

BMI ENGINEERING INC.

17337 Ventura Blvd. Suite 310, Encino CA 91316 Tel: (818)901-8009
e-mail address: bmicalifornia@gmail.com

February 02, 2024

Hydrology Report and Calculations

Santa Maria Airport
Planes of Fame Museum
3217 Terminal Drive
Santa Maria, CA 93455



PREPARED UNDER THE DIRECT SUPERVISION OF:
Ben Manesh, RCE NO. 38454 EXP 3-31-2025

BMI ENGINEERING INC.

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Introduction:

This project involves the construction of four buildings namely A-Alpha, B-Bravo, C-Charlie and D-Delta of which buildings Alpha and Delta will be built as part of the first phase and Bravo and Charlie will be a future date to be determined. The current site is an open field facing Airpark Drive to the north, Radisson Hotel to the West, and existing airport runway to the south and an existing debris basin to the east. A site plan has been provided in Appendix A.

Procedure:

This hydrology report is prepared based on the Santa Barbara County Flood Control and Water Conservation District with an effective date of January, 2011. Our development will comply with all applicable requirements including Santa Barbara County's Floodplain Management Ordinance (Santa Barbara County Code (SBCC) Chapter 15A, "Floodplain Management"), Santa Barbara County's Setback Ordinance (SBCC Chapter 15B, "Development Along Watercourses") and SBCC Chapter 24 "Offenses, Miscellaneous" Section 24-7, "Watercourses – Erecting buildings, etc., which obstruct flow prohibited."

A site plan of the proposed development showing the limits of the special flood hazard areas and base flood elevations as they appear on the most current Federal Emergency Management Agency (FEMA) flood insurance rate map (FIRM) found at <http://msc.fema.gov/> is shown in the figure below. As shown on the site plan, our development is not inside a special flood hazard area, and is also not in or near a watercourse.

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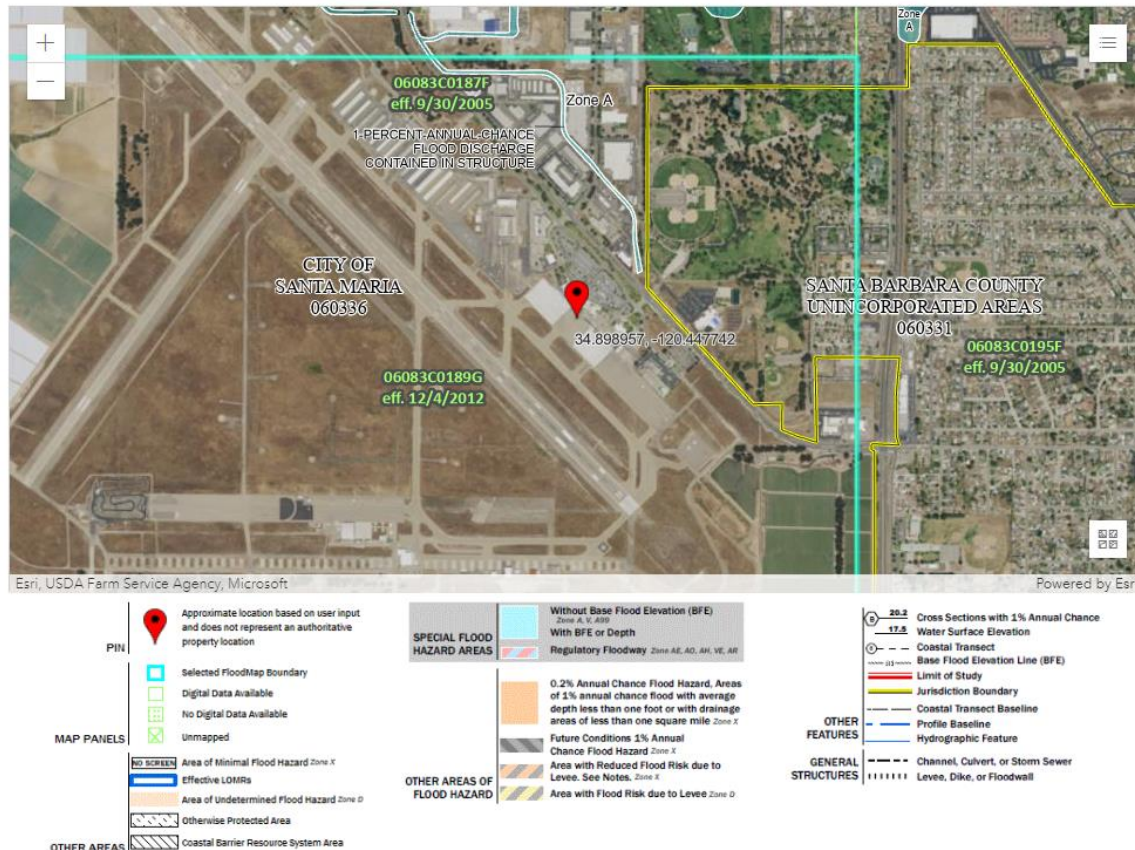


Figure 1: Federal Emergency Management Agency (FEMA) flood insurance rate map (FIRM)

In this hydrology report we will determine the runoff quantities for before and after development stages using the modified rational method. In developing the report, we used rainfall intensity curves and rainfall coefficient vs. rainfall intensity curves incorporated into the District's Program Rational-XL found at <https://www.countyofsb.org/pwd/water/downloads.htm>. Said curves can be found in Appendix B.

Bioretention basins are proposed to reduce the post development peak runoff by an amount equal to the difference between the post development and pre development runoff quantities. Bioretention basins were chosen for this site to facilitate and have minimum interference with airport functions. Using this method, we were able to capture the excess storm runoff that was produced due to the

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construction of the proposed buildings. In effect the storm runoff quantity in the adjoining streets will be unchanged during any storm occurrence.

We retained the services of Earth Systems to determine the percolation rates of storm runoff into our site which will be used to determine the ponding depth of the bioretention basins as a means to mitigate the increased runoff.

A copy of which can be found in Appendix C.

Our design strategy of this project includes permanent overflow pipes to direct overflow of the storm runoff into the adjacent front street (Airpark Drive) or service roads in case of full saturation of the bioretention basin as recommended by the Santa Barbara County Flood Control Standard Conditions.

Our project is not fronting on any arterial major or industrial street classifications and no studies for this subject were provided.

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Project Characteristics:

The actual area that was leased by our client is 22.54 Acres. The leased area includes a debris basin that is 2.92 acres and a gravel parking lot on the north side of Airpark Drive that is 7.51 acres. The debris basin and gravel parking lot areas are not tributary to the development site of the four buildings and therefore were excluded from our hydrology study.

Note:

We prepared a separate hydrology calculation for the tributary area to the existing debris basin and determined the volume and height of storm water to be less than one inch. The gravel parking lot to the north is to remain unchanged and is on the opposite side of Airpark Drive relevant to our construction site. Therefore, it is safe to preclude these areas from our construction site hydrology study. Please refer to Appendix D.

The actual area to be used for our hydrology study is 12.11 acres. The site is naturally split into two sections based on the existing topography. The southern portion flows in a southwesterly direction (Area #1), and the northern portion (Area #2) flows in a northwesterly direction. Please refer to Appendix E.

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Determining the Peak Mitigated Flow Rate:

The peak mitigated flow is calculated using the modified rational method Santa Barbara County Flood Control and Water Conservation District (Program Rational-XL). Time of concentration is equal to:

$$T_C = T_T + T_L \text{ (Travel Time + Lot Time)}$$

In this case travel time is zero.

T_L (Lot Time) = 10 minutes (for urban areas as per Standard Conditions of Project Plan Approval)

$$T_C = 0 + 10 = 10 \text{ minutes}$$

The minimum time of concentration per Santa Barbara County Flood Control and Water Conservation District is 12 minutes, therefore, we will use 12 minutes as our time of concentration.

As per S.B.C.F.D. Standard Conditions of Project Plan Approval: *New developments shall mitigate for the increased runoff by directing drainage to an acceptable watercourse, improving downstream facilities, **mitigating the increased runoff on-site**, and/or as otherwise required by the Public Works Director.*

For this project we are mitigating the increased runoff on-site.

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Area #1:

Pre-Construction:

- Input Data:
1. Location = Santa Maria - Orcutt
 2. Land Use Type = Agriculture
 3. Acres = 6.04
 4. Time of Concentration = 12 minutes

Note: Santa Barbara County Flood Control and Water Conservation District minimum is 12 minutes

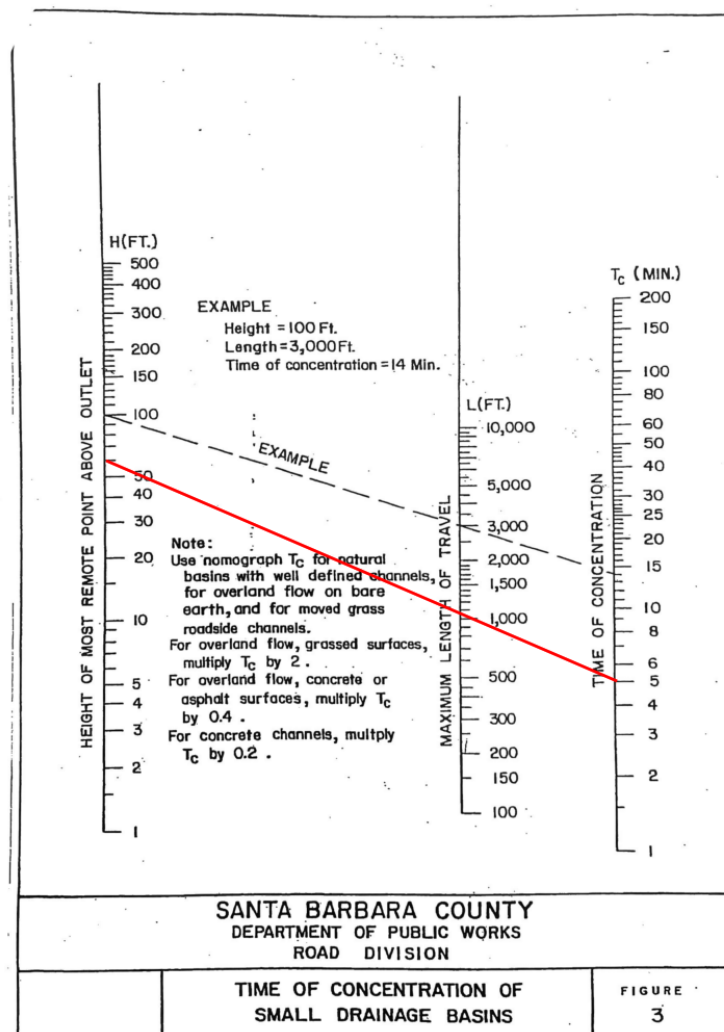


Figure 2: Nomograph for Pre-Construction Area #1

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Program Rational - XL User Form

Santa Barbara County Flood Control and Water Conservation District
Program Rational - XL

User Data:

Project Name: Santa Maria
Date of Run: 1/31/2024 Run By:
Notes:

Input Data:

Location: Santa Maria - Orcutt Land Use Type: Agriculture
Area (Acres): 6.04 Time of Concentration (Min.): 12
Calculated Runoff Coefficient: Q10: 0.38 Q25: 0.46 Q50: 0.50 Q100: 0.54
User Selected Runoff Coefficient (Optional): Calculate

For Large Lot Subdivisions (>10,000 sq. ft.):

Low Value: High Value: User Selected:
Q10: Q25: Q50: Q100: Enter Selection

Results:

	Rainfall Intensity:	Runoff Coef:	Q (cfs):
Q10:	1.92	0.38	4
Q25:	2.34	0.46	7
Q50:	2.63	0.50	8
Q100:	2.93	0.54	10

View RI Curves Print
View RC Curves Exit

Post-Construction:

- Input Data:
1. Location = Santa Maria - Orcutt
 2. Land Use Type = Commercial
 3. Acres = 6.04
 4. Time of Concentration = 12 minutes

Note: Santa Barbara County Flood Control and Water Conservation District minimum is 12 minutes

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Program Rational - XL User Form

Santa Barbara County Flood Control and Water Conservation District
Program Rational - XL

User Data:

Project Name: Santa Maria
Date of Run: 1/31/2024 Run By:
Notes:

Input Data:

Location: Santa Maria - Orcutt Land Use Type: Commercial
Area (Acres): 6.04 Time of Concentration (Min.): 12
Calculated Runoff Coefficient: Q10: 0.69 Q25: 0.72 Q50: 0.74 Q100: 0.75
User Selected Runoff Coefficient (Optional): Calculate

For Large Lot Subdivisions (>10,000 sq. ft.):

	Low Value:	High Value:	User Selected:
Q10:			
Q25:			
Q50:			
Q100:			

Enter Selection

Results:

	Rainfall Intensity:	Runoff Coef:	Q (cfs):
Q10:	1.92	0.69	8
Q25:	2.34	0.72	10
Q50:	2.63	0.74	12
Q100:	2.93	0.75	13

View RI Curves Print
View RC Curves Exit

The increased amount of runoff due to the new development is:
 $13 - 10 = 3$ cfs to be mitigated

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Area #2:

Pre-Construction:

- Input Data:
1. Location = Santa Maria - Orcutt
 2. Land Use Type = Agriculture
 3. Acres = 6.07
 4. Time of Concentration = 12 minutes

Note: Santa Barbara County Flood Control and Water Conservation District minimum is 12 minutes

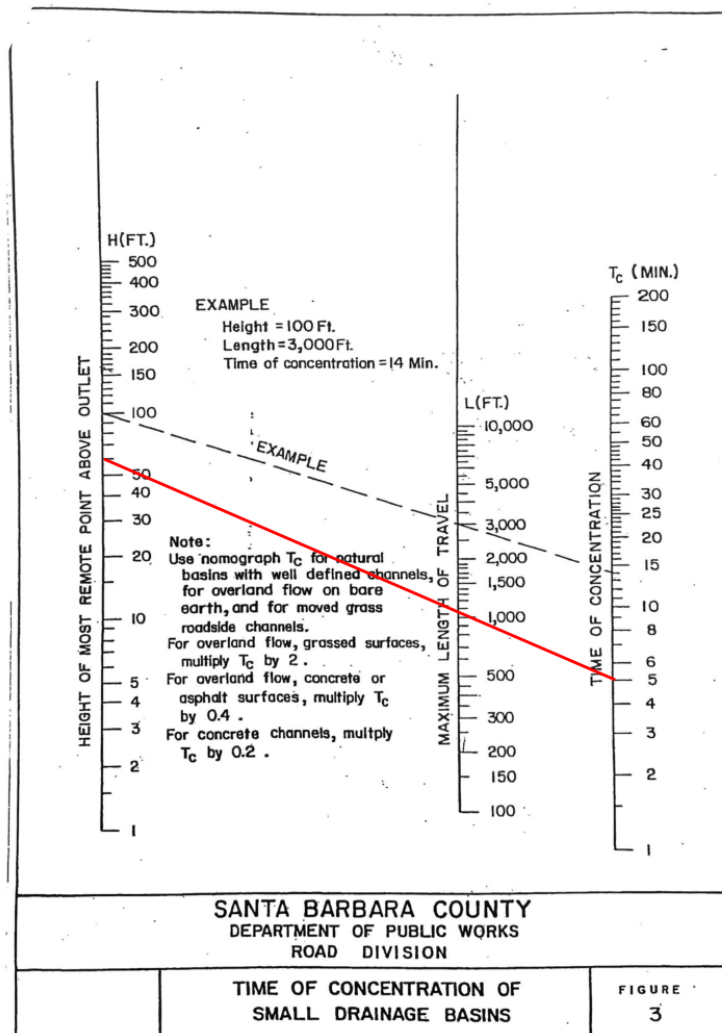


Figure 3: Nomograph for Pre-Construction Area #2

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Program Rational - XL User Form

Santa Barbara County Flood Control and Water Conservation District
Program Rational - XL

User Data:

Project Name: Santa Maria
Date of Run: 1/31/2024 Run By:
Notes:

Input Data:

Location: Santa Maria - Orcutt Land Use Type: Agriculture
Area (Acres): 6.07 Time of Concentration (Min.): 12
Calculated Runoff Coefficient: Q10: 0.38 Q25: 0.46 Q50: 0.50 Q100: 0.54
User Selected Runoff Coefficient (Optional): Calculate

For Large Lot Subdivisions (>10,000 sq. ft.):

Low Value: High Value: User Selected:
Q10: Q25: Q50: Q100: Enter Selection

Results:

	Rainfall Intensity:	Runoff Coef:	Q (cfs):
Q10:	1.92	0.38	4
Q25:	2.34	0.46	7
Q50:	2.63	0.50	8
Q100:	2.93	0.54	10

View RI Curves Print
View RC Curves Exit

Post-Construction:

- Input Data:
1. Location = Santa Maria - Orcutt
 2. Land Use Type = Commercial
 3. Acres = 6.07
 4. Time of Concentration = 12 minutes

Note: Santa Barbara County Flood Control and Water Conservation District minimum is 12 minutes

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Program Rational - XL User Form

Santa Barbara County Flood Control and Water Conservation District
Program Rational - XL

User Data:

Project Name: Santa Maria
Date of Run: 1/31/2024 Run By:
Notes:

Input Data:

Location: Santa Maria - Orcutt Land Use Type: Commercial
Area (Acres): 6.07 Time of Concentration (Min.): 12
Calculated Runoff Coefficient: Q10: 0.69 Q25: 0.72 Q50: 0.74 Q100: 0.75
User Selected Runoff Coefficient (Optional): Calculate

For Large Lot Subdivisions (>10,000 sq. ft.):

Low Value: High Value: User Selected:
Q10: Q25: Q50: Q100: Enter Selection

Results:

	Rainfall Intensity:	Runoff Coef:	Q (cfs):
Q10:	1.92	0.69	8
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Q50:	2.63	0.74	12
Q100:	2.93	0.75	13

View RI Curves Print
View RC Curves Exit

The increased amount of stormwater due to the new development is:
 $13 - 10 = 3$ cfs to be mitigated

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Calculation of Bioretention Basin used to mitigate the increased runoff:

Per Appendix C we took the average of 6 infiltration rate test locations as shown on said report by Earth Systems. The average infiltration rate per test location is the following:

Test Location 1 = 29.42 inches/hour

Test Location 2 = 55.69 inches/hour

Test Location 3 = 29.67 inches/hour

Test Location 4 = 23.33 inches/hour

Test Location 5 = 29.33 inches/hour

Test Location 6 = 16.20 inches/hour

The worst infiltration rate is test location 6 = 16.20 inches/hour = 1.35 ft/hr

Area #1 - Calculation of Bioretention Basin:

For a storm duration of 12 minutes the required volume to be mitigated is:

$3 \text{ cfs} \times 12 \text{ min} \times 60 \text{ sec/min} = 2160 \text{ Cu. Ft.}$

New Bioretention Basin #1 Size = 15,214 sf

Assuming there is no infiltration (Very Conservative)

Depth of standing water in Bioretention Basin #1 = $2160 \text{ cf} / 15,214 \text{ sf} = 0.14 \text{ ft} = 1.7 \text{ inches}$

We will use a ponding depth of 3 inches which achieves a 1.75 factor of safety.

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Area #2 - Calculation of Bioretention Basin:

For a storm duration of 12 minutes the required volume to be mitigated is:

$3 \text{ cfs} \times 12 \text{ min} \times 60 \text{ sec/min} = 2160 \text{ Cu. Ft.}$

Assuming there is no infiltration (Very Conservative)

New Bioretention Basin #2 Size = $30 \text{ ft} \times 160 \text{ ft} = 4800 \text{ sf}$

We will use 2 separate bioretention basins = $4800 \text{ sf} \times 2 = 9600 \text{ sf}$

Depth of standing water in Bioretention Basins = $2160 \text{ cf} / 9600 \text{ sf} = 0.225 \text{ ft} = \mathbf{2.7 \text{ inches}}$

We will use a ponding depth of 4 inches which achieves a 1.50 factor of safety.

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APPENDIX A:

NO
CONSTRUCTION
IN THIS AREA
(127,436 S.F. /
2.92 ACRES)

ACCESS ROAD

TO LEASE LINE

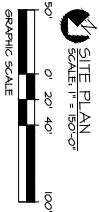
LEASE LINE /
PROPERTY LINE

LEASE LINE /
PROPERTY LINE

AIRPARK DRIVE

SKYWAY DRIVE

LEASE LINE /
PROPERTY LINE



10' CONC. SIDE WALK

10' CONC. SIDE WALK

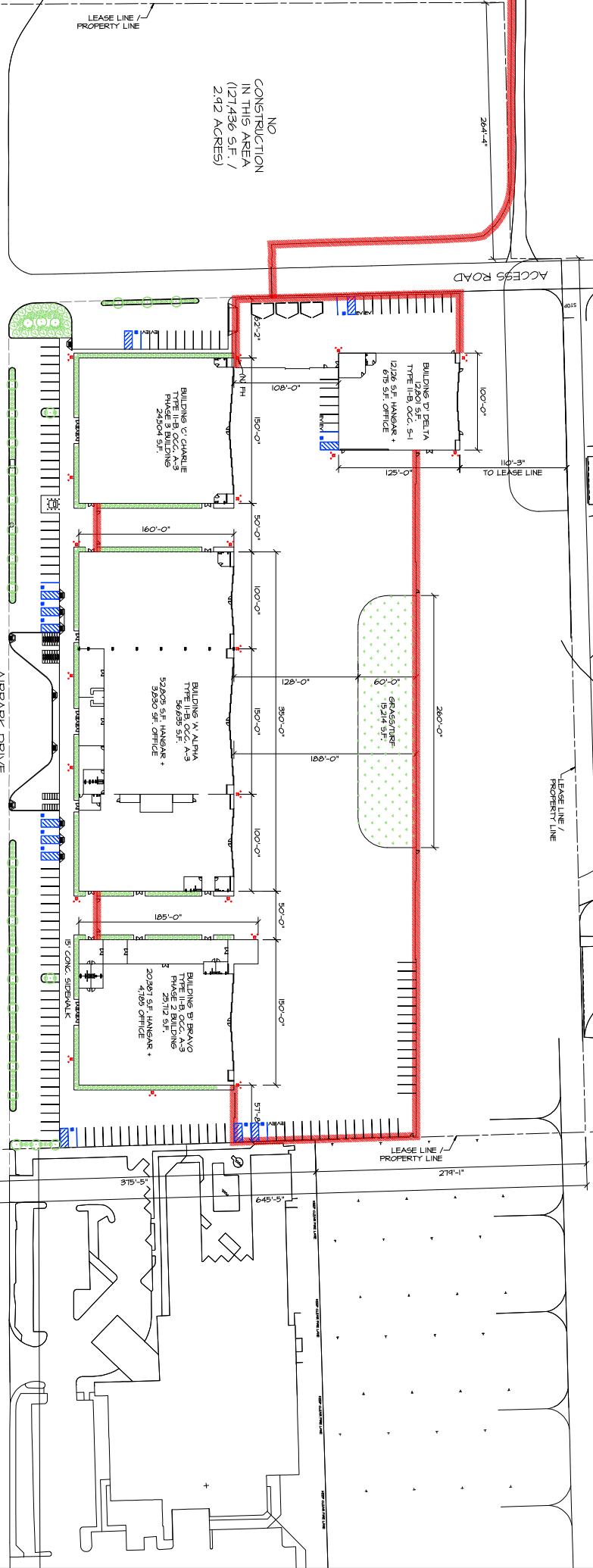
10' CONC. SIDE WALK

10' CONC. SIDE WALK

10' CONC. SIDE WALK

10' CONC. SIDE WALK

10' CONC. SIDE WALK



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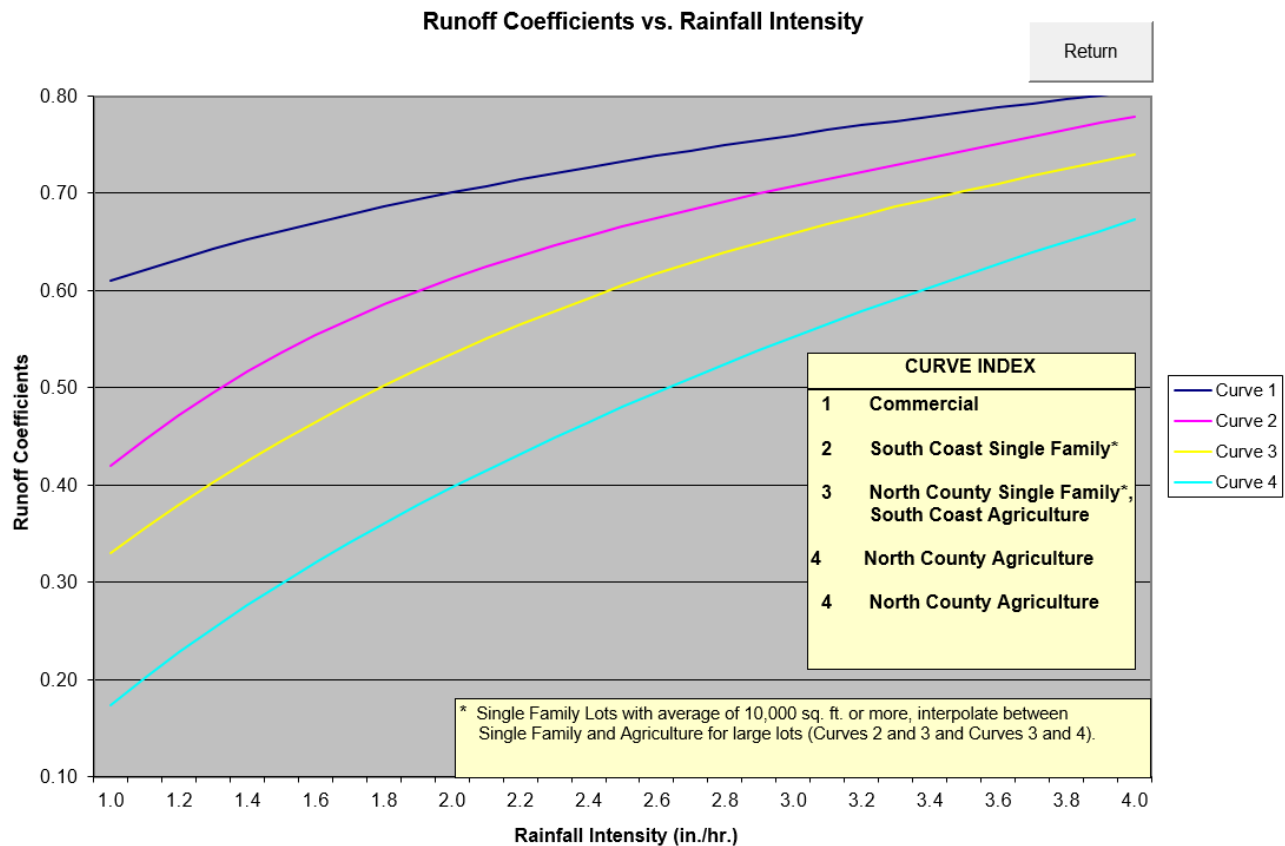
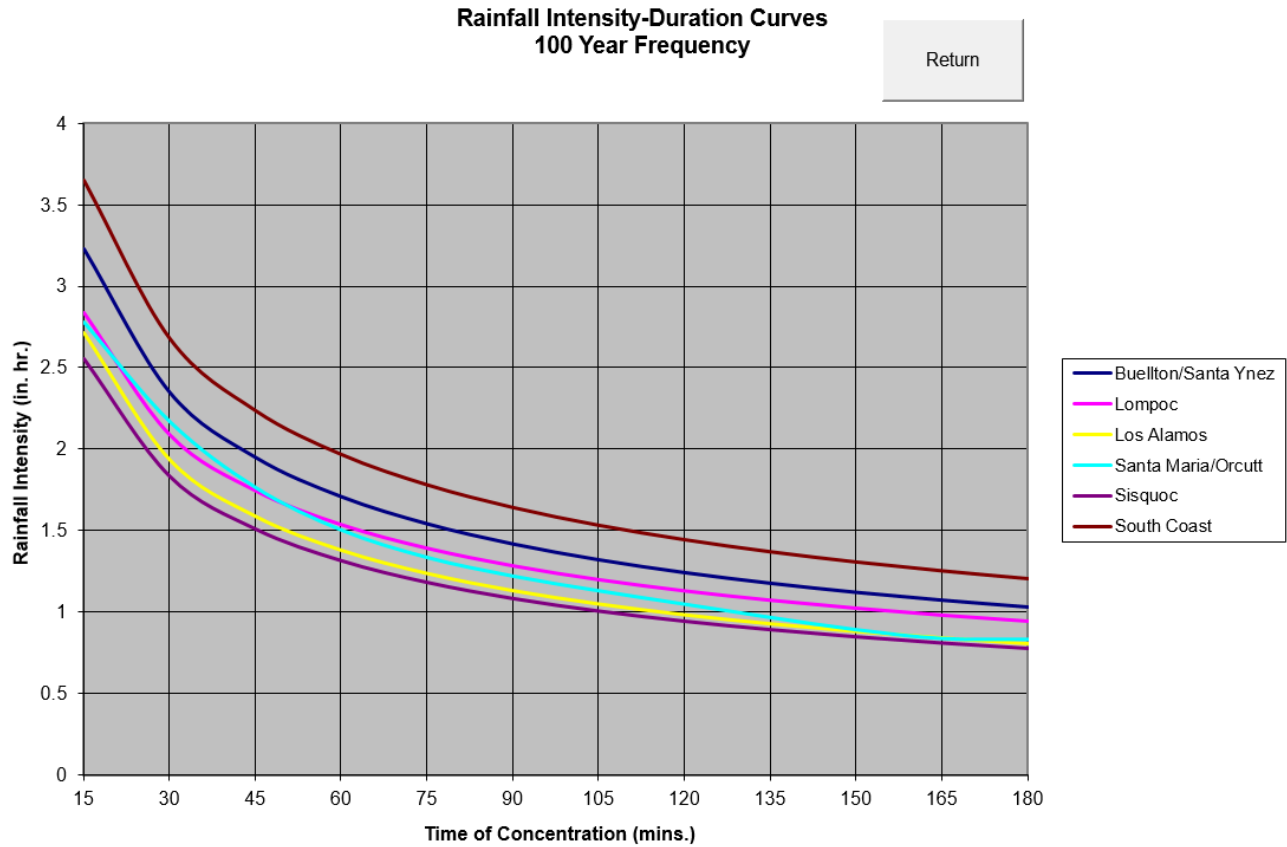
APPENDIX B:

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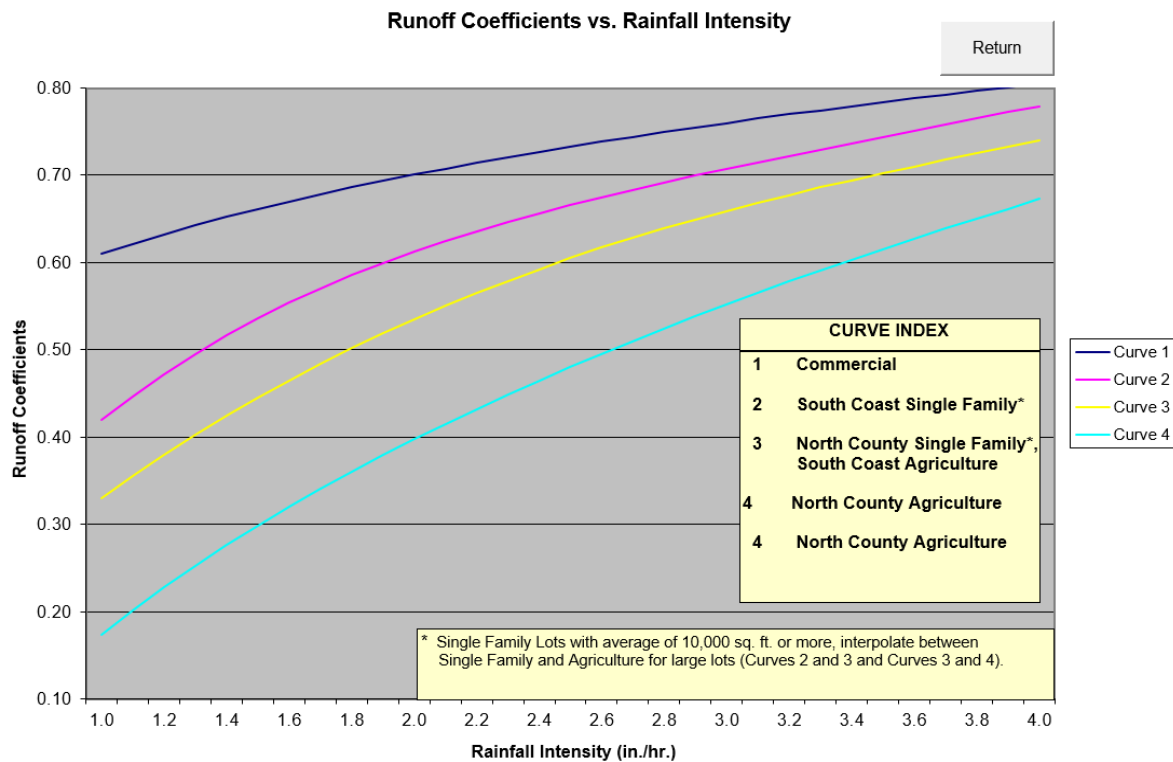
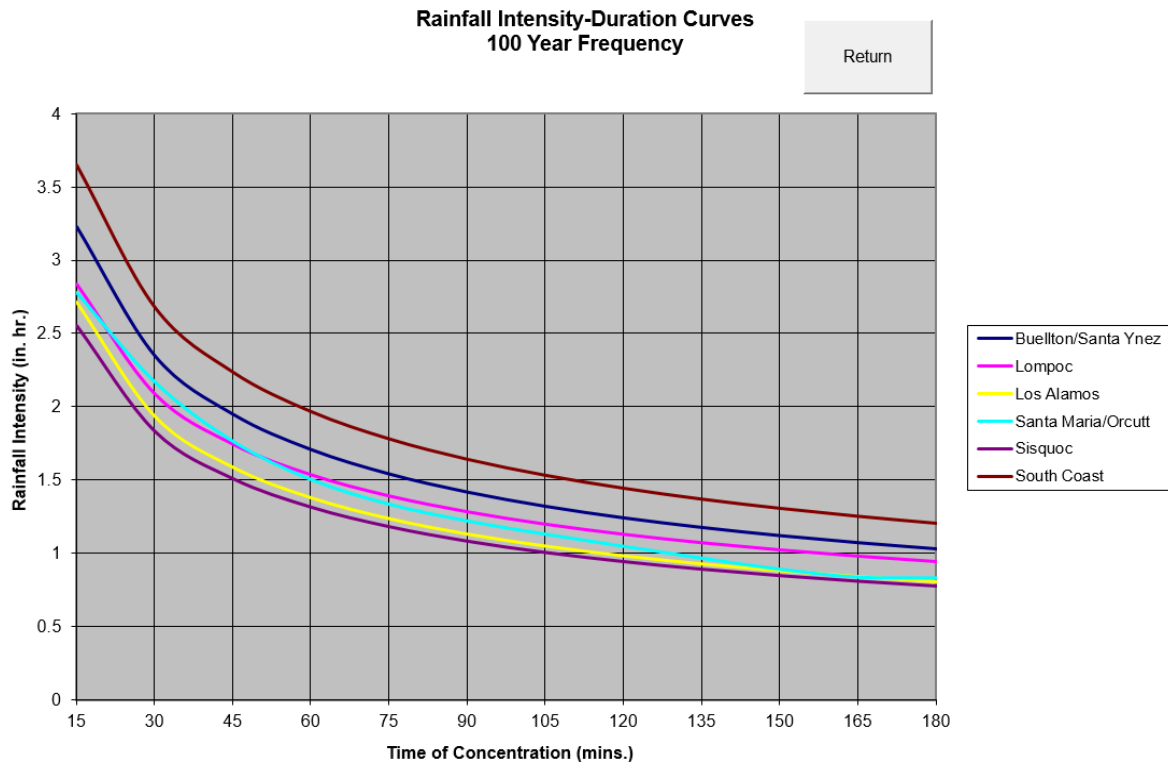
Area #1 Curves for Pre-Construction



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e-mail address: bmicalfornia@gmail.com

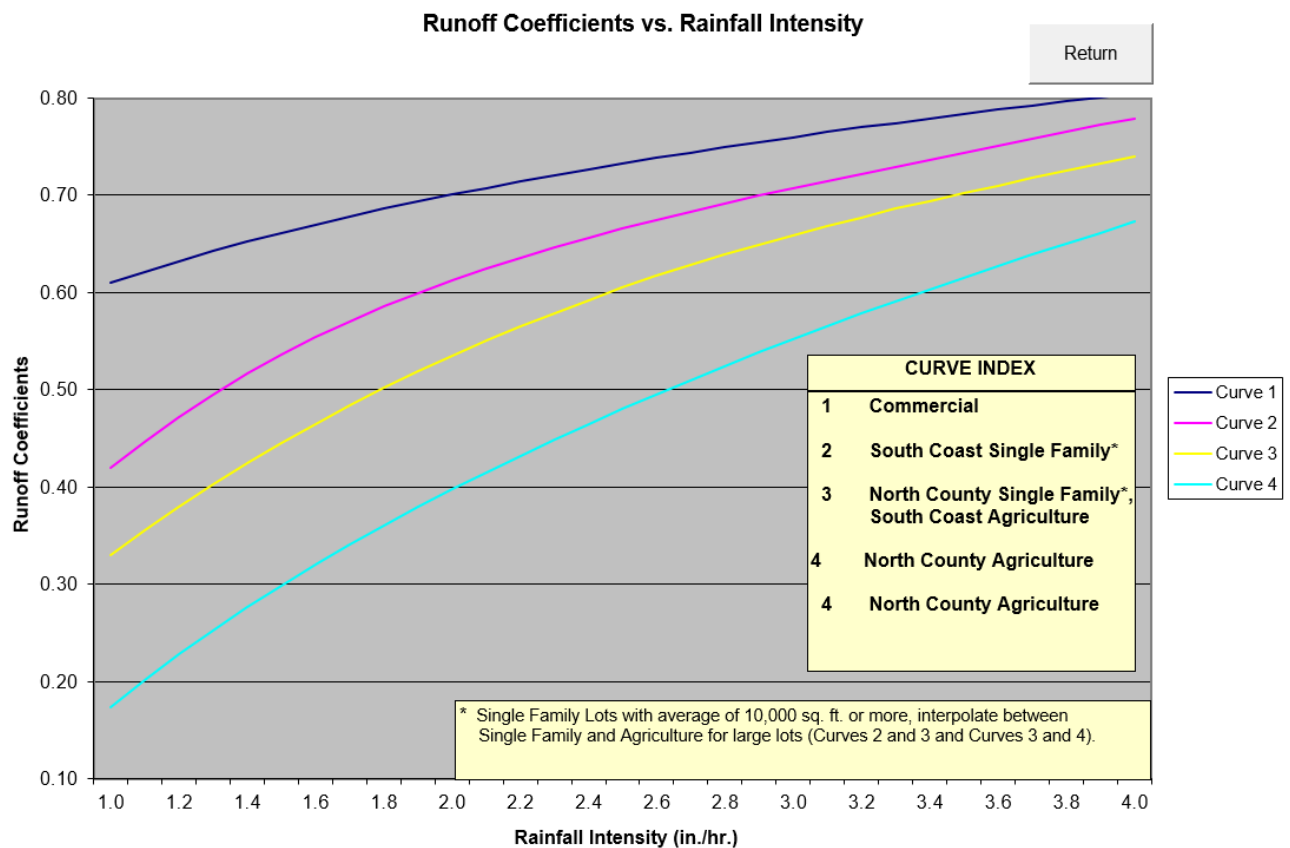
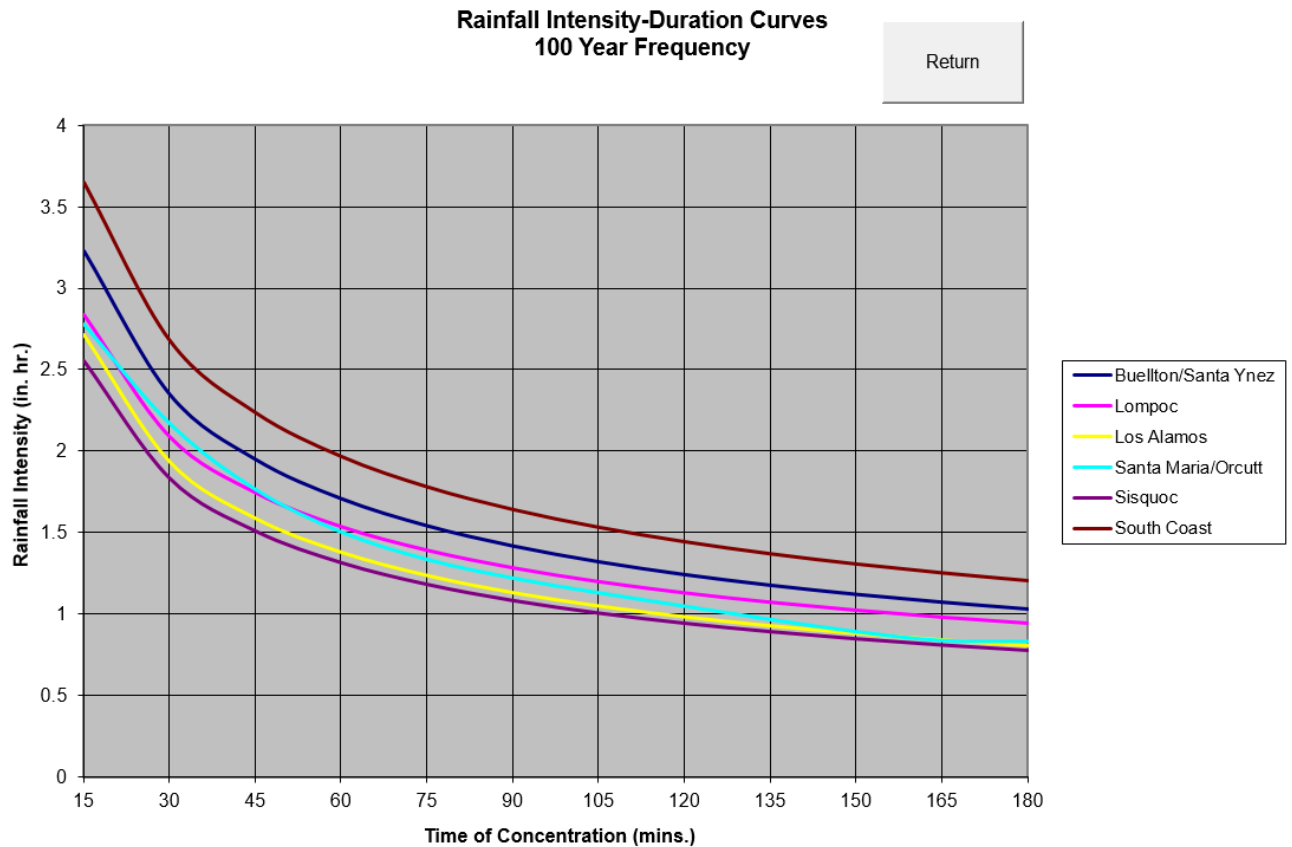
Area #1 Curves for Post-Construction



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Area #2 Curves for Pre-Construction

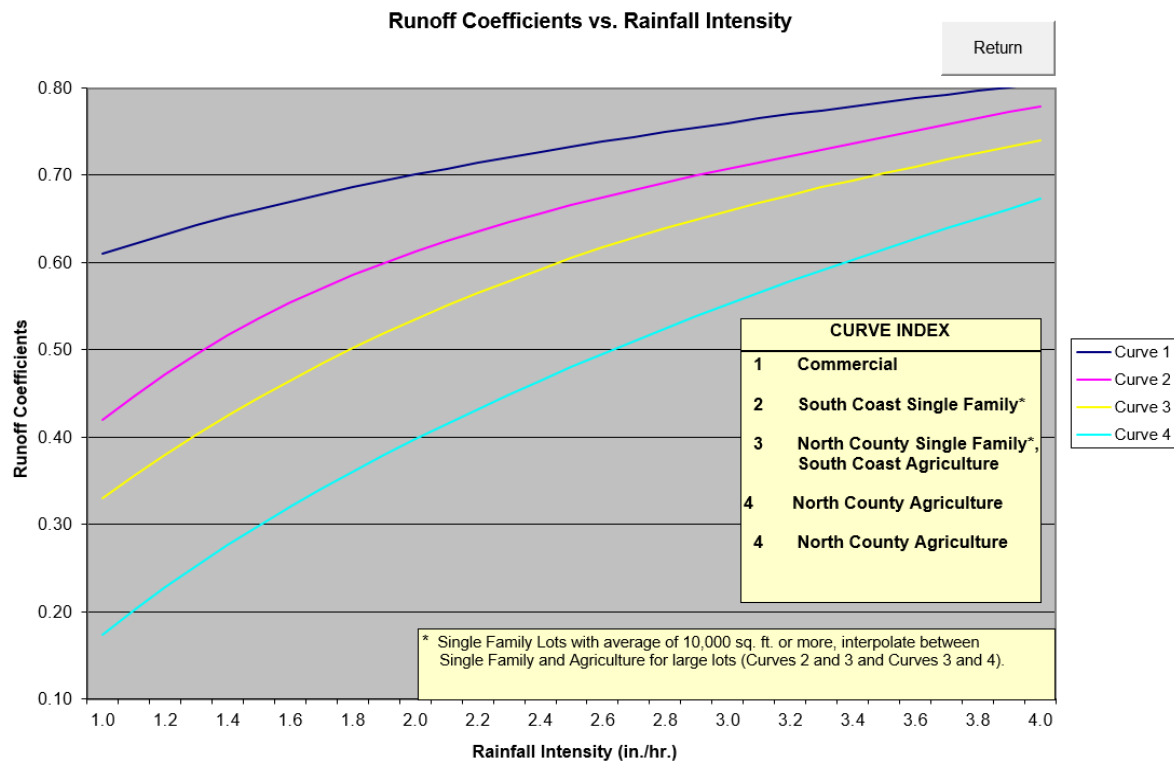
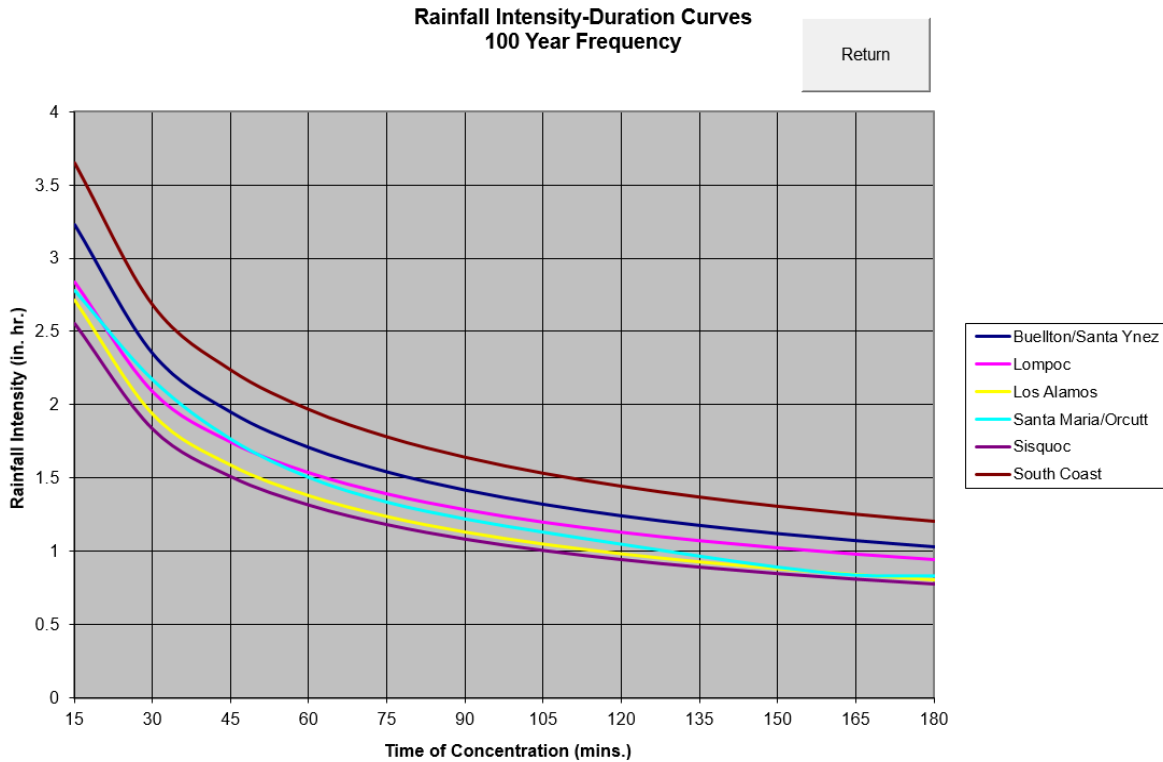


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Area #2 Curves for Post-Construction



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APPENDIX C:



August 4, 2023

FILE NO.: 305581-002

Mr. Steve Argubright
Argubright Steel Structures and Doors
18140 Index Street
Northridge, California 91326

PROJECT: PLANES OF FAME AIR MUSEUM – SANTA MARIA PUBLIC AIRPORT
AIRPARK DRIVE
SANTA MARIA, CALIFORNIA

SUBJECT: Infiltration Testing Results

REF: 1) Proposal for Infiltration Testing, Planes of Fame Air Museum – Santa Maria Public Airport, Airpark Drive, Santa Maria, California, Doc. no. SLO-2305.077.PRP, dated May 24, 2023
2) Geotechnical Engineering Report, Planes of Fame Air Museum – Santa Maria Public Airport, Southeast End of Airpark Drive, Santa Maria, California, Doc. no. SLO-2211-050.SER, dated November 14, 2022

Dear Mr. Argubright:

In accordance with your authorization of the above referenced proposal, this letter has been prepared to summarize the results of the infiltration testing performed on June 27, 2023 for the planned Planes of Fame Air Museum project.

INTRODUCTION AND SCOPE OF SERVICES

Infiltration testing was performed in two locations for the planned Planes of Fame Air Museum to be located at the Santa Maria Public Airport in Santa Maria, California. Three infiltration tests (I1-I3) were performed southwest of proposed building A, to depths ranging from approximately 18.0 to 23.0 feet below ground surface (bgs). The remaining three tests (I4-I6) were performed in the northeast corner of the site to depths of approximately 5.0 feet bgs. The client had requested I1-I3 to be performed inside the airport property fence line, as shown on the attached exploration map. Results from I1-I3 can be translated to the desired locations due to similarities in subgrade conditions as shown in the referenced geotechnical report.

Our scope of services for this project was to drill two sets of three infiltration test borings in the project area for infiltration testing and report data collected during field testing. Field reconnaissance and data were then analyzed for the project. Results of the SCM infiltration testing are summarized and presented in this report.

SCM INFILTRATION TESTING

The borings for infiltration tests were drilled with a GTech GT8 truck-mounted drill rig to depths ranging from approximately 5.0 to 23.0 feet bgs. After drilling operations were completed, a 4-inch diameter



perforated pipe was installed within each of the six infiltration test borings and the annular spaces around the pipes were filled with gravel. Infiltration testing was performed in general accordance with the methods developed by this firm in cooperation with the Central Coast Low Impact Development Initiative (ESP 2013).

Testing initially consisted of introducing water into each of the test borings to just below existing grade. This water level was then maintained at a constant head elevation for 10 minutes. After the 10-minute period, the water supply was shut off and the amount of water introduced into each of the test borings was recorded. Readings of the change in water level or falling head elevations were then recorded at various time intervals over a period of approximately 2 hours. Following testing, the pipes were removed, and the infiltration test borings were backfilled with on-site soil.

Constant head infiltration testing resulted in introducing 2 to 18.5 gallons of water over a period of 10 minutes. Falling head testing resulted in measured infiltration rates varying from 11 to 14 inches per hour near the end of the tests. The LID infiltration test results are attached.

The test results only indicate the infiltration rate at the specific locations tested and under specific conditions. Sound engineering judgment should be exercised in extrapolating the test results for other conditions or locations. Technical design references vary in methods they present for using these types of test results. However, most references include reduction, safety, and/or correction factors for several parameters including, but not limited to, size of the LID system relative to the test volume, number of tests conducted, variability in the soil profile, anticipated silt loading, anticipated biological buildup, anticipated long-term maintenance, and other factors. Typically, in aggregate these factors range from about 2.5 to 50 depending upon the method used. The final determination of the means by which these data are used is left to the design engineer.

We appreciate the opportunity to have provided services for this project and look forward to working with you again in the future. If there are any questions concerning this report, please do not hesitate to contact us.

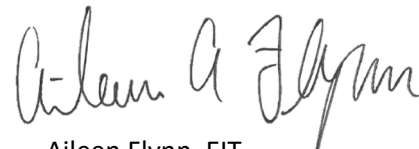
Sincerely,

Earth Systems Pacific


Robert Down, PE
Principal Engineer

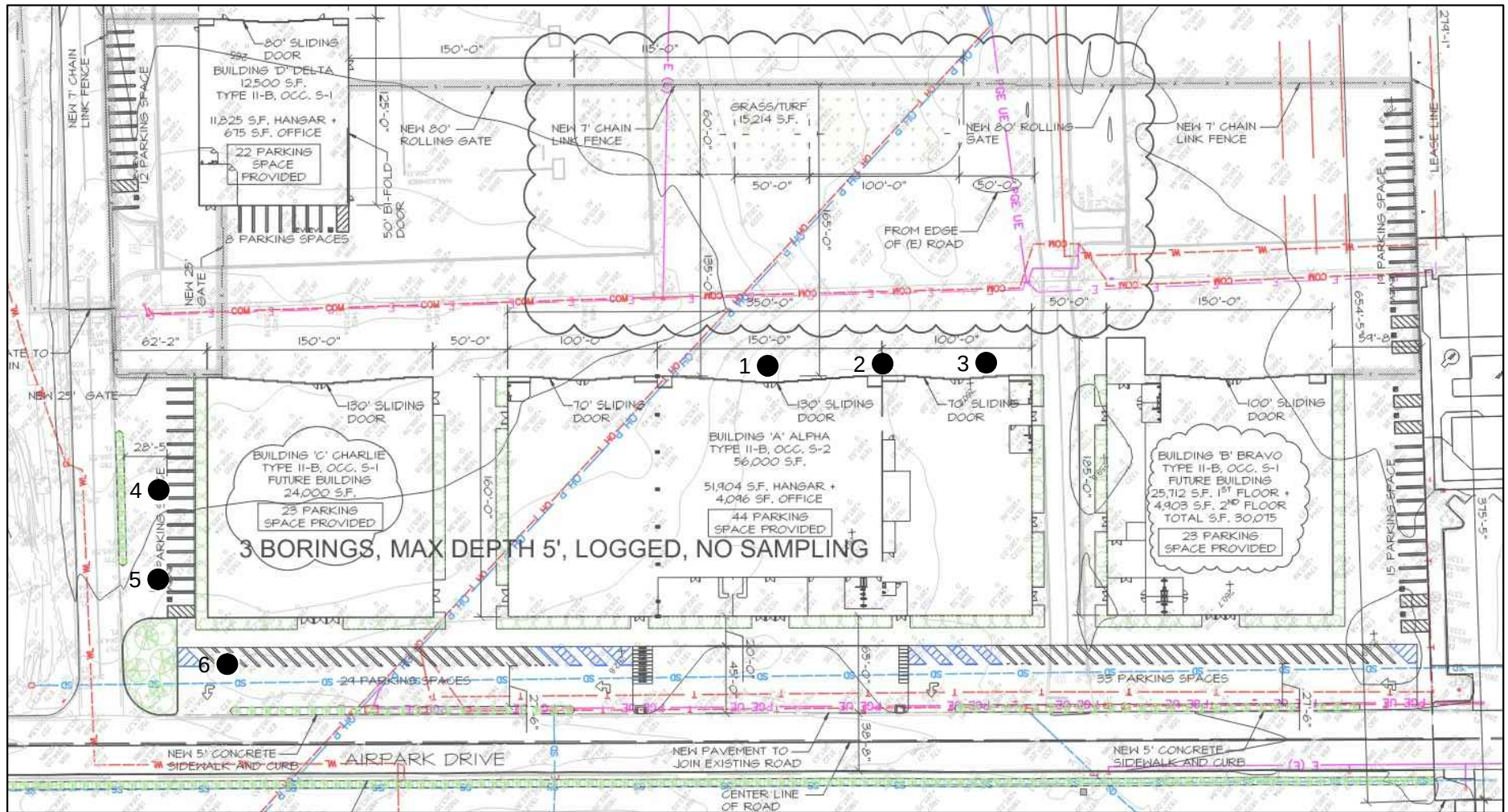



Sydney Johnson
Project Manager


Aileen Flynn, EIT
Staff Engineer

Attachments: Exploration Location Map (1 page)
Infiltration Test Results (6 pages)

Doc. No.: 2307-085.LTR/cr



LEGEND

6 ● Infiltration Test Location (Approx.)

BASE MAP PROVIDED BY: Google Earth (2023)

NOT TO SCALE



Earth Systems Pacific

4378 Old Santa Fe Road, San Luis Obispo, CA 93401
www.earthsystems.com
 (805) 544-3276 • Fax (805) 544-1786

EXPLORATION LOCATION MAP

Planes of Fame Air Museum
 Santa Maria Public Airport
 Airpark Drive
 Santa Maria, California

Date
 July 2023

Project No.
 305581-002

Figure 1

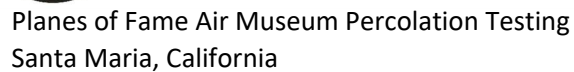
INFILTRATION TEST RESULTS

INFILTRATION TEST: I1

CONSTANT HEAD DATA

FALLING HEAD DATA

[illegible]



INFILTRATION TEST RESULTS

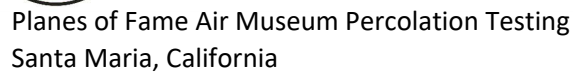
TEST HOLE DIAMETER: 6 inches

TEST HOLE DEPTH: 58 inches

TEST DURATION: 130 minutes

Time of Constant Head: 10 minutes
Volume Added During Constant Head: 3 gallons (0.4 cu.ft.)

[illegible]

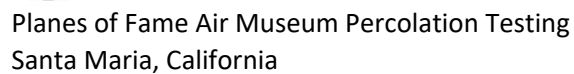


INFILTRATION TEST RESULTS

TEST DURATION: 130 minutes

Time of Constant Head: 10 minutes
Volume Added During Constant Head: 4 gallons (0.5 cu.ft.)

[illegible]



INFILTRATION TEST RESULTS

TEST DURATION: 130 minutes

Time of Constant Head: 10 minutes
Volume Added During Constant Head: 2 gallons (0.3 cu.ft.)

[illegible]

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APPENDIX D:

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Existing Debris Basin:

Program Rational - XL User Form

Santa Barbara County Flood Control and Water Conservation District

Program Rational - XL

User Data:

Project Name: Existing Debris Basin

Date of Run: 12/18/2023 Run By:

Notes:

Input Data:

Location: Santa Maria - Orcutt Land Use Type: Agriculture

Area (Acres): 2.92 Time of Concentration (Min.): 12

Calculated Runoff Coefficient: Q10: 0.38 Q25: 0.46 Q50: 0.50 Q100: 0.54

User Selected Runoff Coefficient (Optional):

For Large Lot Subdivisions (>10,000 sq. ft.):

Low Value: High Value: User Selected:

Q10: Q25: Q50: Q100:

Enter Selection

Results:

	Rainfall Intensity:	Runoff Coef:	Q (cfs):
Q10:	1.92	0.38	2
Q25:	2.34	0.46	3
Q50:	2.63	0.50	4
Q100:	2.93	0.54	5

View RI Curves Print

View RC Curves Exit

For a storm duration of 12 minutes the required volume to be mitigated is:

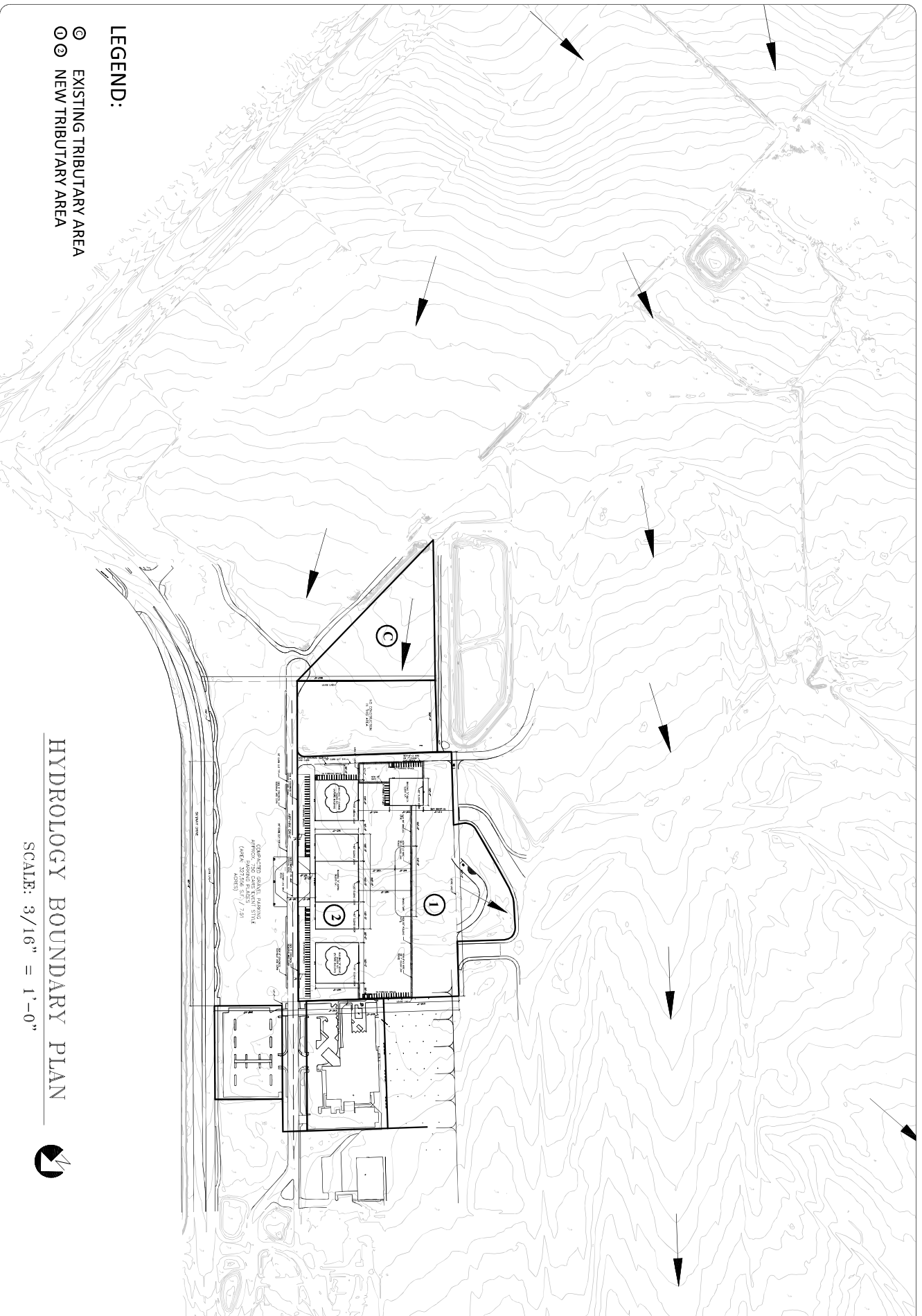
$$5 \text{ cfs} \times 12 \text{ min} \times 60 \text{ sec/min} = 3600 \text{ Cu. Ft.}$$

$$\text{Existing Debris Basin Area} = 127,195.2 \text{ Sq. Ft.}$$

Assuming there is no infiltration (Very Conservative)

$$\text{Depth of standing water in the existing basin} = 3600 / 127195.2 = 0.028 \text{ inches}$$

Therefore the basin captures all the tributary runoff area from the west.



LEGEND:

- ⊙ EXISTING TRIBUTARY AREA
- ⊙⊙ NEW TRIBUTARY AREA

HYDROLOGY BOUNDARY PLAN

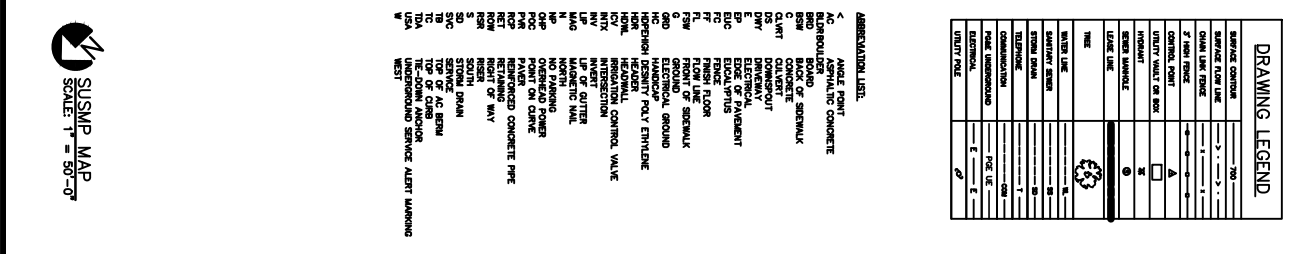
SCALE: 3/16" = 1'-0"



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APPENDIX E:



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5	GRADING & DRAINAGE PLAN		
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DATE: **01/31/202**
SCALE:
JOB :
SHEET:
OF SHEETS