

LEVEL 3 DRAINAGE STUDY

PLNP2022-00353

Watershed: Hagginwood Creek Vertical Datum: NAVD-88

Job Number: 19682

Revised September 13, 2023

Revised August 16, 2023

April 12, 2023

RICK ENGINEERING COMPANY



PRELIMINARY DRAINAGE STUDY AND STORMWATER QUALITY STUDY

FOR SQX WATT

Job Number 19682



M. Scott Lillibridge R.C.E. #52504, Exp. 12/24 Region Manager

Prepared For:

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Revised September 13, 2023 Revised August 16, 2023 April 12, 2023

TABLE OF CONTENTS

1.0 INTRODUCTION
Figure 1: Vicinity Map1
2.0 HYDROLOGY2
2.1 On-Site Hydrology2
2.2 Peak Flow Attenuation
3.0 HYDRAULICS2
3.1 Inlets
3.2 Storm Drain System
3.3 Overland Release
4.0 WATER QUALITY3
4.1 Source Control
4.2 Hydromodification Control
4.3 LID, Treatment, and Full Trash Capture Control
4.4 Operation and Maintenance
5.0 ATTACHMENTS

List of Attachments

Attachment 1: FEMA FIRMette

Attachment 2: Hydrologic Calculations

Attachment 3: Hydraulic Calculations

Attachment 4: Stormwater Quality Conceptual Plan

Attachment 5: Water Quality Calculations

Attachment 6: Proposed Grading Plan

Attachment 7: Electronic Files



Revised September 13, 2023

SUBJECT: Preliminary Drainage Study and Stormwater Quality Study for SQX Watt

(RICK Job Number: 19682)

1.0 Introduction

This memorandum presents the results of the preliminary drainage and stormwater quality analysis prepared for the proposed SQX Watt project in Sacramento County, California. The proposed project site is located at 3438 Watt Ave and is shown on the vicinity map in Figure 1, below. The project site consists of one parcel of approximately 1.6 acres and is currently fully developed with an existing vacant building and parking lot. The proposed project is a multi-story self storage building with associated improvements.

The project site is shown on the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) number 06067C0069H, effective August 16, 2012. The project site is shown within FEMA Zone X (un-shaded), areas of minimal flood hazard, no FEMA submittals are anticipated for the project. An annotated FIRMette is included in Attachment 1.





Preliminary DS/WQ Storquest Watt September 13, 2023 Page 2 of 4

2.0 Hydrology

2.1. On-Site Hydrology

The Nolte flows for the proposed site were determined using SacCalc. A workmap for the hydrologic basins is included in Attachment 4. Output from the SacCalc Nolte model and backup information used to develop the model are included in Attachment 2.

Proposed condition HEC-1 hydrology for the 100-year, 24-hour storm has been calculated utilizing the SacCalc software. Output from the SacCalc models and backup information used to develop the SacCalc model are included in Attachment 2. Executable copies of the SacCalc models are included with the electronic files in Attachment 7. A copy of the projects grading plan is included in Attachment 6.

2.2. Peak Flow Attenuation

The existing project site is completely impervious. The proposed project includes roughly 10% of the site areas as pervious areas. Therefore, it is anticipated that the project will result in reduced peak flows in the proposed condition. Additionally, the project proposes to utilize an underground infiltration basin as part of compliance with water quality requirements for the site. The infiltration basin will provide incidental storage and peak flow attenuation with its inclusion on the site. No detention storage is anticipated to be required for the site.

3.0 Hydraulics

3.1 Inlets

The proposed onsite inlets will be designed using the Hydraflow Storm Sewers Extension for AutoCAD Civil3D (v.12), which utilizes FHWA HEC-22 inlet calculation methodology, to intercept the Nolte flow determined from the SacCalc model described in Section 2. Grate inlets will be designed assuming 50% clogging to account for the grate and debris build up.

3.2 Storm Drain System

The proposed storm drain system will be designed using the Hydraflow Storm Sewers Extension for AutoCAD Civil3D (v.12). The on-site storm drain system will be designed to maintain a minimum of 0.5 foot freeboard to the inlets using the Nolte flow determined from the SacCalc model described in Section 2. The proposed storm drain system will discharge to the existing inlet in Watt Avenue at the southwest corner of the site. As discussed in Section 2.2, the proposed project will result in reduced peak flows from the site so no negative impacts to the existing storm drain system in Watt Avenue or the existing system along the southern property line are anticipated.

3.3 Overland Release

The project has been designed to provide a continuous slope from the northeast corner, to the south and then west, to the southwest corner of the site where it will flow into Watt Avenue. The overland release path is shown on the workmap in Attachment 3. A calculation for the conveyance of the 100-year storm event through the drive aisle is included in Attachment 3 which assumes no flow is intercepted in the proposed storm drain system.

Preliminary DS/WQ Storquest Watt September 13, 2023 Page 3 of 4

4.0 Water Quality

The proposed project is a commercial/industrial development which creates more than 1 acre of impervious area. Pursuant to Table 3-2 of the Stormwater Quality Design Manual for the Sacramento Region (SQDM), dated July 2018; the proposed project is required to provide source control, hydromodification control, low impact development control (LID), treatment control measures, and full trash capture control.

4.1 Source Control

Per Table 3-3 of the SQDM, the project is anticipated to provide the following source control measures:

- Efficient Irrigation
- Landscaping
- Storm Drain Markings and Signs
- Waste Management Areas

4.2 Hydromodification Control

Per Figure 5-2 of the SQDM, the project is located in an area where hydromodification control is exempt. An annotated copy of Figure 5-2 is included in Attachment 5.

4.3 LID, Treatment, and Full Trash Capture Control

The proposed LID measure for the project site is an underground infiltration basin which has been sized to provide a minimum of 100 LID points for the project's DMA. The project site consists of soils with very low infiltration rates. The proposed project LID and treatment strategy was discussed with Archie Wright at Sacramento County on July 20, 2023. Pursuant to the conversation, the proposed underground infiltration basin is acceptable to the county because the project is also proposing to utilize Treatment Control measures in addition to the infiltration basin.

Treatment control and full trash capture for the project is proposed to be provided by utilizing a Contech Stormfilter proprietary device prior to discharging to the municipal drainage system. Calculations for the required LID credits and treatment flow rates are included in Attachment 5. The post-construction stormwater quality plan for the project is included in Attachment 4. A copy of the LID credits worksheet is included with the electronic files in Attachment 7.

4.4 Operation and Maintenance

Operation, maintenance, and long-term performance of the proposed LID, Treatment, and Full Trash Capture Control will be the property owner's responsibility. The property owner will enter into a maintenance agreement, covenant, or permit with Sacramento County as required.

Preliminary DS/WQ Storquest Watt September 13, 2023 Page 4 of 4

5.0 Attachments

Attachment 1: FEMA FIRMette

Attachment 2: Hydrologic Calculations Attachment 3: Hydraulic Calculations

Attachment 4: Stormwater Quality Conceptual Plan

Attachment 5: Water Quality Calculations Attachment 6: Proposed Grading Plan

Attachment 7: Electronic Files

Sincerely,

RICK ENGINEERING COMPANY

PROFESSION AT LISP OF CALIFORNIA

M. Scott Lillibridge R.C.E. #52504, Exp. 12/24 Region Manager

FEMA FIRMette

Hydrologic Calculations

Hydraulic Calculations

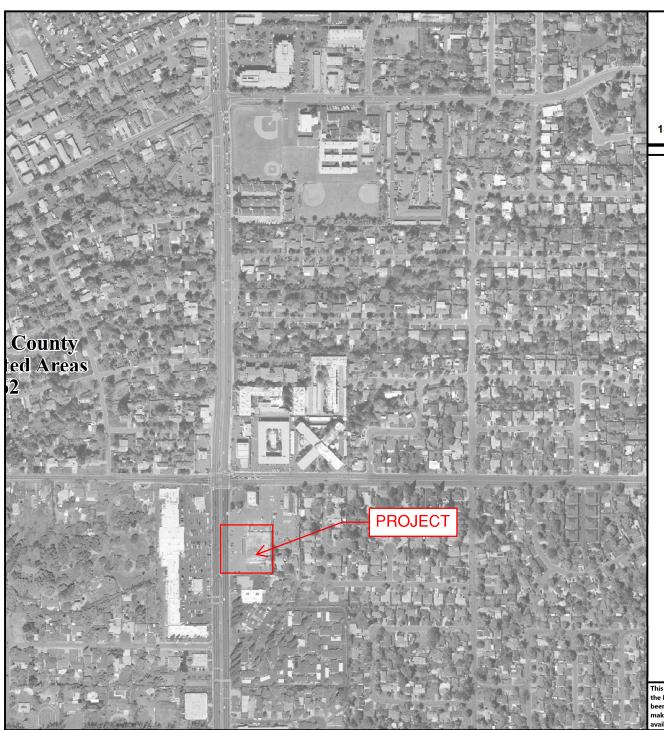
Stormwater Quality Conceptual Plan

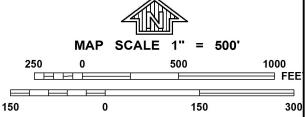
Water Quality Calculations

Proposed Grading Plan

Electronic Files

FEMA FIRMette





FIRM CONTAINS: COMMUNITY

PANEL 0069H

FLOOD INSURANCE RATE MAP

SACRAMENTO COUNTY, CALIFORNIA AND INCORPORATED AREAS

PANEL 69 OF 705

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

SACRAMENTO, CITY OF SACRAMENTO COUNTY

Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.



MAP NUMBER 06067C0069H

EFFECTIVE DATE AUGUST 16, 2012

Federal Emergency Management Agency

This is an official FIRMette showing a portion of the above-referenced flood map created from the MSC FIRMette Web tool. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For additional information about how to make sure the map is current, please see the Flood Hazard Mapping Updates Overview Fact Sheet available on the FEMA Flood Map Service Center home page at https://msc.fema.gov.

Hydrologic Calculations

Nolte method results Page 1 of 1

Nolte method results (Project: 19682 SQX Watt) (Hydrologic zone 3)

ID	Drainage area (acres)	Impervious area (%)	Design Q (cfs)
DMA1	1.33	90.00	0.69

19682 SQX Watt Page 1 of 1

Sacramento Hydrologic Calculator Report
April 11, 2023 | 15:32
Method: Nolte Project Title: 19682 SQX Watt Comments: Nolte method 4/11/2023 Prepared by:

Watershed Hydrologic Summary Data

	Area						Area Per	cent					
Watershed	(acres)	Given as	90	85	80	75	70	60	50	40	30	25	20
DMA1	1.33	fraction	100										

Refer to the Drainage manual for Land Use Impervious Area Percent

View HEC-1 output

Sacramento method results (Project: 19682 Proposed) (100-year, 1-day rainfall)

ID	Peak flow (cfs)	Time of peak (hours)	Basin area (sq. mi)	Peak stage (feet)	Peak storage (ac-ft)	Diversion volume (ac-ft)	
DMA1	4.7	12:02	.00				

19682 Proposed Page 1 of 4

Sacramento Hydrologic Calculator Report
April 11, 2023 15:34
Method: Sacra Project Title: 19682 Proposed Comments: Sacramento County HEC-1 method 4/11/2023

Prepared by:

Watershed Hydrologic Summary Data

		Mean	Lag Times		Basin "n"		Loss	Rates	Percent Impervious		
Watershed	Area (acres)	Elevation (ft)	Method	Lag Time (min)	Method	Basin "n"	Method	Loss Rate (in/hr)	Method	Impervious Area (%)	
DMA1	1.33	75	Basin "n"	-	Computed	-	Computed	-	Computed	-	

19682 Proposed Page 2 of 4

Basin "n" Method Data for Lag Time Computation

	Channel Length	Centroid Length	Slope								Land		npervi % or a	ous A	rea Pe	rcent						
Watershed		(ft)		Channelization	95	90	85	80	75	70	60	50	40	30	25	20	15	10	5	2	1	1*
DMA1	200	100	.005	Undeveloped		0																
DWAI	200	100	.003	Developed		100																

Refer to the Drainage manual for Land Use Impervious Area Percent

^{*}Dense Oaks, Shrubs, Vines

19682 Proposed Page 3 of 4

Infiltration Loss Rate Data

	Soil Cover							I	Land Us		rious Are acres)	ea Perce	nt						
Watershed		95	90	85	80	75	70	60	50	40	30	25	20	15	10	5	2	1	1*
	В																		
DMA1	C		100																
	D																		

Refer to the help file for Land Use Impervious Area Percent

^{*}Dense Oaks, Shrubs, Vines

19682 Proposed Page 4 of 4

Sacramento County, California

220—San Joaquin-Urban land complex, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: hhq2

Elevation: 20 to 500 feet

Mean annual precipitation: 10 to 22 inches Mean annual air temperature: 61 to 63 degrees F

Frost-free period: 250 to 300 days

Farmland classification: Not prime farmland

Map Unit Composition

San joaquin and similar soils: 65 percent

Urban land: 25 percent

Minor components: 10 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of San Joaquin

Setting

Landform: Terraces

Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Alluvium derived from granite

Typical profile

H1 - 0 to 13 inches: fine sandy loam
H2 - 13 to 30 inches: sandy clay loam

H3 - 30 to 35 inches: clay loam H4 - 35 to 60 inches: indurated

H5 - 60 to 67 inches: stratified sandy loam to loam

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches; 35 to 60 inches to

duripan

Drainage class: Moderately well drained

Runoff class: High

Capacity of the most limiting layer to transmit water (Ksat): Very low

(0.00 to 0.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 4.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3s

Hydrologic Soil Group: C

Ecological site: R017XY902CA - Duripan Vernal Pools

Hydric soil rating: No

Description of Urban Land

Typical profile

H1 - 0 to 6 inches: variable

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 8

Hydric soil rating: No

Minor Components

Dierssen

Percent of map unit: 4 percent

Hydric soil rating: No

Bruella

Percent of map unit: 3 percent

Hydric soil rating: No

Xerarents

Percent of map unit: 1 percent

Hydric soil rating: No

Durixeralfs

Percent of map unit: 1 percent

Hydric soil rating: No

Unnamed, clayey subsoil

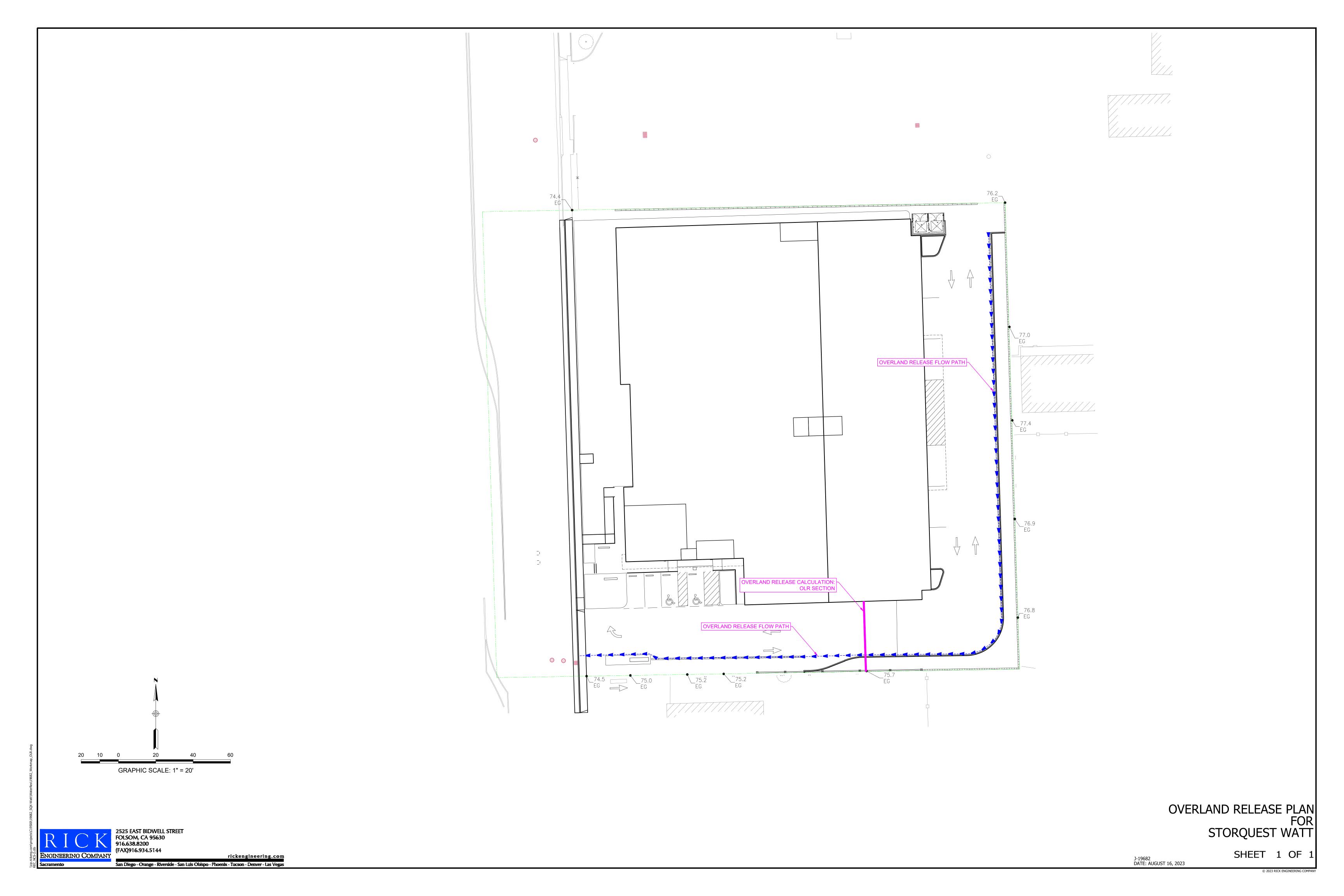
Percent of map unit: 1 percent

Hydric soil rating: No

Data Source Information

Soil Survey Area: Sacramento County, California Survey Area Data: Version 22, Sep 1, 2022

Hydraulic Calculations



Hydraulic Analysis Report

Project Data

Project Title:

Designer:

Project Date: Wednesday, August 16, 2023

Project Units: U.S. Customary Units

Notes:

Channel Analysis: OLR Section

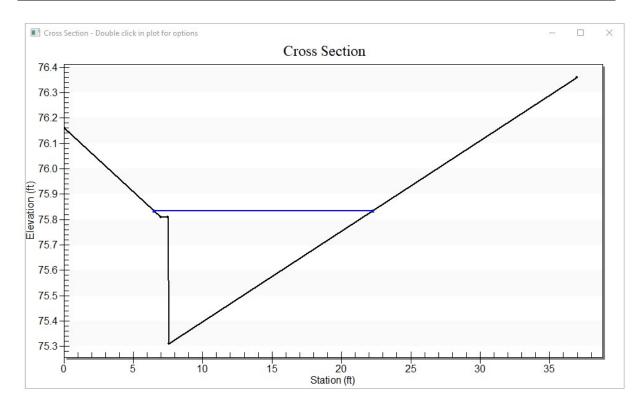
Notes:

Input Parameters

Channel Type: Custom Cross Section

Cross Section Data

Elevation (ft)	Elevation (ft)	Manning's n
0.00	76.16	0.0150
7.00	75.81	0.0150
7.50	75.81	0.0150
7.58	75.31	0.0150
36.96	76.36	



Longitudinal Slope: 0.0010 ft/ft

Flow: 4.7000 cfs

Result Parameters

Depth: 0.5241 ft

Area of Flow: 3.8824 ft^2 Wetted Perimeter: 16.1623 ft Hydraulic Radius: 0.2402 ft Average Velocity: 1.2106 ft/s

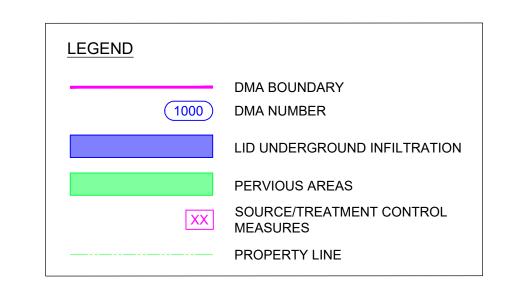
Top Width: 15.7260 ft
Froude Number: 0.4294
Critical Depth: 0.3700 ft
Critical Velocity: 2.4406 ft/s
Critical Slope: 0.0060 ft/ft
Critical Top Width: 10.41 ft

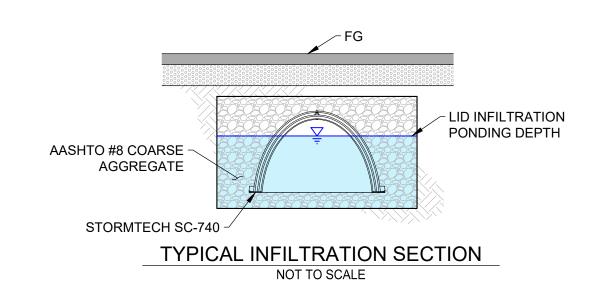
Calculated Max Shear Stress: 0.0327 lb/ft^2 Calculated Avg Shear Stress: 0.0150 lb/ft^2

Composite Manning's n Equation: Lotter method

Manning's n: 0.0150

Stormwater Quality Conceptual Plan

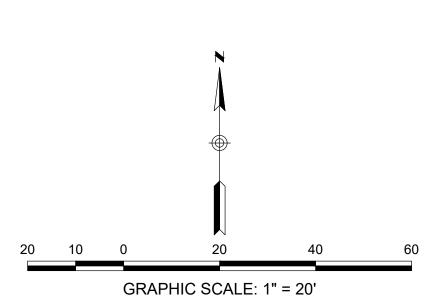




POST CONSTRUCTION MEASURES

	EXISTIN	G (ACRES)	PROPOS	ED (ACRES)	TOTAL		LID MEASURES		TREATMENT MEASURES		
DMA	PERVIOUS	IMPERVIOUS	PERVIOUS	IMPERVIOUS	AREA (ACRES)	VOLUME PROVIDED (ACFT)	TYPE	LID CREDITS PROVIDED	ТҮРЕ	WQF (CFS)	
1	0.00	1.33	0.15	1.18	1.33	0.04	INFILTRATION BASIN	100.0	CONTECH STORMFILTER	0.11	

POST CONSTRUCTION CONTROL MEASURE COMPLIANCE I HEREBY CERTIFY THAT THE POST CONSTRUCTION CONTROL MEASURES WERE CONSTRUCTED AS SHOWN ON THE PLANS APPROVED BY THE COUNTY OF SACRAMENTO. PROJECT MANAGER:



SOURCE CONTROL MEASURE: WASTE MANAGEMENT AREAS (TYP) (DMA1) SOURCE CONTROL MEASURES: EFFICIENT IRRIGATION (TYP) LANDSCAPING (TYP) SOURCE CONTROL MEASURES: STORM DRAIN MARKINGS AND SIGNS (TYP) FULL TRASH CAPTURE DEVICE TREATMENT CONTROL MEASURE: CONTECH STORMFILTER DISCHARGE POINT TO MUNICIPAL DRAINAGE SYSTEM

POST-CONSTRUCTION STORMWATER QUALITY PLAN FOR STORQUEST WATT

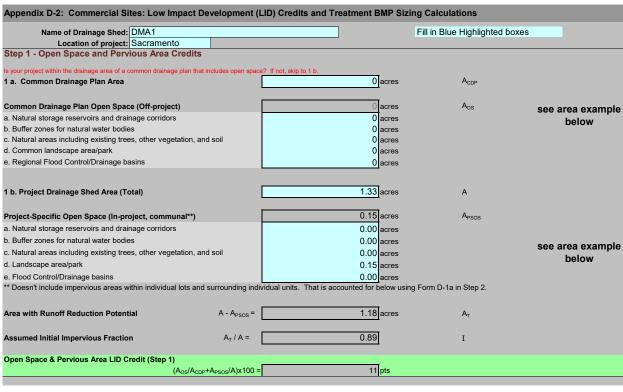
SHEET 1 OF

2525 EAST BIDWELL STREET FOLSOM, CA 95630 916.638.8200 (FAX)916.934.5144

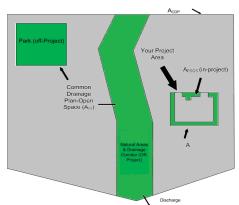
rickengineering.com San Diego - Orange - Riverside - San Luis Obispo - Phoenix - Tucson - Denver - Las Vegas

J-19682 DATE: AUGUST 16, 2023

Water Quality Calculations







Step 2 - Runoff Reduction Credits						
Runoff Reduction Treatments	Impervious Area Managed		Efficiency Factor		Effective Area Managed (A _C)	
Porous Pavement:	- J					
Option 1: Porous Pavement (see Fact Sheet, excludes porous pavement used in Option 2)	0	acres	х	=	0.000	acres
Option 2: Disconnected Pavement used in Option 1)	e Form D-2a for credits				0.00	acres
Landscaping used to Disconnect Pavement (see Fact Sheet)	0.0000	acres		=	0.00	acres
Disconnected Roof Drains (see Fact Sheet and/or Table D-2b for summary of requirement	0	acres		=	0.00	acres
Ecoroof (see Fact Sheet)	0	acres		=	0.00	acres
Interceptor Trees use Form D-2b for cree (see Fact Sheet)	dits				0.00	acres
Total Effective Area Managed by Runoff Reduction Me	asures		Ac		0.00	acres
Runoff Reduction Credit (Step 2)			(A _C)	' A _T)*100 =	0	pts

Table D-2a

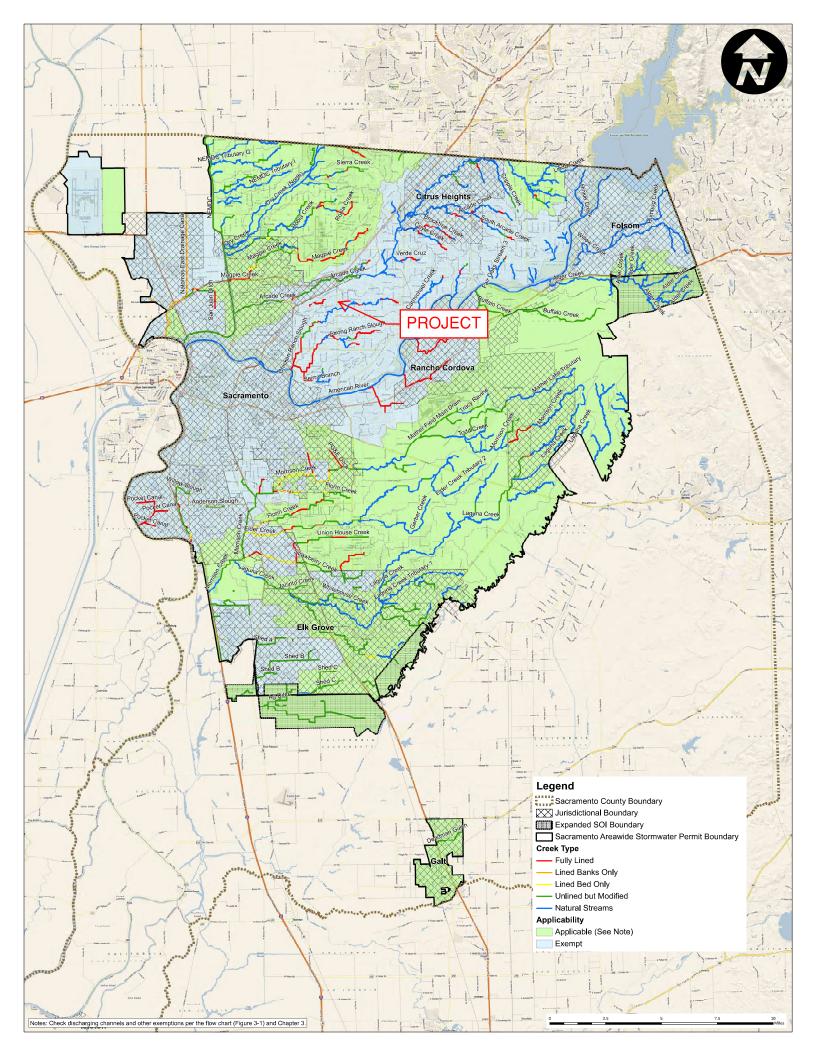
Table D-2b

	Efficiency
Porous Pavement Type	Multiplier
Cobblestone Block Pavement	0.40
Pervious Concrete/Asphalt	0.60
Modular Block Pavement &	0.75
Reinforced Grass Pavement	1.00

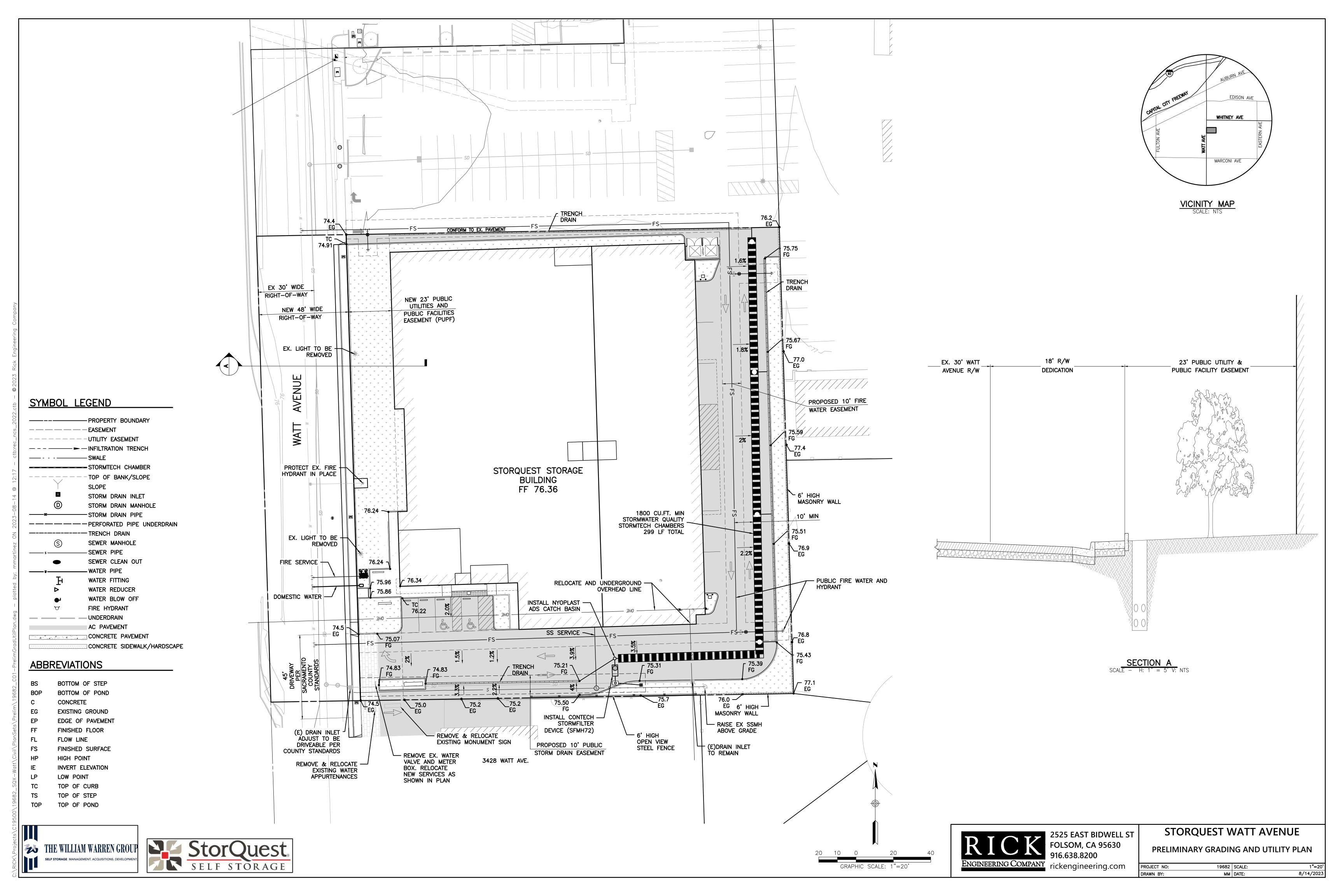
Maximum roof size	Minimum travel distance
≤ 3,500 sq ft	21 ft
≤ 5,000 sq ft	24 ft
≤ 7,500 sq ft	28 ft
≤ 10,000 sq ft	32 ft

Form D-2a: Disconnected Pavement	Worksheet					
See Fact Sheet for more information regarding Disc	connected Pavement cre	edit guidelines				Effective Area Managed (A _C)
Pavement Draining to Porous Pavement						
Enter area draining onto Porous Pavement			0.00	a	acres	Box K1
Enter area of Receiving Porous Pavement			0.00	a	acres	Box K2
(excludes area entered in Step 2 under Porous	Pavement)		0.00			D 160
4. Ratio of Areas (Box K1 / Box K2)			0.00			Box K3
5. Select multiplier using ratio from Box K3 and	l enter into Box K4					
Ratio (Box D) Ratio is ≤ 0.5		Multiplier 1.00				
Ratio is > 0.5 and < 1.0		0.83				Box K4
Ratio is > 1.0 and < 1.5		0.71	1			
Ratio is > 1.5 and < 2.0		0.55				
6. Enter Efficiency of Porous Pavement (see t	table below)					Box K5
	Efficiency					
Porous Pavement Type	Multiplier					
Cobblestone Block Pavement	0.40					
Pervious Concrete Asphalt Pavement	0.60					
Modular Block Pavement	0.75					
Porous Gravel Pavement						
Reinforced Grass Pavement	1.00					
7. Multiply Box K2 by Box K5 and enter into Bo	ox K6		0.00	a	acres	Box K6
8. Multiply Boxes K1,K4, and K5 and enter the	result in Box K7		0.00	a	acres	Box K7
9. Add Box K6 to Box K7 and multiply by 60%,						0.00 acres
This is the amount of area credit to enter into the	he "Disconnected Pav	ement" Box of Form D-2				
Form D-2b: Interceptor Tree Worksho	eet					
See Fact Sheet for more information regarding Inte	rceptor Tree credit guid	elines				
New Evergreen Trees		hana in Day I 4		4	D14	
Enter number of new evergreen trees that q	quality as interceptor i	rees in Box L1.		trees	Box L1	
2. Multiply Box L1 by 200 and enter result in E	Box L2		0	sq. ft.	Box L2	
New Deciduous Trees						
3. Enter number of new deciduous trees that q	qualify as Interceptor T	rees in Box L3.		trees	Box L3	
Multiply Box L3 by 100 and enter result in B	lox L4		0	sq. ft.	Box L4	
Existing Tree Canopy						
Enter square footage of existing tree canopy	v that qualifies as Exis	sting Tree canopy in Box L5.	0	sq. ft.	Box L5	
o. Line, equal o locago el existing a co canop	y arat quamico do Exit	ming 1100 canopy in 20x 20x		oq. 1t.	DOX 20	
C. Multiply Pay I E by 0 E and enter the regult is	n Bay I G			og ft	Pay I 6	
Multiply Box L5 by 0.5 and enter the result in	II BOX LO		0	sq. ft.	Box L6	
Total Interceptor Tree EAM Credits						
Add Boxes L2, L4, and L6 and enter it into Box	: L7		0	sq. ft.	Box L7	
Divide Box L7 by 43,560 and multiply by 20% t	-	-	0.00	acres	Box L8	

Impervious Area Managed by Rain barrels, Cisterr				
(see Fact Sheet)		ns, for simple rain barrels	0.00 acres	
Automated-Control Capture and Use System				
(see Fact Sheet, then enter impervious area managed by the	system)		0.00 acres	
Bioretention/Infiltration Credits				
Impervious Area Managed by Bioretention BMPs	Bioretention Area	sq ft		
(see Fact Sheet)	Subdrain Elevation			
	Ponding Depth, inches	inches	0.00 acres	
Impervious Area Managed by Infiltration BMPs				
(see Fact Sheet)	Drawdown Time, hrs	drawdown_hrs_inf		
	Soil Infiltration Rate, in/hr	soil_inf_rate		
Sizing Option 1:	Capture Volume, acre-ft	0.04 capture_vol_inf	0.52 acres	
Sizing Option 2:	Infiltration BMP surface area, sq ft	0 soil_surface_area	0.00 acres	
Basin or tre	ench?	approximate BMP depth 0.00 ft		
Impervious Area Managed by Amended Soil or Mu	ılch Beds			
(see Fact Sheet)	Mulched Infiltration Area, sq ft	mulch_area	0.00 acres	
Total Effective Area Managed by Capture-and-Use/B	ioretention/Infiltration BMPs		0.52 A _{LIDc}	
Runoff Management Credit (Step 3)		A _{LII}	$_{10}/A_{T}^{*}200 = 88.8$ pts	
Total LID Credits (Step 1+2+3)	LID compliant ch	eck for treatment sizing in	Step 4 100.0	
Does project require hydromodification managemen			Ctop 4 100.0	
		_		
Adjusted Area for Flow-Based, Non-LID Treatment		$A_T - A_C - A_{LIDC} = $	0.66 A _{AT}	
Adjusted Impervious Fraction of A for Volume-Based	d, Non-LID Treatment	A _{AT} / A =	0.49 I _A	
		_		
Frontle an Ana atmospher 12 and 12 and 12 and 12				
Further treatment is required, see	choose flow-based	or volume-based sizing	in Step 4	
	choose flow-based	or volume-based sizing	in Step 4	
a Treatment - Flow-Based (Rational Method)			in Step 4	
a Treatment - Flow-Based (Rational Method)	choose flow-based Flow = Runoff Coefficient x Ra		in Step 4	
a Treatment - Flow-Based (Rational Method)				ity
a Treatment - Flow-Based (Rational Method) e treatment flow (cfs): value for i in Table D-2c (Rainfall Intensity)	Flow = Runoff Coefficient x Ra		Table D-2c Rainfall Intensi Roseville i = 0.20	in/hr
a Treatment - Flow-Based (Rational Method) e treatment flow (cfs):	Flow = Runoff Coefficient x Ra		Table D-2c Rainfall Intensi Roseville i = 0.20 Sacramento i = 0.18	in/hr in/hr
a Treatment - Flow-Based (Rational Method) e treatment flow (cfs): value for i in Table D-2c (Rainfall Intensity)	Flow = Runoff Coefficient x Ra 0.18 i 0.66 A _{AT}		Table D-2c Rainfall Intensi Roseville i = 0.20 Sacramento i = 0.18	in/hr
a Treatment - Flow-Based (Rational Method) e treatment flow (cfs): value for i in Table D-2c (Rainfall Intensity)	Flow = Runoff Coefficient x Ra		Table D-2c Rainfall Intensi Roseville i = 0.20 Sacramento i = 0.18	in/hr in/hr
a Treatment - Flow-Based (Rational Method) e treatment flow (cfs): value for i in Table D-2c (Rainfall Intensity)	Flow = Runoff Coefficient x Ra 0.18 i 0.66 A _{AT}		Table D-2c Rainfall Intensi Roseville i = 0.20 Sacramento i = 0.18	in/hr in/hr
a Treatment - Flow-Based (Rational Method) e treatment flow (cfs): value for i in Table D-2c (Rainfall Intensity) [0.95	Flow = Runoff Coefficient x Ra 0.18 i 0.66 A _{AT} 0.95 C		Table D-2c Rainfall Intensi Roseville i = 0.20 Sacramento i = 0.18	in/hr in/hr
a Treatment - Flow-Based (Rational Method) e treatment flow (cfs): value for i in Table D-2c (Rainfall Intensity) [0.95	Flow = Runoff Coefficient x Ra 0.18 i 0.66 A _{AT} 0.95 C		Table D-2c Rainfall Intensi Roseville i = 0.20 Sacramento i = 0.18	in/hr in/hr
a Treatment - Flow-Based (Rational Method) e treatment flow (cfs): value for i in Table D-2c (Rainfall Intensity) [0.95	Flow = Runoff Coefficient x Ra 0.18 i 0.66 A _{AT} 0.95 C		Table D-2c Rainfall Intensi Roseville i = 0.20 Sacramento i = 0.18	in/hr in/hr
a Treatment - Flow-Based (Rational Method) te treatment flow (cfs): value for i in Table D-2c (Rainfall Intensity) [0.95 Flow = 0.95 * i * A _{AT} b Treatment - Volume-Based (ASCE-WEF)	Flow = Runoff Coefficient x Ra 0.18 i 0.66 A _{AT} 0.95 C	infall Intensity x Area	Table D-2c Rainfall Intensi Roseville i = 0.20 Sacramento i = 0.18	in/hr in/hr
a Treatment - Flow-Based (Rational Method) te treatment flow (cfs): value for i in Table D-2c (Rainfall Intensity) [0.95 Flow = 0.95 * i * A _{AT} b Treatment - Volume-Based (ASCE-WEF)	Flow = Runoff Coefficient x Ra 0.18]i 0.66] A _{AT} 0.95] C 0.11] cfs	infall Intensity x Area	Table D-2c Rainfall Intensi Roseville i = 0.20 Sacramento i = 0.18 Folsom i = 0.20	in/hr in/hr in/hr
a Treatment - Flow-Based (Rational Method) e treatment flow (cfs): value for i in Table D-2c (Rainfall Intensity) var from Step 3 0.95 Flow = 0.95 * i * A _{AT}	Flow = Runoff Coefficient x Ra 0.18 i 0.66 A _{AT} 0.95 C 0.11 cfs WQV = Area x Maximized Dete	ention Volume (P ₀)	Table D-2c Rainfall Intensi Roseville i = 0.20 Sacramento i = 0.18 Folsom i = 0.20	in/hr in/hr in/hr
a Treatment - Flow-Based (Rational Method) e treatment flow (cfs): value for i in Table D-2c (Rainfall Intensity) [AT from Step 3] 0.95 Flow = 0.95 * i * AAT Description of the Treatment - Volume-Based (ASCE-WEF) e water quality volume (Acre-Feet): [A from Step 1] [A Maximized Detention Volume from figures E-1 to E-4]	Flow = Runoff Coefficient x Ra 0.18 i 0.66 A _{AT} 0.995 C 0.11 cfs	infall Intensity x Area	Table D-2c Rainfall Intensi Roseville i = 0.20 Sacramento i = 0.18 Folsom i = 0.20	in/hr in/hr in/hr
a Treatment - Flow-Based (Rational Method) e treatment flow (cfs): value for i in Table D-2c (Rainfall Intensity) var from Step 3 0.95 Flow = 0.95 * i * A _{AT} b Treatment - Volume-Based (ASCE-WEF) e water quality volume (Acre-Feet): various from Step 1 various description of the foundation of	Flow = Runoff Coefficient x Ra 0.18 i 0.66 A _{AT} 0.95 C 0.11 cfs WQV = Area x Maximized Dete	ention Volume (P ₀)	Table D-2c Rainfall Intensi Roseville i = 0.20 Sacramento i = 0.18 Folsom i = 0.20	in/hr in/hr in/hr
a Treatment - Flow-Based (Rational Method) e treatment flow (cfs): value for i in Table D-2c (Rainfall Intensity) AT from Step 3 0.95 Flow = 0.95 * i * AAT D Treatment - Volume-Based (ASCE-WEF) e water quality volume (Acre-Feet): of from Step 1 of Maximized Detention Volume from figures E-1 to E-4 dix E of this manual using I _k from Step 2.	Flow = Runoff Coefficient x Ra 0.18 i 0.66 A _{AT} 0.95 C 0.11 cfs WQV = Area x Maximized Dete	ention Volume (P ₀)	Table D-2c Rainfall Intensi Roseville i = 0.20 Sacramento i = 0.18 Folsom i = 0.20	in/hr in/hr in/hr



Proposed Grading Plan



Electronic Files