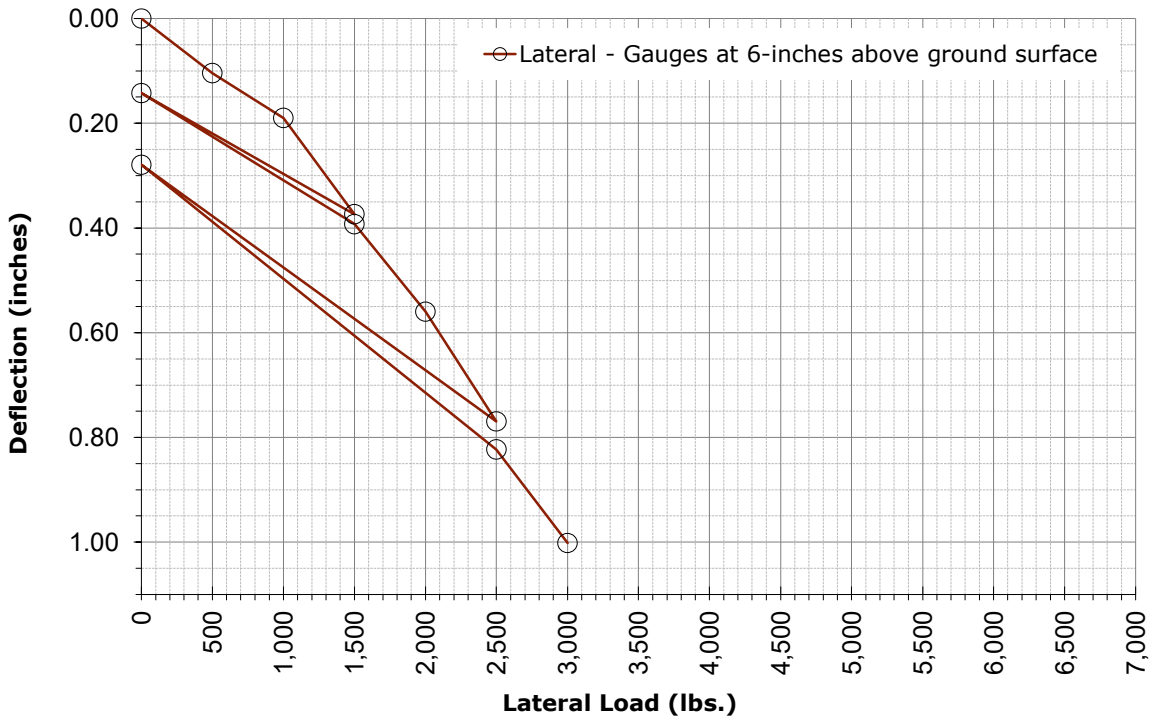
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-4A
Latitude [deg.]:	33.64825
Longitude [deg.]:	-114.72316
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Stick-Up [in.]:	24
Lateral Design Load [lbs.]:	7,000
Drive Time [sec.]:	36

% of Design Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.104	
14%	1,000	0.190	
21%	1,500	0.374	
0%	0	0.143	
21%	1,500	0.393	
29%	2,000	0.560	
36%	2,500	0.770	
0%	0	0.280	
36%	2,500	0.823	
43%	3,000	1.002	
50%	3,500		
0%	0		
50%	3,500		
57%	4,000		
64%	4,500		
71%	5,000		
0%	0		
71%	5,000		
79%	5,500		
86%	6,000		
93%	6,500		
100%	7,000		
0%	0		



Lateral Load Test Results for BPLT-4B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Lateral Load Test Set Up

Number of Top Gauges:	N/A
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	N/A
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	18
Load Cell:	Dillon Ed Junior

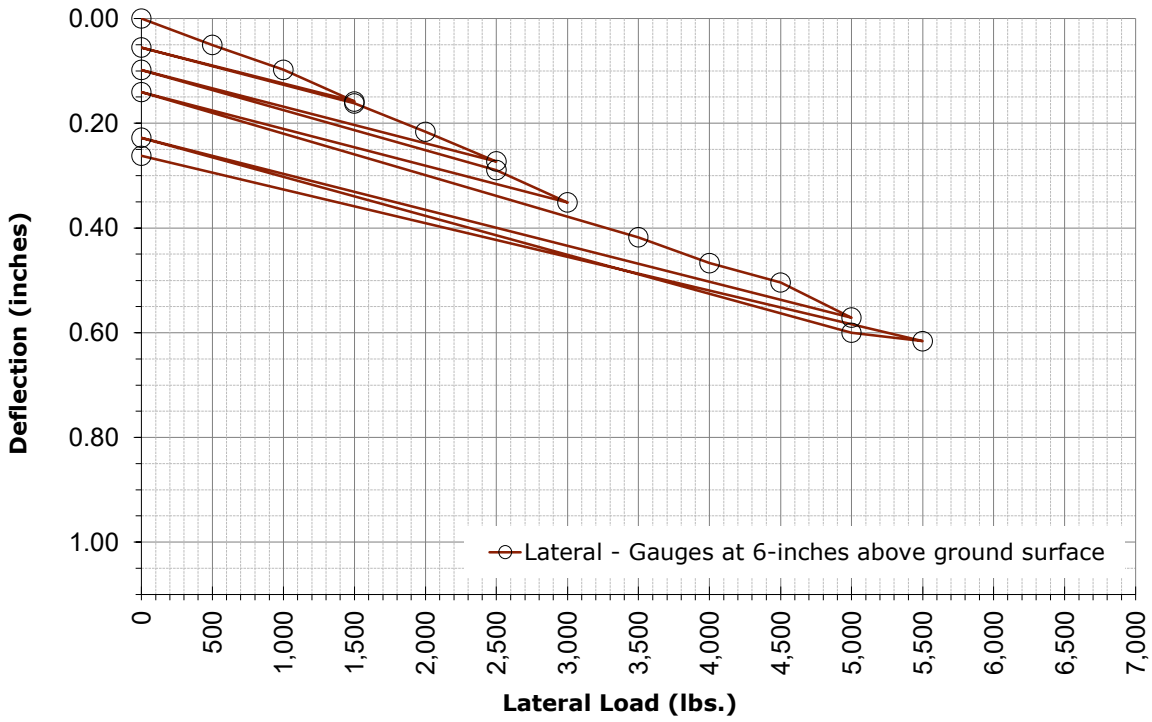
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-4B
Latitude [deg.]:	33.64825
Longitude [deg.]:	-114.72316
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Stick-Up [in.]:	24
Lateral Design Load [lbs.]:	7,000
Drive Time [sec.]:	62

% of Design Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.051	
14%	1,000	0.098	
21%	1,500	0.159	
0%	0	0.055	
21%	1,500	0.162	
29%	2,000	0.216	
36%	2,500	0.273	
0%	0	0.098	
36%	2,500	0.289	
43%	3,000	0.352	
50%	3,500		
0%	0	0.141	
50%	3,500	0.418	
57%	4,000	0.467	
64%	4,500	0.504	
71%	5,000	0.571	
0%	0	0.228	
71%	5,000	0.601	
79%	5,500	0.616	Reaction pile failed
86%	6,000		
93%	6,500		
100%	7,000		
0%	0	0.263	



Lateral Load Test Results for BPLT-5A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Lateral Load Test Set Up

Number of Top Gauges:	N/A
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	N/A
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	18
Load Cell:	Dillon Ed Junior

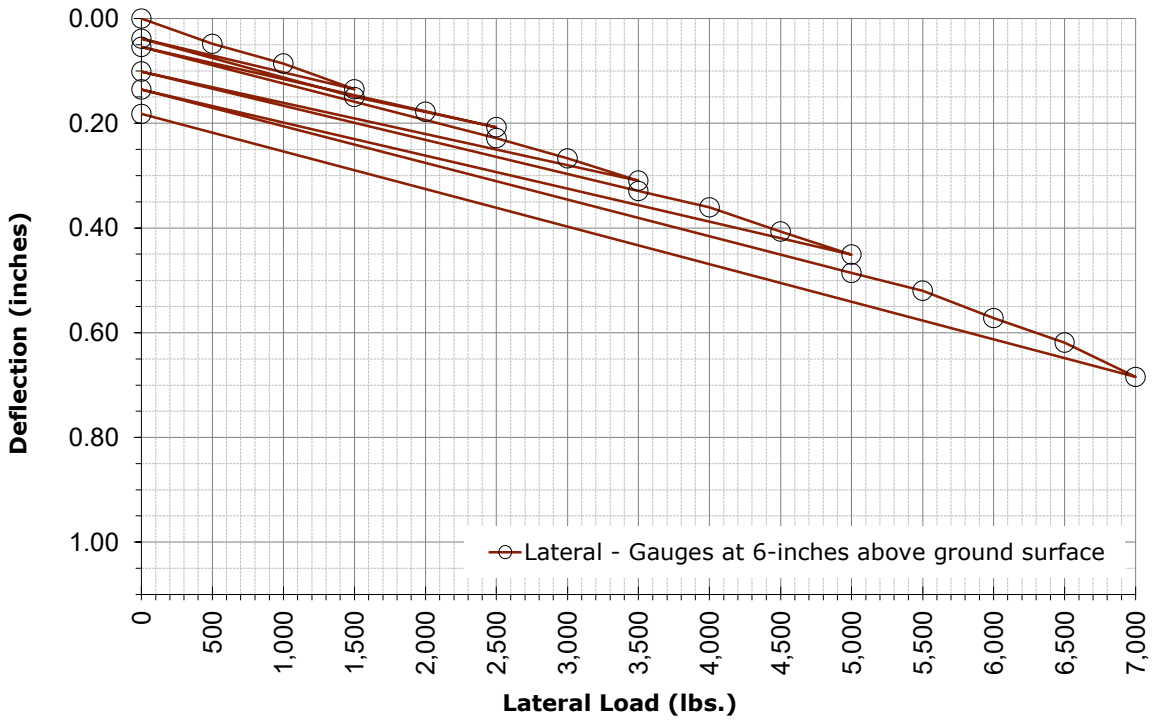
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-5A
Latitude [deg.]:	33.64748
Longitude [deg.]:	-114.72365
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Stick-Up [in.]:	24
Lateral Design Load [lbs.]:	7,000
Drive Time [sec.]:	61

% of Design Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.048	
14%	1,000	0.086	
21%	1,500	0.135	
0%	0	0.039	
21%	1,500	0.150	
29%	2,000	0.178	
36%	2,500	0.208	
0%	0	0.054	
36%	2,500	0.229	
43%	3,000	0.267	
50%	3,500	0.310	
0%	0	0.101	
50%	3,500	0.330	
57%	4,000	0.361	
64%	4,500	0.407	
71%	5,000	0.451	
0%	0	0.136	
71%	5,000	0.486	
79%	5,500	0.520	
86%	6,000	0.572	
93%	6,500	0.619	
100%	7,000	0.685	
0%	0	0.182	



Lateral Load Test Results for BPLT-5B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Lateral Load Test Set Up

Number of Top Gauges:	N/A
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	N/A
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	18
Load Cell:	Dillon Ed Junior

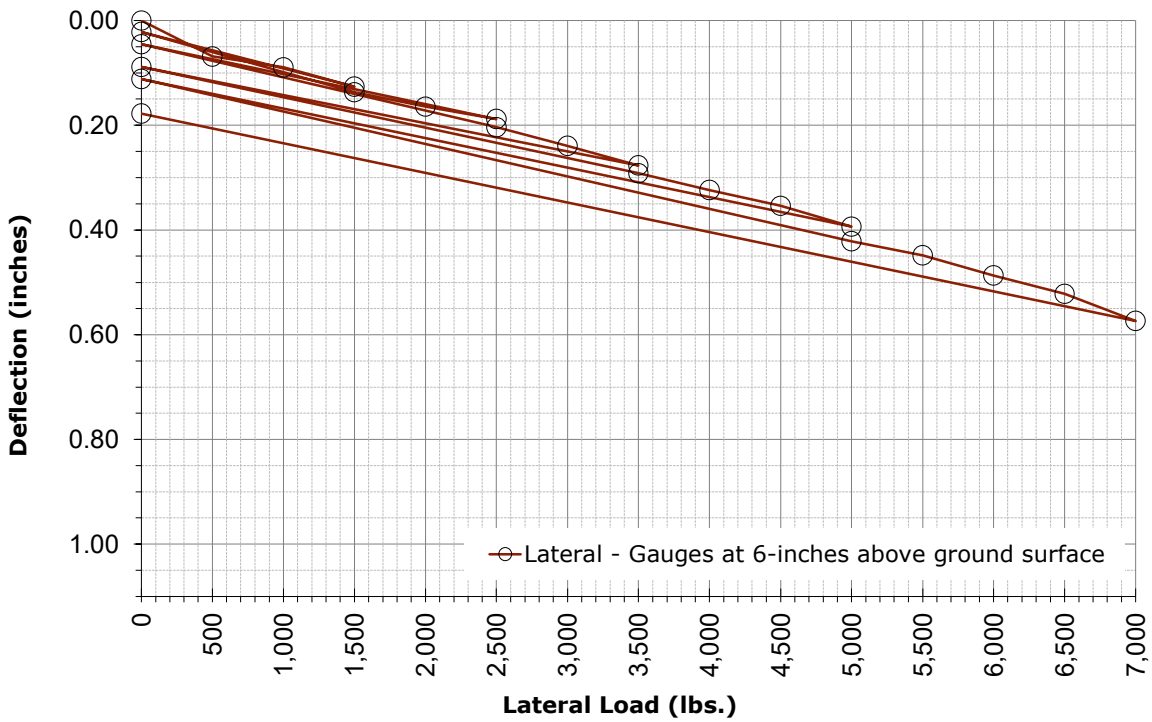
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-5B
Latitude [deg.]:	33.64748
Longitude [deg.]:	-114.72365
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Stick-Up [in.]:	24
Lateral Design Load [lbs.]:	7,000
Drive Time [sec.]:	263

% of Design Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.069	
14%	1,000	0.089	
21%	1,500	0.126	
0%	0	0.022	
21%	1,500	0.137	
29%	2,000	0.164	
36%	2,500	0.188	
0%	0	0.045	
36%	2,500	0.204	
43%	3,000	0.240	
50%	3,500	0.277	
0%	0	0.089	
50%	3,500	0.292	
57%	4,000	0.324	
64%	4,500	0.354	
71%	5,000	0.394	
0%	0	0.112	
71%	5,000	0.422	
79%	5,500	0.449	
86%	6,000	0.487	
93%	6,500	0.522	
100%	7,000	0.574	
0%	0	0.178	



Lateral Load Test Results for BPLT-6A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Lateral Load Test Set Up

Number of Top Gauges:	N/A
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	N/A
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	18
Load Cell:	Dillon Ed Junior

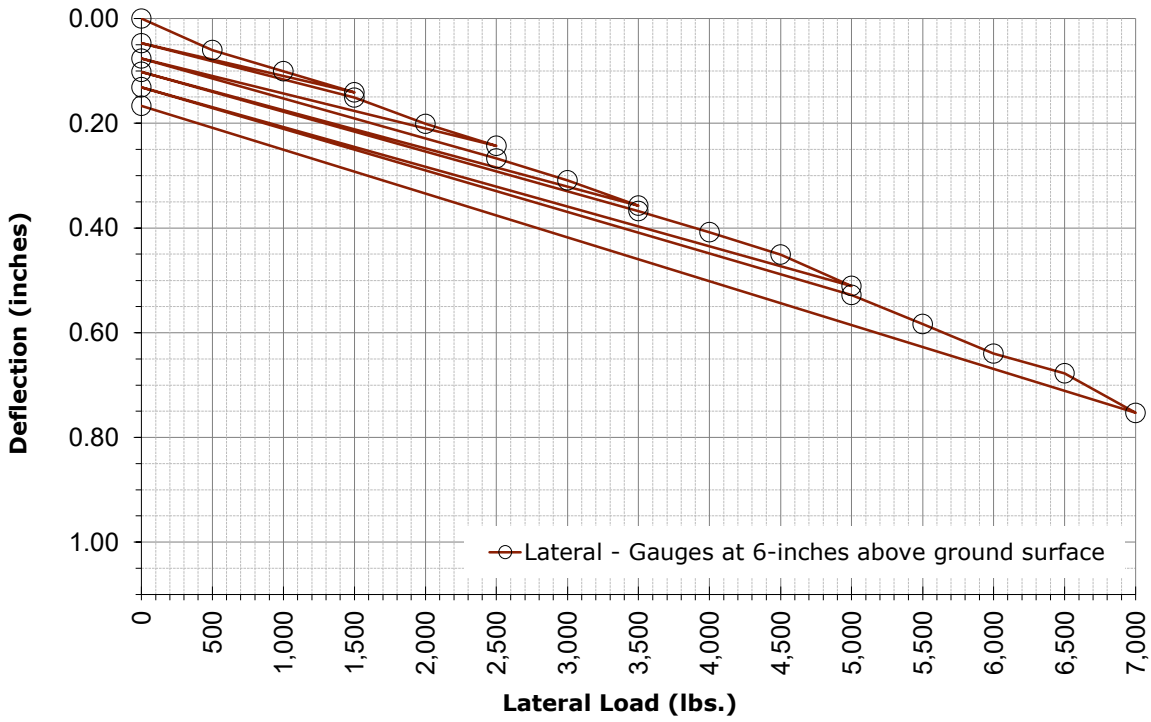
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-6A
Latitude [deg.]:	33.64711
Longitude [deg.]:	-114.72519
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Stick-Up [in.]:	24
Lateral Design Load [lbs.]:	7,000
Drive Time [sec.]:	30

% of Design Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.060	
14%	1,000	0.101	
21%	1,500	0.141	
0%	0	0.047	
21%	1,500	0.151	
29%	2,000	0.201	
36%	2,500	0.243	
0%	0	0.077	
36%	2,500	0.267	
43%	3,000	0.309	
50%	3,500	0.357	
0%	0	0.102	
50%	3,500	0.368	
57%	4,000	0.408	
64%	4,500	0.451	
71%	5,000	0.511	
0%	0	0.132	
71%	5,000	0.528	
79%	5,500	0.584	
86%	6,000	0.640	
93%	6,500	0.678	
100%	7,000	0.753	
0%	0	0.167	



Lateral Load Test Results for BPLT-6B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Lateral Load Test Set Up

Number of Top Gauges:	N/A
Number of Bottom Gauges:	2
Height of Top Gauges [in.]:	N/A
Height of Bottom Gauges [in.]:	6
Height of Applied Load [in.]:	18
Load Cell:	Dillon Ed Junior

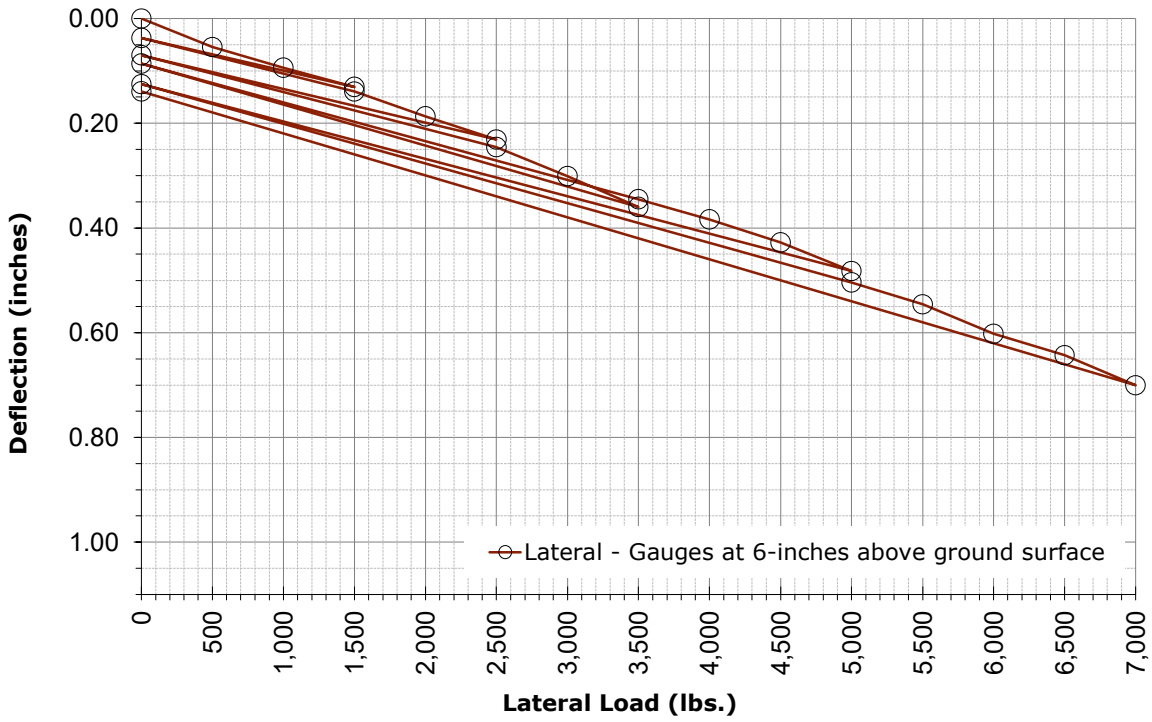
Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-6B
Latitude [deg.]:	33.64711
Longitude [deg.]:	-114.72519
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Stick-Up [in.]:	24
Lateral Design Load [lbs.]:	7,000
Drive Time [sec.]:	48

% of Design Load	Lateral Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.000	
7%	500	0.055	
14%	1,000	0.094	
21%	1,500	0.131	
0%	0	0.037	
21%	1,500	0.140	
29%	2,000	0.187	
36%	2,500	0.231	
0%	0	0.070	
36%	2,500	0.246	
43%	3,000	0.301	
50%	3,500	0.360	
0%	0	0.086	
50%	3,500	0.345	
57%	4,000	0.384	
64%	4,500	0.428	
71%	5,000	0.483	
0%	0	0.125	
71%	5,000	0.504	
79%	5,500	0.546	
86%	6,000	0.602	
93%	6,500	0.643	
100%	7,000	0.701	
0%	0	0.139	



Axial Compression Test Results

Compression Load Test Result for BPLT-1A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-1A
Latitude [deg.]:	33.64923
Longitude [deg.]:	-114.72472
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	59

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.0000	
2%	500	-0.0065	
3%	1,000	-0.0085	
5%	1,500	-0.0110	
7%	2,000	-0.0143	
8%	2,500	-0.0165	
10%	3,000	-0.0170	
12%	3,500	-0.0180	
13%	4,000	-0.0190	
15%	4,500	-0.0193	
17%	5,000	-0.0198	
18%	5,500	-0.0195	
20%	6,000	-0.0198	
22%	6,500	-0.0195	
23%	7,000	-0.0195	
25%	7,500	-0.0190	
27%	8,000	0.1103	metal plate shifted
28%	8,500	0.1120	
30%	9,000	0.1125	
32%	9,500	0.1130	
33%	10,000	0.1150	
35%	10,500	0.1160	
37%	11,000	0.1170	
38%	11,500	0.1183	
40%	12,000	0.1200	
42%	12,500	0.1210	
43%	13,000	0.1220	
45%	13,500	0.1230	
47%	14,000	0.1248	
48%	14,500	0.1258	
50%	15,000	0.1270	
52%	15,500	0.1278	
53%	16,000	0.1288	
55%	16,500	0.1303	
57%	17,000	0.1315	
58%	17,500	0.1323	
60%	18,000	0.1330	
62%	18,500	0.1338	
63%	19,000	0.1355	
65%	19,500	0.1370	
67%	20,000	0.1373	
68%	20,500	0.1385	
70%	21,000	0.1403	
72%	21,500	0.1413	
73%	22,000	0.1418	
75%	22,500	0.1425	
77%	23,000	0.1443	
78%	23,500	0.1448	
80%	24,000	0.1465	
82%	24,500	0.1470	
83%	25,000	0.1480	
85%	25,500	0.1493	
87%	26,000	0.1505	
88%	26,500	0.1518	
90%	27,000	0.1530	
92%	27,500	0.1548	
93%	28,000	0.1563	
95%	28,500	0.1575	
97%	29,000	0.1592	
98%	29,500	0.1608	
100%	30,000	0.1628	
0%	0	0.1018	

Compression Load Test Result for BPLT-1A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

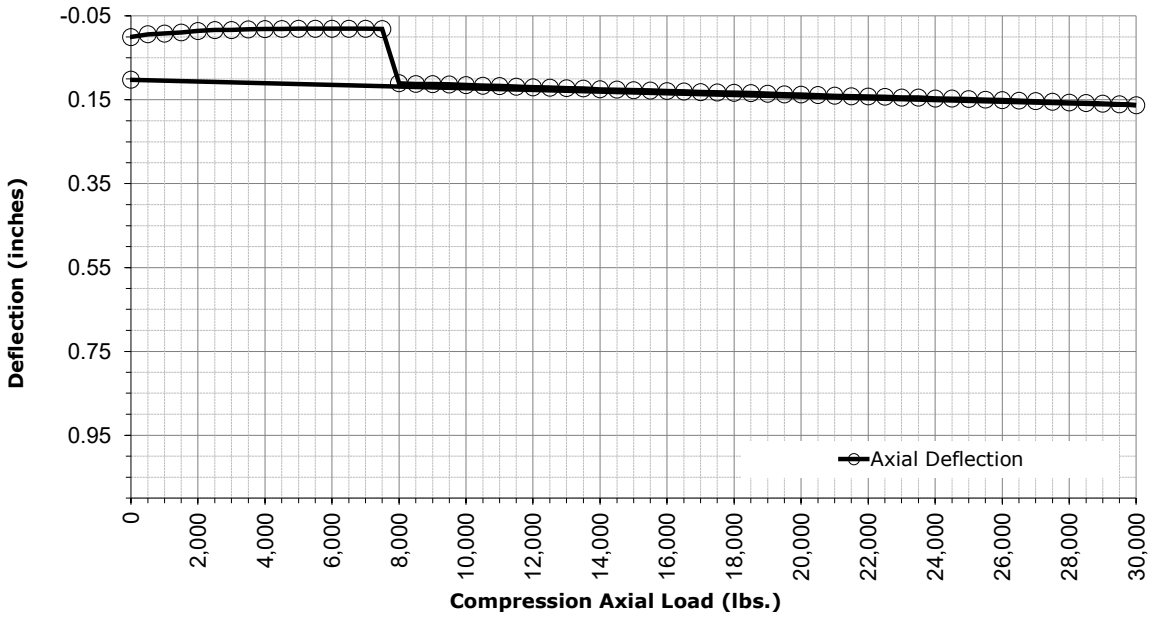
Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-1A
Latitude [deg.]:	33.64923
Longitude [deg.]:	-114.72472
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	59



Compression Load Test Result for BPLT-1B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-1B
Latitude [deg.]:	33.64923
Longitude [deg.]:	-114.72472
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	110

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.0000	
2%	500	0.0028	
3%	1,000	0.0058	
5%	1,500	0.0088	
7%	2,000	0.0103	
8%	2,500	0.0110	
10%	3,000	0.0110	
12%	3,500	0.0113	
13%	4,000	0.0108	
15%	4,500	0.0108	
17%	5,000	0.0105	
18%	5,500	0.0100	
20%	6,000	0.0095	
22%	6,500	0.0100	
23%	7,000	0.0090	
25%	7,500	0.0083	
27%	8,000	0.0080	
28%	8,500	0.0078	
30%	9,000	0.0073	
32%	9,500	0.0073	
33%	10,000	0.0068	
35%	10,500	0.0065	
37%	11,000	0.0055	
38%	11,500	0.0050	
40%	12,000	0.0050	
42%	12,500	0.0048	
43%	13,000	0.0045	
45%	13,500	0.0038	
47%	14,000	0.0035	
48%	14,500	0.0030	
50%	15,000	0.0025	
52%	15,500	0.0023	
53%	16,000	0.0015	
55%	16,500	0.0005	
57%	17,000	-0.0008	
58%	17,500	-0.0013	
60%	18,000	-0.0025	
62%	18,500	-0.0040	
63%	19,000	-0.0053	
65%	19,500	-0.0055	
67%	20,000	-0.0075	
68%	20,500	-0.0093	
70%	21,000	-0.0120	
72%	21,500	-0.0143	
73%	22,000	-0.0198	
75%	22,500	-0.0240	
77%	23,000	-0.0305	
78%	23,500	-0.0305	
80%	24,000	-0.0305	
82%	24,500	-0.0305	
83%	25,000	-0.0305	
85%	25,500	-0.0305	
87%	26,000	-0.0305	
88%	26,500	-0.0305	
90%	27,000	-0.0305	
92%	27,500	-0.0305	
93%	28,000	-0.0305	
95%	28,500	-0.0305	
97%	29,000	-0.0305	
98%	29,500	-0.0305	
100%	30,000	-0.0305	
0%	0	0.0023	

Compression Load Test Result for BPLT-1B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

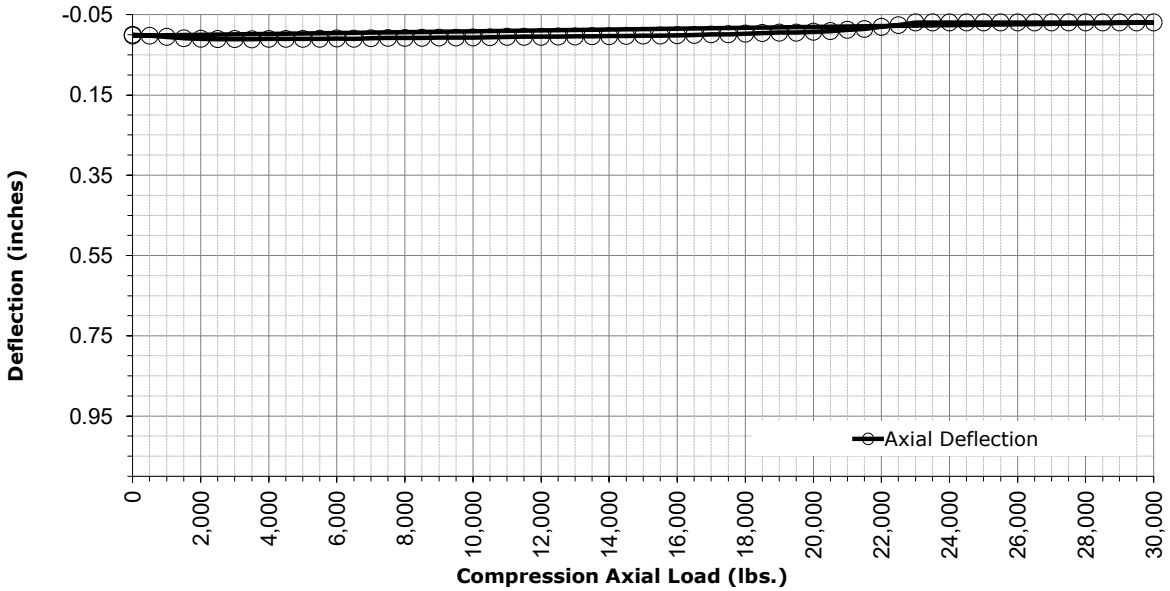
Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-1B
Latitude [deg.]:	33.64923
Longitude [deg.]:	-114.72472
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	110



Compression Load Test Result for BPLT-2A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-2A
Latitude [deg.]:	33.64920
Longitude [deg.]:	-114.72269
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	83

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.0000	
2%	500	0.0090	
3%	1,000	0.0098	
5%	1,500	0.0120	
7%	2,000	0.0173	
8%	2,500	0.0218	
10%	3,000	0.0265	
12%	3,500	0.0300	
13%	4,000	0.0345	
15%	4,500	0.0375	
17%	5,000	0.0375	
18%	5,500	0.0400	
20%	6,000	0.0430	
22%	6,500	0.0455	
23%	7,000	0.0470	
25%	7,500	0.0508	
27%	8,000	0.0520	
28%	8,500	0.0555	
30%	9,000	0.0565	
32%	9,500	0.0585	
33%	10,000	0.0615	
35%	10,500	0.0618	
37%	11,000	0.0655	
38%	11,500	0.0668	
40%	12,000	0.0690	
42%	12,500	0.0708	
43%	13,000	0.0738	
45%	13,500	0.0760	
47%	14,000	0.0770	
48%	14,500	0.0785	
50%	15,000	0.0803	
52%	15,500	0.0820	
53%	16,000	0.0838	
55%	16,500	0.0855	
57%	17,000	0.0870	
58%	17,500	0.0888	
60%	18,000	0.0913	
62%	18,500	0.0930	
63%	19,000	0.0945	
65%	19,500	0.0975	
67%	20,000	0.0990	
68%	20,500	0.1003	
70%	21,000	0.1025	
72%	21,500	0.1055	
73%	22,000	0.1073	
75%	22,500	0.1098	
77%	23,000	0.1115	
78%	23,500	0.1145	
80%	24,000	0.1173	
82%	24,500	0.1197	
83%	25,000	0.1213	
85%	25,500	0.1248	
87%	26,000	0.1270	
88%	26,500	0.1290	
90%	27,000	0.1335	
92%	27,500	0.1355	
93%	28,000	0.1370	
95%	28,500	0.1408	
97%	29,000	0.1428	
98%	29,500	0.1450	
100%	30,000	0.1470	
0%	0	0.0500	

Compression Load Test Result for BPLT-2A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

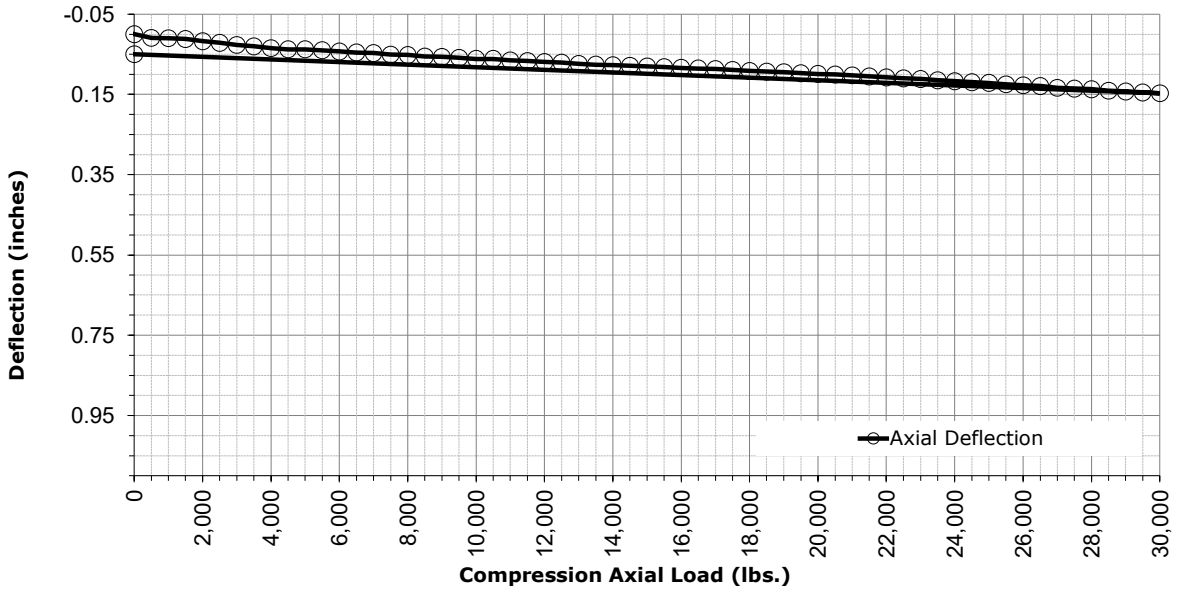
Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-2A
Latitude [deg.]:	33.64920
Longitude [deg.]:	-114.72269
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	83



Compression Load Test Result for BPLT-2B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-2B
Latitude [deg.]:	33.64920
Longitude [deg.]:	-114.72269
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	124

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.0000	
2%	500	0.0000	
3%	1,000	0.0000	
5%	1,500	0.0003	
7%	2,000	0.0003	
8%	2,500	0.0003	
10%	3,000	0.0010	
12%	3,500	0.0010	
13%	4,000	0.0010	
15%	4,500	0.0013	
17%	5,000	0.0013	
18%	5,500	0.0018	
20%	6,000	0.0020	
22%	6,500	0.0023	
23%	7,000	0.0030	
25%	7,500	0.0035	
27%	8,000	0.0043	
28%	8,500	0.0065	
30%	9,000	0.0073	
32%	9,500	0.0083	
33%	10,000	0.0093	
35%	10,500	0.0113	
37%	11,000	0.0128	
38%	11,500	0.0138	
40%	12,000	0.0158	
42%	12,500	0.0173	
43%	13,000	0.0203	
45%	13,500	0.0217	
47%	14,000	0.0238	
48%	14,500	0.0258	
50%	15,000	0.0278	
52%	15,500	0.0303	
53%	16,000	0.0325	
55%	16,500	0.0353	
57%	17,000	0.0378	
58%	17,500	0.0383	
60%	18,000	0.0418	
62%	18,500	0.0448	
63%	19,000	0.0473	
65%	19,500	0.0513	
67%	20,000	0.0525	
68%	20,500	0.0545	
70%	21,000	0.0598	
72%	21,500	0.0623	
73%	22,000	0.0653	
75%	22,500	0.0695	
77%	23,000	0.0725	
78%	23,500	0.0795	
80%	24,000	0.0823	
82%	24,500	0.0863	
83%	25,000	0.0928	
85%	25,500	0.0983	
87%	26,000	0.1048	
88%	26,500	0.1135	
90%	27,000	0.1153	
92%	27,500	0.1218	
93%	28,000	0.1287	
95%	28,500	0.1350	
97%	29,000	0.1425	
98%	29,500	0.1505	
100%	30,000	0.1598	
0%	0	0.0928	

Compression Load Test Result for BPLT-2B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

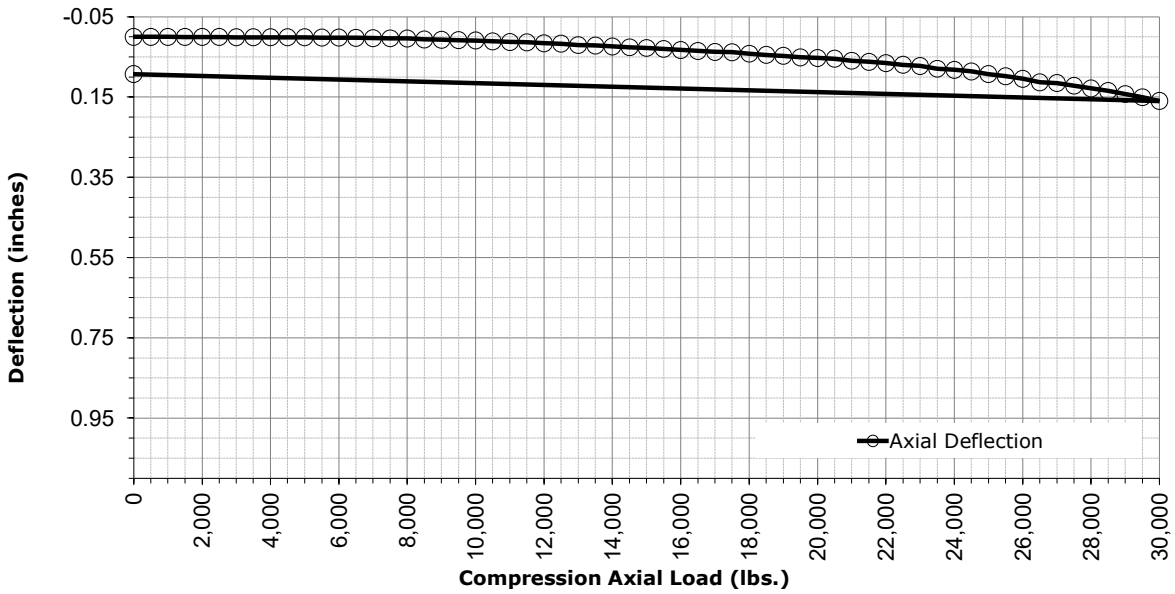
Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-2B
Latitude [deg.]:	33.64920
Longitude [deg.]:	-114.72269
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	124



Compression Load Test Result for BPLT-3A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-3A
Latitude [deg.]:	33.64827
Longitude [deg.]:	-114.72512
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	51

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.0000	
2%	500	0.0020	
3%	1,000	0.0018	
5%	1,500	0.0015	
7%	2,000	0.0010	
8%	2,500	0.0013	
10%	3,000	0.0008	
12%	3,500	0.0013	
13%	4,000	0.0015	
15%	4,500	0.0020	
17%	5,000	0.0023	
18%	5,500	0.0023	
20%	6,000	0.0028	
22%	6,500	0.0028	
23%	7,000	0.0030	
25%	7,500	0.0033	
27%	8,000	0.0033	
28%	8,500	0.0035	
30%	9,000	0.0035	
32%	9,500	0.0035	
33%	10,000	0.0038	
35%	10,500	0.0043	
37%	11,000	0.0045	
38%	11,500	0.0055	
40%	12,000	0.0060	
42%	12,500	0.0070	
43%	13,000	0.0093	
45%	13,500	0.0105	
47%	14,000	0.0125	
48%	14,500	0.0140	
50%	15,000	0.0160	
52%	15,500	0.0170	
53%	16,000	0.0195	
55%	16,500	0.0220	
57%	17,000	0.0250	
58%	17,500	0.0273	
60%	18,000	0.0313	
62%	18,500	0.0340	
63%	19,000	0.0380	
65%	19,500	0.0415	
67%	20,000	0.0460	
68%	20,500	0.0490	
70%	21,000	0.0520	
72%	21,500	0.0575	
73%	22,000	0.0645	
75%	22,500	0.0690	
77%	23,000	0.0750	
78%	23,500	0.0815	
80%	24,000	0.0900	
82%	24,500	0.0985	
83%	25,000	0.1090	
85%	25,500	0.1188	
87%	26,000	0.1295	
88%	26,500	0.1415	
90%	27,000	0.1545	
92%	27,500	0.1730	
93%	28,000	0.1910	
95%	28,500	0.2195	
97%	29,000	0.2275	
98%	29,500	0.2695	
100%	30,000	0.2990	
0%	0		

Compression Load Test Result for BPLT-3A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

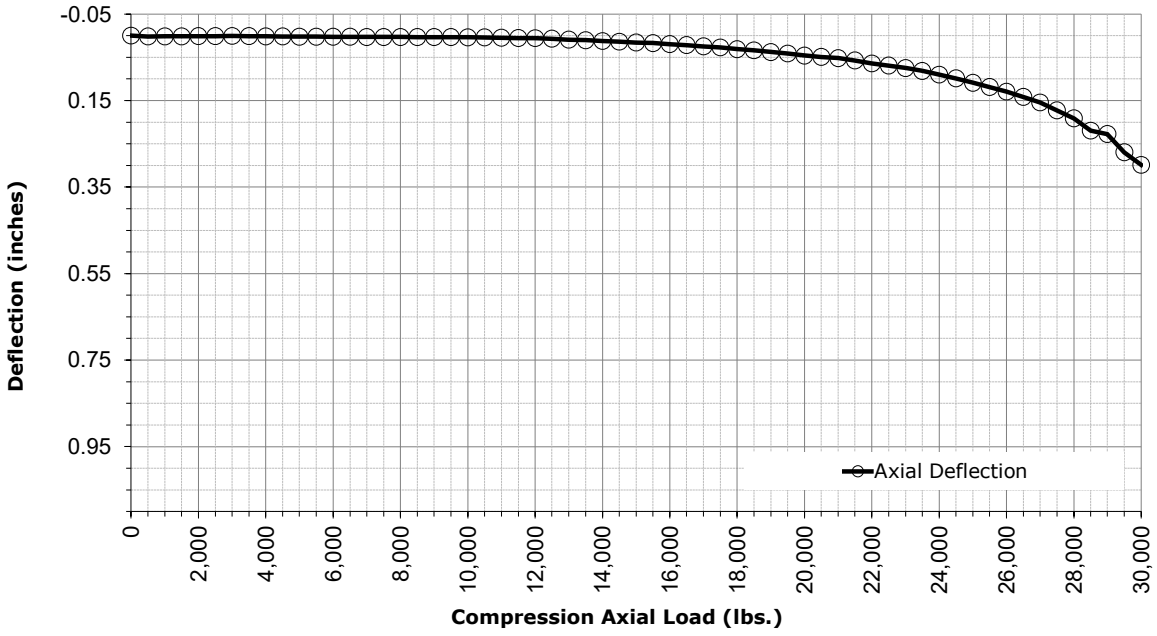
Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-3A
Latitude [deg.]:	33.64827
Longitude [deg.]:	-114.72512
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	51



Compression Load Test Result for BPLT-3B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-3B
Latitude [deg.]:	33.64827
Longitude [deg.]:	-114.72512
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	101

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.0000	
2%	500	0.0008	
3%	1,000	0.0013	
5%	1,500	0.0023	
7%	2,000	0.0038	
8%	2,500	0.0045	
10%	3,000	0.0055	
12%	3,500	0.0065	
13%	4,000	0.0080	
15%	4,500	0.0090	
17%	5,000	0.0100	
18%	5,500	0.0113	
20%	6,000	0.0119	
22%	6,500	0.0128	
23%	7,000	0.0135	
25%	7,500	0.0148	
27%	8,000	0.0160	
28%	8,500	0.0165	
30%	9,000	0.0173	
32%	9,500	0.0180	
33%	10,000	0.0188	
35%	10,500	0.0188	
37%	11,000	0.0200	
38%	11,500	0.0205	
40%	12,000	0.0213	
42%	12,500	0.0218	
43%	13,000	0.0225	
45%	13,500	0.0238	
47%	14,000	0.0243	
48%	14,500	0.0248	
50%	15,000	0.0263	
52%	15,500	0.0268	
53%	16,000	0.0278	
55%	16,500	0.0288	
57%	17,000	0.0295	
58%	17,500	0.0310	
60%	18,000	0.0320	
62%	18,500	0.0333	
63%	19,000	0.0330	
65%	19,500	0.0340	
67%	20,000	0.0353	
68%	20,500	0.0360	
70%	21,000	0.0365	
72%	21,500	0.0385	
73%	22,000	0.0393	
75%	22,500	0.0403	
77%	23,000	0.0418	
78%	23,500	0.0425	
80%	24,000	0.0433	
82%	24,500	0.0443	
83%	25,000	0.0463	
85%	25,500	0.0473	
87%	26,000	0.0490	
88%	26,500	0.0500	
90%	27,000	0.0523	
92%	27,500	0.0535	
93%	28,000	0.0560	
95%	28,500	0.0578	
97%	29,000	0.0598	
98%	29,500	0.0618	
100%	30,000	0.0645	
0%	0	0.0170	

Compression Load Test Result for BPLT-3B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

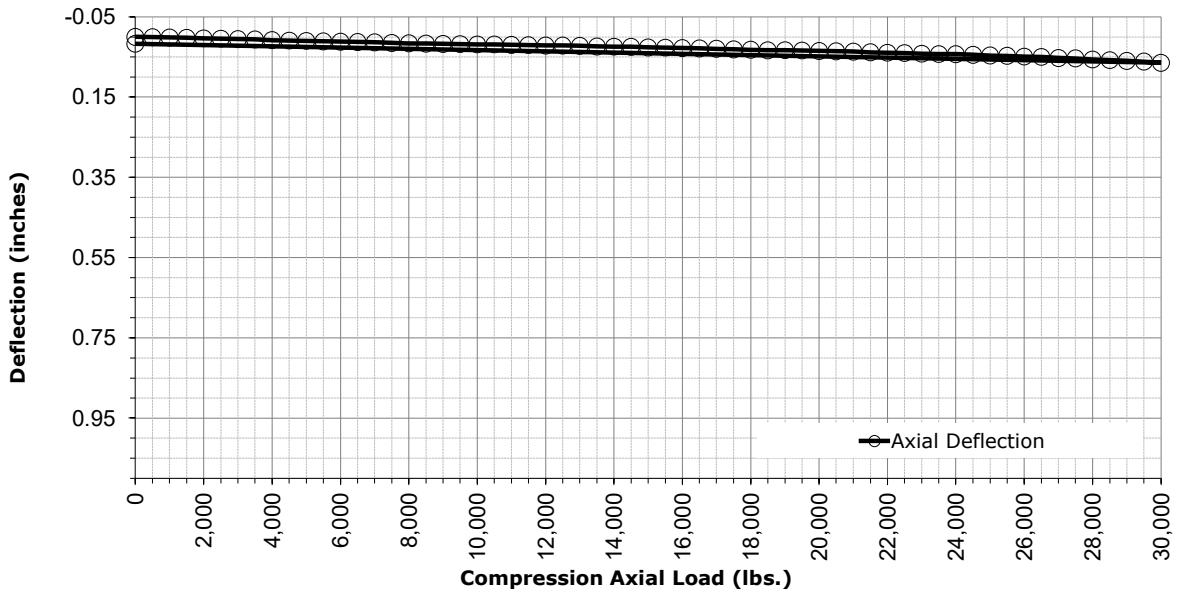
Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-3B
Latitude [deg.]:	33.64827
Longitude [deg.]:	-114.72512
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	101



Compression Load Test Result for BPLT-4A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-4A
Latitude [deg.]:	33.64825
Longitude [deg.]:	-114.72316
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	36

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.0000	
2%	500	0.0235	
3%	1,000	0.0255	
5%	1,500	0.0335	
7%	2,000	0.0400	
8%	2,500	0.0480	
10%	3,000	0.0550	
12%	3,500	0.0605	
13%	4,000	0.0655	
15%	4,500	0.0693	
17%	5,000	0.0775	
18%	5,500	0.0795	
20%	6,000	0.0840	
22%	6,500	0.0890	
23%	7,000	0.0943	
25%	7,500	0.0995	
27%	8,000	0.1058	
28%	8,500	0.1095	
30%	9,000	0.1135	
32%	9,500	0.1200	
33%	10,000	0.1263	
35%	10,500	0.1320	
37%	11,000	0.1385	
38%	11,500	0.1465	
40%	12,000	0.1520	
42%	12,500	0.1600	
43%	13,000	0.1700	
45%	13,500	0.1770	
47%	14,000	0.1873	
48%	14,500	0.1950	
50%	15,000	0.2070	
52%	15,500	0.2165	
53%	16,000	0.2285	
55%	16,500	0.2445	
57%	17,000	0.2525	
58%	17,500	0.2645	
60%	18,000	0.2790	
62%	18,500	0.2900	
63%	19,000	0.3240	
65%	19,500	0.3430	
67%	20,000	0.3650	
68%	20,500	0.3860	
70%	21,000	0.4085	
72%	21,500	0.4465	
73%	22,000	0.4780	
75%	22,500	0.4995	
77%	23,000	0.5090	
78%	23,500	0.5310	
80%	24,000	0.5525	
82%	24,500	0.5780	
83%	25,000	0.5945	
85%	25,500	0.6148	
87%	26,000	0.7685	
88%	26,500	1.0118	
90%	27,000		
92%	27,500		
93%	28,000		
95%	28,500		
97%	29,000		
98%	29,500		
100%	30,000		
0%	0	0.9395	

Compression Load Test Result for BPLT-4A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

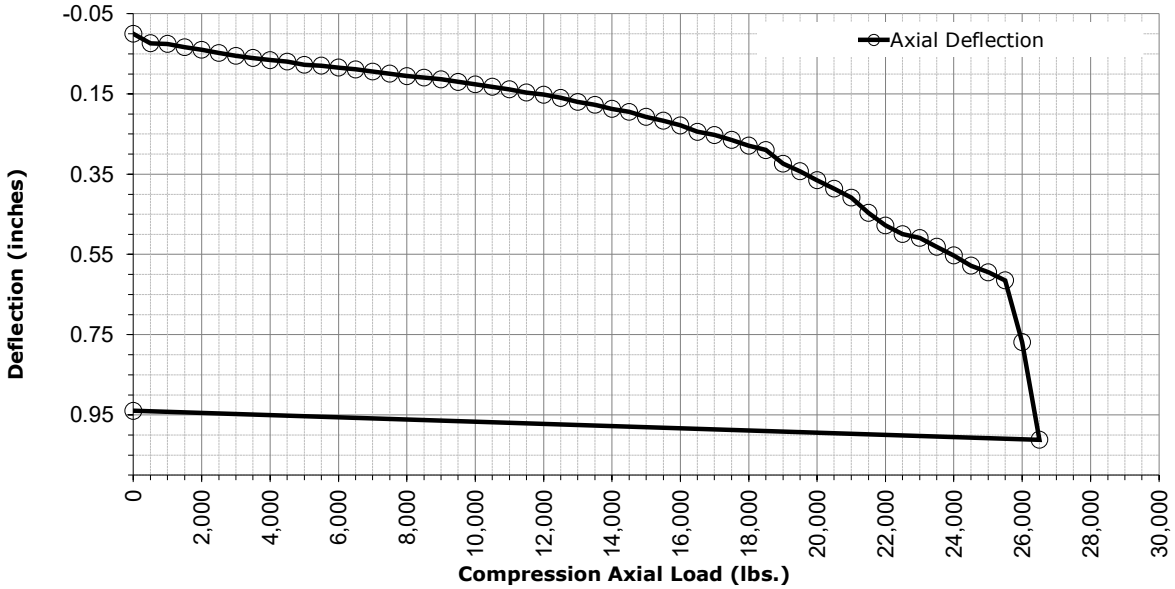
Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-4A
Latitude [deg.]:	33.64825
Longitude [deg.]:	-114.72316
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	36



Compression Load Test Result for BPLT-4B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-4B
Latitude [deg.]:	33.64825
Longitude [deg.]:	-114.72316
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	62

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.0000	
2%	500	-0.0015	
3%	1,000	-0.0013	
5%	1,500	-0.0015	
7%	2,000	-0.0020	
8%	2,500	-0.0020	
10%	3,000	-0.0010	
12%	3,500	0.0003	
13%	4,000	0.0015	
15%	4,500	0.0025	
17%	5,000	0.0038	
18%	5,500	0.0043	
20%	6,000	0.0045	
22%	6,500	0.0043	
23%	7,000	0.0045	
25%	7,500	0.0040	
27%	8,000	0.0035	
28%	8,500	0.0043	
30%	9,000	0.0043	
32%	9,500	0.0043	
33%	10,000	0.0045	
35%	10,500	0.0045	
37%	11,000	0.0045	
38%	11,500	0.0040	
40%	12,000	0.0040	
42%	12,500	0.0035	
43%	13,000	0.0023	
45%	13,500	0.0020	
47%	14,000	0.0013	
48%	14,500	0.0005	
50%	15,000	-0.0003	
52%	15,500	-0.0018	
53%	16,000	-0.0035	
55%	16,500	-0.0050	
57%	17,000	-0.0070	
58%	17,500	-0.0095	
60%	18,000	-0.0110	
62%	18,500	-0.0130	
63%	19,000	-0.0180	
65%	19,500	-0.0203	
67%	20,000	-0.0200	
68%	20,500	-0.0220	
70%	21,000	-0.0240	
72%	21,500	-0.0235	
73%	22,000	-0.0240	
75%	22,500	-0.0248	
77%	23,000	-0.0250	
78%	23,500	-0.0255	
80%	24,000	-0.0260	
82%	24,500	-0.0265	
83%	25,000	-0.0260	
85%	25,500	-0.0251	
87%	26,000	-0.0255	
88%	26,500	-0.0250	
90%	27,000	-0.0250	
92%	27,500	-0.0248	
93%	28,000	-0.0245	
95%	28,500	-0.0240	
97%	29,000	-0.0235	
98%	29,500	-0.0235	
100%	30,000	-0.0235	
0%	0	0.0238	

Compression Load Test Result for BPLT-4B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

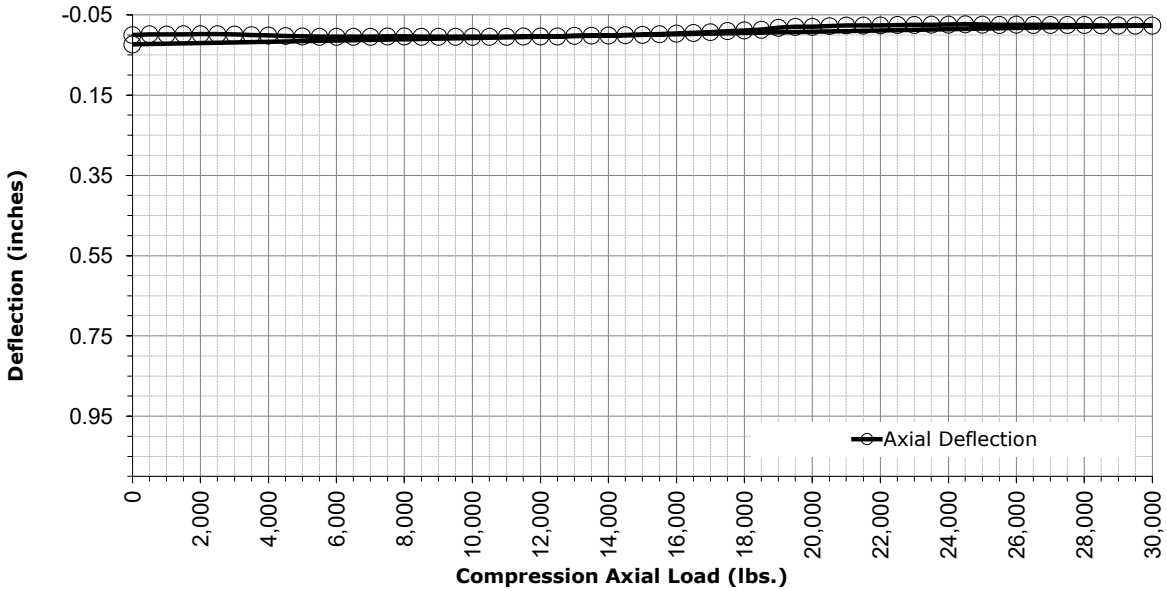
Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-4B
Latitude [deg.]:	33.64825
Longitude [deg.]:	-114.72316
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	62



Compression Load Test Result for BPLT-5A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-5A
Latitude [deg.]:	33.64748
Longitude [deg.]:	-114.72365
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	61

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.0000	
2%	500	0.0000	
3%	1,000	0.0020	
5%	1,500	0.0020	
7%	2,000	0.0020	
8%	2,500	0.0020	
10%	3,000	0.0020	
12%	3,500	0.0020	
13%	4,000	0.0020	
15%	4,500	0.0023	
17%	5,000	0.0020	
18%	5,500	0.0020	
20%	6,000	0.0020	
22%	6,500	0.0020	
23%	7,000	0.0013	
25%	7,500	0.0013	
27%	8,000	0.0013	
28%	8,500	0.0010	
30%	9,000	0.0008	
32%	9,500	0.0008	
33%	10,000	0.0005	
35%	10,500	0.0005	
37%	11,000	0.0005	
38%	11,500	0.0008	
40%	12,000	0.0003	
42%	12,500	0.0008	
43%	13,000	0.0010	
45%	13,500	0.0013	
47%	14,000	0.0010	
48%	14,500	0.0010	
50%	15,000	0.0005	
52%	15,500	0.0000	
53%	16,000	-0.0013	
55%	16,500	-0.0018	
57%	17,000	-0.0028	
58%	17,500	-0.0043	
60%	18,000	-0.0055	
62%	18,500	-0.0068	
63%	19,000	-0.0080	
65%	19,500	-0.0078	
67%	20,000	-0.0073	
68%	20,500	-0.0070	
70%	21,000	-0.0070	
72%	21,500	-0.0063	
73%	22,000	-0.0060	
75%	22,500	-0.0060	
77%	23,000	-0.0050	
78%	23,500	-0.0048	
80%	24,000	-0.0038	
82%	24,500	-0.0030	
83%	25,000	-0.0028	
85%	25,500	-0.0018	
87%	26,000	-0.0015	
88%	26,500	0.0000	
90%	27,000	0.0000	
92%	27,500	0.0013	
93%	28,000	0.0023	
95%	28,500	0.0030	
97%	29,000	0.0038	
98%	29,500	0.0050	
100%	30,000	0.0090	
0%	0	0.0115	

Compression Load Test Result for BPLT-5A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

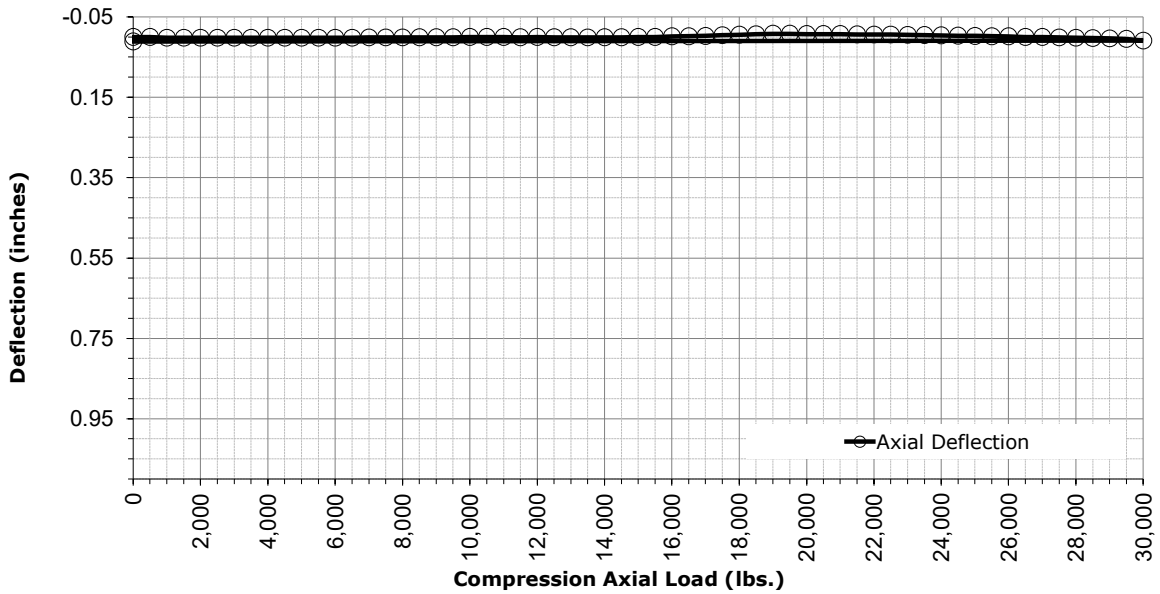
Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-5A
Latitude [deg.]:	33.64748
Longitude [deg.]:	-114.72365
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	61



Compression Load Test Result for BPLT-5B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-5B
Latitude [deg.]:	33.64748
Longitude [deg.]:	-114.72365
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	263

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.0000	
2%	500	0.0003	
3%	1,000	-0.0010	
5%	1,500	-0.0025	
7%	2,000	-0.0035	
8%	2,500	-0.0058	
10%	3,000	-0.0060	
12%	3,500	-0.0010	
13%	4,000	-0.0010	
15%	4,500	-0.0005	
17%	5,000	0.0003	
18%	5,500	0.0007	
20%	6,000	0.0015	
22%	6,500	0.0025	
23%	7,000	0.0038	
25%	7,500	0.0045	
27%	8,000	0.0060	
28%	8,500	0.0075	
30%	9,000	0.0093	
32%	9,500	0.0115	
33%	10,000	0.0133	
35%	10,500	0.0138	
37%	11,000	0.0163	
38%	11,500	0.0175	
40%	12,000	0.0190	
42%	12,500	0.0210	
43%	13,000	0.0225	
45%	13,500	0.0245	
47%	14,000	0.0273	
48%	14,500	0.0290	
50%	15,000	0.0308	
52%	15,500	0.0318	
53%	16,000	0.0330	
55%	16,500	0.0345	
57%	17,000	0.0350	
58%	17,500	0.0368	
60%	18,000	0.0373	
62%	18,500	0.0390	
63%	19,000	0.0398	
65%	19,500	0.0405	
67%	20,000	0.0413	
68%	20,500	0.0425	
70%	21,000	0.0433	
72%	21,500	0.0443	
73%	22,000	0.0455	
75%	22,500	0.0463	
77%	23,000	0.0468	
78%	23,500	0.0478	
80%	24,000	0.0485	
82%	24,500	0.0490	
83%	25,000	0.0505	
85%	25,500	0.0505	
87%	26,000	0.0523	
88%	26,500	0.0520	
90%	27,000	0.0538	
92%	27,500	0.0540	
93%	28,000	0.0545	
95%	28,500	0.0553	
97%	29,000	0.0555	
98%	29,500	0.0560	
100%	30,000	0.0570	
0%	0	0.0108	

Compression Load Test Result for BPLT-5B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

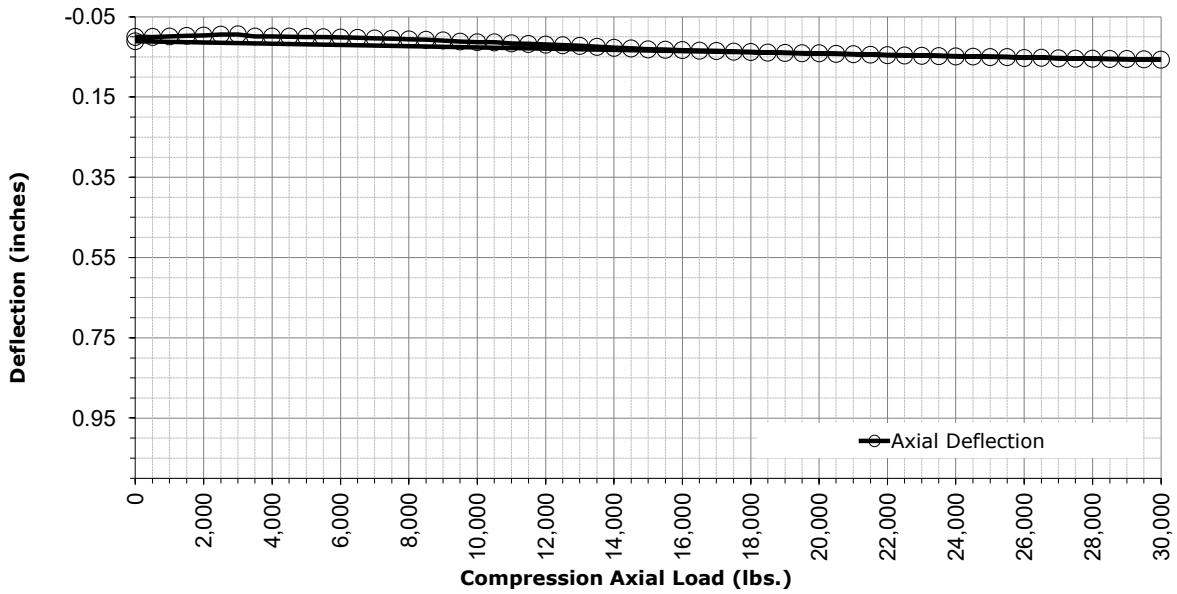
Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-5B
Latitude [deg.]:	33.64748
Longitude [deg.]:	-114.72365
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	263



Compression Load Test Result for BPLT-6A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-6A
Latitude [deg.]:	33.64711
Longitude [deg.]:	-114.72519
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	30

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.0000	
2%	500	0.0005	
3%	1,000	0.0028	
5%	1,500	0.0023	
7%	2,000	0.0025	
8%	2,500	0.0045	
10%	3,000	0.0055	
12%	3,500	0.0070	
13%	4,000	0.0085	
15%	4,500	0.0100	
17%	5,000	0.0120	
18%	5,500	0.0130	
20%	6,000	0.0148	
22%	6,500	0.0163	
23%	7,000	0.0170	
25%	7,500	0.0180	
27%	8,000	0.0195	
28%	8,500	0.0208	
30%	9,000	0.0220	
32%	9,500	0.0230	
33%	10,000	0.0245	
35%	10,500	0.0255	
37%	11,000	0.0270	
38%	11,500	0.0275	
40%	12,000	0.0295	
42%	12,500	0.0305	
43%	13,000	0.0315	
45%	13,500	0.0325	
47%	14,000	0.0335	
48%	14,500	0.0355	
50%	15,000	0.0365	
52%	15,500	0.0375	
53%	16,000	0.0395	
55%	16,500	0.0400	
57%	17,000	0.0405	
58%	17,500	0.0415	
60%	18,000	0.0423	
62%	18,500	0.0438	
63%	19,000	0.0450	
65%	19,500	0.0470	
67%	20,000	0.0485	
68%	20,500	0.0495	
70%	21,000	0.0510	
72%	21,500	0.0523	
73%	22,000	0.0540	
75%	22,500	0.0545	
77%	23,000	0.0565	
78%	23,500	0.0580	
80%	24,000	0.0588	
82%	24,500	0.0595	
83%	25,000	0.0625	
85%	25,500	0.0633	
87%	26,000	0.0650	
88%	26,500	0.0660	
90%	27,000	0.0675	
92%	27,500	0.0690	
93%	28,000	0.0705	
95%	28,500	0.0720	
97%	29,000	0.0740	
98%	29,500	0.0760	
100%	30,000	0.0770	
0%	0	0.0505	

Compression Load Test Result for BPLT-6A

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

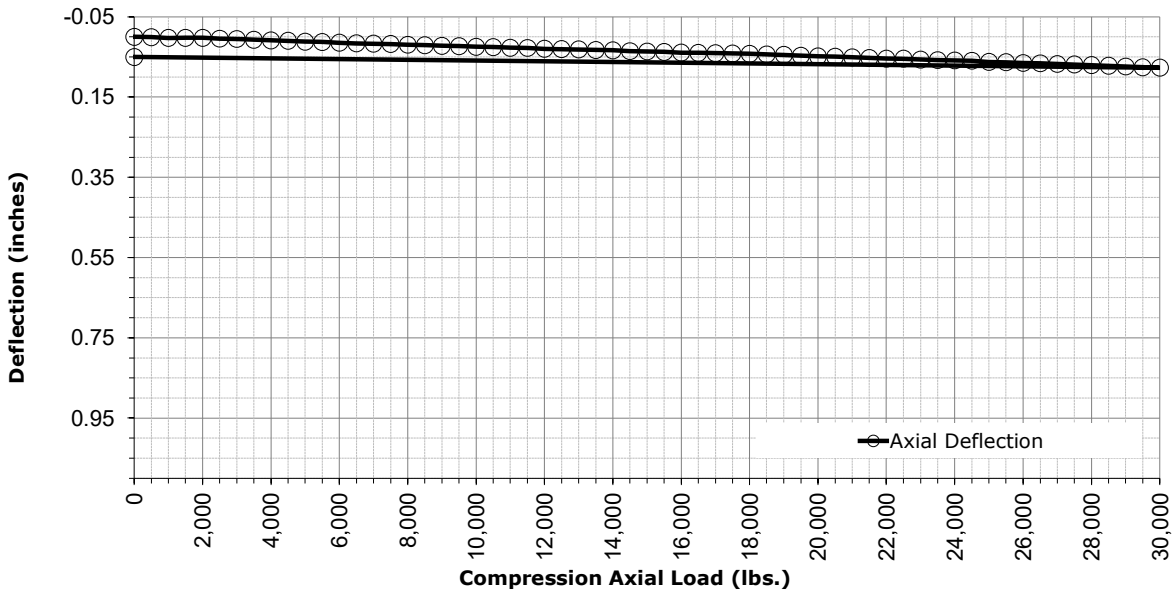
Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-6A
Latitude [deg.]:	33.64711
Longitude [deg.]:	-114.72519
Pile Type:	W6x9
Pile Embedment Depth [in.]:	96
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	30



Compression Load Test Result for BPLT-6B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-6B
Latitude [deg.]:	33.64711
Longitude [deg.]:	-114.72519
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	48

Compression Test Results

% of Design Load	Axial Load [lbs.]	Deflection Δ (in.) Gauges #1 & #2	Comments
0%	0	0.0000	
2%	500	0.0020	
3%	1,000	0.0045	
5%	1,500	0.0075	
7%	2,000	0.0125	
8%	2,500	0.0160	
10%	3,000	0.0205	
12%	3,500	0.0250	
13%	4,000	0.0250	
15%	4,500	0.0385	
17%	5,000	0.0415	
18%	5,500	0.0490	
20%	6,000	0.0540	
22%	6,500	0.0600	
23%	7,000	0.0660	
25%	7,500	0.0720	
27%	8,000	0.0770	
28%	8,500	0.0830	
30%	9,000	0.0900	
32%	9,500	0.0975	
33%	10,000	0.1020	
35%	10,500	0.1055	
37%	11,000	0.1125	
38%	11,500	0.1225	
40%	12,000	0.1290	
42%	12,500	0.1325	
43%	13,000	0.1355	
45%	13,500	0.1428	
47%	14,000	0.1450	
48%	14,500	0.1505	
50%	15,000	0.1565	
52%	15,500	0.1620	
53%	16,000	0.1670	
55%	16,500	0.1730	
57%	17,000	0.1843	
58%	17,500	0.1883	
60%	18,000	0.1970	
62%	18,500	0.2040	
63%	19,000	0.2115	
65%	19,500	0.2195	
67%	20,000	0.2230	
68%	20,500	0.2285	
70%	21,000	0.2305	
72%	21,500	0.2330	
73%	22,000	0.2380	
75%	22,500	0.2410	
77%	23,000	0.2460	
78%	23,500	0.2498	
80%	24,000	0.2530	
82%	24,500	0.2565	
83%	25,000	0.2603	
85%	25,500	0.2640	
87%	26,000	0.2695	
88%	26,500	0.2730	
90%	27,000	0.2788	
92%	27,500	0.2830	
93%	28,000	0.2880	
95%	28,500	0.2915	
97%	29,000	0.2983	
98%	29,500	0.3120	
100%	30,000	0.3080	
0%	0	0.0790	

Compression Load Test Result for BPLT-6B

Project Information

Project Name:	Grace Solar
Project Location:	Blythe, California
Project Number:	60225164
Installation Date:	5/15/2025

Axial Load Test Set Up

Number of Gauges:	2
Height of Gauges [in]:	6
Load Cell:	Rice Lake Compression

Test Date and Representative

Tested By Terracon Rep:	AS
Date Tested:	5/20/2025

Pile Information

Pile ID:	BPLT-6B
Latitude [deg.]:	33.64711
Longitude [deg.]:	-114.72519
Pile Type:	W6x9
Pile Embedment Depth [in.]:	144
Pile Diameter [in.]:	5.9
Pile Stick-Up [in.]:	24
Axial Design Load [lbs.]:	30,000
Pile Area [sq. in.]:	2.68
Elastic Modulus [ksi.]:	29,000
Drive Time [sec.]:	48

