
APPENDIX O



June 30, 2025
Job No.: M086-010

MEMORANDUM

TO: Dara Dungworth – City of Davis
Stan Gryczko – City of Davis

CC: Bre Moebius – Hefner Law
Jason Cole – Black Oak Design, Inc.

FROM: Andrea J. Bellanca, P.E., Principal

SUBJECT: Willowgrove
Central Valley Flood Protection Board
Letter Dated August 13, 2024 – Additional Flows to the Davis Drain.
Davis, CA

Per the letter from the Central Valley Flood Protection Board dated August 13, 2024, the “Board Staff recommend the DEIR include information on how the City of Davis will address these additional flows to the Davis Drain”.

The Preliminary Stormwater Control Plan (SCP) for the Shriner’s Property prepared in June 2025 outlines the requirements for the Project to mitigate peak runoff flows such that the post-development condition is mitigated to less than the pre-development flows. This mitigation of peak flow is shown in Table 4-4. (Exhibit A). Because the post-development flows are all less than the pre-development flows, there is no impact to the Davis Drain due to peak flow.

In addition to peak flow, Table 4-4 of the SCP shows the increase in total volume of stormwater runoff for the various storm events. The largest increase in total volume of runoff occurs during the 200-year, 10-day storm, with a total increase in volume of runoff of 19.2-acre feet.

To mitigate this increase in post-development runoff volume, we are proposing to provide additional volumetric storage on-site.

First, the proposed detention basins along the eastern property line have been oversized so they provide extra volumetric storage during the large storm events. The detention basins have been sized to provide 44.50-acre feet of storage during the 200-year event.

Additional volumetric storage will be provided on-site by an “Overflow” basin (Exhibit B) located to the north of the Davis Drain (Channel A). Channel A is connected to Willow Slough Bypass which in turn is connected to the Yolo Bypass. Per the FEMA FIRM panels, the 100-year WSE at Willow Slough at the confluence of Yolo Bypass is 29.5 feet (halfway between Yolo Bypass BFE of 30 and 29 ft as annotated on FIRM panels) (Exhibit C). It is assumed the Channel A will backwater flood from Willow Slough at an elevation of 29.5. The proposed overflow basin provides 21.5-acre feet of new storage below this 100-year water surface elevation of 29.5. This is greater than the 19.2-acre feet of increased volumetric flow created by the development of the property.

The Project detention basins mitigate the peak flows to less than pre-development conditions. The ‘overflow’ basin provides excess volumetric storage to reduce the volume of flow being discharged from the Project to less than pre-development conditions. As a result, there is no impact to the Davis Drain from the development of the Project.

Exhibit A

- Lag time. DMA mapping information as provided in **Appendices C and D** was used to define the longest flow path lengths and length to centroid for each DMA, and that information was used along with the associated elevation data to calculate the lag time values, which are summarized in **Table 4-3**.

Table 4-3 Summary of Lag time parameters by drainage management area

DMA	n <i>(unitless)</i>	Flowpath				Elevation		Slope <i>(ft/mi)</i>	Lag Time	
		Longest		Centroid		max <i>(feet)</i>	min <i>(feet)</i>		(min)	(hours)
		<i>(feet)</i>	<i>(miles)</i>	<i>(feet)</i>	<i>(miles)</i>					
<i>Pre-project</i>										
1	0.115	5,435	1.029	3,685	0.698	38	28	9.7	110.5	1.84
2	0.115	3,948	0.748	2,000	0.379	35	28	9.4	81.8	1.36
3	0.115	2,120	0.402	1,155	0.219	36	34	5.0	61.7	1.03
<i>Post-project</i>										
1	0.035	4,060	0.769	2,955	0.560	38	24	18.2	25.6	0.43
2	0.04	4,180	0.792	2,000	0.379	38	24	17.7	26.1	0.44
3	0.115	1,560	0.295	630	0.119	36	25	37.2	32.7	0.55

4.2 Model Results

The HEC-HMS modeling results for pre- and post-project peak flow rate and maximum water surface elevation are summarized in **Table 4-4** and show reductions in predicted peak flow for all design storms.

Table 4-4 HEC-HMS Peak Flow and Volume Summary

Design Storm	Peak Discharge (cfs)		Runoff Volume (ac-ft)		Dual Basin	
	Pre	Post	Pre	Post	Max WSE (feet)	Freeboard (feet)
2-year, 24-hour	12.5	3.9	8.0	14.4	25.2	7.8
10-year, 24-hour	37.6	13.0	19.6	27.2	27.1	5.9
100-year, 24-hour	131.1	70.3	62.2	70.0	30.1	2.9
200-year, 24-hour	146.2	77.2	69.5	77.6	30.7	2.3
100-year, 10-day	124.6	84.6	173.7	192.4	31.1	1.9
200-year, 10-day	137.1	99.0	193.4	212.6	31.5	1.5

The "dual basin" in this case refers to the combined North and South Basins.

Exhibit B



*Figure not OCR compatible

Exhibit C

If you have questions about this map or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA-MAP (1-877-336-2627) or visit the FEMA website at www.fema.gov.

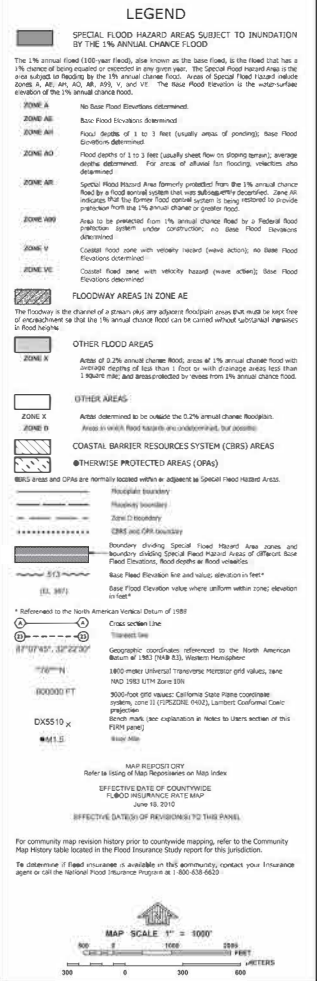


*Figure not OCR compatible