EXHIBIT F

Water Availability Analysis Chappellet Vineyards APN 032-560-022 and 033

Chappellet Vineyard LLC 1581 Sage Canyon Road St. Helena, California 94574

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Introduction

Chappellet Vineyard LLC is seeking to plant 34.2 acres net (41.9 acres gross) of vineyard on two subject parcels with APN's 032-560-022 and 032-560-033 (Figures 1 and 2). Parcel APN 032-560-022 covers about 120 +/- acres and has approximately 12.8 acres of existing vineyard to which would be added approximately 11.2 net acres (13.7 gross acres). Parcel APN 032-560-033 is approximately 118 +/- acres and does not have existing vineyard; 23 net acres (28.2 gross acres) are proposed for new planting on this parcel. The existing vineyard is presently irrigated using primarily diverted surface water stored in reservoirs under existing water rights. The existing irrigation system is described in the water demand section of this report. The proposed new vineyard acreage will be irrigated using an existing well (Well 2, Vineyard Well) along with a new well (Well 14, location shown on Figure 2).

The Chappellet Vineyard LLC and Chappellet Winery Inc. own six additional parcels in the area (032-560-014, 032-560-015, 032-560-016, 032-560-021, 032-560-030, 032-560-031), including the two parcels with proposed vineyard areas; these eight parcels comprise approximately 605 acres (Figure 2). The Chappellet Winery, whose water supply well is located on parcel APN 032-560-031, between to two project parcels (Figures 1 and 2) has previously submitted plans to modify an existing County of Napa use permit to expand winery production, add employees, and increase the number of annual visitors to the winery for tours, tastings, and events. The impacts of that proposed change upon groundwater supply has been documented in a previous study by O'Connor Environmental dated July 27, 2018. The groundwater recharge area defined in the prior report includes the vineyard water supply well (Well 2, Figures 1 and 2) and is used again for this analysis. This report builds on the previous evaluation considering the conditions proposed by the winery project and adding the proposed changes in groundwater demand related to the vineyard expansion.

This Water Availability Analysis (WAA) was developed based on the guidance provided in the Napa County Department of Planning, Building, & Environmental Services' Water Availability Analysis Guidance Document formally adopted by the Napa County Board of Supervisors in May 2015. The WAA includes the following elements: estimates of existing and proposed water uses within the project recharge area, compilation of drillers' logs from the area and characterization of local hydrogeologic conditions, analyses to estimate groundwater recharge relative to proposed uses (Tier 1), and a screening analysis of the potential for well interference at neighboring wells located within 500-ft of the project well (Tier 2), and an analysis of potential for significant stream flow depletion of sensitive streams identified by County of Napa within 1,500 ft of project wells (Tier 3).

This document has been revised in September 2021 and January 2022 to include additional details regarding proposed project wells and an evaluation of drought water use conditions. A memorandum responding to comments detailed in a communication from Don Barrella of Napa County dated November 4, 2021, accompanies this revision. A memorandum responding to comments detailed in a communication from Don Barrella of Napa County dated December 10,



2020, also accompanied the September 2021 revision. Additionally, at the time of this revision the Chappellet Winery Use permit modification #P18-00307-MOD has been officially approved. However, all uses associated with that permit remain referenced as proposed in this report as this approval had not occurred at the time of the original completion of this analysis. The January 2023 revision incorporates a new baseline precipitation for estimating groundwater recharge instituted by County of Napa in late November 2022 and a more detailed Tier 3 analysis.

Limitations

Groundwater systems of Napa County and the Coast Range are typically complex, and available data rarely allows for more than general assessment of groundwater conditions and delineation of aquifers. Hydrogeologic interpretations are based on the drillers' reports made available to us through the California Department of Water Resources, available geologic maps and hydrogeologic studies, and professional judgment. This analysis is based on limited available data and relies significantly on interpretation of data from disparate sources of disparate quality.

Given the significant depths to water in the project wells (350 and 400ft), the relationship between groundwater recharge generated within the project parcel area and groundwater availability at the project wells is not expected to be tightly coupled. It is likely that water flowing to the project wells is primarily supplied by groundwater inflows from surrounding areas rather than from recharge occurring on the overlying landscape. Analysis of the age and sources of the deep groundwater occurring beneath the project parcel is beyond the scope of this study.

Hydrogeologic Conditions

The project parcels are located on Pritchard Hill east of Lake Hennessey on a topographic bench oriented northwest to southeast in the mountains east of the Napa Valley. The bedrock geology mapped in the area of the project parcels is typical of the uplands east of the southern half of Napa Valley. The main geologic unit mapped at the project site and intersected by the project well (Well 1) is andesitic and basaltic lava flows of the Tertiary-aged Sonoma Volcanics (Map unit Tsa, see Figure 2). This portion of the Tsa unit is part of an approximately 32 square-mile northwest to southeast oriented block bound to the north and east by a contact with the older Mesozoic-aged rocks of the Coast Range Ophiolite which is a portion of the Great Valley Complex, the Franciscan Complex, and a relatively large Quaternary landslide and to the west by overlying alluvium of the Napa Valley. The Tsa unit is part of the lower member of the Sonoma Volcanics which was described by Weaver (1949) as individual lava flows displaying great variability in thickness and texture over short distances. Given this heterogeneity it can be expected that hydrogeologic conditions exhibit similar spatial variability and yields from wells completed anywhere in the Tsa unit. Reconnaissance confirmed the mapped bedrock geology.

Rocks of the Sonoma Volcanics overlie the basement rocks of the Mezosoic-age Coast Range Ophiolite and the Franciscan Complex. Several driller's logs including that of the project well (Well 1) report encountering serpentine at depths of 600 ft or more (Appendix A). The Serpentinite (sp) unit of the Coast Range Ophiolite is of Jurassic (144-208 My) age and is mainly sheared serpentinite but also can include harzburgite (Graymer, 2007).



The rocks of the Coast Range Ophiolite are generally considered poor aquifer material; however, successful wells of generally limited capacity are common in this highly variable geologic unit. Primary porosity in the Coast Range Ophiolite is low and groundwater occurs primarily in fractures. Well yields are variable depending on the degree of fracturing; however, yields are generally low and on the order of a few gallons per minute; dry test holes are also common within these rocks (LCSE, 2013).

In general, wells drilled in the Sonoma Volcanics tend to have low to moderate yield. Storativity in the Volcanics are reported to range from 0 to 15% (Nishikawa,2013) Typical yields range from 16 to 50 gallons per minute (gpm) with reported yields as high as several hundred gpm (LSCE 2013). Unwelded sections of tuff are considered to be good water producers (DWR 1982). Bedrock units such as the Andesite to Basalt Lava Flows (map unit Tsa) typically have low primary porosity and are only water yielding where fractured (DWR 1982).



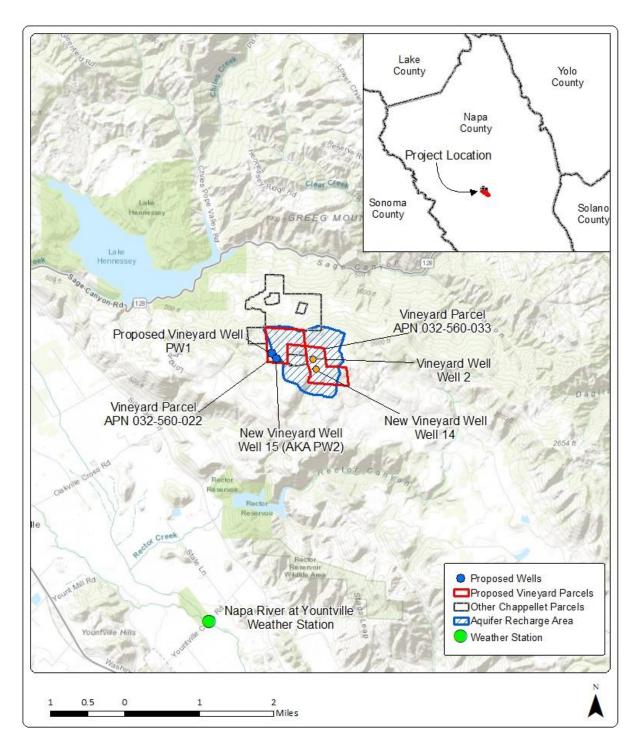


Figure 1: Project location map

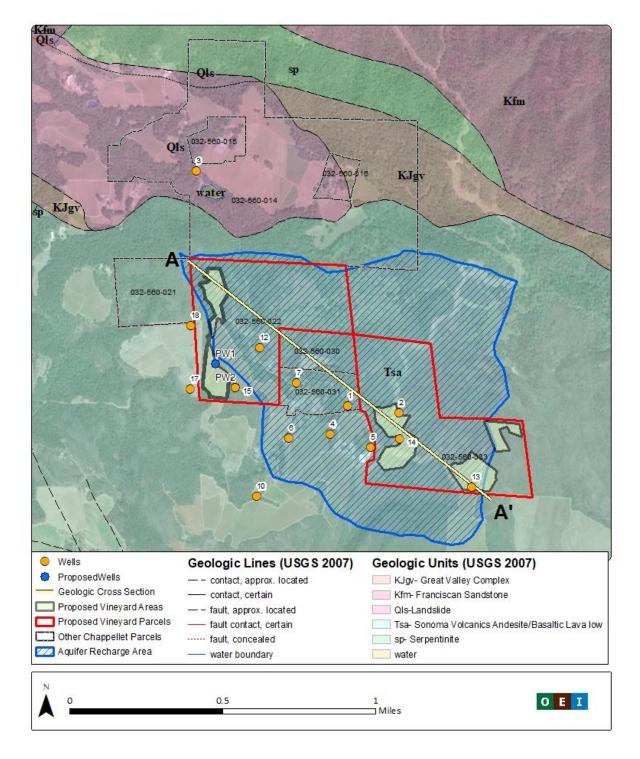


Figure 2: Surficial geology and locations of wells on and near the project parcel. Surficial geology based on data from the Geologic Map of Eastern Sonoma and Western Napa Counties (Graymer et al., 2007).



Well Data

Our search of the Department of Water Resources and County of Napa records found a total of 11 well completion reports for wells completed in similar geology and located within approximately one mile of the project parcels. Of the 11 wells, 10 were located specifically on surrounding parcels while one was only located generally in the Pritchard Hill area; data for this latter well was used to summarize local hydrogeologic characteristics but was not used in any geospatial analysis. Several well locations were identified by Chappellet staff while the remaining wells on surrounding parcels were located using the locations marked on the well logs as guidance and then confirmed using aerial photos of the area.

In July of 2018 Well 12 was drilled to 650 ft and water was first encountered at 580 ft; however, the drill rig lost equipment down the well bore and in the process of recovery the well bore was destroyed so a static water level is not reported in Table 1. A well log describing the geologic materials encountered during drilling was not available at the time of the writing of this report. Later in October of 2018 another well (Well 13, log found in Appendix A) was drilled to the southeast on the eastern vineyard parcel however no water was encountered.

After the submission of the earlier version of this WAA two new wells were proposed (PW1 and PW2 in Figures 2 and 4). At the time of this December 2022 revision both wells have been permitted by Napa County and one (PW2, now Well 15) has been constructed. Well 15 is now considered a project well in addition to Wells 2 and 14. PW1 is expected to be constructed in the next year (2023). This well, if successful, will in addition to existing Wells 2, 14 and 15, supply water to all existing and proposed Chappellet Vineyards, supplementing surface water as needed in some cases (see Water Demand and Drought Water Use Considerations sections later in this report for more details). Another two wells located on neighboring parcels that have been constructed since the original version of this report have also been added to the map and Table 1. Wells 17 and 18 are located just west of the northern project parcel. All well locations are shown in Figure 2. Applicable well information was compiled and is shown in Table 1, see Appendix A for well logs.

Most wells in the area are productive, Well 13 being the exception. Well bores range from 398 to 798 feet deep, with an average depth of about 658 feet. Geologic materials encountered in all wells are described by drillers as hard gray volcanic or dark rock interbedded with ash of varying color and/or red to brown hard rock. These descriptions are consistent with what we would expect to find in an area mapped as andesitic and basaltic lava flows within a larger geologic formation made up of various volcanic rocks. Geologic logs for all wells within the project recharge area report rocks consistent with the Sonoma Volcanics to depths of approximately 600 ft. Wells 1, 2 and 7 all report green rock or green serpentine with gray shale beginning at depths between 600 and 615 ft deep. This boundary is likely the contact between the Sonoma Volcanics and the underlying serpentinite (sp) basement rocks of the of the Coast Range ophiolite. Reported static water level for all 11 wells ranged between 240 and 565 feet below ground surface with an average depth of 401 feet. In several wells the depths at which water was encountered coincided with layers of ash or tuff and fractured basalt or andesite. Well yields



reported on Well Completion Reports ranged from 10 to 150 gallons per minute, with an average of 41 gallons per minute. Typically, operational well yields are about half or less compared to yields reported on Well Completion Reports.

Well Number	1	2	3		4	5	6	7	8
Year Completed	2008	2014	1982	2 20)10 2	2007	1995	1991	1992
Depth (ft)	627	625	640	7	98	750	620	700	398
Estimated Yield (gpm)	45	30	10	3	35	25	20	30	40
Static Water Level (ft)	400	350	450	42	20	460	400	460	249
Top of Screen (ft)	447	225	420	50	00 3	300	420	440	258
Bottom of Screen (ft)	627	625	640	7	98 [.]	750	620	700	398
Geologic Unit	Tsa	Tsa	Tsa	T	sa	Tsa	Tsa	Tsa	Tsa
Well Number	9	10	11	12	13	14	15	17	18
Year Completed	1988	2008	2014	2018	2018	2019	2021	2019	2019
Depth (ft)	650	755	560	650	785	670	890	820	738
Estimated Yield (gpm)	20	45	150	Lost Eqp.	DRY HOLE	25	125	250	38
Static Water Level (ft)	420	565	240	-	-	400	535	429	541
Top of Screen (ft)	440	605	280	-	-	450	530	500	518
Bottom of Screen (ft)	650	755	560	-	-	670	870	800	718
Geologic Unit	Tsa	Tsa	Tsa	Tsa	Tsa	Tsa	Tsa	Tsa	Tsa

Table 1: Well completion details for the project well (Well 2) and nearby wells.

The first project well, Well 2, known as the Vineyard Well, is located on the southern vineyard parcel APN 032-560-033. The Vineyard Well was completed in 2014 to a depth of 625 feet and had a static water level of 350 feet below the ground surface. The well is completed in materials consistent with what would be expected in the Tsa unit: "Hard gray fractured rock", "Hard purple rock" and "Soft green ash", "Black ash" and "White ash" down to a depth of 600 feet. At 600 feet the driller reports "Green Serpentine with Gray Shale" which is evidence that they penetrated the basement rocks of the Coast Range ophiolite. Screened intervals begin at a depth of 225 ft and alternate every 20 feet with blank casing until the bottom of the well at 625 ft. From 605 to 625 ft the casing is blank while 585 to 605 ft is perforated, therefore only 5 ft of the screened interval is within the serpentinite (sp). Due to the small section of perforated pipe within the rocks of the Sonoma Volcanics. After four hours of pumping the well driller reported an estimated yield of 30 gallons per minute with a drawdown of 270 ft.

Well 14, the second project well that will supply water to the proposed vineyards, is located 445 ft south of Well 2 on parcel APN 032-560-033. This well was drilled in July 2019 to a depth of 670 ft. The Geologic Log describes materials similar to those encountered in Well 2 consistent with what would be expected in the Tsa unit down to 600 feet: "Red Black clay" "Black basalt", "Hard grey rock" and "Black red volcanics". At 600 ft feet the driller first reports 20 ft of "Green rock Green S[e]rp[e]ntine" followed by 20 ft of "yellow rock rust signs", 20 ft of "White ash" and a final 20 ft of "Green S[e]rp[e]ntine" at the bottom of the borehole. Again the presence of serpentine rock is evidence that the boring penetrated to the basement rocks of the Coast Range ophiolite at 600 ft. Well 14 is screened between 450 ft and 670 ft with alternating 20 ft lengths



of blank casing. Most of the screened interval intersects rocks within the Sonoma Volcanics with at most 40 ft of perforated casing intersecting serpentine (sp) rocks of the ophiolite. Due to the relatively small section of perforated pipe within the rocks of the sp and the generally poor aquifer characteristics of the sp the project aquifer is assumed to be within the Sonoma Volcanics. Depth to first water is reported as 430 ft; after development the static water level was reported as 400 ft. A four-hour air lift test was performed at the time of development with an estimated yield of 25 gpm; 100 ft of drawdown was observed.

Well 1, is also known as the Corral Well and is located about 900 ft west of Well 2 and on parcel number 032-560-031. This parcel is owned by Alexa Chappellet et al, an official easement allowing the winery to use this water is included in the 2014 Transient Non-community Water System technical, managerial and financial report by Applied Engineering (Applied Civil Engineering, 2014). The Corral Well was drilled in 2008 to a depth of 710 ft and completed to a depth of 627 ft. The geologic log describes a sequence of clays and gray rock for the first 125 ft, ash and gray rock were encountered between 125 ft and 450 ft, and hard light gray and hard green and gray rock from 450 ft to 615 ft. The sequence of rocks described to this depth is consistent with the Tsa unit. At 615 ft rocks described as "gray and green shale with streaks of serpentine" are recorded to the bottom of the hole at 710 ft, indicating that they penetrated the basement rocks of the Coast Range ophiolite. Well 1 is screened between 447 ft and 627 ft. Approximately 12 ft of the screened interval is within the serpentinite (sp). Due to the generally poor aquifer characteristics of this rock and the relatively short section of perforated well casing in the serpentinite, the project aquifer is assumed to be within the Sonoma Volcanics.

The Well Completion Report for Well 1 also indicates depth to first water as 440 ft and a static water level of 400 ft after development in May 2008. A pump test was performed in August 2011 and reported a pre-pumping water level of 408 ft. After six hours of pumping at rate of 30 gallons per minute the water level had drawn down eight ft to 416 ft and remained stable for the last two hours of the test. Within four minutes after shutting off the pump the water level had recovered to its initial level of 408.

Using the spatial distribution of groundwater levels for Wells 1-10, an interpolated groundwater surface was generated using the Kriging method (a procedure fitting a surface to data) in ArcGIS. A contour layer is displayed in Figure 2. It should be noted that the groundwater elevation data used for this interpolation comes from well logs up to 36 years old and may not be representative of current conditions. However, more recent water elevations are generally consistent with older elevations. As shown in Figure 2, the groundwater contours indicate a general flow direction from east to west with a slight ridge of groundwater running east-west along a line from Well 2 to Well 7. This ridge marks a potential divide in the groundwater flow directions, one to the northwest and one to the southwest. Wells 2 and 1 are located along this ridge due to the nature of the interpolation. A cross section displaying the ground surface, interpolated groundwater surface and well locations is shown in Figure 3.



Geologic Cross-Section

A geologic cross-section oriented northwest by southeast is shown in Figure 3 (see Figure 2 for location). The interpolated groundwater surface is displayed along with the approximated contact between Tsa and sp. Depths and casing intervals are also shown for Wells 1, 2, 7, 12 and 14. This representation shows the groundwater table dipping to the northwest at a depth of approximately 400 to 700 feet below ground surface. Water was not encountered in the recently drilled Well 13, as such the approximate groundwater surface does not extend to Well 13.

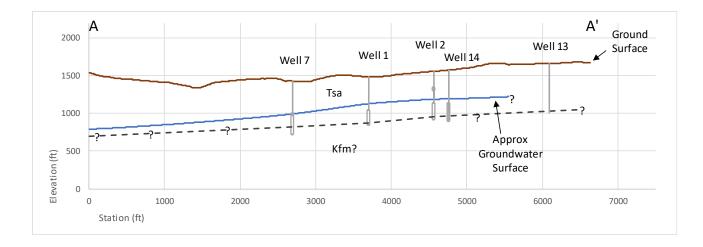


Figure 3: Hydrogeologic cross section A -A' through the vicinity of the project parcel (see Figure 2 for location).

Project Aquifer

The extent of the project aquifer/project recharge area is defined by surface water drainage patterns and the interpolated groundwater surface. The northern, northwestern, and eastern boundaries of the project aquifer are defined by surface drainage divides directing flows towards the main channel that flows through the northeast corner of the winery well parcel. The southern and southwestern boundary extends to include a portion of the drainage to the south of the winery well parcel to include a portion of the interpolated groundwater surface draining to the south that is potentially intersected by Well 1, 2, and 14.

The total area of the project aquifer is 459 acres. The recharge area is underlain by the Tsa unit of the Sonoma Volcanics. The project wells (Well 2, Vineyard Well and Well 14) are screened within the Sonoma Volcanics and proposed wells PW1 and PW2 are also expected to be completed in the same formation. Given that depths to groundwater are relatively deep and given that impermeable layers of material including clays are present within the Sonoma Volcanics, the project aquifer is likely confined or semi-confined.

The extent of the project aquifer/recharge area was initially delineated to include the recharge area to Project Wells 1, 2 and 14 prior to the planning for the newly proposed project wells PW1 and PW2. In a previous draft of this WAA (February 2020) we evaluated water demand within this recharge area on the project parcel and neighboring parcels and compared proposed



groundwater use to estimated recharge. In our tabulation of uses we made the conservative assumption that all uses on neighboring parcels to the west associated with the Continuum Winery (APN's 032-560-029, 032-560-032, 032-030-043 and 032-030-044 in Figure 4) use groundwater associated with the project recharge area (Figure 4). Proposed Project well PW2 is to be located less than 200 ft outside of the recharge boundary delineated for Wells 1, 2 and 14 (Figure 2). The existing analysis already accounts for groundwater use outside the recharge boundary in this area, so the addition of this well outside the recharge boundary does not alter our estimate of groundwater use.

Groundwater Storage Estimate

An estimate of the total available groundwater storage within the aquifer recharge area can be obtained as the product of the aquifer area in units of acres, the saturated aquifer thickness in units of feet, and the specific yield of the aquifer. This method is generally applicable to unconfined aquifers with primary porosity and are not generally appropriate for confined aquifers. The project aquifer is likely semi-confined or confined with water storage primarily in fractures. The estimate of groundwater storage is an approximation for general interpretive and comparative purposes.

The saturated thickness of the aquifer was estimated as the difference between the average static water level and average base of screened interval in the wells in the project vicinity (Table 1). The mean depth of the static water level of the productive wells within the project recharge area is 397 feet while the average depth to the bottom of the screened interval is 643 feet. Subtracting the mean static water level from the mean depth to the bottom of the screened interval estimates a saturated thickness of 246 feet. This is a conservative estimate of aquifer thickness as the Sonoma Volcanics Formation likely extends to significantly greater depths within the project recharge area. As reported above specific yield estimates for the Sonoma Volcanics range from 0 to 15% (Nishikawa, 2013). To be conservative, we have used a low-end estimate of specific yield of 2% for the project aquifer. This results in an estimated groundwater storage of 2,257 ac-ft (459 acres x 246 feet x 0.02).

Water Demand

Within the project recharge area, water demand was estimated for both the existing and proposed conditions. Water use at the winery and vineyards on surrounding parcels owned by Chappellet was determined using information provided by Chappellet and verified using available satellite imagery. The project recharge area also includes portions of six neighboring parcels. Use on these parcels was estimated using Napa County agricultural and winery GIS database information along with satellite imagery. One additional parcel that has the same ownership and a contiguous vineyard with a parcel intersecting the recharge area was included in the existing water use estimate. Uses within the recharge area include winery use, residential use and irrigation for vineyards and small orchards.



Existing Use

In the existing condition, the Chappellet winery parcel (APN 032-560-015) contains a residence, a portion of vineyard, a barrel storage building and the winery. The demand of the modestly sized single-family residence was 0.5 ac-ft annually; this rate is in the middle of the range provided by Napa County guidance for single family dwellings. The vineyard on the winery parcel part of a larger block of vines that extend onto the adjacent parcels APN's 032-560-014 and 032-560-016 also owned by Chappellet. These vines are irrigated using surface water diversions and do not require groundwater from the project well. Irrigation practices of vineyards on adjacent parcels are discussed later in this report.

The following summary of existing groundwater use focuses on total uses within the project well recharge area (Figure 2 and Figure 4), and therefore includes groundwater use estimated for other parcels along with the Chappellet parcels (Tables 2-7). Total existing water use by the Chappellet Winery parcel, served by Well 1, is described in detail below.

Currently the winery is permitted to produce 150,000 gallons of wine a year with a total of 24 combined full time and part time employees. Based on Napa County water use guidelines, demand for winery processing water is 2.15 ac-ft per 100,000 gallons of wine while winery domestic and landscaping demand is an additional 0.5 ac-ft per. Annual production of 150,000 gallons gives a total demand of approximately 4 ac-ft. Employee daily use is estimated to be 15 gallons per employee per Napa County. Assuming the 24 employees work five days a week all year or 260 days the total demand equals 0.29 ac-ft annually in addition to the 4 ac-ft required by the winery.

Daily tours and tastings at the winery are approved to host a maximum of 40 visitors a day. Assuming 40 visitors a day, 365 days a year, this totals to a maximum of 14,600 tasting visitors annually. For marketing events that include on-site catering Chappellet is approved for several events with varying numbers of guests totaling a maximum of 2,470 guests annually. Napa County guidance assumes a daily water use of 3 gallons per tasting visitor and 15 gallons per marketing events visitors. Using these rates, the existing maximum annual demand for all 17,070 visitors to the winery is 0.25 ac-ft.

In addition to uses on the Chappellet winery parcel, use on the surrounding parcels within the project recharge area includes three additional residences, landscaping associated with these houses, vineyard, a small orchard, and the Continuum winery. Two residences are large and are assumed to have a demand of 0.75 ac-ft per year the upper limit suggested by Napa County guidance (Table 3). The third residence is located on the winery well parcel and is smaller so a demand of 0.5 ac-ft per year (similar to that of the winery parcel residence) is applied. Lawn and landscaping areas above the first 1,000 ft² on these parcels total 32,165 ft² and 5,050 ft² respectively (Table 3).

A total of 88.1 acres of vineyard is located on or connected to parcels with wells within the project recharge area. Of this Chappellet owns 14.2 acres and irrigates with water collected via surface water diversions stored in existing reservoirs. Four existing water rights (A020616, A026508, A027298 and D032686 are registered to Chappellet Vineyard LLC in the California Department of



Water Resources database with a total of 134 ac-ft allowed annually. Although the rights allow for the diversion of 134 ac-ft, the existing reservoirs are only able to capture approximately 50 ac-ft. In addition to diversions, 3 ac-ft of water is recycled from winery use and approximately 3 ac-ft of rainwater is collected from roofs and stored in the onsite ponds. An additional 93.7 acres of vineyard farmed by Chappellet is located on parcels to the north of the recharge area (032-560-014, 015, and 016, and 320-010-010 see Figure 4). These vines are also irrigated using recycled water, rainwater and water collected into the reservoirs as mentioned above. However, under extreme drought conditions they would require to be irrigated with groundwater from project wells, an evaluation of this condition is made later in this report. Chappellet estimates irrigation demand for all 107.9 acres of existing vineyard varies between 30 and 40 ac-ft depending on the season. Applying this demand to the total area of vineyard computes to a range of demand per unit area of 0.28 to 0.37 ac-ft/ac/yr.

Nearly all the water used for irrigation of <u>existing</u> Chappellet vineyards is recycled water or stored surface water; in addition, a very small amount of water from Well 2 and Well 14 (and Wells PW1 and PW2 in the future) may be used at the end of the growing season. Chappellet estimates this amount to be less than 1% of the total irrigation volume. If we assume conservatively that this amount is 1% of the upper end of the annual demand of 40 ac-ft this would be a demand of 0.4 ac-ft (Table 4).

For the January 2022 revision of this WAA, Chappellet staff provided additional irrigation details for the years 2017 through 2021 summarized Appendix B which contains a detailed summary of 2017-2021 irrigation data. All water for irrigation for these years was provided by surface water diverted and stored under existing Water Rights, recycled wastewater, rainwater catchment and groundwater. Over the last five years, which have included both relatively wet (WY2017 and WY 2019) and very dry (WY2020 and WY 2021) conditions, irrigation has ranged from 0.16 ac-ft/ac to 0.38 ac-ft/ac with an average rate of 0.24 ac-ft/ac. Extremely dry winters over the past two seasons (2020 and 2021) have required reductions in irrigation to conserve severely limited surface water supplies.

Total irrigation has ranged from 17.5 ac-ft to 24.5 ac-ft annually for the 2017 -2021 period. This recorded range is far below the original assumptions (30-40 ac-ft annual use) presented in previous versions of this analysis and indicate that our estimates of use (which include the upper end of County Guidance irrigation rates) have been quite conservative.

The remaining 73.6 acres of vineyard within the project recharge area are located on four parcels west of the project well (APN's 032-560-029, 032-560-032, 032-030-043 and 032-030-044 in Figure 4) these parcels are all associated with the Continuum Winery. Although the parcel boundary for 032-030-043 does not intersect the recharge area it does have continuous vineyard with the adjacent parcel to the east (APN 032-030-044). To be conservative, it is assumed that along with all vineyards on Continuum this vineyard uses water from a well located within the recharge area. Two reservoirs are located on these parcels and are associated with three appropriative water rights totaling 25 ac-ft annually. Assuming annual vineyard irrigation demand of 0.5 ac-ft per acre per year, the 73.6 acres of vines would require 36.8 ac-ft annually. Although the specific practices are not known for these vineyards it is highly likely that this



diverted water is used to irrigate these 73.6 acres of vineyard. The allotted 25 ac-ft would be sufficient to meet about 68% of the estimated demand of 36.8 ac-ft; it is assumed that the remaining 11.8 ac-ft of vineyard irrigation (equivalent to 23.6 acres of vineyard) is supplied by one of the wells located on the Continuum parcels within the project recharge area. Additional vineyard area located on neighboring parcel (032-030-010 Figure 4) on the southwest edge of the recharge area was not included because no wells were identified within the recharge area.

In addition to the vineyards, 0.7 acres of orchard were identified on a parcel within the recharge area. Napa county guidance lists an annual demand of 4 ac-ft per acre for orchards which results in an annual demand of 2.8 ac-ft for the existing condition.

Water use for the Continuum winery was estimated using information reported in the Napa County Winery GIS shapefile. The current information associated with permit P10-00255-MOD for Continuum shows an annual production of 28,000 gallons with 16 employees which amounts to a total winery demand of 0.93 ac-ft. Tastings are by appointment only and a maximum of 2 visitors/day (728 annually) are allowed. For marketing events a maximum annual count of 450 visitors is listed. Assuming a usage of 3 gallons per visitor for tastings and 15 gallons per visitor for marketing events the maximum visitor use for Continuum is 0.03 ac-ft annually.

Based on these uses, the existing water demand within the project recharge area is estimated to be 26.4 ac-ft/yr (Table 2). Residential water demand is estimated to be 6 ac-ft/yr (Table 3), irrigation demand is estimated to be 15 ac-ft/yr (Table 4), winery use is estimated to be 4.7 ac-ft/yr (Table 5) winery guest use is estimated to be 0.28 ac-ft/yr (Table 6), and winery employee use is estimated to be 0.48 ac-ft/yr (Table 7).



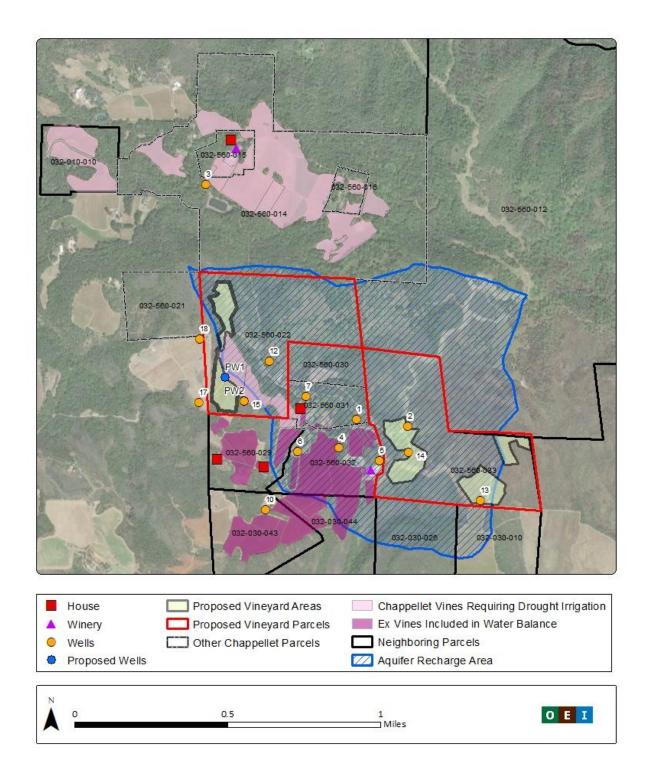


Figure 4. Existing and proposed uses within project recharge area.



	Irrigation Use (acre-ft/yr)	Residential use (acre-ft/yr)	Winery Use (acre-ft/yr)	Employee Use (acre-ft/yr)	Event Use (acre-ft/yr)	Total Use (acre-ft/yr)
Existing Use	15.0	6.0	4.7	0.5	0.3	26.4
Proposed Use Low - High Vineyard Irrigation Rate (includes increased demand from Winery and Additional Vines)	23.2 - 32.1	6.0	7.4	0.6	0.5	37.6 - 46.5

Table 2: Existing and proposed groundwater uses within the project recharge area.

Table 3: Estimated existing and proposed residential groundwater use within the project recharge area.

Use Category	# of Units	Use per Unit (ac-ft/yr)	Use per 1,000 square feet above first 1,000 (ac-ft/yr)	Annual Water Use (ac-ft/yr)
Oversized Main Residence	2	0.75		1.50
Main Residence	2	0.50		1.00
Lawn	32		0.10	3.22
Other Landscaping	5		0.05	0.25
TOTAL				6.0

Table 4: Estimated existing vineyard and orchard irrigation groundwater use within the project recharge area.

Use Category	Number of Acres	Use per Acre (ac-ft/yr)	Supplemental Chappellet Use (ac-ft/yr)	Annual Water Use (ac-ft/yr)
Existing Non- Chappellet Vineyard Irrigation	23.6	0.5	0.4	12.2
Existing Orchard Irrigation	0.70	4		2.8
TOTAL				15.0

Note: Under normal rainfall conditions all existing Chappellet vineyards are irrigated using recycled water, collected rainwater and surface water except the additional 0.4 ac-ft end of season irrigation which is provided by groundwater. New Chappellet vineyard irrigation demand will be met with groundwater from Wells 2, 14 PW1 and PW2. Non Chappellet vineyard irrigation represents a portion of the total acreage as a portion of these vines are assumed to be irrigated by surface water.

 Table 5: Estimated existing winery groundwater use by wells located within the project recharge area. This includes Chappellet winery use (pre 2020 Use Permit modification) and Continuum winery use.

Use Category	Annual Production (gal/yr)	Use per 100,000 gal of production	Annual Water Use (ac-ft/yr)
Winery Process Use	178,000	2.15	3.83
Winery Domestic Use	178,000	0.50	0.89
TOTAL			4.7

Table 6: Estimated existing winery guest groundwater use by wells located within the project recharge area. This includes Chappellet winery use (pre 2020 Use Permit modification) and Continuum winery use.

	# of	Use per	Annual
Visitor Category	Vistors	Visitor	Water Use
Tours and Tastings	15,288	3	0.14
Marketing w/ Onsite Catering	2,920	15	0.13
TOTAL			0.28

Table 7: Estimated existing employee groundwater use by wells located within the project recharge area. This includes Chappellet winery use (pre 2020 Use Permit modification) and Continuum winery use.

Work Category	# of Employees	# Work Days per Year	Use per Employee (gal/day)	Annual Water Use (ac-ft/yr)
Full-time	40	260	15	0.48
TOTAL				0.48

Proposed Use

In the proposed condition, 34.2 net acres (41.9 acres gross) of vineyard will be added to two project parcels (032-560-022 and 032-560-033) and the Chappellet Winery will increase wine production by 100,000 gallons for a total annual production of 250,000 gallons. The total number of visitors annually will increase by 21,835 to 38,905. Six new employees will be added for a total of 30 employees. New vineyard will be irrigated using groundwater from the vineyard well (Well 2), the new Well 14 (Figures 1 and 2) and if successful proposed wells PW1 and PW2.

As explained above irrigation rates for existing Chappellet vineyards averaged 0.24 ac-ft/ac annually for the past five years. To account for new vine establishment periods when irrigation rates are greater, proposed use is estimated for a range of rates between 0.24 acre ft/ acre/yr (the five year average) and 0.5 ac-ft/ac/yr (the maximum rate listed in the Napa County guidance). It is assumed that during most years, including dry years, Chappellet will be using the lower irrigation rate. During extreme drought years such as water years 2020 and 2021 Chappellet will need to reduce rates even further as they did in 2020/2021. This scenario is described in detail in the Drought Water Use Considerations section below.



Applying the range of demands per unit area (ac-ft/ac) produces estimated demand ranging from 8.6 to 17.5ac-ft/yr for the proposed vineyard areas (Table 8). Winery water demand (Tables 9-11) will be met by groundwater pumped from the Corral Well (Well 1).

No other uses will change as part of the proposed projects. In this condition the estimated water use will increase by a maximum of 20.1 ac-ft/yr to 46.5 ac-ft/yr; 3 ac-ft/yr for the winery use modification and 17.1 ac-ft/yr for the additional vineyard acreage (Table 2). All increases in groundwater use are from increases in irrigation use (Table 8), winery processing and domestic use (Table 9), winery guest use (Table 10) and winery employees (Table 11).

Table 8. Proposed vineyard and orchard irrigation groundwater use by wells located within the project recharge area.

	Number of	Use per Acre	Supplemental	Annual Water
Use Category	Acres	(ac-ft/yr)	Chappellet Use	Use (ac-ft/yr)
Non Chappellet Vineyard Irrigation	23.6	0.5		11.8
Proposed Chappellet Vineyard Irrigation	34.2	0.24 - 0.5	0.4	8.6 - 17.5
Orchard Total	0.70	4.0		2.8
TOTAL				23.2 - 32.1

 Table 9: Estimated proposed winery groundwater use by wells located within the project recharge area. This includes Chappellet winery use post 2020 Use Permit modification and Continuum winery use.

	Annual	Use per	Annual Water
Use Category	Production	100,000 gal of	Use (ac-ft/yr)
Winery Process Use	278,000	2.15	6.0
Winery Domestic Use	278,000	0.50	1.4
TOTAL			7.4

 Table 10: Estimated proposed winery guest groundwater use by wells located within the project recharge area.

 This includes Chappellet winery use post 2020 Use Permit modification and Continuum winery use.

	# of	Use per	Annual
Visitor Category	Vistors	Visitor	Water Use
Tours and Tastings	35,403	3	0.33
Marketing w/ Onsite Catering	4,680	15	0.22
TOTAL			0.54

Table 11: Estimated proposed employee groundwater use by wells located within the project recharge area. This includes Chappellet winery use post 2020 Use Permit modification and Continuum winery use.

Work Category	# of Employees	# Work Days per Year	Use per Employee (gal/day)	Annual Water Use (ac-ft/yr)
Full-time	46	260	15	0.55
TOTAL				0.55



Groundwater Recharge Analysis

Groundwater recharge within the project recharge area was estimated using a Soil Water Balance (SWB) of Napa County developed by OEI. This model implements the U.S. Geologic Survey's SWB modeling software and produces a spatially distributed estimate of annual recharge. This model operates on a daily timestep and calculates runoff based on the Natural Resources Conservation Service (NRCS) curve number approach and Actual Evapotranspiration (AET) and recharge based on a modified Thornthwaite-Mather soil-water-balance approach (Westenbroek et al., 2010). Details of this model are included in Appendix C.

To address elevated concerns regarding groundwater availability during the current extended regional drought, Napa County has specified that groundwater recharge estimates must use a 10-year precipitation average from Water Years 2012 to 2021 developed by the PRISM Group at Oregon State University for Napa County. The PRISM data provides spatially distributed data adjusted for orographic factors based on gauged precipitation data. OEI's SWB modeling has also utilized PRISM precipitation data.

OEI's earliest application of SWB for WAA for Chappellet projects pre-dated OEI's comprehensive SWB implementation covering all of Napa County. For the January 2023 WAA revision, OEI is utilizing SWB simulations described in Appendix C as described below, in addition to a unique SWB implementation for Water Year 2021 previously developed for a prior WAA revision to address County concerns regarding the potential effects of extended drought on groundwater recharge.

OEI's use of the SWB model is believed to provide more accurate estimates of potential groundwater recharge because it is a physically based distributed model that incorporates information characterizing the water balance in the soil column. Calculation of evapotranspiration using local climate data along with soil moisture storage and precipitation is believed to provide a more accurate representation of local conditions; evapotranspiration is the largest component of the water balance. Unfortunately, the SWB model structure does not allow for a groundwater recharge calculation based on a mathematical average because the model is driven by daily climate data. Consequently, OEI has adapted the SWB model estimates for the prior "average year" (WY 2010), the "drought year" (WY 2014), and the "extreme drought year" (WY 2021) to provide an estimate for the average annual rainfall for the period 2012-2021 developed by County of Napa.

The first, Water Year 2010, was selected to represent average year conditions because annual precipitation totals across most of Napa County were close to their long-term 30-year averages. The second, Water Year 2014, was selected to represent drought average conditions because annual precipitation totals were between 41 and 73% of long-term 30-year averages for much of Napa County. The third year, Water Year 2021, was selected to represent extreme drought conditions in Napa County.

OEI has utilized SWB models for WY 2010 and WY 2014 for dozens of project sites in the County of Napa. We have observed that potential recharge for WY 2010 is consistently much greater



than for WY 2014 across a wide variety of terrain, vegetation, soils and climate. This is most easily characterized by the percentage of annual precipitation available for recharge that we calculate for each project site. Our approach for adapting the SWB model outputs to estimate groundwater recharge for the specified annual average precipitation (2012- 2021) is to assume that the percentage of annual rainfall available for groundwater recharge is a linear function of annual rainfall and interpolating between the recharge percentage for WY 2010, WY 2014 and WY 2021. The interpolation procedure is unique for each project site; the application for this project site is graphically displayed in Figure 5. The water balance data from the SWB model years is tabulated in Table 12.

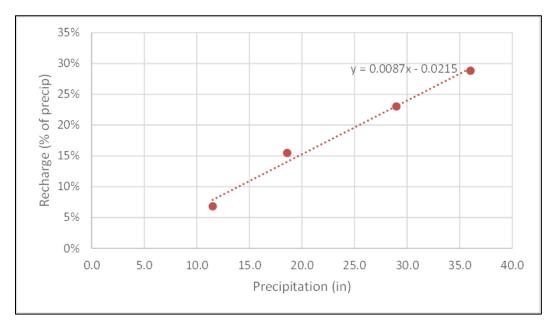


Figure 5: Relationship between precipitation and percent of precipitation as recharge.

Results

The simulated Water Year 2010 (average water year) recharge results indicate that recharge varied across the project recharge area from near zero to 14.8 inches. Spatially averaged over the project recharge area, recharge accounted for 10.4 of the 36.0 inches (29%) of precipitation in 2010 (Table 12). The simulated Water Year 2014 (drought water year) recharge results indicate that recharge varied across the project recharge area from near zero to 6.52 inches). Spatially averaged over the project recharge area, only 2.9 of the 18.6 inches of precipitation (15%) was recharge (Table 12). Results from the simulated Water Year 2021 (extreme drought water year) recharge were spatially averaged over the project recharge area to be 0.8 inches or 7% of the 11.5 inches of precipitation for Water Year 2021 (Table 12). The spatially-averaged precipitation for the project recharge area for the Water Years 2012-21 PRISM average is 29.0 inches. Based on the linear relationship between annual precipitation for WY 2010, 2014 and 2021, and corresponding SWB estimates of groundwater recharge as a percentage of annual precipitation (Figure 5), Water Years 2012 to 2021 had an average of 6.7 inches of recharge or 23% of the 29.0 inches of precipitation (Table 12).



	2010 Nor	mal Year	2014 Dry Year		2021 Extreme Drought Year		2012-2021 WY Average	
	inches	% of precip	inches	% of precip	inches	% of precip	inches	% of precip
Precipitation	36.0	-	18.6	-	11.5	-	29.0	-
AET	18.8	52%	14.3	77%	13.5	117%	-	-
Runoff	7.5	21%	4.6	25%	0.9	8%	-	-
∆ Soil Moisture	-0.6	-2%	-3.3	-18%	-3.7	-32%	-	-
Recharge	10.4	29%	2.9	15%	0.8	7%	6.7	23%

Groundwater recharge estimates can also be expressed as a total volume by multiplying the calculated recharge by the project aquifer recharge area of 459 acres. This calculation yields an estimate of total recharge of 110.2 ac-ft/yr during the drought conditions of Water Year 2014 and of 397.1 ac-ft/yr for the average Water Year of 2010. For the extreme drought year conditions of 2021 total recharge calculated to 30.1 ac-ft/yr. For the impact area the averaged 2012-2021 WYs the recharge is estimated to be 255.3 ac-ft/yr (Table 13).

LSCE (2013) estimated recharge based on water balance modeling in several watersheds in the county underlain primarily by Sonoma Volcanics (Milliken Creek, Tulucay Creek, Conn Creek and Napa River above Calistoga). The recharge estimates in these watersheds ranged from 5 to 21% of annual precipitation. The recharge estimates produced from this study (29% of average water year precipitation, 15% of the dry water year precipitation and 23% of the 2012-2021 average water year precipitation) using SWB fall within and slightly above the range of the LSCE estimates for larger watershed areas underlain by Sonoma Volcanics and appear to be reasonable.

The total proposed groundwater use for the project recharge area is estimated to range from 37.6 ac-ft/yr to 46.5 ac-ft/yr combined for all parcels intersecting the recharge area (Table 3). Estimated groundwater use in the project recharge area is equivalent to between 15% and 18% of the estimated average of 2012-2021 water years groundwater recharge of 255.3 ac-ft/yr. This use is equivalent to between 9% and 12% of the estimated average water year groundwater recharge of 397.1 ac-ft/yr. Assuming Chappellet irrigates at the rate of 0.24 ac-ft/ac during dry years similar to WY 2014, total use of 37.6 ac-ft represents 34% of the estimated dry water year recharge of 110.2 ac-ft/yr (Table 14). Average year groundwater recharge should be used for comparison with groundwater use for determination of long-term water availability.

These comparisons indicate that there is a substantial surplus of groundwater resources in terms of estimated average annual groundwater recharge to the project recharge area. This is considering both the WY2010 SWB recharge estimate and the interpolated recharge estimate based on the WY 2012-2021 average PRISM rainfall (Table 13). <u>Given the magnitude of this surplus, the increase in water use associated with the proposed increase in vineyard acreage (8.2)</u>



– 17.1 ac-ft) along with the additional winery production, employees and guest attendance associated with the use permit modification (3.0 ac-ft) is unlikely to result in significant long-term reductions in groundwater levels or depletion of groundwater resources.

Table 13: Evaluation of groundwater demand as a percentage of estimated groundwater recharge comparing water use estimated for the project recharge area including proposed project use for both low and high vineyard irrigation estimates on the project parcels.

		Average Water Year (2010)				Dry Water Year (2014)		
USE	Total Proposed Demand (ac-ft/yr)	Recharge (ac-ft/yr)	Recharge Surplus (ac-ft/yr)	Demand as % of Recharge	Recharge (ac-ft/yr)	Recharge Surplus (ac-ft/yr)	Demand as % of Recharge	
Project Recharge Area Low Vineyard Irrigation Rate	37.6	397.1	359.4	9%	110.2	72.5	34%	
Project Recharge Area High Vineyard Irrigation Rate	46.5	397.1	350.5	12%	-	-	-	

		2012-2021 WY Average				
USE	Total Proposed Demand (ac-ft/yr)	Recharge (ac-ft/yr)	Recharge Surplus (ac-ft/yr)	Demand as % of Recharge		
Project Recharge Area Low Vineyard Irrigation Rate	37.6	255.3	217.7	15%		
Project Recharge Area High Vineyard Irrigation Rate	46.5	255.3	208.8	18%		

Similarly, to give a perspective of demand versus estimated recharge at the project parcel scale, recharge can also be estimated for each project parcel. The northern vineyard parcel (APN 032-010-076) is approximately 120 acres and has a spatially averaged recharge of 103.8 ac-ft/yr for average conditions (Water Year 2010), 28.8 ac-ft/yr of recharge for drought conditions (Water Year 2014), and 66.7 ac-ft/yr of recharge for the average of 2012-2021 Water Years. The southern project parcel (APN 320-010-094) is approximately 118 acres has a spatially averaged recharge of 102.1 ac-ft/yr for average conditions (Water Year 2010), 28.3 ac-ft/yr of recharge for drought conditions (Water Year 2014), and 65.6 ac-ft/yr of recharge for the average of 2012-2021 Water Years. Table 14 presents a summary of estimated recharge on the two project parcels.

At the project parcel scale, proposed vineyard water use (the only proposed use) can be compared to estimated recharge across each project parcel. On the northern vineyard project parcel (APN 032-560-022) the estimated demand ranges from 2.7 ac-ft/yr to 5.6 ac-ft/yr which is 4% and 8% of the 66.7 ac-ft of estimated recharge during the averaged 2012-2021 water years period. Demand on the northern vineyard parcel ranges from 2.7 corresponds tobetween 3% and 5% of the 103.8 ac-ft of estimated recharge during the average water year. Assuming only the



low vineyard irrigation rate (0.24 ac-ft/ac) is used during the dry years, the 2.7 ac-ft demand represents and 9% of the 28.8 ac-ft of estimated dry year recharge (Table 14). In the southern vineyard project parcel (APN 032-560-033) the estimated demand ranges from 5.5 ac-ft/yr to 11.5 ac-ft/yr which is equivalent to 5% and 11% of the 65.6 ac-ft of estimated recharge during the averaged 2012-2021 water years period. The estimated southern parcel represents between 8% and 18% of the 102.1 ac-ft of estimated average year recharge. The demand is19% of the 28.3ac-ft of estimated dry year recharge (Table 14).

Table 14: Comparison of proposed water use to average, dry year and the 2012-2021 groundwater recharge on proposed vineyard parcels. Note during dry year conditions the vineyard irrigation rate of 0.24 ac-ft/ac is applied.

		Average	Average Water Year (2010)			Dry Water Year (2014)		
USE	Total Proposed Demand (ac-ft/yr)	Recharge (ac-ft/yr)	Recharge Surplus	Demand as % of	Recharge (ac-ft/yr)	Recharge Surplus	Demand as % of	
Project Parcel North 032-560-022 Low Vineyard Irrigation Rate	2.7	103.8	101.1	3%	28.8	26.1	9%	
Project Parcel South 032-560-033 Low Vineyard Irrigation Rate	5.5	102.1	96.6	5%	28.3	22.8	19%	
Project Parcel North 032-560-022 High Vineyard Irrigation Rate	5.6	103.8	98.2	5%	-	-	-	
Project Parcel South 032-560-033 High Vineyard Irrigation Rate	11.5	102.1	90.6	11%	-	-	-	

		2012-3	age	
USE	Total Proposed Demand (ac-ft/yr)	Recharge (ac-ft/yr)	Recharge Surplus	Demand as % of
Project Parcel North 032-560-022 Low Vineyard Irrigation Rate	2.7	66.7	64.0	4%
Project Parcel South 032-560-033 Low Vineyard Irrigation Rate	5.5	65.6	60.1	8%
Project Parcel North 032-560-022 High Vineyard Irrigation Rate	5.6	66.7	61.1	8%
Project Parcel South 032-560-033 High Vineyard Irrigation Rate	11.5	65.6	54.1	18%



Drought Water Use Considerations

Recent drought conditions have required many growers to adjust water use including the project applicant. For growers like Chappellet Vineyards, who use surface water diversions, if winter rains fail to fill their storage reservoirs, they will likely turn to groundwater as their alternative water source. Representatives of Chappellet Vineyards have indicated that this would be the case and so we have evaluated a scenario where this occurs. During severe drought conditions Chappellet Vineyards report that they have reduced irrigation rates down to minimum rate of approximately 0.16 ac-ft/ac/yr as shown in Appendix B.

Table 15 shows proposed irrigation demand across the project recharge area where Chappellet use has been reduced to 0.16 ac-ft/ac and in addition to the proposed 34.2 acres of new vineyard, includes an additional 107.9 acres of Chappellet vineyard normally irrigated with diverted surface water (See Figure 4). It should be noted that even during the very dry 2021 growing season Chappellet staff report that a small amount of surface water (approximately 25% of normal capacity) was available for irrigation indicating that water demand assumptions presented for the drought condition are conservative. For this scenario we assume that all other water uses within the recharge area, except for vineyard irrigation on neighboring parcels, remained the same. In the case of vineyard irrigation, we also assume that all reservoirs on the neighboring Continuum parcels to the west are dry and vineyards on these neighboring parcels are entirely irrigated with groundwater. Drought year irrigation rates for the Continuum vineyards are not known and we have assumed a range of irrigation rates between 0.2 ac-ft/ac and 0.5ac-ft/ac (Table 15). Table 16 shows a summary of proposed water use within the project recharge area under the drought conditions where total annual use increases from 26.4 ac-ft to between 54.7 to 76.8 ac-ft.

Use Category	Number of	Use per Acre	Annual Water
	Acres	(ac-ft/yr)	Use (ac-ft/yr)
Existing Neighboring Vineyard Irrigation	73.6	0.2 - 0.5	14.7 - 36.8
Proposed Chappellet Vineyard Irrigation	142.1	0.16	22.7
Existing Orchard Total	0.70	4.0	2.8
TOTAL			40.3 - 62.3

Table 16: Total proposed groundwater use in the project recharge area under drought water use conditions.

	Irrigation Use (acre-ft/yr)	Residential use (acre-ft/yr)	Winery Use (acre-ft/yr)	Employee Use (acre-ft/yr)		Total Use (acre-ft/yr)
Existing Use	15.0	6.0	4.7	0.48	0.28	26.4
Proposed Use (includes increased demand from Winery)	40.3 - 62.3	6.0	7.4	0.55	0.51	54.7 - 76.8

Table 17: Comparison of proposed groundwater use by all wells within the project recharge area during drought conditions to extreme drought year groundwater recharge under drought water use conditions. Note a range of irrigation values were used for irrigation of non-project vineyards.

		Extreme Drought Water Year (20					
USE	Total Proposed Demand (ac-ft/yr)	Recharge (ac-ft/yr)	Recharge Surplus (ac-ft/yr)	Demand as % of Recharge			
Project Recharge Area Low Vineyard Irrigation Rate	54.7	30.1	-24.6	182%			
Project Recharge Area High Vineyard Irrigation Rate	76.8	30.1	-46.7	255%			

The total proposed groundwater use for the project recharge area during drought conditions where all Chappellet vineyards are irrigated with groundwater at a reduced rate equivalent to what was used in 2021(0.16 ac-ft/ac) and neighboring vineyards are irrigated over a range of rates between 0.2 ac-ft/ac and 0.5 ac-ft/ac is estimated to range from 54.7 ac-ft/yr to 76.8 ac-ft/yr for all parcels intersecting the recharge area. Estimated groundwater use in the project recharge area would be equivalent to between 182% and 255% of the extreme drought water year groundwater recharge of 30.1 ac-ft (Table 17).

This comparison shows that under extreme drought conditions, even if irrigation rates are reduced to conserve water an overdraft of groundwater resources in terms of estimated average annual groundwater recharge to the project recharge area will result. This would mean that during years when annual rainfall was not sufficient to recharge the project aquifer local wells would be accessing groundwater reserves stored in the aquifer. As described earlier in this report the project aquifer has approximately 2,257 ac-ft in storage. The total maximum demand of 76.8 ac-ft under extreme drought conditions represents 3% of this storage reservoir. Although there is uncertainty regarding the volume of groundwater storage, the magnitude of storage relative to use indicates that there is a significant quantity of water stored in the aquifer that will buffer imbalances in recharge and use that occurs in dry years. Higher rates of recharge in wet years compensate for extreme dry years when water use exceeds recharge. Water use greater than recharge may occur in some years, but aquifer storage is believed to be sufficient to maintain access to groundwater in wells in the project areas.

Well Interference Analysis (Tier 2 WAA)

There are no non-project wells within 500 feet of the project well(s). The nearest neighboring well that could be precisely located (Well 5) is 517 feet southwest of the proposed well location on the southern project parcel (Figure 2). Based on the WAA guidance document, a Tier 2 well interference analysis is not required given that all non-project wells are located greater than 500-feet from the project wells.



Groundwater/Surface Water Interactions (Tier 3 WAA)

Napa County Tier 3 WAA guidance for assessment of groundwater-surface water interactions was modified in late-2022, in part owing to the emergency policy adopted in June 2022. The County has identified streams of concern for potential streamflow depletion by groundwater pumping and now requires analysis of this potential for all wells within 1,500 feet of designated streams.

Project Wells and Potential Streamflow Depletion

The nearest surface waters to the project wells belong to two unnamed tributaries to Conn Creek and one unnamed tributary to Rector Creek (Figure 6). Project wells 2, 14, 15, and proposed project well PW1 are all within 1,500 feet of these waterbodies and per Napa County guidance a Tier 3 analysis is necessary to evaluate potential project impacts to surface waters.

The northern unnamed tributary to Conn Creek flows to the north bisecting the northern project parcel. The southern unnamed tributary to Conn Creek flows to the northwest approximately 870 feet to the southwest of the northern project parcel. Both watercourses flow into Lake Hennessey which empties into Conn Creek and thence the Napa River. The proposed project well PW1 and Well 15 are located approximately 740 feet and 825 feet from the northern watercourse, respectively. Project Well 15 is located 1,400 feet from the southern unnamed tributary to Conn Creek while PW1 is 1,550 feet away.

The unnamed tributary to Rector Creek originates on a neighboring parcel just south of the southern project parcel and flows to the south and east into the Rector Reservoir which empties into Rector Creek and thence the Napa River. Project wells 14 and 2 are located 1,025 and 1,480 feet respectively from the head of this unnamed tributary (Figure 6).

These stream reaches have been identified as having intermittent flow in a 2015 mapping effort by the Napa County Resource Conservation District (LSCE, 2022). In contrast with perennial streams, which are assumed to have a hydraulic connection to groundwater, intermittent streams are believed to have the potential to be connected for only limited periods of time. Typically, intermittent streams are connected during times of higher flows during winter and spring (seasonal intermittency). The periods when these streams would be expected to contain surface flow do not coincide with periods of higher groundwater use associated with the irrigation season. During the late summer and fall both creeks would most likely be disconnected from groundwater and not susceptible to potential impacts related to pumping of the project well.

All project wells are understood to be screened entirely within rocks of the Sonoma Volcanics, a fractured bedrock aquifer which is known to have relatively low permeability. Based on local geologic mapping and field inspections both tributaries intersect the Tsa unit of the Sonoma Volcanics (Figures 2 and 3). Although there is a likely to be some exchange of groundwater between the stream and underlying bedrock, connectivity to groundwater accessed by project wells is expected to be very limited by the relatively low hydraulic conductivity of the fine-grained rocks across the significant thickness of material between the surface and elevation at which project wells encounter groundwater. Regardless of the hydraulic characteristics of the aquifer



material, there is significant horizontal and vertical separation between the wells and the streams of concern.

Groundwater elevations reported in well completion reports were used with Napa County LiDAR to estimate the vertical separation between the stream bed and groundwater surface. The vertical separation between 1) groundwater elevation and the nearest stream elevation and 2) uppermost well perforations (well screen) and nearest stream elevation are summarized in Table 18 along with other relevant information about the wells. The vertical separations range from 257 feet in Well 14's WSE to the unnamed tributary to Rector Creek up to 445 feet separating Well 15's WSE from the bed of the northern unnamed tributary to Conn Creek. Similarly, the vertical separation between the nearby stream bed and the upper limit of well screens in project wells was determined for each well; these values range from 180 feet in Well 2 to 440 feet in Well 15. Figures 7 and 8 show relative elevations based on the 2003 Napa County LiDAR elevation DEM. Note that proposed project well PW1 is expected to be constructed within very similar materials with a similar depth and extent of perforations to Well 15 and so we expect a very similar depth to groundwater and therefore vertical separation from the nearby stream beds as seen with Well 15.

Well Number	2	14	15
Year Completed	2014	2019	2021
Depth (ft)	625	670	890
Estimated Yield (gpm)	30	25	125
Static Water Level Elevation (ft)	1219.5	1161.8	919.6
Top of Screen Elevation (ft)	1344.5	1141.8	954.6
Bottom of Screen Elevation (ft)	944.5	921.8	584.6
Depth of Cement Seal (ft)	56	55	58
Geologic Unit	Tsa	Tsa	Tsa
Approximate vertical			
separtion from WSE to closest stream channel elevation (ft)	305	257	445
Approximate vertical separation from top of screen to closest stream channel elevation (ft)	180	307	440

Table 18. Project well completion details and vertical separation of groundwater elevation

For additional context regarding potential streamflow depletion, the nearest locations on each tributary with the same elevation as the groundwater elevation in corresponding wells has been identified for Wells 2, 14 and 15 in Figure 6. Distances between Well 15 and the 920 foot elevation point on each of the unnamed tributaries to Conn Creek range from 3,130 feet on the southern tributary to 4,550 feet on the northern tributary. Well 2 is located approximately 2,700 feet from where the unnamed tributary to Rector creek crosses the 1,220 foot elevation while Well 14 is about 3,050 feet from the 1,162 foot elevation. The heterogenous nature of the andesite of the Sonoma Volcanics, a fractured bedrock aquifer, makes it very unlikely that at these distances any significant connection to streamflow could occur. Note that groundwater elevations in these uplands generally mimic the surface topography and it is expected that at the set of points identified above the local groundwater elevation is likely to be at substantial depth below the stream elevation.

Due to the low permeability of the project aquifer, relatively large vertical and horizontal separation between the project well and surface waters of the tributaries we do not expect the proposed project to have any significant impacts on streamflow in the nearby intermittent streams of concern. Further discussion of Tier 3 WAA guidance with respect to well pumping capacity, well construction and distance from streams is provided below.



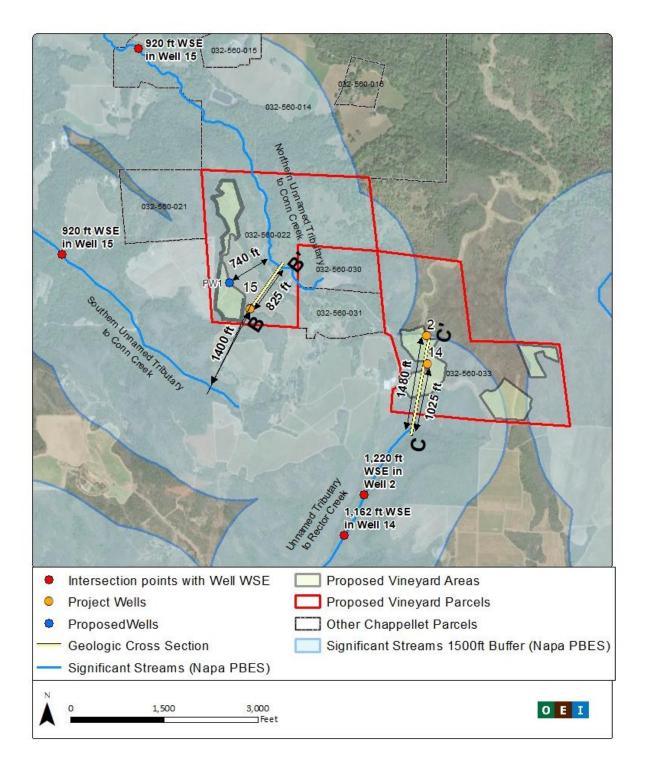
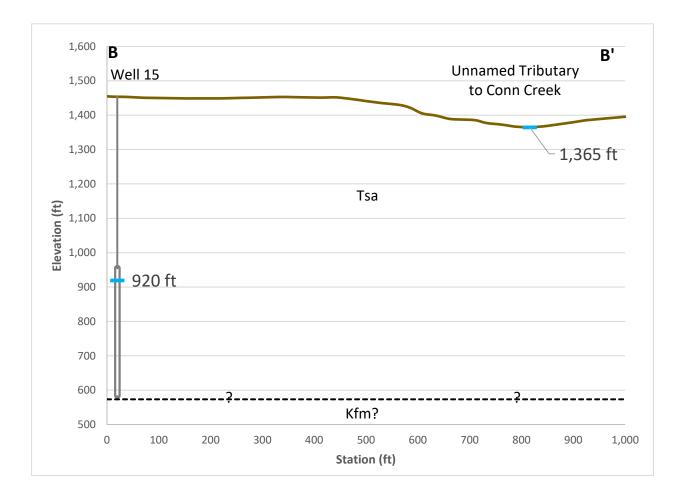


Figure 6. Project well locations in relation to Napa County defined Significant Streams.





Well

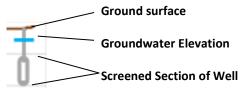
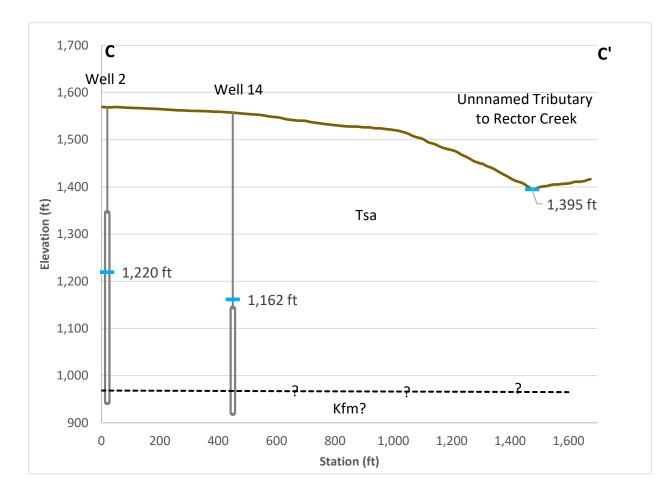


Figure 7. Hydrogeologic cross section B -B' through the vicinity of project Well 15 (see Figure 6 for location).







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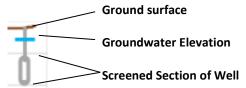


Figure 8. Hydrogeologic cross section C -C' through the vicinity of project Wells 2 and 14 (see Figure 6 for location).



Tier 3 WAA Guidance

As noted above, Napa County guidance for Tier 3 analyses presents well distance standards and construction assumptions that "if applicable would be expected to preclude and significant adverse effects on surface waters" (Napa, 2015). Specifically the "Tier 3 Groundwater Surface Water Interaction Criteria" section (Pgs 10-13 of the Napa) states:

Tier 3 analysis is only conducted when substantial evidence in the record determines the need for such an analysis. The groundwater/surface water criteria are presumptively met if the distance standards and project well construction assumptions are met (see Tables 3, 4, and 5).

These standards consider the planned pumping rate of the project well, depth of screened interval and well seal along with aquifer hydraulic conductivity values and present acceptable distances based on specific combinations of these parameters. Tables 3, 4 and 5 in the Napa WAA guidance document present these distance standards and assumptions for wells constructed in unconsolidated aquifer materials. The project wells (Wells 2, 14, and 15) are screened entirely within the andesite unit of the Sonoma Volcanics. The screened intervals and surface seal depths are shown in Table 18 above. All wells are screened at depths greater than or equal to 225 feet and all are sealed with cement to a depth of 55 feet or greater. Well yields range from 25 gallons per minute (gpm) to 125 gpm (Tables 1 and 19 and Appendix A).

The project wells have reported yields of 25, 30 and 125 gpm for Wells 14, 2 and 15 respectively. Wells 2 and 14 have yields that place them in the "Low capacity pumping rate" category of wells (defined by Napa County to be between 10 gpm and 30 gpm); therefore, distance standards are evaluated using Table 4 (reproduced below from page 12 of the Napa WAA Guidance document).

Table 4. Well Distance Standards and Construction Assumptions; Low capacity pumping rates (i.e., between 10 gpm and 30 gpm), constructed in unconsolidated deposits in the upper part of the aquifer system (unconfined aquifer conditions).

Aquifer Acceptable Distance from Surface Hydraulic Water Channel		Minimum Surface Seal Depth (feet)	Depth of Uppermost Perforations (feet)		
Conductivity (ft/day)	500 feet	1000 feet	1500 feet		
80			1	50	150
50			1	50	150
30			1	50	100
0.5		1		50	100

Well 15's yield puts it into the "Moderate to High capacity pumping rate" category of wells (defined by Napa County to be greater than 30 gpm); therefore, distance standards are evaluated using Table 5 (reproduced below form page 13 of the Napa WAA Guidance document).



Table 5. Well Distance Standards and Construction Assumptions; Moderate to high capacity pumping rates (i.e., greater than 30 gpm), constructed in unconsolidated deposits in the upper part of the aquifer system (unconfined aquifer conditions).

Aquifer Hydraulic Conductivity (ft/day)	Acceptable Distance from Surface Water Channel			Minimum Surface Seal Depth (feet)	Depth of Uppermost Perforations (feet)
	500 feet	1000 feet	1500 feet		
80			1	50	150
50			1	50	150
30			1	50	100
0.5			1	50	100

It should be noted that these tables have been constructed based on well construction in unconsolidated materials and the Napa Guidance (2015) states on page 11:

Distance standards for project wells completed in consolidated formations will generally be no more restrictive than those shown in Tables 3, 4, and 5 for hydraulic conductivity values of 0.5 ft/day.

For a well in a consolidated formation such as the andesite unit of the Sonoma Volcanics, the *de facto* hydraulic conductivity category would be 0.5 ft/day. In our professional experience in performing and reviewing well pumping tests in the region; hydraulic conductivity of the Sonoma Volcanics can be less by one or more factors of 10. Per Table 4 above an acceptable distance of 1,000 ft is recommended for Wells 2 and 14 in low pumping capacity wells category. Project Wells 2 and 14 are both located at distances greater than 1,000 feet from the unnamed tributary to Rector Creek indicating that distance and construction standards are met and that impacts of pumping of Wells 2 and 14 related to the proposed project upon flows in this creek are not likely to be significant.

Per Table 5 above project Well 15 which has a stated capacity of 125 gpm would require a distance of 1,500 feet to meet construction standards indicating that pumping will not impact streamflows. Well 15 is located approximately 825 feet from the northern unnamed tributary to Conn Creek and 1,400 feet from the southern unnamed tributary to Conn Creek (Figure 5). Although these distances do not meet the distance standards presented in Table 5, as described above, the depth to upper perforations in Well 15 (530 ft) is significant. The stream bed intersects this elevation 3,130 feet on the southern tributary to Conn Creek and 4,550 feet on the northern tributary to Conn Creek. We presume the proposed project well PW1 will have characteristics similar to Well 15.



Based on the preceding characterization of the unnamed tributaries to both Conn Creek and Rector Creek as intermittent streams with little connection to groundwater throughout the season of most groundwater use, the relatively large horizontal separation from the wells (740 ft at the least with PW1), vertical separation between project wells and each of the unnamed tributaries (at least 180 ft from the upper perforations in Well 2 up to the channel bottom of the unnamed tributary to Rector Creek), and the low permeability of the local bedrock, we do not expect that the proposed project will have a significant impact on flows in the unnamed tributaries to Conn Creek or the unnamed tributary to Rector Creek.

Summary

Application of the Soil Water Balance model (SWB) to the project recharge area revealed that average water year (based upon Water Year 2010 data) recharge was approximately 10.4 inches/yr or 397.1 ac-ft/yr. The total proposed water use for the project aquifer recharge area is estimated to be at most 46.5 ac-ft/yr during normal rainfall years. This represents about 12% of the mean annual recharge (Water Year 2010). An additional analysis of the averaged 10-year period of precipitation for Water Years 2012-2021 was performed to estimate recharge for a more recent time period including years of significant drought. This resulted in an estimated recharge of 6.7 inches/yr or 255.3 acre-ft/yr across the project recharge area. Proposed total project use (46.5 ac-ft/yr) represents 18% of the mean annual recharge for the averaged 2012-2021 water year period. Comparison of proposed project groundwater use to both normal rainfall year (Water Year 2010) and multiyear average (Water Years 2012 – 2021) recharge estimates show substantial surplus of recharge will occur indicating that the project is unlikely to result in declines in groundwater elevations or depletion of groundwater resources over time.

The nearest neighboring wells are all located more than 500-ft from the project wells indicating that a Tier 2 well interference analysis is not required. Three intermittent streams of concern are located within 1,500 feet of project wells. Based on the horizontal and vertical separation between project wells and these streams, as well as the low permeability of the local bedrock we do not expect the proposed project will have any significant impact to surface water flows.



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APPENDIX A

WELL COMPLETION REPORTS



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470	500	Black, Red Ash			N	$(\leq$		CATHODIC PROTECTION HEAT EXCHANGE DIRECT PUSH					
500	680	Dark Gray Fractured 1	Rock					INJECTION					
680	760	Dark Gray Rock & Bla	ck Fractured rec	k	SPARGIN SPARGIN SPARGIN Illustrate or Describe Distance of Well from Roads, Buildings, Fences, Rivers, etc. and attach a map. Use additional paper if OTHER (SPECIF)								
760	800	Hard Green Rock			necessary. PLEASE BE	ACCURATE & COMP	LETE.						
	(WATER I	LEVEL & YIELD	OF COMPL	LETED WELL					
	i 	·			DEPTH OF STATIC								
						35 (GPM) &	E MEASURED	<u>5-14-10</u> In 11FT					
	L DEPTH OF		7		TEST LENGTH 5	(Hrs.) TOTAL DRAW	DOWNUK	_ (Ft.)GPM at day					
TOTAI	L DEPTH OF	COMPLETED WELL	(Feet)		* May not be represer	tative of a well's lor	ıg-term yield.	of test					
EBON	DEPTH M SURFACE	BORE-	CASING (S)	r 		DEPTH FROM SURFACE	ANN	ULAR MATERIAL					
		HOLE TYPE (∠) DIA. K K K K K K K K K K K K K K K K K K K	TERIAL / INTERNAL	GAUGE OR WAL			CE- BEN-						
Ft.	to Ft.	DIA. (Inches) BI YANK CODI- DICON- DICON- DICON- CON- CON- CON- CON- CON- CON- CON-	GRADE DIAMETER (Inches)	THICKNES		Ft. to Ft.		E FILL FILTER PACK (TYPE/SIZE) (≚)					
0	55	124 V PI	astic 6	F48	0=21	0 55	i						
55	500	9 V n	<u> </u>	66		55 798		Well Pack					
500	798	9 V V	L ^L L	<u> </u>	072	, I 		bravel					
		HMENTS (∠)		L	CERTIFICATI	ON STATEMENT		<u> </u>					
	Geologic		NAME Pulliam	Well E	nis report is complete a ploration Inc		best of my k	nowledge and belief.					
	Geophys	ical Log(s)			(typed or printed) way 128 Napa	CA 94558							
	Soil/Wat	er Chemical Analyses	ADDRESS		11.	CITY		STATE ZIP					
ATTAC		INFORMATION, IF IT EXISTS.	Signed	WELL CON	Ractor		5-19-	10 808-508 C-57 LICENSE NUMBER					
L				f		UA	TE GIUNED	OSP 03 78836					

DWR 188 REV. 05-03

IF ADDITIONAL SPACE IS NEEDED, USE NEXT CONSECUTIVELY NUMBERED FORM

			·		DNLY - DO	Well 5						
ORIGINAL File with DWR	WELL	STATE OF CALIF	ORNIA ON REPORT	DITINIO	TWOR							
Page of	•	Refer to Instruction	Pamphlet 38182	STAT	E WELL NO./SI	TATION NO.						
Owner's Well No.	03/21/2007, Ended		0102									
Local Permit Ag	ency Napa-County											
Permit No.	2 67-00091 Perm	it_Date <u>03/12/</u>	2007		APN/TRS/OTH	IER						
				(mail on	11770							
	XVERTICALHORIZONTAL DRILLING METHODROLARY	FLUID <u>Air</u>	⊳M			-						
DEPTH FROM SURFACE	DESCRIPTION Describe material, grain si	1//7										
Ft. to Ft.		~ 115)	Address 1683	WELL LOCA	TION —— Road							
0 13	Brown Clay & Gray R	lock W	City Saint Hel	<u>.ena 10) //</u>	· · · · ·							
13 30	Red Clay & Red Ash	- <u>\\</u>	County Napa		rcel 061-(200						
1 t			Township		ction							
30 125	Hard Gray, Red & Br	own Ash	Eat Long Long									
125 215	Hard Gray Volcanic	Rock		TION SKETCH -		ACTIVITY (∠) -						
				NORTH		ODIFICATION/REPAIR						
215 225	Red Ash	<u>C</u>	<u>ار ا</u>			Deepen Other (Specify)						
225 340	Green & Black Ash	Yellow Clay	- Rugo CANYON PL									
					/ -	DESTROY (Describe Procedures and Materials Under "GEOLOGIC LOG"						
340 360	JRed Ash & Red Clay		. `	MS		SES (⊥) ATER SUPPLY						
360 500	Hard Gray & Black R	ock, some		3)	3 -	Formestic Public Irrigation Industriat						
· · · · · · · · · · · · · · · · · · ·	Yellow Ash		MEST	न /	า ั โล้ไ							
500 512	Pod Ach		>	ă S	Prives							
	Red Ash	· · · ·		-6 //	9	HEAT EXCHANGE						
512 630	Gray Rock					DIRECT PUSH INJECTION						
630 655	Gray Rock & Gray Cl	01/		Que								
	OLAY NOCK & GLAY OL	ay	Wustrate or Describe Dist	SOUTH		SPARGING REMEDIATION						
655 715	Green & Gray Ash	•	Fences, Rivers, etc. and at necessary. PLEASE BE A	ance of Well from Roads, I tach a map. Use additional CCURATE & COMPLET	paper if TE.	OTHER (SPECIFY)						
715 730	White Ash			EVEL & YIELD OF	COMPLET	ED WELL						
			DEPTH TO FIRST WATE	R 770 (Ft.) BELO	W SURFACE							
730 750 750 790	Gray & Green Shale Green Shale		DEPTH OF STATIC			-29-07						
TOTAL DEPTH OF			ESTIMATED YIELD	(GPM) & TES . (Hrs.) TOTAL DRAWDO		(FL) CPMat day						
	COMPLETED WELL 750 (Feet)		tative of a well's long-t		of test						
DEPTH	BODE	CASING (S)		DEPTH	ANNUL	AR MATERIAL						
FROM SURFACE	BORE-, HOLE <u>TYPE (∽)</u> DIA. <u>× ⊼ , ≅ </u> MATERIAL /	INTERNAL GAUGE	1 11	FROM SURFACE	E- BEN-	ТҮРЕ						
Ft. to Ft.	DIA. (Inches)	DIAMETER OR WAL (inches) THICKNE	LL. IF. ANY	Ft. to Ft. MI	ENT TONITE FI	ILL FILTER PACK (TYPE/SIZE)						
0:30	11 V Plact		80	R : 26		<u> </u>						
30 300	9 9 - 10	<u> </u>		26 750	- L	Vel tack						
300 750	9 V n	p 20.	0032			U-PAU CI						
· · · · · · · · · · · · · · · · · · ·												
Geologic	HMENTS (∠) I, the un	ndersigned, certify that t	CERTIFICATIOn CERTIFICATIFICATION CERTIFICATION CERTIFICATIFICATIFICA	ON STATEMENT • nd accurate to the bes	st of my know	vledge and belief.						
-		Pulliam Well F ERSON, FIRM, OR CORPORATION)	Exploration									
Soit/Wate	er Chemical AnalysesADDRESS	<u>lO Highway 128</u>	Napa	<u>CA 9455</u> city	SI	TATE ZIP						
	INFORMATION, IF IT EXISTS. Signed	Jompse	lleg 2	4:-	-1-07	808-508						
DWR 188 REV. 05-03		SZ LICENSED WATER WELL CON IS NEEDED, USE NEX			IGALD	C-57 LICENSE NUMBER						

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-	DRIGINA File with						WELI		of cali PLETI		IA	Г		ZINI	04	W	018	
F	Page 1	_ of1						Refer to In		Pan	ipblet		<u>ا</u>		STATE	WELL N	10./STA	TION NO.
(1) Wher's V Date Wor	k Began.	1 2/27/	95		,		31/95	• 54	17	43 3			LATTUDE				
\	Local F	Permit Ag	ency <u>N</u>	apa	Co	unt	y Environ	mental	<u>Healt</u>	<u>h</u>		_			<u> </u>		11	╘╦╍╄╌╴╵╶╌╹╌╾┺╼╌┦╿
	Pern	ait No. 🔔					Permit	Date	<u>2/17/9</u>	5						APN/IH	S/OTHE	
ſ	ORIENTAT						LOG	(SPECIF									
┟	DEPTH	FROM	DEPTH	TO	FIRST		ER(Ft)	BELOW SUI	RFACE									-1
ł	Ft. te	PACE		r	Tecril		SCRIPTION terial, grain size, co	alor etc ($\langle \langle \rangle \rangle_{2}$	-0117			(÷	—
ŀ	0		Boulder			/ //	array grant and, to	1.1.6	C. Star	100		Sa		ell (ío anyon	Road	d –		
t	5		Grav no	_	frac	:t.	6		-1 6	Git	C+ U			S.		×		
ľ	20	40	Red roc			~ ~ ~	N 15-	Sector and the sector of the s			mty Nap							
	40	5	Gray ro	ck,	bro	wn/3	<u>ail (7)</u>	<u></u>		AP	N Book 32	<u>}</u>	Page	010	Parcel	6	1	
ļ	55	70			_	_	ers brown so				wiship	!	Range .		Section	n		
ŀ	70	85		<u> </u>			<u>(rock soft</u>		1	Lat	itude	MIN.		NOHTH]	Longi	tude _	DEG.	MIN. SEC.
┟	85		Dk brow		_			A State of the second s	<u> </u>	LOCATION SRETCH						CTIVITY (∠) -		
┟	100	115	BROWN 8			S	(soft)	have NORTH									NEW WELL	
ŀ	<u>115</u> 145	<u>145</u> 160	<u>DK red.</u> Black 8	<u>Dig(</u>	- 62 -	1	wn rock han								MODU	FICATION/REPAIR		
ł	140	4				_	i rock fract	<u></u>		Sage Conyou Rd.					1	Deepen		
ł	175	199					fract med	·		1								Other (Specify)
ľ	190	220				12	ock fract			1		DESTROY						
	220	250	Grav ro	6		12]				3(Procedures and Materials Under "GEOLOGIC LOG")
Ľ	250	280	Grav &	blax	жr	ock	hand fract											ANNED USE(S)
ļ	280	295	Grav &	red	roc	<u>k ha</u>	<u>ind & fract</u>										(⊻) MONITORING	
ŀ	295						ock fract			1	G 3			· · }	3		WATE	ir Supply
ļ	310		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				<u>in rock hard</u>	fract			5			_ <u>}</u> •	2000	~	Į	X Domestic
	355		Grav & I	brov			hard fract			l T	7	-		1	5)		Public
	400	415	Ked, DI	<u>ack</u>	_		gray rock	hard		/		٩.		20.	K S		1	irrigation
7	<u>415</u> 445	<u>445</u> 460	Gray ro		1901-90		ract vrock frac	, t have	÷	ľ	₹ø	nor A	۱.		30			, Industriai
ŀ	460		Black n	_				<u>L IIdru</u>				م			8 g		-	"TEST WELL"
ţ	505				_		and fract	- <u> </u>					SOUTH	C 987-17 6			ł -	CATHODIC PROTEC- TION
ŀ	550			_			s black har	d		\$U	ustrate or Descri ch as Roads, Bati	ding	, Fences	Rivers, etc	2	TILITKS		- OTHER (Specify)
Ī	605	_			-	_	med hard				EASE BE ALL	_						
	625						s black med						otes		I	FLUID _	Wte	<u>r foam</u>
ļ							·				WATER	LEV						
ł		I 							_ _	WA	TH OF STATIC	40	<u>0</u>	(Ft.) & D4	TE ME	ASURE	n <u>3</u>	/30/95
┟		· · · · -		<u>cc</u> r						EST	IMATED YIELD		<u>.u </u>	(GPM) & '	test t	YPE	Air	<u>_</u>
		EPTH OF				_ (Fe	Μ			TES	T LENGTH	2 ((Hra.) TC				up Le	Ele 👗
Ł	TOTAL D	EPTH OF	COMPLETI	ED W	/ELL		<u>ZU (Feet)</u>			<u>*</u> M	lay not be repre	senta	tive of a	well's lon	g-term	yield.		
ſ	DEF						· C	ASING(S))				DEP	тн	· 1	NNU	LAR	MATERIAL
1	FROM		BORE- HOLE	ΤY	PE (2	<u>1</u>		INTERNAL	GAUG	=	SLOT SIZE	F	ROM SU	IRFACE	— —		TY	(PE
ļ	E4 ·		DIA. (Inches)	N.	SCREEN CON-	195 195	MATERIAL/ GRADE	DIAMETER	OR WA	LL	IF ANY				CE- MENT	BEN- TONITE	FILL	FILTER PACK
	Ft. to		9 7/8"		_	리린		(Inches)			(inches)		Ft. ño	Ft.		(土)		(TYRE/SIZE)
	620	420			<u>X _</u>	┼╌┡	<u> I-C-1 </u>	6	F-480		.032	Ľ.	665	<u>; 30</u>	ļ		X	Pea_gravel
·	- 420	1	40 - 41	X	+-	+	<u>I-C-1</u>	6	F-480			┣	30	29		X		
┢		1	12 3/4	2 3/4 X							·	 	<u>29 ¦</u>	1	×.			├ ──── ─ ──
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Ļ		ATTACE	IMENTS			<u>i </u>		L		=	CERTIFICA	L FIO	N STA	TEMEN	<u>т</u>	!		f
		Geologia		, <u> </u>	•	•	I, the unde	rsigned, ce	rtify that i							at of any	y know	ledge and belief.
	-	-	struction Dia	otom			NAME	Dosh	ier-G	rea	son Inc.							220
			ical Log(s)	मि। मारा							SON Inc.							
	_	· ·	er Chemical	Analy	888		5365	<u>Napa V</u>	<u>allej</u>	<u>р Н</u>	<u>ighway, A</u>	\me	<u>rica</u> r	<u>Canv</u>	on.	CA.	945	89
	-	Other					- II <i>II</i> .			<i>{</i>]	14.		-					1
ļ	ATTACH A	DDITIONAL	INFORMATIC)N.)≓	÷ <i>⊨</i> ∈	XISTS	Signer	MAN ANTHER AUTH	MU -	Ľ	BSW	<u>ب</u>			3/3	0/95	<u>. </u>	258826
L D	WR 188 REV	7.7-90	. <u> </u>	IF A	DDI		AL SPACE IS		_	_		NU	MBERE			~~		<u> 03010001 20002000 1997</u>
					1													

		ELE	Well 7						
ORIGINAL	STATE OF C	e of California Do not fill in							
File with DWR		CES AGENCY	No. 120020						
			NU. 120020						
e of Intent No	_ WATER WELL D	KILLERS REPORT	State Well No.						
Permit No, or Date	-		Other Well No.07/04008						
		(12) WELL LOG: Tank	lepth 710 ft. Depth of completed well 700 ft.						
			escribe by color, character, size or material)						
(-	<u>0 - 30 Br. rock</u>	hard						
(2) LOCATION OF WELL (Se	e instructions):		ck rock m-hard						
	Owner's Well Number_032-010-27		k some red hard						
Well address if different from above TownshipRangeRange	W Section 08		k hand to m-hard						
Distance from cities, roads, railroads, fences, e		206 - 237 Black & b	rown rock m-hard						
		237 - 263 Black	some stringers of red rock						
		<u>263 - 285 Black & </u>	and green rock stringers of brown &						
<u> </u>		285 - 340 Rhack roc							
(个	(3) TYPE OF WORK:		with hard stringers of black rock						
Wet >E	New Well ⊠ Deepening □ Reconstruction □		k with hard stringers of green rock						
LAKE	Reconditioning		Kreek fractured						
HENNESSEY 5	Horizontal Well	604 - 611 Red rock							
-	Destruction _ (Describe destruction materials and procedures in Item 12	END- 670 Green roc							
h	procedures in Item 12		k with m-hand stringers of grey roc						
\sim	(4) PROPOSED USE	695 - 700 Soft gree	n pock (2) ~						
	Domestic D		D						
	Industrial	A - A	<u></u>						
well,		$\overline{(1)}$	♦						
	Stock	$(0) - (0)^{\circ}$							
OLD WELL	Municipal	-							
WELL LOCATION SKETCH	Other	P -58							
	GRAVED PACK:								
Rotary 🛛 Reverse 🗆 🖓 Rotary 🖾 Rotary									
//	preter of bore	$\mathbb{R}^{\mathbb{V}_{-}}$							
	PERFORATIONS:	<u>-</u>							
Steel 🗆 Plastic 🛛 Concrete Typ	be of pertendion or size of screen	9 -							
From To Dia. Gage or	From To See	-	· · · · · · · · · · · · · · · · · · ·						
ft ft ft val	ft. size	-							
	140 460 032 180 588 0 032	-							
	$\frac{100}{500}$	_							
(9) WELL SEAL:	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	_							
Was surface sanitary seal provided? Yes 🕅	No \Box If yes, to depth <u>25</u> ft.	-							
Were strata sealed against pollution? Yes Method of sealing Bentonite & Cons		- 12 16	90 Completed 1-14 1991						
Method of sealing <u>BENEONITE & LOFA</u> (10) WATER LEVELS:		Work started 12-16 19 WELL DRILLER'S STATEM							
Depth of first water, if known	<u>470</u> ft	This well was drilled under my just	IENI: righticition and this report is true to the best of my						
Standing level after well completion	<u>460ft</u> .		V-June						
	If yes, by whom? driller	SIGNED	(Well Driller)						
Type of test Pump	Bailer 🗍 Air lift 🕅	NAME Doshier-Greg	son, Inc.						
20 1	t. At end of test <u>400</u> _ft hours Water temperature	Address 5365 Napa-Va	llejo Hwy.						
	If yes, by whom?	CityVallejo, CA	zip94589-9679						
	If yes, attach copy to this report	License No258826	Date of this report1-21-91						

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<u>สรรสวัตร์สาวารค้าเจ็ชีสรวร สุขรัฐระการณ์ที่สรา</u>ย และสุขุญญาตรรร การการการและการการการการการการการการการการการก

DWR 188 (REV. 7-76) IF ADDITIONAL SPACE IS NEEDED, USE NEXT CONSECUTIVELY NUMBERED FORM

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ORIGINAL	STATE OF C	CALIFORNIA 08 N/P	Do not fill in
Pile suitale maaam	THE RESOUR	CES AGENCY	
		VATER RESOURCES	No. 120017
ve of Intent No	WATER WELL D	RILLERS REPORT	State Well No.
Permit No. or Date			Other Well No. 07N04W08N
1677 suge chuy0 (032.	010-091)	(12) WELL LOG: Total diftion ft. to ft. Formation (De	epth 610 ft. Depth of completed well 610 ft. scribe by color, character, size or material)
	v .	2 - 20 Red & Brow	n clay
(2) LOCATION OF WELL (See instru CountyNapaOwner's	ctions): 22 010 20		ack rock med hard ack rock stringer brown clay
Well address if different from above Pritchard	Well Number 32-010-29 Hill		in a ked rock med hard
TownshipRange	Section	100 04001 0104	ad nock hand fract
Distance from cities, roads, railroads, fences, etc.		160 - 220 Black Red	& gray rock hard fract
		220 - 265 Brown hear	black rock hard fract
		265 - 295 (Black brok) 295 - 390 Black grav	n red rock hard fract
N	(3) TYPE OF WORK:		red rock stringers dark & light red rock hard fract brown
	Now Well Z Deepening		ok. green brown rock hard fract
Pre	Reconstruction	600 - 640 Gray rock	hand
E PPITA	Reconditioning	all - C	×
CHARD	Horizontal Well	- 1991 1991 C	2
W Gate) PRITCHARD	Destruction [] (Describe destruction materials and procedures in Item 12		
N Gate) E	(4) PROPOSED USE?	- 60	- CO
b) Fence	Domestic		
) (Stande	Irrigation (alto A	<u>ک</u>
	Industrial	<u>(1)-~ (1)</u>	
@K50 fr	Test Well	\mathbb{N}^{-}	
	Municipal		
WELL LOCATION SKETCH	Other 🔘 🗆	<u> </u>	
(5) EQUIPMENT: (8) GRAVE			
	Size Pea		- · · · · · · · · · · · · · · · · · · ·
Cable _ Air 20 Dibutter of b Other _ Mid Bucket _ Packed from	610 25	$\mathbb{A}^{\mathbb{W}^{-}}$	<u></u>
Other Mid Bucket Packed from (7) CASING INSTALLED: (8) PERFOR			
	andon or size of screen		····
From To Dia. Cape or From	D To Sha	-	
ft. ft. Wall ft.	ft.		
0 480 6 200 480	610 032	-	
	- all the	-	
(9) WELL SEAL:	- Allo		
Was surface sanitary seal provided? Yes 🕅 No 🗋	If yes, to depth 25 ft.	_	
Were strata scaled against pollution? Yes . N Method of scaling Bentonite Pellets &	concrete		Completed 1-8 19.91
(10) WATER LEVELS:		Work started 12-14 19.94 WELL DRILLER'S STATEM	
Depth of first water, if known	<u>495</u>	This well was drilled under my juri knowledge and peliej.	adjetion any this report is true to the best of my
Standing level after well completion(11) WELL TESTS:	,n.	SIGNED AUMONO	Stebsur
Was well test made? Yes 🖄 No 🗌 H yes, b Type of test Pump 🗋 Bailer 🕞	whom? driller	Dechion Or	(Well Driller)
Depth to water at start of test 495 ft.	At end of testft	NAME Doshier - Gre (Person, firm, or c	convoration) (Typed or printed)
parge 50 gal/min after 3 bours	Water temperature	Address 5365 Napa-Val City Vallejo, CA	<u>710</u> 94589–9679
nical analysis made? Yes 🔯 – No 🗖 If yes, b		250026	
Was electric log made? Yes D No X If yes, at	tach copy to this report	Liernie No	wate of this report

2.14

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يوالحا الجراب والارتجاب فالتنجيل فيتحصب المراجع فتصور وهوما مرود ومواليا الترورك والعادي لحادثت المتعق

DWR 188 (REV. 7-76) IF ADDITIONAL SPACE IS NEEDED. USE NEXT CONSECUTIVELY NUMBERED FORM



ORIGINAL

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Well 9 Do not fill in

No. 245581

File with DWR

of Intent No.

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STATE OF CALIFORNIA THE RESOURCES AGENCY DEPARTMENT OF WATER RESOURCES WATER WELL DRILLERS REPORT

State Well No.

Permit No. or Date $\neq \mathcal{R}^{\dagger}$	CHARE HIL Other Well No. OTNO4W08
(
	(12) WELL LOG: Total depth <u>665</u> ft. Depth of completed well <u>650</u> ft. from ft. to ft. Formation (Describe by color, character, size or material)
· (0 - 1 Top Soil
	<u>1 - 3 Gray rock hard</u>
(2) LOCATION OF WELL (See instructions): CountyNapaOwner's Well Number32-010-27	<u>3 - 20</u> Red rock stringer brown clay
Well address if different from above	20 - 40 Red and black hard
TownshipSt. Helena RangeSection	40 - 55 Red rock med hard
Distance from cities, roads, railroads, fences, etc	55 - 85 Black red and gray rock med hard
	85 - 145 Red black rock med hard
	145 - 175 (Brown and gray rock hard
	175 - 245 Black brown gray hard
(3) TYPE OF WORK:	245 305 Black and brown rock hard
New Well Deepening	305 325 Dk red rock hard fract
(3) TYPE OF WORK: New Well & Deepening [] Reconstruction []	325 - 365 Dk red black and brown rock hard fract
Reconstruction □ Horizontal Well □ Destruction □ (Describe	.365 -425 Black rock hard fract
Horizontal Well	425 -485 Dk red and black rock hard fract
CHAR PLAN Destruction I (Describe destruction materials and procedures in Item 12)	485 -505 Dk red bock med hard fract
procedures in Item 12	505 - 525 Green white and gray tock med hard fract
(4) PROPOSED USE	stringers black rock
Domestic Domestic	525 585 Et green and gray hard fract
Irrigation	585 615 Gray rock hard
- Industrial	615 -650 Black red Green gray stringers white rock soft
Test Well 🗸 🗆	650 \bigvee -665 Black green white stringers shale
SOLE CANYON ROAD Stock	
Sole Municipal	
WELL LOCATION SKETCH	<u> </u>
(5) EQUIPMENT: (6) GRAVEL PACK:	
Rotary Reverse Reverse Size Real	
	\mathbb{Q}
(7) CASING INSTALLED: (8) PERFORATIONS: Machine	<u></u>
Steel Plastic A Concrete Type of performing or size of screen	<u> </u>
From To Dia. Cage of From To Slot	-
ft. ft. Wall ft ft.	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	-
<u>500 520 6 200 520 650 .032</u>	
(9) WELL SEAL: Was surface sanitary seal provided? Yes X No _ If yes, to depth26_ft.	
•	
Were strata sealed against pollution? Yes No D Intervalft. Method of sealing Bentinite Pellets and Concrete	Work started 7/1/188 Completed 7/12/88 19
(10) WATER LEVELS:	WELL DRILLER'S STATEMENT:
Depth of first water, if knownft.	This well was drilled under my initialities and this report is true to the best of my
Standing level after well completionft_	knowledge and belief.
(11) WELL TESTS: Was well test made? Yes Xi No D If yes by phom? Driller	SIGNED (Well Driller)
Type of test Pump \Box Bailer \Box Air lift \mathbf{X}	NAME Doshier-Gregson, Inc.
Depth to water at start of test 420 ft. At end of test 650 ft	(Person, firm, or corporation) (Typed or printed)
arge 20 gal/min after 4 hours Water temperature	Valleio – 0/590
fcal analysis made? Yes 🗆 No 🕅 If yes, by whom?	258826 7/1//88
Was electric log made? Yes D No X If yes, attach copy to this report	License NoDate of this report //14/00

DWR 188 (REV. 7-76) IF ADDITIONAL SPACE IS NEEDED. USE NEXT CONSECUTIVELY NUMBERED FORM

ORIGINAL		، متناب با تتن			DWB US	E ONLY	vo Well 10						
File with DWR	,	WELL COM		ON REPOR	T DITINI	24/101							
Page of		=	Instruction		s	TATE WELL NO	D./STATION NO.						
Owner's Well No.	05/16/2008E			73633									
U	ency <u>Napa Count</u>												
Permit NoE		Permit Date	03/21/			APN/TRS/	OTHER						
	GEOLOGIC L			ACTION	WELL O	OWNER —							
ORIENTATION (∠)	XX_venticalHORIZ DRILLING Rotary		(SPECIFY)	Name Mailing ^A									
DEPTH FROM SURFACE		GCRIPTION											
Ft. to Ft.	Describe materia	al, grain size, color,	1 4 3 4 4		WELL LO	OCATION	TE ZIP						
		d Ash all	NOV C	Address 1677	lená VOV	Road							
	Red Clay & Re	$\frac{\operatorname{Asn}}{\sqrt{2}}$	1/2	County Napa									
25 70	Brown Clay &	hard Gray Ro		APN Book 032	<u>V</u> Page 0.30	Parcel _04	+3-000						
	· · · · · · · · · · · · · · · · · · ·			Township 1 Range Section									
70 - 115	Hard Gray Vol	canic Rock	10	The sector of									
	Brown Ash	1-13-	<u></u>										
			NU/V	MODIFICATION/REPAIR									
150 255	Hard Gray & G	teen Volcani	C_KOCK_	- Despen - Other (Specify) - DESTROY (Describe Procedures and Materials									
255 290-	->Red_Brown_8	White Ash		K traile - DESTROY (Describe									
	- AL	14-		Under "GEOLOGIC LOG									
290 360	Hard Gray Roc	k & Red Volv	anic_As	Sage Ca	NYON-RA		USES (∠) WATER SUPPLY						
360 440	Hard Gray Roc						Domestic Public						
				ment ment	752	EAST							
440 495	Red Ash & Gra	ay Rock		1 71	5	·	CATHODIC PROTECTION						
495 530	Hard Gray_Roc	k					HEAT EXCHANGE						
				N 1	\mathcal{T}		DIRECT PUSH						
<u>530590 -</u>	Red & Green V	/olcanic_Rock		1	No inclu		VAPOR EXTRACTION						
590 660		ock		Illustrate or Describe I	SOUTH SOUTH	ds. Buildings.	REMEDIATION						
				Fences, Rivers, etc. and necessary. PLEASE Bi	l attach a map. Use addit E ACCURATE & COMP	ional paper if LETE.	OTHER (SPECIFY)						
- 660 - 755	Hard Black &	Green Rock			LEVEL & YIELD								
					ATER 600 (FL) BE	ELOW SURFAC	E						
				DEPTH OF STATIC WATER LEVEL	565_(Ft.) & DATE	MEASURED _	5-28-08						
	766			ESTIMATED YIELD	_45 (GPM) &	TEST TYPE	in liFT						
TOTAL DEPTH OF I	BORING <u>755</u> (Feet) COMPLETED WELL <u>75</u>				(Hrs.) TOTAL DRAW sentative of a well's lor		- (Ft.) GPM at day of test						
		· · · · ·	(6)	i in the second se		<u> </u>							
DEPTH FROM SURFACE	BORE- HOLE TYPE (∠)	CASING	(3)		DEPTH FROM SURFACE	ANN	ULAR MATERIAL TYPE						
		MATERIAL / INTERN GRADE DIAMET	ER OR WA	LL, IF ANY		CE- BEN-	FILL FILTER PACK						
Ft. to Ft.		(Inches		SS (Inches)	Ft. to Ft.	(⊻) (¥)	(⊻) (TYPE/SIZE)						
0 53	12 V P	lastic 6	F46	0	0 53								
53 60 R	9 1	n n	11		53 750	·	Well Pack						
							Gravel						
600 755	4 V	<u>p 7</u>	61	.030									
ATTACH	IMENTS (∠)	<u> </u>		CERTIFICAT	I ION STATEMENT	<u> </u>	<u> </u>						
Geologic	Log	*		his report is complete			nowledge and belief.						
	struction Diagram	NAME Pulli	am Well	Exploration									
1	cal Log(s) r Chemical Analyses				Vapa, CA 94	4558							
Soli/Wate		ADDRESS	DA	/ .		<u></u>	STATE ZIP						
ATTACH ADDITIONAL II	NFORMATION, IF IT EXISTS.	Signed C-57 LICENSED	ATER WELL CON	TRACTOR		5-30-6	808-508						
DWR 188 REV. 05-03	IF ADDITIO	NAL SPACE IS NEED					OSP 03 7883						

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Page 1		of 1	<u> </u>		W		mpleti In Indrudion		Repo	π		1	1			
Owner's	Wall Num	ber K	rupp Knief #2				e026057				[]	Stat		mpar/Si	le Number	
Date Wo	rk Began	03/05	/2015	Data	Work En	ted <u>3/25</u>				- 1		Latitude			Longituda	
			nning, Building	and Envi	ronment	8!				1						
Permit N	umber E	14-005	<u>578 </u>	Permit D	ite <u>7/21</u>	14				L			APN	RS/OII	ê/	
			Geolo	gic Log				זר				Well	Owner			Î
Orle	Intation	⊙ Vai			OAngle	Specif	y	15	Jame K	rupp Vin	evards					
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	from Sul 0 Fe		David		cription			11	City nap				Sta	to C8	Zip 94558	
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300	400		soft yellow ast					-114	alitude	Deg.	-		N Longitu	ide	Win Sec.	
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										first water	240			_ (Fee	t below surface)	
									Depth to Mater Le	Stalic vel 240)	1500	t) Date	Меази	red 03/26/2015	
Total D	epth of B	orino	590			Feet		_		d Yield '					Air Lift	
1		-								oth 2.0						
Tolal D	epth of C	omplet	ed Well <u>560</u>			_ Feet				be repres						
			······	Car	lage	_				and topical	1		Annul			i
Dent	h from	Bareh	olo _	Cas		Wall	Öutalde	6	croen	Siot Size	Depti	h frem	AIRIU			
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and the second se	lon Faet	(Inche	1			(Inches)	(Inchee)	T		(Inchos)		b Foet	Bostonit		coal	
0	20	14	Blank	PVC Sch. 8			8	<u> </u>			0	20	Bentonite Filter Por		seal pea gravel	1
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	Geologic				I, the un	densigned	, certify th	at thi					the bes	t of my	knowledge and belief	1
-	*	~	n Diagram		Name	D. Bess (<u>y & amuc</u>	Vell			-	_	_			1
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			nical Analyses				Agoresa				City		5	ala	Ζφ	1
	Other 🔔				Signed .	<u> </u>	and	12						87027		
Anoch add	Toch schlannel Information, if it exerts C-57 Licensed W						need Water	Well Co	ontrector			Date Sig	ned C	-57 Uc	ense Number	J .

Artigch additional Information, if it extension DWR 168 REV 1/2006

IF ADDITIONAL SPACE IS NEEDED, USE NEXT CONSECUTIVELY NUMBERED FORM

State of California Well Completion Report Form DWR 188 Submitted 11/27/2018 WCR2018-010626

Owner's V	Vell Numb	per 032-010-094-000 Date Work Began 09/26/2018 Date Work Ended 10/06/2018
Local Pen	mit Agenc	y Napa County Planning Building and Environmental Services
Secondar	y Permit A	Agency Permit Number e18-00795 Permit Date 10/08/2018
Well C	Dwner (must remain confidential pursuant to Water Code 13752) Planned Use and Activity
Mailing A	ddress	1581 SAGE CANYON ROAD Drill and Destroy
		Planned Use Destruction
City ST	. HELENA	A State CA Zip 94574
		Well Location
Address	1581 9	APN 032-010-094-000
	ST HELEN	Tourship 07 N
_		Paper 04 W
Latitude	38	29 12,3871 N Longitude -122 21 12,4804 W Section 06
	Deg.	Min. Sec. Deg. Min. Sec. Baseline Meridian Mount Diablo
Dec. Lat.		Ground Surface Elevation
Vertical C		Horizontal Datum WGS84 Elevation Accuracy
Location /	Accuracy	Location Determination MethodElevation Determination Method
	-	Borehole Information Water Level and Yield of Completed Well
Orientatio	on Verti	cal Specify Depth to first water 0 (Feet below surface)
Drilling M		Direct Rotary Drilling Fluid Air
Britteng ivi		Water Level 0 (Feet) Date Measured 10/06/2018
Total Dep	oth of Bori	ng 780 Feet Estimated Yield* 0 (GPM) Test Type Air Lift
Total Dep	oth of Corr	Test Length 0 (Hours) Total Drawdown 0 (feet) *May not be representative of a well's long term yield.
		may not be representative of a wears long term yield.
		Geologic Log - Free Form
Depth Surfa Feet to	ace	Description
0	40	RED ASH CLAY
40	80	HARD BLACK BASALT
80	120	BROWN ASH
120	200	HARD GREY ROCK
200	220	BROWN ASH
220	420	HARD GREY ROCK
420	440	RED ASH
440	620	GREEN HARD FRACTURED ROCK
620	660	BLACK BASALT FRACTURED ROCK
660	750	GREEN SURPINTINE
750	780	GREEN SERPINTINE

2 3

WELL 13

					Casing	s								
Casing #	Depth from Feet to		Casing Type	Material	Casings Specificatons	Wall Thickness (Inches)	Outside Diameter (inches)	Screen Type	Slot Size If any (Inchos)	Desc	ription			
					Annular Ma	terial			<u>1</u>					
Sur	face	Fill		Fill	Type Details		Filter Pack	Size		Description	1			
0	780	Filter P	ack Other Gr	ravel Pack		6			BIRDS EYE	GRAVEL				
NFFD	FD MATER	IAI .			EPTH, CEMENT TO FRO			DAGATIL						
	E	loreho	le Specifica	ations			Certific	ation S	Statement					
Su	th from Inface t to Feet		Borehole Dia	meter (inches) Name									
0	780	12			11	Person, Firm or Corporation 1663 HOWELL MTN RD ANGWIN C								
					Signed	Address C-57 Licensed	suffere for	Contractor	City 11/27/2018 Date Signed		Zip 08508 ense Number			
						DWR Use Only								
					CSG #	State Well	Number	S	ite Code	Local W	ell Number			
					Lati TRS: APN:	tude Deg/	· /Min/Sec	N [Longitud	l le Deg/Mi	w m/Sec			

State of California Well Completion Report Form DWR 188 Submitted 8/23/2019 WCR2019-011930

Owner's W	ell Numb	er 032-010-094-000 Date Work B	Degan 07/17/2019 Date Work Ended 07/30/2019
Local Perm	nit Agency	y Napa County Planning Building and Environmental S	iervices
Secondary	Permit A	gency Permit N	umber E19-00154 Permit Date 07/26/2019
Well O	wner (must remain confidential pursuant to V	Vater Code 13752) Planned Use and Activity
Name (CHAPPEL	LLET VINEYARD LLC,	Activity New Well
Mailing Ad	Idress	1581 SAGE CANYON ROAD	
			Agriculture
City ST.	HELENA	A State C	CA Zip 94574
		Well	Location
Address	1581 S	AGE CANYON RD	APN 032-010-094-000
City ST	THELEN	A Zin 04574 County	Nena Township 07 N
Latitude			Panas 04 W
-			Section 06
Dec. Lat.			Baseline Meridian Mount Diablo
			Ground Sunace Elevation
			Lievation Accuracy
Location A	Accuracy	Location Determination Method	Elevation Determination Mathod
	Parmit Agency Napa County Planning Building and Environmental Barvices Disk tructured United		
Orientation	n Vertic	cal Specify	
Drilling Me	thod D	Direct Rotary Dritting Fluid Air	
Total Dept	th of Borin	ng 670 Feet	
Total Dept	th of Com	pleted Well 670 Feet	
		Geologic	
Depth 5	tom 1	Geologic L	by -rice roim
Surfa	ce		Description
0	60	HARD GREY BOULDERS AND BROW CLAY	
60	140	HARD GREY ROCK	
140	160	RED BLACK ASH	
160	300	BLACK BASALT	
300	560	HARD GREEN ROCK	
560	580	BLACK BASALT	
580	600	BLACK RED VOLCANICS	
600	620	GREEN ROCK GREEN SURPINTINE	
620	640	YELLOW ROCK RUST SIGNS	
640	660	WHITE ASH	
660	670	GREEN SURPINTINE	

WELL 14

-					Casing	5				and the second sec
Seeing	Depth from Sur Feet to Feet		sing Type	Material	Casings Specifications	Wall Thickness (inches)	Outside Diameter (inches)	Screen Type	Slot Size if any (inches)	Description
1	0 4	io Bla	ink	PVC	OD: 6.625 in. SDR: 21 Thickness: 0.316 in.	0.316	6.625			*
1	450 4	0 Sc	reen	PVC	OD: 6.625 in. SDR: 21 Thickness: 0.316 in.	0.316	6.625	Milled Slots	0.032	
1	470 49	IO Bla	ink	PVC	OD: 6.625 in. SDR: 21 Thickness: 0,316 in.	0,316	6,625			
1	490 5	0 Sc	reen	PVC	OD: 6.625 in. SDR: 21 Thickness: 0.316 In.	0.316	6.625	Milled Slots	0.032	
1	510 5	io Bla	nk	PVC	OD: 6,625 in. SDR: 21 Thickness: 0.316 in.	0.316	6.625			
1	530 54	i0 Sa	reen	PVC	OD: 6.625 in. SDR: 21 Thickness: 0.316 in.	0.316	6.625	Milled Slots	0.032	
1	550 51	0 Bla	nk	PVC	OD: 6.625 in. SDR: 21 Thickness: 0.316 in.	0.316	6.625	-		
1	570 50	10 Sc	reen	PVC	OD: 6.625 in. SDR: 21 Thickness: 0.316 in.	0.316	6.625	Mitted Slots	0.032	
1	590 61	0 Bla	nk	PVC	OD: 3.600 In. Thickness: 0.300 In.	0.3	3.5			
1	610 63	0 Sc	reen	PVC	OD: 6.625 In. SDR: 21 Thickness: 0.316 In.	0.316	6.625	Milled Slots	0.032	
1	630 65	i0 Bla	nk	PVC	OD: 6.625 in. SDR: 21 Thickness: 0.316 in.	0.316	6.625			
1	650 67	0 Sci	reen	PVC	OD: 6.625 in. SDR: 21 Thickness: 0.316 in.	0.316	6.625	Milled Slots	0.032	
			1		Annular Mat	terial				
Sur	n from face to Feet	Fill		Fill	Type Details		Filter Pack Size		Description	
0	55 0	ement	10.3 Sac	k Mix						
0	670 FI	ter Pack	Other Gr	avel Pack		10			Bird's Eye Gr	avel Well Pack

-	Borehole Specifications			Certif	ication	n Sta	tement			
Depth from Surface Feet to Feet	Borehole Diameter (Inches)	I, the undersigned, certify that this report is complete and accurate to the best of my knowledge and belief Name PULLIAM WELL EXPLORATION INC								lef
0 5	12		Person, Fin		notion		NGWIN	~		
55 67	12		electronic	55			City 08/23/2019 Date Signed	CA State	308508	
		CSG#	DWR Us				site Code Local Well Nu			umber
					N				1	w
		Latitude Deg/Min/Sec Longitude Deg/Min/ TRS: APN:							lin/So	BC

WELL 15

State of California

Well Completion Report Form DWR 188 Submitted 9/9/2021 WCR2021-011524

Owner's V	Vell Numb	
Local Perr	mit Agency	y Napa County Planning Building and Environmental Services
Secondary	y Permit A	Agency Permit Number E21-00234 Permit Date 07/30/2021
Well C	Owner (must remain confidential pursuant to Water Code 13752) Planned Use and Activity
Name	CHAPPEL	LLET VINEYARD, LLC, Activity New Well
Mailing A	ddress	1581 Sage Canyon Road Planned Use Water Supply Irrigation -
		Agriculture
City St.	. Helena	State Ca Zip 94574
		Well Location
Address	1581 S	Sage Canyon RD APN 032-560-022
City S	St. Helena	Zip 94574 County Napa Township 07 N
Latitude	38	28 11 N Longitude -122 20 6 W Range 04 W
	Deg.	Min Section 08
Dec. Lat.	_	Baseline Meridian Mount Diablo
Vertical D		Horizontal Datum WGS84 Elevation Accuracy
	Accuracy	Location Determination Method Elevation Determination Method
Location	Hoodracy	
		Borehole Information Water Level and Yield of Completed Well
Orientatio	on Vertie	
Drilling M	lethod D	Direct Rotary Drilling Fluid Air Depth to Static
		Water Level 535 (Feet) Date Measured 09/07/2021 Estimated Yield* 125 (GPM) Test Type Air Lift
Total Dep	pth of Bori	ng 920 Feet Test Length 2 (Hours) Total Drawdown (feet)
Total Dep	pth of Corr	hpleted Well 890 Feet *May not be representative of a well's long term yield.
		Geologic Log - Free Form
Depth	from	
Surf Feet to		Description
0	2	brown clay with cobbles
2	25	brown clay
25	50	brown volcanics with tuff
50	55	soft mixed volcanics with red ash
55	68	50% dark gray volcanics / 50% brown volcanics
68	80	hard dark gray volcanics
80	128	hard brown volcanics
128	140	soft red volcanics
140	175	hard dark gray volcanics
175	230	soft mixed volcanics
230	300	dark gray volcanics
300	330	soft gray & red volcanics
330	448	hard gray volcanics
448	500	brown volcanics
500	520	soft gray & green volcanics
300	520	שייין איז

520	595	hard gray volcanics
595	605	soft gray volcanics
605	635	fractured gray volcanics
635	655	soft gray volcanics
655	675	hard gray volcanics
675	685	hard red volcanics
685	798	hard gray volcanics
798	800	red volcanics
800	815	brown volcanics
815	820	gray volcanics
820	840	soft gray volcanics
840	870	gray sandy tuff
870	875	gray ash
875	880	brown ash
880	920	80% shale / 20% sandstone

					Casing	s				
Casing #	Depth from Feet to		Casing 1	Type Material	Casings Specificatons	Wall Thickness (inches)	Outside Diameter (inches)	Screen Type	Slot Size if any (inches)	Description
1	0	530	Blank	PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625			
1	530	610	Screen	PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625	Milled Slots	0.032	
1	610	630	Blank	PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625			
1	630	750	Screen	PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625	Milled Slots	0.032	
1	750	770	Blank	PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625			
1	770	810	Screen	PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625	Milled Slots	0.032	
1	810	830	Blank	PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625			
1	830	870	Screen	PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625	Milled Slots	0.032	
1	870	890	Blank	PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625			
					Annular Ma	terial				
Su	n from rface to Feet	Fill		Fi	ll Type Details		Filter Pack	(Size		Description
0	58	Ceme	ent 1	0.3 Sack Mix						
58	400	Other	Fill S	ee description.					pea gravel	
400	890	Other	Fill S	ee description.					#6 sand	

	E	orehole Specifications		Certification	Statement		
Depth Surf Feet to	ace	Borehole Diameter (inches)	l, the under Name		accurate to the best of m WELL DRILLING I		and belief
0	900	12		Person, Firm or Corporation 994 KAISER ROAD	NAPA	CA	94558
900	920	9		Address	City	State	Zip
			Signed	Signed electronic signature received C-57 Licensed Water Well Contractor		439746 C-57 License Num	
				e Only			
			CSG #	Local W	Local Well Numbe		
				atitude Deg/Min/Sec	Longitude	Deg/Mi	Win/Sec
			TRS: APN:				

WELL 17

State of California

Well Completion Report Form DWR 188 Submitted 8/29/2019 WCR2019-012338

	9
Local Permit Agency Napa County Planning Building and Environmental Services	
Secondary Permit Agency Permit Number E16-00645 Permit Date 12/12/201	8
Well Owner (must remain confidential pursuant to Water Code 13752) Planned Use and Act	ivity
Name COLGIN CELLARS, Activity New Well	
Mailing Address P.O. Box 254 Planned Use Water Supply Irrigat	ico
Agriculture	011-
City St. Helena State CA Zip 94574	
Well Location	
Address 220 Long Ranch RD APN 032-010-070	
City St. Helena Zip 94574 County Napa Township 07 N	
Latitude 38 28 11 N Longitude -122 20 16 W Range 04 W	
Section 07	
Baseline Meridian Mount Diablo	
Ground Surface Elevation	
Vertical Datum Horizontal Datum WGS84 Elevation Accuracy	
Location Accuracy Location Determination Method Elevation Determination Method	
Borehole Information Water Level and Yield of Completed W	Vell
Orientation Vertical Specify Depth to first water 485 (Feet below surface)
Drilling Method Direct Rotary Drilling Fluid Air Depth to Static	
Water Level 429 (Feet) Date Measured C	8/28/2019
lotal Depth of Boring 840 Feet	Air Lift
Total Depth of Completed Well 820 Feet Test Length 2 (Hours) Total Drawdown *May not be representative of a well's long term yield.	(feet)
Geologic Log - Free Form	
Depth from Surface Description Feet to Feet	
0 8 boulder & red clay	
8 110 hard light gray volcanics	
110 115 hard fractured gray volcanics	
115 155 soft red volcanics	
155 180 hard black volcanics	
180 200 mixed volcanic sands	
200 230 soft tan, gray volcanics	
230 280 black, red volcanics	
280 485 hard gray volcanics	
485 525 red, tan volcanics	
525 560 light gray volcanics	
560 660 black, gray volcanics	
660 685 black, red volcanics	
685 700 black, green volcanics	

720	770	gray volcanics
770	785	soft red volcanics
785	800	light red, white volcanics
800	805	soft brown volcanics
805	810	soft green volcanics
810	815	soft gray volcanics
815	840	serpentine

						Casing	5				
Casing #	Depth from Feet to	n Surface 5 Feet	Casing	Туре	Material	Casings Specificatons	Wali Thickness (inches)	Outside Diameter (inches)	Screen Type	Slot Size if any (inches)	Description
1	0	500	Blank		PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625			
1	500	640	Screen		PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625	Milled Slots	0.032	
1	640	660	Blank		PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625			
1	660	740	Screen		PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625	Milled Slots	0.032	
1	740	760	Blank		PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625			
1	760	800	Screen		PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 In.	0.508	8.625	Milled Slots	0.032	
1	800	820	Blank		PVC	OD: 8.625 in. SDR: 17 Thickness: 0.508 in.	0.508	8.625			
						Annular Ma	terial				
Su	from face to Feet	Fill			Fill	Type Details		Filter Pack	Size		Description
0	56	Ceme	ent 1	0.3 Sac	k Mix						
56	200	Other	Fill S	See desc	cription.					pea gravel	
200	820	Other	Fill S	See desc	cription.					#6 sand	
820	840	Other	Fill S	See desc	cription.					cuttings	

	E	Borehole Specifications		Certifie	cation S	statement				
Depth Surfa	ace	Borehole Diameter (inches)	I, the undersigned, certify that this report is complete and accurate to the best of my knowledge and t Name HUCKFELDT WELL DRILLING INC							
0 55	55 825	15		Person, Firm or Corpora 994 KAISER ROAD	tion	NAPA	CA	94558		
55 825 825 840		9	_	Address		City	State	Zip		
			Signed	electronic signature re C-57 Licensed Water Well (08/29/2019 Date Signed	439746 C-57 License Num			
			DWR Use Only							
			CSG # State Well Number			te Code	Local W	Local Well Numbe		
		$\overline{\cdot}$		titude Deg/Min/Sec	N	Longitude	Deg/Mi	W W		
			TRS: APN:			Longhuot				

5

E	6	.00(e	46	w
	· \v			

State of California Well Completion Report Form DWR 188 Submitted 7/18/2019 WCR2019-009834

WELL 18

		WCR2019-009834										
Owner's Well N	lumber	1-2019 Date Work Began 07/03/2019 Date Work Ended 07/17/2019										
Local Permit Ag	gency	Napa County Planning Building and Environmental Services										
Secondary Perr	mit Ager	ncy Permit Number E16-00646 Permit Date 12/12/2018										
Well Own	or /m	ust remain confidential pursuant to Water Code 13752) Planned Use and Activity										
	GIN CEL											
Mailing Addres												
		Planned Use Water Supply Irrigation - Agriculture										
City St. Hele	ina —	State CA Zip 94574										
		Well Location										
Address												
City St.Hel		Zip 94574 County Napa Range 04 W										
	8	28 21 N Longitude -122 20 13 W Section 07										
De	-	Min. Sec. Deg. Min. Sec. Baseline Meridian Mount Diablo										
	.4725	Dec. Long122.3369444 Ground Surface Elevation										
Vertical Datum		Horizontal Datum WGS84 Elevation Accuracy										
Location Accur	racy -	Location Determination Method Elevation Determination Method										
	Borehole Information Water Level and Yield of Completed Well											
Orientation	Vertical	Specify Depth to first water 550 (Feet below surface)										
Drilling Method	d Dire	cct Rotary Drilling Fluid Air Depth to Static Water Level 541 (Feet) Date Measured 07/17/2019										
		Water Level 541 (Feet) Date Measured 07/17/2019 Estimated Yield* 38 (GPM) Test Type Air Lift										
Total Depth of	Boring	840 Feet Test Length 2 (Hours) Total Drawdown (feet)										
Total Depth of	Comple	eted Well 738 Feet •May not be representative of a well's long term yield.										
		Geologic Log - Free Form										
Depth from	1											
Surface Feet to Feet	t	Description										
0 2	0 lar	rge boulders with red clay										
20 7		ard black fractured volcanics										
72 9	5 tar	in sandy ash										
95 13	35 bia	ack volcanic rock										
135 14	i0 tai	n volcanic ash										
140 20	00 bla	ack volcanic rock										
200 26	50 rei	d volcanic rock										
260 37	75 ha	ard black volcanic rock										
375 42	20 rei	d volcanic rock										
420 46	50 ha	ard gray volcanic rock										
460 48	30 da	ark red, gray volcanics										
480 72	22 ha	ard black volcanic rock										
722 76	60 gr	reen volcanic ash										
760 77	75 tai	n volcanic ash										
775 78	30 gr	reen volcanic ash										

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	780	810	blue, gray volcanic ash
	810	830	sandstone
ſ	830	840	sandstone & shale

					Casing	S						
Casing #		m Surface to Feet	Casing Ty	/pe Material	Casings Specifications	Wall Thickness (inches)	Outside Diameter (inches)	Screen Type	Slot Size if any (inches)	Description		
1	0	398	Blank	PVC	OD: 6.625 in. SDR: 17 Thickness: 0.390 in.	0.39	6.625					
1	398	518	Blank	PVC	OD: 6.625 in. SDR: 0.3 21 Thickness: 0.316 in.		6.625					
1	518	618	Screen	PVC	OD: 6.625 in. SDR: 21 Thickness: 0.316 in.	0.316	6.625	Milled Slots	0.032			
1	618	638	Blank	PVC	OD: 6.625 in. SDR: 0.31 21 Thickness: 0.316 in.		6.625					
1	638	718	Screen	PVC	OD: 6.625 in. SDR: 21 Thickness: 0.316 in.	0.316	6.625	Milled Stots	0.032			
1	718	738	Blank	PVC	OD: 6.625 in. SDR; 21 Thickness: 0.316 in.	0.316	6.625					
					Annular Ma	terial		<u> </u>				
	from face	Fill		Fill	Type Details		Filter Pack	Size		Description		
0	76	Ceme	ent 10.	3 Sack Mix								
76	750	Filter P	ack Ott	ner Gravel Pack		#	6		sand			
750	840	Filter P	ack Oth	ner Gravel Pack					cuttings			

Other Observations:

	E	Boreho	ole Specifications			Certif	ication S	statement		
Depth	from			I, the under	signed, certily that	t this report is co	mplete and acci	urate to the best of m	iy knowledge a	ind belief
Surf			Borehole Diameter (inches)	Name		HUC		ELL DRILLING I	NC	
0	80	12			Person, Fir	m or Corpor	ation			
-					994 KAISEI	R ROAD		NAPA	CA	94558
80	840	9			Address			City	State	Zip
				Signed	-	signature n ed Water Well		07/18/2019 Date Signed		9746 ense Numbe
						D	WR Use	Only		
				CSG #	State We	ell Number	Si	te Code	Local W	ell Numbe
				I I I I I I I I I I I I I I I I I I I	g/Min/Sec	N	Longitude	Deg/Mi	Wn/Sec	
				TRS:						
				APN:						

APPENDIX B

CHAPPELLET IRRIGATION RECORDS 2017 - 2021



Chappellet Annual Irrigation Summary 2017-2021

		Irrigation	Hours				(Gallons/Block						Acre Feet/	Block				/	Acre Feet/	Acre				
	Acres	2017	2018	2019	2020	2021		2017	2018	2019	2020	2021		2017	2018	2019	2020	2021		2017	2018	2019	2020	2021	
CB4	3	71	83	63	77	118	CB4	196730	114990	87281	213355	326959	CB4	0.60	0.35	0.27	0.65	1.00	CB4	0.20	0.12	0.09	0.22	0.33	CB4
CF1	1.51	18	0	10	0	22	CF1	20912	0	11618	0	25559	CF1	0.06	0.00	0.04	0.00	0.08	CF1	0.04	0.00	0.02	0.00	0.05	CF1
CF8	3.14	110	105	107	83	52	CF8	455323	434626.5	442905.1	343561.9	215243.6	CF8	1.40	1.33	1.36	1.05	0.66	CF8	0.45	0.42	0.43	0.34	0.21	CF8
CS3	3.92	32	11	18	34	38	CS3	144804	49777	81453	153855	171955	CS3	0.44	0.15	0.25	0.47	0.53	CS3	0.11	0.04	0.06	0.12	0.13	CSE
CS4	4.37	19	17	12	19	19	CS4	79875	71467	50448	79875	79875	CS4	0.25	0.22	0.15	0.25	0.25	CS4	0.06	0.05	0.04	0.06	0.06	CS4
CS9	4.29	61	34	42	35	48	CS9	302092	168379	207997	173331	237711	CS9	0.93	0.52	0.64	0.53	0.73	CS9	0.22	0.12	0.15	0.12	0.17	CS
CS10	3.57	72	41	57	67	64	CS10	296732	168972	234913	276126	263762	CS10	0.91	0.52	0.72	0.85	0.81	CS10	0.26	0.15	0.20	0.24	0.23	CS1
CS11	1.47	73	72	77	75	51	CS11	123885	122188	130674	127280	86550	CS11	0.38	0.37	0.40	0.39	0.27	CS11	0.26	0.26	0.27	0.27	0.18	CS1
CS13	4.34	36	28	22	22	38	CS13	180459	140357	110280	110280	190484	CS13	0.55	0.43	0.34	0.34	0.58	CS13	0.13	0.10	0.08	0.08	0.13	CS
CS16	1.52	67	62	94	31	16	CS16	154824	143270	217215	71635	36973	CS16	0.48	0.44	0.67	0.22	0.11	CS16	0.31	0.29	0.44	0.14	0.07	CS2
CS17	3	28	29	33	31	20	CS17	77613	80385	91473	85929	55438	CS17	0.24	0.25	0.28	0.26	0.17	CS17	0.08	0.08	0.09	0.09	0.06	CS1
CS18	1	25	31	28	42	18	CS18	23135	28687	25911	38866	16657	CS18	0.07	0.09	0.08	0.12	0.05	CS18	0.07	0.09	0.08	0.12	0.05	CS1
CS19	2.7	69	79	55	29	32	CS19	215178	246363	171519	90437	99793	CS19	0.66	0.76	0.53	0.28	0.31	CS19	0.24	0.28	0.19	0.10	0.11	CS1
CS20	1.46	35	28	21	36	17	CS20	59026	47221	35416	60713	28670	CS20	0.18	0.14	0.11	0.19	0.09	CS20	0.12	0.10	0.07	0.13	0.06	CS2
CS21	2	33	28	21	36	25	CS21	75907	64406	48304	82807	57505	CS21	0.23	0.20	0.15	0.25	0.18	CS21	0.12	0.10	0.07	0.13	0.09	CS2
CS22	0.73	17	22	22	34	26	CS22	23913	30946	30946	47825	36572	CS22	0.07	0.09	0.09	0.15	0.11	CS22	0.10	0.13	0.13	0.20	0.15	CS2
CS23	0.72	19	36	21	48	38	CS23	26424	50066	29205	66755	52847	CS23	0.08	0.15	0.09	0.20	0.16	CS23	0.11	0.21	0.12	0.28	0.23	CS
CS24	0.6	19	21	0	16	24	CS24	22033	24352	0	18554	27831	CS24	0.07	0.07	0.00	0.06	0.09	CS24	0.11	0.12	0.00	0.09	0.14	CS
CS25	3	72	51	64	60	37	CS25	356185	252298	316609	296821	183040	CS25	1.09	0.77	0.97	0.91	0.56	CS25	0.36	0.26	0.32	0.30	0.19	CS
CS26	1.94	88	73	53	10	18	CS26	196914	163349	118596	22377	40278	CS26	0.60	0.50	0.36	0.07	0.12	CS26	0.31	0.26	0.19	0.04	0.06	CS
CS29	1.61	109	95	83	70	70	CS29	231773	202004	176488	148845	148845	CS29	0.71	0.62	0.54	0.46	0.46	CS29	0.44	0.39	0.34	0.28	0.28	CS
CS30	1.85	90	83	65	63	60	CS30	220088	202970	158952	154061	146725	CS30	0.68	0.62	0.49	0.47	0.45	CS30	0.37	0.34	0.26	0.26	0.24	CS
CS31	1.93	93	83	92	95	68	CS31	236691	211240	234146	241781	173064	CS31	0.73	0.65	0.72	0.74	0.53	CS31	0.38	0.34	0.37	0.38	0.28	CS
CS32	2.01	94	96	97	96	71	CS32	249100	254400	257050	254400	188150	CS32	0.76	0.78	0.79	0.78	0.58	CS32	0.38	0.39	0.39	0.39	0.29	CS
CS33	2.56	85	86	102	98	65	CS33	287419	290800	344903	331377	219791	CS33	0.88	0.89	1.06	1.02	0.67	CS33	0.34	0.35	0.41	0.40	0.26	CS
CS34	5.05	103	71	74	34	46	CS34	500044	344691	359255	165063	223321	CS34	1.53	1.06	1.10	0.51	0.69	CS34	0.30	0.21	0.22	0.10	0.14	CS
CS35	2.68	67	57	54	37	42	CS35	236709.66	201379.86	190780.92	130720.26	148385.16	CS35	0.73	0.62	0.59	0.40	0.46	CS35	0.27	0.23	0.22	0.15	0.17	CS
CS36	2.25	57	64	77	35	38	CS36	168873.9	189612.8	228127.9	103694.5	112582.6	CS36	0.52	0.58	0.70	0.32	0.35	CS36	0.23	0.26	0.31	0.14	0.15	CS
CS37	5.12		63	81	59	44	CS37	0	621054	798498	581622	433752	CS37	0.00	1.91	2.45	1.78	1.33	CS37	0.00	0.37	0.48	0.35	0.26	CS
MB1	2.94	8	0	0	22	22	MB1	27144.48	0	0	74647.32	74647.32	MB1	0.08	0.00	0.00	0.23	0.23	MB1	0.03	0.00	0.00	0.08	0.08	ME
MB2	1.62	44	26	0	12	28	MB2	68560.8	40513.2	0	18698.4	43629.6	MB2	0.21	0.12	0.00	0.06	0.13	MB2	0.13	0.08	0.00	0.04	0.08	M
MB3	2.71	49	31	18	30	35	MB3	128084.04	81032.76	47051.28	78418.8	91488.6	MB3	0.39	0.25	0.14	0.24	0.28	MB3	0.15	0.09	0.05	0.09	0.10	ME
PV2	4	83	90	77	81	69	PV2	383240.88	415562.4	355536.72	374006.16	318597.84	PV2	1.18	1.28	1.09	1.15	0.98	PV2	0.29	0.32	0.27	0.29	0.24	PV
PV3	3.93			115	44	18	PV3	0	0	596091	228069.6	93301.2	PV3	0.00	0.00	1.83	0.70	0.29	PV3	0.00	0.00	0.47	0.18	0.07	PV
WRCS1	0.56	72	84	88	57	52	WRCS1	53424	62328	65296	42294	38584	WRCS1	0.16	0.19	0.20	0.13	0.12	WRCS1	0.29	0.34	0.36	0.23	0.21	WR
WRCS2	1.41	66	88	91	49	56	WRCS2	133712.64	163426.56	183854.88	90998.88	103998.72	WRCS2	0.41	0.50	0.56	0.28	0.32	WRCS2	0.29	0.36	0.40	0.20	0.23	WR
WRCS3	1.2	72	88	99	49	62	WRCS3	113793.12	139080.48	156465.54	77442.54	97988.52	WRCS3	0.35	0.43	0.48	0.24	0.30	WRCS3	0.29	0.36	0.40	0.20	0.25	WR
WRCS4	1.14	55	92	90	49	52	WRCS4	82786	138478.4	135468	73754.8	78270.4	WRCS4	0.25	0.42	0.42	0.23	0.24	WRCS4	0.22	0.37	0.36	0.20	0.21	WR
WRCS5	1.13	65	73	67	36	45	WRCS5	97080.1	109028.42	100067.18	53767.44	67209.3	WRCS5	0.30	0.33	0.31	0.17	0.21	WRCS5	0.26	0.30	0.27	0.15	0.18	WR
WRCS6	1.49	66	91	71	36	45	WRCS6	129845.76	179029.76	139682.56	70824.96	88531.2	WRCS6	0.40	0.55	0.43	0.22	0.27	WRCS6	0.27	0.37	0.29	0.15	0.18	WRO
WRCS7	1.27	68	62	82	35	41	WRCS7	168306.8	153456.2	202958.2	86628.5	101479.1	WRCS7	0.52	0.47	0.62	0.27	0.31	WRCS7	0.41	0.37	0.49	0.21	0.25	WR
WRCS8	0.48	72	61	81	36	50	WRCS8	45639.36	38666.68	51344.28	22819.68	31694	WRCS8	0.14	0.12	0.16	0.07	0.10	WRCS8	0.29	0.25	0.33	0.15	0.20	WR
WRCS9	2.5	67	141	107	54	47	WRCS9	220943.22	464970.06	352849.62	178073.64	154990.02	WRCS9	0.68	1.43	1.08	0.55	0.48	WRCS9	0.27	0.57	0.43	0.22	0.19	WR
WRCS10	0.32	63	61	88	40	37	WRCS10	38732.4	37502.8	54102.4	24592	22747.6	WRCS10	0.12	0.12	0.17	0.08	0.07	WRCS10	0.37	0.36	0.52	0.24	0.22	WRC
WRCS11	2.05	71	66	86	44	63	WRCS11	192138.78	178607.88	232731.48	119071.92	170489.34	WRCS11	0.59	0.55	0.71	0.37	0.52	WRCS11	0.29	0.27	0.35	0.18	0.26	WRC
WRCS12	1.1	54	62	90	41	61	WRCS12	78590.52	90233.56	130984.2	59670.58	88778.18	WRCS12	0.24	0.28	0.40	0.18	0.27	WRCS12	0.22	0.25	0.37	0.17	0.25	WRC
	103.19	2637	2665	2795	2087	2008		7124684	7212168	7995445	6145725	5694744		21.86	22.13	24.54	18.86	17.48		0.23	0.23	0.25	0.18	0.17	

103.	19 2637	2665 27	795	2087	2008		7124684	7212168	7995445	6145725	5694744		21.86	22.13	24.54	18.86	17.48		0.23	0.23	0.25	0.18	0.17	
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Vineyard 5 Year water use summary

C, MB and P Blocks

0,															
Average ac	re feet/bloc	k			Average ac	re feet/acre				Total acre feet					
2017	2018	2019	2020	2021	2017	2018	2019	2020	2021	2017	2018	2019	2020	2021	
0.52	0.49	0.56	0.47	0.42	0.21	0.19	0.20	0.18	0.16	17.71	16.75	19.00	16.10	14.27	
WR Blocks															
Average aci	re feet/bloc	:k			Average ac	re feet/acre				Total Acre feet					
2017	2018	2019	2020	2021	2017	2018	2019	2020	2021	2017	2018	2019	2020	2021	
0.35	0.45	0.46	0.23	0.27	0.29	0.35	0.38	0.19	0.22	4.16	5.39	5.54	2.76	3.21	

2017	2018	2019	2020	2022

APPENDIX C

NAPA COUNTY SWB RECHARGE ANALYSIS

OEI



Napa County Groundwater Recharge Analysis

Introduction

Developing accurate estimates of the spatial and temporal distribution of groundwater recharge is a key component of sustainable groundwater management. Efforts to quantify recharge are inherently difficult owing to the wide variability of factors controlling hydrologic processes, the wide range of available tools/methods for estimating recharge, and the difficulty in assessing the accuracy of estimates because direct measurement of recharge rates is, for the most part, infeasible (Healy 2010, Seiler and Gat 2007).

Numerical modeling is a common approach for developing recharge estimates. Soil-waterbalance modeling is one category of numerical models particularly well-suited for estimating recharge across large areas with modest data requirements. This study describes an application of the U.S. Geological Survey's (USGS) Soil Water Balance Model (SWB) (Westenbroek et al. 2010) to develop spatial and temporal distributions of groundwater recharge across Napa County. This model operates on a daily timestep and calculates surface runoff based on the Natural Resources Conservation Service (NRCS) curve number method and potential evapotranspiration based on the Hargreaves-Samani methods (Hargreaves and Samani 1985). Actual evapotranspiration (AET) and recharge are calculated using a modified Thornthwaite-Mather soil-water-balance approach (Westenbroek et al. 2010).

It is important to note that the SWB model focuses on surface and soil-zone processes and does not simulate the groundwater system or track groundwater storage over time. The model also does not simulate surface water/groundwater interaction or baseflow; thus, the runoff estimates represent only the surface runoff component of streamflow resulting from rainstorms and the recharge estimates represent only the infiltration recharge component (also referred to as diffuse recharge) of total recharge (stream-channel recharge is not simulated).

This modeling work and summary report has been prepared by O'Connor Environmental, Inc., for it's private use in relation to Water Availability Analyses (WAA) prepared on behalf of private clients for projects using groundwater in "hillside" areas of Napa County as required by Napa Planning, Building & Environmental Services. The modeling to-date is complete in its current form but remains subject to revision; it is considered a working draft with information suitable for use to support WAA projects. Parties interested in obtaining more information regarding the modeling or who may wish to offer comments should contact O'Connor Environmental, Inc.



Model Development

The model was developed using a 30-meter (98.4 ft) resolution rectangular grid. Water budget calculations were made on a daily time step. Key spatial inputs included a flow direction map developed from the USGS 1 arc-second resolution Digital Elevation Model (DEM), a land cover map derived from the U.S. Forest Service (USFS) CALVEG dataset that was supplemented by a database of agricultural areas maintained by the County of Napa (Figure 1), a distribution of Hydrologic Soil Groups (A through D classification from lowest to highest runoff potential; Figure 2), and a distribution of Available Water Capacity (AWC) developed from the NRCS Soil Survey Geographic Database (SSURGO) (Figure 3).

A series of model parameters were assigned for each land cover type/soil group combination including an infiltration rate, a curve number, dormant and growing season interception storage values, and a rooting depth (Table 1).

Infiltration rates for hydrologic soil groups A through D were applied based on Cronshey et al. (1986) (Table 2) along with default soil-moisture-retention relationships based on Thornthwaite and Mather (1957) (Figure 4). Curve numbers were assigned based on standard NRCS methods. Interception storage values and rooting depths were assigned based on literature values and from previous modeling experience including a SWB model covering Sonoma County and calibrated using runoff volumes from several stream gages (OEI 2017).



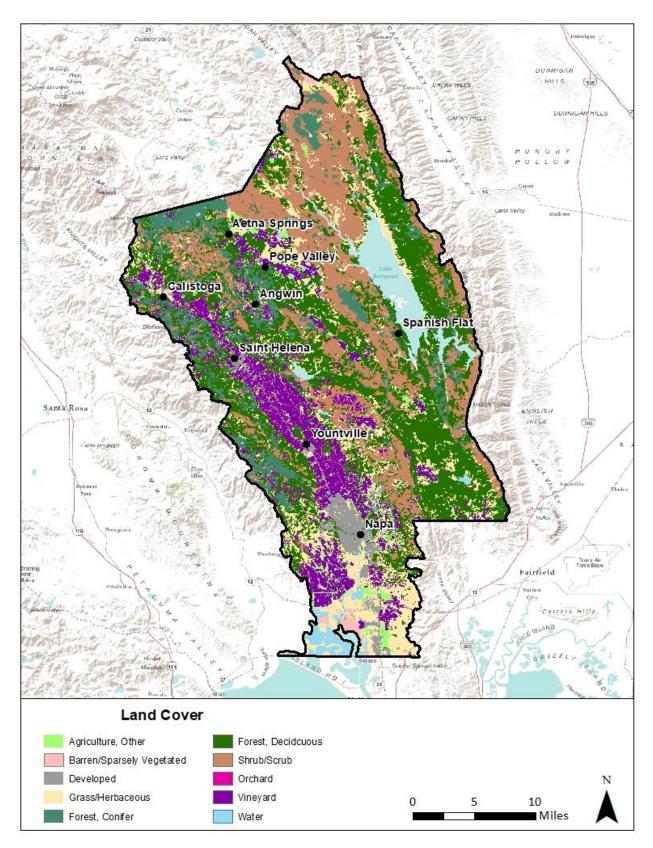


Figure 1: Land cover distribution used in the Napa County SWB model.



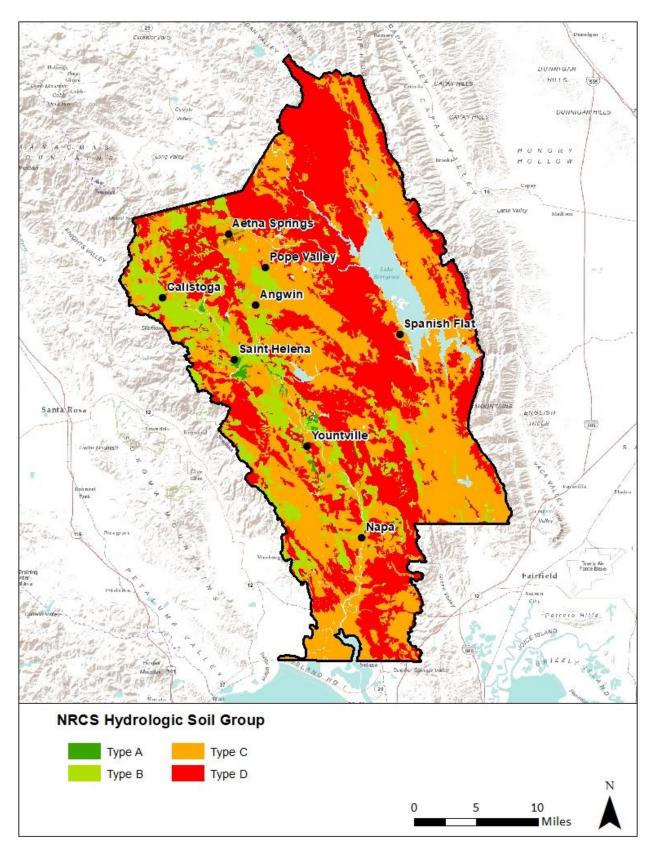


Figure 2: Hydrologic soil group distribution used in the Napa County SWB model.



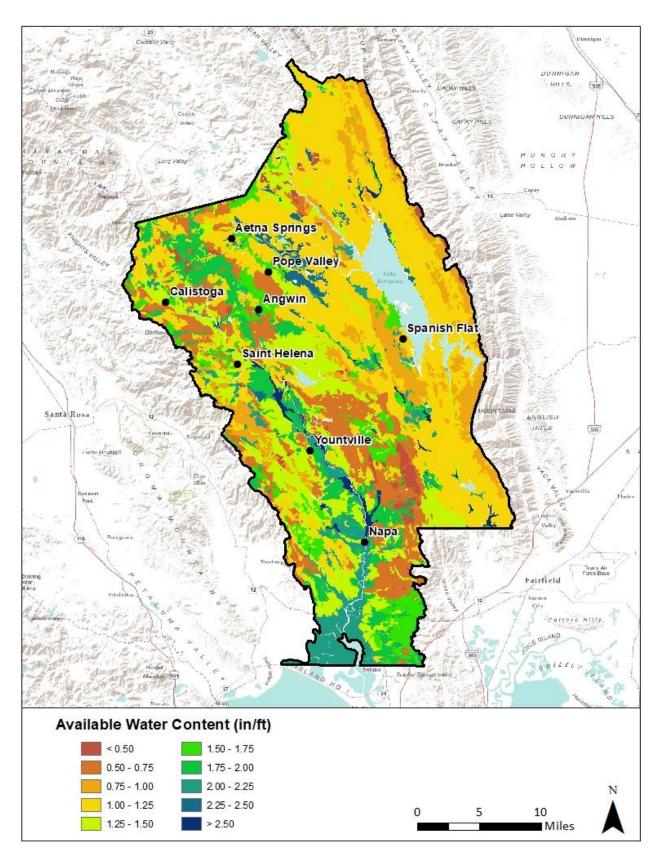


Figure 3: Available water capacity distribution used in the Napa County SWB model.



Land Cover		eption Values ()		Curve Nu NRCS Soi	•		Rooting Depth by NRCS Soil Type (ft)					
	Growing Season	Dormant Season	Туре А	Type B	Type C	Type D	Туре А	Type B	Type C	Type D		
Agriculture, Other	0.080	0.040	38	61	75	81	2.0	1.9	1.8	1.7		
Barren	0.000	0.000	77	86	91	94	0.0	0.0	0.0	0.0		
Developed	0.005	0.002	61	75	83	87	2.3	2.1	2.0	1.8		
Grassland/Herbaceous	0.005	0.004	30	58	71	78	1.3	1.1	1.0	1.0		
Forest, Coniferous	0.050	0.050	30	55	70	77	5.9	5.1	4.9	4.7		
Forest, Deciduous	0.050	0.020	30	55	70	77	5.9	5.1	4.9	4.7		
Shrub/Scrub	0.080	0.015	30	48	65	73	3.2	2.8	2.7	2.6		
Orchard	0.050	0.015	38	61	75	81	3.2	2.8	2.7	2.6		
Vineyard	0.080	0.015	38	61	75	81	2.2	2.1	2.0	1.9		
Water	0.000	0.000	100	100	100	100	0.0	0.0	0.0	0.0		

Table 1: Soil and land cover properties used in the Napa County SWB model.

Table 2: Infiltration rates for NRCS hydrologicsoil groups (Cronshey et al. 1986).

Soil Group	Infiltration Rate (in/hr)
А	> 0.3
В	0.15 - 0.3
С	0.05 - 0.15
D	<0.05

ACCUMULATED POTENTIAL WATER LOSS, IN INCHES MAXIMUM SOIL-MOISTURE CAPACITY, IN INCHES

SOIL MOISTURE RETAINED, IN INCHES

Figure 4: Soil-moisture-retention table (Thornthwaite and Mather 1957).



The SWB model utilizes daily precipitation and mean daily temperature data derived from climate stations. To account for the spatial variability of these parameters, daily precipitation and mean daily temperature were input as gridded (spatially-distributed) time-series. The gridded precipitation time-series was created using data from 15 weather stations in Napa County, and the gridded mean temperature time-series was created using data from 8 stations (Table 3). These stations were selected based on completeness of the records and to provide station data representative of the range of climates experienced in the county. Data was obtained from the California Data Exchange Center (CDEC), the National Climatic Data Center (NCDC), and from Napa One Rain.

To create the gridded time-series, the model domain was divided into discrete areas represented by individual weather stations (Figures 5 and 6). This delineation was based on climate variations described by existing gridded mean annual (1981-2010) precipitation and temperature data (PRISM 2010) and local knowledge of climatic variations across the county.

For the precipitation time-series, each area representing a weather station was subdivided into four to twenty-three zones based on 1-inch average annual precipitation contours. Within each zone the raw station data was multiplied by a unique scaling factor. This scaling factor was calculated as the ratio of average annual precipitation within a zone to average annual precipitation at the representative rain gage. In certain locations, typically near the boundary of areas represented by gages located on the valley bottom and at higher elevations, this scaling was unable to smoothly resolve differences in annual and event precipitation totals. To more accurately estimate precipitation near these boundaries, precipitation records from the two gages in question were averaged using weights calculated proportionally to the difference between PRISM mean annual precipitation at a rain gage and within a selected zone. The resulting gridded time-series is comprised of 220 individual time-series based on the scaled station data from 15 stations.

The assignment of temperature stations was based on the understanding that the spatial variability of temperatures across Napa County is relatively homogenous, with elevation being the primary variable. Temperature records were classified either as Mountain, Valley Bottom, or East County and applied within areas the PRISM datasets described as being similar. To smooth the transition from Mountain zones to Valley Bottom and East County zones, Hillside zones were created where the temperature records of the two nearest gages were averaged.

Missing and suspect data was encountered in the raw precipitation and temperature data from the weather stations used by the model. Values that were significantly outside the typical range, and where similar observations were not found at nearby stations, were removed from the datasets. These and missing values were filled using scaled data from other nearby stations. Precipitation data used for gap filling was scaled using the ratio of the 1981 to 2010 mean annual precipitation (PRISM 2010) between the two stations. Temperature data was scaled using the ratio of the 1981 to 2010 mean monthly minimum and maximum temperatures (PRISM 2010) between the two stations.



The current analysis focuses on Water Year 2010 (October 1, 2009 – September 30, 2010) and Water Year 2014 (October 1, 2013 – September 30, 2014). These years were selected because they represent periods with data available from most weather stations in the county and where most stations reported annual precipitation totals close to the long-term average (WY 2010) and significantly below the long term average (WY 2014). Based on a comparison between station data and PRISM average precipitation depths during Water Year 2010, rainfall averaged 101% of long-term average conditions and ranged from 78% at Lake Hennessey to 111% at the Napa County Airport. In Water Year 2014, rainfall averaged 55% of long-term average conditions and ranged from 41% at Lake Hennessey to 73% at the Napa State Hospital (Table 3).

Station	Data Used	Data Used 1981 - 2010 Mean Annual Precip (in)		WY 2010 Precip (in) % Avg		014 % Avg
Angwin ¹	Precip & Temp	42.54	44.64	105%	25.04	59%
Atlas Peak ¹	Precip & Temp	41.76	39.04	93%	20.08	48%
Berryessa ¹	Precip & Temp	28.97	28.16	97%	13.97	48%
Calistoga ²	Precip	39.41	41.75	106%	18.18	46%
Knoxville Creek ¹	Temp Only	-	-	-	-	-
Lake Hennessey ³	Precip Only	34.09	26.52	78%	13.92	41%
Mt. George ³	Precip Only	31.15	29.64	95%	18.24	59%
Mt. Veeder ³	Precip Only	44.81	46.44	104%	28.6	64%
Napa County Airport ²	Precip & Temp	21.14	23.56	111%	9.87	47%
Napa River at Yountville Cross Rd ³	Precip Only	31.86	32.72	103%	14.93	47%
Napa State Hospital ²	Precip & Temp	26.81	28.85	108%	19.66	73%
Petrified Forest ³	Precip Only	42.39	46.6	110%	22.84	54%
Redwood Creek At Mt. Veeder Road ³	Precip Only	34.71	37.36	108%	23.48	68%
Saint Helena ²	Precip & Temp	37.43	39.11	104%	19.11	51%
Saint Helena 4WSW ¹	Precip & Temp	45.44	47.88	105%	28.88	64%
Sugarloaf Peak ³	Precip Only	32.20	26.16	81%	17.12	53%

Table 3: Weather stations used in the Napa Count	y SWB model. See Figures 7-9 for associated timeseries.
Tuble 5. Weather Stations asea in the Hapa count	y styp model see ngares / s for associated inteseries.

1 – Data accessed from California Data Exchange Center (CDEC)

2 – Data accessed from National Climate Data Center (NCDC)

3 - Data access from Napa One Rain



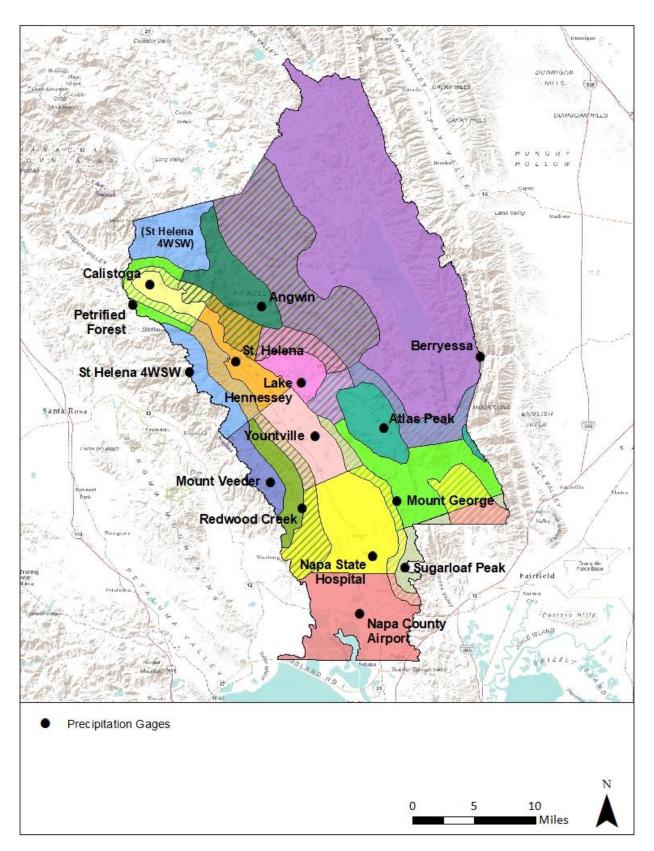


Figure 5: Precipitation zones used in the Napa County SWB model. Hatching indicates areas where two precipitation records were averaged across a zone.



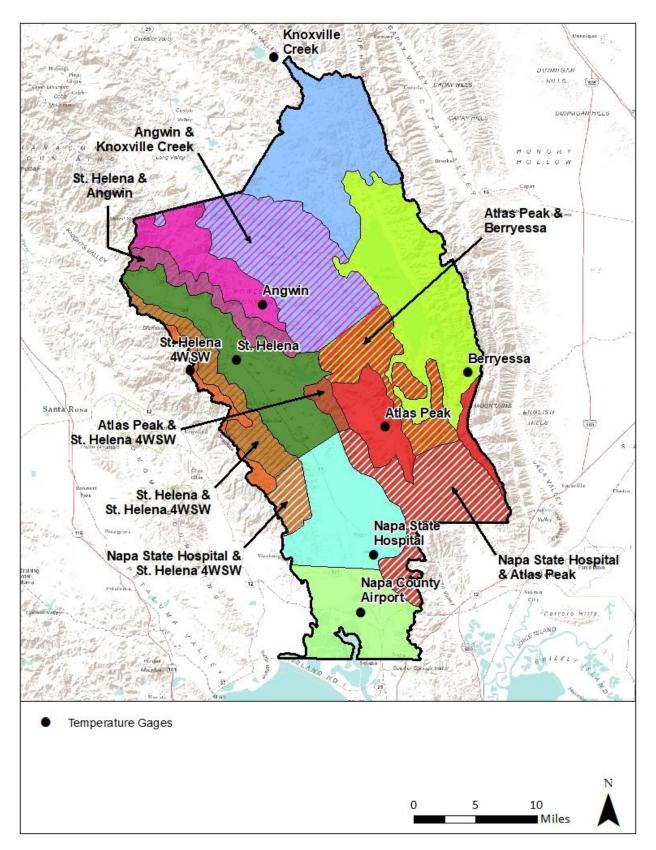


Figure 6: Temperature zones used in the Napa County SWB model. Hatching indicates areas where two temperature records were averaged across a zone.



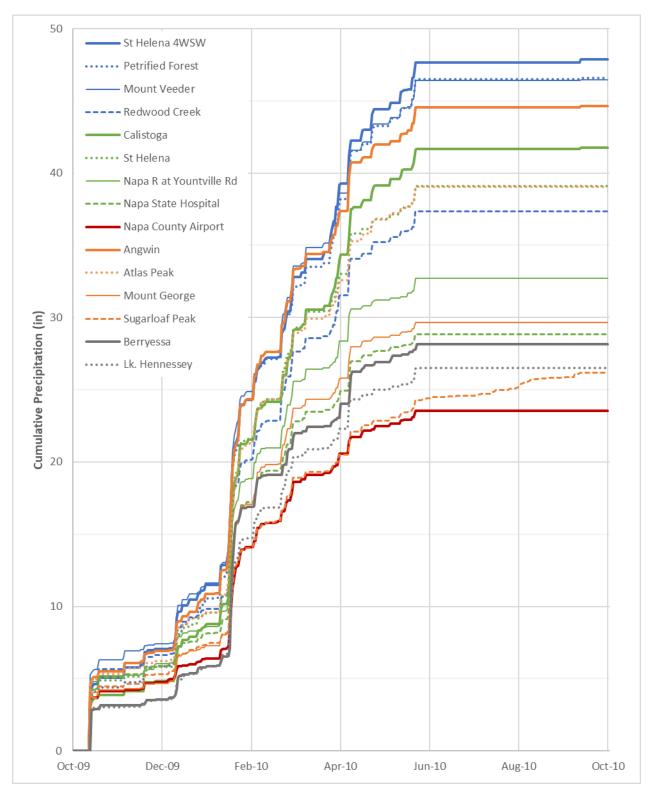


Figure 7a: Daily precipitation data used in the Napa County SWB model for WY 2010.

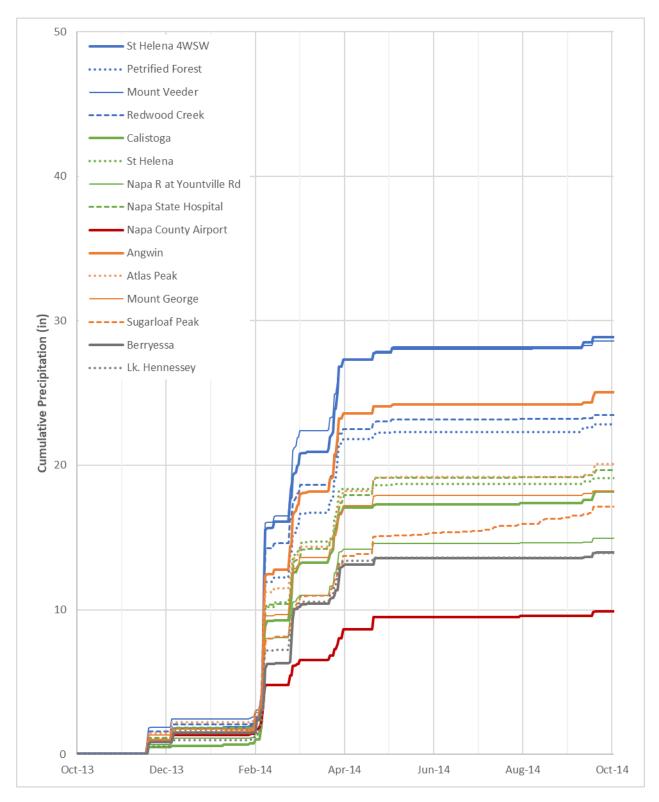


Figure 7b: Daily precipitation data used in the Napa County SWB model for WY 2014.

OEI

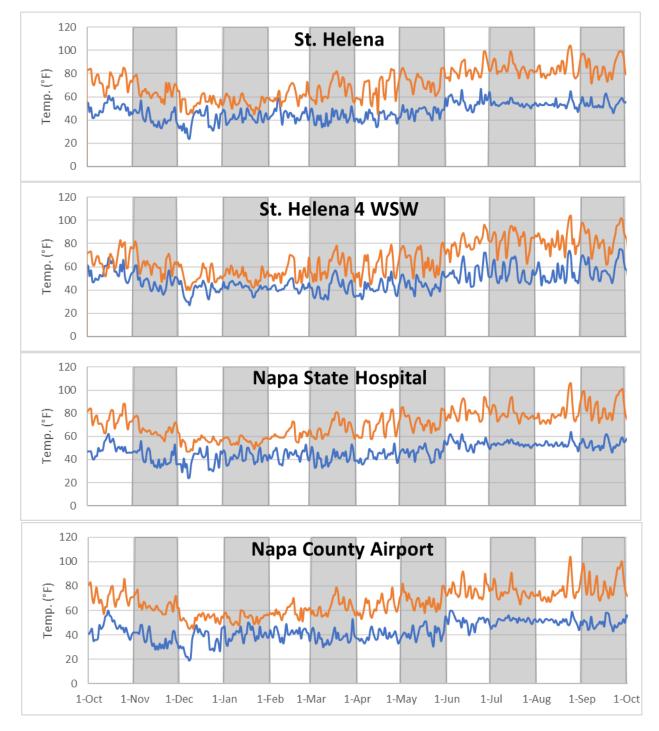


Figure 8: Daily minimum and maximum temperature data used in the Sonoma County SWB model for WY 2010.



DRAFT

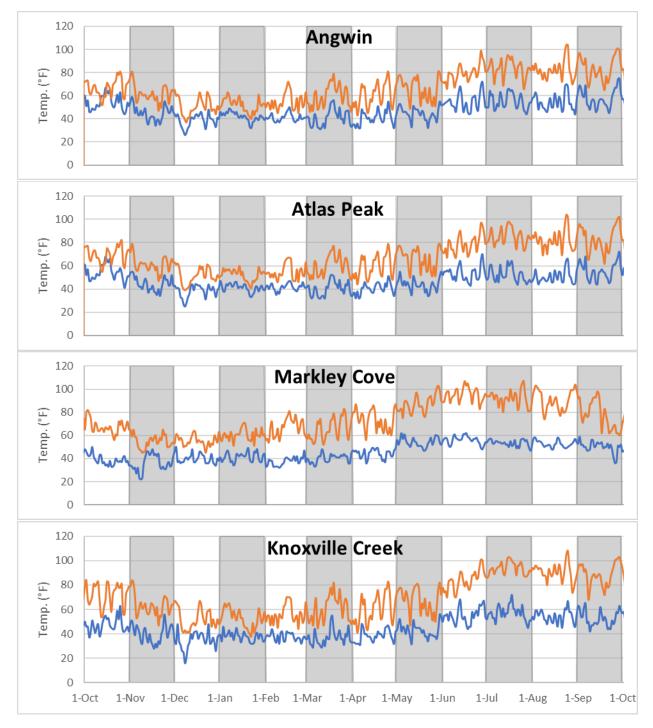


Figure 8 – cont.



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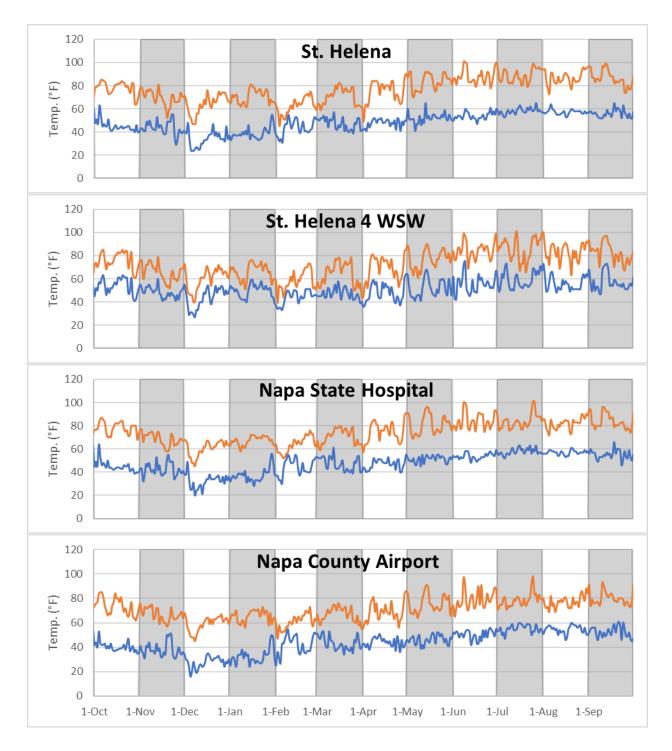


Figure 9: Daily minimum and maximum temperature data used in the Sonoma County SWB model for WY 2010.



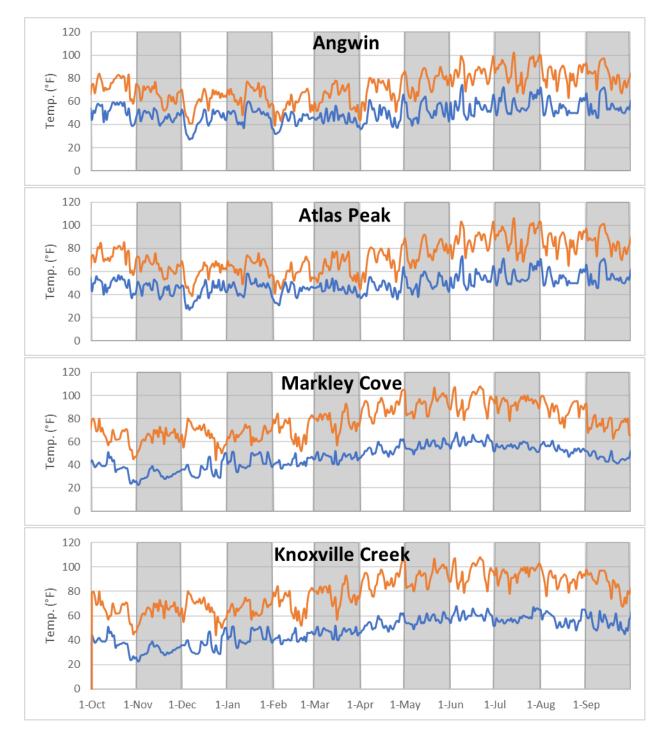


Figure 9 – cont.



Model Calibration

Available data are insufficient to calibrate the Water Year 2010 and 2014 SWB simulations; however, the land cover and soil properties used in the model were obtained from a previously prepared and calibrated SWB model of Sonoma County (OEI 2017). The Sonoma County model was calibrated against total monthly runoff volumes derived using baseflow separation of streamflow data for five watersheds within Sonoma County. Gages were selected because they represented relatively small watersheds ($1.2 - 14.3 \text{ mi}^2$) without significant urbanization, diversions, groundwater abstraction, reservoir impoundments, or large alluvial bodies where significant exchanges between surface water and groundwater may be expected. These attributes are desirable because the hydrographs can more readily be separated into surface runoff and baseflow components and the surface runoff pattern is more directly comparable to the SWB simulated surface runoff which does not account for water use, reservoir operations, or surface water/groundwater exchange.

SWB utilizes a simplified routing scheme whereby surface runoff is routed to downslope cells or out of the model domain on the same day in which it originates as rainfall, thus it is not capable of accurately estimating streamflow over short time periods. The use of the total monthly surface runoff volumes provided a means of calibrating the Sonoma County SWB model to measured surface runoff data within the limitations of the model's approach to simulating surface runoff.

The SWB model of Sonoma County reproduced seasonal variations in surface runoff in all five calibration watersheds. Monthly Mean Errors (ME) ranged from -0.2 to 0.4 inches with a mean value of 0.1 inches. Annual surface runoff totals ranged from an under-prediction of approximately 10% at Franchini Creek to an over-prediction of approximately 19% at Buckeye Creek, with a mean over-prediction of approximately 6% across the five watersheds. These results indicate that the SWB model was able to reproduce monthly surface runoff volumes with a reasonable degree of accuracy and that the model tends to over-predict surface runoff somewhat, suggesting that the model may generate a low-range estimate of recharge.

Although the climate in Napa County is slightly drier than in Sonoma County, the vegetation, soils, and geology are similar and parameters calibrated using data from Sonoma County should be applicable to Napa County. Calibration of the Napa County SWB model was not performed due to a lack of publicly-available contemporary discharge records in suitable watersheds. Contemporary discharge records exist for USGS gaging stations located along the Napa River near St. Helena and Napa, but the watersheds above these gages are large and contain significant groundwater abstraction, reservoir impoundments, and alluvial bodies. USGS gages on smaller watersheds in Napa County have been inactive since 1983 or earlier. Discharge records exist through Napa One Rain for several streams gaged by the Napa County Resource Conservation District (RCD) but the RCD has cautioned against use of these discharge records for calibration purposes due to incomplete rating curve development.



Estimates of groundwater recharge are also available from an earlier model prepared by Luhdorff and Scalmanini Engineers and MBK Engineers (LSCE 2013). This report provided estimates of average annual recharge as a percentage of average annual precipitation for nine watersheds in Napa County. Averaged across the same nine watersheds, the SWB model predicts significantly higher rates of recharge than the model prepared by LSCE, which predicts slightly lower AET but significantly more runoff (Table 4). Differences in methodology between these two models complicate direct comparisons. The LSCE model calculated infiltration into the soil as the difference between monthly precipitation and discharge volumes within each watershed. Discharge volumes were calculated from USGS stream gages and included both direct runoff and baseflow from groundwater. Inclusion of baseflow with direct runoff in these calculations may inappropriately reduce the estimated volume of water infiltrated into the soil and available for recharge.

USGS Gage	HUC	C Mean Precip, 2010 (in)		Mean AET, 2010 (% Precip)		Mean Runoff, 2010 (% Precip)		Mean Recharge, 2010 (% Precip)	
			SWB	LSCE	SWB	LSCE	SWB	LSCE	
Conn Ck nr Oakville	11456500	34.8	59%	53%	21%	25%	21%	21%	
Dry Ck nr Napa	11457000	41.5	56%	50%	18%	43%	25%	6%	
Milliken Ck nr Napa	11458100	32.3	52%	41%	20%	51%	28%	8%	
Napa Ck at Napa	11458300	36.6	61%	43%	16%	46%	23%	11%	
Napa R nr Napa	11458000	39.5	56%	48%	20%	35%	24%	17%	
Napa R nr St Helena	11456000	47.9	46%	45%	23%	42%	30%	14%	
Redwood Ck nr Napa	11458200	39.6	53%	49%	26%	40%	22%	10%	
Tulucay Ck nr Napa	11458300	27.0	64%	49%	16%	47%	20%	5%	

Table 4: Comparison of results from SWB model and Luhdorff and Scalmanini model.

Model Results

The principal elements of the annual water budget simulated with the Napa County SWB model for Water Years 2010 and 2014 are presented in map form in Figures 10 - 19 and in tabular form for 27 major watershed areas in Napa County (Tables 5 - 8). The watersheds are based on USGS HUC-12 watersheds and are named for the stream which comprises the largest proportion of the area; in many cases the areas consist of multiple tributary streams (Figure 20).

In Water Year 2010 (representing "average" hydrologic conditions) precipitation varied from 21.8 inches in the Ledgewood Creek watershed to 53.3 inches in the Saint Helena Creek watershed (Figure 10, Table 5). Actual evapotranspiration (AET) ranged from 13.4 inches in the Jackson Creek watershed to 25.2 inches in the Saint Helena Creek watershed (Figure 11). Surface runoff ranged from 3.4 inches in the Ledgewood Creek watershed to 13.5 inches in the Saint Helena Creek watershed (Figure 12). Recharge ranged from 3.3 inches in the Ledgewood Creek watershed to 14.4 inches in the Saint Helena watershed. (Figure 13). Small decreases in soil moisture storage (up to 1.8 inches) occurred in most watersheds, with changes in most



watersheds being less than an inch (Figure 14). Note that the San Pablo Bay estuaries have been excluded from these comparisons.

Expressed as a percentage of the annual precipitation, AET ranged from 77% in the Ledgewood Creek watershed to 45% in the Jackson Creek watershed (Table 6). Surface runoff ranged from 15% of precipitation in the Ledgewood Creek watershed to 42% in the Jackson Creek watershed. Recharge ranged from 10% of the precipitation in the Jackson Creek watershed to 27% in the Saint Helena watershed.

In Water Year 2014 (representing "dry" hydrologic conditions during the second year of an extreme three-year drought) precipitation varied from 10.1 inches in the American Canyon Creek watershed to 32.2 inches in the Saint Helena Creek watershed (Figure 15, Table 7). Actual evapotranspiration (AET) ranged from 10.3 inches in the Jackson Creek watershed to 17.8 inches in the Saint Helena Creek watershed (Figure 16). Surface runoff ranged from 0.7 inches in the American Canyon Creek watershed to 13.2 inches in the Saint Helena Creek watershed to 13.2 inches in the Saint Helena Creek watershed (Figure 17). Recharge ranged from 0.6 inches in the Wragg Canyon watershed to 4.1 inches in the Saint Helena watershed. (Figure 18). Large decreases in soil moisture storage of between 2.3 and 4.3 inches were also simulated (Figure 19).

Expressed as a percentage of the annual precipitation, AET ranged from 55% in the Saint Helena Creek watershed to 121% in the Jackson Creek watershed (Table 8). These very large AET rates caused significant decreases in soil moisture. Decreases in soil moisture ranged from 9% of precipitation in the Saint Helena watershed to 36% in the American Canyon Creek watershed. Surface runoff ranged from 7% of precipitation in the American Canyon Creek watershed to 41% in the Saint Helena Watershed. Recharge ranged from 18% in the Milliken Creek Watershed to 5% in the Jackson Creek and Wragg Canyon watersheds.



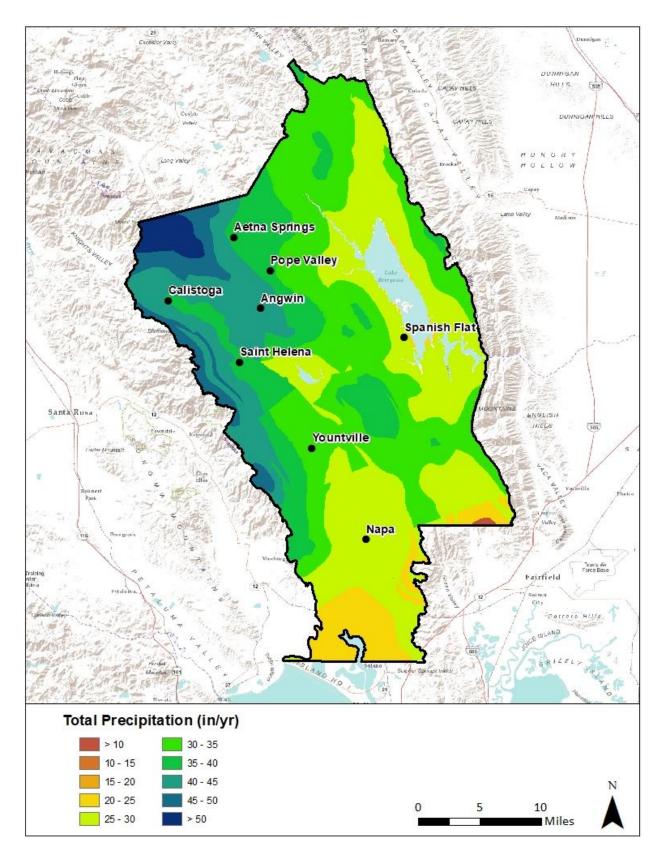


Figure 10: Water Year 2010 precipitation simulated with the Napa County SWB model.



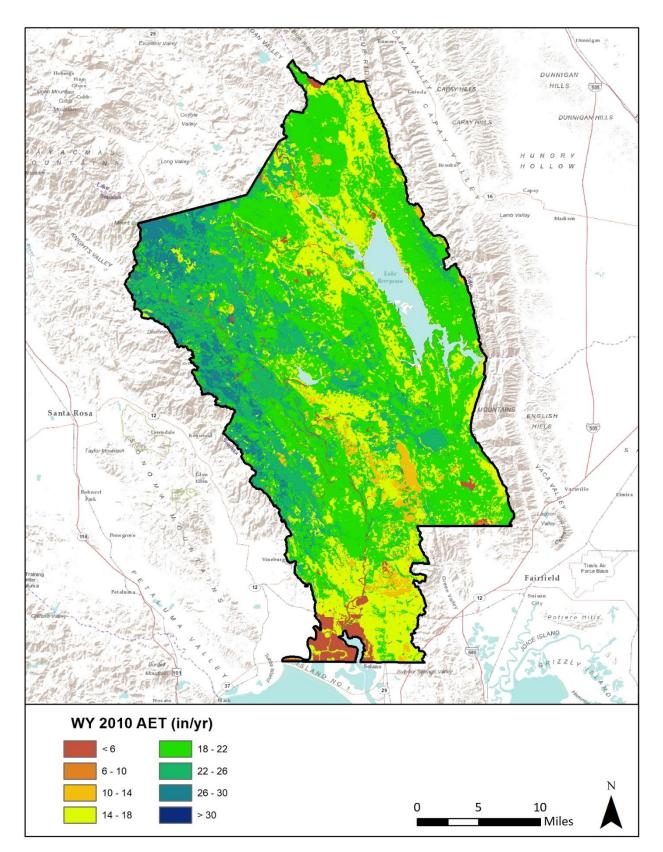


Figure 11: Water Year 2010 AET simulated with the Napa County SWB model.



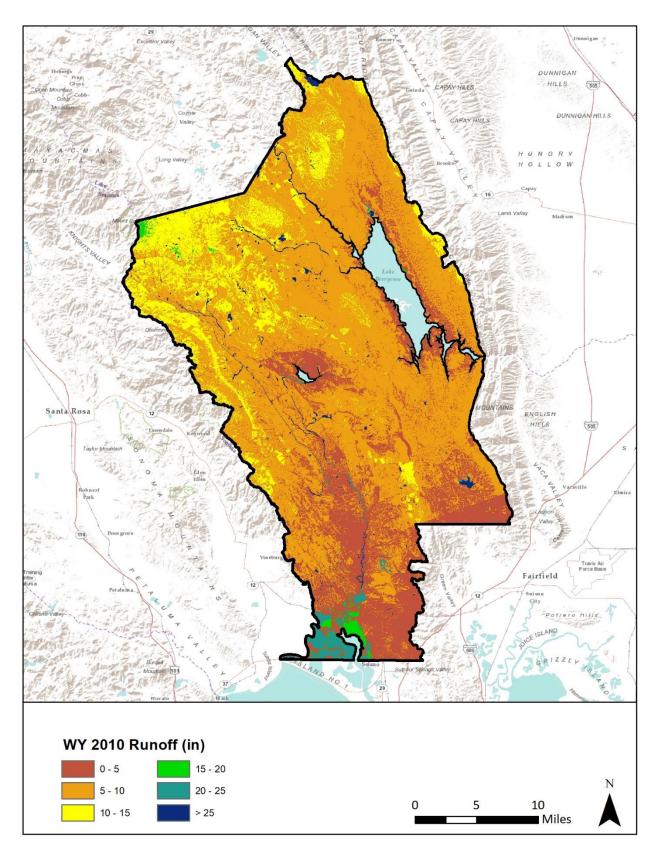


Figure 12: Water Year 2010 runoff simulated with the Napa County SWB model.



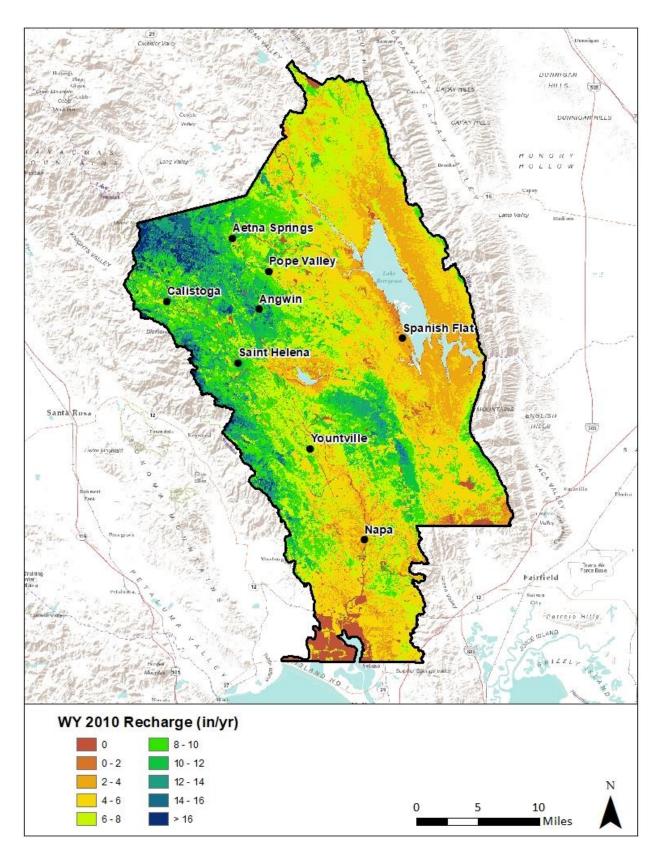


Figure 13: Water Year 2010 recharge simulated with the Napa County SWB model.



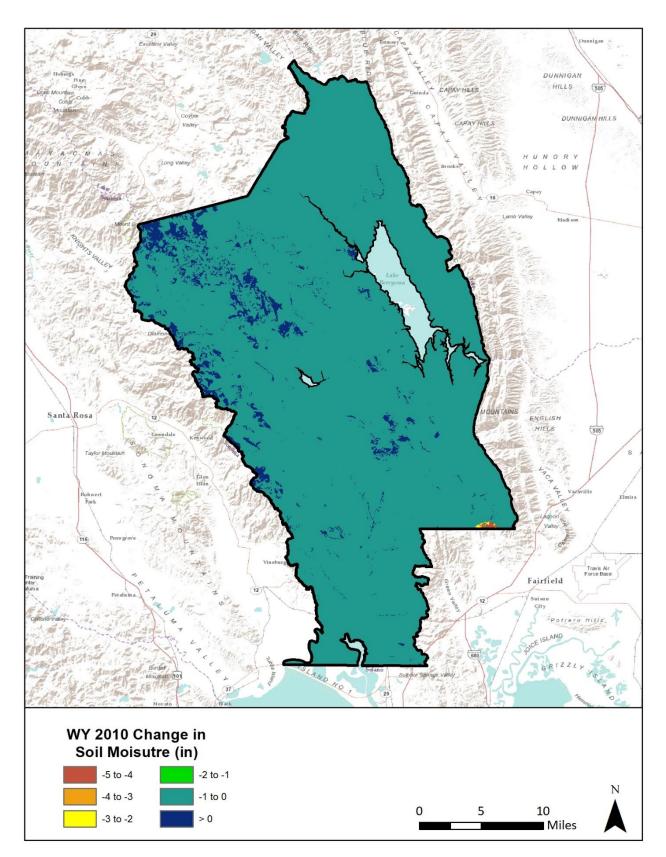


Figure 14: Water Year 2010 change in soil moisture content simulated with the Napa County SWB model.



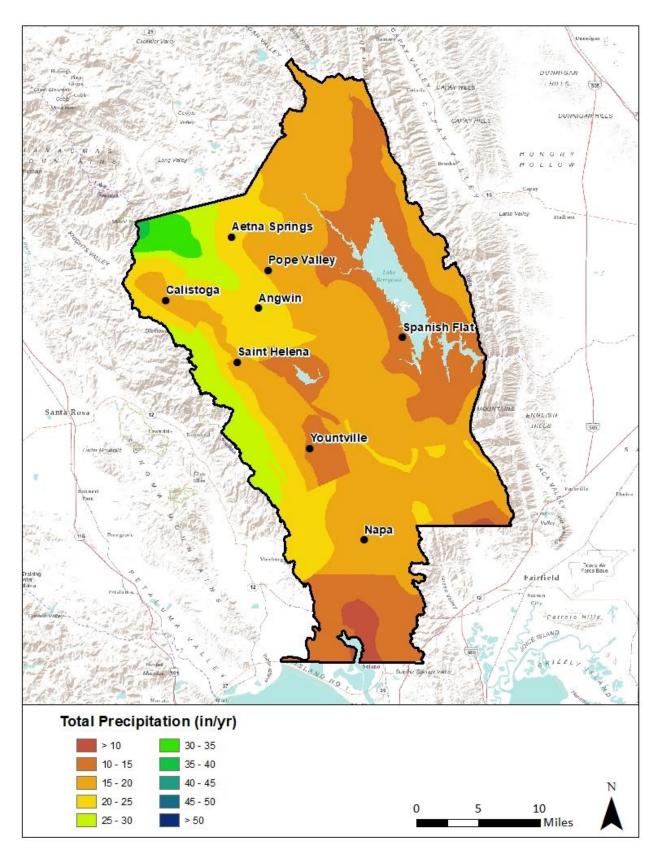


Figure 15: Water Year 2014 precipitation simulated with the Napa County SWB model.



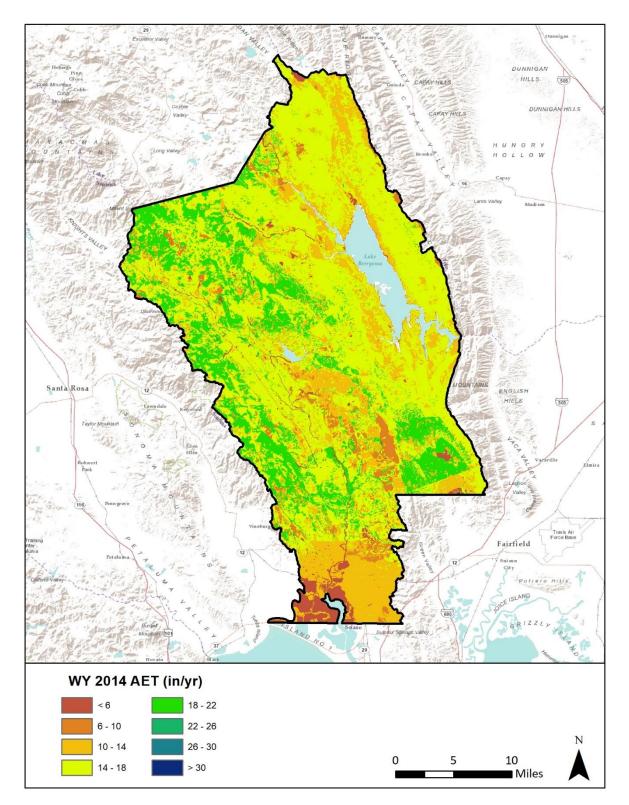


Figure 16: Water Year 2014 AET simulated with the Napa County SWB model.



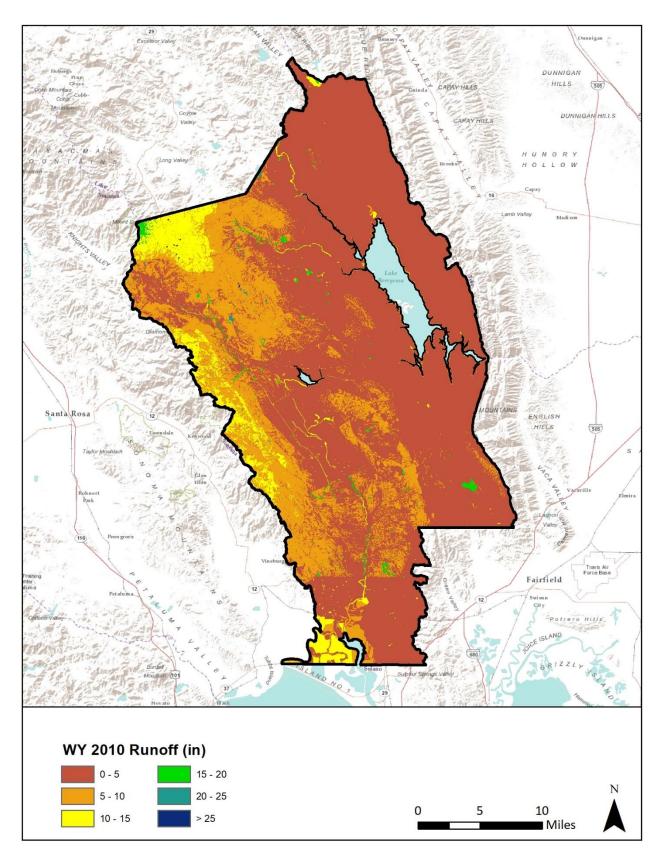


Figure 17: Water Year 2014 recharge simulated with the Napa County SWB model.



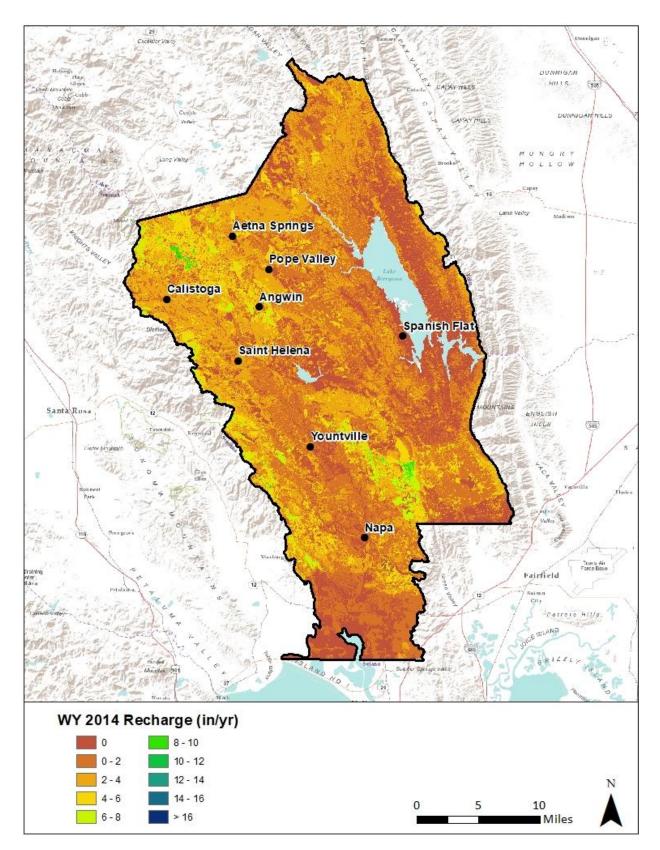


Figure 18: Water Year 2014 recharge simulated with the Napa County SWB model.



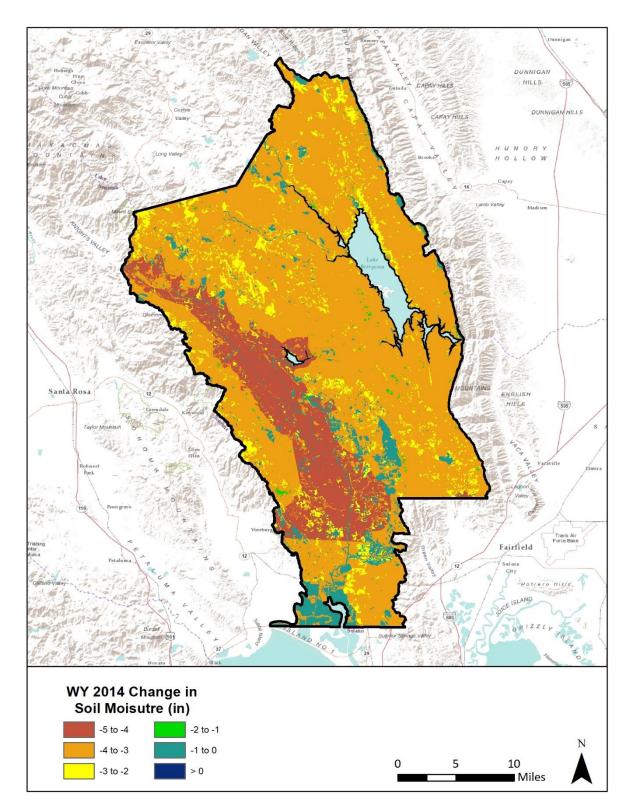


Figure 19: Water Year 2014 change in soil moisture content simulated with the Napa County SWB model.



 Table 5: Simulated precipitation and recharge values averaged across HUC-12 watersheds in Napa County for

 Water Year 2010 expressed as depths.
 See Figure 20 for watershed locations.

Name	Drainage Area (mi ²)	Precipitation (in)	AET (in)	Surface Runoff (in)	Recharge (in)	Soil Moisture Change (in)
American Canyon Creek	10.8	24.1	16.3	3.7	4.7	-0.6
Bucksnort Creek	1.9	47.9	24.5	12.1	11.1	0.1
Butts Creek-Putah Creek	49.9	33.0	17.4	9.7	6.2	-0.7
Capell Creek	43.0	31.1	19.1	7.4	5.0	-0.6
Carneros Creek	29.7	28.0	18.6	5.2	5.5	-0.6
Chiles Creek	32.0	34.6	21.1	7.1	6.8	-0.5
Dry Creek	28.8	37.0	22.2	7.2	8.4	-0.5
Hunting Creek	12.0	33.7	19.0	9.7	5.7	-0.8
Jackson Creek-Putah Creek	54.5	29.9	13.4	12.6	3.0	-0.5
Lake Curry-Suisun Creek	16.4	30.7	18.9	6.5	5.9	-0.6
Lake Hennessey-Conn Creek	20.0	35.1	19.6	8.5	7.3	-0.4
Ledgewood Creek	6.4	21.8	16.9	3.4	3.3	-1.8
Lower Eticuera Creek	44.0	30.0	17.7	8.1	4.7	-0.7
Lower Napa River	45.0	31.7	19.9	5.6	6.7	-0.6
Lower Pope Creek	31.8	33.9	18.0	9.7	6.5	-0.6
Maxwell Creek	35.1	34.7	19.6	8.7	6.9	-0.6
Middle Napa River	60.3	39.9	22.8	8.5	9.2	-0.5
Milliken Creek	29.7	30.9	16.9	6.6	7.9	-0.6
Rector Creek-Conn Creek	22.3	32.8	18.0	7.1	8.2	-0.7
Saint Helena Creek	7.7	53.3	25.2	13.5	14.4	0.1
San Pablo Bay Estuaries	19.5	23.9	8.1	13.8	2.3	-0.3
Tulucay Creek	34.2	26.1	16.7	4.6	5.4	-0.7
Upper Eticuera Creek	25.6	31.2	17.2	8.6	6.1	-0.8
Upper Napa River	44.6	44.7	23.6	10.6	10.8	-0.4
Upper Pope Creek	21.7	44.5	22.7	10.5	11.5	-0.3
Wooden Valley & Suisun Creeks	23.3	29.0	19.0	5.1	5.5	-0.6
Wragg Canyon-Putah Creek	34.2	28.3	16.3	8.6	3.3	-0.6



 Table 6: Simulated precipitation and recharge values averaged across HUC-12 watersheds in Napa County for

 Water Year 2010 expressed as a percentage of precipitation.

 See Figure 20 for watershed locations.

Name	Drainage Area (mi ²)	Precipitation (in)	AET (%)	Surface Runoff (%)	Recharge (%)	Soil Moisture Change (%)
American Canyon Creek	10.8	24.1	67%	15%	19%	-3%
Bucksnort Creek	1.9	47.9	51%	25%	23%	0%
Butts Creek-Putah Creek	49.9	33.0	53%	29%	19%	-2%
Capell Creek	43.0	31.2	61%	24%	16%	-2%
Carneros Creek	29.7	29.7	66%	19%	20%	-2%
Chiles Creek	32.0	34.6	61%	21%	20%	-1%
Dry Creek	28.8	37.8	60%	20%	23%	-1%
Hunting Creek	12.0	33.7	56%	29%	17%	-2%
Jackson Creek-Putah Creek	54.5	29.7	45%	42%	10%	-2%
Lake Curry-Suisun Creek	16.4	30.7	61%	21%	19%	-2%
Lake Hennessey-Conn Creek	20.0	36.0	56%	24%	21%	-1%
Ledgewood Creek	6.4	21.8	77%	15%	15%	-8%
Lower Eticuera Creek	44.0	30.0	59%	27%	16%	-2%
Lower Napa River	45.0	31.7	63%	18%	21%	-2%
Lower Pope Creek	31.8	33.9	53%	29%	19%	-2%
Maxwell Creek	35.1	34.7	56%	25%	20%	-2%
Middle Napa River	60.3	40.4	57%	21%	23%	-1%
Milliken Creek	29.7	30.9	55%	21%	26%	-2%
Rector Creek-Conn Creek	22.3	32.8	55%	22%	25%	-2%
Saint Helena Creek	7.7	53.3	47%	25%	27%	0%
San Pablo Bay Estuaries	19.5	23.9	34%	58%	10%	-1%
Tulucay Creek	34.2	26.1	64%	18%	21%	-3%
Upper Eticuera Creek	25.6	31.2	55%	28%	19%	-3%
Upper Napa River	44.6	44.7	53%	24%	24%	-1%
Upper Pope Creek	21.7	44.5	51%	23%	26%	-1%
Wooden Valley & Suisun Creeks	23.3	29.0	65%	18%	19%	-2%
Wragg Canyon-Putah Creek	34.2	28.3	58%	31%	12%	-2%



 Table 7: Simulated precipitation and recharge values averaged across HUC-12 watersheds in Napa County for

 Water Year 2014 expressed as depths.
 See Figure 20 for watershed locations.

Name	Drainage Area (mi ²)	Precipitation (in)	AET (in)	Surface Runoff (in)	Recharge (in)	Soil Moisture Change (in)
American Canyon Creek	10.8	10.1	12.3	0.7	0.7	-3.6
Bucksnort Creek	1.9	28.8	17.6	11.5	2.6	-3.0
Butts Creek-Putah Creek	49.9	16.9	14.2	3.9	1.9	-3.2
Capell Creek	43.0	15.8	14.8	3.1	1.1	-3.1
Carneros Creek	29.7	15.0	14.7	4.6	2.0	-3.7
Chiles Creek	32.0	18.3	16.5	3.7	1.5	-3.3
Dry Creek	28.8	21.5	16.5	6.8	2.5	-3.7
Hunting Creek	12.0	16.7	15.4	3.1	1.6	-3.4
Jackson Creek-Putah Creek	54.5	14.9	10.3	6.1	0.7	-2.3
Lake Curry-Suisun Creek	16.4	18.4	16.1	3.7	1.9	-3.4
Lake Hennessey-Conn Creek	20.0	19.1	14.8	5.7	2.2	-3.2
Ledgewood Creek	6.4	12.2	13.9	1.7	0.8	-4.3
Lower Eticuera Creek	44.0	14.9	14.0	2.6	1.3	-3.1
Lower Napa River	45.0	19.4	15.9	5.0	2.2	-3.6
Lower Pope Creek	31.8	17.8	14.5	4.5	2.0	-3.2
Maxwell Creek	35.1	18.3	15.9	3.8	2.0	-3.3
Middle Napa River	60.3	21.3	16.5	6.6	2.5	-3.7
Milliken Creek	29.7	18.7	13.7	4.5	3.4	-2.9
Rector Creek-Conn Creek	22.3	16.5	13.6	4.0	2.3	-3.4
Saint Helena Creek	7.7	32.2	17.8	13.2	4.1	-3.0
San Pablo Bay Estuaries	19.5	10.4	6.0	5.6	0.5	-1.6
Tulucay Creek	34.2	14.6	13.5	2.6	1.7	-3.3
Upper Eticuera Creek	25.6	15.5	14.1	2.5	2.1	-3.2
Upper Napa River	44.6	22.9	16.2	6.9	3.3	-3.5
Upper Pope Creek	21.7	25.6	16.8	8.5	3.5	-3.2
Wooden Valley & Suisun Creeks	23.3	17.9	16.4	3.1	2.0	-3.5
Wragg Canyon-Putah Creek	34.2	14.1	12.6	3.6	0.6	-2.8



 Table 8: Simulated precipitation and recharge values averaged across HUC-12 watersheds in Napa County for

 Water Year 2014 expressed as a percentage of precipitation.

 See Figure 20 for watershed locations.

Name	Drainage Area (mi ²)	Precipitation (in)	AET (%)	Surface Runoff (%)	Recharge (%)	Soil Moisture Change (%)
American Canyon Creek	10.8	10.1	121%	7%	7%	-36%
Bucksnort Creek	1.9	28.8	61%	40%	9%	-10%
Butts Creek-Putah Creek	49.9	16.8	84%	23%	11%	-19%
Capell Creek	43.0	15.8	94%	20%	7%	-20%
Carneros Creek	29.7	17.6	98%	30%	13%	-25%
Chiles Creek	32.0	18.4	90%	20%	8%	-18%
Dry Creek	28.8	22.1	77%	32%	12%	-17%
Hunting Creek	12.0	16.7	92%	18%	10%	-20%
Jackson Creek-Putah Creek	54.5	14.7	69%	41%	5%	-16%
Lake Curry-Suisun Creek	16.4	18.4	88%	20%	10%	-19%
Lake Hennessey-Conn Creek	20.0	19.6	78%	30%	12%	-17%
Ledgewood Creek	6.4	12.2	114%	14%	7%	-35%
Lower Eticuera Creek	44.0	14.9	94%	18%	9%	-21%
Lower Napa River	45.0	19.4	82%	26%	11%	-19%
Lower Pope Creek	31.8	17.8	81%	25%	11%	-18%
Maxwell Creek	35.1	18.3	87%	21%	11%	-18%
Middle Napa River	60.3	21.8	77%	31%	12%	-18%
Milliken Creek	29.7	18.7	74%	24%	18%	-16%
Rector Creek-Conn Creek	22.3	16.5	83%	24%	14%	-21%
Saint Helena Creek	7.7	32.2	55%	41%	13%	-9%
San Pablo Bay Estuaries	19.5	10.4	58%	53%	4%	-16%
Tulucay Creek	34.2	14.6	93%	18%	12%	-23%
Upper Eticuera Creek	25.6	15.5	91%	16%	14%	-21%
Upper Napa River	44.6	22.9	71%	30%	14%	-15%
Upper Pope Creek	21.7	25.6	66%	33%	14%	-12%
Wooden Valley & Suisun Creeks	23.3	17.9	91%	17%	11%	-20%
Wragg Canyon-Putah Creek	34.2	14.1	90%	26%	5%	-20%



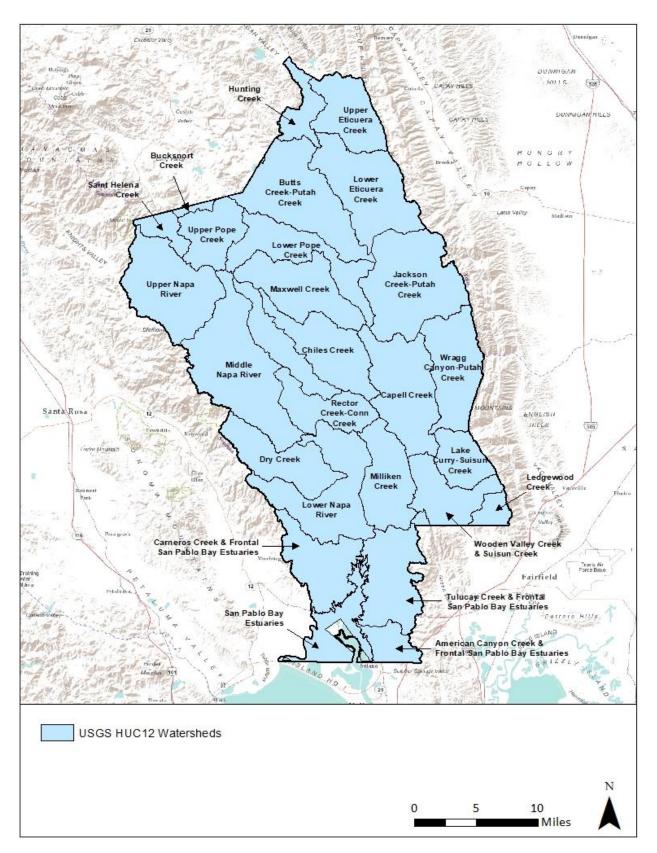


Figure 20: Major watersheds areas used to summarize water budget information in Tables 5 - 8.



Discussion and Conclusion

Numerous previous modeling studies have estimated water budget components in several larger watershed areas in Sonoma and Napa Counties including the Santa Rosa Plain, the Green Valley and Dutch Bill Creek watersheds, and the Sonoma Valley (Farrar et. al., 2006; Kobor and O'Connor, 2016; Woolfenden and Hevesi, 2014). Comparisons to these water budgets are useful for evaluating the SWB results, but one would not expect precise agreement owing to significant variations in climate, land cover, soil types, underlying hydrogeologic conditions, and different spatial scales of modeling studies. These regional analyses estimate that average annual recharge varies from 7% to 19% of the annual precipitation. The equivalent county-wide value from this study is slightly higher at 20%.

Water budgets for the Napa River and selected sub-basins were also estimated in a previous study by Luhdorff and Scalmanini Engineers and MBK Engineers (LSCE 2013). The LSCE study estimated that, as a percentage of annual precipitation, AET comprised slightly less, runoff significantly more, and recharge substantially less of the typical annual water budget. LSCE (2013) calculated infiltration of precipitation based on the difference between total monthly streamflow at selected gaging stations and total monthly precipitation for the gages' drainage area. Streamflow volumes include both direct runoff (overland flow and interflow) and baseflow Inclusion of baseflow with direct runoff in these calculations may from groundwater. inappropriately reduce the estimated volume of water infiltrated into the soil and available for recharge; the LSCE approach therefore tends to underestimate groundwater recharge. Additionally, many of the gauging stations used for the analysis are located in reaches that may be significantly influenced by upstream reservoir releases, surface water diversions, groundwater abstraction, and/or surface water groundwater exchanges, further complicating the interpretation of the LSCE (2013) runoff rates and the interrelated calculations of AET and recharge rates. In contrast, the SWB model presented here is based on calibrated parameter values developed for a similar model in Sonoma County which was calibrated to gauges specifically selected to minimize the effects of reservoir releases, water use, or significant surface water/groundwater interaction, and after separating and removing the baseflow component of streamflow.

The recharge estimates presented here arguably represent the best available county-wide estimates produced at a fine spatial resolution using a consistent and objective data-driven approach. This analysis focused on two Water Years, 2010 and 2014, which represent average and drought conditions respectively. Input parameters were determined based on literature values and values calibrated through prior modeling experience in Sonoma County.



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