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HIGHLAND GROVE VISTAS 4 TTM NO.38927

TRAFFIC ANALYSIS

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15933-03 TA Report	County of Riverside	August 15, 2024

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LIST OF ABBREVIATED TERMS

(1)	Reference
ADT	Average Daily Traffic
CA MUTCD	California Manual on Uniform Traffic Control Devices
Caltrans	California Department of Transportation
CEQA	California Environmental Quality Act
CMP	Congestion Management Program
DIF	Development Impact Fee
EAP	Existing Plus Ambient Growth Plus Project
EAPC	Existing Plus Ambient Growth Plus Project Plus Cumulative
HCM	Highway Capacity Manual
ITE	Institute of Transportation Engineers
LOS	Level of Service
OPR	Office of Planning and Research
PHF	Peak Hour Factor
Project	Highland Grove Vistas 4
RCTC	Riverside County Transportation Commission
RTA	Riverside Transit Authority
TA	Traffic Analysis
TTM	Tentative Tract Map
TUMF	Transportation Uniform Mitigation Fee
WRCOG	Western Riverside Council of Governments
v/c	Volume to Capacity
VMT	Vehicle Miles Traveled

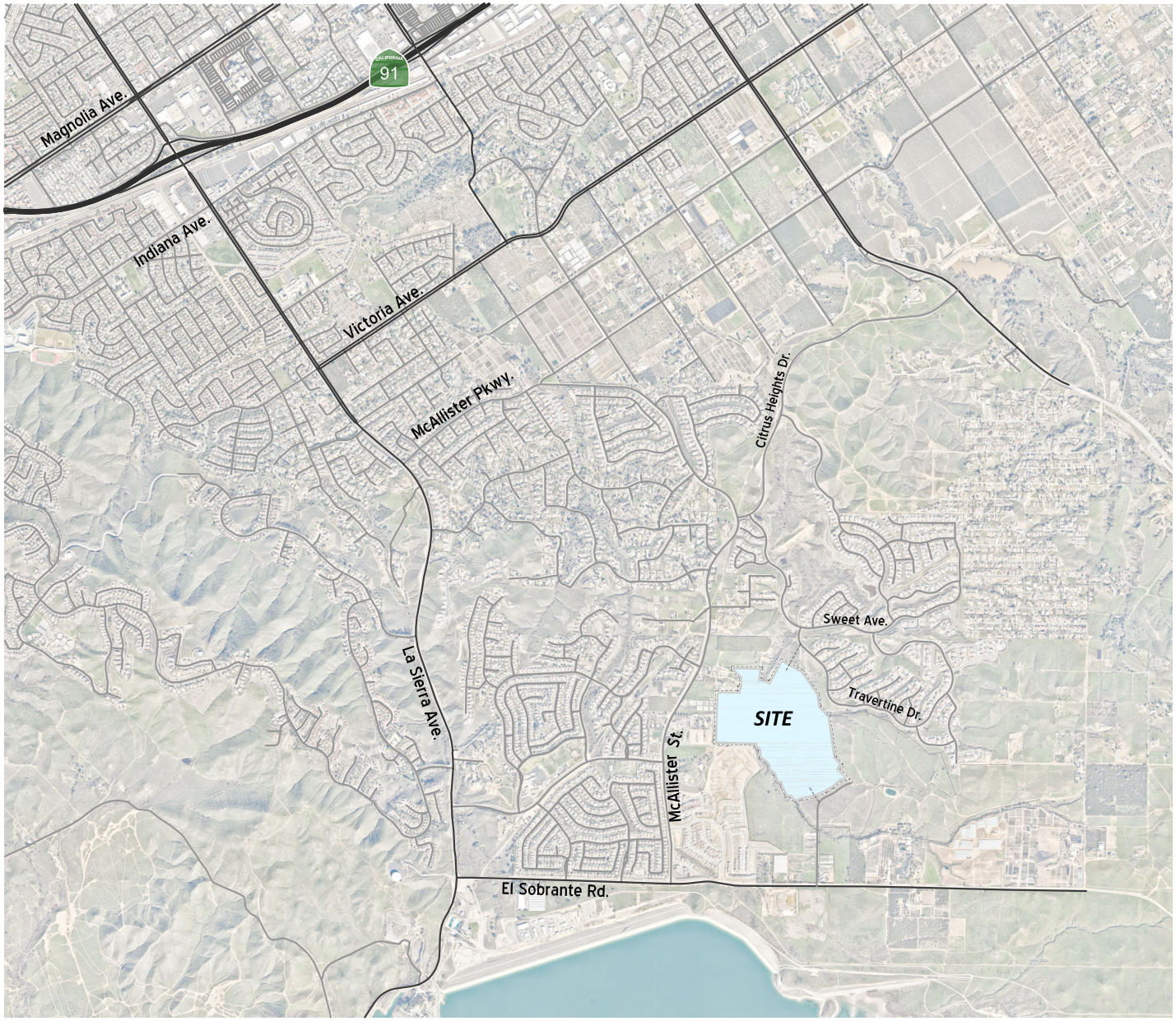
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1 SUMMARY OF FINDINGS

This report presents the results of the Traffic Analysis (TA) for the Highland Grove Vistas 4 Tentative Tract Map (**TTM**) No. 38927 (**Project**), which is located immediately north of El Sobrante Road, east of McAllister Street, and south of Travertine Drive in the County of Riverside, as shown on Exhibit 1-1.

The purpose of this TA is to evaluate the potential circulation system deficiencies that may result from the development of the proposed Project and, where necessary, recommend improvements to achieve acceptable operations consistent with General Plan level of service goals and policies. This TA has been prepared in accordance with the County of Riverside's Transportation Analysis Guidelines for Level of Service Vehicle Miles Traveled (December 2020) (**County Guidelines**) and through consultation with County of Riverside staff during the scoping process. (1) The approved Project Traffic Study Scoping Agreement is provided in Appendix 1.1 of this TA.

EXHIBIT 1-1 : LOCATION MAP



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1.1 PROJECT OVERVIEW

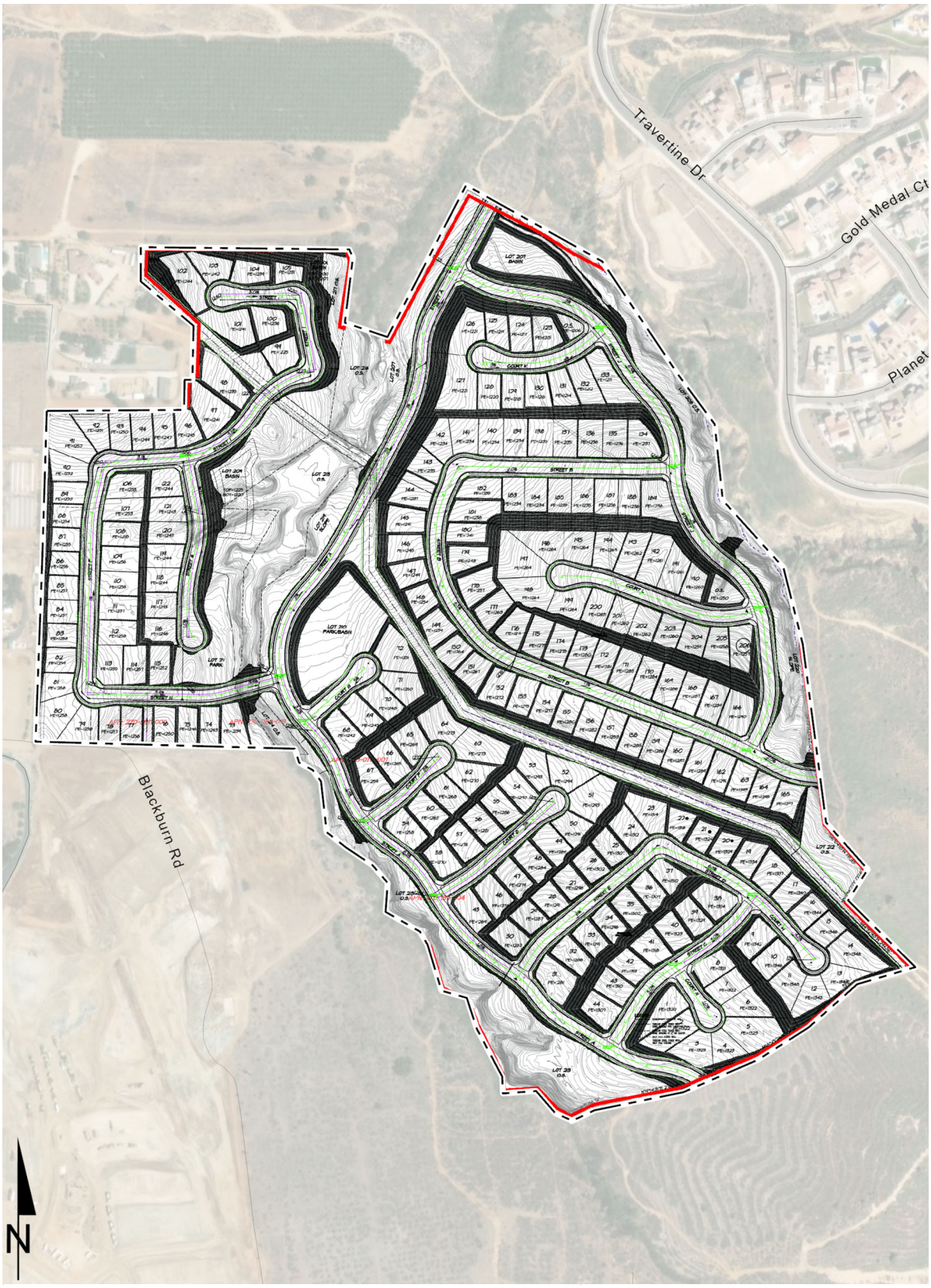
1.1.1 LAND USE

The Project consists of the development of 206 single-family detached residential dwelling units. A preliminary site plan for the Project is shown in Exhibit 1-2. Primary access to the Project site will be accommodated via a new connection to El Sobrante Road (via Street A) and a new connection onto Travertine Drive. Street A on El Sobrante Road and on Travertine Drive are assumed to allow for full access (no left turn restrictions). The Project is anticipated to have an Opening Year of 2027.

1.1.2 TRIP GENERATION

In order to develop the traffic characteristics of the Project, trip-generation statistics published in the Institute of Transportation Engineers (ITE) Trip Generation Manual (11th Edition, 2021) were used to estimate the Project's trip generation based on the Single Family Detached Housing (ITE Land Use Code 210) land use category. The Project is anticipated to generate 1,944 two-way trip-ends per day with 144 AM peak hour trips and 194 PM peak hour trips. The assumptions and methods used to estimate the Project's trip generation characteristics are discussed in greater detail in Section 4.1 *Project Trip Generation* of this report.

EXHIBIT 1-2 : PRELIMINARY SITE PLAN



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1.2 ANALYSIS SCENARIOS

For the purposes of this TA, potential deficiencies to intersection level of service (LOS) have been assessed for each of the following conditions:

- Existing (2024) Conditions
- Existing plus Ambient Growth plus Project (EAP) (2027) Conditions – Project only
- EAP (2027) Conditions – Project plus Highland Grove II plus Greentree
- Existing plus Ambient Growth plus Project plus Cumulative (EAPC) (2027) Conditions

1.2.1 EXISTING (2024) CONDITIONS

Information for Existing (2024) conditions is disclosed to represent the baseline traffic conditions as they existed at the time this report was prepared. Local schools were in session and operating on normal bell schedules at the time traffic counts were collected. Construction activity was occurring at the intersection of McAllister Street at El Sobrante Road (#7); however, the usual number of lanes were not affected. Traffic counts were conducted in April 2024.

1.2.2 EAP (2027) CONDITIONS

The EAP (2027) (Project only) conditions analysis determines the potential circulation system deficiencies based on a comparison of the EAP (Project only) traffic conditions to Existing conditions. The roadway network is similar to Existing conditions except for new connections to be constructed by the Project. To account for background traffic growth, an ambient growth factor from Existing (2024) conditions of 6.12% (2 percent per year, compounded over 3 years) is included for EAP (2027) traffic conditions. The assumed ambient growth factor is based on the requirements per the County Guidelines. Consistent with County Guidelines, the EAP analysis is intended to identify "Opening Year" deficiencies associated with the development of the Project based on the expected background growth within the study area.

The EAP (2027) (Project plus Highland Grove II plus Greentree) conditions analysis determines the potential near-term circulation system deficiencies based on a comparison of the EAP (Project plus Highland Grove II plus Greentree) traffic conditions to Existing conditions. The EAP (Project plus Highland Grove II plus Greentree) traffic conditions are similar to EAP (Project only) traffic conditions except for the inclusion of traffic from the Highland Grove II and Greentree developments. The analysis is intended to identify "Opening Year" deficiencies associated with the development of the Project and the adjacent Highland Grove II and Greentree developments, based on the expected background growth within the study area.

1.2.3 EAPC (2027) CONDITIONS

The EAPC (2027) traffic conditions analysis determines the potential near-term cumulative circulation system deficiencies. The roadway network is similar to Existing conditions except for new connections to be constructed by the Project. To account for background traffic growth, an ambient growth factor from Existing (2024) conditions of 6.12% (2 percent per year, compounded over 3 years) is included for EAPC (2027) traffic. Conservatively, this TA estimates the area ambient traffic growth and then adds traffic generated by other known or probable related projects. These related projects are at least in part already accounted for in the assumed ambient growth rates; and some of these related projects may not be implemented and operational within the 2027 Opening Year time frame assumed for the Project. The resulting traffic growth utilized in the TA (ambient growth factor plus traffic generated by related projects) would therefore tend to overstate rather than understate background cumulative traffic deficiencies under 2027 conditions.

1.3 STUDY AREA

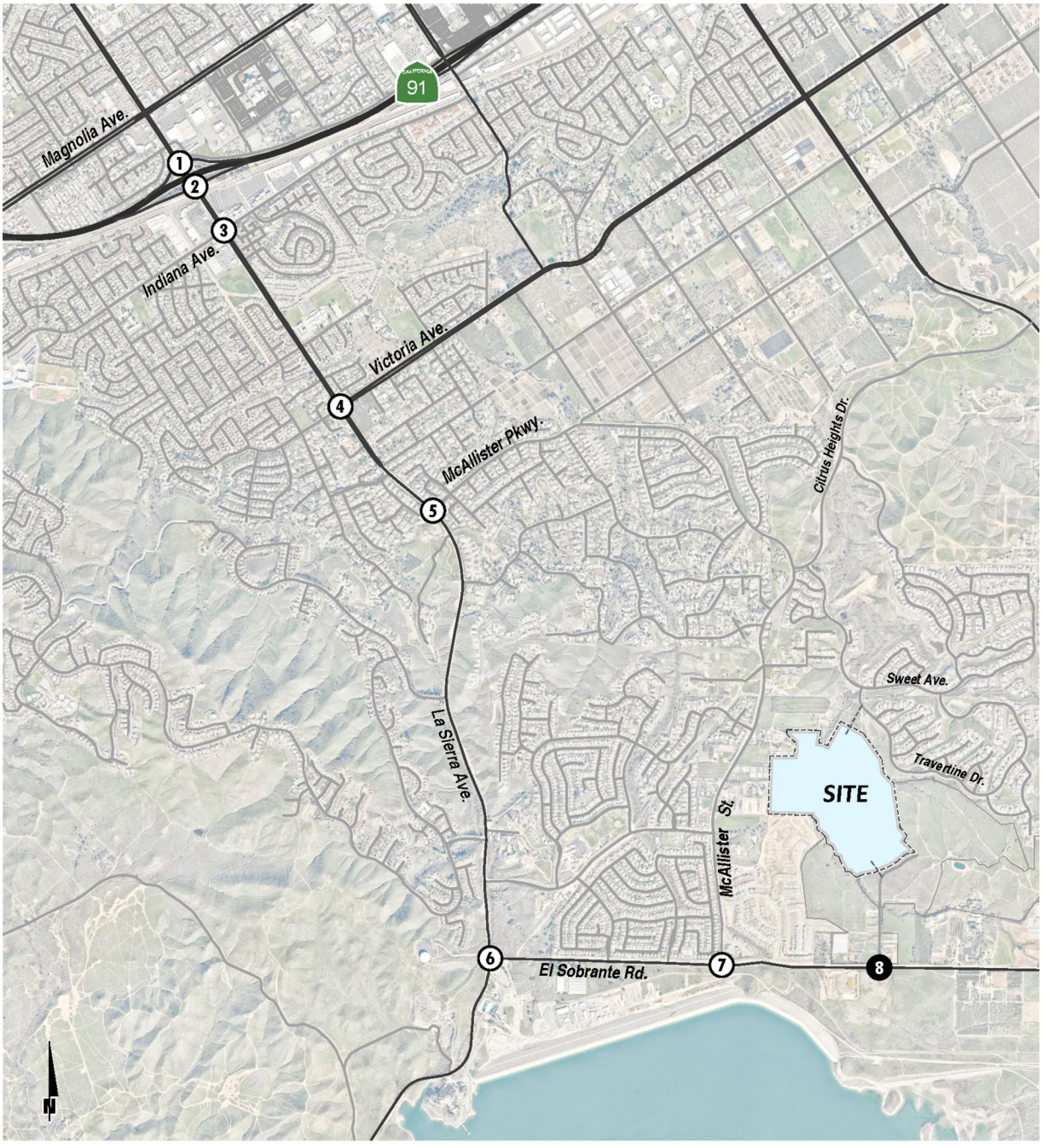
For the purposes of this TA, 8 study area intersections were selected for evaluation based on County Guidelines and consultation with County staff. (1) The 8 study area intersections shown on Exhibit 1-3 and listed in Table 1-1 have been selected for assessment based on the County's analysis criteria of 50 or more peak hour trips. The study area intersections will be evaluated using the Highway Capacity Manual (HCM) 7th Edition methodology.

TABLE 1-1: INTERSECTION ANALYSIS LOCATIONS

#	Intersection	Jurisdiction	CMP?
1	La Sierra Av. & SR-91 WB Ramps	City of Riverside, Caltrans	No
2	La Sierra Av. & SR-91 EB Ramps	City of Riverside, Caltrans	No
3	La Sierra Av. & Indiana Av.	City of Riverside	No
4	La Sierra Av. & Victoria Av.	County of Riverside, City of Riverside	No
5	La Sierra Av. & McAllister St.	County of Riverside	No
6	La Sierra Av. & El Sobrante Rd.	County of Riverside	No
7	McAllister St. & El Sobrante Rd.	County of Riverside	No
8	Street A & El Sobrante Rd.	County of Riverside	No

The intent of a Congestion Management Program (CMP) is to directly link land use, transportation, and air quality, thereby prompting reasonable growth management programs that will effectively utilize new transportation funds, alleviate traffic congestion and related deficiencies, and improve air quality. The County of Riverside CMP became effective with the passage of Proposition 111 in 1990 and most recently updated in 2019 as part of the Riverside County Long Range Transportation Study. The Riverside County Transportation Commission (RCTC) adopted the 2019 CMP for the County of Riverside in December 2019. (2) There are no study area intersections identified as a Riverside County CMP intersection.

EXHIBIT 1-3 : STUDY AREA



LEGEND:

(0) = *Existing Intersection Analysis Location*

(0) = *Future Intersection Analysis Location*

1.4 GENERAL PLAN CONSISTENCY REQUIREMENTS FOR INTERSECTIONS

This section provides a brief overview of the intersection LOS deficiencies identified in this report. The LOS standards used for this assessment are consistent with those identified in the County of Riverside General Plan. Section 2 *Methodologies* provides a detailed explanation on the methodologies used for this assessment, while Section 3 *Area Conditions,* Section 5 *EAP (2027) Traffic Conditions,* and Section 6 *EAPC (2027) Traffic Conditions* include the operations analysis results for each analysis scenario. Additionally, a summary of the peak hour LOS analysis results is presented in Table 1-2.

1.4.1 EXISTING (2024) CONDITIONS

The assessment of Existing (2024) traffic conditions is performed to establish a baseline of current LOS operations from which the effects of the Project's future contribution of traffic can be measured. The study area intersections were found to currently operate at an acceptable LOS (i.e., at or above the County of Riverside's adopted General Plan thresholds) during the peak hours, with the exception of the following intersections:

- La Sierra Avenue & El Sobrante Road (#6) – LOS F PM peak hour only
- McAllister Street & El Sobrante Road (#7) – LOS E PM peak hour only

1.4.2 EAP AND EAPC (2027) CONDITIONS

An assessment of Opening Year and Cumulative traffic conditions is performed to identify whether the traffic contribution associated with both the Project and that of other reasonably foreseeable future development projects in the area have the potential to cause intersection LOS to fall below the City's adopted thresholds. The Project's contribution to these forecasted LOS deficiencies is typically addressed through fee payments to an established fee program (both local and regional).

The results of the LOS assessments for Opening Year and Cumulative traffic conditions are also summarized in Table 1-3, while Section 5 *EAP (2027) Traffic Conditions* and Section 6 *EAPC (2027) Traffic Conditions* of the report provide a more detailed list of the forecast LOS deficiencies.

TABLE 1-2: SUMMARY OF LOS

		Existing (2024)	EAP (2027) Project Only	EAP (2027) Project + Highland Grove II + Greentree	EAPC (2027)
1	**La Sierra Av. & SR-91 WB Ramps**	A-D	A-D	A-D	A-D
2	**La Sierra Av. & SR-91 EB Ramps**	A-D	A-D	A-D	A-D
3	**La Sierra Av. & Indiana Av.**	A-D	A-D	A-D	A-D
4	**La Sierra Av. & Victoria Av.**	A-D	A-D	A-D	A-D
5	**La Sierra Av. & McAllister St.**	A-D	A-D	A-D	A-D
6	**La Sierra Av. & El Sobrante Rd.**	F	F	F	F
7	**McAllister St. & El Sobrante Rd.**	E	F	F	F
8	**Street A & El Sobrante Rd.**	N/A	A-D	A-D	A-D

LEGEND

= AM Peak Hour
= PM Peak Hour
= A-D
= E
= F

1.5 RECOMMENDATIONS

1.5.1 INTERSECTION IMPROVEMENT RECOMMENDATIONS NECESSARY TO MAINTAIN GENERAL PLAN CONSISTENCY

The summary of intersection improvement recommendations to maintain General Plan LOS standards for EAP and EAPC (2027) are listed in Table 1-3 below.

1.5.2 ON-SITE CIRCULATION RECOMMENDATIONS

The following recommendations, based on the improvements needed to accommodate site access and maintain acceptable peak hour LOS at site access and on-site intersections, are described below and shown on Exhibit 1-4. The site adjacent queuing analysis worksheets are provided in Appendix 1.2 for EAPC (2027) traffic conditions. No site adjacent queues are anticipated with the proposed improvements.

Recommendation 1 – Street A & El Sobrante Road (#8) – The following improvements are necessary to accommodate site access:

- Project to install a stop control on the southbound approach (Street A) to implement a cross-street stop-controlled intersection and construct a shared left-right turn lane.
- Project to construct an eastbound left turn lane with a minimum of 100-feet of storage.

Recommendation 2 – Street A is a north-south oriented roadway bisecting the Project site, providing primary Project access to El Sobrante Road. Project to construct Street A at its ultimate full-section width as a Local roadway (60-foot right-of-way), from El Sobrante Road to the Project's northern boundary, consistent with the County's standards.

On-site traffic signing and striping should be implemented agreeable with the provisions of the California Manual on Uniform Traffic Control Devices (CA MUTCD) and in conjunction with detailed construction plans for the Project site.

Sight distance at each Project access point should be reviewed with respect to standard Caltrans and County of Riverside sight distance standards at the time of preparation of final grading, landscape, and street improvement plans.

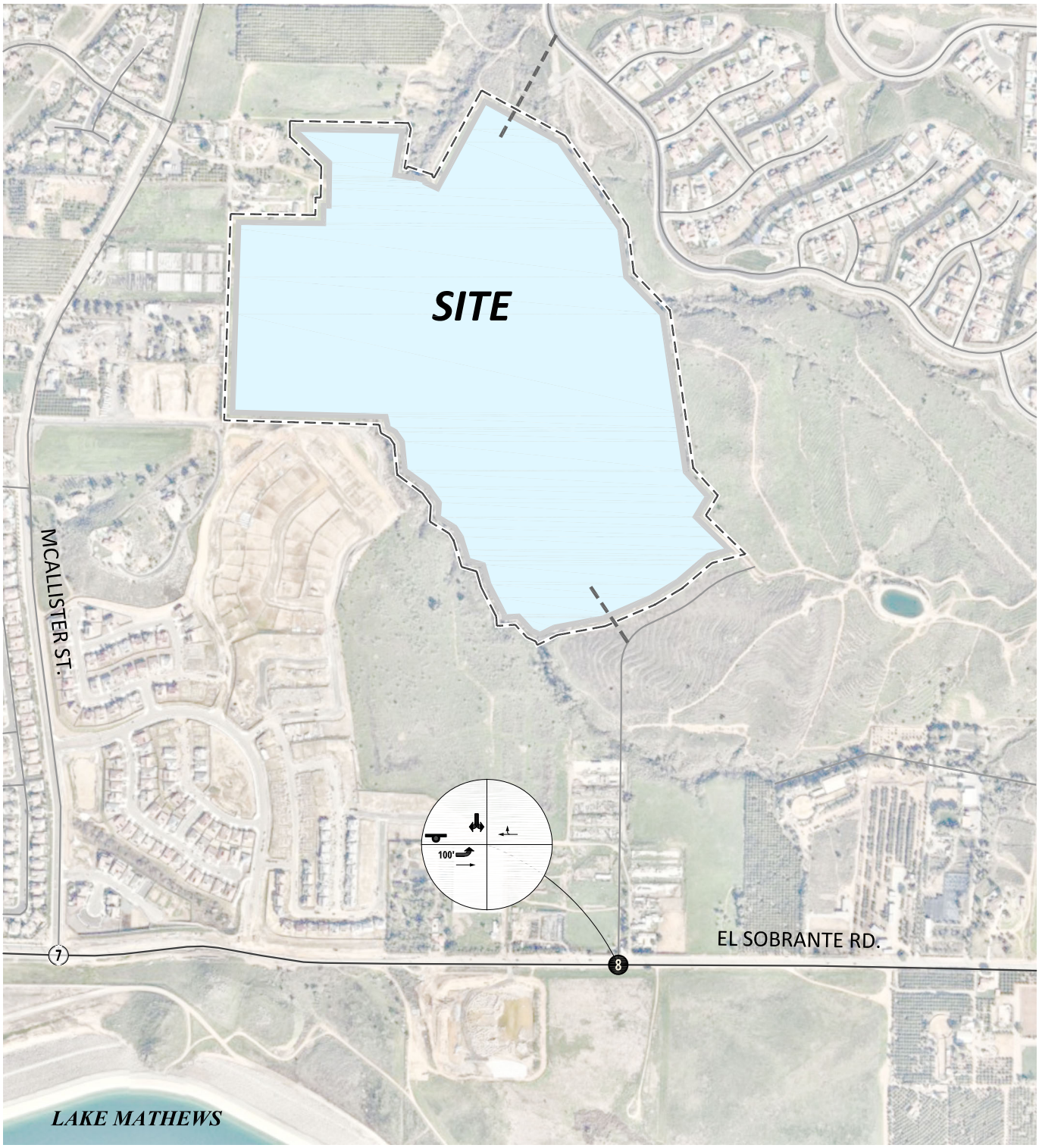
TABLE 1-3: SUMMARY OF IMPROVEMENTS BY ANALYSIS SCENARIO

#	Intersection Location	Jurisdiction	Analysis Scenario				Improvements in DIF, TUMF, etc.[1,2]	Project Responsibility	Project Fair Share
			Existing (2024)	EAP (2027) (Project Only)	EAP (2027) (Project plus Highland Grove II plus Greentree)	EAPC (2027)			
6	La Sierra Av. & El Sobrante Rd.	County of Riverside	Install a Traffic Signal	Same	Same	Same	No	Fair Share	26.6%
						Add WB right turn lane with overlap phasing	No	Fair Share	
7	McAllister St. & El Sobrante Rd.	County of Riverside	Install a Traffic Signal	Same	Same	Same	No	Fair Share	28.0%

[1] Improvements included in regional/City DIF programs have been identified as such.

[2] Program improvements constructed by project may be eligible for fee credit. In lieu fee payment is at the discretion of the City.

EXHIBIT 1-4 : SITE ACCESS RECOMMENDATIONS



LEGEND

- = Future Intersection Analysis Location
- = Existing Traffic Lane
- 100' = Minimum Turn Pocket Length Improvement
- = Stop Sign Improvement
- = Traffic Lane Improvement

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1.6 QUEUING ANALYSIS

The traffic modeling and signal timing optimization software package SimTraffic has been utilized to assess the queues, for both on-site roadways (Project design features) and off-site interchange off-ramp queues. SimTraffic is designed to model networks of signalized and unsignalized intersections, with the primary purpose of checking and fine-tuning signal operations. SimTraffic uses the input parameters from Synchro to generate random simulations. These random simulations generated by SimTraffic have been utilized to determine the 95th percentile queue lengths observed for each applicable turn lane. A SimTraffic simulation has been recorded up to 5 times, during the weekday AM and weekday PM peak hours, and has been seeded for 15-minute periods with 60-minute recording intervals.

1.6.1 OFF-SITE QUEUING

Existing (2024) Conditions

A queuing analysis has also been conducted at all signalized study area intersections along La Sierra Avenue at the request of the County. Based on the results of this analysis, the following movements currently experience queuing issues during the weekday AM or weekday PM peak 95th percentile traffic flows:

- La Sierra Avenue & SR-91 WB Ramps (#1), northbound left turn lane – AM and PM peak hours
- La Sierra Avenue & SR-91 WB Ramps (#1), southbound through lane – PM peak hour only
- La Sierra Avenue & SR-91 EB Ramps (#2), southbound left turn lane – PM peak hour only
- La Sierra Avenue & Indiana Avenue (#3), northbound left turn lane – AM peak hour only
- La Sierra Avenue & Indiana Avenue (#3), eastbound left turn lane – AM and PM peak hours
- La Sierra Avenue & Indiana Avenue (#3), eastbound right turn lane – PM peak hour only
- La Sierra Avenue & Victoria Avenue (#4), southbound left turn lane – PM peak hour only
- La Sierra Avenue & Victoria Avenue (#4), westbound left turn lane – PM peak hour only
- La Sierra Avenue & McAllister Parkway (#5), southbound left turn lane – PM peak hour only

Opening Year Traffic Conditions – EAP and EAPC

The following additional movement is anticipated to experience queuing issues during the weekday AM or weekday PM peak 95th percentile traffic flows, in addition to the movements identified under Existing (2024) Conditions, under EAP (2027) (Project only) traffic conditions:

- La Sierra Avenue & Indiana Avenue (#3), northbound through lane – AM peak hour only
- La Sierra Avenue & Indiana Avenue (#3), westbound right turn lane – AM and PM peak hours

The following additional movement is anticipated to experience queuing issues during the weekday AM or weekday PM peak 95th percentile traffic flows with the addition of Highland Grove II and Greentree traffic under EAP (2027) (Project plus Highland Grove II plus Greentree) traffic conditions:

- La Sierra Avenue & Indiana Avenue (#3), northbound left turn lane – PM peak hour

With the addition of the remaining cumulative traffic, the following movements, which were previously identified to experience queuing issues during the weekday PM 95th percentile traffic flows, are also anticipated to experience queuing issues during the weekday AM 95th percentile traffic flows under EAPC (2027) traffic conditions:

- La Sierra Avenue & SR-91 EB Ramps (#2), southbound left turn lane
- La Sierra Avenue & Victoria Avenue (#4), southbound left turn lane

1.6.2 ON-SITE QUEUING

The results of the site adjacent queuing analysis worksheets for the weekday AM and PM peak hours are provided in Appendix 1.2 of this report for EAPC (2027) traffic conditions and have been used to verify the recommended pocket lengths for each of the future intersections anticipated to be completed as part of the Project, as discussed in Section 1.5.2 *On-Site Circulation Recommendations*. Exhibit 1-4 provides the necessary turn pocket lengths, based on the results of the on-site queuing analysis.

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2 METHODOLOGIES

This section of the report presents the methodologies used to perform the traffic analyses summarized in this report. The methodologies described are consistent with County of Riverside's Traffic Study Guidelines.

2.1 LEVEL OF SERVICE

Traffic operations of roadway facilities are described using the term "Level of Service" (LOS). LOS is a qualitative description of traffic flow based on several factors, such as speed, travel time, delay, and freedom to maneuver. Six levels are typically defined ranging from LOS A, representing completely free-flow conditions, to LOS F, representing a breakdown in flow resulting in stop-and-go conditions. LOS E represents operations at or near capacity, an unstable level where vehicles are operating with the minimum spacing for maintaining uniform flow.

2.2 INTERSECTION CAPACITY ANALYSIS

The definitions of LOS for interrupted traffic flow (flow restrained by the existence of traffic signals and other traffic control devices) differ slightly depending on the type of traffic control. The LOS is typically dependent on the quality of traffic flow at the intersections along a roadway. The 7th Edition Highway Capacity Manual (HCM) methodology expresses the LOS at an intersection in terms of delay time for the various intersection approaches. (3) The HCM uses different procedures depending on the type of intersection control.

2.2.1 SIGNALIZED INTERSECTIONS

The County of Riverside, City of Riverside, and California Department of Transportation (Caltrans) require signalized intersection operations analysis based on the methodology described in the HCM. (3) Intersection LOS operations are based on an intersection's average control delay. Control delay includes initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay. For signalized intersections, LOS is related to the average control delay per vehicle and is correlated to a LOS designation as described in Table 2-1.

The traffic modeling and signal timing optimization software package Synchro (Version 12) has been utilized to analyze signalized intersections. Synchro is a macroscopic traffic software program that is based on the signalized intersection capacity analysis as specified in the HCM. Macroscopic level models represent traffic in terms of aggregate measures for each movement at the study intersections. Equations are used to determine measures of effectiveness such as delay and queue length. The level of service and capacity analysis performed by Synchro takes into consideration optimization and coordination of signalized intersections within a network.

TABLE 2-1: SIGNALIZED INTERSECTION LOS THRESHOLDS

Description	Average Control Delay (Seconds), V/C ≤ 1.0	Level of Service, V/C ≤ 1.0[1]
Operations with very low delay occurring with favorable progression and/or short cycle length.	0 to 10.00	A
Operations with low delay occurring with good progression and/or short cycle lengths.	10.01 to 20.00	B
Operations with average delays resulting from fair progression and/or longer cycle lengths. Individual cycle failures begin to appear.	20.01 to 35.00	C
Operations with longer delays due to a combination of unfavorable progression, long cycle lengths, or high V/C ratios. Many vehicles stop and individual cycle failures are noticeable.	35.01 to 55.00	D
Operations with high delay values indicating poor progression, long cycle lengths, and high V/C ratios. Individual cycle failures are frequent occurrences. This is considered to be the limit of acceptable delay.	55.01 to 80.00	E
Operation with delays unacceptable to most drivers occurring due to over saturation, poor progression, or very long cycle lengths.	80.01 and up	F

Source: HCM, 7th Edition

[1] If V/C is greater than 1.0, then LOS is F per HCM.

A saturation flow rate of 1900 has been utilized for all study area intersections located within the County of Riverside and City of Riverside. The peak hour traffic volumes have been adjusted using a peak hour factor (PHF) to reflect peak 15-minute volumes. Customary practice for LOS analysis is to use a peak 15-minute rate of flow. However, flow rates are typically expressed in vehicles per hour. The PHF is the relationship between the peak 15-minute flow rate and the full hourly volume (e.g., PHF = [Hourly Volume] / [4 x Peak 15-minute Flow Rate]). The use of a 15-minute PHF produces a more detailed analysis as compared to analyzing vehicles per hour. Existing PHFs have been used for all analysis scenarios. Per the HCM, PHF values over 0.95 often are indicative of high traffic volumes with capacity constraints on peak hour flows while lower PHF values are indicative of greater variability of flow during the peak hour. (3)

2.2.2 UNSIGNALIZED INTERSECTIONS

The County of Riverside and City of Riverside require the operations of unsignalized intersections be evaluated using the methodology described in the HCM. (3) The LOS rating is based on the weighted average control delay expressed in seconds per vehicle (see Table 2-2). At two-way or side-street stop-controlled intersections, LOS is calculated for each controlled movement and for the left turn movement from the major street, as well as for the intersection as a whole. For approaches composed of a single lane, the delay is computed as the average of all movements in that lane. Delay for the intersection is reported for the worst individual movement at a two-way stop-controlled intersection. For all-way stop-controlled intersections, LOS is computed for the intersection as a whole (average delay).

TABLE 2-2: UNSIGNALIZED INTERSECTION LOS THRESHOLDS

Description	Average Control Delay (Seconds), V/C ≤ 1.0	Level of Service, V/C ≤ 1.0[1]
Little or no delays.	0 to 10.00	A
Short traffic delays.	10.01 to 15.00	B
Average traffic delays.	15.01 to 25.00	C
Long traffic delays.	25.01 to 35.00	D
Very long traffic delays.	35.01 to 50.00	E
Extreme traffic delays with intersection capacity exceeded.	> 50.00	F

Source: HCM, 7th Edition

[1] If V/C is greater than 1.0, then LOS is F per HCM.

2.3 TRAFFIC SIGNAL WARRANT ANALYSIS METHODOLOGY

The term “signal warrants” refers to the list of established criteria used by Caltrans and other public agencies to quantitatively justify or determine the potential need for installation of a traffic signal at an otherwise unsignalized intersection. This TA uses the signal warrant criteria presented in the latest edition of the Caltrans California Manual on Uniform Traffic Control Devices (CA MUTCD). (4)

The signal warrant criteria for Existing study area intersections are based upon several factors, including volume of vehicular and pedestrian traffic, frequency of accidents, and location of school areas. The CA MUTCD indicates that the installation of a traffic signal should be considered if one or more of the signal warrants are met. (4) Specifically, this TA utilizes the Peak Hour Volume-based Warrant 3 as the appropriate representative traffic signal warrant analysis for existing traffic conditions and for all future analysis scenarios for existing unsignalized intersections. Warrant 3 is appropriate to use for this TA because it provides specialized warrant criteria for intersections with rural characteristics. For the purposes of this study, the speed limit was the basis for determining whether Urban or Rural warrants were used for a given intersection. Rural warrants have been used on the major roadways with unsignalized intersections with posted speed limits over 40 miles per hour, while urban warrants have been used where speeds are 40 miles per hour or below.

Future intersections that do not currently exist have been assessed regarding the potential need for new traffic signals based on future average daily traffic (ADT) volumes, using the Caltrans planning level ADT-based signal warrant analysis worksheets. Similarly, the speed limit has been used as the basis for determining the use of Urban and Rural warrants. Traffic signal warrant analyses were performed for the following study area intersections shown in Table 2-3.

TABLE 2-3: TRAFFIC SIGNAL WARRANT ANALYSIS LOCATIONS

#	Intersection
6	La Sierra Av. & El Sobrante Rd.
7	McAllister St. & El Sobrante Rd.
8	Street A & El Sobrante Rd.

The Existing conditions traffic signal warrant analysis is presented in the subsequent section, Section 3 *Area Conditions* of this report. The traffic signal warrant analyses for future conditions are presented in Section 5 *EAP (2027) Traffic Conditions* and Section 6 *EAPC (2027) Traffic Conditions* of this report. It is important to note that a signal warrant defines the minimum condition under which the installation of a traffic signal might be warranted. Meeting this threshold condition does not require that a traffic control signal be installed at a particular location, but rather, that other traffic factors and conditions be evaluated in order to determine whether the signal is truly justified. It should also be noted that signal warrants do not necessarily correlate with LOS. An intersection may satisfy a signal warrant condition and operate at or above acceptable LOS or operate below acceptable LOS and not meet a signal warrant.

2.4 QUEUING ANALYSIS

Consistent with Caltrans requirements, the 95th percentile queuing of vehicles has been assessed at the off-ramps to determine potential queuing deficiencies at the freeway ramp intersections of the SR-91 Freeway at the existing La Sierra Avenue interchanges. Specifically, the off-ramp queuing analysis is utilized to identify any potential queuing and "spill back" onto the SR-91 Freeway mainline from the off-ramps. The 95th percentile queue has also been utilized to assess the queues at La Sierra Avenue to identify any potential queuing. Additionally, the County has requested a queueing analysis at all signalized study area intersections, for both turn lanes and through lanes. The results of this analysis are presented in the subsequent sections of this traffic study.

The traffic progression analysis tool and HCM intersection analysis program, Synchro, has been used to assess the potential deficiencies/needs of the intersections with traffic added from the proposed Project. Storage (turn-pocket) length recommendations at the ramps have been based upon the 95th percentile queue resulting from the Synchro progression analysis. The footnote from the Synchro output sheets indicates if the 95th percentile cycle exceeds capacity. Traffic is simulated for two complete cycles of the 95th percentile traffic in Synchro in order to account for the effects of spillover between cycles. In practice, the 95th percentile queue shown will rarely be exceeded and the queues shown with the footnote are acceptable for the design of storage bays. The 95th percentile queue is derived from the average queue plus 1.65 standard deviations.

At the request of the County of Riverside, the traffic modeling and signal timing optimization software package Synchro/SimTraffic (Version 12) has been utilized to assess queues at the Project access points. Synchro is a macroscopic traffic software program that is based on the signalized and unsignalized intersection capacity analyses as specified in the HCM. SimTraffic is designed to model networks of signalized and unsignalized intersections, with the primary purpose of checking and fine-tuning signal operations. SimTraffic uses the input parameters from Synchro to generate random simulations.

The 95th percentile queue is not necessarily ever observed; it is simply based on statistical calculations (or Average Queue plus 1.65 standard deviations). Many jurisdictions utilize the 95th percentile queues for design purposes. SimTraffic simulations have been recorded 5 times, during the weekday AM and weekday PM peak hours, and has been seeded for 15-minute periods with 60-minute recording intervals. For the purposes of this traffic analysis, the County has requested the maximum queues be reported from the Simtraffic software. The results of the queuing analysis are presented in the subsequent sections. Queuing worksheets for the site adjacent queuing analysis are included in Appendix 1.2.

2.5 MINIMUM ACCEPTABLE LEVELS OF SERVICE (LOS)

Minimum acceptable LOS and associated definitions of intersection deficiencies has been obtained from each of the applicable surrounding jurisdictions.

2.5.1 COUNTY OF RIVERSIDE

The definition of an intersection deficiency has been obtained from the County of Riverside General Plan. Riverside County General Plan Policy C 2.1 states that the County will maintain the following County-wide target LOS:

The following minimum target levels of service have been designated for the review of development proposals in the unincorporated areas of Riverside County with respect to transportation impacts on roadways designated in the Riverside County Circulation Plan which are currently County maintained, or are intended to be accepted into the County maintained roadway system:

- *LOS C shall apply to all development proposals in any area of the Riverside County not located within the boundaries of an Area Plan, as well as those areas located within the following Area Plans: REMAP, Eastern Coachella Valley, Desert Center, Palo Verde Valley, and those non-Community Development areas of the Elsinore, Lake Mathews/Woodcrest, Mead Valley and Temescal Canyon Area Plans.*
- *LOS D shall apply to all development proposals located within any of the following Area Plans: Eastvale, Jurupa, Highgrove, Reche Canyon/Badlands, Lakeview/Nuevo, Sun City/Menifee Valley, Harvest Valley/Winchester, Southwest Area, The Pass, San Jacinto Valley, Western Coachella Valley and those Community Development Areas of the Elsinore, Lake Mathews/Woodcrest, Mead Valley and Temescal Canyon Area Plans.*
- *LOS E may be allowed by the Board of Supervisors within designated areas where transit-oriented development and walkable communities are proposed.*

The applicable minimum LOS utilized for the purposes of this analysis is LOS D per the County-wide target LOS for projects located within the Lake Mathews/Woodcrest Area Plan.

2.5.2 CITY OF RIVERSIDE

Per the City of Riverside traffic study guidelines, for Projects that are in conformance with the General Plan:

a) LOS C is to be maintained at all street intersections;

b) LOS D is to be maintained at intersections of Collector or higher classification (see General Plan Policy CCM 2.3).

As such, and consistent with the County of Riverside, LOS D has been utilized as the minimum acceptable LOS for intersections within the City of Riverside.

2.5.3 CALTRANS

Senate Bill 743 (SB 743), approved in 2013, endeavors to change the way transportation impacts will be determined according to the California Environmental Quality Act (CEQA). The Office of Planning and Research (OPR) has recommended the use of vehicle miles traveled (VMT) as the replacement for automobile delay-based LOS. Caltrans acknowledges automobile delay will no longer be considered a CEQA impact for development projects and will use VMT as the metric for determining impacts on the State Highway System. However, LOS D has been utilized as the target LOS for Caltrans facilities, consistent with the County of Riverside.

2.6 DEFICIENCY CRITERIA

This section outlines the methodology used in this analysis related to identifying circulation system deficiencies. The following deficiency criteria has been utilized for the County of Riverside. To determine whether the addition of project-related traffic at a study intersection would result in a deficiency, the following will be utilized:

- A deficiency occurs at study area intersections if the pre-Project condition is at or better than LOS D (i.e., acceptable LOS), and the addition of project trips causes the peak hour LOS of the study area intersection to operate at unacceptable LOS (i.e., LOS E or F). Per the County of Riverside traffic study guidelines, for intersections currently operating at unacceptable LOS (LOS E or F), a deficiency will occur if the Project contributes peak hour trips to pre-project traffic conditions.

2.7 PROJECT FAIR SHARE CALCULATION METHODOLOGY

Improvements found to be included in the County TUMF and/or DIF programs will be identified as such. For improvements that do not appear to be in either of the pre-existing fee programs, a fair share contribution based on the Project's proportional share may be imposed in order to address the Project's share of deficiencies in lieu of construction. It should be noted that fair share calculations are for informational purposes only and the County Traffic Engineer will determine the appropriate improvements to be implemented by a project (to be identified in the conditions of approval). The Project's fair share cost of improvements would be determined based on the following equation, which is the ratio of Project traffic to new traffic, where new traffic is total future traffic less existing baseline traffic:

Project Fair Share % = Project Traffic / (EAPC (2027) Total Traffic – Existing (2024) Traffic)

3 AREA CONDITIONS

This section provides a summary of the existing circulation network, the County of Riverside General Plan Circulation Network, and a review of existing peak hour intersection operations, traffic signal warrant, and off-ramp queuing analyses.

3.1 EXISTING CIRCULATION NETWORK

Pursuant to the scoping agreement with County of Riverside staff (Appendix 1.1), the study area includes a total of 8 existing and future intersections as shown previously on Exhibit 1-3, where the Project is anticipated to contribute 50 or more peak hour trips or were added at the County's request during the scoping process. Exhibit 3-1 illustrates the study area intersections located near the proposed Project and identifies the number of through traffic lanes for existing roadways and intersection traffic controls.

3.2 COUNTY OF RIVERSIDE GENERAL PLAN CIRCULATION ELEMENT

As noted previously, the Project site is located within the County of Riverside. The roadway classifications and planned (ultimate) roadway cross-sections of the major roadways within the study area, as identified on County of Riverside General Plan Circulation Element, are described subsequently. Exhibit 3-2 shows the County of Riverside General Plan Circulation Element and Exhibit 3-3 illustrates the County of Riverside General Plan roadway cross-sections.

Arterials are four-lane divided roadways (typically divided by a raised median or painted two-way turn-lane) with a 128-foot right-of-way and an 86-foot curb-to-curb measurement. These roadways serve both regional through-traffic and inter-city traffic and typically direct traffic onto and off-of the freeways. The following study area roadways within the County of Riverside are classified as an Arterial:

- La Sierra Avenue
- El Sobrante Road

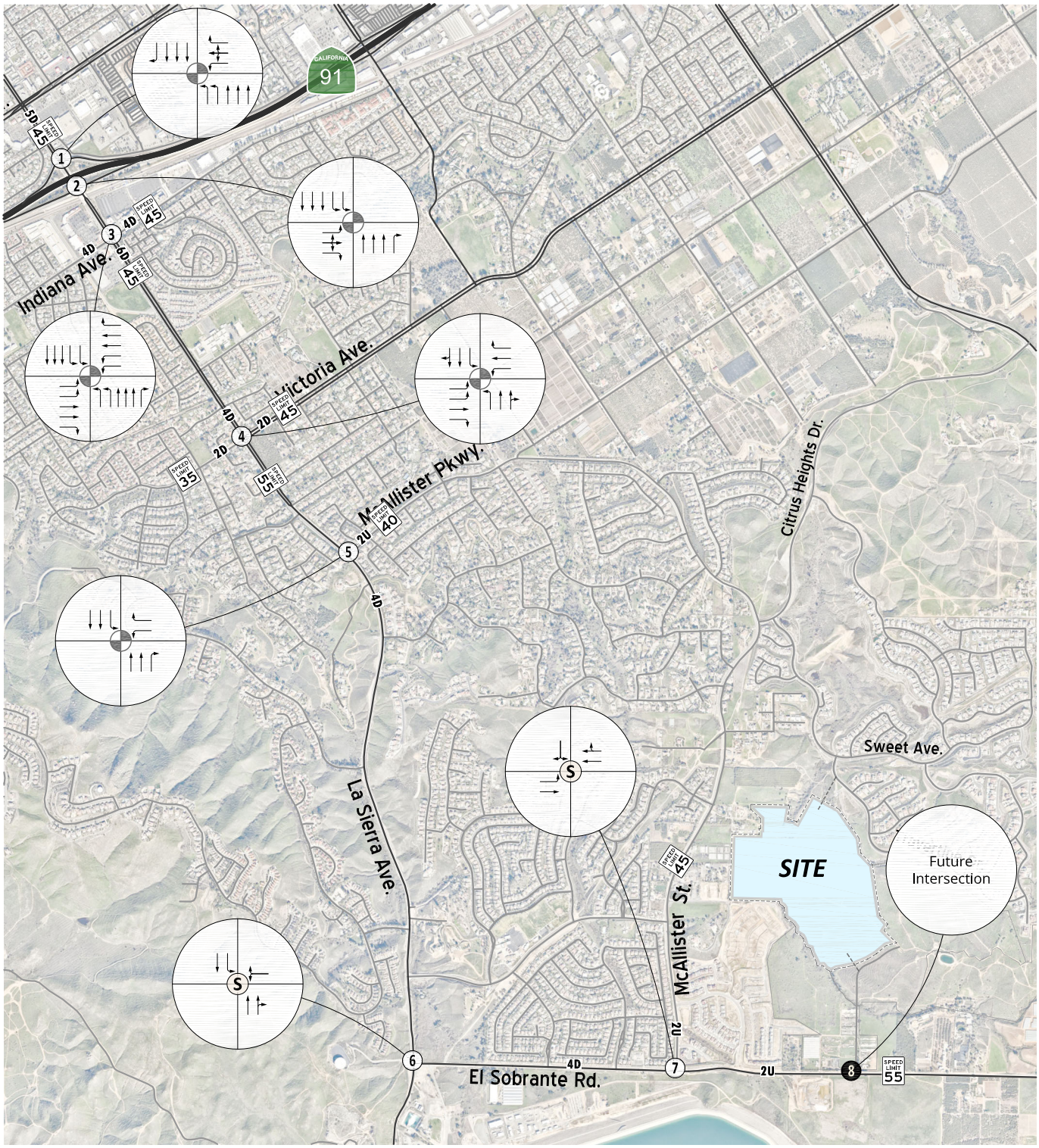
Collectors are two-lane roadways. These roadways typically have a 74-foot right-of-way and a 40-foot curb-to-curb measurement. The following study area roadway within the County of Riverside is classified as a Collector:

- McAllister Street

3.3 CITY OF RIVERSIDE GENERAL PLAN CIRCULATION ELEMENT

Exhibit 3-4 shows the City of Riverside General Plan Circulation Element.

EXHIBIT 3-1 : EXISTING NUMBER OF THROUGH LANES AND INTERSECTION CONTROLS



LEGEND

- (0) = *Existing Intersection Analysis Location*
- (0) = *Future Intersection Analysis Location*
- = *Existing Traffic Signal*
- (S) = *Existing All-Way Stop*
- = *Existing Traffic Lane*

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EXHIBIT 3-2 : COUNTY OF RIVERSIDE GENERAL PLAN CIRCULATION ELEMENT

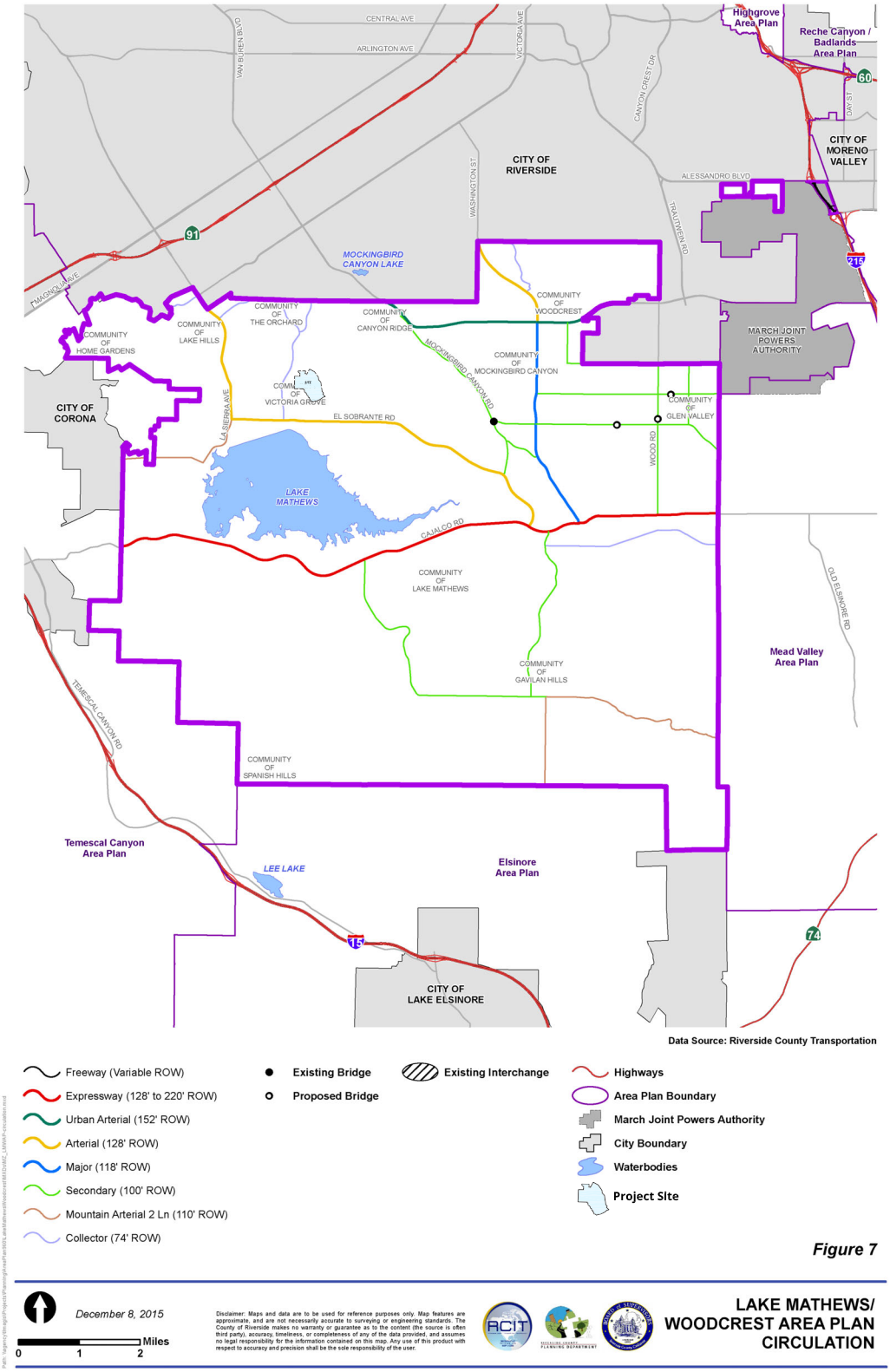
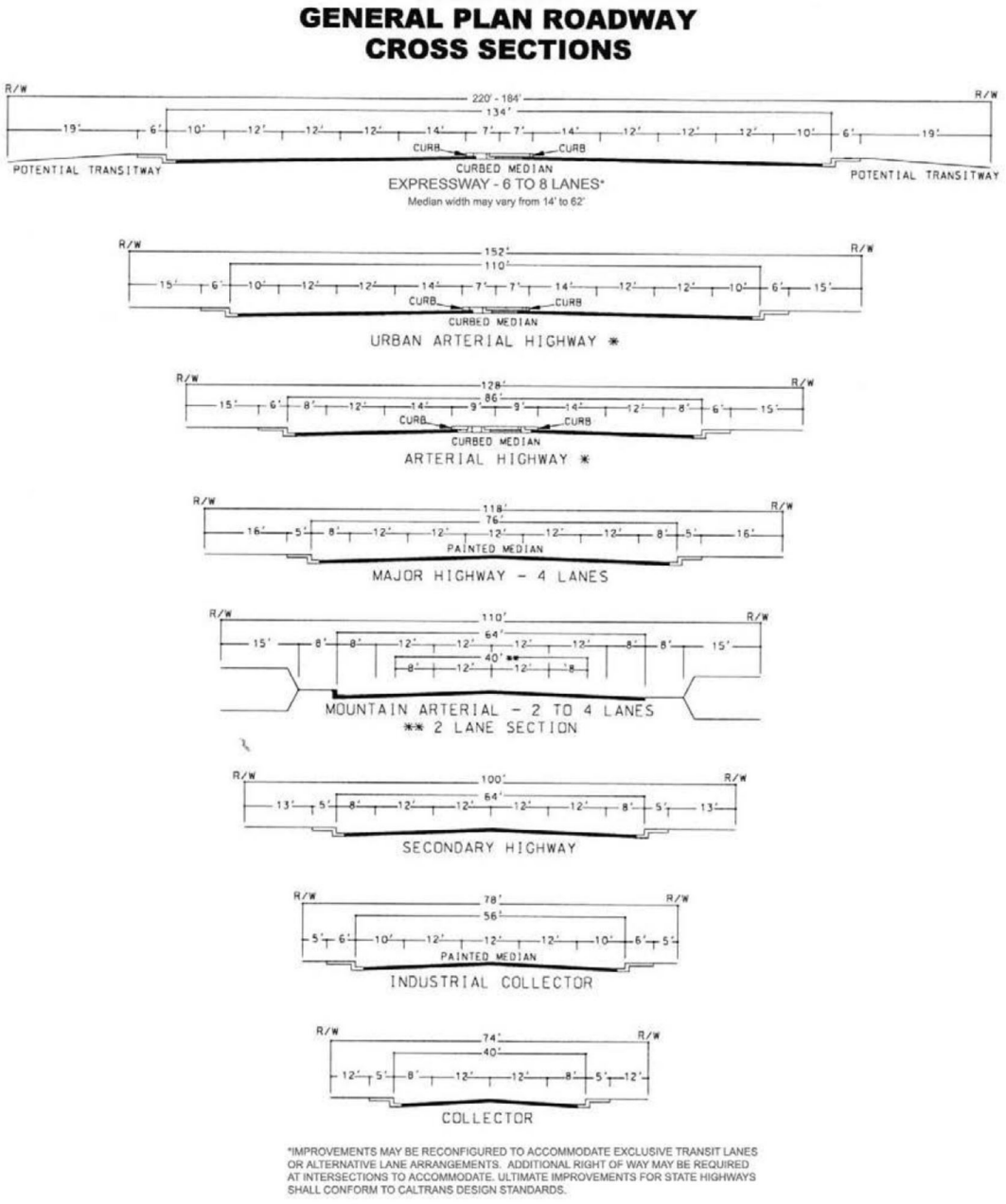


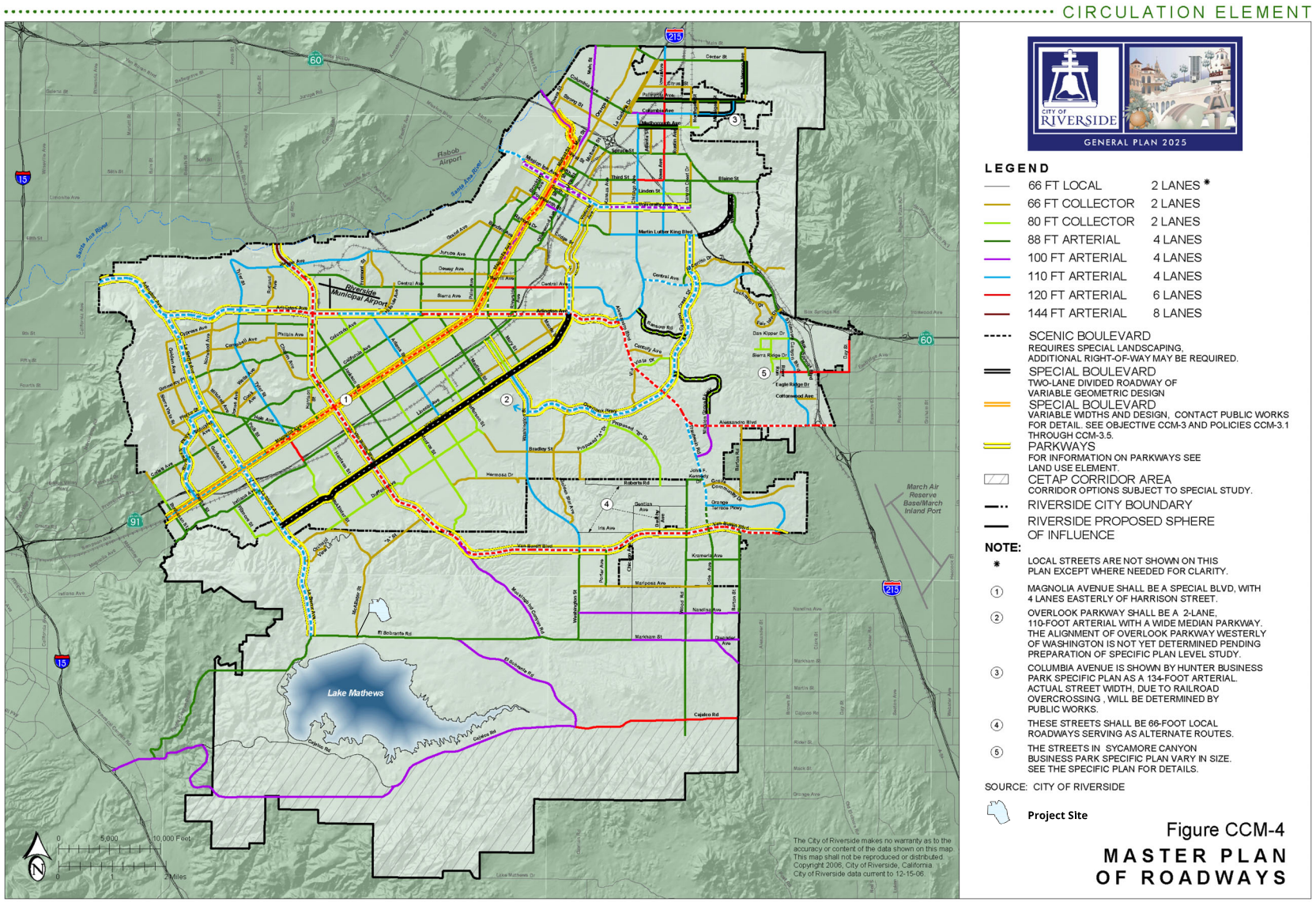
EXHIBIT 3-3 : COUNTY OF RIVERSIDE GENERAL PLAN ROADWAY CROSS-SECTIONS

Figure C-3 Street Classification Cross-Sections



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EXHIBIT 3-4 : CITY OF RIVERSIDE GENERAL PLAN CIRCULATION ELEMENT



3.4 BICYCLE & PEDESTRIAN FACILITIES

The County of Riverside and City of Riverside bike networks are shown on Exhibit 3-5 and Exhibit 3-6, respectively. As shown on Exhibit 3-6, there is a planned Combination Trail (Regional Trail and Class I Bike Path proposed along La Sierra Avenue and El Sobrante Road. There is a proposed Regional Trail (Urban/Suburban) proposed along McAllister Street and a Community Trail proposed in the vicinity of the Project site.

Exhibit 3-7 illustrates the existing crosswalks throughout the study area. As shown on Exhibit 3-7, there are pedestrian facilities in place in the vicinity of the Project site along portions of McAllister Street and El Sobrante Road.

3.5 TRANSIT SERVICE

The study area is currently served by Riverside Transit Agency (RTA). There are currently no transit routes or stops along El Sobrante Road near the proposed Project. The transit services are illustrated on Exhibit 3-8. Transit service is reviewed and updated by RTA periodically to address ridership, budget, and community demand needs. Changes in land use can affect these periodic adjustments which may lead to either enhanced or reduced service where appropriate.

3.6 EXISTING (2024) TRAFFIC COUNTS

The intersection LOS analysis is based on the traffic volumes observed during the peak hour conditions using traffic count data collected in April 2024 when local schools were in session and operating on normal bell schedules. The following peak hours were selected for analysis:

- Weekday AM Peak Hour (peak hour between 7:00 AM and 9:00 AM)
- Weekday PM Peak Hour (peak hour between 4:00 PM and 6:00 PM)

Local schools were in session and operating on normal bell schedules at the time traffic counts were collected. Construction activity was occurring at the intersection of McAllister Street at El Sobrante Road (#7); however, the usual number of lanes were not affected. The raw manual peak hour turning movement traffic count data sheets are included in Appendix 3.1.

Existing weekday ADT volumes on arterial highways throughout the study area are shown on Exhibit 3-9. Existing ADT volumes were based upon factored intersection peak hour counts collected by Urban Crossroads, Inc. using the following formula for each intersection leg:

Weekday PM Peak Hour (Approach Volume + Exit Volume) x 10.57 = Leg Volume

A comparison of the PM peak hour and daily traffic volumes of various roadway segments within the study area indicated that the peak-to-daily relationship is approximately 9.46 percent. As such, the above equation utilizing a factor of 10.57 estimates the ADT volumes on the study area roadway segments assuming a peak-to-daily relationship of approximately 9.46 percent (i.e., 1/0.0946 = 10.57) and was assumed to sufficiently estimate ADT volumes for planning-level analyses. This factor is consistent with that used for other traffic studies within the study area. Existing weekday AM and weekday PM peak hour intersection volumes are shown on Exhibit 3-9.

EXHIBIT 3-5 : COUNTY OF RIVERSIDE GENERAL PLAN BIKE NETWORK

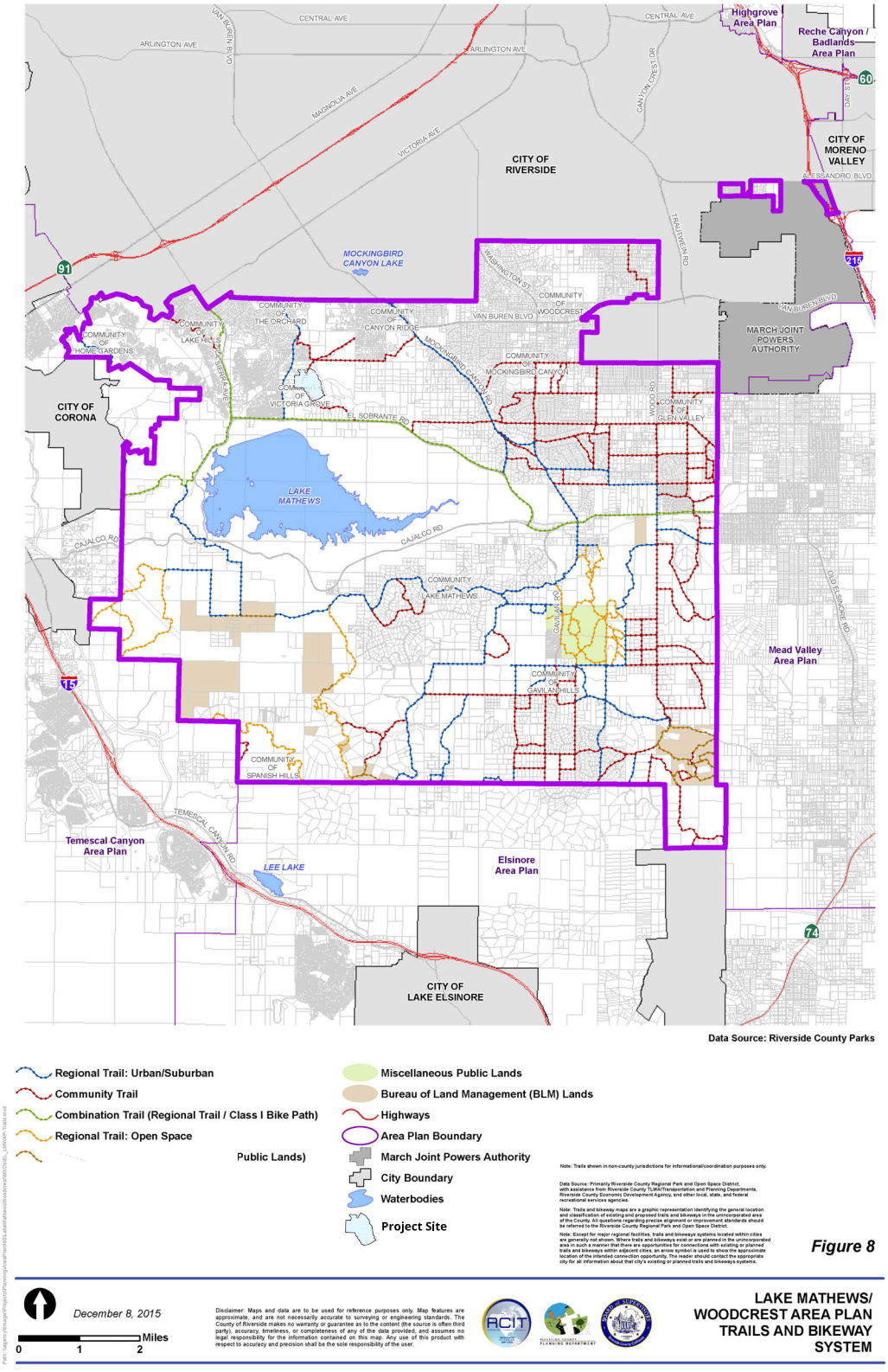
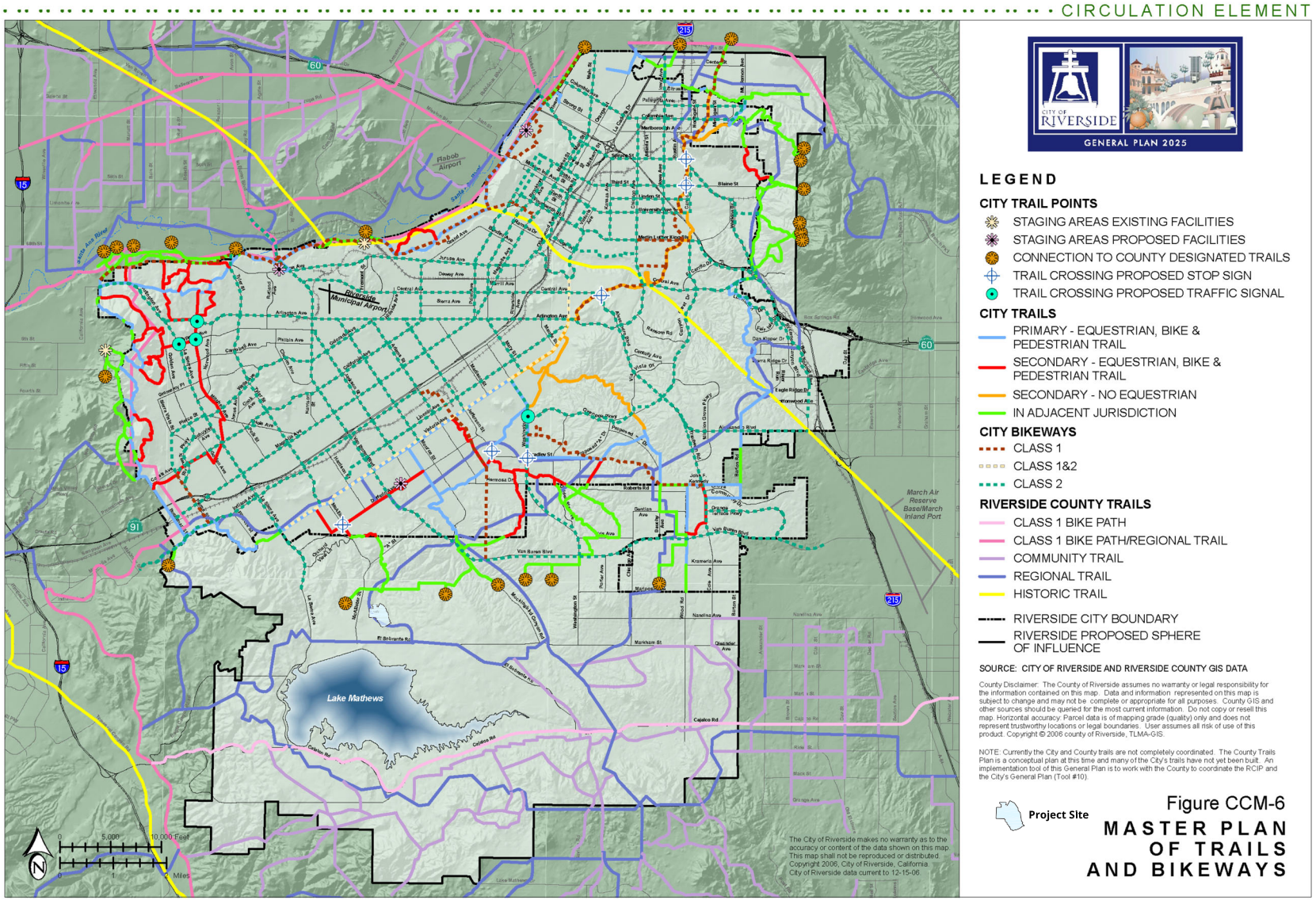
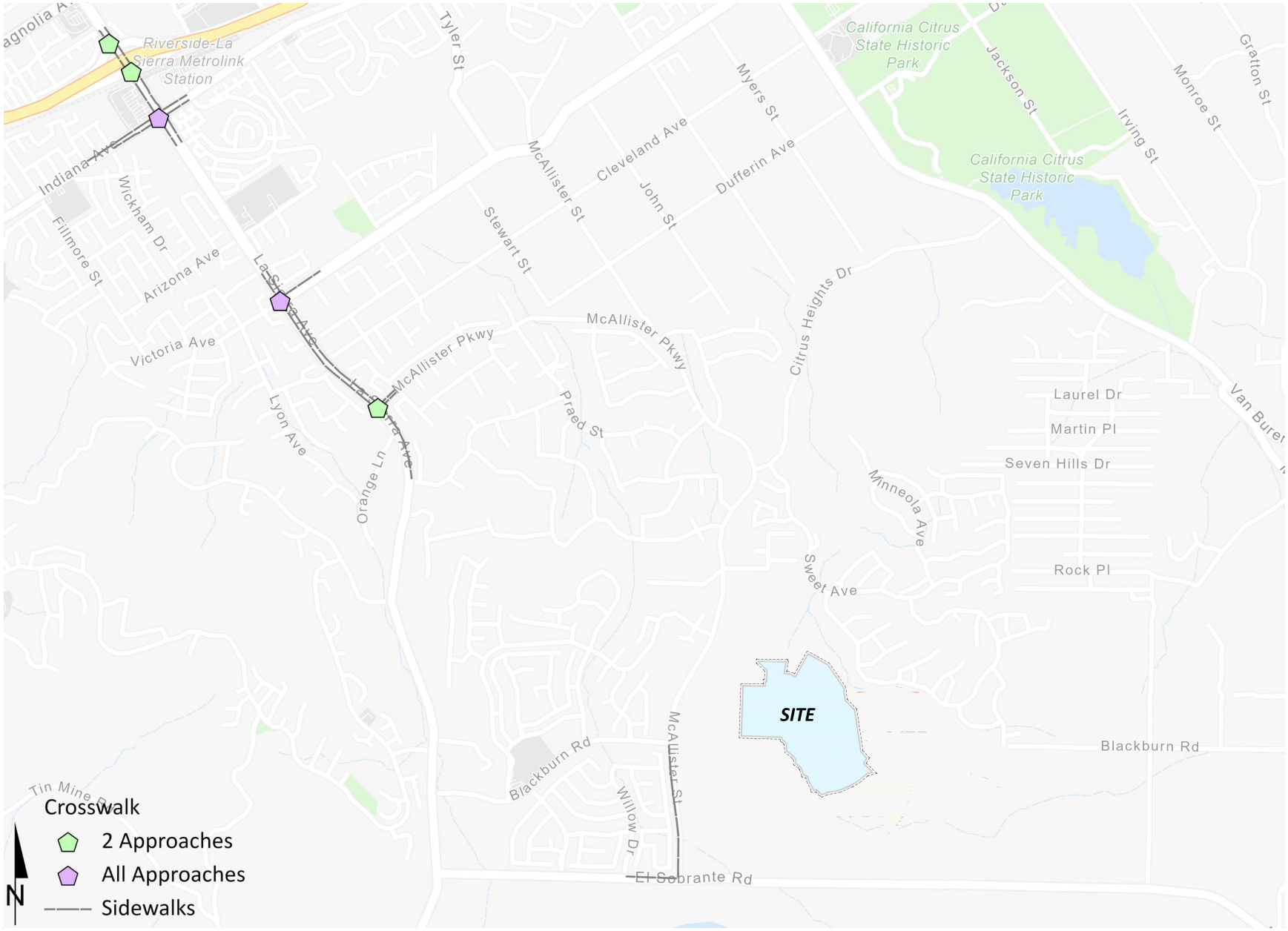


EXHIBIT 3-6 : CITY OF RIVERSIDE BIKE PLAN



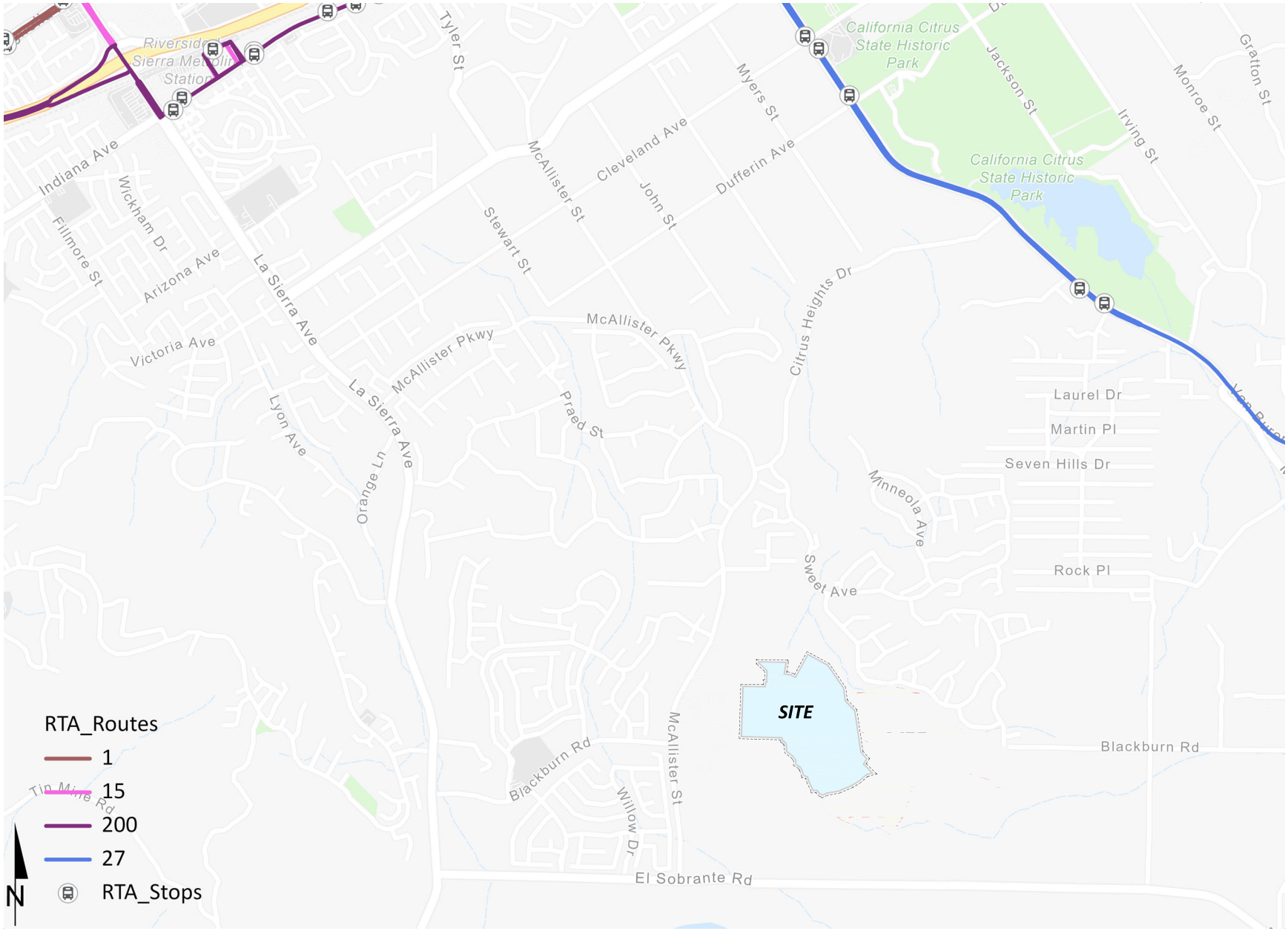
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EXHIBIT 3-7 : EXISTING PEDESTRIAN FACILITIES



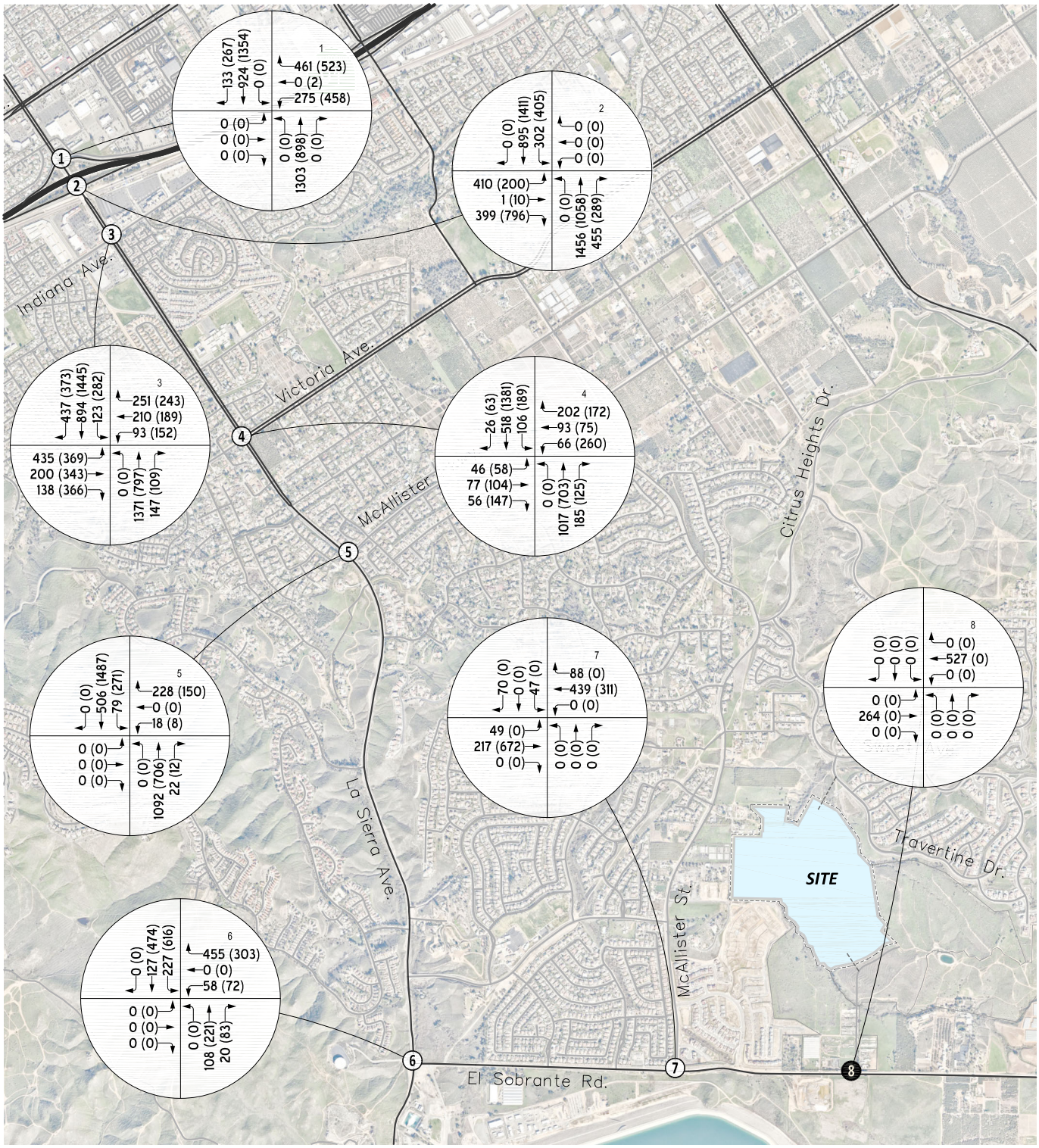
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EXHIBIT 3-8 : EXISTING TRANSIT ROUTES



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EXHIBIT 3-9 : EXISTING (2024) TRAFFIC VOLUMES



LEGEND

(0) = *Existing Intersection Analysis Location*

(0) = *Future Intersection Analysis Location*

00 (00) = *Peak Hour Intersection Volume AM (PM)*

##.# = *Average Daily Trips (1000's)*

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3.7 INTERSECTION OPERATIONS ANALYSIS

Existing peak hour traffic operations have been evaluated for the study area intersections based on the analysis methodologies presented in Section 2.2 *Intersection Capacity Analysis* of this report. The intersection operations analysis results are summarized in Table 3-1, which indicates that all existing study area intersections are currently operating at acceptable LOS during the peak hours, with the exception of the following intersections:

- La Sierra Avenue & El Sobrante Road (#6) – LOS F PM peak hour only
- McAllister Street & El Sobrante Road (#7) – LOS E PM peak hour only

The intersection operations analysis worksheets are included in Appendix 3.2 of this TA.

TABLE 3-1: INTERSECTION ANALYSIS FOR EXISTING (2024) CONDITIONS

#	Intersection	Traffic Control[2]	Delay[1] (secs.) AM	Delay[1] (secs.) PM	Level of Service AM	Level of Service PM
1	La Sierra Av. & SR-91 WB Ramps	TS	17.9	20.0	B	C
2	La Sierra Av. & SR-91 EB Ramps	TS	22.0	27.8	C	C
3	La Sierra Av. & Indiana Av.	TS	31.8	34.9	C	C
4	La Sierra Av. & Victoria Av.	TS	23.0	36.0	C	D
5	La Sierra Av. & McAllister St.	TS	11.6	10.2	B	B
6	La Sierra Av. & El Sobrante Rd.	AWS	16.3	**75.3**	C	**F**
7	McAllister St. & El Sobrante Rd.	AWS	10.7	**39.4**	B	**E**
8	Street A & El Sobrante Rd.		Future Intersection			

* BOLD = Level of Service (LOS) does not meet the applicable jurisdictional requirements (i.e., unacceptable LOS).

[1] Per the Highway Capacity Manual (7th Edition), overall average intersection Delay and level of service are shown for intersections with a traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown. HCM delay reported in seconds.

[2] TS = Traffic Signal; AWS = All-way Stop

3.8 TRAFFIC SIGNAL WARRANTS ANALYSIS

Traffic signal warrants for Existing traffic conditions are based on existing peak hour intersection turning volumes. The following unsignalized study area intersections currently meet a peak hour traffic signal under Existing traffic conditions:

- La Sierra Avenue & El Sobrante Road (#6)
- McAllister Street & El Sobrante Road (#7)

Existing conditions traffic signal warrant analysis worksheets are provided in Appendix 3.3.

3.9 QUEUING ANALYSIS

A queuing analysis was performed for all movements at signalized study area intersections. Queuing analysis findings are presented in Table 3-2. As shown in Table 3-2, the following movements currently experience queuing issues during the weekday AM or weekday PM peak 95th percentile traffic flows:

- La Sierra Avenue & SR-91 WB Ramps (#1), northbound left turn lane – AM and PM peak hours
- La Sierra Avenue & SR-91 WB Ramps (#1), southbound through lane – PM peak hour only
- La Sierra Avenue & SR-91 EB Ramps (#2), southbound left turn lane – PM peak hour only
- La Sierra Avenue & Indiana Avenue (#3), northbound left turn lane – AM peak hour only
- La Sierra Avenue & Indiana Avenue (#3), eastbound left turn lane – AM and PM peak hours
- La Sierra Avenue & Indiana Avenue (#3), eastbound right turn lane – PM peak hour only
- La Sierra Avenue & Victoria Avenue (#4), southbound left turn lane – PM peak hour only
- La Sierra Avenue & Victoria Avenue (#4), westbound left turn lane – PM peak hour only
- La Sierra Avenue & McAllister Parkway (#5), southbound left turn lane – PM peak hour only

It should be noted that the Simtraffic maximum queues have been reported at the request of the County. However, improvements have been identified to address the 95th percentile queueing deficiencies. Worksheets for Existing (2024) traffic conditions off-ramp queuing analysis are provided in Appendix 3.4.

TABLE 3-2: PEAK HOUR QUEUING SUMMARY FOR EXISTING (2024) CONDITIONS

Intersection	Movement	Available Stacking Distance (Feet)	Synchro: 95th Percentile Queue (Feet)		Acceptable?[1]		Simtraffic: Maximum Queue (Feet)	
			AM Peak Hour	PM Peak Hour	AM	PM	AM Peak Hour	PM Peak Hour
La Sierra Av. & SR-91 WB Ramps (#1)	NBL	150	**271**	**176**	**No**	**No**	347	224
	NBT	450	313	251	Yes	Yes	263	237
	SBT	300	245	**342**	Yes	**No**	402	667
	SBR	150	49	96	Yes	Yes	138	188
	WBL	500	226	300	Yes	Yes	225	267
	WBT	1,200	187	235	Yes	Yes	327	364
	WBR	500	180	197	Yes	Yes	270	316
La Sierra Av. & SR-91 EB Ramps (#2)	NBT	900	359	284	Yes	Yes	435	338
	NBR	415	61	62	Yes	Yes	214	137
	SBL	150	120	**188** [2]	Yes	**No**	204	227
	SBT	450	29	281	Yes	Yes	198	337
	EBL	1,690	252	128	Yes	Yes	372	224
	EBT	1,690	218	346	Yes	Yes	489	373
	EBR	450	153	290	Yes	Yes	417	359
La Sierra Av. & Indiana Av. (#3)	NBL	200	**234** [2]	174 [2]	**No**	Yes	256	202
	NBT	590	508 [2]	254	Yes	Yes	363	251
	NBR	315	49	44	Yes	Yes	146	86
	SBL	355	111 [2]	207 [2]	Yes	Yes	103	220
	SBT	900	312	542 [2]	Yes	Yes	223	393
	SBR	540	91	72	Yes	Yes	257	195
	EBL	205	**294** [2]	**263** [2]	**No**	**No**	260	263
	EBT	1,010	84	151	Yes	Yes	328	386
	EBR	205	41	**236**	Yes	**No**	85	255
	WBL	200	70	101	Yes	Yes	82	114
	WBT	535	101	91	Yes	Yes	191	116
	WBR	130	122	116	Yes	Yes	180	136
La Sierra Av. & Victoria Av. (#4)	NBL	210	73	55	Yes	Yes	170	188
	NBT	1,300	578	397	Yes	Yes	374	338
	SBL	210	187 [2]	**301** [2]	Yes	**No**	163	275
	SBT	1,310	202	841 [2]	Yes	Yes	198	526
	EBL	130	41	48	Yes	Yes	83	101
	EBT	325	98	121	Yes	Yes	128	147
	EBR	325	7	30	Yes	Yes	63	135
	WBL	200	129 [2]	**412** [2]	Yes	**No**	110	252
	WBT	530	111	81	Yes	Yes	138	175
	WBR	200	89	50	Yes	Yes	157	121
La Sierra Av. & McAllister St. (#5)	NBT	1,475	436	306	Yes	Yes	228	206
	NBR	200	16	14	Yes	Yes	30	30
	SBL	150	114	**280**	Yes	**No**	112	285
	SBT	1,065	107	431	Yes	Yes	117	296
	WBL	150	32	19	Yes	Yes	41	29
	WBR	400	62	53	Yes	Yes	180	103

[1] Stacking distance is acceptable if the required stacking distance is less than or equal to the stacking distance provided for the 95th percentile queue only. An additional 15 feet of stacking which is assumed to be provided in the transition for turn pockets is reflected in the stacking distance shown on this table, where applicable.

[2] 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

3.10 PROJECT DEFICIENCIES AND RECOMMENDED IMPROVEMENTS

This section provides a summary of Project deficiencies and recommended improvements based on the County of Riverside deficiency criteria discussed in Section 2.6 *Deficiency Criteria*.

3.10.1 INTERSECTIONS

Intersection improvements necessary to improve project-related traffic deficiencies are shown in Table 3-3. The improvements have been identified to improve the deficiencies back to acceptable levels. Worksheets for Existing (2024) conditions, with improvements, HCM calculation worksheets are provided in Appendix 3.5.

TABLE 3-3: INTERSECTION ANALYSIS FOR EXISTING (2024) CONDITIONS WITH IMPROVEMENTS

			Intersection Approach Lanes[1]												Delay[2] (secs.)		Level of Service	
		Traffic	Northbound			Southbound			Eastbound			Westbound						
#	Intersection	Control[3]	L	T	R	L	T	R	L	T	R	L	T	R	AM	PM	AM	PM
6	La Sierra Av. & El Sobrante Rd.																	
	- Without Improvements	AWS	0	2	0	1	1	0	0	0	0	0	1	0	16.3	**75.3**	C	**F**
	- With Improvements	**TS**	0	2	0	1	1	0	0	0	0	0	1	0	17.2	21.8	B	C
7	McAllister St. & El Sobrante Rd.																	
	- Without Improvements	AWS	0	0	0	0	1	0	1	1	0	0	2	0	10.7	**39.4**	B	**E**
	- With Improvements	**TS**	0	0	0	0	1	0	1	1	0	0	2	0	8.5	8.1	A	A

BOLD = LOS does not meet the applicable jurisdictional requirements (i.e., unacceptable LOS).

[1] When a right turn is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; **1** = Improvement

[2] Per the Highway Capacity Manual (7th Edition), overall average intersection Delay and level of service are shown for intersections with a traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown. HCM delay reported in seconds.

[3] TS = Traffic Signal; AWS = All-way Stop; **TS** = Improvement

3.10.2 QUEUES

Improvements to address queuing deficiencies have been identified where feasible and are shown in Table 3-4. Improvements have not been identified at the following movements, as improvements are not considered feasible due to physical constraints on the existing roadways:

- La Sierra Avenue & SR-91 westbound Ramps (#1), northbound left turn lane
- La Sierra Avenue & SR-91 westbound Ramps (#1), southbound through lane
- La Sierra Avenue & SR-91 eastbound Ramps (#2), southbound left turn lane
- La Sierra Avenue & Indiana Avenue (#3), eastbound left turn lane

TABLE 3-4: PEAK HOUR QUEUING SUMMARY FOR EXISTING (2024) CONDITIONS WITH IMPROVEMENTS

Intersection	Movement	Available Stacking Distance (Feet)[3]	Synchro: 95th Percentile Queue (Feet) AM Peak Hour	Synchro: 95th Percentile Queue (Feet) PM Peak Hour	Acceptable? [1] AM	Acceptable? [1] PM
La Sierra Av. & Indiana Av. (#3)	NBL	**325**	234 [2]	174 [2]	Yes	Yes
La Sierra Av. & Victoria Av. (#4)	SBL	**325**	187 [2]	301 [2]	Yes	Yes
	WBL	**500**	129 [2]	412 [2]	Yes	Yes
La Sierra Av. & McAllister St. (#5)	SBL	**400**	137	280	Yes	Yes

[1] Stacking Distance is acceptable if the required stacking distance is less than or equal to the stacking distance provided for the 95th percentile queue only. An additional 15 feet of stacking which is assumed to be provided in the transition for turn pockets is reflected in the stacking distance shown on this table, where applicable.

[2] 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

[3] **100** = Improvement

4 PROJECTED FUTURE TRAFFIC

This section presents the traffic volumes estimated to be generated by the Project, as well as the Project's trip assignment onto the study area roadway network. The Project is anticipated to have an Opening Year of 2027. The Project includes the development of 206 single-family detached residential dwelling units. Primary access to the Project site will be accommodated via a new connection to El Sobrante Road (via Street A) and a new connection onto Travertine Drive. Street A on El Sobrante Road and on Travertine Drive are assumed to allow for full access (no left turn restrictions). Regional access to the Project site is available from the SR-91 Freeway via the La Sierra Avenue interchange.

4.1 PROJECT TRIP GENERATION

4.1.1 PROPOSED PROJECT TRIP GENERATION

Trip generation represents the amount of traffic which is both attracted to and produced by a development. Determining traffic generation for a specific project is therefore based upon forecasting the amount of traffic that is expected to be both attracted to and produced by the specific land uses being proposed for a given development. In order to develop the traffic characteristics of the proposed Project, trip-generation statistics published in the ITE Trip Generation Manual (11th Edition, 2021) for Single Family Detached Residential (ITE Land Use Code 210) was used to calculate the trip generation. (5)

The trip generation summary illustrating daily and peak hour trip generation estimates for the proposed Project are shown in Table 4-1. The Project is anticipated to generate 1,944 two-way trip-ends per day with 144 AM peak hour trips and 194 PM peak hour trips.

TABLE 4-1: PROJECT TRIP GENERATION SUMMARY

Land Use[1]	Units[2]	ITE LU Code	AM Peak Hour In	AM Peak Hour Out	AM Peak Hour Total	PM Peak Hour In	PM Peak Hour Out	PM Peak Hour Total	Daily
Single Family Detached Residential	DU	210	0.18	0.52	0.70	0.59	0.35	0.94	9.43

[1] Trip Generation Source: Institute of Transportation Engineers (ITE), Trip Generation Manual, Eleventh Edition (2021).

[2] DU = Dwelling Units

Land Use	Quantity	Units[1]	AM Peak Hour In	AM Peak Hour Out	AM Peak Hour Total	PM Peak Hour In	PM Peak Hour Out	PM Peak Hour Total	Daily
Highland Grove Vistas 4 (TTM No. 38927)	206	DU	37	107	144	122	72	194	1,944

[1] DU = Dwelling Units

4.2 PROJECT TRIP DISTRIBUTION

The Project trip distribution represents the directional orientation of traffic to and from the Project site. Trip distribution is the process of identifying the probable destinations, directions, or traffic routes that will be utilized by Project traffic. The potential interaction between the planned land uses and surrounding regional access routes are considered, to identify the route where the Project traffic would distribute. Exhibit 4-1 shows the Project trip distribution patterns. Project trip distribution patterns are based on traffic counts from 2023 for the applicable study area intersections (Attachment B of the Scoping Agreement in Appendix 1.1)

4.3 MODAL SPLIT

The potential for Project trips (non-truck) to be reduced by the use of public transit, walking, or bicycling have not been included as part of the Project's estimated trip generation. Essentially, the Project's traffic projections are "conservative" in that these alternative travel modes would reduce the forecasted traffic volumes.

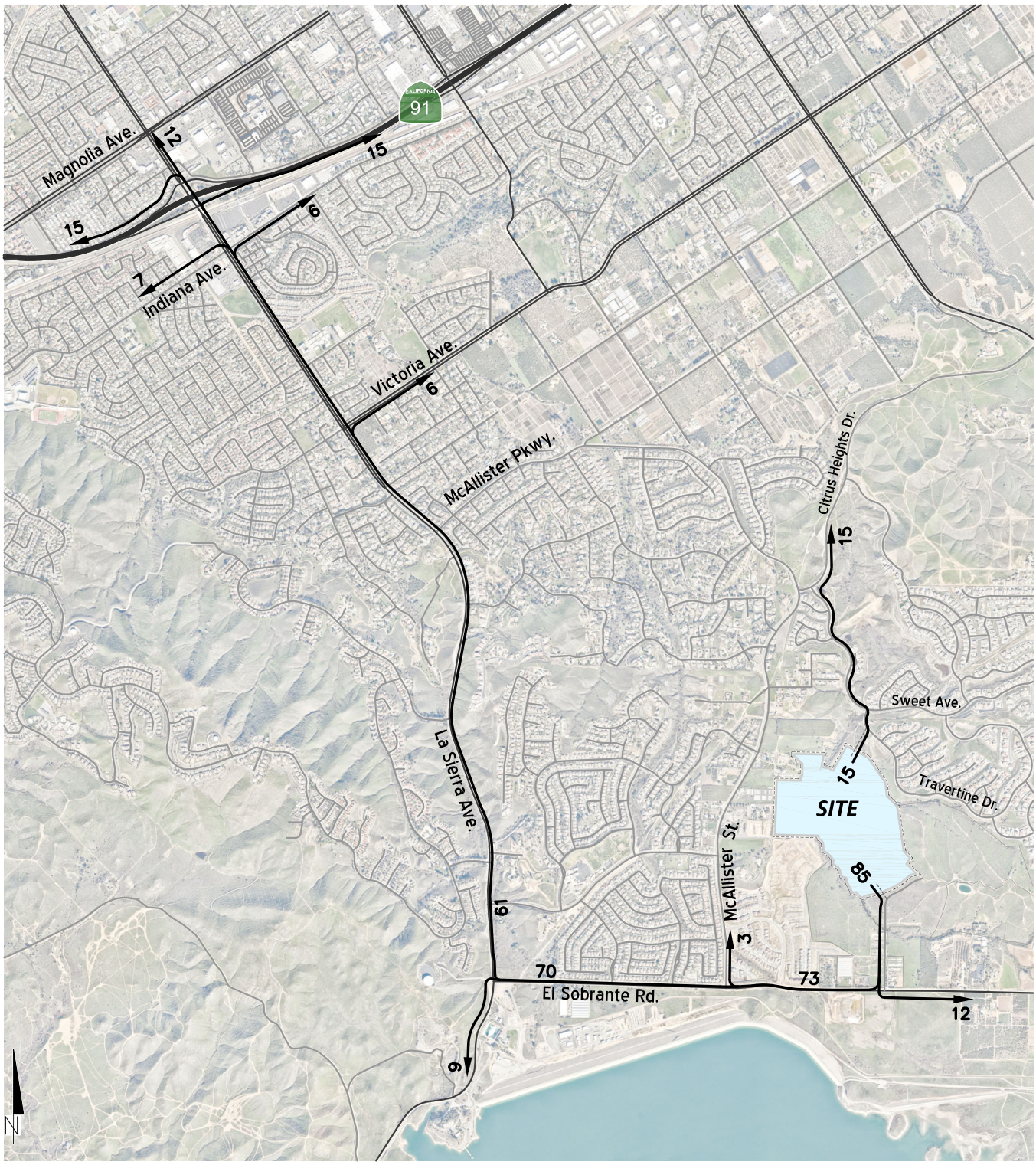
4.4 PROJECT TRIP ASSIGNMENT

The assignment of traffic from the Project area to the adjoining roadway system is based upon the Project trip generation, trip distribution, and the arterial highway and local street system improvements that would be in place by the time of initial occupancy of the Project. Based on the identified Project traffic generation and trip distribution patterns, the Project only ADT and peak hour intersection turning movement volumes are shown on Exhibit 4-2.

4.5 BACKGROUND TRAFFIC

Future year traffic forecasts have been based upon background (ambient) growth at 2% per year, compounded annually, for 2027 conditions. The total ambient growth is 6.12% for 2027 traffic conditions (compounded growth of 2 percent per year over 3 years or $1.02^{3\ years}$). The ambient growth factor is intended to approximate regional traffic growth. This ambient growth rate is added to existing traffic volumes to account for area-wide growth not reflected by cumulative development projects. Ambient growth has been added to daily and peak hour traffic volumes on surrounding roadways, in addition to traffic generated by the development of future projects that have been approved but not yet built and/or for which development applications have been filed and are under consideration by governing agencies.

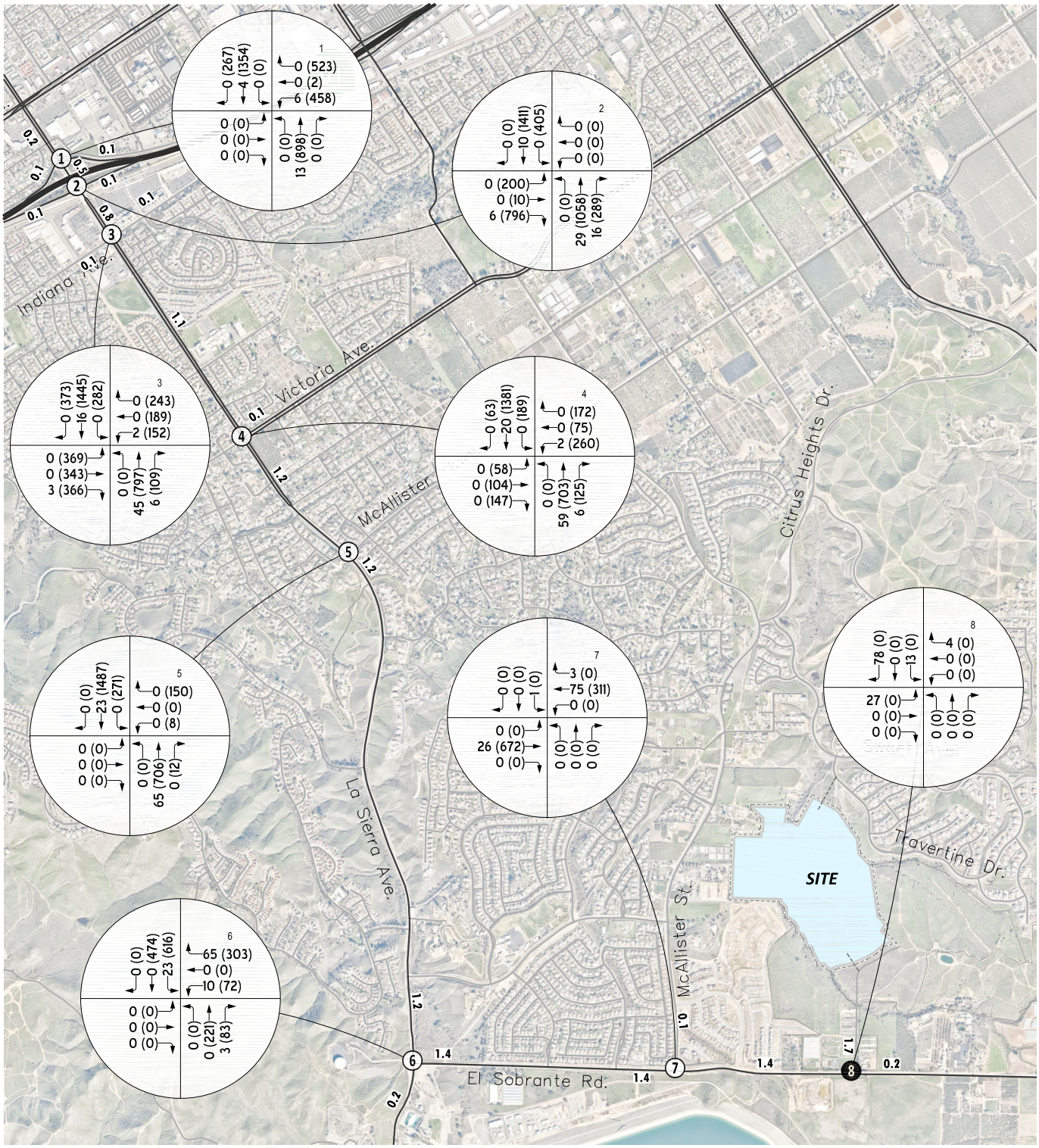
EXHIBIT 4-1 : PROJECT TRIP DISTRIBUTION



LEGEND

10 = *Percent To/From Project*
→ = *Trip Distribution*

EXHIBIT 4-2 : PROJECT ONLY TRAFFIC VOLUMES



LEGEND

(0) = *Existing Intersection Analysis Location*	00 (00) = *Peak Hour Intersection Volume AM (PM)*
❶ = *Future Intersection Analysis Location*	##.# = *Average Daily Trips (1000's)*

4.6 CUMULATIVE DEVELOPMENT TRAFFIC

A cumulative project list was developed for the purposes of this analysis through consultation with planning and engineering staff from the County of Riverside and City of Riverside. The cumulative project list includes known and foreseeable projects that are anticipated to contribute traffic to the study area intersections.

Where applicable, cumulative projects anticipated to contribute measurable traffic (i.e., 50 or more peak hour trips) to study area intersections have been manually added to the study area network to generate EAPC forecasts. In other words, this list of cumulative development projects has been reviewed to determine which projects would likely contribute measurable traffic through the study area intersections (e.g., those cumulative projects in close proximity to the proposed Project). For the purposes of this analysis, the cumulative projects that were determined to affect one or more of the study area intersections are shown on Exhibit 4-3, listed in Table 4-2, and have been considered for inclusion. Any additional traffic generated by other projects not on the cumulative projects list is likely accounted for through background ambient growth factors that have been applied to the peak hour volumes at study area intersections as discussed in Section 4.5 *Background Traffic*. Cumulative Only ADT and peak hour intersection turning movement volumes are shown on Exhibit 4-4.

TABLE 4-2: CUMULATIVE DEVELOPMENT LAND USE SUMMARY

No.	Project Name	Land Use[1]	Quantity	Units[2]
County of Riverside				
RC1	PM37227	Single Family Residential	9	DU
RC2	Highland Grove II (TTM No. 38910)	Single Family Residential	128	DU
RC3	TTM38520	Single Family Residential	45	DU
RC4	Greentree (TTM No. 38605)	Single Family Residential	163	DU
City of Riverside				
R1	P13-0165, P13-0166, P13-0167, P13-0168	Gas Sation w/ Car Wash	16.0	AC
R2	P18-0600	Retail/Restaurant w/ Drive Thru	4.494	TSF
R3	PR-2021-001151	Apartment Units & Dental Office	8.139	TSF
R4	P19-0410, P19-0411, P19-0412	Single Family Residential	34	DU
R5	PR-2023-001476	Coffee Shop w/ Drive-Thru (Starbucks)	2.300	TSF
R6	P18-0396, P18-0370	Multi-Family Residential	57	DU
R7	PR-2021-000713	Hospital Tower Expansion	318.433	TSF
R8	PR-2022-001468	Multi-Family Residential	38	DU
R9	P19-0863	Multi-Family Residential	450	DU
		Commercial Retail	9.000	TSF
R10	P14-0176	Single Family Residential	14	DU

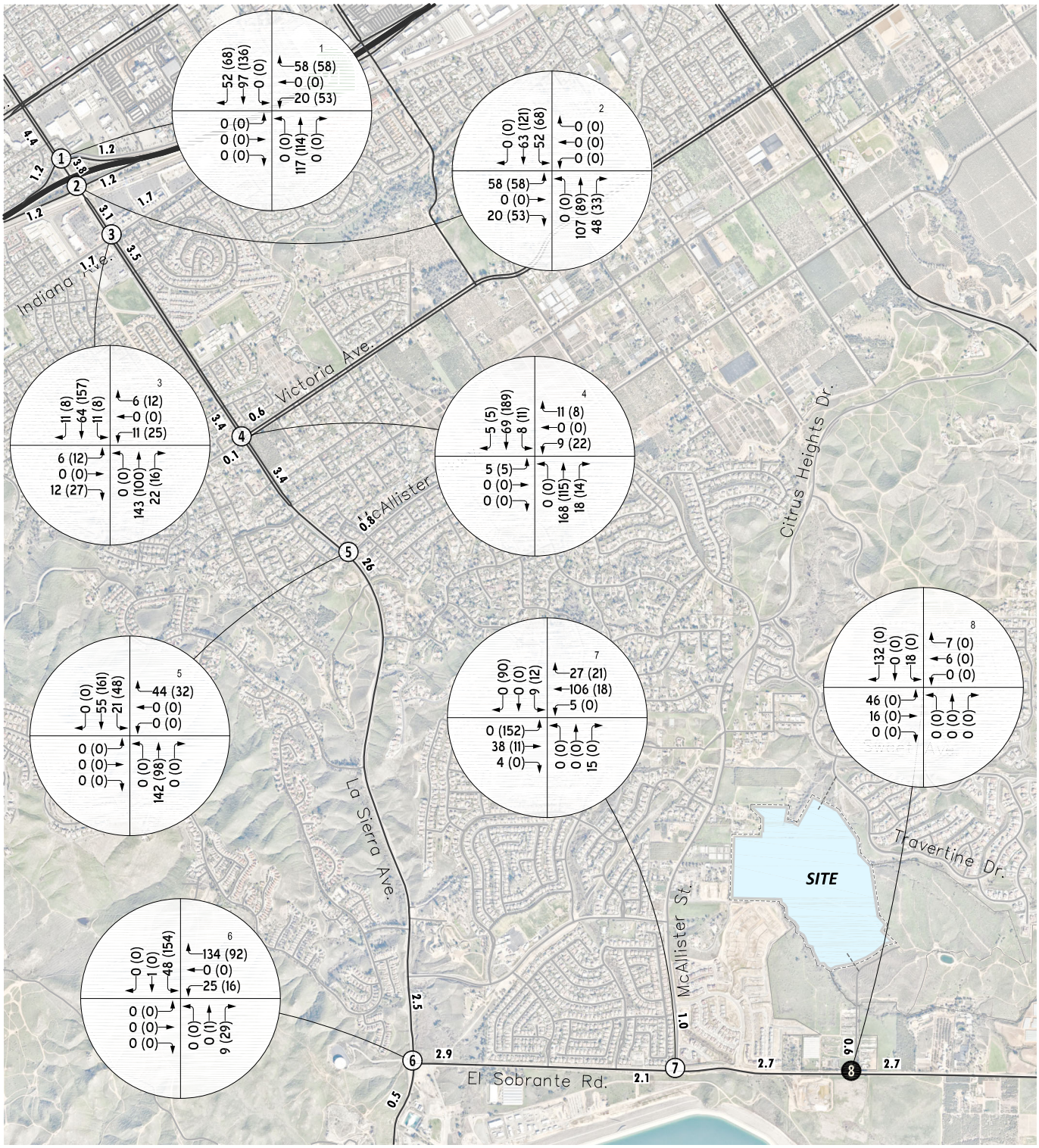
[1] TSF = Thousand Square Feet; DU = Dwelling Units; AC = Acres

EXHIBIT 4-3 : CUMULATIVE DEVELOPMENT LOCATION MAP



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EXHIBIT 4-4 : CUMULATIVE ONLY TRAFFIC VOLUMES



LEGEND

0 = *Existing Intersection Analysis Location*

0 = *Future Intersection Analysis Location*

00 (00) = *Peak Hour Intersection Volume AM (PM)*

##.# = *Average Daily Trips (1000's)*

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5 EAP (2027) TRAFFIC CONDITIONS

This section discusses the traffic forecasts for EAP (2027) conditions and the resulting intersection operations, traffic signal warrant, and queuing analyses.

5.1 ROADWAY IMPROVEMENTS

The lane configurations and traffic controls assumed to be in place for EAP (2027) conditions are consistent with those shown previously on Exhibit 3-1, with the exception of the following:

- Project driveways and those facilities assumed to be constructed by the Project to provide site access are also assumed to be in place for EAP conditions only (e.g., intersection and roadway improvements at the Project's frontage and driveways).

5.2 EAP (2027) TRAFFIC VOLUME FORECASTS

5.2.1 EAP (2027) (PROJECT ONLY) GROWTH TRAFFIC VOLUME FORECASTS

This scenario includes Existing (2024) traffic volumes plus an ambient growth factor of 6.12% and the addition of Project traffic. The weekday ADT volumes and peak hour volumes which can be expected for EAP (2027) (Project only) traffic conditions are shown on Exhibit 5-1.

5.2.2 EAP (2027) (PROJECT PLUS HIGHLAND GROVE II PLUS GREENTREE) GROWTH TRAFFIC VOLUME FORECASTS

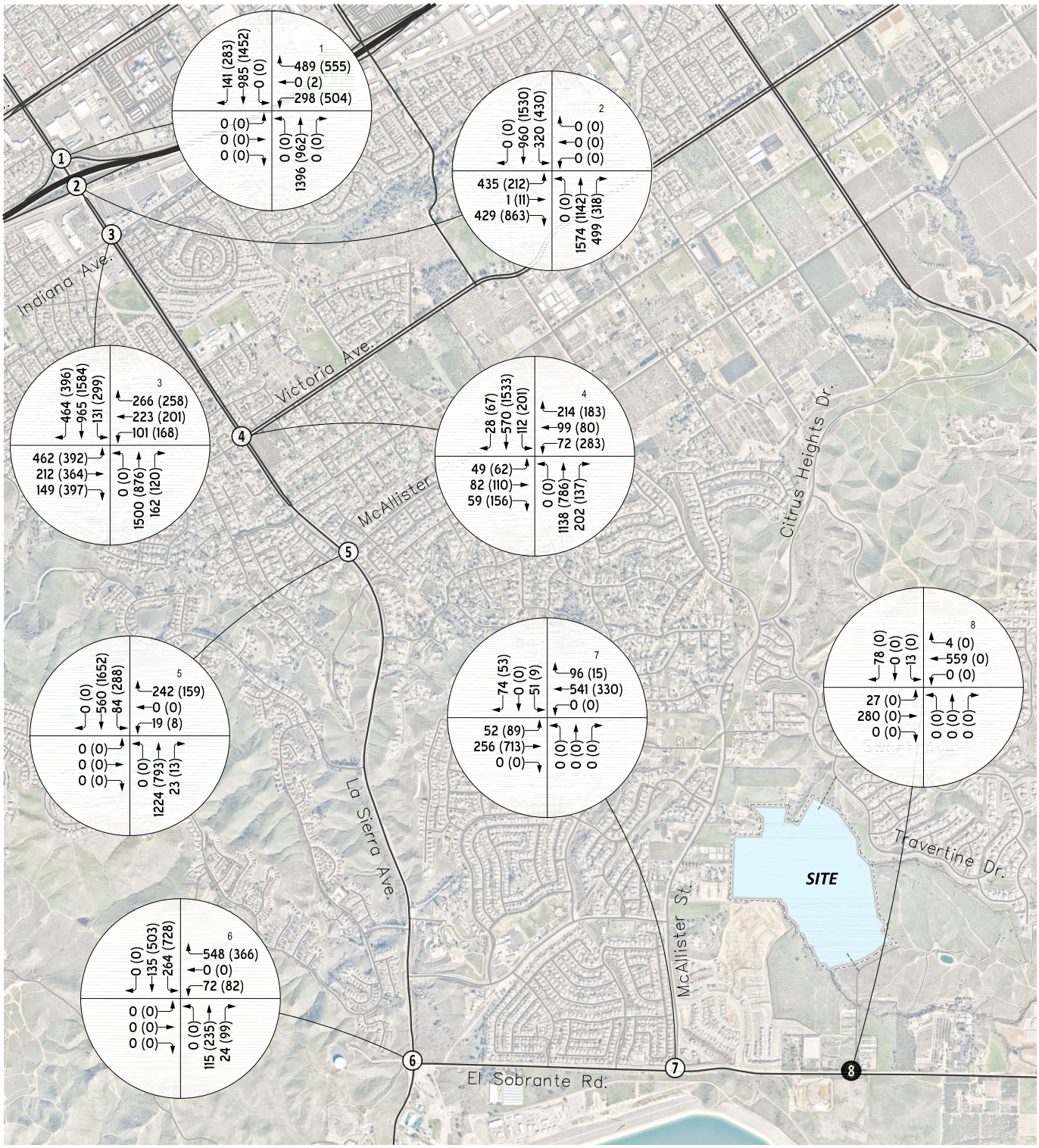
This scenario includes Existing (2024) traffic volumes plus an ambient growth factor of 6.12%, the addition of Project traffic, the addition of traffic from the Highland Grove II and Greentree developments. The weekday ADT volumes and peak hour volumes which can be expected for EAP (2027) (Project plus Highland Grove II plus Greentree) traffic conditions are shown on Exhibit 5-2.

5.3 INTERSECTION OPERATIONS ANALYSIS

EAP (2027) peak hour traffic operations have been evaluated for the study area intersections based on the analysis methodologies presented in Section 2 Methodologies of this TA. The intersection analysis results are summarized in Table 5-1 for EAP traffic conditions, which indicate that all of the study area intersections are anticipated to continue to operate at an acceptable LOS under EAP (2027) (Project only) traffic conditions, with the exception of the following intersections:

- La Sierra Avenue & El Sobrante Road (#6) – LOS F PM peak hour only
- McAllister Street & El Sobrante Road (#7) – LOS F PM peak hour only

EXHIBIT 5-1 : EAP (2027) (PROJECT ONLY) TRAFFIC VOLUMES



LEGEND

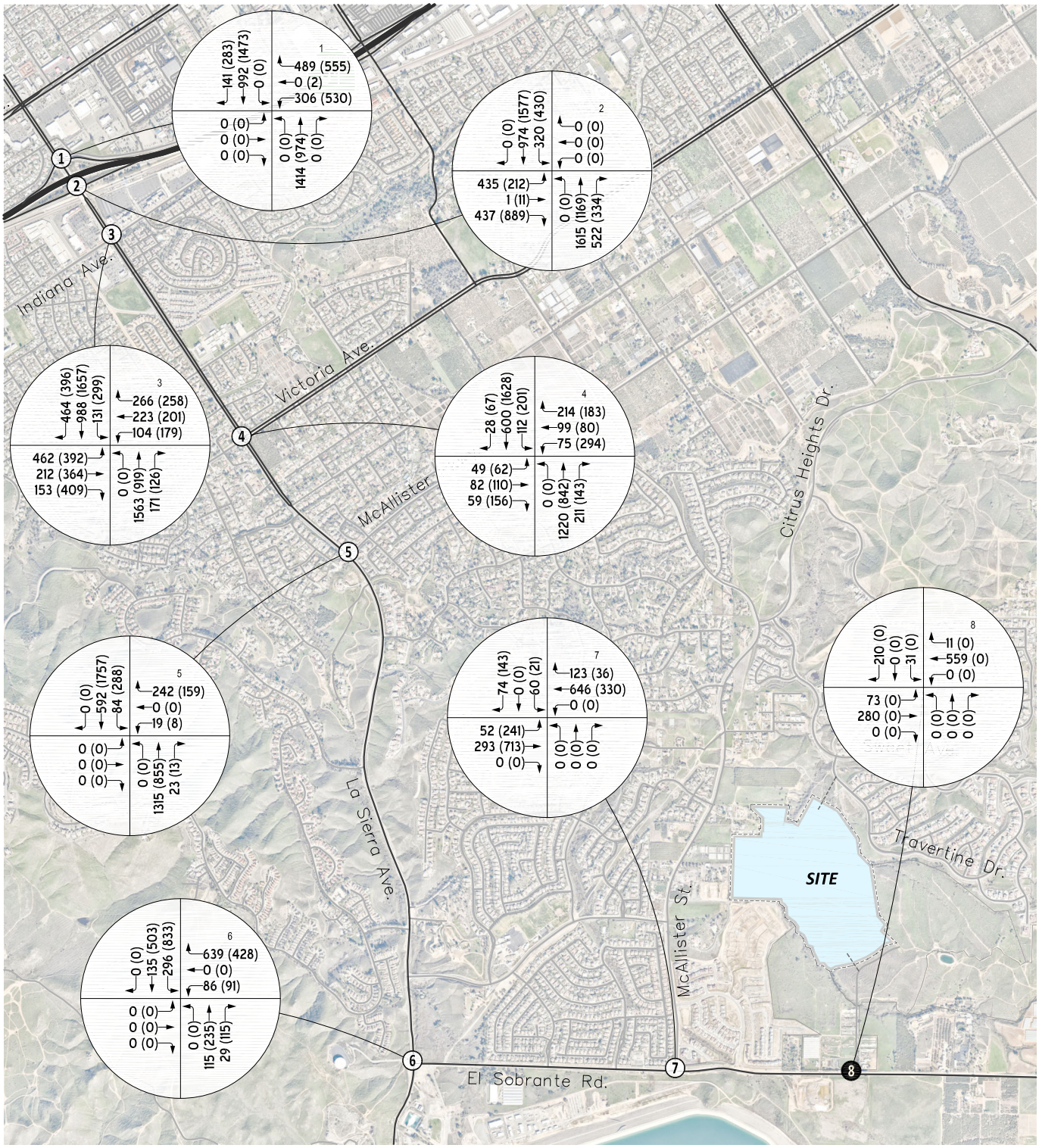
(0) = *Existing Intersection Analysis Location*

(0) = *Future Intersection Analysis Location*

00 (00) = *Peak Hour Intersection Volume AM (PM)*

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EXHIBIT 5-2 : EAP (2027) (PROJECT PLUS HIGHLAND GROVE II PLUS GREENTREE) TRAFFIC VOLUMES



LEGEND

(0) = *Existing Intersection Analysis Location*

0 = *Future Intersection Analysis Location*

00 (00) = *Peak Hour Intersection Volume AM (PM)*

TABLE 5-1: INTERSECTION ANALYSIS FOR EAP (2027) CONDITIONS

#	Intersection	Traffic Control[2]	Existing (2024) Delay[1] (secs.) AM	Existing (2024) Delay[1] (secs.) PM	Existing (2024) Level of Service AM	Existing (2024) Level of Service PM	EAP (2027) Project Only Delay[1] (secs.) AM	EAP (2027) Project Only Delay[1] (secs.) PM	EAP (2027) Project Only Level of Service AM	EAP (2027) Project Only Level of Service PM	EAP (2027) Project plus Highland Grove II plus Greentree Delay[1] (secs.) AM	EAP (2027) Project plus Highland Grove II plus Greentree Delay[1] (secs.) PM	EAP (2027) Project plus Highland Grove II plus Greentree Level of Service AM	EAP (2027) Project plus Highland Grove II plus Greentree Level of Service PM
1	La Sierra Av. & SR-91 WB Ramps	TS	17.9	20.0	B	C	18.4	21.1	B	C	18.6	21.6	B	C
2	La Sierra Av. & SR-91 EB Ramps	TS	22.0	27.8	C	C	22.9	29.4	C	C	23.0	29.8	C	C
3	La Sierra Av. & Indiana Av.	TS	31.8	34.9	C	C	35.6	38.9	D	D	36.5	41.3	D	D
4	La Sierra Av. & Victoria Av.	TS	23.0	36.0	C	D	26.2	36.7	C	D	28.2	39.8	C	D
5	La Sierra Av. & McAllister St.	TS	11.6	10.2	B	B	12.2	11.1	B	B	12.5	11.4	B	B
6	La Sierra Av. & El Sobrante Rd.	AWS	16.3	**75.3**	C	**F**	27.4	**>100.0**	D	**F**	**56.5**	**>100.0**	**F**	**F**
7	McAllister St. & El Sobrante Rd.	AWS	10.7	**39.4**	B	**E**	12.2	**86.6**	B	**F**	14.9	**>100.0**	B	**F**
8	Street A & El Sobrante Rd.	CSS	Future Intersection				14.5	12.3	B	B	22.0	16.0	C	C

* **BOLD** = Level of Service (LOS) does not meet the applicable jurisdictional requirements (i.e., unacceptable LOS).

[1] Per the Highway Capacity Manual (7th Edition), overall average intersection Delay and level of service are shown for intersections with a traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown. HCM delay reported in seconds.

[2] TS = Traffic Signal; CSS = Cross-street Stop; AWS = All-way Stop; TS = Improvement

There are no additional study area intersections anticipated to operate at an unacceptable LOS during the peak hours, with the addition of the Highland Grove II and Greentree development traffic, under EAP (2027) (Project plus Highland Grove II plus Greentree) traffic conditions. However, La Sierra Avenue & El Sobrante Road (#6) is anticipated to operate at an unacceptable LOS (LOS F) during the AM peak hour in addition to the previously identified deficiency during the PM peak hour. The intersection operations analysis worksheets for EAP traffic conditions are included in Appendices 5.1 and 5.2 of this TA.

5.4 TRAFFIC SIGNAL WARRANTS ANALYSIS

The traffic signal warrant analysis for EAP (2027) traffic conditions are based on the planning level ADT volume-based traffic signal warrants. No additional unsignalized study area intersections are anticipated to meet either peak hour volume or ADT volume-based warrants under EAP (2027) (Project only) conditions (see Appendix 5.3).

With the addition of Highland Grove II and Greentree development traffic, the following unsignalized study area intersection is anticipated to meet an ADT volume-based traffic signal warrant under EAP (2027) (Project plus Highland Grove II plus Greentree) conditions (see Appendix 5.4):

- Street A & El Sobrante Road (#6)

5.5 QUEUING ANALYSIS

Queuing analysis findings are presented in Table 5-2. As shown in Table 5-2, the following additional movements are anticipated to experience queuing issues during the weekday AM or weekday PM peak 95th percentile traffic flows under EAP (2027) (Project only) conditions, in addition to the movements identified under Existing (2024) Conditions:

- La Sierra Avenue & Indiana Avenue (#3), northbound through lane – AM peak hour only
- La Sierra Avenue & Indiana Avenue (#3), westbound right turn lane – AM and PM peak hours

The following additional movement is anticipated to experience queuing issues during the weekday AM or weekday PM peak 95th percentile traffic flows with the addition of Highland Grove II and Greentree traffic under EAP (2027) (Project plus Highland Grove II plus Greentree) traffic conditions:

- La Sierra Avenue & Indiana Avenue (#3), northbound left turn lane – PM peak hour

It should be noted that the Simtraffic maximum queues have been reported at the request of the County. However, improvements have been identified to address the 95th percentile queues. Worksheets for EAP (2027) traffic conditions off-ramp queuing analysis are provided in Appendices 5.5 and 5.6.

TABLE 5-2: PEAK HOUR QUEUING SUMMARY FOR EAP (2027) CONDITIONS

			Existing (2024)						EAP (2027) (Project Only)						EAP (2027) (Project plus Highland Grove II plus Greentree)					
			Synchro: 95th Percentile Queue (Feet)		Acceptable? [1]		Simtraffic: Maximum Queue (Feet)		Synchro: 95th Percentile Queue (Feet)		Acceptable? [1]		Simtraffic: Maximum Queue (Feet)		Synchro: 95th Percentile Queue (Feet)		Acceptable? [1]		Simtraffic: Maximum Queue (Feet)	
Intersection	Movement	Available Stacking Distance (Feet)	AM Peak Hour	PM Peak Hour	AM	PM	AM Peak Hour	PM Peak Hour	AM Peak Hour	PM Peak Hour	AM	PM	AM Peak Hour	PM Peak Hour	AM Peak Hour	PM Peak Hour	AM	PM	AM Peak Hour	PM Peak Hour
La Sierra Av. & SR-91 WB Ramps (#1)	NBL	150	**271**	**176**	**No**	**No**	347	224	**292**	**189**	**No**	**No**	384	250	**301**	**196** [2]	**No**	**No**	317	258
	NBT	450	313	251	Yes	Yes	263	237	373	267	Yes	Yes	274	236	379	270	Yes	Yes	296	242
	SBT	300	245	**342**	Yes	**No**	402	667	265	**375**	Yes	**No**	444	784	268	**383**	Yes	**No**	560	805
	SBR	150	49	96	Yes	Yes	138	188	55	109	Yes	Yes	172	190	55	111	Yes	Yes	150	190
	WBL	500	226	300	Yes	Yes	225	267	238	326	Yes	Yes	266	301	244	333	Yes	Yes	280	290
	WBT	1,200	187	235	Yes	Yes	327	364	202	274	Yes	Yes	320	374	201	295	Yes	Yes	349	363
	WBR	500	180	197	Yes	Yes	270	316	189	235	Yes	Yes	272	333	188	244	Yes	Yes	291	303
La Sierra Av. & SR-91 EB Ramps (#2)	NBT	900	359	284	Yes	Yes	435	338	415	311	Yes	Yes	732	380	430	320	Yes	Yes	751	428
	NBR	415	61	62	Yes	Yes	214	137	67	64	Yes	Yes	389	218	69	66	Yes	Yes	468	150
	SBL	150	120	**188** [2]	Yes	**No**	204	227	118	**208** [2]	Yes	**No**	219	212	119	**208** [2]	Yes	**No**	198	224
	SBT	450	29	281	Yes	Yes	198	337	35	314	Yes	Yes	185	286	36	333	Yes	Yes	130	276
	EBL	1,690	252	128	Yes	Yes	372	224	261	134	Yes	Yes	335	288	263	134	Yes	Yes	272	115
	EBT	1,690	218	346	Yes	Yes	489	373	230	392	Yes	Yes	491	413	227	409	Yes	Yes	444	386
	EBR	450	153	290	Yes	Yes	417	359	185	327	Yes	Yes	439	391	190	342	Yes	Yes	402	370
La Sierra Av. & Indiana Av. (#3)	NBL	200	**234** [2]	174 [2]	**No**	Yes	256	202	**261** [2]	192 [2]	**No**	Yes	275	200	**272** [2]	203 [2]	**No**	**No**	275	221
	NBT	590	508 [2]	254	Yes	Yes	363	251	**592** [2]	283	**No**	Yes	565	276	**633** [2]	296	**No**	Yes	529	278
	NBR	315	49	44	Yes	Yes	146	86	55	46	Yes	Yes	280	103	61	40	Yes	Yes	357	109
	SBL	355	111 [2]	207 [2]	Yes	Yes	103	220	121 [2]	223 [2]	Yes	Yes	95	1,396	121 [2]	208 [2]	Yes	Yes	133	402
	SBT	900	312	542 [2]	Yes	Yes	223	393	341	631 [2]	Yes	Yes	270	674	350	649 [2]	Yes	Yes	264	568
	SBR	540	91	72	Yes	Yes	257	195	120	75	Yes	Yes	231	386	120	72	Yes	Yes	255	374
	EBL	205	**294** [2]	**263** [2]	**No**	**No**	260	263	**321** [2]	**286** [2]	**No**	**No**	264	265	**321** [2]	**310** [2]	**No**	**No**	265	265
	EBT	1,010	84	151	Yes	Yes	328	386	88	160	Yes	Yes	545	493	88	163	Yes	Yes	450	871
	EBR	205	41	**236**	Yes	**No**	85	255	42	**278**	Yes	**No**	113	278	43	**307**	Yes	**No**	107	288
	WBL	200	70	101	Yes	Yes	82	114	74	110	Yes	Yes	100	117	76	123 [2]	Yes	Yes	89	140
	WBT	535	101	91	Yes	Yes	191	116	106	96	Yes	Yes	206	152	106	96	Yes	Yes	236	158
	WBR	130	122	116	Yes	Yes	180	136	**137**	**135**	**No**	**No**	187	169	**137**	**138**	**No**	**No**	188	169
La Sierra Av. & Victoria Av. (#4)	NBL	210	73	55	Yes	Yes	170	188	77	58 [2]	Yes	Yes	245	103	77	58	Yes	Yes	274	236
	NBT	1,300	578	397	Yes	Yes	374	338	743 [2]	501 [2]	Yes	Yes	486	365	829 [2]	561 [2]	Yes	Yes	488	469
	SBL	210	187 [2]	**301** [2]	Yes	**No**	163	275	202 [2]	**313** [2]	Yes	**No**	137	275	202 [2]	**283** [2]	Yes	**No**	187	275
	SBT	1,310	202	841 [2]	Yes	Yes	198	526	224	991 [2]	Yes	Yes	224	991	237	1,045 [2]	Yes	Yes	224	1,297
	EBL	130	41	48	Yes	Yes	83	101	43	49	Yes	Yes	80	121	43	50	Yes	Yes	85	108
	EBT	325	98	121	Yes	Yes	128	147	102	127	Yes	Yes	154	191	102	127	Yes	Yes	124	171
	EBR	325	7	30	Yes	Yes	63	135	9	37	Yes	Yes	66	176	9	37	Yes	Yes	64	168
	WBL	200	129 [2]	**412** [2]	Yes	**No**	110	252	144 [2]	**450** [2]	Yes	**No**	132	289	152 [2]	**502** [2]	Yes	**No**	127	305
	WBT	530	111	81	Yes	Yes	138	175	118	86	Yes	Yes	137	483	118	57	Yes	Yes	138	1,279
	WBR	200	89	50	Yes	Yes	157	121	110	52	Yes	Yes	203	204	114	53	Yes	Yes	163	330
La Sierra Av. & McAllister St. (#5)	NBT	1,475	436	306	Yes	Yes	228	206	524	361	Yes	Yes	269	225	590	396	Yes	Yes	307	243
	NBR	200	16	14	Yes	Yes	30	30	18	15	Yes	Yes	35	29	18	16	Yes	Yes	31	31
	SBL	150	114	**280**	Yes	**No**	112	285	119	**305**	Yes	**No**	117	312	119	**305**	Yes	**No**	134	271
	SBT	1,065	107	431	Yes	Yes	117	296	121	522	Yes	Yes	157	249	128	593	Yes	Yes	151	273
	WBL	150	32	19	Yes	Yes	41	29	33	19	Yes	Yes	50	33	33	19	Yes	Yes	86	33
	WBR	400	62	53	Yes	Yes	180	103	83	55	Yes	Yes	184	117	87	55	Yes	Yes	237	86

[1] Stacking Distance is acceptable if the required stacking distance is less than or equal to the stacking distance provided for the 95th percentile queue only. An additional 15 feet of stacking which is assumed to be provided in the transition for turn pockets is reflected in the stacking distance shown on this table, where applicable.
[2] 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

5.6 DEFICIENCIES AND RECOMMENDED IMPROVEMENTS

This section provides a summary of deficiencies and recommended improvements based on the County of Riverside deficiency criteria discussed in Section 2.6 *Deficiency Criteria*.

5.6.1 INTERSECTIONS

Intersection improvements necessary to improve project-related traffic deficiencies are shown in Table 5-3. The improvements have been identified to improve the deficiencies back to acceptable levels. Worksheets for EAP (2027) conditions, with improvements, HCM calculation worksheets are provided in Appendices 5.7 and 5.8.

5.6.1 QUEUES

Improvements to address queuing deficiencies have been identified where feasible and are shown in Table 5-4. Improvements have not been identified at the following movements, as improvements are not considered feasible due to physical constraints on the existing roadways:

- La Sierra Avenue & SR-91 WB Ramps (#1), northbound left turn lane
- La Sierra Avenue & SR-91 WB Ramps (#1), southbound through lane
- La Sierra Avenue & SR-91 RB Ramps (#2), southbound left turn lane
- La Sierra Avenue & Indiana Avenue (#3), northbound through lane
- La Sierra Avenue & Indiana Avenue (#3), eastbound left turn lane
- La Sierra Avenue & Indiana Avenue (#3), eastbound right turn lane

TABLE 5-3: INTERSECTION ANALYSIS FOR EAP (2027) CONDITIONS WITH IMPROVEMENTS

#	Intersection	Traffic Control[3]	Intersection Approach Lanes[1] Northbound L	T	R	Southbound L	T	R	Eastbound L	T	R	Westbound L	T	R	Delay[2] (secs.) AM	PM	Level of Service AM	PM
6	La Sierra Av. & El Sobrante Rd.																	
	Project Only Without Improvements:	AWS	0	2	0	1	1	0	0	0	0	0	1	0	27.4	**>100.0**	D	**F**
	Project plus Highland Grove II plus Greentree Without Improvements:	AWS	0	2	0	1	1	0	0	0	0	0	1	0	**56.5**	**>100.0**	**F**	**F**
	Project Only With Improvements:	**TS**	0	2	0	1	1	0	0	0	0	0	1	0	20.1	31.7	C	C
	Project plus Highland Grove II plus Greentree With Improvements:	**TS**	0	2	0	1	1	0	0	0	0	0	1	0	28.4	45.0	C	D
7	McAllister St. & El Sobrante Rd.																	
	Project Only Without Improvements:	AWS	0	0	0	0	1	0	1	1	0	0	2	0	12.2	**86.6**	B	**F**
	Project plus Highland Grove II plus Greentree Without Improvements:	AWS	0	0	0	0	1	0	1	1	0	0	2	0	14.9	**>100.0**	B	**F**
	Project Only With Improvements:	**TS**	0	0	0	0	1	0	1	1	0	0	2	0	8.7	8.6	A	A
	Project plus Highland Grove II plus Greentree With Improvements:	**TS**	0	0	0	0	1	0	1	1	0	0	2	0	9.4	9.9	A	A

BOLD = LOS does not meet the applicable jurisdictional requirements (i.e., unacceptable LOS).

[1] When a right turn is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; **1** = Improvement

[2] Per the Highway Capacity Manual 6th Edition, overall average intersection delay and level of service are shown for intersections with a traffic signal or all-way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

[3] TS = Traffic Signal; AWS = All-way Stop; **TS** = Improvement

TABLE 5-4: PEAK HOUR QUEUING SUMMARY FOR EAP (2027) CONDITIONS WITH IMPROVEMENTS

Intersection	Movement	Available Stacking Distance (Feet)	Project Only Synchro: 95th Percentile Queue (Feet) AM Peak Hour	PM Peak Hour	Acceptable? [1] AM	PM	Project plus Highland Grove II plus Greentree Synchro: 95th Percentile Queue (Feet) AM Peak Hour	PM Peak Hour	Acceptable? [1] AM	PM
La Sierra Av. & Indiana Av. (#3)	NBL	**325**	261 [2]	192 [2]	Yes	Yes	272 [2]	203 [2]	Yes	Yes
La Sierra Av. & Victoria Av. (#4)	SBL	**325**	202 [2]	313 [2]	Yes	Yes	202 [2]	283 [2]	Yes	Yes
	WBL	**500**	144 [2]	450 [2]	Yes	Yes	152 [2]	**502** [2]	Yes	**No**
La Sierra Av. & McAllister St. (#5)	SBL	**475**	119	305	Yes	Yes	119	305	Yes	Yes

[1] Stacking Distance is acceptable if the required stacking distance is less than or equal to the stacking distance provided for the 95th percentile queue only. An additional 15 feet of stacking which is assumed to be provided in the transition for turn pockets is reflected in the stacking distance shown on this table, where applicable.

[2] 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

[3] **100** = Improvement

6 EAPC (2027) TRAFFIC CONDITIONS

This section discusses the traffic forecasts for EAPC (2027) conditions and the resulting intersection operations, traffic signal warrant, and queuing analyses.

6.1 ROADWAY IMPROVEMENTS

The lane configurations and traffic controls assumed to be in place for EAPC (2027) conditions are consistent with those shown previously on Exhibit 3-1, with the exception of the following:

- Project driveways and those facilities assumed to be constructed by the Project to provide site access are also assumed to be in place for EAPC (2027) conditions only (e.g., intersection and roadway improvements at the Project's frontage and driveways).
- Driveways and those facilities assumed to be constructed by cumulative developments to provide site access are also assumed to be in place for EAPC (2027) conditions only (e.g., intersection and roadway improvements along the cumulative development's frontages).

6.2 EAPC (2027) TRAFFIC VOLUME FORECASTS

This scenario includes Existing (2024) traffic volumes plus an ambient growth factor of 6.12%, the addition of Project traffic, plus the addition of traffic generated by known cumulative development projects. The weekday ADT volumes and peak hour volumes which can be expected for EAPC (2027) traffic conditions are shown on Exhibit 6-1.

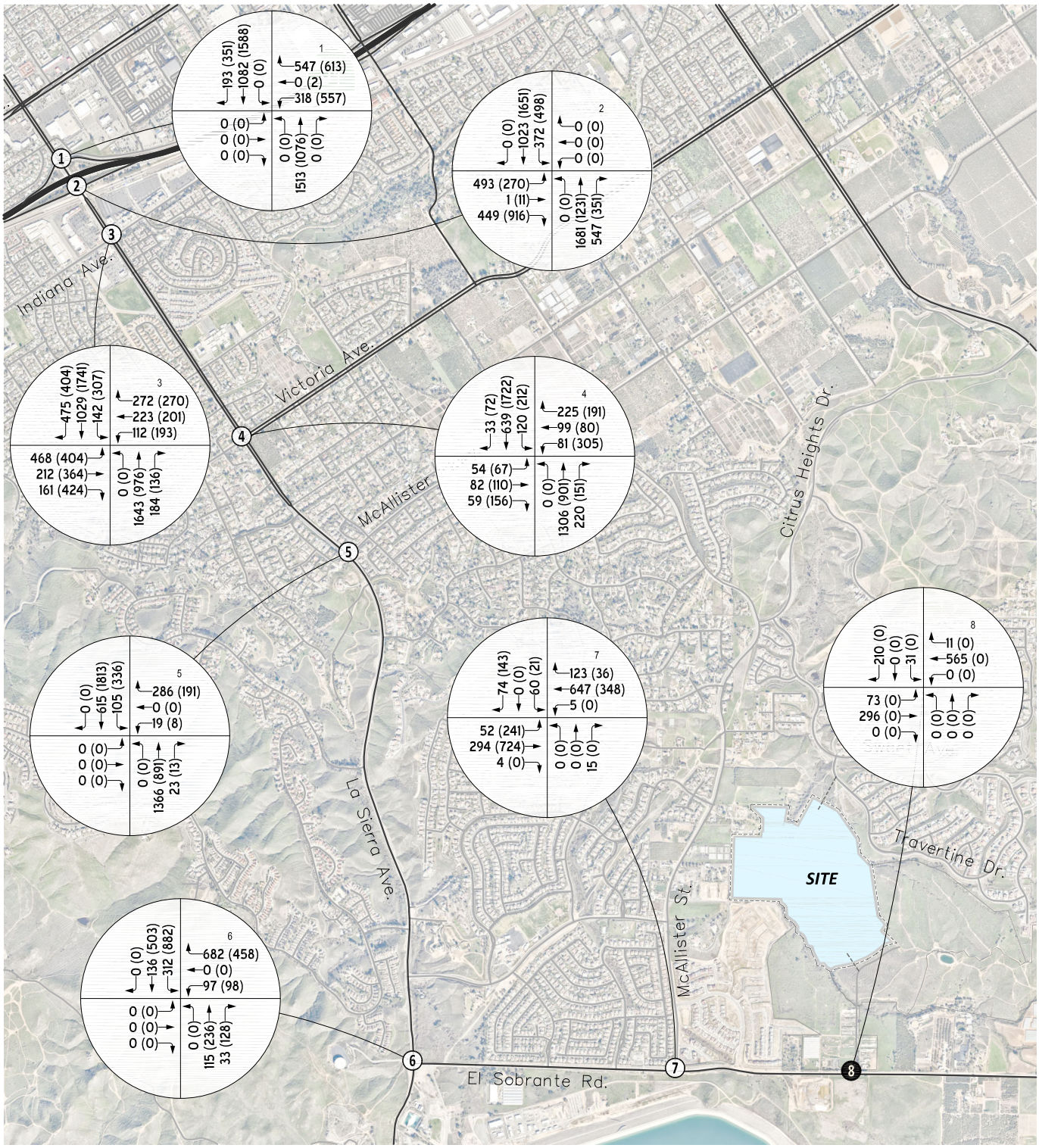
6.3 INTERSECTION OPERATIONS ANALYSIS

EAPC (2027) peak hour traffic operations have been evaluated for the study area intersections based on the analysis methodologies presented in Section 2 *Methodologies* of this TA. The intersection analysis results are summarized in Table 5-1 for EAPC traffic conditions, which indicate that all of the study area intersections are anticipated to continue to operate at an acceptable LOS under EAPC (2027) traffic conditions, with the exception of the following intersections:

- La Sierra Avenue & El Sobrante Road (#6) – LOS F AM and PM peak hours
- McAllister Street & El Sobrante Road (#7) – LOS F PM peak hour only

The intersection operations analysis worksheets for EAPC traffic conditions are included in Appendix 6.1 of this TA.

EXHIBIT 6-1 : EAPC (2027) TRAFFIC VOLUMES



LEGEND

(0) = *Existing Intersection Analysis Location*

(0) = *Future Intersection Analysis Location*

00 (00) = *Peak Hour Intersection Volume AM (PM)*

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TABLE 6-1: INTERSECTION ANALYSIS FOR EAPC (2027) CONDITIONS

#	Intersection	Traffic Control[2]	EAPC (2027) Delay[1] (secs.) AM	PM	Level of Service AM	PM
1	La Sierra Av. & SR-91 WB Ramps	TS	19.0	22.4	B	C
2	La Sierra Av. & SR-91 EB Ramps	TS	25.1	32.7	C	C
3	La Sierra Av. & Indiana Av.	TS	39.1	42.8	D	D
4	La Sierra Av. & Victoria Av.	TS	31.0	43.7	C	D
5	La Sierra Av. & McAllister St.	TS	15.7	13.3	B	B
6	La Sierra Av. & El Sobrante Rd.	AWS	**79.9**	**>100.0**	**F**	**F**
7	McAllister St. & El Sobrante Rd.	AWS	15.0	**>100.0**	C	**F**
8	Street A & El Sobrante Rd.	CSS	22.4	16.4	C	C

* **BOLD** = Level of Service (LOS) does not meet the applicable jurisdictional requirements (i.e., unacceptable LOS).

[1] Per the Highway Capacity Manual (7th Edition), overall average intersection Delay and level of service are shown for intersections with a traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown. HCM delay reported in seconds.

[2] TS = Traffic Signal; CSS = Cross-street Stop; AWS = All-way Stop; TS = Improvement

6.4 TRAFFIC SIGNAL WARRANTS ANALYSIS

All unsignalized study area intersections are anticipated to meet a traffic signal warrant under previous analysis scenarios. As such, traffic signal warrants have not been evaluated EAPC (2027) traffic conditions.

6.5 QUEUING ANALYSIS

Queuing analysis findings are presented in Table 6-2. As shown in Table 6-2, the following movements are anticipated to experience queuing issues during the weekday AM or weekday PM peak 95th percentile traffic flows:

- La Sierra Avenue & SR-91 WB Ramps (#1), northbound left turn lane – AM and PM peak hours
- La Sierra Avenue & SR-91 WB Ramps (#1), southbound through lane – PM peak hour only
- La Sierra Avenue & SR-91 EB Ramps (#2), southbound left turn lane – AM and PM peak hours
- La Sierra Avenue & Indiana Avenue (#3), northbound left turn lane – AM and PM peak hours
- La Sierra Avenue & Indiana Avenue (#3), northbound through lane – AM peak hour only
- La Sierra Avenue & Indiana Avenue (#3), eastbound left turn lane – AM and PM peak hours
- La Sierra Avenue & Indiana Avenue (#3), eastbound right turn lane – PM peak hour only
- La Sierra Avenue & Indiana Avenue (#3), westbound right turn lane – AM and PM peak hours
- La Sierra Avenue & Victoria Avenue (#4), southbound left turn lane – AM and PM peak hours
- La Sierra Avenue & Victoria Avenue (#4), westbound left turn lane – PM peak hour only
- La Sierra Avenue & McAllister Parkway (#5), southbound left turn lane –PM peak hour only

It should be noted that the Simtraffic maximum queues have been reported at the request of the County. However, improvements have been identified to address the 95th percentile queues. Worksheets for EAPC (2027) traffic conditions off-ramp queuing analysis are provided in Appendix 6.2.

TABLE 6-2: PEAK HOUR QUEUING SUMMARY FOR EAPC (2027) CONDITIONS

Intersection	Movement	Available Stacking Distance (Feet)	Synchro: 95th Percentile Queue (Feet)		EAPC (2027) Acceptable? [1]		Simtraffic: Maximum Queue (Feet)	
			AM Peak Hour	PM Peak Hour	AM	PM	AM Peak Hour	PM Peak Hour
La Sierra Av. & SR-91 WB Ramps (#1)	NBL	150	**311**	**185**	**No**	**No**	329	222
	NBT	450	391	294	Yes	Yes	302	237
	SBT	300	296	**427** [2]	Yes	**No**	574	990
	SBR	150	75	148	Yes	Yes	186	190
	WBL	500	253	367	Yes	Yes	297	322
	WBT	1,200	227	325	Yes	Yes	349	407
	WBR	500	216	292	Yes	Yes	298	342
La Sierra Av. & SR-91 EB Ramps (#2)	NBT	900	440	364 [2]	Yes	Yes	762	494
	NBR	415	66	68	Yes	Yes	458	162
	SBL	150	**171** [2]	**244** [2]	**No**	**No**	217	227
	SBT	450	40	390	Yes	Yes	194	413
	EBL	1,690	293	172	Yes	Yes	620	527
	EBT	1,690	275	452 [2]	Yes	Yes	712	723
	EBR	450	222	360	Yes	Yes	540	524
La Sierra Av. & Indiana Av. (#3)	NBL	200	**288** [2]	**214** [2]	**No**	**No**	275	232
	NBT	590	**684** [2]	319	**No**	Yes	799	299
	NBR	315	68	48	Yes	Yes	424	99
	SBL	355	133 [2]	213 [2]	Yes	Yes	97	445
	SBT	900	371 [2]	703 [2]	Yes	Yes	275	887
	SBR	540	134	73	Yes	Yes	258	660
	EBL	205	**326** [2]	**322** [2]	**No**	**No**	260	265
	EBT	1,010	88	164	Yes	Yes	479	1,012
	EBR	205	44	**333**	Yes	**No**	109	355
	WBL	200	81	126 [2]	Yes	Yes	101	136
	WBT	535	106	96	Yes	Yes	229	180
	WBR	130	**144**	**149**	**No**	**No**	190	178
La Sierra Av. & Victoria Av. (#4)	NBL	210	77	58	Yes	Yes	274	233
	NBT	1,300	920 [2]	643 [2]	Yes	Yes	632	507
	SBL	210	**221** [2]	**248**	**No**	**No**	183	275
	SBT	1,310	257	1,051 [2]	Yes	Yes	228	1,343
	EBL	130	46	52	Yes	Yes	94	52
	EBT	325	102	127	Yes	Yes	152	121
	EBR	325	9	37	Yes	Yes	62	151
	WBL	200	167 [2]	**533** [2]	Yes	**No**	165	305
	WBT	530	118	88	Yes	Yes	186	1,542
	WBR	200	131	54	Yes	Yes	178	330
La Sierra Av. & McAllister St. (#5)	NBT	1,475	633	422	Yes	Yes	370	257
	NBR	200	18	16	Yes	Yes	36	31
	SBL	150	144	**356** [2]	Yes	**No**	158	311
	SBT	1,065	133	636	Yes	Yes	145	222
	WBL	150	34	19	Yes	Yes	38	28
	WBR	400	138	60	Yes	Yes	250	123

[1] Stacking Distance is acceptable if the required stacking distance is less than or equal to the stacking distance provided for the 95th percentile queue only. An additional 15 feet of stacking which is assumed to be provided in the transition for turn pockets is reflected in the stacking distance shown on this table, where applicable.

[2] 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

6.6 DEFICIENCIES AND RECOMMENDED IMPROVEMENTS

This section provides a summary of deficiencies and recommended improvements based on the County of Riverside deficiency criteria discussed in Section 2.6 *Deficiency Criteria*.

6.6.1 INTERSECTIONS

Intersection improvements necessary to improve project-related traffic deficiencies are shown in Table 6-3. The improvements have been identified to improve the deficiencies back to acceptable levels. Worksheets for EAPC (2027) conditions, with improvements, HCM calculation worksheets are provided in Appendices 6.3.

TABLE 6-3: INTERSECTION ANALYSIS FOR EAPC (2027) CONDITIONS WITH IMPROVEMENTS

			Intersection Approach Lanes[1]												Delay[2] (secs.)		Level of Service	
		Traffic	Northbound			Southbound			Eastbound			Westbound						
#	Intersection	Control[3]	L	T	R	L	T	R	L	T	R	L	T	R	AM	PM	AM	PM
6	La Sierra Av. & El Sobrante Rd.																	
	- Without Improvements	AWS	0	2	0	1	1	0	0	0	0	0	1	0	**79.9**	**>100.0**	**F**	**F**
	- With Improvements	**TS**	0	2	0	1	1	0	0	0	0	1	0	**1>**	13.7	22.0	B	C
7	McAllister St. & El Sobrante Rd.																	
	- Without Improvements	AWS	0	0	0	0	1	0	1	1	0	0	2	0	15.0	**>100.0**	C	**F**
	- With Improvements	**TS**	0	0	0	0	1	0	1	1	0	0	2	0	8.9	9.9	A	A

BOLD = LOS does not meet the applicable jurisdictional requirements (i.e., unacceptable LOS).

[1] When a right turn is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; > = Right-Turn Overlap Phasing; **1** = Improvement

[2] Per the Highway Capacity Manual (7th Edition), overall average intersection Delay and level of service are shown for intersections with a traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown. HCM delay reported in seconds.

[3] TS = Traffic Signal; AWS = All-way Stop; **TS** = Improvement

6.6.1 QUEUES

Improvements to address queuing deficiencies have been identified where feasible and are shown in Table 3-4. Improvements have not been identified at the following movements, as improvements are not considered feasible due to physical constraints on the existing roadways:

- La Sierra Avenue & SR-91 WB Ramps (#1), northbound left turn lane
- La Sierra Avenue & SR-91 WB Ramps (#1), southbound through lane
- La Sierra Avenue & SR-91 RB Ramps (#2), southbound left turn lane
- La Sierra Avenue & Indiana Avenue (#3), northbound through lane
- La Sierra Avenue & Indiana Avenue (#3), eastbound left turn lane
- La Sierra Avenue & Indiana Avenue (#3), eastbound right turn lane

TABLE 6-4: PEAK HOUR QUEUING SUMMARY FOR EAPC (2027) CONDITIONS WITH IMPROVEMENTS

Intersection	Movement	Available Stacking Distance (Feet)	EAPC (2027) Synchro: 95th Percentile Queue (Feet) AM Peak Hour	EAPC (2027) Synchro: 95th Percentile Queue (Feet) PM Peak Hour	Acceptable? [1] AM	Acceptable? [1] PM
La Sierra Av. & Indiana Av. (#3)	NBL	**325**	288 [2]	214 [2]	Yes	Yes
La Sierra Av. & Victoria Av. (#4)	SBL	**325**	221 [2]	248 [2]	Yes	Yes
	WBL	**500**	167 [2]	**533** [2]	Yes	**No**
La Sierra Av. & McAllister St. (#5)	SBL	**475**	144 [2]	356 [2]	Yes	Yes

[1] Stacking Distance is acceptable if the required stacking distance is less than or equal to the stacking distance provided for the 95th percentile queue only. An additional 15 feet of stacking which is assumed to be provided in the transition for turn pockets is reflected in the stacking distance shown on this table, where applicable.

[2] 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

[3] **100** = Improvement

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7 LOCAL AND REGIONAL FUNDING MECHANISMS

Transportation improvements within the County of Riverside are funded through a combination of improvements constructed by the Project, and development impact fee programs. Fee programs applicable to the Project are described below.

7.1 RIVERSIDE COUNTY TRANSPORTATION UNIFORM MITIGATION FEE (TUMF)

The TUMF program is administered by the Western Riverside Council of Governments (WRCOG) based upon a regional Nexus Study most recently updated in 2016 to address major changes in right of way acquisition and improvement cost factors. (6) This regional program was put into place to ensure that development pays its fair share, and that funding is in place for construction of facilities needed to maintain the requisite level of service and critical to mobility in the region. TUMF is a truly regional mitigation fee program and is imposed and implemented in every jurisdiction in Western Riverside County.

7.2 RIVERSIDE COUNTY DEVELOPMENT IMPACT FEE (DIF) PROGRAM

The Project is located within the County's Lake Mathews/Woodcrest Area Plan and therefore will be subject to County of Riverside DIF in an effort by the County to address development throughout its unincorporated area. The DIF program consists of two separate transportation components: the Roads, Bridges and Major Improvements component and the Traffic Signals component. Eligible facilities for funding by the County DIF program are identified on the County's Public Needs List, which currently extends through the year 2020. (7) A comprehensive review of the DIF program is now planned in order to update the nexus study. This will result in development of a revised "needs list" extending the program time horizon from 2010 to 2027.

The cost of signalizing DIF network intersections is identified under the Traffic Signals component of the DIF program. County staff generally defines DIF eligible intersections as those consisting of two intersecting general plan roadways. If the intersection meets this requirement, it is potentially eligible for up to $235,000 of credit, which is subject to negotiations with the County.

7.3 MEASURE A

Measure A, Riverside County's half-cent sales tax for transportation, was adopted by voters in 1988 and extended in 2002. It will continue to fund transportation improvements through 2038. Measure A funds a wide variety of transportation projects and services throughout the County. RCTC is responsible for administering the program. Measure A dollars are spent in accordance with a voter-approved expenditure plan that was adopted as part of the 1988 election.

7.4 FAIR SHARE CONTRIBUTION

Project improvements may include a combination of fee payments to established programs, construction of specific improvements, payment of a fair share contribution toward future improvements, or a combination of these approaches. Improvements constructed by development may be eligible for a fee credit or reimbursement through the program where appropriate. When off-site improvements are identified with a minor share of responsibility assigned to proposed development, the approving jurisdiction may elect to collect a fair share contribution or require the development to construct improvements. Detailed fair share calculations, for each peak hour, have been provided in Table 7-1 for the applicable deficient study area intersections. These fees are collected with the proceeds solely used as part of a funding mechanism aimed at ensuring that regional highways and arterial expansions keep pace with the projected population increases.

TABLE 7-1: PROJECT FAIR SHARE CALCULATIONS

#	Intersection		Existing (2024)	Project	EAPC (2027)	Total New Traffic	Project % of New Traffic[1]
6	La Sierra Av. & El Sobrante Rd.						
		AM:	995	101	1,374	379	**26.6%**
		PM:	1,769	135	2,304	535	25.2%
7	McAllister St. & El Sobrante Rd.						
		AM:	910	105	1,286	376	27.9%
		PM:	1,148	141	1,651	503	**28.0%**

[1] **BOLD =** Highest fair share percentage is highlighted.

8 REFERENCES

1. **County of Riverside Transportation Department.** *Transportation Analysis Guidelines for Level of Service and Vehicle Miles Traveled.* County of Riverside : s.n., December 2020.
2. **VRPA Technologies, Inc. for Riverside County Transportation Commission.** *Riverside County Long Range Transportation Study.* County of Riverside : VRPA Technologies, Inc., December 2019.
3. **Transportation Research Board.** *Highway Capacity Manual (HCM).* 7th Edition. s.l. : National Academy of Sciences, 2022.
4. **California Department of Transportation.** California Manual on Uniform Traffic Control Devices (CA MUTCD). [book auth.] California Department of Transportation. *California Manual on Uniform Traffic Control Devices (CA MUTCD).* 2014, Updated March 30, 2021 (Revision 6).
5. **Institute of Transportation Engineers.** *Trip Generation Manual.* 11th Edition. 2021.
6. **Western Riverside Council of Governments.** *TUMF Nexus Study, 2016 Program Update.* July 2017.
7. **Willdan Financial Services.** *County of Riverside Development Impact Fee Study Update.* County of Riverside : s.n., 2013.

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APPENDIX 1.1: APPROVED TRAFFIC STUDY SCOPING AGREEMENT

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MEMORANDUM

URBAN | CROSSROADS

DATE: April 19, 2024
TO: Kevin Tsang, County of Riverside
FROM: Charlene So, Urban Crossroads, Inc.
JOB NO: 15933-02 TA Scope

HIGHLAND GROVE VISTAS 4 TTM NO. 38927 TRAFFIC STUDY SCOPING AGREEMENT

Urban Crossroads, Inc. is pleased to provide the following Traffic Study Scoping Agreement for the Highland Grove Vistas 4 TTM No. 38927 development (**Project**), which is located immediately north of El Sobrante Road, east of McCallister Parkway, and south of Travertine Drive in the County of Riverside. This letter describes the proposed Project trip generation, trip distribution, and analysis methodology, which have been used to establish the draft proposed Project study area and analysis locations. The proposed analysis methodology and assumptions are in accordance with the County's Transportation Analysis Guidelines for Level of Service Vehicle Miles Traveled (dated December 2020) (**County Guidelines**). The County scoping form is included in Attachment A.

PROJECT DESCRIPTION

The Project is anticipated to have an Opening Year of 2027. The Project includes the development of 206 single family detached residential dwelling units. A preliminary site plan for the proposed Project is shown on Exhibit 1. Primary access to the Project site will be accommodated via a new connection to El Sobrante Road (via Street A) and a new connection onto Travertine Drive. Street A on El Sobrante Road and on Travertine Drive are assumed to allow for full access (no left turn restrictions).

EXHIBIT 1: PRELIMINARY SITE PLAN



TRIP GENERATION

Trip generation represents the amount of traffic that is attracted and produced by a development and is based upon the specific land uses planned for a given project. In order to develop the traffic characteristics of the proposed project, trip-generation statistics published in the Institute of Transportation Engineers (ITE) Trip Generation Manual (11th Edition, 2021) were used to estimate the Project's trip generation based on the Single Family Detached Housing (ITE Land Use Code 210) land use category. Trip generation rates are summarized on Table 1.

The trip generation summary illustrating daily, and peak hour trip generation estimates for the proposed Project are also shown on Table 1. The proposed Project is anticipated to generate 1,944 two-way trip-ends per day with 144 AM peak hour trips and 194 PM peak hour trips (see Table 1).

TABLE 1: PROJECT TRIP GENERATION SUMMARY

Land Use[1]	Units[2]	ITE LU Code	AM Peak Hour In	AM Peak Hour Out	AM Peak Hour Total	PM Peak Hour In	PM Peak Hour Out	PM Peak Hour Total	Daily
Single Family Detached Residential	DU	210	0.18	0.52	0.70	0.59	0.35	0.94	9.43

[1] Trip Generation Source: Institute of Transportation Engineers (ITE), Trip Generation Manual, Eleventh Edition (2021).

[2] DU = Dwelling Units

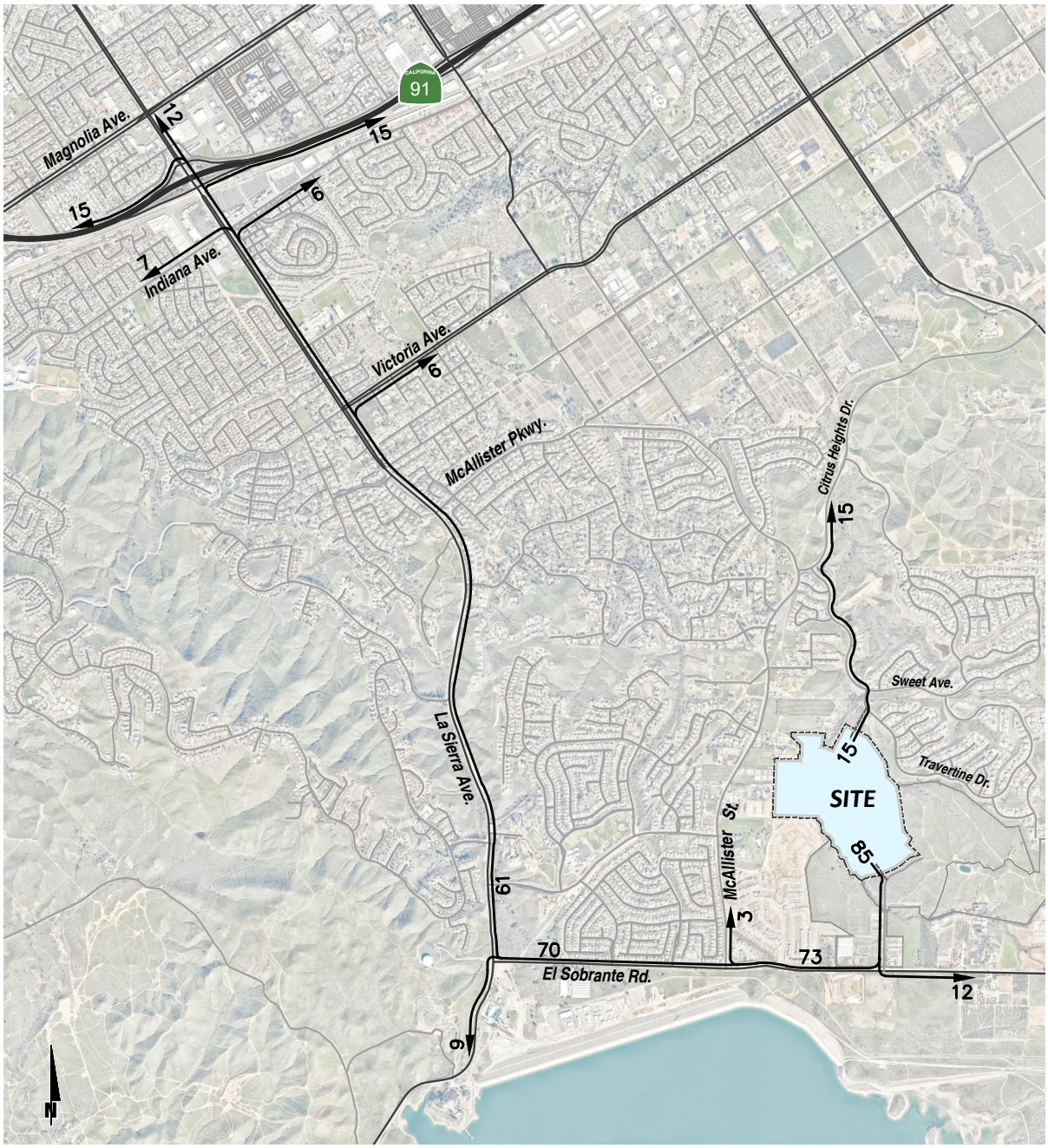
Land Use	Quantity	Units[1]	AM Peak Hour In	AM Peak Hour Out	AM Peak Hour Total	PM Peak Hour In	PM Peak Hour Out	PM Peak Hour Total	Daily
Highland Grove Vistas 4 (TTM No. 38927)	206	DU	37	107	144	122	72	194	1,944

[1] DU = Dwelling Units

TRIP DISTRIBUTION

The Project trip distribution represents the directional orientation of traffic to and from the Project site. Trip distribution is the process of identifying the probable destinations, directions or traffic routes that will be utilized by Project traffic. The potential interaction between the planned land uses and surrounding regional access routes are considered, to identify the route where the Project traffic would distribute. Exhibit 2 shows the Project trip distribution patterns. Project trip distribution patterns are based on traffic counts from 2023 for the applicable study area intersections (see Attachment B).

EXHIBIT 2: PROJECT TRIP DISTRIBUTION



LEGEND:

10 = *Percent To/From Project*

→ = *Trip Distribution*

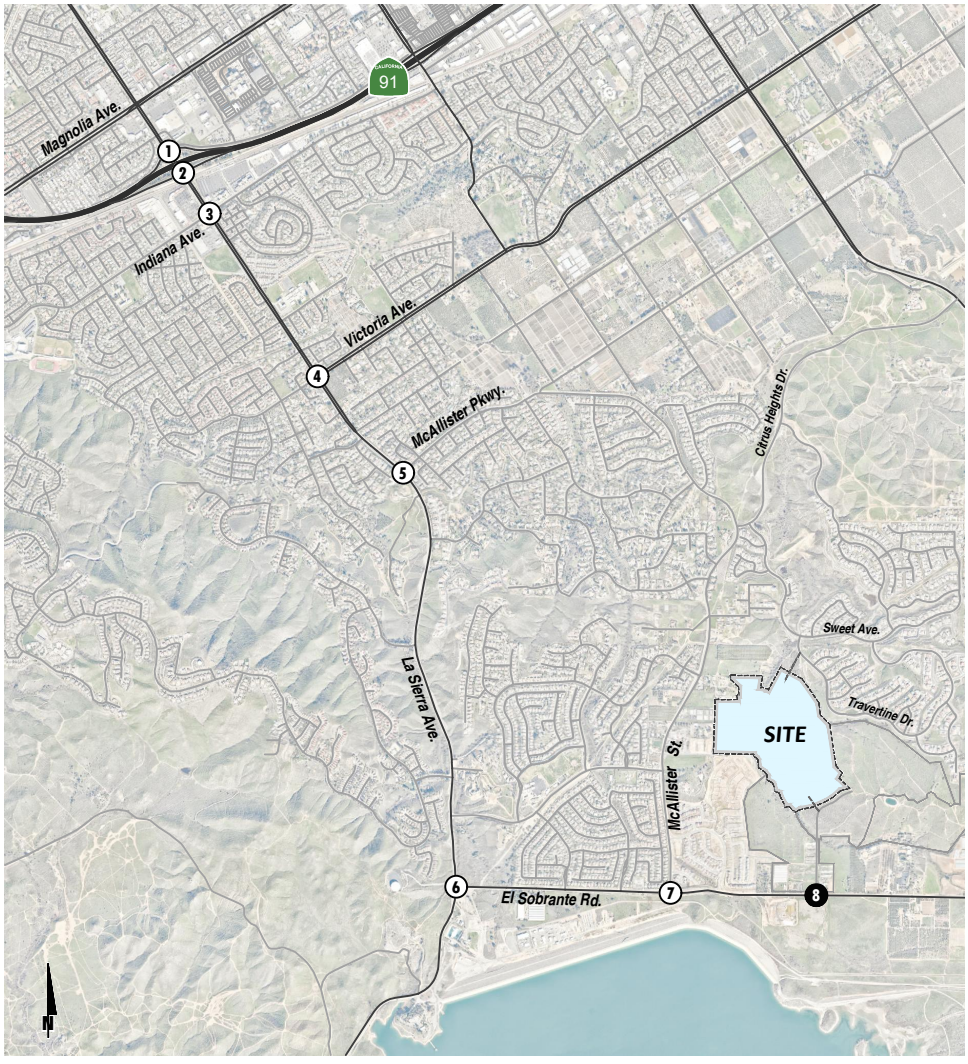
ANALYSIS SCENARIOS

Consistent with the County's Guidelines, intersection analysis will be provided for the following analysis scenarios:

- Existing (2024) Conditions
- Existing plus Ambient Growth plus Project (EAP) (2027) Conditions
- EAP (2030) Conditions – Project plus Highland Grove II plus Greentree
- Existing plus Ambient Growth plus Project plus Cumulative (EAPC) (2027) Conditions

All study area intersections will be evaluated using the Highway Capacity Manual (HCM) 7th Edition analysis methodology. The study area that is proposed to be evaluated is shown on Exhibit 3.

EXHIBIT 3: STUDY AREA



LEGEND:

(0) = *Existing Intersection Analysis Location*

0 = *Future Intersection Analysis Location*

TRAFFIC COUNTS

New traffic counts (classified by vehicle type) will be conducted during a typical weekday when local schools are in session and operating on a typical bell schedule. No adjustments are proposed to the traffic counts for the baseline traffic condition with the exception of volume balancing between closely spaced intersections. Traffic counts will be conducted once the scoping agreement has been approved.

AMBIENT GROWTH

An ambient growth rate of 2% per year is proposed for the study area intersection to approximate background growth not identified by nearby cumulative development projects. As such, a total of 6.12% will be applied to the baseline (2% per year, compounded over 3 years).

CUMULATIVE PROJECTS

It is requested that the County of Riverside provide current cumulative projects within the study area for inclusion in the Traffic Analysis. The City of Riverside will also be contacted to obtain a current list of cumulative projects for their agency. A preliminary list of cumulative projects is provided on Table 2 and locations are graphically depicted on Exhibit 4.

TABLE 2: CUMULATIVE DEVELOPMENT LAND USE SUMMARY

No.	Project Name	Land Use[1]	Quantity	Units[2]
County of Riverside				
RC1	PM37227	Single Family Residential	9	DU
RC2	Highland Grove II (TTM No. 38910)	Single Family Residential	128	DU
RC3	TTM38520	Single Family Residential	45	DU
RC4	Greentree (TTM No. 38605)	Single Family Residential	163	DU
City of Riverside				
R1	P13-0165, P13-0166, P13-0167, P13-0168	Gas Sation w/ Car Wash	16.0	AC
R2	P18-0600	Retail/Restaurant w/ Drive Thru	4.494	TSF
R3	PR-2021-001151	Apartment Units & Dental Office	8.139	TSF
R4	P19-0410, P19-0411, P19-0412	Single Family Residential	34	DU
R5	PR-2023-001476	Coffee Shop w/ Drive-Thru (Starbucks)	2.300	TSF
R6	P18-0396, P18-0370	Multi-Family Residential	57	DU
R7	PR-2021-000713	Hospital Tower Expansion	318.433	TSF
R8	PR-2022-001468	Multi-Family Residential	38	DU
R9	P19-0863	Multi-Family Residential	450	DU
		Commercial Retail	9.000	TSF
R10	P14-0176	Single Family Residential	14	DU

[1] TSF = Thousand Square Feet; DU = Dwelling Units; AC = Acres

EXHIBIT 4: CUMULATIVE DEVELOPMENT LOCATION MAP



SPECIAL ISSUES

The following special issues will also be addressed:

- VMT analysis will be evaluated in a separate document.
- Conduct traffic signal warrant analysis for all existing and future unsignalized study area intersections, which also includes Street A at El Sobrante Road. If a traffic signal is warranted under existing traffic conditions, 24-hour approach counts will be collected to conduct 8-hour and 4-hour warrants.
- Provide a queuing analysis for the Project driveway (Street A at El Sobrante). This will be used to determine the appropriate turn pocket lengths needed to accommodate peak hour queues.
- Queuing analysis will also be conducted at all signalized study area intersections along La Sierra Avenue. Report through movement queues in addition to the turn-lane queues. Include a summary table for the queues for each applicable analysis scenario. Tables should report the available storage, 95th percentile queue from Synchro and Max Queues from SimTraffic.
- Evaluate the peak hour off-ramp queuing at each of the applicable study area off-ramp intersections on La Sierra Avenue to ensure no spillback onto the SR-91 Freeway mainline.

If you have any questions or comments, I can be reached at cso@urbanxroads.com.

ATTACHMENT A: COUNTY SCOPING FORM

EXHIBIT B

SCOPING AGREEMENT FOR TRAFFIC IMPACT STUDY

This letter acknowledges the Riverside County Transportation Department requirements for traffic impact analysis of the following project. The analysis must follow the Riverside County Transportation Department Traffic Study Guidelines dated December 2020.

Case No.	TTM No. 38927
Related Cases-	
SP No.	
EIR No.	
GPA No.	
CZ No.	
Project Name:	Highland Grove Vistas 4 (also known as Willow Creek)
Project Address:	north of Highland Grove II on Street A, north El Sobrante Road
Project Description:	206 single family detached residential dwelling units

	Consultant	Developer Representative
Name:	Urban Crossroads Inc. - Charlene So	Adkan Engineers - Mitch Adkison
Address:	1133 Camelback St. #8329	6879 Airport Drive
	Newport Beach, CA 92658	Riverside, CA 92504
Telephone:	(949) 861-0177	951-688-0241
email:	cso@urbanxroads.com	madkison@adkan.com

A. Trip Generation Source: ITE Trip Generation Manual (11th Edition, 2021)

Current GP Land Use	RC-MDR & RC-LDR	Proposed Land Use	RC-MDR
Current Zoning	A-1-10 & A-P	Proposed Zoning	R-1-10,000

	Current Trip Generation			Proposed Trip Generation		
	In	Out	Total	In	Out	Total
AM Trips				**37**	**107**	**144**
PM Trips				**122**	**72**	**194**

Internal Trip Allowance ☐ Yes ■ No (0 % Trip Discount)
Pass-By Trip Allowance ☐ Yes ■ No (0 % Trip Discount)

A passby trip discount of 25% is allowed for appropriate land uses. The passby trips at adjacent study area intersections and project driveways shall be indicated on a report figure.

B. Trip Geographic Distribution:

N **30** % S **9** % E **39** % W **22** %

C. Background Traffic

Project Build-out Year: 2027 Annual Ambient Growth Rate: 2 %
Phase Year(s) N/A

Other area Projects to be analyzed: To be provided by the County
Model/Forecast Methodology: Not Applicable

URBAN CROSSROADS

D. Study Intersections: *(NOTE: Subject to revision after other projects, trip generation and distribution are determined, or comments form other agencies).* (See Exhibit 2)

1. La Sierra Av. & SR-91 WB Ramps
2. La Sierra Av. & SR-91 EB Ramps
3. La Sierra Av. & Indiana Av.
4. La Sierra Av. & Victoria Av.
5. La Sierra Av. & McAllister Pkwy.
6. La Sierra Av. & El Sobrante Rd.
7. McAllister Pkwy. & El Sobrante Rd.
8. Street A & El Sobrante Rd. - Future Itnersection
9.
10.
11.
12.
13.
14.
15.
16.
17.
18.
19.
20.

E. Study Roadway Segments: *(NOTE: Subject to revision after other projects, trip generation and distribution are determined, or comments form other agencies).*

1. 2.

F. Other Jurisdictional Impacts

Is this project within a City's Sphere of influence or one mile radius of City boundaries? ☐ Yes ■ No

If so, name of City jurisdiction:

G. Site Plan (please attach reduced copy)

H. Specific issues to be addressed in the Study (in addition to the standard analysis described in the Guideline) (To be filled out by Transportation Department)
(NOTE: If the traffic study states that "a traffic signal is warranted" (or "a traffic signal appears to be warranted", or similar statement) at an existing unsignalized intersection under existing conditions, 8-hour approach traffic volume information must be submitted in addition to the peak hourly turning movement counts for that intersection.

I. Existing Conditions
Traffic count data must be new or recent. Provide traffic count dates if using other than new counts.
Date of counts: New counts to be conducted once scoping agreement is approved

***NOTE* Traffic Study Submittal Form and appropriate fee must be submitted with, or prior to submittal of this form. Transportation Department staff will not process the Scoping Agreement prior to receipt of the fee.**

Recommended by:

Charlene So — 3/19/2024

Consultant's Representative — Date

Scoping Agreement Revised on 4/19/2024

Approved Scoping Agreement:

Eva Cavarrubias — 4/22/2024

Riverside County Transportation Department — Date

URBAN CROSSROADS

ATTACHMENT B: 2023 TRAFFIC COUNTS

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
(951) 268-6268

City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W AM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

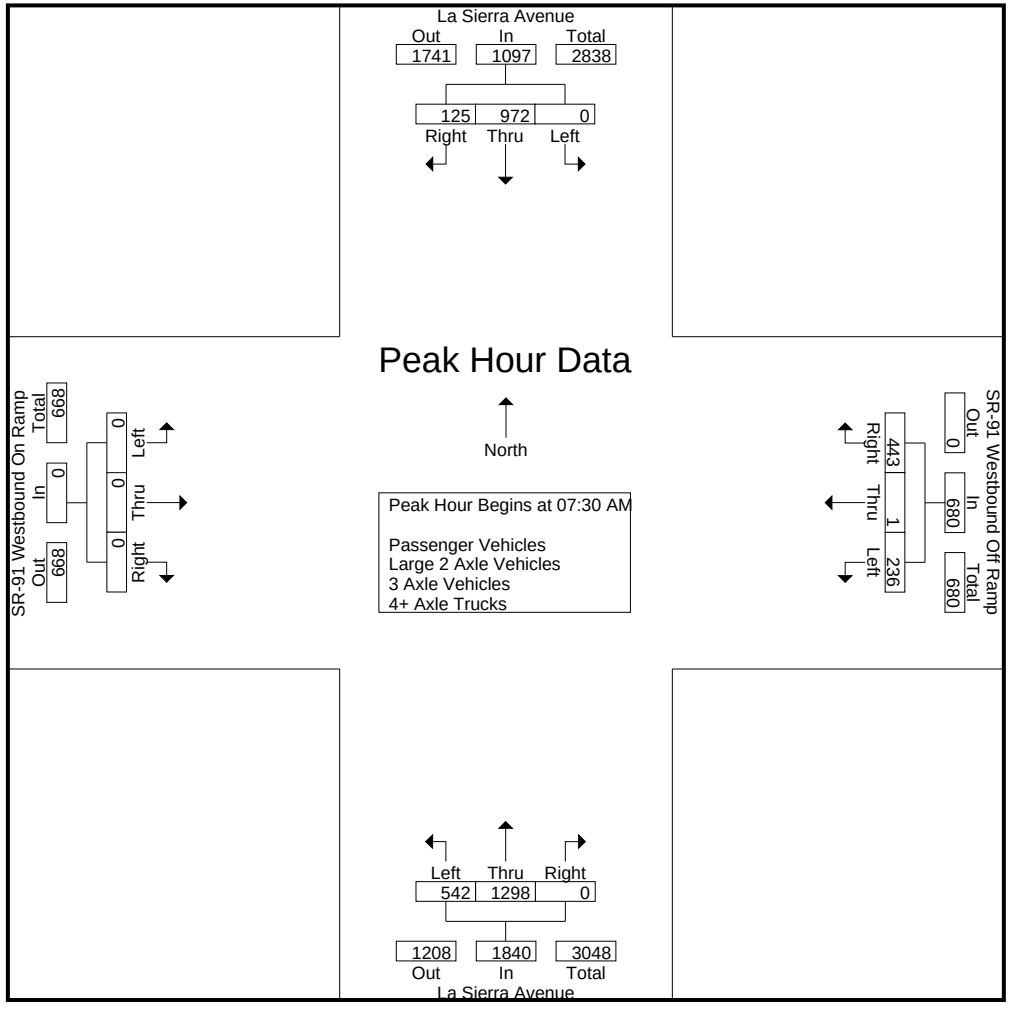
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	172	41	3	213	45	0	63	42	108	139	169	0	0	308	0	0	0	0	0	45	629	674
07:15 AM	0	178	29	7	207	59	0	109	56	168	156	232	0	0	388	0	0	0	0	0	63	763	826
07:30 AM	0	251	36	8	287	57	0	94	64	151	118	218	0	0	336	0	0	0	0	0	72	774	846
07:45 AM	0	289	26	6	315	66	0	104	46	170	147	324	0	0	471	0	0	0	0	0	52	956	1008
Total	0	890	132	24	1022	227	0	370	208	597	560	943	0	0	1503	0	0	0	0	0	232	3122	3354
08:00 AM	0	231	28	11	259	65	0	126	25	191	143	415	0	0	558	0	0	0	0	0	36	1008	1044
08:15 AM	0	201	35	14	236	48	1	119	53	168	134	341	0	0	475	0	0	0	0	0	67	879	946
08:30 AM	0	168	24	15	192	40	2	112	58	154	135	258	0	0	393	0	0	0	0	0	73	739	812
08:45 AM	0	148	32	18	180	32	0	84	42	116	135	237	0	0	372	0	0	0	0	0	60	668	728
Total	0	748	119	58	867	185	3	441	178	629	547	1251	0	0	1798	0	0	0	0	0	236	3294	3530
Grand Total	0	1638	251	82	1889	412	3	811	386	1226	1107	2194	0	0	3301	0	0	0	0	0	468	6416	6884
Apprch %	0	86.7	13.3			33.6	0.2	66.2			33.5	66.5	0			0	0	0					
Total %	0	25.5	3.9		29.4	6.4	0	12.6		19.1	17.3	34.2	0		51.4	0	0	0		0	6.8	93.2	
Passenger Vehicles	0	1600	246		1928	401	2	798		1581	1076	2143	0		3219	0	0	0		0	0	0	6728
% Passenger Vehicles	0	97.7	98	100	97.8	97.3	66.7	98.4	98.4	98.1	97.2	97.7	0	0	97.5	0	0	0	0	0	0	0	97.7
Large 2 Axle Vehicles	0	23	4		27	10	1	9		25	20	40	0		60	0	0	0		0	0	0	112
% Large 2 Axle Vehicles	0	1.4	1.6	0	1.4	2.4	33.3	1.1	1.3	1.6	1.8	1.8	0	0	1.8	0	0	0	0	0	0	0	1.6
3 Axle Vehicles	0	9	1		10	0	0	3		4	5	5	0		10	0	0	0		0	0	0	24
% 3 Axle Vehicles	0	0.5	0.4	0	0.5	0	0	0.4	0.3	0.2	0.5	0.2	0	0	0.3	0	0	0	0	0	0	0	0.3
4+ Axle Trucks	0	6	0		6	1	0	1		2	6	6	0		12	0	0	0		0	0	0	20
% 4+ Axle Trucks	0	0.4	0	0	0.3	0.2	0	0.1	0	0.1	0.5	0.3	0	0	0.4	0	0	0	0	0	0	0	0.3

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	0	251	**36**	287	57	0	94	151	118	218	0	336	0	0	0	0	774
07:45 AM	0	**289**	26	**315**	**66**	0	104	170	**147**	324	0	471	0	0	0	0	956
08:00 AM	0	231	28	259	65	0	**126**	**191**	143	**415**	0	**558**	0	0	0	0	**1008**
08:15 AM	0	201	35	236	48	**1**	119	168	134	341	0	475	0	0	0	0	879
Total Volume	0	972	125	1097	236	1	443	680	542	1298	0	1840	0	0	0	0	3617
% App. Total	0	88.6	11.4		34.7	0.1	65.1		29.5	70.5	0		0	0	0		
PHF	.000	.841	.868	.871	.894	.250	.879	.890	.922	.782	.000	.824	.000	.000	.000	.000	.897

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
(951) 268-6268

City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W AM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 2



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Site Code : 05123396
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Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:45 AM				07:45 AM				07:00 AM				
+0 mins.	0	251	**36**	287	**66**	0	104	170	**147**	324	0	471	0	0	0	0	
+15 mins.	0	**289**	26	**315**	65	0	**126**	**191**	143	**415**	0	**558**	0	0	0	0	
+30 mins.	0	231	28	259	48	1	119	168	134	341	0	475	0	0	0	0	
+45 mins.	0	201	35	236	40	**2**	112	154	135	258	0	393	0	0	0	0	
Total Volume	0	972	125	1097	219	3	461	683	559	1338	0	1897	0	0	0	0	
% App. Total	0	88.6	11.4		32.1	0.4	67.5		29.5	70.5	0		0	0	0		
PHF	.000	.841	.868	.871	.830	.375	.915	.894	.951	.806	.000	.850	.000	.000	.000	.000	

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Groups Printed- Passenger Vehicles

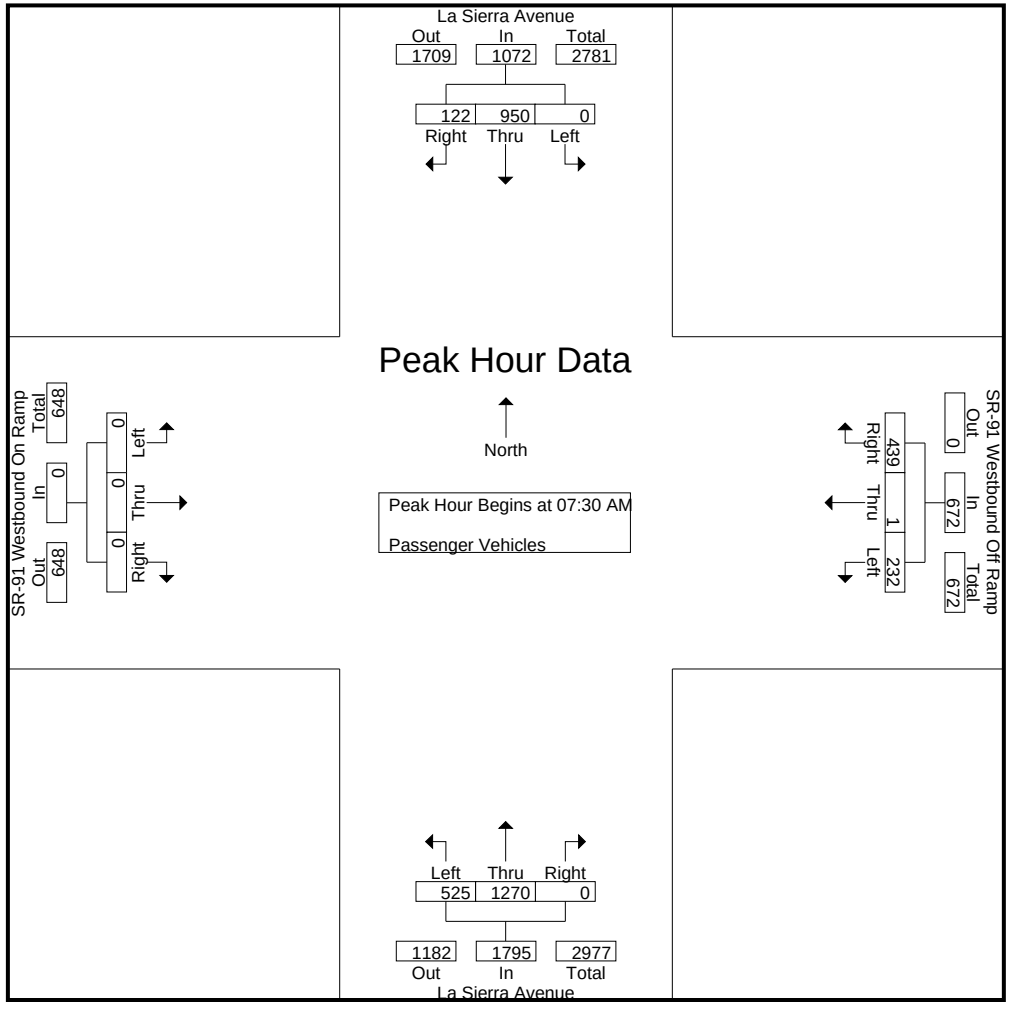
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Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	167	40	3	207	41	0	62	41	103	134	163	0	0	297	0	0	0	0	0	44	607	651
07:15 AM	0	172	29	7	201	58	0	105	54	163	154	227	0	0	381	0	0	0	0	0	61	745	806
07:30 AM	0	246	36	8	282	56	0	93	64	149	114	211	0	0	325	0	0	0	0	0	72	756	828
07:45 AM	0	283	26	6	309	65	0	103	46	168	143	316	0	0	459	0	0	0	0	0	52	936	988
Total	0	868	131	24	999	220	0	363	205	583	545	917	0	0	1462	0	0	0	0	0	229	3044	3273
08:00 AM	0	224	27	11	251	65	0	125	25	190	142	407	0	0	549	0	0	0	0	0	36	990	1026
08:15 AM	0	197	33	14	230	46	1	118	52	165	126	336	0	0	462	0	0	0	0	0	66	857	923
08:30 AM	0	166	23	15	189	39	1	110	56	150	131	249	0	0	380	0	0	0	0	0	71	719	790
08:45 AM	0	145	32	18	177	31	0	82	42	113	132	234	0	0	366	0	0	0	0	0	60	656	716
Total	0	732	115	58	847	181	2	435	175	618	531	1226	0	0	1757	0	0	0	0	0	233	3222	3455
Grand Total	0	1600	246	82	1846	401	2	798	380	1201	1076	2143	0	0	3219	0	0	0	0	0	462	6266	6728
Apprch %	0	86.7	13.3			33.4	0.2	66.4			33.4	66.6	0			0	0	0					
Total %	0	25.5	3.9		29.5	6.4	0	12.7		19.2	17.2	34.2	0		51.4	0	0	0		0	6.9	93.1	

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Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	0	246	**36**	282	56	0	93	149	114	211	0	325	0	0	0	0	756
07:45 AM	0	**283**	26	**309**	**65**	0	103	168	**143**	316	0	459	0	0	0	0	936
08:00 AM	0	224	27	251	65	0	**125**	**190**	142	**407**	0	**549**	0	0	0	0	**990**
08:15 AM	0	197	33	230	46	**1**	118	165	126	336	0	462	0	0	0	0	857
Total Volume	0	950	122	1072	232	1	439	672	525	1270	0	1795	0	0	0	0	3539
% App. Total	0	88.6	11.4		34.5	0.1	65.3		29.2	70.8	0		0	0	0		
PHF	.000	.839	.847	.867	.892	.250	.878	.884	.918	.780	.000	.817	.000	.000	.000	.000	.894

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File Name : 01_RIV_La S_91W AM
Site Code : 05123396
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Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	0	246	**36**	282	56	0	93	149	114	211	0	325	0	0	0	0	
+15 mins.	0	**283**	26	**309**	**65**	0	103	168	**143**	316	0	459	0	0	0	0	
+30 mins.	0	224	27	251	65	0	**125**	**190**	142	**407**	0	**549**	0	0	0	0	
+45 mins.	0	197	33	230	46	**1**	118	165	126	336	0	462	0	0	0	0	
Total Volume	0	950	122	1072	232	1	439	672	525	1270	0	1795	0	0	0	0	
% App. Total	0	88.6	11.4		34.5	0.1	65.3		29.2	70.8	0		0	0	0		
PHF	.000	.839	.847	.867	.892	.250	.878	.884	.918	.780	.000	.817	.000	.000	.000	.000	

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Groups Printed- Large 2 Axle Vehicles

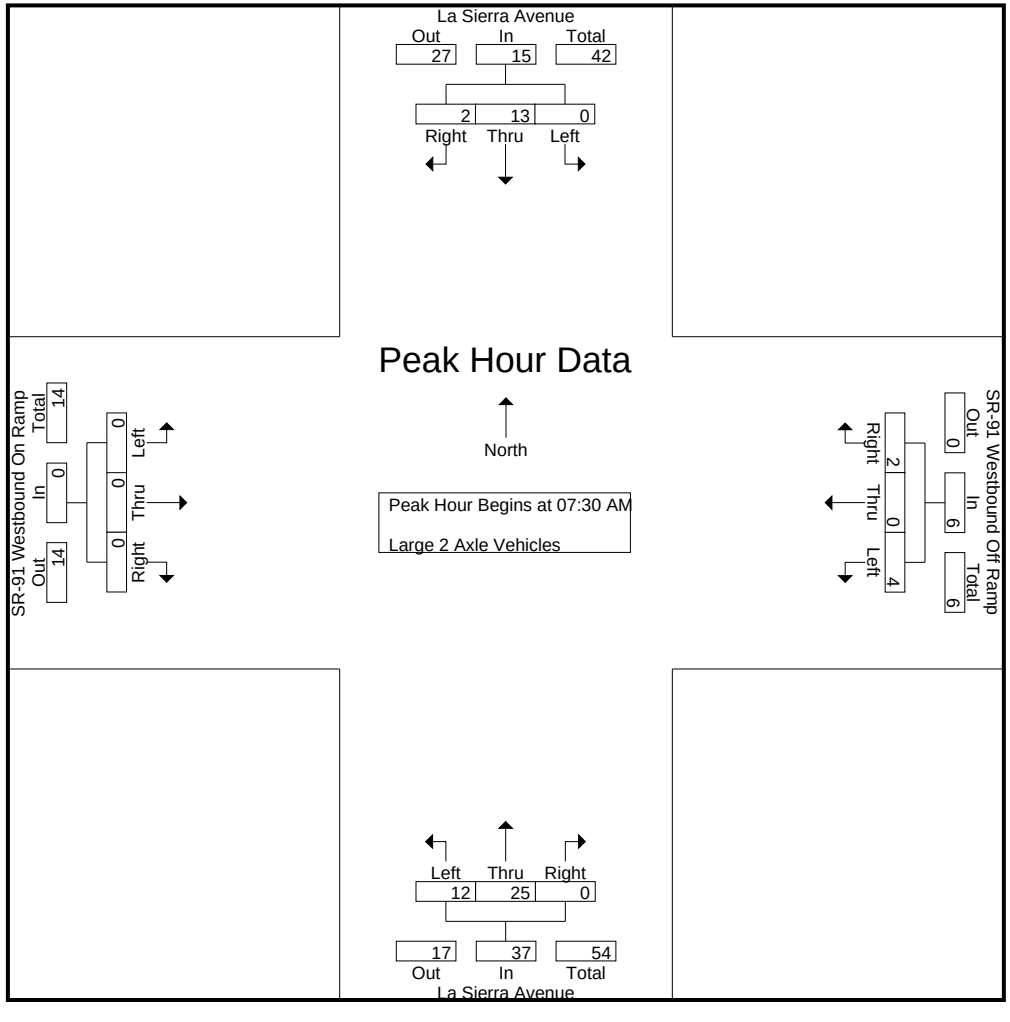
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	4	1	0	5	3	0	1	1	4	4	4	0	0	8	0	0	0	0	0	1	17	18
07:15 AM	0	3	0	0	3	1	0	4	2	5	1	2	0	0	3	0	0	0	0	0	2	11	13
07:30 AM	0	3	0	0	3	1	0	0	0	1	4	7	0	0	11	0	0	0	0	0	0	15	15
07:45 AM	0	3	0	0	3	1	0	0	0	1	1	6	0	0	7	0	0	0	0	0	0	11	11
Total	0	13	1	0	14	6	0	5	3	11	10	19	0	0	29	0	0	0	0	0	3	54	57
08:00 AM	0	4	1	0	5	0	0	1	0	1	1	7	0	0	8	0	0	0	0	0	0	14	14
08:15 AM	0	3	1	0	4	2	0	1	1	3	6	5	0	0	11	0	0	0	0	0	1	18	19
08:30 AM	0	0	1	0	1	1	1	1	1	3	3	6	0	0	9	0	0	0	0	0	1	13	14
08:45 AM	0	3	0	0	3	1	0	1	0	2	0	3	0	0	3	0	0	0	0	0	0	8	8
Total	0	10	3	0	13	4	1	4	2	9	10	21	0	0	31	0	0	0	0	0	2	53	55
Grand Total	0	23	4	0	27	10	1	9	5	20	20	40	0	0	60	0	0	0	0	0	5	107	112
Apprch %	0	85.2	14.8			50	5	45			33.3	66.7	0			0	0	0					
Total %	0	21.5	3.7		25.2	9.3	0.9	8.4		18.7	18.7	37.4	0		56.1	0	0	0		0	4.5	95.5	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	0	3	0	3	1	0	0	1	4	**7**	0	**11**	0	0	0	0	15
07:45 AM	0	3	0	3	1	0	0	1	1	6	0	7	0	0	0	0	11
08:00 AM	0	**4**	**1**	**5**	0	0	**1**	1	1	7	0	8	0	0	0	0	14
08:15 AM	0	3	1	4	**2**	0	1	**3**	**6**	5	0	11	0	0	0	0	**18**
Total Volume	0	13	2	15	4	0	2	6	12	25	0	37	0	0	0	0	58
% App. Total	0	86.7	13.3		66.7	0	33.3		32.4	67.6	0		0	0	0		
PHF	.000	.813	.500	.750	.500	.000	.500	.500	.500	.893	.000	.841	.000	.000	.000	.000	.806

Counts Unlimited, Inc.
PO Box 1178
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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W AM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W AM
Site Code : 05123396
Start Date : 8/17/2023

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	0	3	0	3	1	0	0	1	4	**7**	0	**11**	0	0	0	0	
+15 mins.	0	3	0	3	1	0	0	1	1	6	0	7	0	0	0	0	
+30 mins.	0	**4**	**1**	**5**	0	0	**1**	1	1	7	0	8	0	0	0	0	
+45 mins.	0	3	1	4	**2**	0	1	**3**	**6**	5	0	11	0	0	0	0	
Total Volume	0	13	2	15	4	0	2	6	12	25	0	37	0	0	0	0	
% App. Total	0	86.7	13.3		66.7	0	33.3		32.4	67.6	0		0	0	0		
PHF	.000	.813	.500	.750	.500	.000	.500	.500	.500	.893	.000	.841	.000	.000	.000	.000	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W AM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- 3 Axle Vehicles

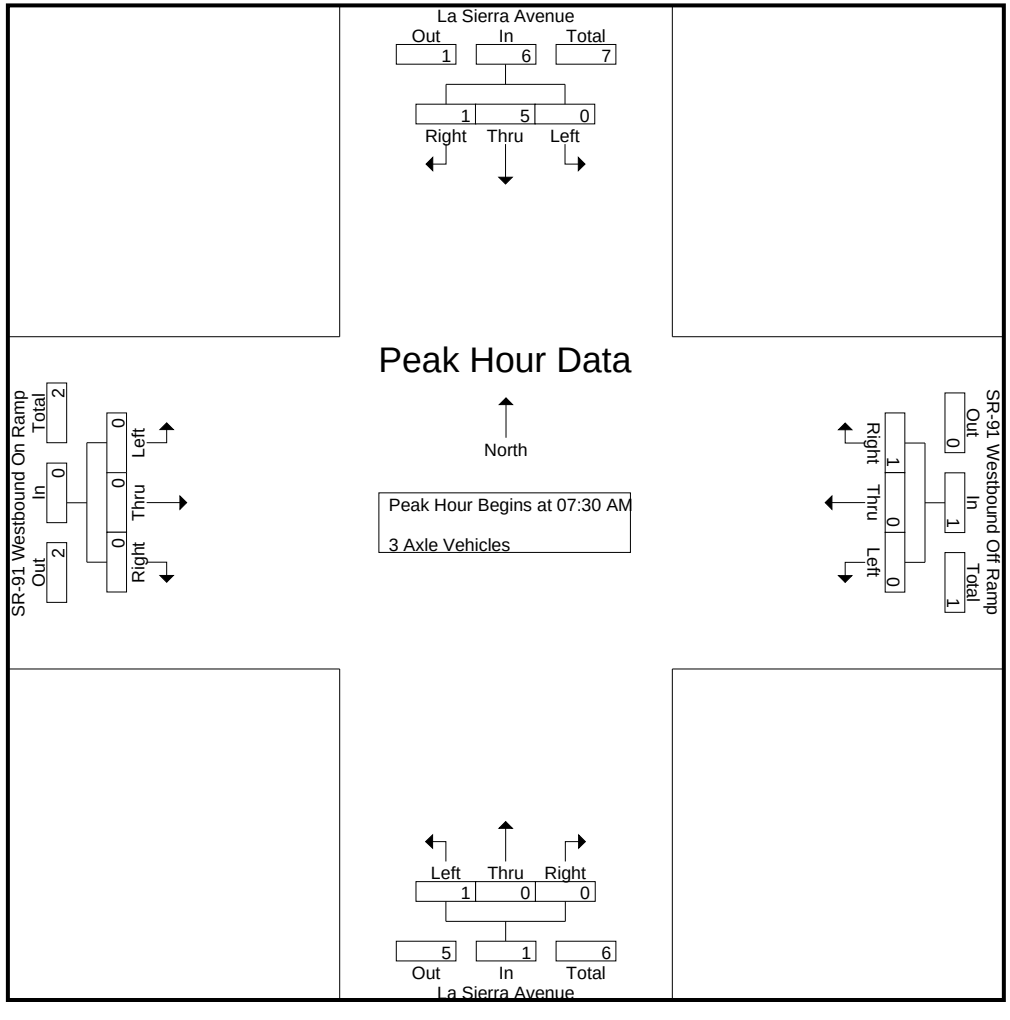
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	1	0	0	1	0	0	0	0	0	1	1	0	0	2	0	0	0	0	0	0	3	3
07:15 AM	0	2	0	0	2	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	4	4
07:30 AM	0	1	0	0	1	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	2	2
07:45 AM	0	1	0	0	1	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	2	2
Total	0	5	0	0	5	0	0	1	0	1	2	3	0	0	5	0	0	0	0	0	0	11	11
08:00 AM	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2
08:15 AM	0	1	1	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2
08:30 AM	0	1	0	0	1	0	0	1	1	1	0	2	0	0	2	0	0	0	0	0	1	4	5
08:45 AM	0	0	0	0	0	0	0	1	0	1	3	0	0	0	3	0	0	0	0	0	0	4	4
Total	0	4	1	0	5	0	0	2	1	2	3	2	0	0	5	0	0	0	0	0	1	12	13
Grand Total	0	9	1	0	10	0	0	3	1	3	5	5	0	0	10	0	0	0	0	0	1	23	24
Apprch %	0	90	10			0	0	100			50	50	0			0	0	0					
Total %	0	39.1	4.3		43.5	0	0	13		13	21.7	21.7	0		43.5	0	0	0		0	4.2	95.8	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	0	1	0	1	0	0	**1**	**1**	0	0	0	0	0	0	0	0	**2**
07:45 AM	0	1	0	1	0	0	0	0	**1**	0	0	**1**	0	0	0	0	2
08:00 AM	0	**2**	0	**2**	0	0	0	0	0	0	0	0	0	0	0	0	2
08:15 AM	0	1	**1**	2	0	0	0	0	0	0	0	0	0	0	0	0	2
Total Volume	0	5	1	6	0	0	1	1	1	0	0	1	0	0	0	0	8
% App. Total	0	83.3	16.7		0	0	100		100	0	0		0	0	0		
PHF	.000	.625	.250	.750	.000	.000	.250	.250	.250	.000	.000	.250	.000	.000	.000	.000	1.00

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W AM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W AM
Site Code : 05123396
Start Date : 8/17/2023

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	0	1	0	1	0	0	**1**	**1**	0	0	0	0	0	0	0	0	
+15 mins.	0	1	0	1	0	0	0	0	**1**	0	0	**1**	0	0	0	0	
+30 mins.	0	**2**	0	**2**	0	0	0	0	0	0	0	0	0	0	0	0	
+45 mins.	0	1	**1**	2	0	0	0	0	0	0	0	0	0	0	0	0	
Total Volume	0	5	1	6	0	0	1	1	1	0	0	1	0	0	0	0	
% App. Total	0	83.3	16.7		0	0	100		100	0	0		0	0	0		
PHF	.000	.625	.250	.750	.000	.000	.250	.250	.250	.000	.000	.250	.000	.000	.000	.000	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W AM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- 4+ Axle Trucks

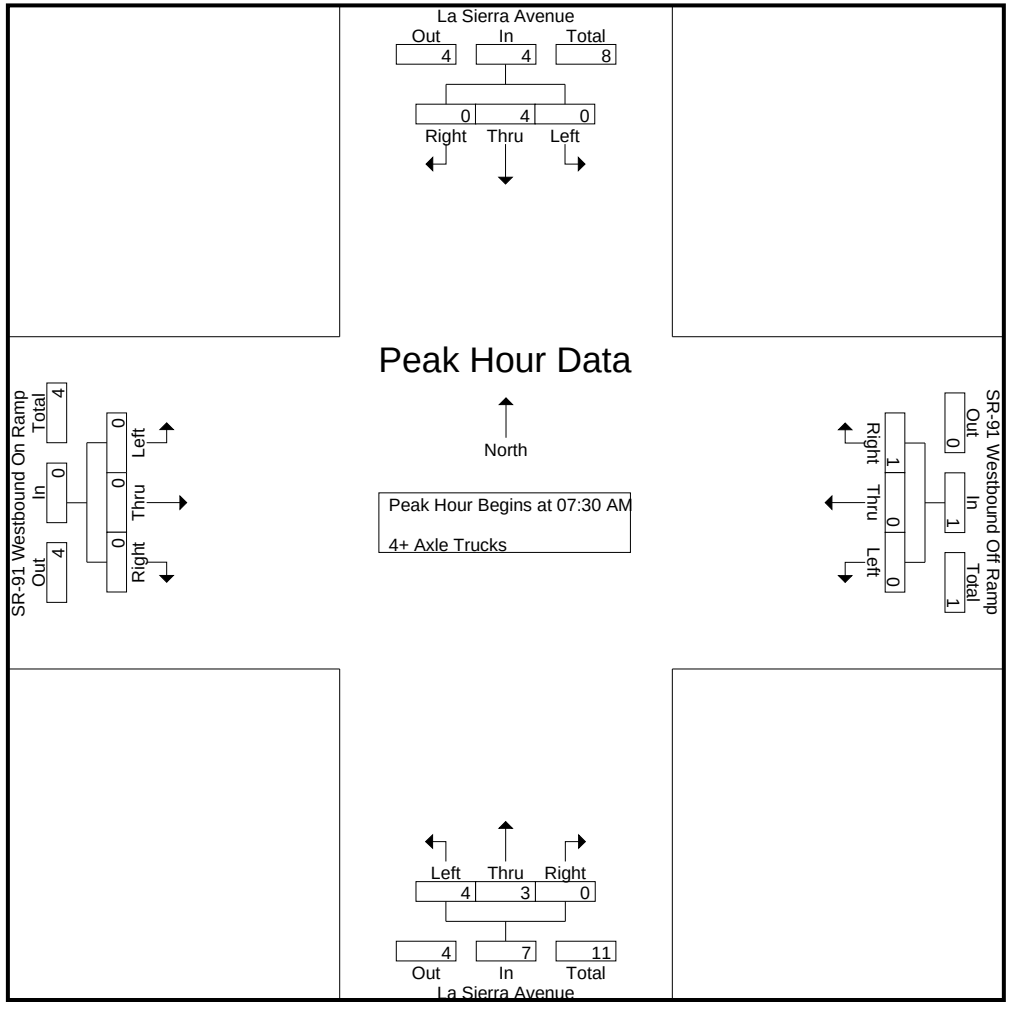
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	0	0	0	1	0	0	0	1	0	1	0	0	1	0	0	0	0	0	0	2	2
07:15 AM	0	1	0	0	1	0	0	0	0	0	1	1	0	0	2	0	0	0	0	0	0	3	3
07:30 AM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
07:45 AM	0	2	0	0	2	0	0	1	0	1	2	2	0	0	4	0	0	0	0	0	0	7	7
Total	0	4	0	0	4	1	0	1	0	2	3	4	0	0	7	0	0	0	0	0	0	13	13
08:00 AM	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	2	2
08:15 AM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	0	2	2
08:30 AM	0	1	0	0	1	0	0	0	0	0	1	1	0	0	2	0	0	0	0	0	0	3	3
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	2	0	0	2	0	0	0	0	0	3	2	0	0	5	0	0	0	0	0	0	7	7
Grand Total	0	6	0	0	6	1	0	1	0	2	6	6	0	0	12	0	0	0	0	0	0	20	20
Apprch %	0	100	0			50	0	50			50	50	0			0	0	0					
Total %	0	30	0		30	5	0	5		10	30	30	0		60	0	0	0		0	0	100	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
07:45 AM	0	**2**	0	**2**	0	0	**1**	**1**	**2**	**2**	0	**4**	0	0	0	0	**7**
08:00 AM	0	1	0	1	0	0	0	0	0	1	0	1	0	0	0	0	2
08:15 AM	0	0	0	0	0	0	0	0	2	0	0	2	0	0	0	0	2
Total Volume	0	4	0	4	0	0	1	1	4	3	0	7	0	0	0	0	12
% App. Total	0	100	0		0	0	100		57.1	42.9	0		0	0	0		
PHF	.000	.500	.000	.500	.000	.000	.250	.250	.500	.375	.000	.438	.000	.000	.000	.000	.429

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W AM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W AM
Site Code : 05123396
Start Date : 8/17/2023

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	
+15 mins.	0	**2**	0	**2**	0	0	**1**	**1**	**2**	**2**	0	**4**	0	0	0	0	
+30 mins.	0	1	0	1	0	0	0	0	0	1	0	1	0	0	0	0	
+45 mins.	0	0	0	0	0	0	0	0	2	0	0	2	0	0	0	0	
Total Volume	0	4	0	4	0	0	1	1	4	3	0	7	0	0	0	0	
% App. Total	0	100	0		0	0	100		57.1	42.9	0		0	0	0		
PHF	.000	.500	.000	.500	.000	.000	.250	.250	.500	.375	.000	.438	.000	.000	.000	.000	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

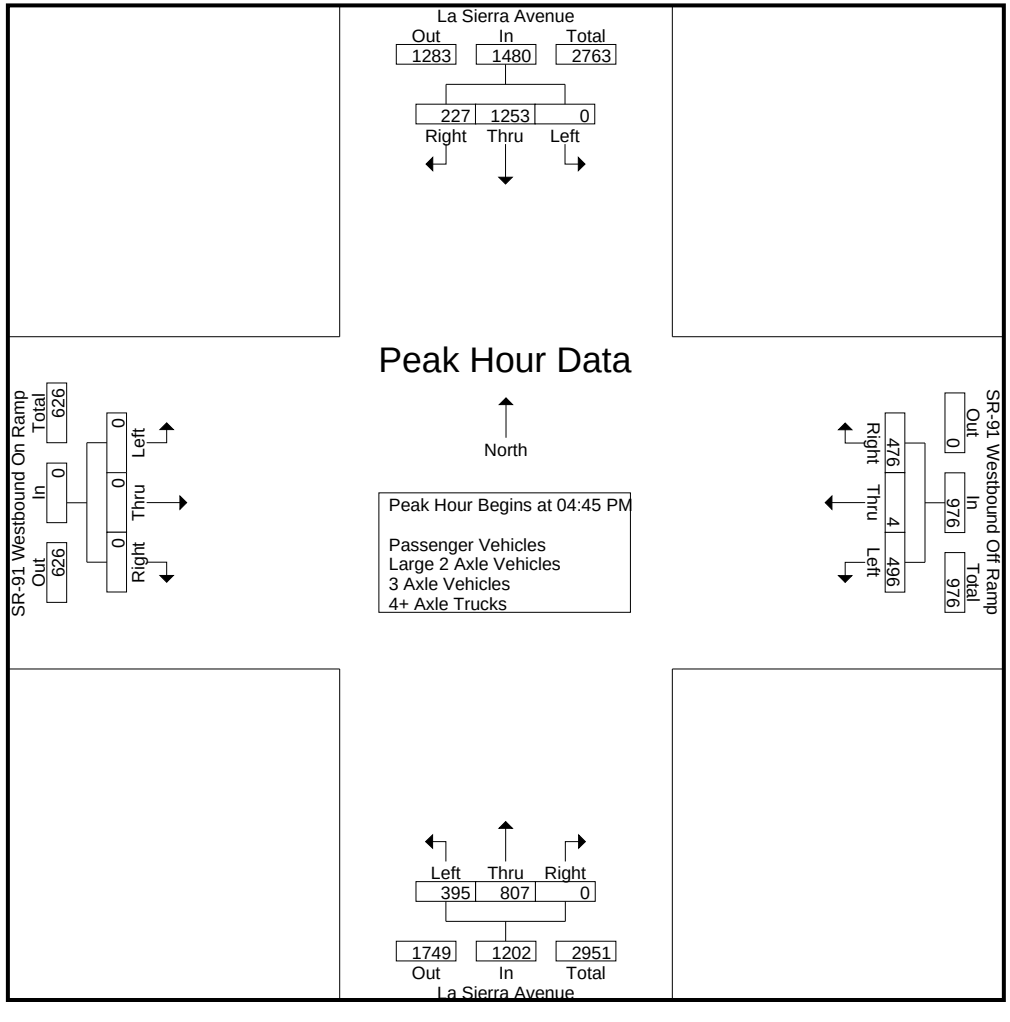
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	282	56	8	338	127	0	125	60	252	104	209	0	0	313	0	0	0	0	0	68	903	971
04:15 PM	0	251	34	11	285	129	2	125	58	256	90	210	0	0	300	0	0	0	0	0	69	841	910
04:30 PM	0	276	55	14	331	109	1	102	61	212	86	200	0	0	286	0	0	0	0	0	75	829	904
04:45 PM	0	276	48	12	324	126	1	127	68	254	93	186	0	0	279	0	0	0	0	0	80	857	937
Total	0	1085	193	45	1278	491	4	479	247	974	373	805	0	0	1178	0	0	0	0	0	292	3430	3722
05:00 PM	0	330	63	12	393	128	1	117	64	246	107	198	0	0	305	0	0	0	0	0	76	944	1020
05:15 PM	0	342	59	4	401	134	1	126	71	261	101	214	0	0	315	0	0	0	0	0	75	977	1052
05:30 PM	0	305	57	10	362	108	1	106	55	215	94	209	0	0	303	0	0	0	0	0	65	880	945
05:45 PM	0	267	41	11	308	102	0	130	69	232	75	192	0	0	267	0	0	0	0	0	80	807	887
Total	0	1244	220	37	1464	472	3	479	259	954	377	813	0	0	1190	0	0	0	0	0	296	3608	3904
Grand Total	0	2329	413	82	2742	963	7	958	506	1928	750	1618	0	0	2368	0	0	0	0	0	588	7038	7626
Apprch %	0	84.9	15.1			49.9	0.4	49.7			31.7	68.3	0			0	0	0					
Total %	0	33.1	5.9		39	13.7	0.1	13.6		27.4	10.7	23	0		33.6	0	0	0		0	7.7	92.3	
Passenger Vehicles	0	2294	411		2787	953	6	951		2414	729	1590	0		2319	0	0	0		0	0	0	7520
% Passenger Vehicles	0	98.5	99.5	100	98.7	99	85.7	99.3	99.6	99.2	97.2	98.3	0	0	97.9	0	0	0	0	0	0	0	98.6
Large 2 Axle Vehicles	0	29	2		31	6	1	4		13	17	22	0		39	0	0	0		0	0	0	83
% Large 2 Axle Vehicles	0	1.2	0.5	0	1.1	0.6	14.3	0.4	0.4	0.5	2.3	1.4	0	0	1.6	0	0	0	0	0	0	0	1.1
3 Axle Vehicles	0	4	0		4	3	0	1		4	3	3	0		6	0	0	0		0	0	0	14
% 3 Axle Vehicles	0	0.2	0	0	0.1	0.3	0	0.1	0	0.2	0.4	0.2	0	0	0.3	0	0	0	0	0	0	0	0.2
4+ Axle Trucks	0	2	0		2	1	0	2		3	1	3	0		4	0	0	0		0	0	0	9
% 4+ Axle Trucks	0	0.1	0	0	0.1	0.1	0	0.2	0	0.1	0.1	0.2	0	0	0.2	0	0	0	0	0	0	0	0.1

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	0	276	48	324	126	**1**	**127**	254	93	186	0	279	0	0	0	0	857
05:00 PM	0	330	**63**	393	128	1	117	246	**107**	198	0	305	0	0	0	0	944
05:15 PM	0	**342**	59	**401**	**134**	1	126	**261**	101	**214**	0	**315**	0	0	0	0	**977**
05:30 PM	0	305	57	362	108	1	106	215	94	209	0	303	0	0	0	0	880
Total Volume	0	1253	227	1480	496	4	476	976	395	807	0	1202	0	0	0	0	3658
% App. Total	0	84.7	15.3		50.8	0.4	48.8		32.9	67.1	0		0	0	0		
PHF	.000	.916	.901	.923	.925	1.00	.937	.935	.923	.943	.000	.954	.000	.000	.000	.000	.936

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
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File Name : 01_RIV_La S_91W PM
Site Code : 05123396
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	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:45 PM				04:45 PM				04:00 PM				
+0 mins.	0	276	48	324	126	**1**	**127**	254	93	186	0	279	0	0	0	0	
+15 mins.	0	330	**63**	393	128	1	117	246	**107**	198	0	305	0	0	0	0	
+30 mins.	0	**342**	59	**401**	**134**	1	126	**261**	101	**214**	0	**315**	0	0	0	0	
+45 mins.	0	305	57	362	108	1	106	215	94	209	0	303	0	0	0	0	
Total Volume	0	1253	227	1480	496	4	476	976	395	807	0	1202	0	0	0	0	
% App. Total	0	84.7	15.3		50.8	0.4	48.8		32.9	67.1	0		0	0	0		
PHF	.000	.916	.901	.923	.925	1.000	.937	.935	.923	.943	.000	.954	.000	.000	.000	.000	

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City of Riverside
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File Name : 01_RIV_La S_91W PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- Passenger Vehicles

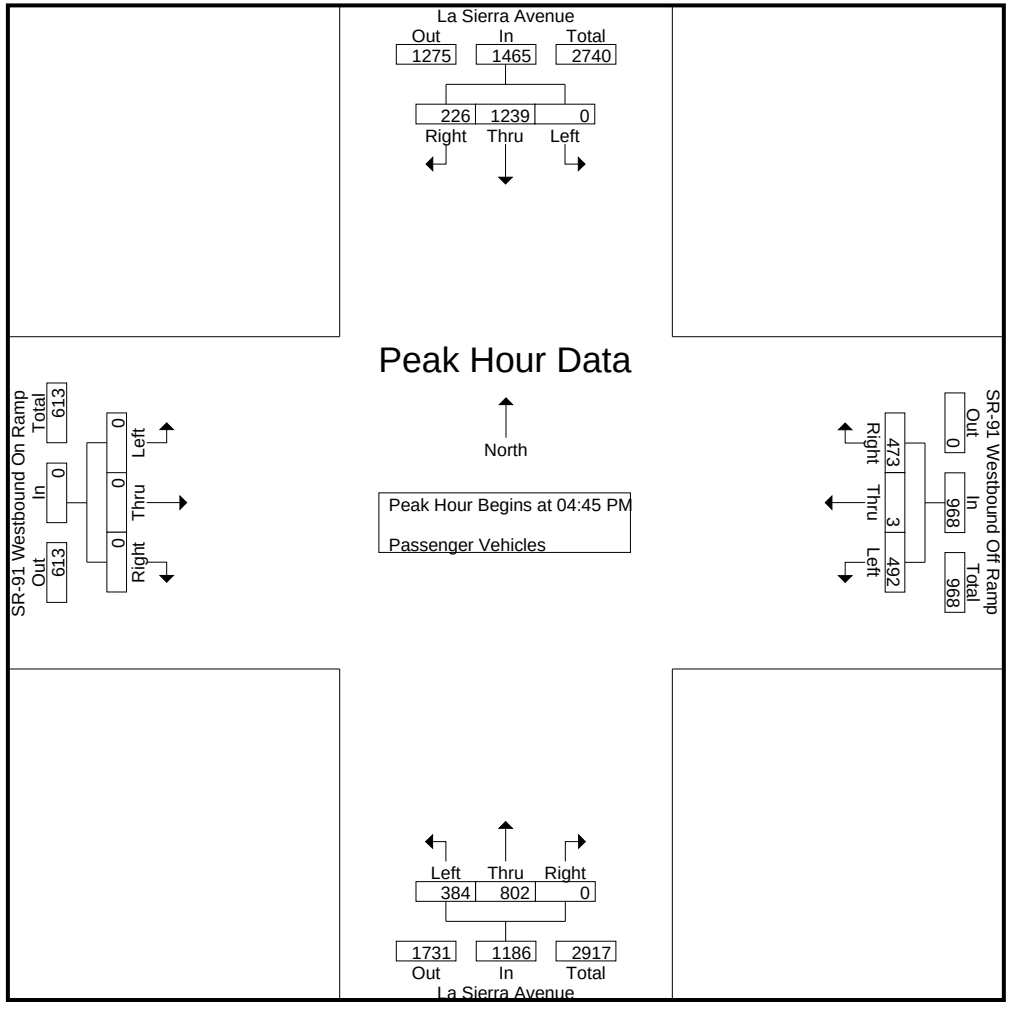
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	273	56	8	329	124	0	124	60	248	101	205	0	0	306	0	0	0	0	0	68	883	951
04:15 PM	0	244	34	11	278	128	2	124	57	254	87	201	0	0	288	0	0	0	0	0	68	820	888
04:30 PM	0	272	55	14	327	108	1	101	61	210	83	195	0	0	278	0	0	0	0	0	75	815	890
04:45 PM	0	273	48	12	321	126	0	126	68	252	91	185	0	0	276	0	0	0	0	0	80	849	929
Total	0	1062	193	45	1255	486	3	475	246	964	362	786	0	0	1148	0	0	0	0	0	291	3367	3658
05:00 PM	0	325	62	12	387	127	1	116	64	244	104	196	0	0	300	0	0	0	0	0	76	931	1007
05:15 PM	0	337	59	4	396	133	1	125	71	259	96	214	0	0	310	0	0	0	0	0	75	965	1040
05:30 PM	0	304	57	10	361	106	1	106	55	213	93	207	0	0	300	0	0	0	0	0	65	874	939
05:45 PM	0	266	40	11	306	101	0	129	68	230	74	187	0	0	261	0	0	0	0	0	79	797	876
Total	0	1232	218	37	1450	467	3	476	258	946	367	804	0	0	1171	0	0	0	0	0	295	3567	3862
Grand Total	0	2294	411	82	2705	953	6	951	504	1910	729	1590	0	0	2319	0	0	0	0	0	586	6934	7520
Apprch %	0	84.8	15.2			49.9	0.3	49.8			31.4	68.6	0			0	0	0					
Total %	0	33.1	5.9		39	13.7	0.1	13.7		27.5	10.5	22.9	0		33.4	0	0	0		0	7.8	92.2	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	0	273	48	321	126	0	**126**	252	91	185	0	276	0	0	0	0	849
05:00 PM	0	325	**62**	387	127	**1**	116	244	**104**	196	0	300	0	0	0	0	931
05:15 PM	0	**337**	59	**396**	**133**	1	125	**259**	96	**214**	0	**310**	0	0	0	0	**965**
05:30 PM	0	304	57	361	106	1	106	213	93	207	0	300	0	0	0	0	874
Total Volume	0	1239	226	1465	492	3	473	968	384	802	0	1186	0	0	0	0	3619
% App. Total	0	84.6	15.4		50.8	0.3	48.9		32.4	67.6	0		0	0	0		
PHF	.000	.919	.911	.925	.925	.750	.938	.934	.923	.937	.000	.956	.000	.000	.000	.000	.938

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W PM
Site Code : 05123396
Start Date : 8/17/2023

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:45 PM				04:45 PM				04:45 PM				
+0 mins.	0	273	48	321	126	0	**126**	252	91	185	0	276	0	0	0	0	
+15 mins.	0	325	**62**	387	127	**1**	116	244	**104**	196	0	300	0	0	0	0	
+30 mins.	0	**337**	59	**396**	**133**	1	125	**259**	96	**214**	0	**310**	0	0	0	0	
+45 mins.	0	304	57	361	106	1	106	213	93	207	0	300	0	0	0	0	
Total Volume	0	1239	226	1465	492	3	473	968	384	802	0	1186	0	0	0	0	
% App. Total	0	84.6	15.4		50.8	0.3	48.9		32.4	67.6	0		0	0	0		
PHF	.000	.919	.911	.925	.925	.750	.938	.934	.923	.937	.000	.956	.000	.000	.000	.000	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- Large 2 Axle Vehicles

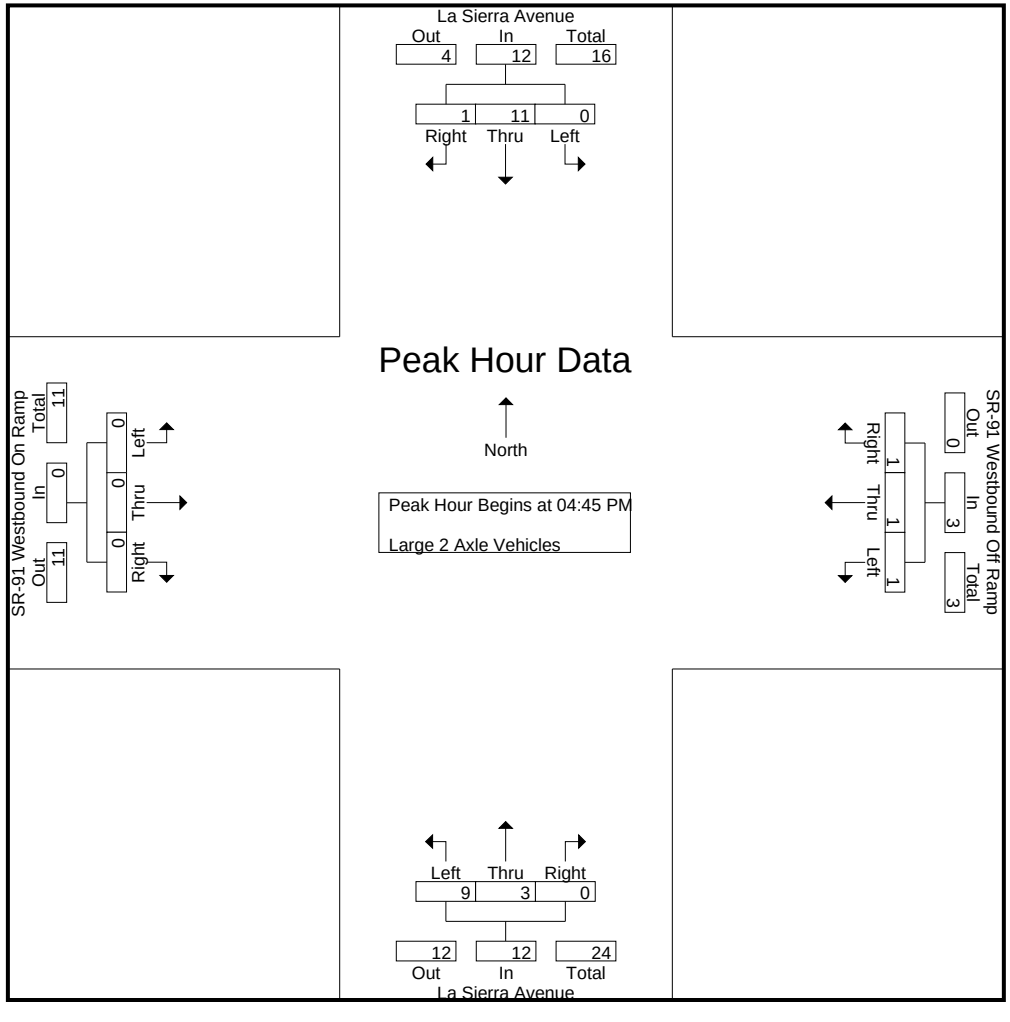
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	7	0	0	7	3	0	0	0	3	2	3	0	0	5	0	0	0	0	0	0	15	15
04:15 PM	0	7	0	0	7	1	0	1	1	2	3	7	0	0	10	0	0	0	0	0	1	19	20
04:30 PM	0	3	0	0	3	0	0	1	0	1	2	4	0	0	6	0	0	0	0	0	0	10	10
04:45 PM	0	3	0	0	3	0	1	0	0	1	2	0	0	0	2	0	0	0	0	0	0	6	6
Total	0	20	0	0	20	4	1	2	1	7	9	14	0	0	23	0	0	0	0	0	1	50	51
05:00 PM	0	2	1	0	3	0	0	0	0	0	3	2	0	0	5	0	0	0	0	0	0	8	8
05:15 PM	0	5	0	0	5	0	0	1	0	1	3	0	0	0	3	0	0	0	0	0	0	9	9
05:30 PM	0	1	0	0	1	1	0	0	0	1	1	1	0	0	2	0	0	0	0	0	0	4	4
05:45 PM	0	1	1	0	2	1	0	1	1	2	1	5	0	0	6	0	0	0	0	0	1	10	11
Total	0	9	2	0	11	2	0	2	1	4	8	8	0	0	16	0	0	0	0	0	1	31	32
Grand Total	0	29	2	0	31	6	1	4	2	11	17	22	0	0	39	0	0	0	0	0	2	81	83
Apprch %	0	93.5	6.5			54.5	9.1	36.4			43.6	56.4	0			0	0	0					
Total %	0	35.8	2.5		38.3	7.4	1.2	4.9		13.6	21	27.2	0		48.1	0	0	0		0	2.4	97.6	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	0	3	0	3	0	**1**	0	**1**	2	0	0	2	0	0	0	0	6
05:00 PM	0	2	**1**	3	0	0	0	0	**3**	**2**	0	**5**	0	0	0	0	8
05:15 PM	0	**5**	0	**5**	0	0	**1**	1	3	0	0	3	0	0	0	0	**9**
05:30 PM	0	1	0	1	**1**	0	0	1	1	1	0	2	0	0	0	0	4
Total Volume	0	11	1	12	1	1	1	3	9	3	0	12	0	0	0	0	27
% App. Total	0	91.7	8.3		33.3	33.3	33.3		75	25	0		0	0	0		
PHF	.000	.550	.250	.600	.250	.250	.250	.750	.750	.375	.000	.600	.000	.000	.000	.000	.750

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 3

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:45 PM				04:45 PM				04:45 PM				
+0 mins.	0	3	0	3	0	**1**	0	**1**	2	0	0	2	0	0	0	0	
+15 mins.	0	2	**1**	3	0	0	0	0	**3**	**2**	0	**5**	0	0	0	0	
+30 mins.	0	**5**	0	**5**	0	0	**1**	1	3	0	0	3	0	0	0	0	
+45 mins.	0	1	0	1	**1**	0	0	1	1	1	0	2	0	0	0	0	
Total Volume	0	11	1	12	1	1	1	3	9	3	0	12	0	0	0	0	
% App. Total	0	91.7	8.3		33.3	33.3	33.3		75	25	0		0	0	0		
PHF	.000	.550	.250	.600	.250	.250	.250	.750	.750	.375	.000	.600	.000	.000	.000	.000	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- 3 Axle Vehicles

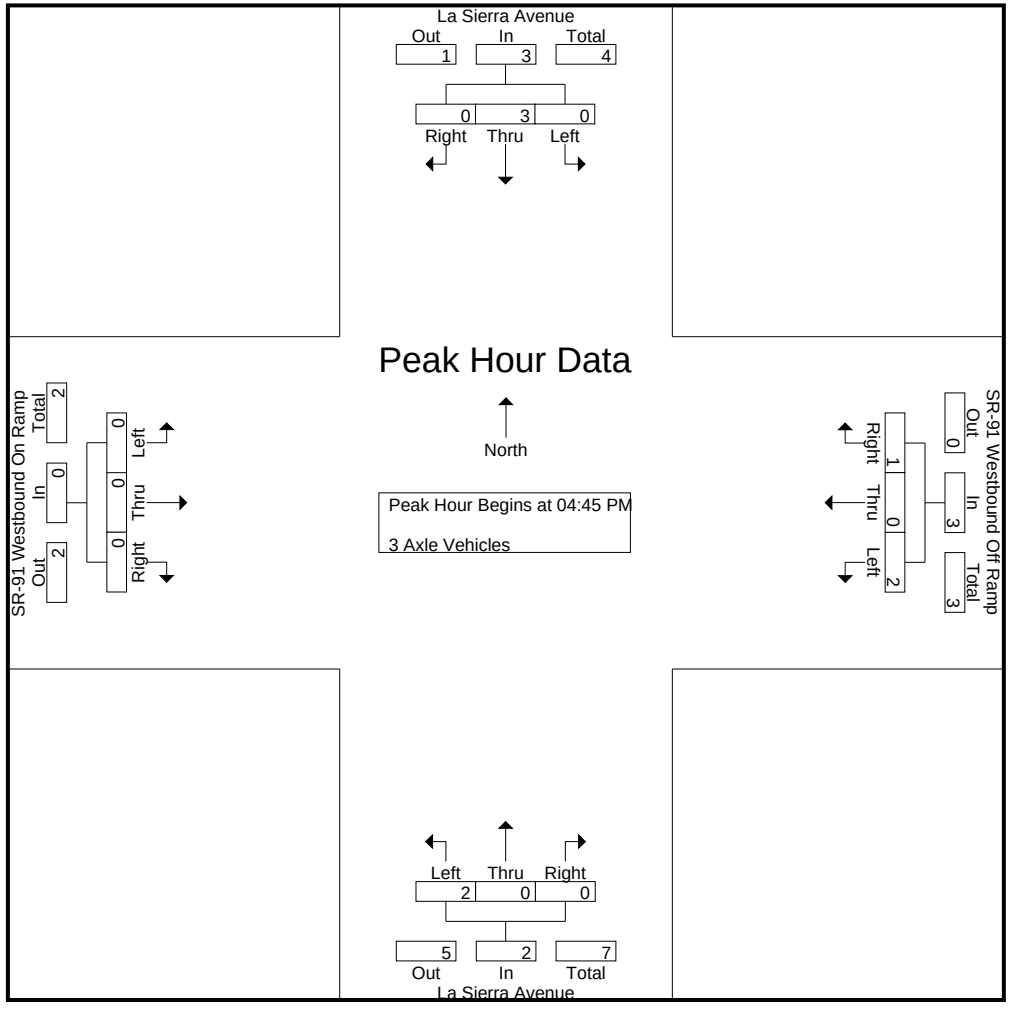
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	1	0	0	1	0	0	0	0	0	1	1	0	0	2	0	0	0	0	0	0	3	3
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	1	1
04:30 PM	0	0	0	0	0	1	0	0	0	1	0	1	0	0	1	0	0	0	0	0	0	2	2
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	1	0	0	1	1	0	0	0	1	1	3	0	0	4	0	0	0	0	0	0	6	6
05:00 PM	0	3	0	0	3	1	0	1	0	2	0	0	0	0	0	0	0	0	0	0	0	5	5
05:15 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	0	2	2
05:30 PM	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	1
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	3	0	0	3	2	0	1	0	3	2	0	0	0	2	0	0	0	0	0	0	8	8
Grand Total	0	4	0	0	4	3	0	1	0	4	3	3	0	0	6	0	0	0	0	0	0	14	14
Apprch %	0	100	0			75	0	25			50	50	0			0	0	0					
Total %	0	28.6	0		28.6	21.4	0	7.1		28.6	21.4	21.4	0		42.9	0	0	0		0	0	100	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00 PM	0	**3**	0	**3**	**1**	0	**1**	**2**	0	0	0	0	0	0	0	0	**5**
05:15 PM	0	0	0	0	0	0	0	0	**2**	0	0	**2**	0	0	0	0	2
05:30 PM	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	1
Total Volume	0	3	0	3	2	0	1	3	2	0	0	2	0	0	0	0	8
% App. Total	0	100	0		66.7	0	33.3		100	0	0		0	0	0		
PHF	.000	.250	.000	.250	.500	.000	.250	.375	.250	.000	.000	.250	.000	.000	.000	.000	.400

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W PM
Site Code : 05123396
Start Date : 8/17/2023



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W PM
Site Code : 05123396
Start Date : 8/17/2023

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:45 PM				04:45 PM				04:45 PM				
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
+15 mins.	0	**3**	0	**3**	**1**	0	**1**	**2**	0	0	0	0	0	0	0	0	
+30 mins.	0	0	0	0	0	0	0	0	**2**	0	0	**2**	0	0	0	0	
+45 mins.	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	
Total Volume	0	3	0	3	2	0	1	3	2	0	0	2	0	0	0	0	
% App. Total	0	100	0		66.7	0	33.3		100	0	0		0	0	0		
PHF	.000	.250	.000	.250	.500	.000	.250	.375	.250	.000	.000	.250	.000	.000	.000	.000	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- 4+ Axle Trucks

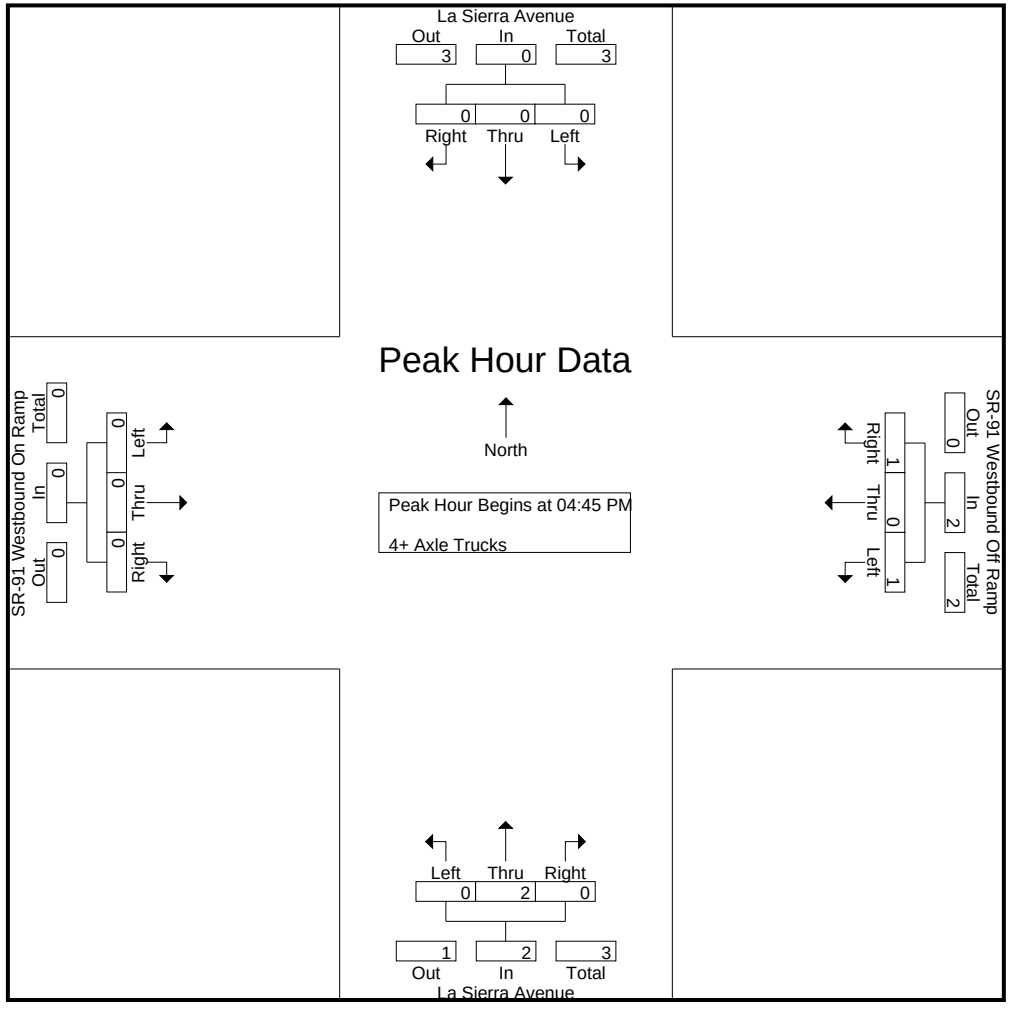
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	1	0	0	1	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	2	2
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	1	1
04:30 PM	0	1	0	0	1	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	2	2
04:45 PM	0	0	0	0	0	0	0	1	0	1	0	1	0	0	1	0	0	0	0	0	0	2	2
Total	0	2	0	0	2	0	0	2	0	2	1	2	0	0	3	0	0	0	0	0	0	7	7
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	1
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	1	1
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	1	0	0	0	1	0	1	0	0	1	0	0	0	0	0	0	2	2
Grand Total	0	2	0	0	2	1	0	2	0	3	1	3	0	0	4	0	0	0	0	0	0	9	9
Apprch %	0	100	0			33.3	0	66.7			25	75	0			0	0	0					
Total %	0	22.2	0		22.2	11.1	0	22.2		33.3	11.1	33.3	0		44.4	0	0	0		0	0	100	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	0	0	0	0	0	0	**1**	**1**	0	**1**	0	**1**	0	0	0	0	**2**
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	**1**	0	0	1	0	0	0	0	0	0	0	0	1
05:30 PM	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1
Total Volume	0	0	0	0	1	0	1	2	0	2	0	2	0	0	0	0	4
% App. Total	0	0	0		50	0	50		0	100	0		0	0	0		
PHF	.000	.000	.000	.000	.250	.000	.250	.500	.000	.500	.000	.500	.000	.000	.000	.000	.500

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_La S_91W PM
Site Code : 05123396
Start Date : 8/17/2023

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:45 PM				04:45 PM				04:45 PM				
+0 mins.	0	0	0	0	0	0	**1**	**1**	0	**1**	0	**1**	0	0	0	0	
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
+30 mins.	0	0	0	0	**1**	0	0	1	0	0	0	0	0	0	0	0	
+45 mins.	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	
Total Volume	0	0	0	0	1	0	1	2	0	2	0	2	0	0	0	0	
% App. Total	0	0	0		50	0	50		0	100	0		0	0	0		
PHF	.000	.000	.000	.000	.250	.000	.250	.500	.000	.500	.000	.500	.000	.000	.000	.000	

Location: Riverside
N/S: La Sierra Avenue
E/W: SR-91 WB Ramps

Date: 8/17/2023
Day: Thursday

PEDESTRIANS

	North Leg La Sierra Avenue	East Leg SR-91 WB Ramps	South Leg La Sierra Avenue	West Leg SR-91 WB Ramps	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	0	4	0	1	5
7:15 AM	0	1	0	2	3
7:30 AM	0	0	0	0	0
7:45 AM	0	2	0	1	3
8:00 AM	0	0	0	2	2
8:15 AM	0	1	0	0	1
8:30 AM	0	0	0	2	2
8:45 AM	0	1	0	1	2
TOTAL VOLUMES:	0	9	0	9	18

	North Leg La Sierra Avenue	East Leg SR-91 WB Ramps	South Leg La Sierra Avenue	West Leg SR-91 WB Ramps	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	0	3	0	2	5
4:15 PM	0	2	0	2	4
4:30 PM	0	0	0	1	1
4:45 PM	0	0	0	2	2
5:00 PM	0	1	0	1	2
5:15 PM	0	0	0	2	2
5:30 PM	0	0	0	1	1
5:45 PM	0	0	0	1	1
TOTAL VOLUMES:	0	6	0	12	18

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Location: Riverside
N/S: La Sierra Avenue
E/W: SR-91 WB Ramps

Date: 8/17/2023
Day: Thursday

BICYCLES

	Southbound La Sierra Avenue			Westbound SR-91 WB Ramps			Northbound La Sierra Avenue			Eastbound SR-91 WB Ramps			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	3	0	0	0	0	0	1	0	0	0	0	4
7:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	1
8:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	1
8:15 AM	0	0	0	0	0	0	0	1	0	0	0	0	1
8:30 AM	0	0	0	0	0	0	0	1	0	0	0	0	1
8:45 AM	0	0	0	0	0	0	0	1	0	0	0	0	1
TOTAL VOLUMES:	0	5	0	0	0	0	0	4	0	0	0	0	9

	Southbound La Sierra Avenue			Westbound SR-91 WB Ramps			Northbound La Sierra Avenue			Eastbound SR-91 WB Ramps			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	1	0	0	0	0	1
4:30 PM	0	0	0	0	0	0	0	1	0	0	0	0	1
4:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	1
5:00 PM	0	0	0	0	0	0	0	2	0	0	0	0	2
5:15 PM	0	2	0	0	0	0	0	0	0	0	0	0	2
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	1	0	0	0	0	0	1	0	0	0	0	2
TOTAL VOLUMES:	0	4	0	0	0	0	0	5	0	0	0	0	9

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E AM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

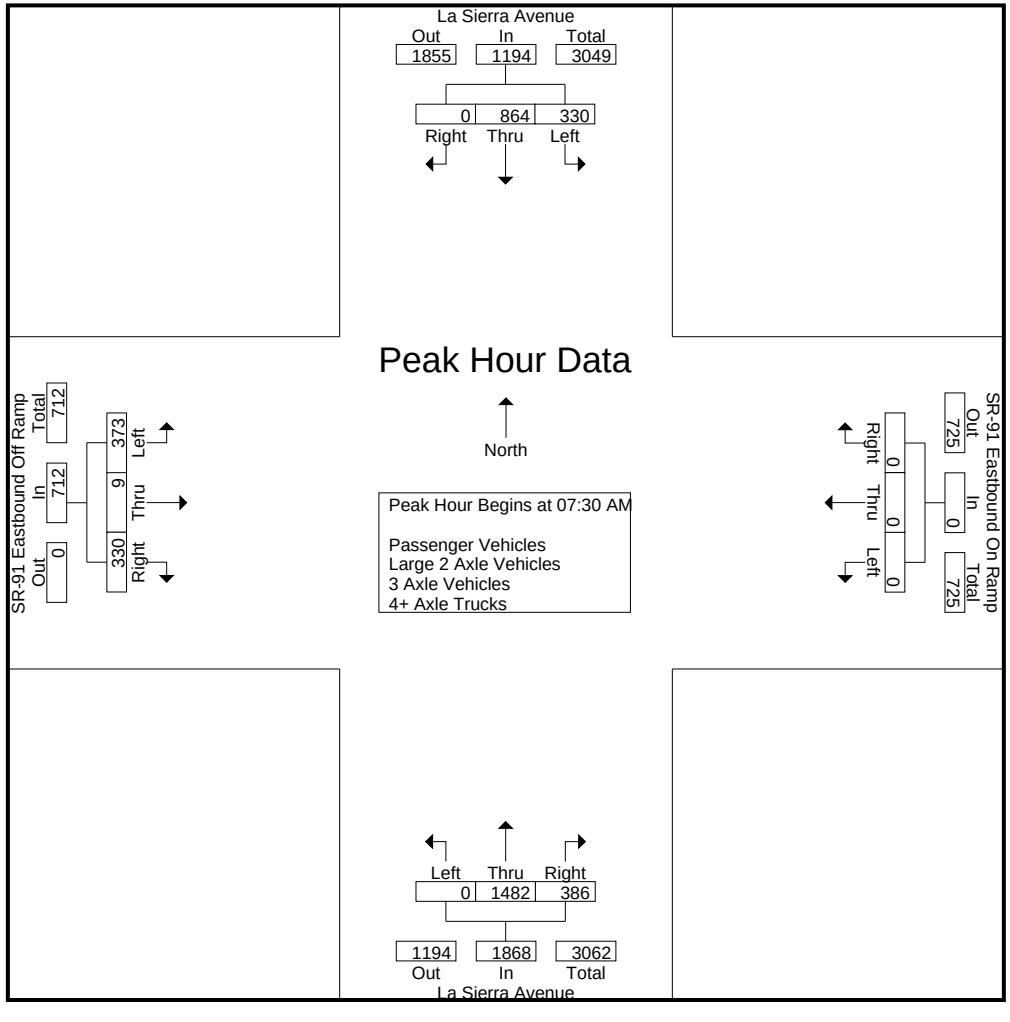
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	88	121	0	0	209	0	0	0	0	0	0	247	119	31	366	60	0	72	53	132	84	707	791
07:15 AM	106	151	0	0	257	0	0	0	0	0	0	299	112	45	411	74	0	66	45	140	90	808	898
07:30 AM	91	213	0	0	304	0	0	0	0	0	0	289	95	33	384	73	1	77	46	151	79	839	918
07:45 AM	67	274	0	0	341	0	0	0	0	0	0	359	89	37	448	98	2	101	60	201	97	990	1087
Total	352	759	0	0	1111	0	0	0	0	0	0	1194	415	146	1609	305	3	316	204	624	350	3344	3694
08:00 AM	86	214	0	0	300	0	0	0	0	0	0	442	93	37	535	107	5	81	54	193	91	1028	1119
08:15 AM	86	163	0	0	249	0	0	0	0	0	0	392	109	40	501	95	1	71	47	167	87	917	1004
08:30 AM	63	132	0	0	195	0	0	0	0	0	0	303	108	43	411	84	0	84	57	168	100	774	874
08:45 AM	74	111	0	0	185	0	0	0	0	0	0	306	99	28	405	73	1	65	43	139	71	729	800
Total	309	620	0	0	929	0	0	0	0	0	0	1443	409	148	1852	359	7	301	201	667	349	3448	3797
Grand Total	661	1379	0	0	2040	0	0	0	0	0	0	2637	824	294	3461	664	10	617	405	1291	699	6792	7491
Apprch %	32.4	67.6	0			0	0	0			0	76.2	23.8			51.4	0.8	47.8					
Total %	9.7	20.3	0		30	0	0	0		0	0	38.8	12.1		51	9.8	0.1	9.1		19	9.3	90.7	
Passenger Vehicles	644	1348	0		1992	0	0	0		0	0	2570	810		3673	645	8	560		1586	0	0	7251
% Passenger Vehicles	97.4	97.8	0	0	97.6	0	0	0	0	0	0	97.5	98.3	99.7	97.8	97.1	80	90.8	92.1	93.5	0	0	96.8
Large 2 Axle Vehicles	5	25	0		30	0	0	0		0	0	52	9		62	11	1	25		50	0	0	142
% Large 2 Axle Vehicles	0.8	1.8	0	0	1.5	0	0	0	0	0	0	2	1.1	0.3	1.7	1.7	10	4.1	3.2	2.9	0	0	1.9
3 Axle Vehicles	6	5	0		11	0	0	0		0	0	8	3		11	3	1	23		40	0	0	62
% 3 Axle Vehicles	0.9	0.4	0	0	0.5	0	0	0	0	0	0	0.3	0.4	0	0.3	0.5	10	3.7	3.2	2.4	0	0	0.8
4+ Axle Trucks	6	1	0		7	0	0	0		0	0	7	2		9	5	0	9		20	0	0	36
% 4+ Axle Trucks	0.9	0.1	0	0	0.3	0	0	0	0	0	0	0.3	0.2	0	0.2	0.8	0	1.5	1.5	1.2	0	0	0.5

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	**91**	213	0	304	0	0	0	0	0	289	95	384	73	1	77	151	839
07:45 AM	67	**274**	0	**341**	0	0	0	0	0	359	89	448	98	2	**101**	**201**	990
08:00 AM	86	214	0	300	0	0	0	0	0	**442**	93	**535**	**107**	**5**	81	193	**1028**
08:15 AM	86	163	0	249	0	0	0	0	0	392	**109**	501	95	1	71	167	917
Total Volume	330	864	0	1194	0	0	0	0	0	1482	386	1868	373	9	330	712	3774
% App. Total	27.6	72.4	0		0	0	0		0	79.3	20.7		52.4	1.3	46.3		
PHF	.907	.788	.000	.875	.000	.000	.000	.000	.000	.838	.885	.873	.871	.450	.817	.886	.918

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E AM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 2



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City of Riverside
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Start Date : 8/17/2023
Page No : 3

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:15 AM				07:00 AM				07:45 AM				07:45 AM				
+0 mins.	**106**	151	0	257	0	0	0	0	0	359	89	448	98	2	**101**	**201**	
+15 mins.	91	213	0	304	0	0	0	0	0	**442**	93	**535**	**107**	**5**	81	193	
+30 mins.	67	**274**	0	**341**	0	0	0	0	0	392	**109**	501	95	1	71	167	
+45 mins.	86	214	0	300	0	0	0	0	0	303	108	411	84	0	84	168	
Total Volume	350	852	0	1202	0	0	0	0	0	1496	399	1895	384	8	337	729	
% App. Total	29.1	70.9	0		0	0	0		0	78.9	21.1		52.7	1.1	46.2		
PHF	.825	.777	.000	.881	.000	.000	.000	.000	.000	.846	.915	.886	.897	.400	.834	.907	

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File Name : 02_RIV_La S_91E AM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- Passenger Vehicles

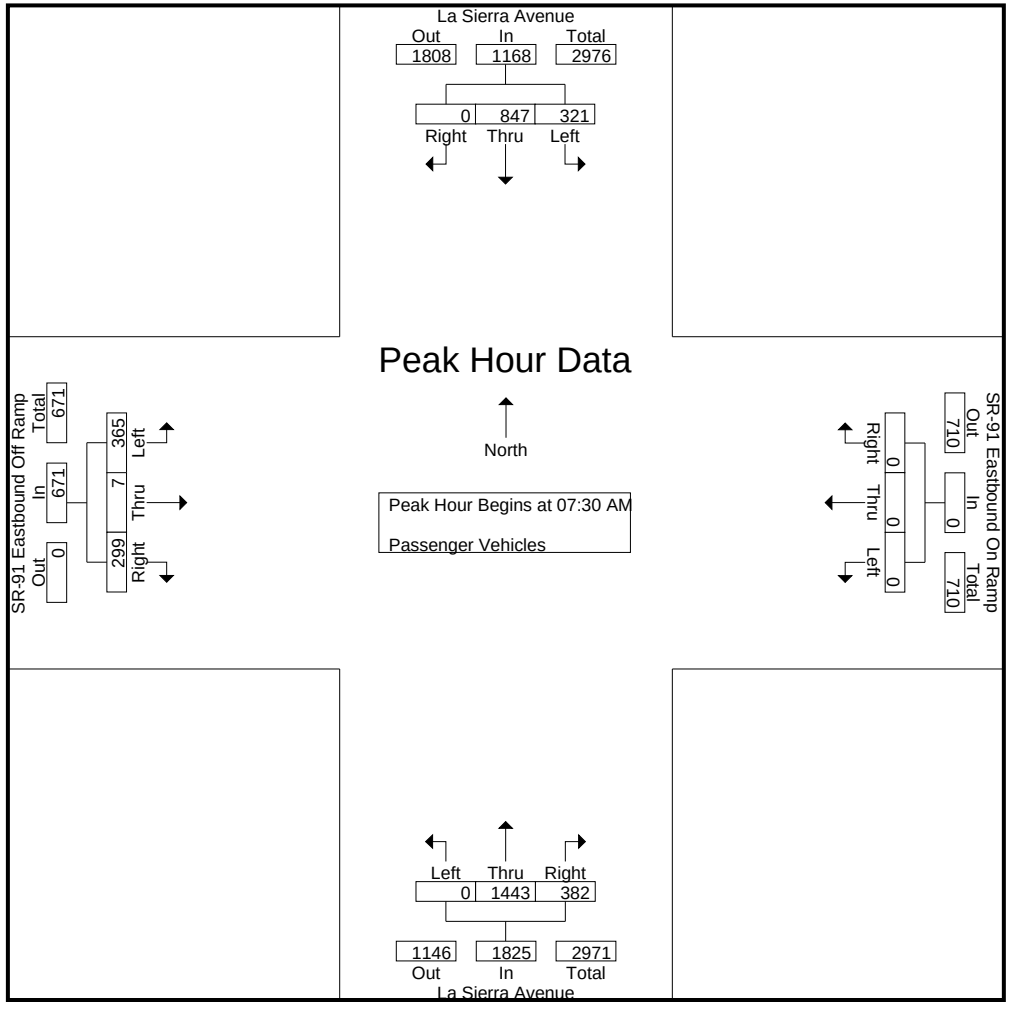
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	86	113	0	0	199	0	0	0	0	0	0	236	116	30	352	58	0	65	48	123	78	674	752
07:15 AM	103	147	0	0	250	0	0	0	0	0	0	295	112	45	407	72	0	61	41	133	86	790	876
07:30 AM	89	210	0	0	299	0	0	0	0	0	0	280	94	33	374	71	1	67	40	139	73	812	885
07:45 AM	63	270	0	0	333	0	0	0	0	0	0	349	87	37	436	95	2	93	57	190	94	959	1053
Total	341	740	0	0	1081	0	0	0	0	0	0	1160	409	145	1569	296	3	286	186	585	331	3235	3566
08:00 AM	83	211	0	0	294	0	0	0	0	0	0	435	92	37	527	106	4	74	52	184	89	1005	1094
08:15 AM	86	156	0	0	242	0	0	0	0	0	0	379	109	40	488	93	0	65	44	158	84	888	972
08:30 AM	62	131	0	0	193	0	0	0	0	0	0	294	105	43	399	80	0	77	51	157	94	749	843
08:45 AM	72	110	0	0	182	0	0	0	0	0	0	302	95	28	397	70	1	58	40	129	68	708	776
Total	303	608	0	0	911	0	0	0	0	0	0	1410	401	148	1811	349	5	274	187	628	335	3350	3685
Grand Total	644	1348	0	0	1992	0	0	0	0	0	0	2570	810	293	3380	645	8	560	373	1213	666	6585	7251
Apprch %	32.3	67.7	0			0	0	0			0	76	24			53.2	0.7	46.2					
Total %	9.8	20.5	0		30.3	0	0	0		0	0	39	12.3		51.3	9.8	0.1	8.5		18.4	9.2	90.8	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	**89**	210	0	299	0	0	0	0	0	280	94	374	71	1	67	139	812
07:45 AM	63	**270**	0	**333**	0	0	0	0	0	349	87	436	95	2	**93**	**190**	959
08:00 AM	83	211	0	294	0	0	0	0	0	**435**	92	**527**	**106**	**4**	74	184	**1005**
08:15 AM	86	156	0	242	0	0	0	0	0	379	**109**	488	93	0	65	158	888
Total Volume	321	847	0	1168	0	0	0	0	0	1443	382	1825	365	7	299	671	3664
% App. Total	27.5	72.5	0		0	0	0		0	79.1	20.9		54.4	1	44.6		
PHF	.902	.784	.000	.877	.000	.000	.000	.000	.000	.829	.876	.866	.861	.438	.804	.883	.911

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E AM
Site Code : 05123396
Start Date : 8/17/2023
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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E AM
Site Code : 05123396
Start Date : 8/17/2023
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	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	**89**	210	0	299	0	0	0	0	0	280	94	374	71	1	67	139	
+15 mins.	63	**270**	0	**333**	0	0	0	0	0	349	87	436	95	2	**93**	**190**	
+30 mins.	83	211	0	294	0	0	0	0	0	**435**	92	**527**	**106**	**4**	74	184	
+45 mins.	86	156	0	242	0	0	0	0	0	379	**109**	488	93	0	65	158	
Total Volume	321	847	0	1168	0	0	0	0	0	1443	382	1825	365	7	299	671	
% App. Total	27.5	72.5	0		0	0	0		0	79.1	20.9		54.4	1	44.6		
PHF	.902	.784	.000	.877	.000	.000	.000	.000	.000	.829	.876	.866	.861	.438	.804	.883	

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City of Riverside
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File Name : 02_RIV_La S_91E AM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- Large 2 Axle Vehicles

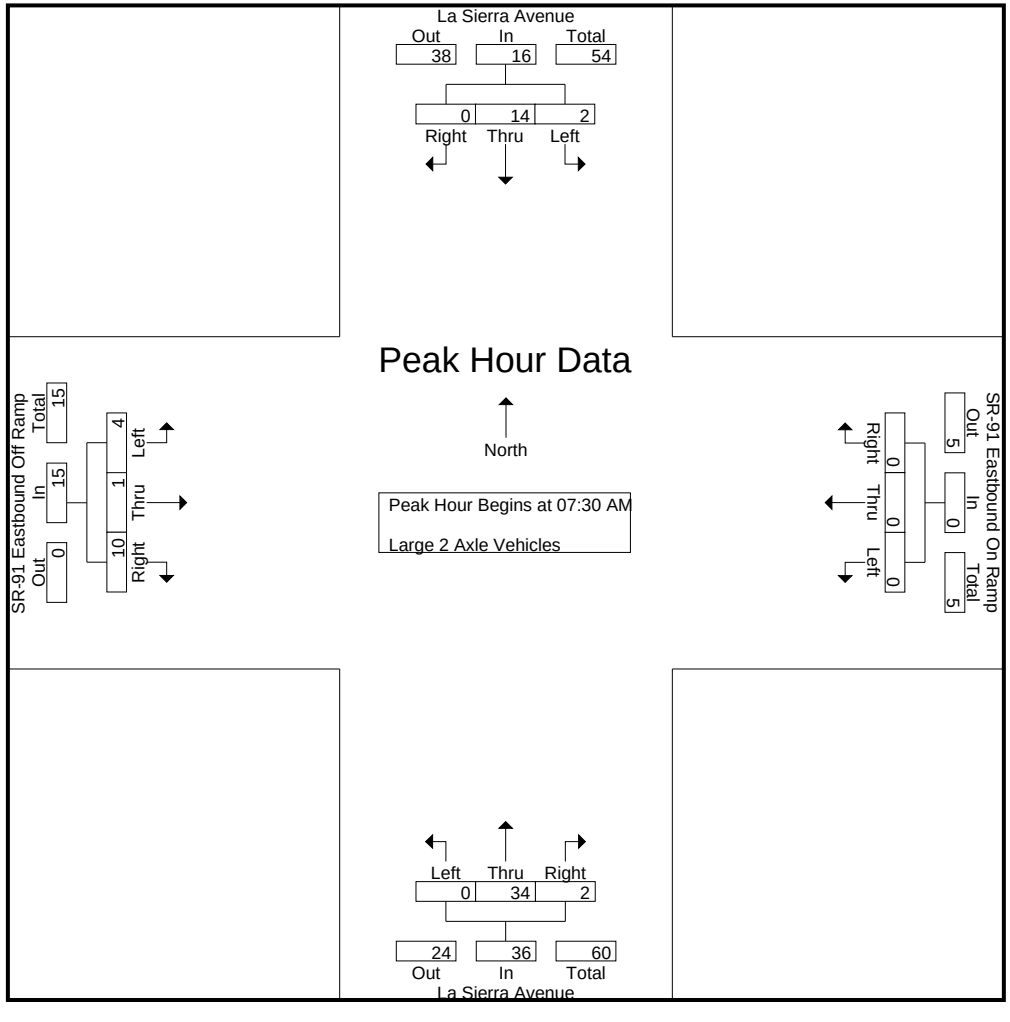
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	1	7	0	0	8	0	0	0	0	0	0	7	3	1	10	2	0	3	2	5	3	23	26
07:15 AM	1	3	0	0	4	0	0	0	0	0	0	3	0	0	3	0	0	2	2	2	2	9	11
07:30 AM	1	2	0	0	3	0	0	0	0	0	0	9	1	0	10	2	0	4	1	6	1	19	20
07:45 AM	1	4	0	0	5	0	0	0	0	0	0	7	1	0	8	1	0	2	0	3	0	16	16
Total	4	16	0	0	20	0	0	0	0	0	0	26	5	1	31	5	0	11	5	16	6	67	73
08:00 AM	0	2	0	0	2	0	0	0	0	0	0	7	0	0	7	0	1	3	1	4	1	13	14
08:15 AM	0	6	0	0	6	0	0	0	0	0	0	11	0	0	11	1	0	1	0	2	0	19	19
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	8	1	0	9	2	0	5	5	7	5	16	21
08:45 AM	1	1	0	0	2	0	0	0	0	0	0	0	3	0	3	3	0	5	2	8	2	13	15
Total	1	9	0	0	10	0	0	0	0	0	0	26	4	0	30	6	1	14	8	21	8	61	69
Grand Total	5	25	0	0	30	0	0	0	0	0	0	52	9	1	61	11	1	25	13	37	14	128	142
Apprch %	16.7	83.3	0			0	0	0			0	85.2	14.8			29.7	2.7	67.6					
Total %	3.9	19.5	0		23.4	0	0	0		0	0	40.6	7		47.7	8.6	0.8	19.5		28.9	9.9	90.1	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	**1**	2	0	3	0	0	0	0	0	9	**1**	10	**2**	0	**4**	**6**	**19**
07:45 AM	1	4	0	5	0	0	0	0	0	7	1	8	1	0	2	3	16
08:00 AM	0	2	0	2	0	0	0	0	0	7	0	7	0	**1**	3	4	13
08:15 AM	0	**6**	0	**6**	0	0	0	0	0	**11**	0	**11**	1	0	1	2	19
Total Volume	2	14	0	16	0	0	0	0	0	34	2	36	4	1	10	15	67
% App. Total	12.5	87.5	0		0	0	0		0	94.4	5.6		26.7	6.7	66.7		
PHF	.500	.583	.000	.667	.000	.000	.000	.000	.000	.773	.500	.818	.500	.250	.625	.625	.882

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
(951) 268-6268

City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E AM
Site Code : 05123396
Start Date : 8/17/2023
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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E AM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 3

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	**1**	2	0	3	0	0	0	0	0	9	**1**	10	**2**	0	**4**	**6**	
+15 mins.	1	4	0	5	0	0	0	0	0	7	1	8	1	0	2	3	
+30 mins.	0	2	0	2	0	0	0	0	0	7	0	7	0	**1**	3	4	
+45 mins.	0	**6**	0	**6**	0	0	0	0	0	**11**	0	**11**	1	0	1	2	
Total Volume	2	14	0	16	0	0	0	0	0	34	2	36	4	1	10	15	
% App. Total	12.5	87.5	0		0	0	0		0	94.4	5.6		26.7	6.7	66.7		
PHF	.500	.583	.000	.667	.000	.000	.000	.000	.000	.773	.500	.818	.500	.250	.625	.625	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E AM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- 3 Axle Vehicles

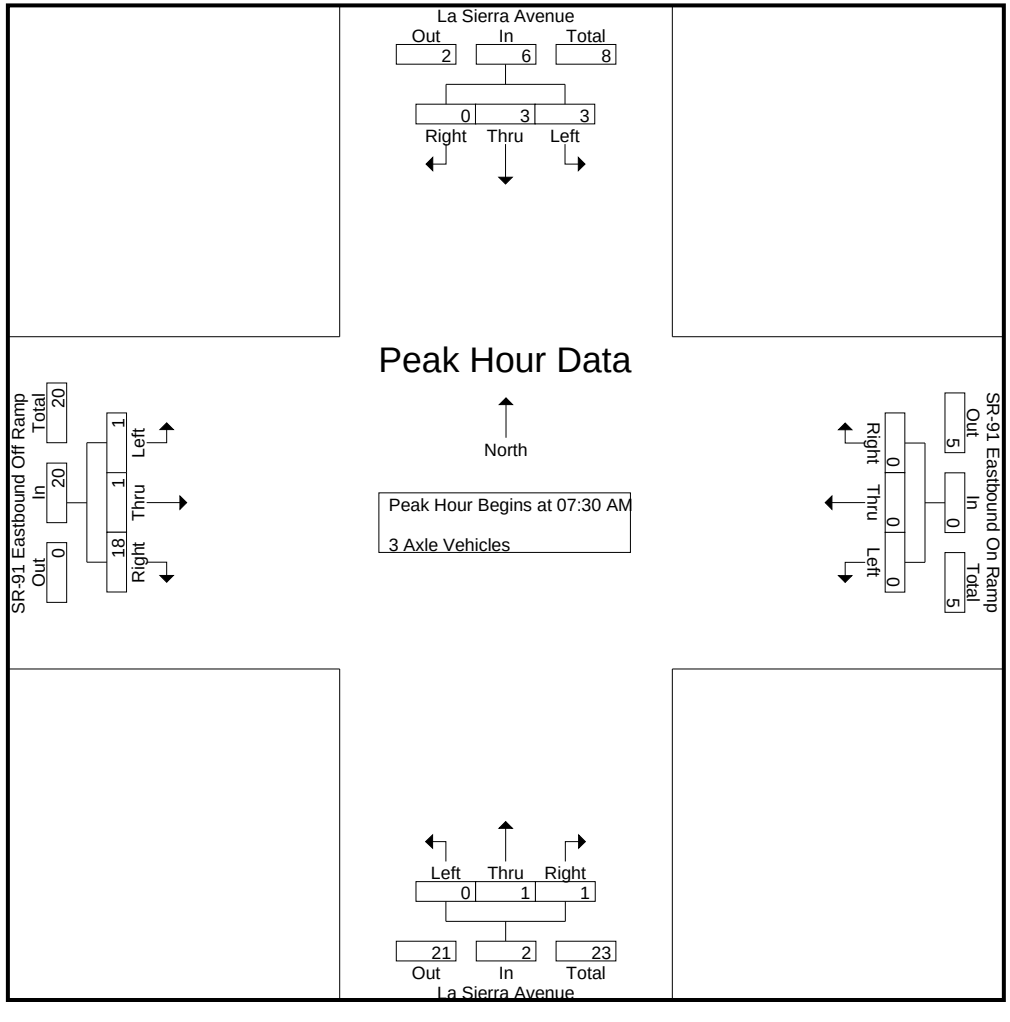
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	1	0	0	0	1	0	0	0	0	0	0	3	0	0	3	0	0	1	1	1	1	5	6
07:15 AM	1	1	0	0	2	0	0	0	0	0	0	0	0	0	0	1	0	2	1	3	1	5	6
07:30 AM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	5	4	5	4	6	10
07:45 AM	1	0	0	0	1	0	0	0	0	0	0	1	0	0	1	0	0	4	2	4	2	6	8
Total	3	2	0	0	5	0	0	0	0	0	0	4	0	0	4	1	0	12	8	13	8	22	30
08:00 AM	2	1	0	0	3	0	0	0	0	0	0	0	1	0	1	0	0	4	1	4	1	8	9
08:15 AM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1	5	3	7	3	8	11
08:30 AM	0	1	0	0	1	0	0	0	0	0	0	0	1	0	1	1	0	2	1	3	1	5	6
08:45 AM	1	0	0	0	1	0	0	0	0	0	0	4	1	0	5	0	0	0	0	0	0	6	6
Total	3	3	0	0	6	0	0	0	0	0	0	4	3	0	7	2	1	11	5	14	5	27	32
Grand Total	6	5	0	0	11	0	0	0	0	0	0	8	3	0	11	3	1	23	13	27	13	49	62
Apprch %	54.5	45.5	0			0	0	0			0	72.7	27.3			11.1	3.7	85.2					
Total %	12.2	10.2	0		22.4	0	0	0		0	0	16.3	6.1		22.4	6.1	2	46.9		55.1	21	79	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	0	**1**	0	1	0	0	0	0	0	0	0	0	0	0	**5**	5	6
07:45 AM	1	0	0	1	0	0	0	0	0	**1**	0	**1**	0	0	4	4	6
08:00 AM	**2**	1	0	**3**	0	0	0	0	0	0	**1**	1	0	0	4	4	**8**
08:15 AM	0	1	0	1	0	0	0	0	0	0	0	0	**1**	**1**	5	**7**	8
Total Volume	3	3	0	6	0	0	0	0	0	1	1	2	1	1	18	20	28
% App. Total	50	50	0		0	0	0		0	50	50		5	5	90		
PHF	.375	.750	.000	.500	.000	.000	.000	.000	.000	.250	.250	.500	.250	.250	.900	.714	.875

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E AM
Site Code : 05123396
Start Date : 8/17/2023
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	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	0	**1**	0	1	0	0	0	0	0	0	0	0	0	0	**5**	5	
+15 mins.	1	0	0	1	0	0	0	0	0	**1**	0	**1**	0	0	4	4	
+30 mins.	**2**	1	0	**3**	0	0	0	0	0	0	**1**	1	0	0	4	4	
+45 mins.	0	1	0	1	0	0	0	0	0	0	0	0	**1**	**1**	5	**7**	
Total Volume	3	3	0	6	0	0	0	0	0	1	1	2	1	1	18	20	
% App. Total	50	50	0		0	0	0		0	50	50		5	5	90		
PHF	.375	.750	.000	.500	.000	.000	.000	.000	.000	.250	.250	.500	.250	.250	.900	.714	

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City of Riverside
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File Name : 02_RIV_La S_91E AM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- 4+ Axle Trucks

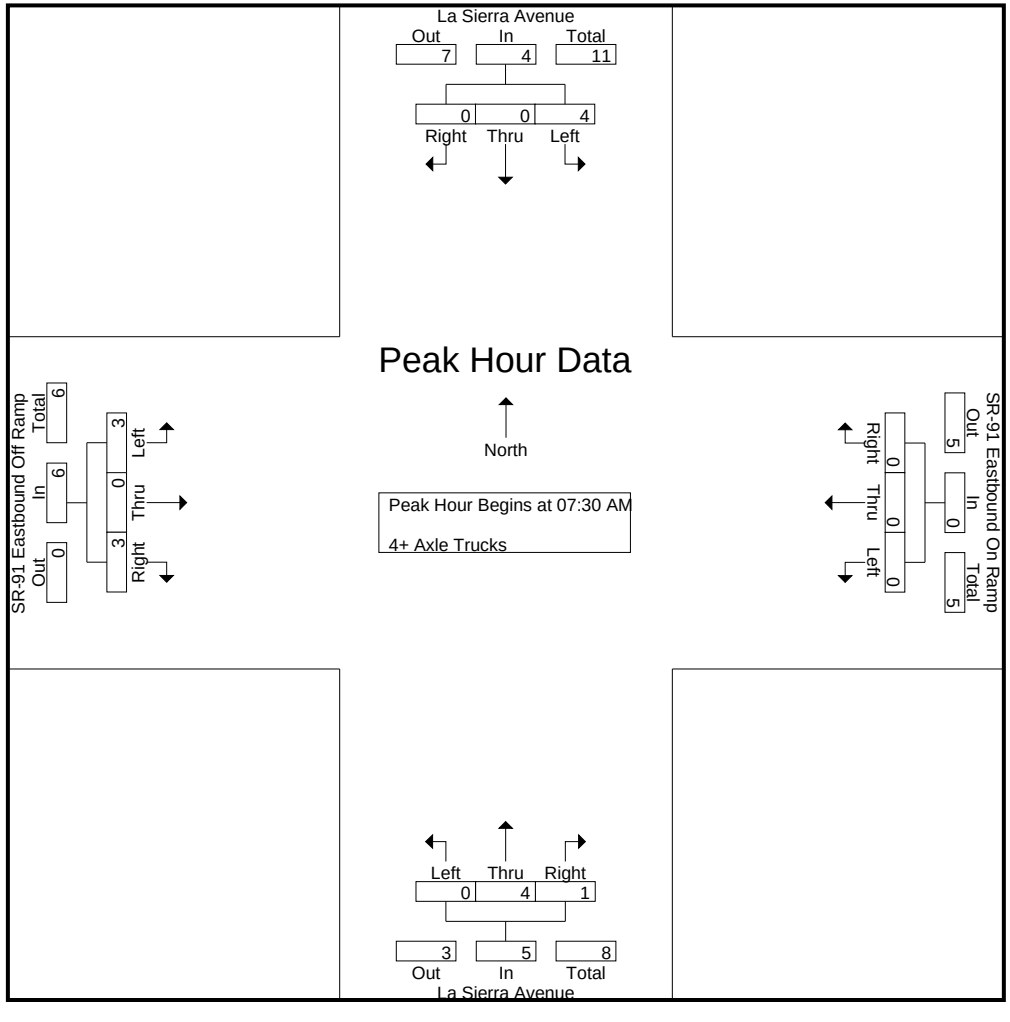
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	0	0	3	2	3	2	5	7
07:15 AM	1	0	0	0	1	0	0	0	0	0	0	1	0	0	1	1	0	1	1	2	1	4	5
07:30 AM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	1	2	3
07:45 AM	2	0	0	0	2	0	0	0	0	0	0	2	1	0	3	2	0	2	1	4	1	9	10
Total	4	1	0	0	5	0	0	0	0	0	0	4	1	0	5	3	0	7	5	10	5	20	25
08:00 AM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	2	2
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	2	2
08:30 AM	1	0	0	0	1	0	0	0	0	0	0	1	1	0	2	1	0	0	0	1	0	4	4
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	1	2	1	2	3
Total	2	0	0	0	2	0	0	0	0	0	0	3	1	0	4	2	0	2	1	4	1	10	11
Grand Total	6	1	0	0	7	0	0	0	0	0	0	7	2	0	9	5	0	9	6	14	6	30	36
Apprch %	85.7	14.3	0			0	0	0			0	77.8	22.2			35.7	0	64.3					
Total %	20	3.3	0		23.3	0	0	0		0	0	23.3	6.7		30	16.7	0	30		46.7	16.7	83.3	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1	2
07:45 AM	**2**	0	0	**2**	0	0	0	0	0	**2**	**1**	**3**	**2**	0	**2**	**4**	**9**
08:00 AM	1	0	0	1	0	0	0	0	0	0	0	0	1	0	0	1	2
08:15 AM	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	2
Total Volume	4	0	0	4	0	0	0	0	0	4	1	5	3	0	3	6	15
% App. Total	100	0	0		0	0	0		0	80	20		50	0	50		
PHF	.500	.000	.000	.500	.000	.000	.000	.000	.000	.500	.250	.417	.375	.000	.375	.375	.417

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E AM
Site Code : 05123396
Start Date : 8/17/2023
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City of Riverside
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	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1	
+15 mins.	**2**	0	0	**2**	0	0	0	0	0	**2**	**1**	**3**	**2**	0	**2**	**4**	
+30 mins.	1	0	0	1	0	0	0	0	0	0	0	0	1	0	0	1	
+45 mins.	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	
Total Volume	4	0	0	4	0	0	0	0	0	4	1	5	3	0	3	6	
% App. Total	100	0	0		0	0	0		0	80	20		50	0	50		
PHF	.500	.000	.000	.500	.000	.000	.000	.000	.000	.500	.250	.417	.375	.000	.375	.375	

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File Name : 02_RIV_La S_91E PM
Site Code : 05123396
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Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

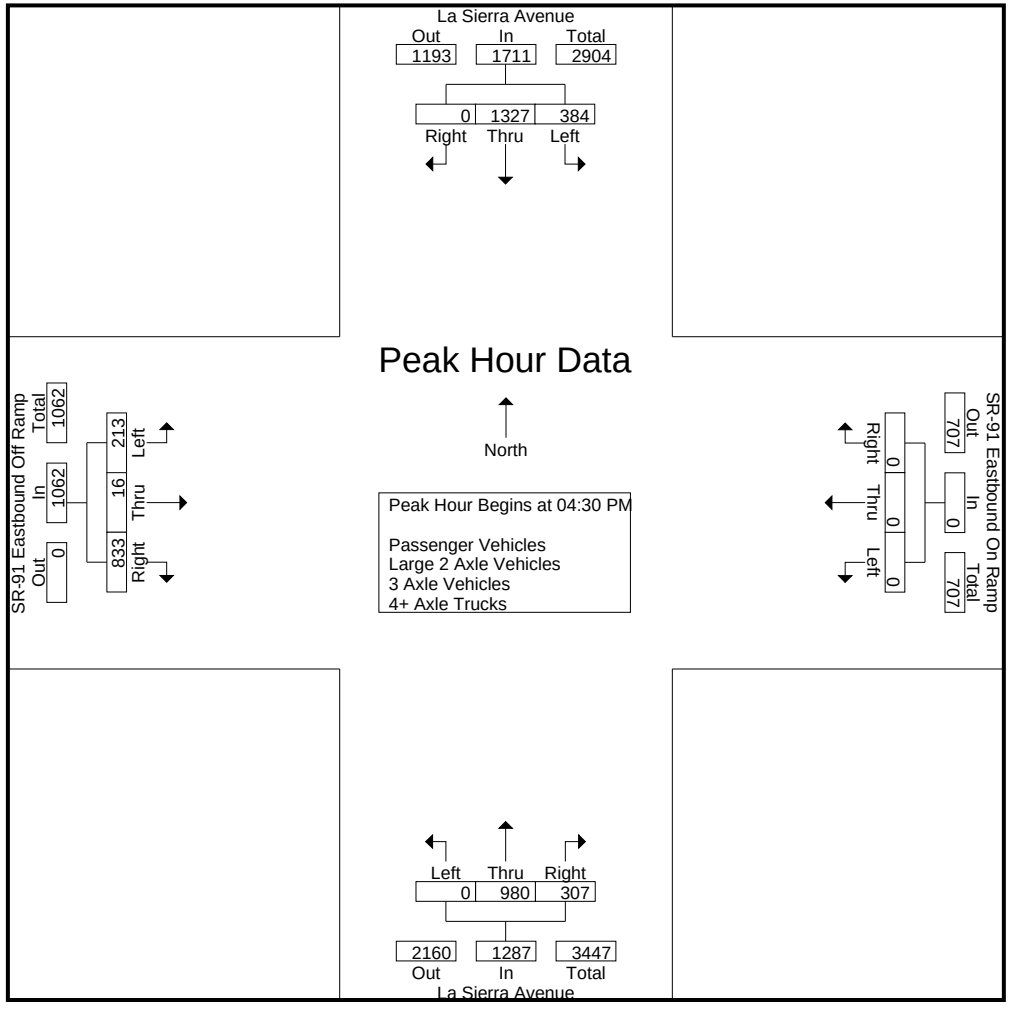
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	91	317	0	0	408	0	0	0	0	0	0	252	74	13	326	63	1	235	53	299	66	1033	1099
04:15 PM	87	285	0	0	372	0	0	0	0	0	0	231	72	9	303	47	2	201	57	250	66	925	991
04:30 PM	86	289	0	0	375	0	0	0	0	0	0	230	89	8	319	54	4	227	73	285	81	979	1060
04:45 PM	68	319	0	0	387	0	0	0	0	0	0	230	85	7	315	52	2	211	68	265	75	967	1042
Total	332	1210	0	0	1542	0	0	0	0	0	0	943	320	37	1263	216	9	874	251	1099	288	3904	4192
05:00 PM	127	343	0	0	470	0	0	0	0	0	0	264	64	8	328	55	2	202	47	259	55	1057	1112
05:15 PM	103	376	0	0	479	0	0	0	0	0	0	256	69	8	325	52	8	193	46	253	54	1057	1111
05:30 PM	94	314	0	0	408	0	0	0	0	0	0	250	82	16	332	50	1	175	46	226	62	966	1028
05:45 PM	71	288	0	0	359	0	0	0	0	0	0	218	62	9	280	46	4	188	68	238	77	877	954
Total	395	1321	0	0	1716	0	0	0	0	0	0	988	277	41	1265	203	15	758	207	976	248	3957	4205
Grand Total	727	2531	0	0	3258	0	0	0	0	0	0	1931	597	78	2528	419	24	1632	458	2075	536	7861	8397
Apprch %	22.3	77.7	0			0	0	0			0	76.4	23.6			20.2	1.2	78.7					
Total %	9.2	32.2	0		41.4	0	0	0		0	0	24.6	7.6		32.2	5.3	0.3	20.8		26.4	6.4	93.6	
Passenger Vehicles	718	2493	0		3211	0	0	0		0	0	1888	583		2548	411	23	1581		2460	0	0	8219
% Passenger Vehicles	98.8	98.5	0	0	98.6	0	0	0	0	0	0	97.8	97.7	98.7	97.8	98.1	95.8	96.9	97.2	97.1	0	0	97.9
Large 2 Axle Vehicles	5	32	0		37	0	0	0		0	0	36	7		43	4	1	39		55	0	0	135
% Large 2 Axle Vehicles	0.7	1.3	0	0	1.1	0	0	0	0	0	0	1.9	1.2	0	1.7	1	4.2	2.4	2.4	2.2	0	0	1.6
3 Axle Vehicles	3	4	0		7	0	0	0		0	0	6	3		10	1	0	6		9	0	0	26
% 3 Axle Vehicles	0.4	0.2	0	0	0.2	0	0	0	0	0	0	0.3	0.5	1.3	0.4	0.2	0	0.4	0.4	0.4	0	0	0.3
4+ Axle Trucks	1	2	0		3	0	0	0		0	0	1	4		5	3	0	6		9	0	0	17
% 4+ Axle Trucks	0.1	0.1	0	0	0.1	0	0	0	0	0	0	0.1	0.7	0	0.2	0.7	0	0.4	0	0.4	0	0	0.2

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 PM																	
04:30 PM	86	289	0	375	0	0	0	0	0	230	**89**	319	54	4	**227**	**285**	979
04:45 PM	68	319	0	387	0	0	0	0	0	230	85	315	52	2	211	265	967
05:00 PM	**127**	343	0	470	0	0	0	0	0	**264**	64	**328**	**55**	2	202	259	**1057**
05:15 PM	103	**376**	0	**479**	0	0	0	0	0	256	69	325	52	**8**	193	253	1057
Total Volume	384	1327	0	1711	0	0	0	0	0	980	307	1287	213	16	833	1062	4060
% App. Total	22.4	77.6	0		0	0	0		0	76.1	23.9		20.1	1.5	78.4		
PHF	.756	.882	.000	.893	.000	.000	.000	.000	.000	.928	.862	.981	.968	.500	.917	.932	.960

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E PM
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	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:00 PM				04:45 PM				04:00 PM				
+0 mins.	68	319	0	387	0	0	0	0	0	230	**85**	315	**63**	1	**235**	**299**	
+15 mins.	**127**	343	0	470	0	0	0	0	0	**264**	64	328	47	2	201	250	
+30 mins.	103	**376**	0	**479**	0	0	0	0	0	256	69	325	54	**4**	227	285	
+45 mins.	94	314	0	408	0	0	0	0	0	250	82	**332**	52	2	211	265	
Total Volume	392	1352	0	1744	0	0	0	0	0	1000	300	1300	216	9	874	1099	
% App. Total	22.5	77.5	0		0	0	0		0	76.9	23.1		19.7	0.8	79.5		
PHF	.772	.899	.000	.910	.000	.000	.000	.000	.000	.947	.882	.979	.857	.563	.930	.919	

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Groups Printed- Passenger Vehicles

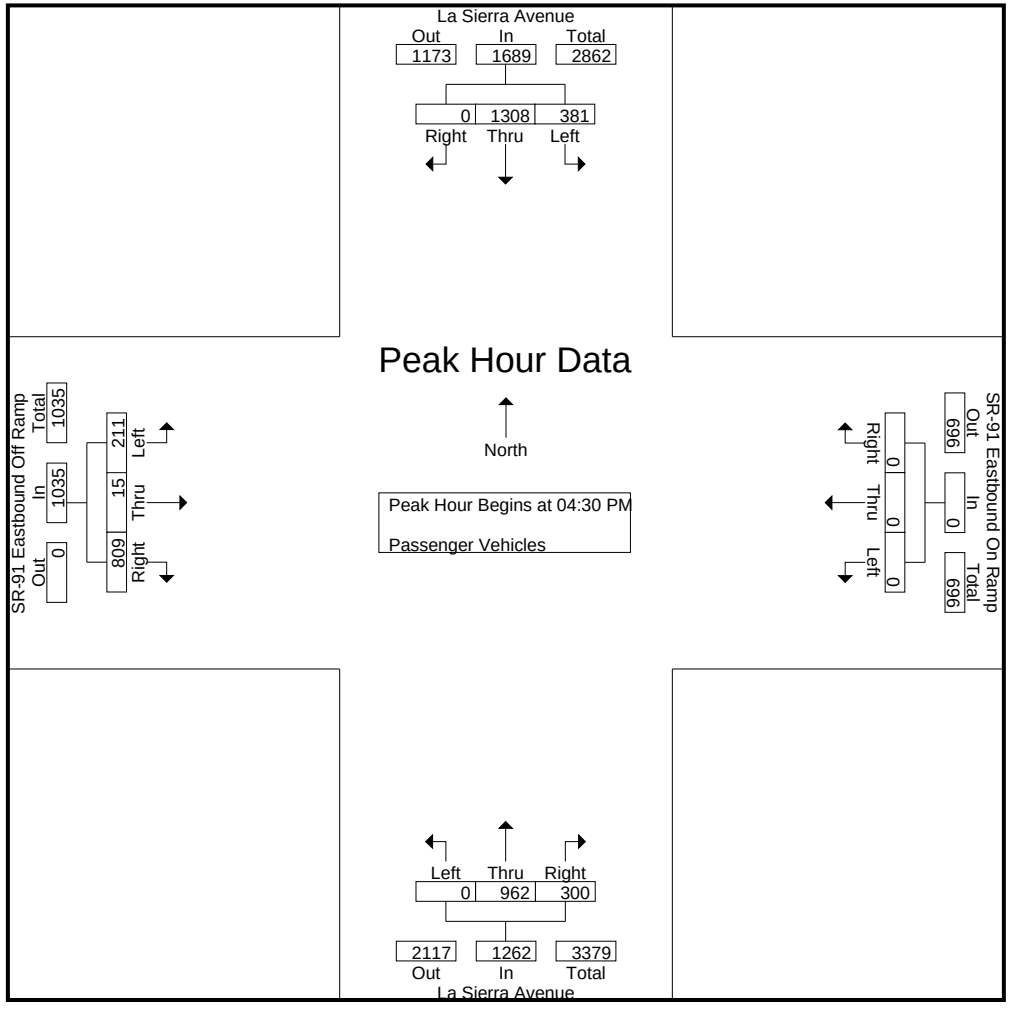
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	88	308	0	0	396	0	0	0	0	0	0	247	74	13	321	62	1	223	48	286	61	1003	1064
04:15 PM	85	279	0	0	364	0	0	0	0	0	0	221	68	9	289	45	2	193	56	240	65	893	958
04:30 PM	86	284	0	0	370	0	0	0	0	0	0	224	85	8	309	53	3	224	73	280	81	959	1040
04:45 PM	68	317	0	0	385	0	0	0	0	0	0	228	83	7	311	51	2	202	65	255	72	951	1023
Total	327	1188	0	0	1515	0	0	0	0	0	0	920	310	37	1230	211	8	842	242	1061	279	3806	4085
05:00 PM	125	336	0	0	461	0	0	0	0	0	0	259	64	8	323	55	2	194	45	251	53	1035	1088
05:15 PM	102	371	0	0	473	0	0	0	0	0	0	251	68	7	319	52	8	189	45	249	52	1041	1093
05:30 PM	94	311	0	0	405	0	0	0	0	0	0	247	81	16	328	48	1	171	45	220	61	953	1014
05:45 PM	70	287	0	0	357	0	0	0	0	0	0	211	60	9	271	45	4	185	68	234	77	862	939
Total	391	1305	0	0	1696	0	0	0	0	0	0	968	273	40	1241	200	15	739	203	954	243	3891	4134
Grand Total	718	2493	0	0	3211	0	0	0	0	0	0	1888	583	77	2471	411	23	1581	445	2015	522	7697	8219
Apprch %	22.4	77.6	0			0	0	0			0	76.4	23.6			20.4	1.1	78.5					
Total %	9.3	32.4	0		41.7	0	0	0		0	0	24.5	7.6		32.1	5.3	0.3	20.5		26.2	6.4	93.6	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:30 PM to 05:15 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 PM																	
04:30 PM	86	284	0	370	0	0	0	0	0	224	**85**	309	53	3	**224**	**280**	959
04:45 PM	68	317	0	385	0	0	0	0	0	228	83	311	51	2	202	255	951
05:00 PM	**125**	336	0	461	0	0	0	0	0	**259**	64	**323**	**55**	2	194	251	1035
05:15 PM	102	**371**	0	**473**	0	0	0	0	0	251	68	319	52	**8**	189	249	**1041**
Total Volume	381	1308	0	1689	0	0	0	0	0	962	300	1262	211	15	809	1035	3986
% App. Total	22.6	77.4	0		0	0	0		0	76.2	23.8		20.4	1.4	78.2		
PHF	.762	.881	.000	.893	.000	.000	.000	.000	.000	.929	.882	.977	.959	.469	.903	.924	.957

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
(951) 268-6268

City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 3

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:30 PM to 05:15 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:30 PM				04:30 PM				04:30 PM				04:30 PM				
+0 mins.	86	284	0	370	0	0	0	0	0	224	**85**	309	53	3	**224**	**280**	
+15 mins.	68	317	0	385	0	0	0	0	0	228	83	311	51	2	202	255	
+30 mins.	**125**	336	0	461	0	0	0	0	0	**259**	64	**323**	**55**	2	194	251	
+45 mins.	102	**371**	0	**473**	0	0	0	0	0	251	68	319	52	**8**	189	249	
Total Volume	381	1308	0	1689	0	0	0	0	0	962	300	1262	211	15	809	1035	
% App. Total	22.6	77.4	0		0	0	0		0	76.2	23.8		20.4	1.4	78.2		
PHF	.762	.881	.000	.893	.000	.000	.000	.000	.000	.929	.882	.977	.959	.469	.903	.924	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- Large 2 Axle Vehicles

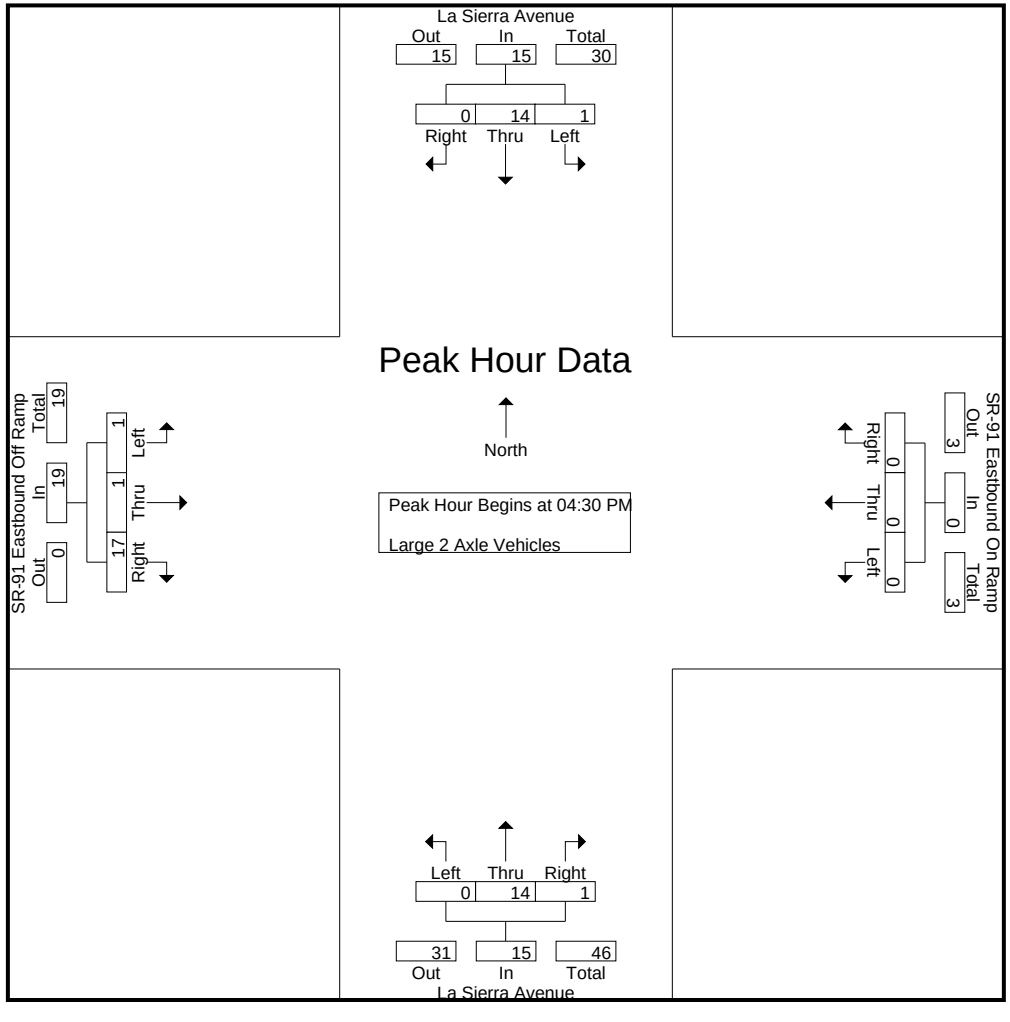
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	1	9	0	0	10	0	0	0	0	0	0	4	0	0	4	0	0	11	5	11	5	25	30
04:15 PM	2	6	0	0	8	0	0	0	0	0	0	9	4	0	13	1	0	6	1	7	1	28	29
04:30 PM	0	3	0	0	3	0	0	0	0	0	0	4	1	0	5	1	1	3	0	5	0	13	13
04:45 PM	0	2	0	0	2	0	0	0	0	0	0	2	0	0	2	0	0	4	1	4	1	8	9
Total	3	20	0	0	23	0	0	0	0	0	0	19	5	0	24	2	1	24	7	27	7	74	81
05:00 PM	0	5	0	0	5	0	0	0	0	0	0	5	0	0	5	0	0	7	2	7	2	17	19
05:15 PM	1	4	0	0	5	0	0	0	0	0	0	3	0	0	3	0	0	3	1	3	1	11	12
05:30 PM	0	2	0	0	2	0	0	0	0	0	0	3	0	0	3	1	0	4	1	5	1	10	11
05:45 PM	1	1	0	0	2	0	0	0	0	0	0	6	2	0	8	1	0	1	0	2	0	12	12
Total	2	12	0	0	14	0	0	0	0	0	0	17	2	0	19	2	0	15	4	17	4	50	54
Grand Total	5	32	0	0	37	0	0	0	0	0	0	36	7	0	43	4	1	39	11	44	11	124	135
Apprch %	13.5	86.5	0			0	0	0			0	83.7	16.3			9.1	2.3	88.6					
Total %	4	25.8	0		29.8	0	0	0		0	0	29	5.6		34.7	3.2	0.8	31.5		35.5	8.1	91.9	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:30 PM to 05:15 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 PM																	
04:30 PM	0	3	0	3	0	0	0	0	0	4	**1**	**5**	**1**	**1**	3	5	13
04:45 PM	0	2	0	2	0	0	0	0	0	2	0	2	0	0	4	4	8
05:00 PM	0	**5**	0	**5**	0	0	0	0	0	**5**	0	5	0	0	**7**	**7**	**17**
05:15 PM	**1**	4	0	5	0	0	0	0	0	3	0	3	0	0	3	3	11
Total Volume	1	14	0	15	0	0	0	0	0	14	1	15	1	1	17	19	49
% App. Total	6.7	93.3	0		0	0	0		0	93.3	6.7		5.3	5.3	89.5		
PHF	.250	.700	.000	.750	.000	.000	.000	.000	.000	.700	.250	.750	.250	.250	.607	.679	.721

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E PM
Site Code : 05123396
Start Date : 8/17/2023

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:30 PM to 05:15 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:30 PM				04:30 PM				04:30 PM				04:30 PM				
+0 mins.	0	3	0	3	0	0	0	0	0	4	**1**	**5**	**1**	**1**	3	5	
+15 mins.	0	2	0	2	0	0	0	0	0	2	0	2	0	0	4	4	
+30 mins.	0	**5**	0	**5**	0	0	0	0	0	**5**	0	5	0	0	**7**	**7**	
+45 mins.	**1**	4	0	5	0	0	0	0	0	3	0	3	0	0	3	3	
Total Volume	1	14	0	15	0	0	0	0	0	14	1	15	1	1	17	19	
% App. Total	6.7	93.3	0		0	0	0		0	93.3	6.7		5.3	5.3	89.5		
PHF	.250	.700	.000	.750	.000	.000	.000	.000	.000	.700	.250	.750	.250	.250	.607	.679	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- 3 Axle Vehicles

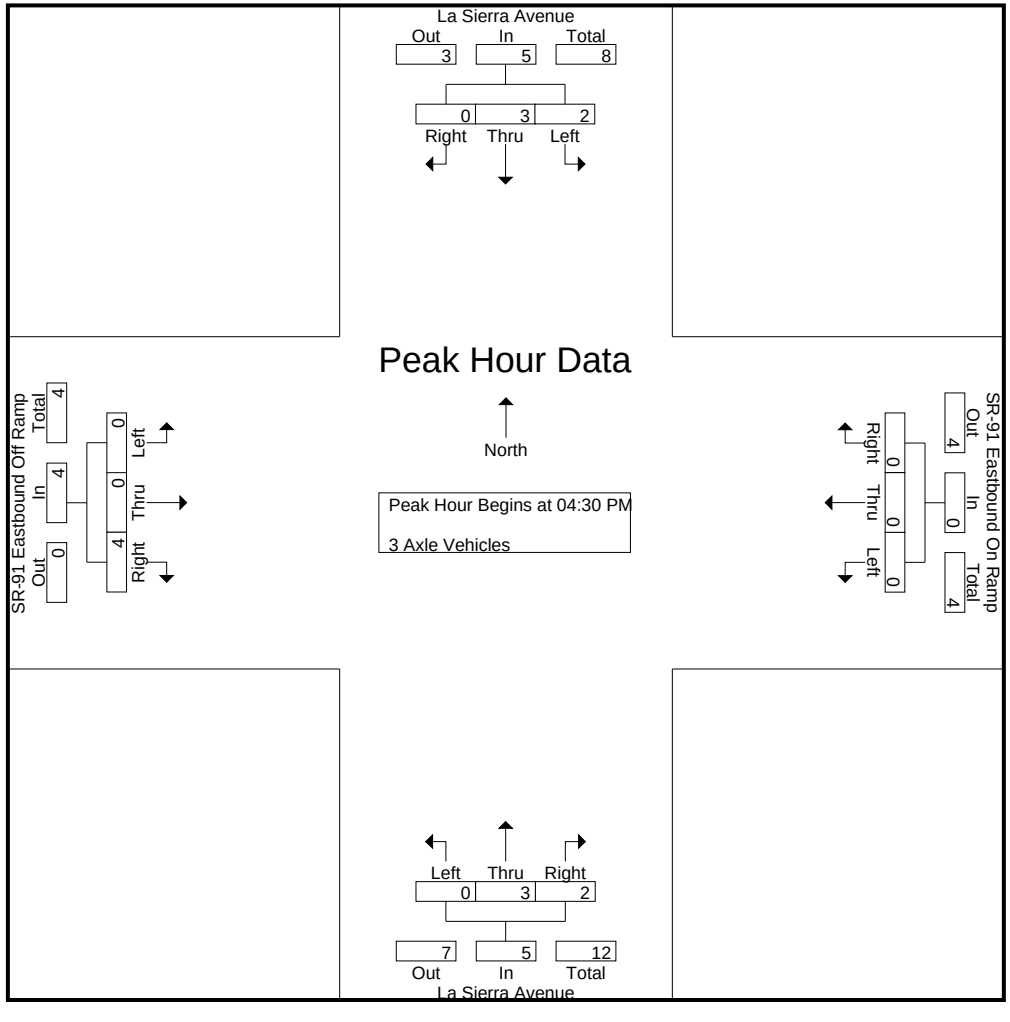
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	1	0	0	0	1	0	0	0	0	0	0	1	0	0	1	1	0	0	0	1	0	3	3
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	1	0	1	0	2	2
04:30 PM	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	2	2
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	3	2	3	2	4	6
Total	1	1	0	0	2	0	0	0	0	0	0	3	1	0	4	1	0	4	2	5	2	11	13
05:00 PM	2	2	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	5	5
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	2	1	1	3	0	0	0	0	0	1	3	4
05:30 PM	0	1	0	0	1	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	2	2
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	1	0	1	0	2	2
Total	2	3	0	0	5	0	0	0	0	0	0	3	2	1	5	0	0	2	0	2	1	12	13
Grand Total	3	4	0	0	7	0	0	0	0	0	0	6	3	1	9	1	0	6	2	7	3	23	26
Apprch %	42.9	57.1	0			0	0	0			0	66.7	33.3			14.3	0	85.7					
Total %	13	17.4	0		30.4	0	0	0		0	0	26.1	13		39.1	4.3	0	26.1		30.4	11.5	88.5	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:30 PM to 05:15 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 PM																	
04:30 PM	0	1	0	1	0	0	0	0	0	1	0	1	0	0	0	0	2
04:45 PM	0	0	0	0	0	0	0	0	0	0	**1**	1	0	0	**3**	**3**	4
05:00 PM	**2**	**2**	0	**4**	0	0	0	0	0	0	0	0	0	0	1	1	**5**
05:15 PM	0	0	0	0	0	0	0	0	0	**2**	1	**3**	0	0	0	0	3
Total Volume	2	3	0	5	0	0	0	0	0	3	2	5	0	0	4	4	14
% App. Total	40	60	0		0	0	0		0	60	40		0	0	100		
PHF	.250	.375	.000	.313	.000	.000	.000	.000	.000	.375	.500	.417	.000	.000	.333	.333	.700

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 2



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Site Code : 05123396
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	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:30 PM to 05:15 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:30 PM				04:30 PM				04:30 PM				04:30 PM				
+0 mins.	0	1	0	1	0	0	0	0	0	1	0	1	0	0	0	0	
+15 mins.	0	0	0	0	0	0	0	0	0	0	**1**	1	0	0	**3**	**3**	
+30 mins.	**2**	**2**	0	**4**	0	0	0	0	0	0	0	0	0	0	1	1	
+45 mins.	0	0	0	0	0	0	0	0	0	**2**	1	**3**	0	0	0	0	
Total Volume	2	3	0	5	0	0	0	0	0	3	2	5	0	0	4	4	
% App. Total	40	60	0		0	0	0		0	60	40		0	0	100		
PHF	.250	.375	.000	.313	.000	.000	.000	.000	.000	.375	.500	.417	.000	.000	.333	.333	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 1

Groups Printed- 4+ Axle Trucks

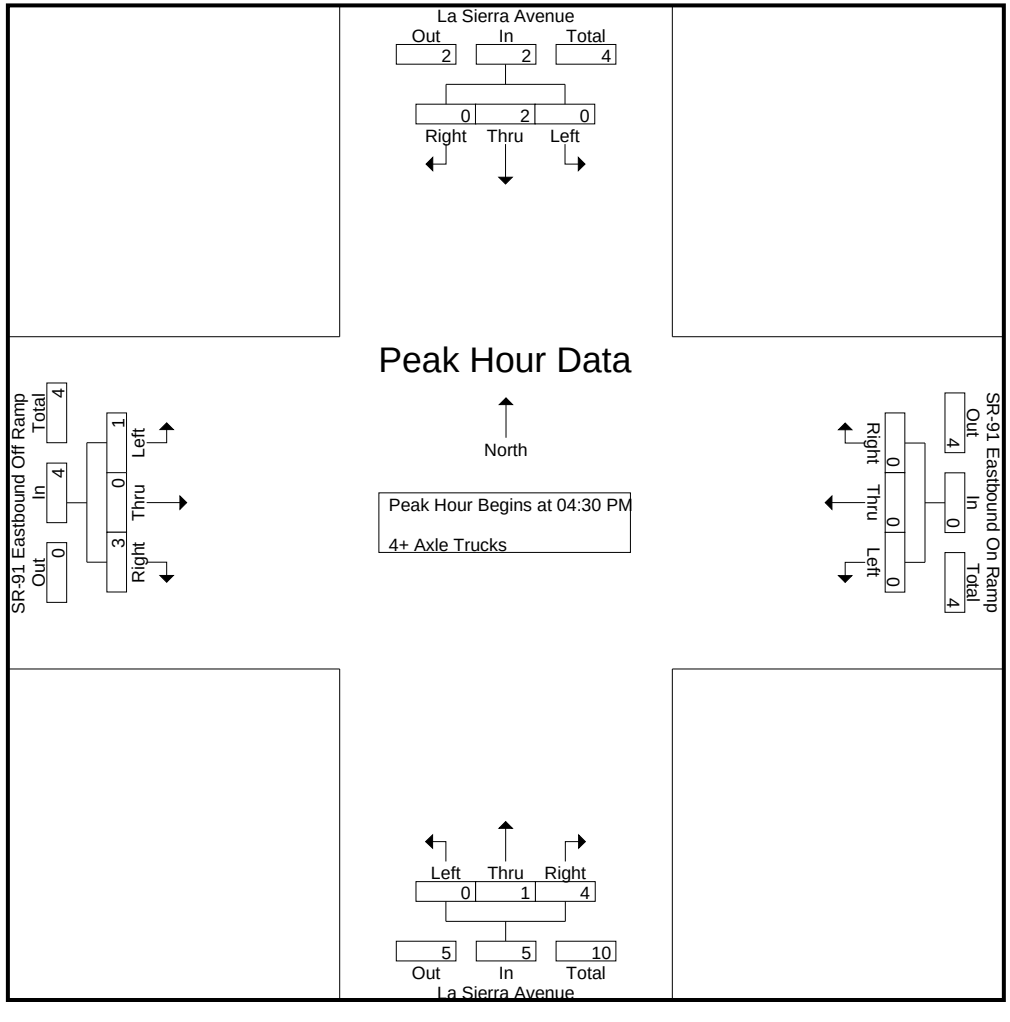
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	2	2
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	2	0	2	2
04:30 PM	0	1	0	0	1	0	0	0	0	0	0	1	3	0	4	0	0	0	0	0	0	5	5
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	0	2	0	3	0	4	4
Total	1	1	0	0	2	0	0	0	0	0	0	1	4	0	5	2	0	4	0	6	0	13	13
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	2	2
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	1	1
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	1	1
Total	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1	0	2	0	3	0	4	4
Grand Total	1	2	0	0	3	0	0	0	0	0	0	1	4	0	5	3	0	6	0	9	0	17	17
Apprch %	33.3	66.7	0			0	0	0			0	20	80			33.3	0	66.7					
Total %	5.9	11.8	0		17.6	0	0	0		0	0	5.9	23.5		29.4	17.6	0	35.3		52.9	0	100	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:30 PM to 05:15 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 PM																	
04:30 PM	0	**1**	0	**1**	0	0	0	0	0	**1**	**3**	**4**	0	0	0	0	**5**
04:45 PM	0	0	0	0	0	0	0	0	0	0	1	1	**1**	0	**2**	**3**	4
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	1	1	2
Total Volume	0	2	0	2	0	0	0	0	0	1	4	5	1	0	3	4	11
% App. Total	0	100	0		0	0	0		0	20	80		25	0	75		
PHF	.000	.500	.000	.500	.000	.000	.000	.000	.000	.250	.333	.313	.250	.000	.375	.333	.550

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E PM
Site Code : 05123396
Start Date : 8/17/2023
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_La S_91E PM
Site Code : 05123396
Start Date : 8/17/2023

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:30 PM to 05:15 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:30 PM				04:30 PM				04:30 PM				04:30 PM				
+0 mins.	0	**1**	0	**1**	0	0	0	0	0	**1**	**3**	**4**	0	0	0	0	
+15 mins.	0	0	0	0	0	0	0	0	0	0	1	1	**1**	0	**2**	**3**	
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
+45 mins.	0	1	0	1	0	0	0	0	0	0	0	0	0	0	1	1	
Total Volume	0	2	0	2	0	0	0	0	0	1	4	5	1	0	3	4	
% App. Total	0	100	0		0	0	0		0	20	80		25	0	75		
PHF	.000	.500	.000	.500	.000	.000	.000	.000	.000	.250	.333	.313	.250	.000	.375	.333	

Location: Riverside
N/S: La Sierra Avenue
E/W: SR-91 EB Ramps

Date: 8/17/2023
Day: Thursday

PEDESTRIANS

	North Leg La Sierra Avenue	East Leg SR-91 EB Ramps	South Leg La Sierra Avenue	West Leg SR-91 EB Ramps	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	0	3	0	3	6
7:15 AM	0	2	0	4	6
7:30 AM	0	0	0	1	1
7:45 AM	0	3	0	1	4
8:00 AM	0	1	0	2	3
8:15 AM	0	1	0	0	1
8:30 AM	0	0	0	0	0
8:45 AM	0	1	0	3	4
TOTAL VOLUMES:	0	11	0	14	25

	North Leg La Sierra Avenue	East Leg SR-91 EB Ramps	South Leg La Sierra Avenue	West Leg SR-91 EB Ramps	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	0	2	0	1	3
4:15 PM	0	2	0	2	4
4:30 PM	0	0	0	1	1
4:45 PM	0	1	0	3	4
5:00 PM	0	0	0	3	3
5:15 PM	0	0	0	1	1
5:30 PM	0	0	0	1	1
5:45 PM	0	0	0	1	1
TOTAL VOLUMES:	0	5	0	13	18

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Location: Riverside
N/S: La Sierra Avenue
E/W: SR-91 EB Ramps

Date: 8/17/2023
Day: Thursday

BICYCLES

	Southbound La Sierra Avenue			Westbound SR-91 EB Ramps			Northbound La Sierra Avenue			Eastbound SR-91 EB Ramps			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	2	0	0	0	0	0	1	0	0	0	0	3
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	1	0	0	0	0	1
8:45 AM	0	0	0	0	0	0	0	1	0	0	0	0	1
TOTAL VOLUMES:	0	2	0	0	0	0	0	3	0	0	0	0	5

	Southbound La Sierra Avenue			Westbound SR-91 EB Ramps			Northbound La Sierra Avenue			Eastbound SR-91 EB Ramps			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	1	0	0	0	0	1
4:45 PM	0	0	0	0	0	0	0	1	0	0	0	0	1
5:00 PM	0	1	0	0	0	0	0	2	0	0	0	0	3
5:15 PM	0	2	0	0	0	0	0	0	0	0	0	0	2
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	1	0	0	0	0	0	1	0	0	0	0	2
TOTAL VOLUMES:	0	4	0	0	0	0	0	5	0	0	0	0	9

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City of Riverside
N/S: La Sierra Avenue
E/W: Indiana Avenue
Weather: Clear

File Name : 03_RIV_La S_Indi AM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 1

Groups Printed- Total Volume

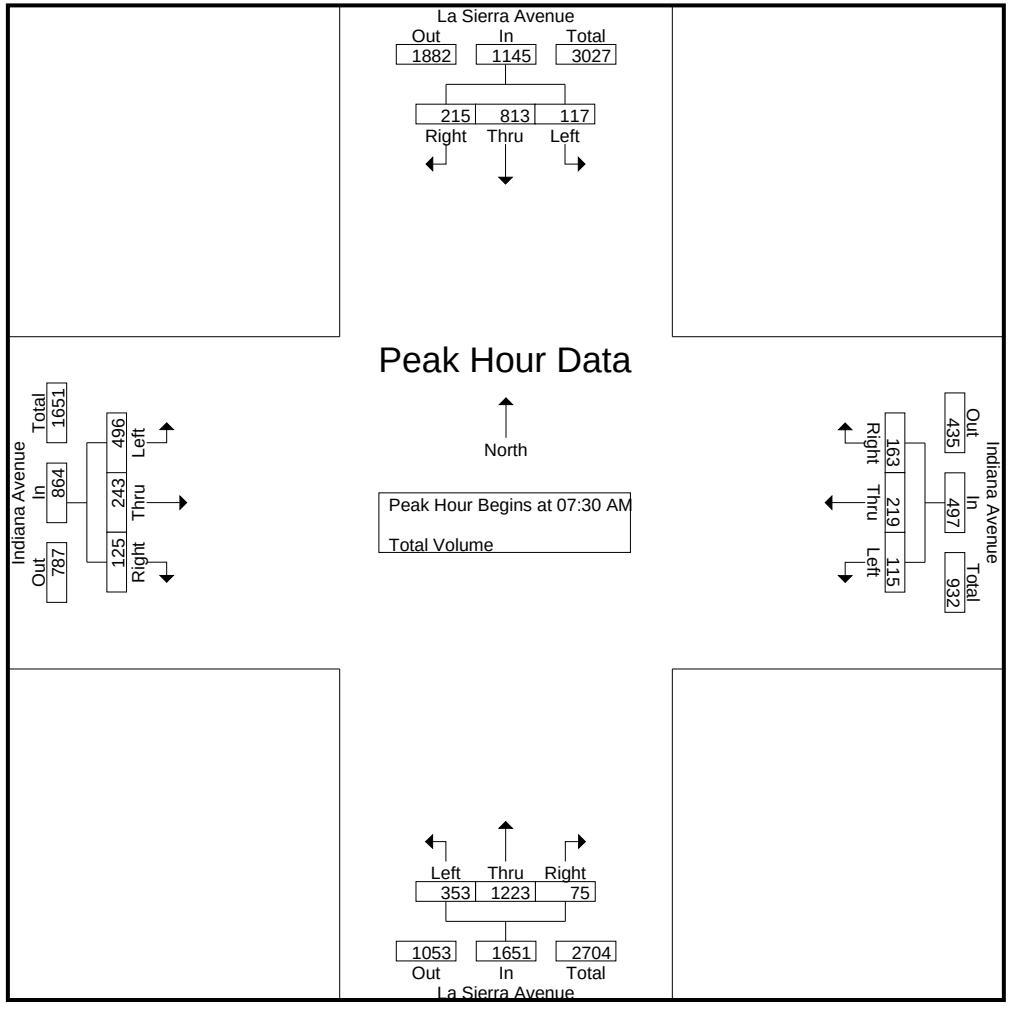
	La Sierra Avenue Southbound					Indiana Avenue Westbound					La Sierra Avenue Northbound					Indiana Avenue Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	21	117	28	13	166	18	33	36	18	87	53	263	8	2	324	67	30	15	12	112	45	689	734
07:15 AM	36	125	40	28	201	22	45	43	20	110	62	284	8	5	354	61	32	18	11	111	64	776	840
07:30 AM	30	201	41	19	272	33	39	39	31	111	75	257	19	8	351	109	36	28	22	173	80	907	987
07:45 AM	27	276	50	22	353	31	54	40	15	125	94	343	14	5	451	114	44	21	11	179	53	1108	1161
Total	114	719	159	82	992	104	171	158	84	433	284	1147	49	20	1480	351	142	82	56	575	242	3480	3722
08:00 AM	33	196	64	43	293	24	58	45	15	127	103	327	27	19	457	150	61	41	20	252	97	1129	1226
08:15 AM	27	140	60	47	227	27	68	39	22	134	81	296	15	7	392	123	102	35	18	260	94	1013	1107
08:30 AM	39	129	38	27	206	15	54	27	16	96	57	299	12	4	368	124	44	31	15	199	62	869	931
08:45 AM	33	105	28	18	166	8	38	31	22	77	49	241	12	3	302	88	36	13	9	137	52	682	734
Total	132	570	190	135	892	74	218	142	75	434	290	1163	66	33	1519	485	243	120	62	848	305	3693	3998
Grand Total	246	1289	349	217	1884	178	389	300	159	867	574	2310	115	53	2999	836	385	202	118	1423	547	7173	7720
Apprch %	13.1	68.4	18.5			20.5	44.9	34.6			19.1	77	3.8			58.7	27.1	14.2					
Total %	3.4	18	4.9		26.3	2.5	5.4	4.2		12.1	8	32.2	1.6		41.8	11.7	5.4	2.8		19.8	7.1	92.9	

	La Sierra Avenue Southbound				Indiana Avenue Westbound				La Sierra Avenue Northbound				Indiana Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	30	201	41	272	**33**	39	39	111	75	257	19	351	109	36	28	173	907
07:45 AM	27	**276**	50	**353**	31	54	40	125	94	**343**	14	451	114	44	21	179	1108
08:00 AM	**33**	196	**64**	293	24	58	**45**	127	**103**	327	**27**	**457**	**150**	61	**41**	252	**1129**
08:15 AM	27	140	60	227	27	**68**	39	**134**	81	296	15	392	123	**102**	35	**260**	1013
Total Volume	117	813	215	1145	115	219	163	497	353	1223	75	1651	496	243	125	864	4157
% App. Total	10.2	71	18.8		23.1	44.1	32.8		21.4	74.1	4.5		57.4	28.1	14.5		
PHF	.886	.736	.840	.811	.871	.805	.906	.927	.857	.891	.694	.903	.827	.596	.762	.831	.921

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City of Riverside
N/S: La Sierra Avenue
E/W: Indiana Avenue
Weather: Clear

File Name : 03_RIV_La S_Indi AM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: Indiana Avenue
Weather: Clear

File Name : 03_RIV_La S_Indi AM
Site Code : 05123396
Start Date : 8/15/2023

	La Sierra Avenue Southbound				Indiana Avenue Westbound				La Sierra Avenue Northbound				Indiana Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:45 AM				07:45 AM				
+0 mins.	30	201	41	272	**33**	39	39	111	94	**343**	14	451	114	44	21	179	
+15 mins.	27	**276**	50	**353**	31	54	40	125	**103**	327	**27**	**457**	**150**	61	**41**	252	
+30 mins.	**33**	196	**64**	293	24	58	**45**	127	81	296	15	392	123	**102**	35	**260**	
+45 mins.	27	140	60	227	27	**68**	39	**134**	57	299	12	368	124	44	31	199	
Total Volume	117	813	215	1145	115	219	163	497	335	1265	68	1668	511	251	128	890	
% App. Total	10.2	71	18.8		23.1	44.1	32.8		20.1	75.8	4.1		57.4	28.2	14.4		
PHF	.886	.736	.840	.811	.871	.805	.906	.927	.813	.922	.630	.912	.852	.615	.780	.856	

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City of Riverside
N/S: La Sierra Avenue
E/W: Indiana Avenue
Weather: Clear

File Name : 03_RIV_La S_Indi PM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 1

Groups Printed- Total Volume

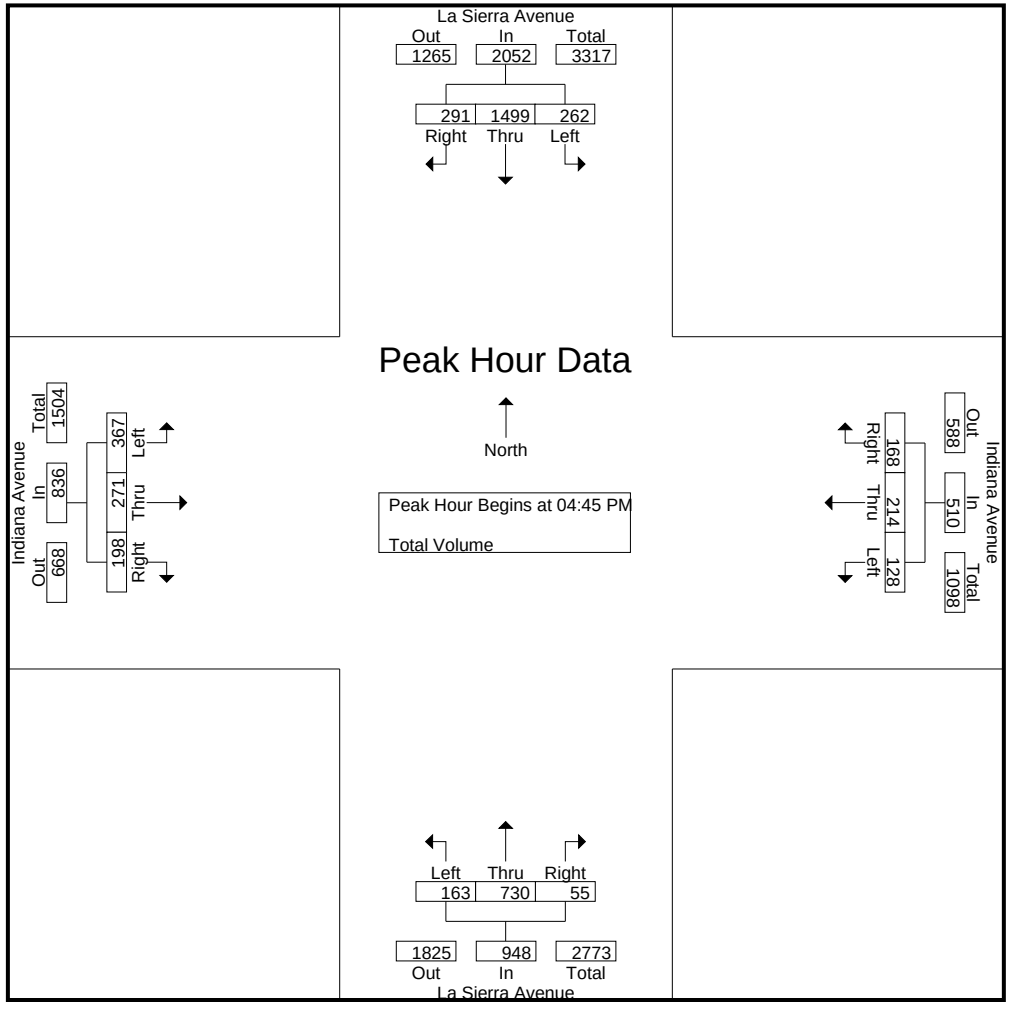
	La Sierra Avenue Southbound					Indiana Avenue Westbound					La Sierra Avenue Northbound					Indiana Avenue Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	74	331	88	35	493	33	60	30	18	123	42	178	17	13	237	90	76	49	11	215	77	1068	1145
04:15 PM	54	367	84	24	505	33	50	33	25	116	56	183	17	10	256	106	69	53	16	228	75	1105	1180
04:30 PM	67	320	71	20	458	45	47	33	24	125	50	172	11	2	233	80	80	46	13	206	59	1022	1081
04:45 PM	66	374	76	21	516	32	50	40	24	122	36	180	11	4	227	87	60	42	13	189	62	1054	1116
Total	261	1392	319	100	1972	143	207	136	91	486	184	713	56	29	953	363	285	190	53	838	273	4249	4522
05:00 PM	61	389	76	23	526	43	58	47	28	148	42	177	16	7	235	94	70	54	19	218	77	1127	1204
05:15 PM	61	376	76	23	513	25	57	37	32	119	43	191	16	9	250	89	76	60	21	225	85	1107	1192
05:30 PM	74	360	63	13	497	28	49	44	30	121	42	182	12	8	236	97	65	42	10	204	61	1058	1119
05:45 PM	60	351	70	19	481	28	31	26	21	85	40	174	14	10	228	84	63	45	15	192	65	986	1051
Total	256	1476	285	78	2017	124	195	154	111	473	167	724	58	34	949	364	274	201	65	839	288	4278	4566
Grand Total	517	2868	604	178	3989	267	402	290	202	959	351	1437	114	63	1902	727	559	391	118	1677	561	8527	9088
Apprch %	13	71.9	15.1			27.8	41.9	30.2			18.5	75.6	6			43.4	33.3	23.3					
Total %	6.1	33.6	7.1		46.8	3.1	4.7	3.4		11.2	4.1	16.9	1.3		22.3	8.5	6.6	4.6		19.7	6.2	93.8	

	La Sierra Avenue Southbound				Indiana Avenue Westbound				La Sierra Avenue Northbound				Indiana Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	66	374	**76**	516	32	50	40	122	36	180	11	227	87	60	42	189	1054
05:00 PM	61	**389**	76	**526**	**43**	**58**	**47**	**148**	42	177	**16**	235	94	70	54	218	**1127**
05:15 PM	61	376	76	513	25	57	37	119	**43**	**191**	16	**250**	89	**76**	**60**	**225**	1107
05:30 PM	**74**	360	63	497	28	49	44	121	42	182	12	236	**97**	65	42	204	1058
Total Volume	262	1499	291	2052	128	214	168	510	163	730	55	948	367	271	198	836	4346
% App. Total	12.8	73.1	14.2		25.1	42	32.9		17.2	77	5.8		43.9	32.4	23.7		
PHF	.885	.963	.957	.975	.744	.922	.894	.861	.948	.955	.859	.948	.946	.891	.825	.929	.964

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City of Riverside
N/S: La Sierra Avenue
E/W: Indiana Avenue
Weather: Clear

File Name : 03_RIV_La S_Indi PM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: Indiana Avenue
Weather: Clear

File Name : 03_RIV_La S_Indi PM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 3

	La Sierra Avenue Southbound				Indiana Avenue Westbound				La Sierra Avenue Northbound				Indiana Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:30 PM				04:00 PM				04:15 PM				
+0 mins.	66	374	**76**	516	**45**	47	33	125	42	178	**17**	237	**106**	69	53	**228**	
+15 mins.	61	**389**	76	**526**	32	50	40	122	**56**	**183**	17	**256**	80	**80**	46	206	
+30 mins.	61	376	76	513	43	**58**	**47**	**148**	50	172	11	233	87	60	42	189	
+45 mins.	**74**	360	63	497	25	57	37	119	36	180	11	227	94	70	**54**	218	
Total Volume	262	1499	291	2052	145	212	157	514	184	713	56	953	367	279	195	841	
% App. Total	12.8	73.1	14.2		28.2	41.2	30.5		19.3	74.8	5.9		43.6	33.2	23.2		
PHF	.885	.963	.957	.975	.806	.914	.835	.868	.821	.974	.824	.931	.866	.872	.903	.922	

Location: Riverside
N/S: La Sierra Avenue
E/W: Indiana Avenue

Date: 8/17/2023
Day: Thursday

PEDESTRIANS

	North Leg La Sierra Avenue	East Leg Indiana Avenue	South Leg La Sierra Avenue	West Leg Indiana Avenue	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	0	0	0	0	0
7:15 AM	3	3	3	0	9
7:30 AM	0	1	0	0	1
7:45 AM	2	1	0	1	4
8:00 AM	5	1	1	3	10
8:15 AM	3	1	2	1	7
8:30 AM	2	0	2	1	5
8:45 AM	0	3	1	1	5
TOTAL VOLUMES:	15	10	9	7	41

	North Leg La Sierra Avenue	East Leg Indiana Avenue	South Leg La Sierra Avenue	West Leg Indiana Avenue	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	2	0	2	2	6
4:15 PM	0	1	1	0	2
4:30 PM	2	0	0	1	3
4:45 PM	1	0	0	0	1
5:00 PM	1	0	0	0	1
5:15 PM	0	0	0	0	0
5:30 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0
TOTAL VOLUMES:	6	1	3	3	13

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951-268-6268

Location: Riverside
N/S: La Sierra Avenue
E/W: Indiana Avenue

Date: 8/17/2023
Day: Thursday

BICYCLES

	Southbound La Sierra Avenue			Westbound Indiana Avenue			Northbound La Sierra Avenue			Eastbound Indiana Avenue			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	1	0	0	0	0	0	1
7:30 AM	1	2	0	0	0	0	0	0	0	0	0	0	3
7:45 AM	0	0	0	0	1	0	0	0	0	0	0	0	1
8:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	1
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	1	0	0	23	0	24
TOTAL VOLUMES:	1	3	0	0	1	0	1	1	0	0	23	0	30

	Southbound La Sierra Avenue			Westbound Indiana Avenue			Northbound La Sierra Avenue			Eastbound Indiana Avenue			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	1	0	1
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	1	0	0	0	1	0	0	0	0	2
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	1	0	0	0	0	0	0	0	0	1
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	2	0	0	0	1	0	0	1	0	4

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City of Riverside
N/S: La Sierra Avenue
E/W: Victoria Avenue
Weather: Clear

File Name : 04_RIV_La S_Vic AM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 1

Groups Printed- Total Volume

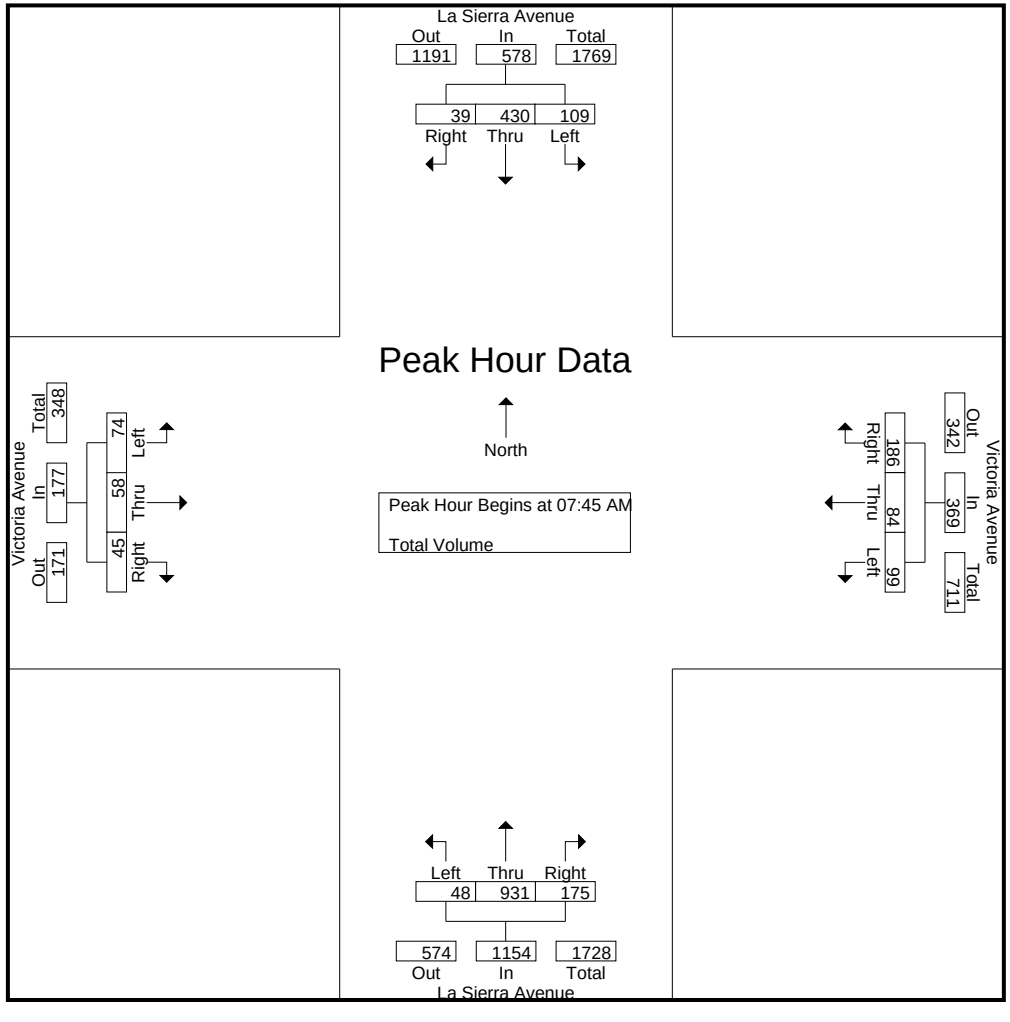
	La Sierra Avenue Southbound					Victoria Avenue Westbound					La Sierra Avenue Northbound					Victoria Avenue Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	18	104	6	0	128	13	17	28	16	58	3	263	20	8	286	20	5	4	3	29	27	501	528
07:15 AM	14	87	4	2	105	18	12	33	25	63	10	284	42	8	336	19	9	12	8	40	43	544	587
07:30 AM	20	84	5	2	109	22	16	42	15	80	7	237	28	3	272	12	18	13	12	43	32	504	536
07:45 AM	28	129	5	1	162	26	25	53	29	104	13	237	54	3	304	24	16	14	9	54	42	624	666
Total	80	404	20	5	504	79	70	156	85	305	33	1021	144	22	1198	75	48	43	32	166	144	2173	2317
08:00 AM	27	105	11	1	143	26	23	56	18	105	18	233	53	10	304	24	12	11	6	47	35	599	634
08:15 AM	32	104	13	4	149	19	16	40	33	75	11	218	35	3	264	15	17	14	10	46	50	534	584
08:30 AM	22	92	10	3	124	28	20	37	23	85	6	243	33	5	282	11	13	6	5	30	36	521	557
08:45 AM	12	87	3	1	102	17	20	42	18	79	4	199	13	4	216	17	8	3	2	28	25	425	450
Total	93	388	37	9	518	90	79	175	92	344	39	893	134	22	1066	67	50	34	23	151	146	2079	2225
Grand Total	173	792	57	14	1022	169	149	331	177	649	72	1914	278	44	2264	142	98	77	55	317	290	4252	4542
Apprch %	16.9	77.5	5.6			26	23	51			3.2	84.5	12.3			44.8	30.9	24.3					
Total %	4.1	18.6	1.3		24	4	3.5	7.8		15.3	1.7	45	6.5		53.2	3.3	2.3	1.8		7.5	6.4	93.6	

	La Sierra Avenue Southbound				Victoria Avenue Westbound				La Sierra Avenue Northbound				Victoria Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:45 AM																	
07:45 AM	28	**129**	5	**162**	26	**25**	53	104	13	237	**54**	**304**	**24**	16	**14**	**54**	**624**
08:00 AM	27	105	11	143	26	23	**56**	**105**	**18**	233	53	304	24	12	11	47	599
08:15 AM	**32**	104	**13**	149	19	16	40	75	11	218	35	264	15	**17**	14	46	534
08:30 AM	22	92	10	124	**28**	20	37	85	6	**243**	33	282	11	13	6	30	521
Total Volume	109	430	39	578	99	84	186	369	48	931	175	1154	74	58	45	177	2278
% App. Total	18.9	74.4	6.7		26.8	22.8	50.4		4.2	80.7	15.2		41.8	32.8	25.4		
PHF	.852	.833	.750	.892	.884	.840	.830	.879	.667	.958	.810	.949	.771	.853	.804	.819	.913

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City of Riverside
N/S: La Sierra Avenue
E/W: Victoria Avenue
Weather: Clear

File Name : 04_RIV_La S_Vic AM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
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File Name : 04_RIV_La S_Vic AM
Site Code : 05123396
Start Date : 8/15/2023

	La Sierra Avenue Southbound				Victoria Avenue Westbound				La Sierra Avenue Northbound				Victoria Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:45 AM				07:45 AM				07:15 AM				07:30 AM				
+0 mins.	28	**129**	5	**162**	26	**25**	53	104	10	**284**	42	**336**	12	**18**	13	43	
+15 mins.	27	105	11	143	26	23	**56**	**105**	7	237	28	272	**24**	16	**14**	**54**	
+30 mins.	**32**	104	**13**	149	19	16	40	75	13	237	**54**	304	24	12	11	47	
+45 mins.	22	92	10	124	**28**	20	37	85	**18**	233	53	304	15	17	14	46	
Total Volume	109	430	39	578	99	84	186	369	48	991	177	1216	75	63	52	190	
% App. Total	18.9	74.4	6.7		26.8	22.8	50.4		3.9	81.5	14.6		39.5	33.2	27.4		
PHF	.852	.833	.750	.892	.884	.840	.830	.879	.667	.872	.819	.905	.781	.875	.929	.880	

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City of Riverside
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File Name : 04_RIV_La S_Vic PM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 1

Groups Printed- Total Volume

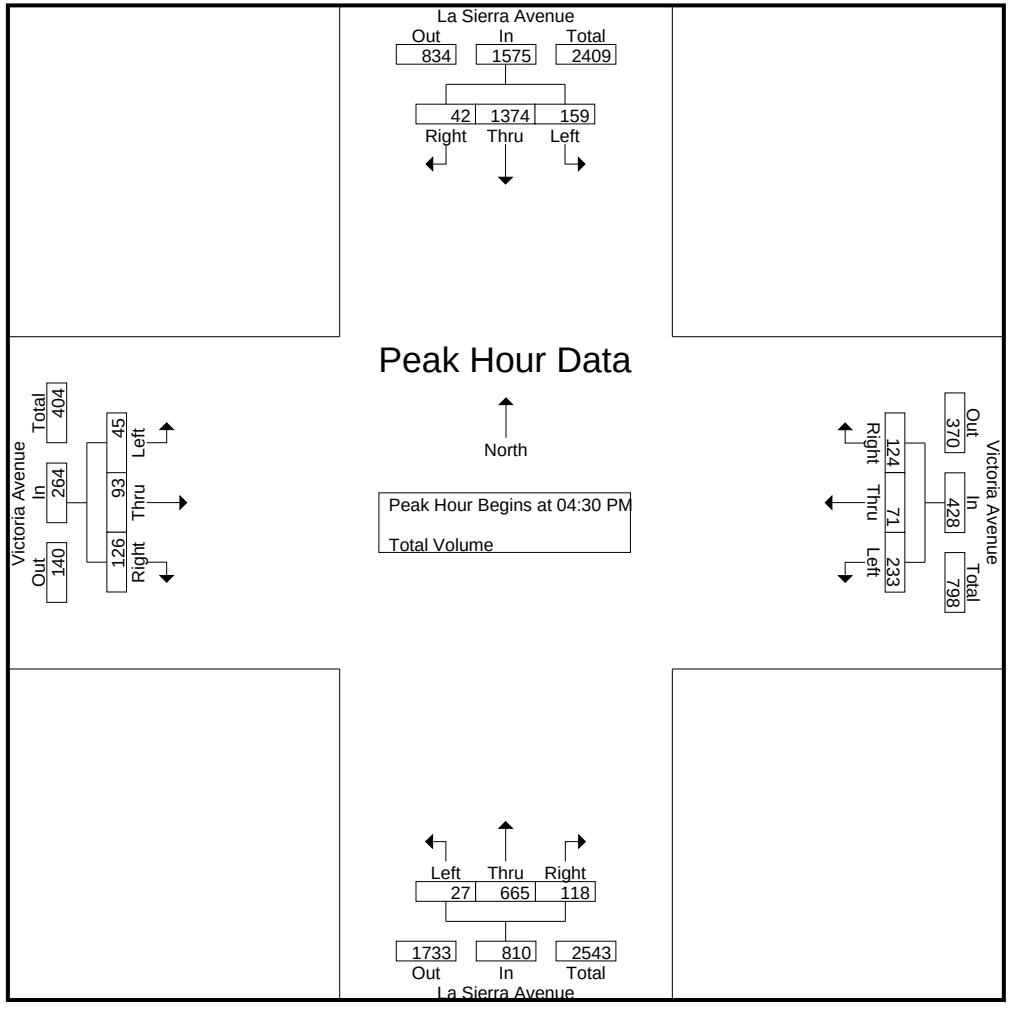
	La Sierra Avenue Southbound					Victoria Avenue Westbound					La Sierra Avenue Northbound					Victoria Avenue Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	38	274	19	5	331	56	17	39	20	112	5	161	32	9	198	6	19	29	14	54	48	695	743
04:15 PM	42	330	12	3	384	53	17	31	23	101	5	156	23	3	184	4	20	31	16	55	45	724	769
04:30 PM	29	297	12	0	338	53	12	34	20	99	7	166	36	11	209	11	23	33	13	67	44	713	757
04:45 PM	40	353	6	2	399	58	17	30	22	105	11	163	37	2	211	11	24	27	8	62	34	777	811
Total	149	1254	49	10	1452	220	63	134	85	417	28	646	128	25	802	32	86	120	51	238	171	2909	3080
05:00 PM	43	362	12	3	417	61	16	35	22	112	6	158	22	10	186	13	20	26	14	59	49	774	823
05:15 PM	47	362	12	6	421	61	26	25	15	112	3	178	23	5	204	10	26	40	14	76	40	813	853
05:30 PM	28	305	14	3	347	64	13	27	17	104	11	152	27	10	190	20	24	26	10	70	40	711	751
05:45 PM	38	290	12	4	340	68	15	27	15	110	5	153	28	11	186	10	18	28	9	56	39	692	731
Total	156	1319	50	16	1525	254	70	114	69	438	25	641	100	36	766	53	88	120	47	261	168	2990	3158
Grand Total	305	2573	99	26	2977	474	133	248	154	855	53	1287	228	61	1568	85	174	240	98	499	339	5899	6238
Apprch %	10.2	86.4	3.3			55.4	15.6	29			3.4	82.1	14.5			17	34.9	48.1					
Total %	5.2	43.6	1.7		50.5	8	2.3	4.2		14.5	0.9	21.8	3.9		26.6	1.4	2.9	4.1		8.5	5.4	94.6	

	La Sierra Avenue Southbound				Victoria Avenue Westbound				La Sierra Avenue Northbound				Victoria Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 PM																	
04:30 PM	29	297	**12**	338	53	12	34	99	7	166	36	209	11	23	33	67	713
04:45 PM	40	353	6	399	58	17	30	105	**11**	163	**37**	**211**	11	24	27	62	777
05:00 PM	43	**362**	12	417	**61**	16	**35**	**112**	6	158	22	186	**13**	20	26	59	774
05:15 PM	**47**	362	12	**421**	61	**26**	25	112	3	**178**	23	204	10	**26**	**40**	**76**	**813**
Total Volume	159	1374	42	1575	233	71	124	428	27	665	118	810	45	93	126	264	3077
% App. Total	10.1	87.2	2.7		54.4	16.6	29		3.3	82.1	14.6		17	35.2	47.7		
PHF	.846	.949	.875	.935	.955	.683	.886	.955	.614	.934	.797	.960	.865	.894	.788	.868	.946

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City of Riverside
N/S: La Sierra Avenue
E/W: Victoria Avenue
Weather: Clear

File Name : 04_RIV_La S_Vic PM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
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File Name : 04_RIV_La S_Vic PM
Site Code : 05123396
Start Date : 8/15/2023

	La Sierra Avenue Southbound				Victoria Avenue Westbound				La Sierra Avenue Northbound				Victoria Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				05:00 PM				04:30 PM				04:45 PM				
+0 mins.	40	353	6	399	61	16	**35**	**112**	7	166	36	209	11	24	27	62	
+15 mins.	43	**362**	12	417	61	**26**	25	112	**11**	163	**37**	**211**	13	20	26	59	
+30 mins.	**47**	362	12	**421**	64	13	27	104	6	158	22	186	10	**26**	**40**	**76**	
+45 mins.	28	305	**14**	347	**68**	15	27	110	3	**178**	23	204	**20**	24	26	70	
Total Volume	158	1382	44	1584	254	70	114	438	27	665	118	810	54	94	119	267	
% App. Total	10	87.2	2.8		58	16	26		3.3	82.1	14.6		20.2	35.2	44.6		
PHF	.840	.954	.786	.941	.934	.673	.814	.978	.614	.934	.797	.960	.675	.904	.744	.878	

Location: Riverside
N/S: La Sierra Avenue
E/W: Victoria Avenue

Date: 8/17/2023
Day: Thursday

PEDESTRIANS

	North Leg La Sierra Avenue	East Leg Victoria Avenue	South Leg La Sierra Avenue	West Leg Victoria Avenue	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	1	1	0	0	2
7:15 AM	0	0	0	0	0
7:30 AM	0	0	0	0	0
7:45 AM	0	1	0	0	1
8:00 AM	2	0	0	0	2
8:15 AM	0	0	1	0	1
8:30 AM	2	0	0	0	2
8:45 AM	0	0	0	1	1
TOTAL VOLUMES:	5	2	1	1	9

	North Leg La Sierra Avenue	East Leg Victoria Avenue	South Leg La Sierra Avenue	West Leg Victoria Avenue	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	0	0	0	0	0
4:15 PM	0	1	0	0	1
4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0
5:15 PM	0	0	0	0	0
5:30 PM	0	0	0	0	0
5:45 PM	2	0	0	0	2
TOTAL VOLUMES:	2	1	0	0	3

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Location: Riverside
N/S: La Sierra Avenue
E/W: Victoria Avenue

Date: 8/17/2023
Day: Thursday

BICYCLES

	Southbound La Sierra Avenue			Westbound Victoria Avenue			Northbound La Sierra Avenue			Eastbound Victoria Avenue			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	0	0	0	0	0	0	0	1	0	1
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	1	0	0	0	0	0	1	0	2
7:45 AM	0	0	0	0	2	0	0	0	0	0	2	0	4
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	1	0	0	0	0	0	0	1
8:45 AM	0	0	1	0	0	0	0	0	0	0	0	0	1
TOTAL VOLUMES:	0	0	1	0	3	1	0	0	0	0	4	0	9

	Southbound La Sierra Avenue			Westbound Victoria Avenue			Northbound La Sierra Avenue			Eastbound Victoria Avenue			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	1	0	0	0	0	0	0	0	1
4:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	1
5:00 PM	0	0	0	0	0	0	0	0	0	0	2	0	2
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	1	1
5:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	1
TOTAL VOLUMES:	0	0	0	0	1	0	0	0	0	0	4	1	6

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City of Riverside
N/S: La Sierra Avenue
E/W: McAllister Parkway
Weather: Clear

File Name : 05_RIV_La S_McA AM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 1

Groups Printed- Total Volume

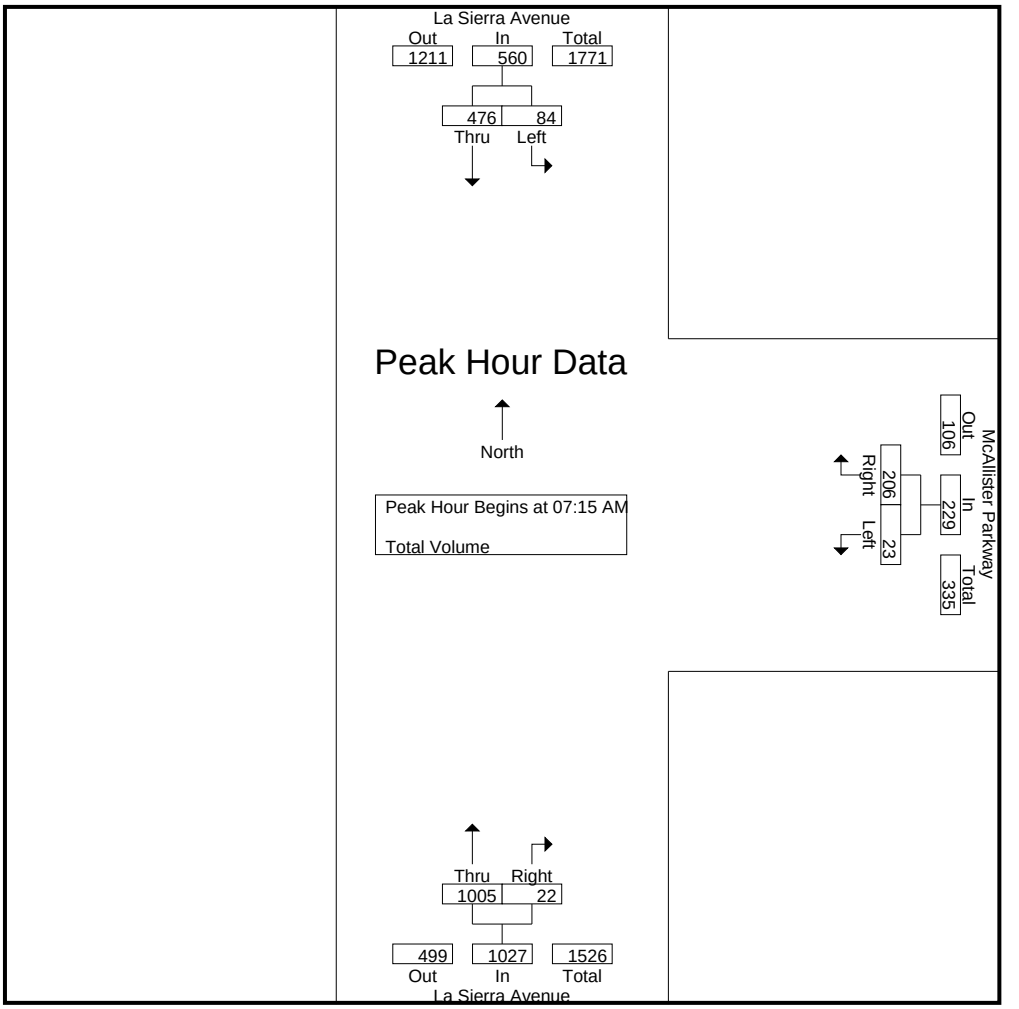
	La Sierra Avenue Southbound			McAllister Parkway Westbound				La Sierra Avenue Northbound						
Start Time	Left	Thru	App. Total	Left	Right	RTOR	App. Total	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	24	95	119	3	47	39	50	226	1	0	227	39	396	435
07:15 AM	16	105	121	2	60	48	62	251	4	2	255	50	438	488
07:30 AM	23	97	120	4	52	42	56	211	2	0	213	42	389	431
07:45 AM	21	150	171	6	57	40	63	274	8	0	282	40	516	556
Total	84	447	531	15	216	169	231	962	15	2	977	171	1739	1910
08:00 AM	24	124	148	11	37	30	48	269	8	1	277	31	473	504
08:15 AM	26	109	135	2	43	34	45	195	4	1	199	35	379	414
08:30 AM	23	109	132	4	43	36	47	233	0	0	233	36	412	448
08:45 AM	24	85	109	1	39	27	40	234	0	0	234	27	383	410
Total	97	427	524	18	162	127	180	931	12	2	943	129	1647	1776
Grand Total	181	874	1055	33	378	296	411	1893	27	4	1920	300	3386	3686
Apprch %	17.2	82.8		8	92			98.6	1.4					
Total %	5.3	25.8	31.2	1	11.2		12.1	55.9	0.8		56.7	8.1	91.9	

	La Sierra Avenue Southbound			McAllister Parkway Westbound			La Sierra Avenue Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:15 AM										
07:15 AM	16	105	121	2	**60**	62	251	4	255	438
07:30 AM	23	97	120	4	52	56	211	2	213	389
07:45 AM	21	**150**	**171**	6	57	**63**	**274**	**8**	**282**	**516**
08:00 AM	**24**	124	148	**11**	37	48	269	8	277	473
Total Volume	84	476	560	23	206	229	1005	22	1027	1816
% App. Total	15	85		10	90		97.9	2.1		
PHF	.875	.793	.819	.523	.858	.909	.917	.688	.910	.880

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City of Riverside
N/S: La Sierra Avenue
E/W: McAllister Parkway
Weather: Clear

File Name : 05_RIV_La S_McA AM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:45 AM			07:00 AM			07:15 AM		
+0 mins.	21	**150**	**171**	3	47	50	251	4	255
+15 mins.	24	124	148	2	**60**	62	211	2	213
+30 mins.	**26**	109	135	4	52	56	**274**	**8**	**282**
+45 mins.	23	109	132	**6**	57	**63**	269	8	277
Total Volume	94	492	586	15	216	231	1005	22	1027
% App. Total	16	84		6.5	93.5		97.9	2.1	
PHF	.904	.820	.857	.625	.900	.917	.917	.688	.910

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City of Riverside
N/S: La Sierra Avenue
E/W: McAllister Parkway
Weather: Clear

File Name : 05_RIV_La S_McA PM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 1

Groups Printed- Total Volume

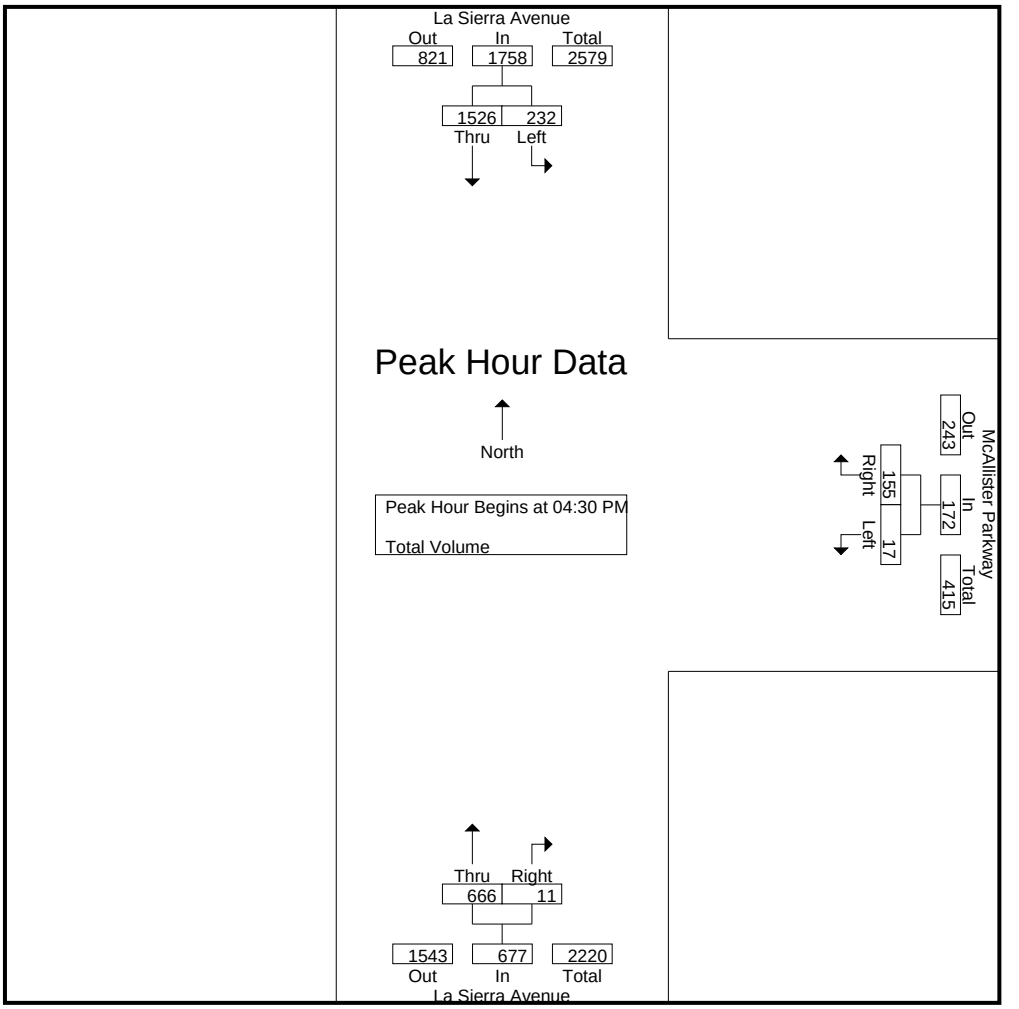
	La Sierra Avenue Southbound			McAllister Parkway Westbound				La Sierra Avenue Northbound						
Start Time	Left	Thru	App. Total	Left	Right	RTOR	App. Total	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	59	316	375	1	36	35	37	191	3	0	194	35	606	641
04:15 PM	55	364	419	8	34	28	42	125	3	1	128	29	589	618
04:30 PM	44	369	413	3	42	38	45	192	3	0	195	38	653	691
04:45 PM	59	365	424	4	40	34	44	164	0	0	164	34	632	666
Total	217	1414	1631	16	152	135	168	672	9	1	681	136	2480	2616
05:00 PM	64	383	447	8	36	32	44	156	2	0	158	32	649	681
05:15 PM	65	409	474	2	37	34	39	154	6	0	160	34	673	707
05:30 PM	73	342	415	2	36	33	38	163	3	0	166	33	619	652
05:45 PM	58	339	397	7	38	34	45	138	2	0	140	34	582	616
Total	260	1473	1733	19	147	133	166	611	13	0	624	133	2523	2656
Grand Total	477	2887	3364	35	299	268	334	1283	22	1	1305	269	5003	5272
Apprch %	14.2	85.8		10.5	89.5			98.3	1.7					
Total %	9.5	57.7	67.2	0.7	6		6.7	25.6	0.4		26.1	5.1	94.9	

	La Sierra Avenue Southbound			McAllister Parkway Westbound			La Sierra Avenue Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 04:30 PM										
04:30 PM	44	369	413	3	**42**	**45**	**192**	3	**195**	653
04:45 PM	59	365	424	4	40	44	164	0	164	632
05:00 PM	64	383	447	**8**	36	44	156	2	158	649
05:15 PM	**65**	**409**	**474**	2	37	39	154	**6**	160	**673**
Total Volume	232	1526	1758	17	155	172	666	11	677	2607
% App. Total	13.2	86.8		9.9	90.1		98.4	1.6		
PHF	.892	.933	.927	.531	.923	.956	.867	.458	.868	.968

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City of Riverside
N/S: La Sierra Avenue
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File Name : 05_RIV_La S_McA PM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:45 PM			04:15 PM			04:00 PM		
+0 mins.	59	365	424	**8**	34	42	191	**3**	194
+15 mins.	64	383	447	3	**42**	**45**	125	3	128
+30 mins.	65	**409**	**474**	4	40	44	**192**	3	**195**
+45 mins.	**73**	342	415	8	36	44	164	0	164
Total Volume	261	1499	1760	23	152	175	672	9	681
% App. Total	14.8	85.2		13.1	86.9		98.7	1.3	
PHF	.894	.916	.928	.719	.905	.972	.875	.750	.873

Location: Riverside
N/S: La Sierra Avenue
E/W: McAllister Parkway

Date: 8/17/2023
Day: Thursday

PEDESTRIANS

	North Leg La Sierra Avenue	East Leg McAllister Parkway	South Leg La Sierra Avenue	West Leg McAllister Parkway	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	0	0	0	0	0
7:15 AM	0	2	0	0	2
7:30 AM	0	1	0	0	1
7:45 AM	0	0	0	0	0
8:00 AM	0	0	0	0	0
8:15 AM	0	0	0	0	0
8:30 AM	0	0	0	0	0
8:45 AM	0	0	0	0	0
TOTAL VOLUMES:	0	3	0	0	3

	North Leg La Sierra Avenue	East Leg McAllister Parkway	South Leg La Sierra Avenue	West Leg McAllister Parkway	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	0	0	0	0	0
4:15 PM	0	0	0	0	0
4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0
5:15 PM	0	0	0	0	0
5:30 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0

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Location: Riverside
N/S: La Sierra Avenue
E/W: McAllister Parkway

Date: 8/17/2023
Day: Thursday

BICYCLES

	Southbound La Sierra Avenue			Westbound McAllister Parkway			Northbound La Sierra Avenue			Eastbound McAllister Parkway			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	1	0	0	0	0	1
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0	0	0	1	0	0	0	0	1

	Southbound La Sierra Avenue			Westbound McAllister Parkway			Northbound La Sierra Avenue			Eastbound McAllister Parkway			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	1
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	1	0	0	0	0	0	0	0	0	0	0	1

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City of Riverside
N/S: La Sierra Avenue
E/W: El Sobrante Road
Weather: Clear

File Name : 06_RIV_La S_El S AM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 1

Groups Printed- Total Volume

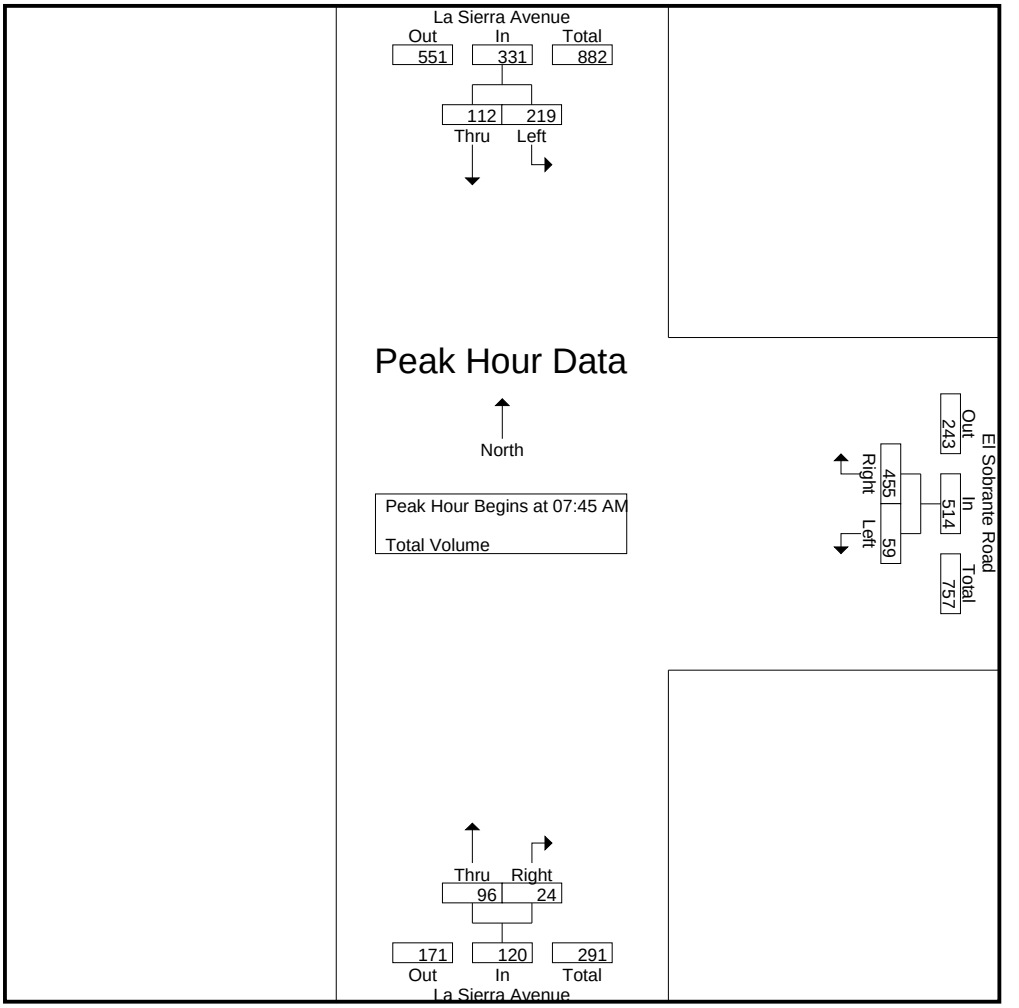
	La Sierra Avenue Southbound			El Sobrante Road Westbound			La Sierra Avenue Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
07:00 AM	54	31	85	18	112	130	22	5	27	242
07:15 AM	47	25	72	21	100	121	17	7	24	217
07:30 AM	36	32	68	19	104	123	23	8	31	222
07:45 AM	69	29	98	12	144	156	14	7	21	275
Total	206	117	323	70	460	530	76	27	103	956
08:00 AM	54	32	86	15	103	118	36	6	42	246
08:15 AM	52	29	81	14	83	97	23	7	30	208
08:30 AM	44	22	66	18	125	143	23	4	27	236
08:45 AM	39	32	71	8	131	139	17	9	26	236
Total	189	115	304	55	442	497	99	26	125	926
Grand Total	395	232	627	125	902	1027	175	53	228	1882
Apprch %	63	37		12.2	87.8		76.8	23.2		
Total %	21	12.3	33.3	6.6	47.9	54.6	9.3	2.8	12.1	

	La Sierra Avenue Southbound			El Sobrante Road Westbound			La Sierra Avenue Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:45 AM										
07:45 AM	**69**	29	**98**	12	**144**	**156**	14	**7**	21	**275**
08:00 AM	54	**32**	86	15	103	118	**36**	6	**42**	246
08:15 AM	52	29	81	14	83	97	23	7	30	208
08:30 AM	44	22	66	**18**	125	143	23	4	27	236
Total Volume	219	112	331	59	455	514	96	24	120	965
% App. Total	66.2	33.8		11.5	88.5		80	20		
PHF	.793	.875	.844	.819	.790	.824	.667	.857	.714	.877

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City of Riverside
N/S: La Sierra Avenue
E/W: El Sobrante Road
Weather: Clear

File Name : 06_RIV_La S_El S AM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:30 AM			07:00 AM			08:00 AM		
+0 mins.	36	**32**	68	18	112	130	**36**	6	**42**
+15 mins.	**69**	29	**98**	**21**	100	121	23	7	30
+30 mins.	54	32	86	19	104	123	23	4	27
+45 mins.	52	29	81	12	**144**	**156**	17	**9**	26
Total Volume	211	122	333	70	460	530	99	26	125
% App. Total	63.4	36.6		13.2	86.8		79.2	20.8	
PHF	.764	.953	.849	.833	.799	.849	.688	.722	.744

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City of Riverside
N/S: La Sierra Avenue
E/W: El Sobrante Road
Weather: Clear

File Name : 06_RIV_La S_El S PM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 1

Groups Printed- Total Volume

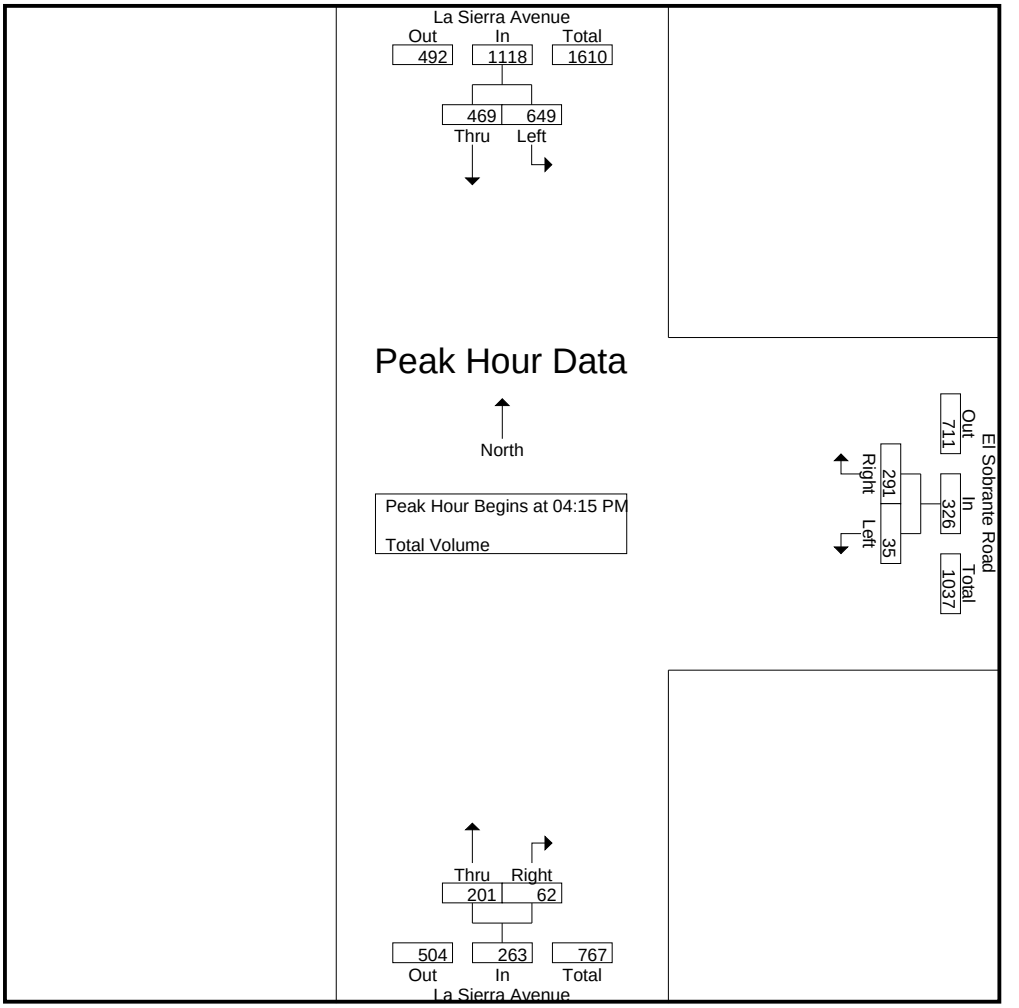
	La Sierra Avenue Southbound			El Sobrante Road Westbound			La Sierra Avenue Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
04:00 PM	150	94	244	9	84	93	43	20	63	400
04:15 PM	159	108	267	7	71	78	54	15	69	414
04:30 PM	163	102	265	11	86	97	42	10	52	414
04:45 PM	170	132	302	9	66	75	64	19	83	460
Total	642	436	1078	36	307	343	203	64	267	1688
05:00 PM	157	127	284	8	68	76	41	18	59	419
05:15 PM	167	124	291	12	53	65	40	12	52	408
05:30 PM	147	109	256	11	77	88	55	15	70	414
05:45 PM	139	77	216	8	66	74	38	11	49	339
Total	610	437	1047	39	264	303	174	56	230	1580
Grand Total	1252	873	2125	75	571	646	377	120	497	3268
Apprch %	58.9	41.1		11.6	88.4		75.9	24.1		
Total %	38.3	26.7	65	2.3	17.5	19.8	11.5	3.7	15.2	

	La Sierra Avenue Southbound			El Sobrante Road Westbound			La Sierra Avenue Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 04:15 PM										
04:15 PM	159	108	267	7	71	78	54	15	69	414
04:30 PM	163	102	265	**11**	**86**	**97**	42	10	52	414
04:45 PM	**170**	**132**	**302**	9	66	75	**64**	**19**	**83**	**460**
05:00 PM	157	127	284	8	68	76	41	18	59	419
Total Volume	649	469	1118	35	291	326	201	62	263	1707
% App. Total	58.1	41.9		10.7	89.3		76.4	23.6		
PHF	.954	.888	.925	.795	.846	.840	.785	.816	.792	.928

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City of Riverside
N/S: La Sierra Avenue
E/W: El Sobrante Road
Weather: Clear

File Name : 06_RIV_La S_El S PM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:30 PM			04:00 PM			04:00 PM		
+0 mins.	163	102	265	9	84	93	43	**20**	63
+15 mins.	**170**	**132**	**302**	7	71	78	54	15	69
+30 mins.	157	127	284	**11**	**86**	**97**	42	10	52
+45 mins.	167	124	291	9	66	75	**64**	19	**83**
Total Volume	657	485	1142	36	307	343	203	64	267
% App. Total	57.5	42.5		10.5	89.5		76	24	
PHF	.966	.919	.945	.818	.892	.884	.793	.800	.804

Location: Riverside
N/S: La Sierra Avenue
E/W: El Sobrante Road

Date: 8/17/2023
Day: Thursday

PEDESTRIANS

	North Leg La Sierra Avenue	East Leg El Sobrante Road	South Leg La Sierra Avenue	West Leg El Sobrante Road	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	0	0	0	0	0
7:15 AM	0	0	0	0	0
7:30 AM	0	0	0	0	0
7:45 AM	0	0	0	0	0
8:00 AM	0	0	0	0	0
8:15 AM	0	0	0	0	0
8:30 AM	0	0	0	0	0
8:45 AM	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0

	North Leg La Sierra Avenue	East Leg El Sobrante Road	South Leg La Sierra Avenue	West Leg El Sobrante Road	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	0	0	0	0	0
4:15 PM	0	0	0	0	0
4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0
5:15 PM	0	0	0	0	0
5:30 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0

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Location: Riverside
N/S: La Sierra Avenue
E/W: El Sobrante Road

Date: 8/17/2023
Day: Thursday

BICYCLES

	Southbound La Sierra Avenue			Westbound El Sobrante Road			Northbound La Sierra Avenue			Eastbound El Sobrante Road			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0	0	0	0	0	0	0	0	0

	Southbound La Sierra Avenue			Westbound El Sobrante Road			Northbound La Sierra Avenue			Eastbound El Sobrante Road			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0	0	0	0	0	0	0	0	0

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City of Riverside
N/S: McAllister Street
E/W: El Sobrante Road
Weather: Clear

File Name : 07_RIV_McA_El S AM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 1

Groups Printed- Total Volume

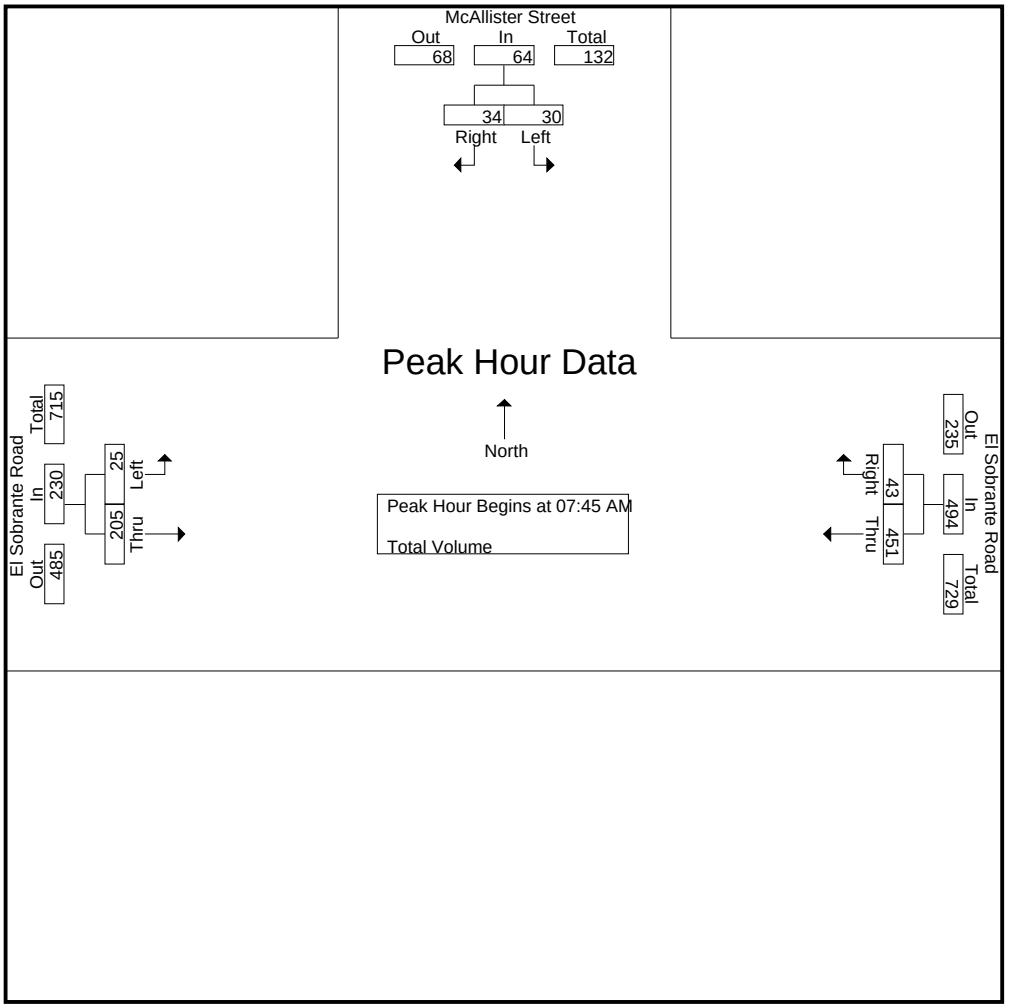
	McAllister Street Southbound			El Sobrante Road Westbound			El Sobrante Road Eastbound			
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
07:00 AM	1	8	9	97	2	99	12	54	66	174
07:15 AM	4	12	16	102	3	105	6	49	55	176
07:30 AM	2	12	14	120	4	124	4	44	48	186
07:45 AM	8	6	14	127	25	152	9	60	69	235
Total	15	38	53	446	34	480	31	207	238	771
08:00 AM	7	9	16	88	14	102	9	57	66	184
08:15 AM	13	2	15	111	2	113	5	42	47	175
08:30 AM	2	17	19	125	2	127	2	46	48	194
08:45 AM	2	8	10	124	3	127	6	35	41	178
Total	24	36	60	448	21	469	22	180	202	731
Grand Total	39	74	113	894	55	949	53	387	440	1502
Apprch %	34.5	65.5		94.2	5.8		12	88		
Total %	2.6	4.9	7.5	59.5	3.7	63.2	3.5	25.8	29.3	

	McAllister Street Southbound			El Sobrante Road Westbound			El Sobrante Road Eastbound			
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:45 AM										
07:45 AM	8	6	14	**127**	**25**	**152**	**9**	**60**	**69**	**235**
08:00 AM	7	9	16	88	14	102	9	57	66	184
08:15 AM	**13**	2	15	111	2	113	5	42	47	175
08:30 AM	2	**17**	**19**	125	2	127	2	46	48	194
Total Volume	30	34	64	451	43	494	25	205	230	788
% App. Total	46.9	53.1		91.3	8.7		10.9	89.1		
PHF	.577	.500	.842	.888	.430	.813	.694	.854	.833	.838

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City of Riverside
N/S: McAllister Street
E/W: El Sobrante Road
Weather: Clear

File Name : 07_RIV_McA_El S AM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:45 AM			07:45 AM			07:00 AM		
+0 mins.	8	6	14	**127**	**25**	**152**	**12**	54	66
+15 mins.	7	9	16	88	14	102	6	49	55
+30 mins.	**13**	2	15	111	2	113	4	44	48
+45 mins.	2	**17**	**19**	125	2	127	9	**60**	**69**
Total Volume	30	34	64	451	43	494	31	207	238
% App. Total	46.9	53.1		91.3	8.7		13	87	
PHF	.577	.500	.842	.888	.430	.813	.646	.863	.862

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City of Riverside
N/S: McAllister Street
E/W: El Sobrante Road
Weather: Clear

File Name : 07_RIV_McA_El S PM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 1

Groups Printed- Total Volume

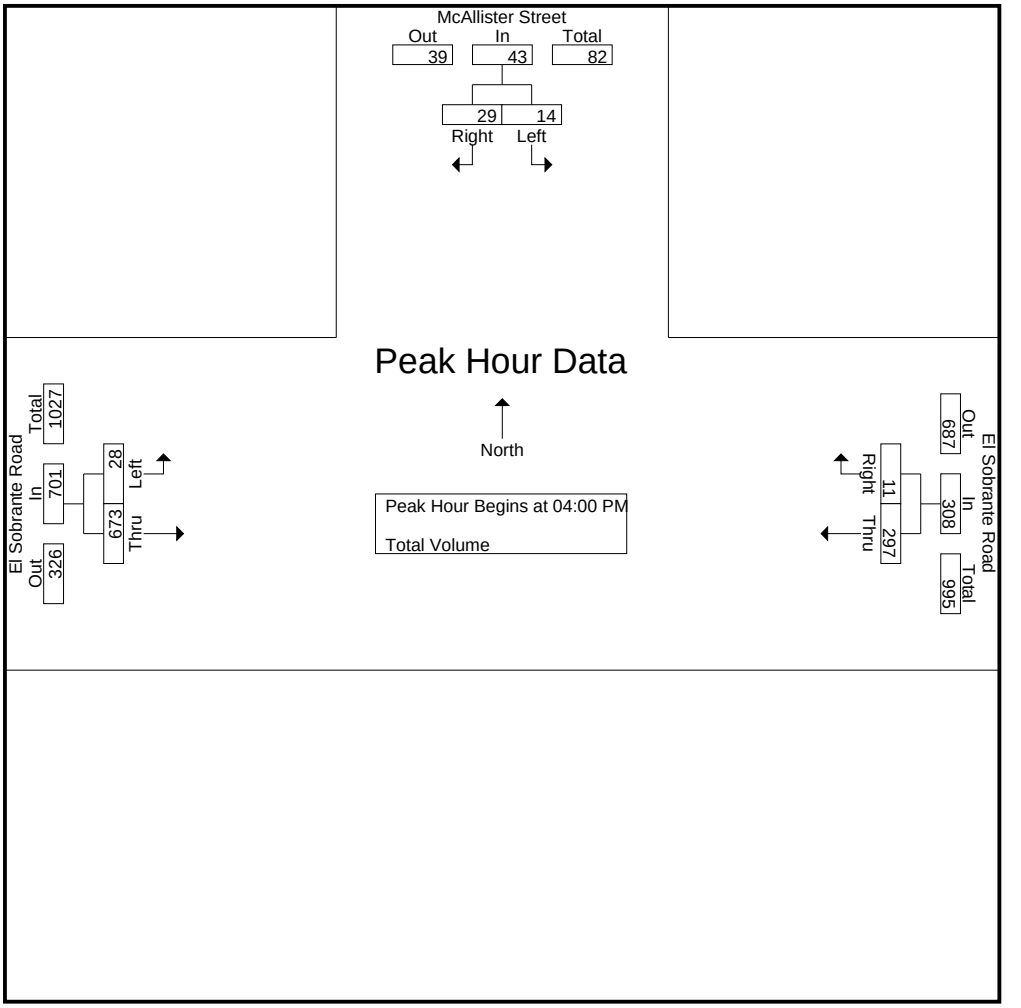
	McAllister Street Southbound			El Sobrante Road Westbound			El Sobrante Road Eastbound			
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
04:00 PM	1	7	8	87	3	90	8	159	167	265
04:15 PM	4	5	9	75	5	80	7	166	173	262
04:30 PM	7	8	15	77	0	77	5	172	177	269
04:45 PM	2	9	11	58	3	61	8	176	184	256
Total	14	29	43	297	11	308	28	673	701	1052
05:00 PM	4	5	9	74	2	76	5	166	171	256
05:15 PM	6	15	21	56	3	59	11	158	169	249
05:30 PM	4	15	19	69	12	81	7	150	157	257
05:45 PM	1	8	9	61	17	78	8	143	151	238
Total	15	43	58	260	34	294	31	617	648	1000
Grand Total	29	72	101	557	45	602	59	1290	1349	2052
Apprch %	28.7	71.3		92.5	7.5		4.4	95.6		
Total %	1.4	3.5	4.9	27.1	2.2	29.3	2.9	62.9	65.7	

	McAllister Street Southbound			El Sobrante Road Westbound			El Sobrante Road Eastbound			
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 04:00 PM										
04:00 PM	1	7	8	**87**	3	**90**	**8**	159	167	265
04:15 PM	4	5	9	75	**5**	80	7	166	173	262
04:30 PM	**7**	8	**15**	77	0	77	5	172	177	**269**
04:45 PM	2	**9**	11	58	3	61	8	**176**	**184**	256
Total Volume	14	29	43	297	11	308	28	673	701	1052
% App. Total	32.6	67.4		96.4	3.6		4	96		
PHF	.500	.806	.717	.853	.550	.856	.875	.956	.952	.978

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City of Riverside
N/S: McAllister Street
E/W: El Sobrante Road
Weather: Clear

File Name : 07_RIV_McA_El S PM
Site Code : 05123396
Start Date : 8/15/2023
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:45 PM			04:00 PM			04:15 PM		
+0 mins.	2	9	11	**87**	3	**90**	7	166	173
+15 mins.	4	5	9	75	**5**	80	5	172	177
+30 mins.	**6**	**15**	**21**	77	0	77	**8**	**176**	**184**
+45 mins.	4	15	19	58	3	61	5	166	171
Total Volume	16	44	60	297	11	308	25	680	705
% App. Total	26.7	73.3		96.4	3.6		3.5	96.5	
PHF	.667	.733	.714	.853	.550	.856	.781	.966	.958

Location: Riverside
N/S: McAllister Street
E/W: El Sobrante Road

Date: 8/17/2023
Day: Thursday

PEDESTRIANS

	North Leg McAllister Street	East Leg El Sobrante Road	South Leg McAllister Street	West Leg El Sobrante Road	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	0	0	0	0	0
7:15 AM	0	0	0	0	0
7:30 AM	0	0	0	0	0
7:45 AM	0	0	0	0	0
8:00 AM	0	0	0	0	0
8:15 AM	0	0	0	0	0
8:30 AM	0	0	0	0	0
8:45 AM	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0

	North Leg McAllister Street	East Leg El Sobrante Road	South Leg McAllister Street	West Leg El Sobrante Road	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	0	0	0	0	0
4:15 PM	0	0	0	0	0
4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0
5:15 PM	0	0	0	0	0
5:30 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
951-268-6268

Location: Riverside
N/S: McAllister Street
E/W: El Sobrante Road

Date: 8/17/2023
Day: Thursday

BICYCLES

	Southbound McAllister Street			Westbound El Sobrante Road			Northbound McAllister Street			Eastbound El Sobrante Road			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0	0	0	0	0	0	0	0	0

	Southbound McAllister Street			Westbound El Sobrante Road			Northbound McAllister Street			Eastbound El Sobrante Road			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	1
4:45 PM	1	0	0	0	0	0	0	0	0	0	0	0	1
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	1	0	1	0	0	0	0	0	0	0	0	0	2

Counts Unlimited, Inc.

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City of Riverside
El Sobrante Road
E/ McAllister Street
24 Hour Directional Volume Count

RIV004
Site Code: 051-23396

Start Time	8/17/23 Thu	Eastbound Morning	Eastbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Westbound Morning	Westbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Combined Totals Morning	Combined Totals Afternoon
12:00		11	71			11	50				
12:15		19	60			18	74				
12:30		12	68			15	47				
12:45		20	75	62	274	12	55	56	226	118	500
01:00		4	75			8	67				
01:15		9	90			3	86				
01:30		8	89			5	75				
01:45		8	121	29	375	8	68	24	296	53	671
02:00		7	119			10	83				
02:15		6	142			6	74				
02:30		16	127			19	85				
02:45		7	156	36	544	19	86	54	328	90	872
03:00		7	145			18	61				
03:15		8	145			26	82				
03:30		7	160			32	72				
03:45		11	151	33	601	48	**89**	124	304	157	905
04:00		11	163			77	**89**				
04:15		10	**171**			90	**79**				
04:30		15	**183**			116	**77**				
04:45		18	**177**	54	694	104	61	387	306	441	1000
05:00		25	**175**			113	78				
05:15		42	166			95	60				
05:30		30	157			110	79				
05:45		61	145	158	643	110	81	428	298	586	941
06:00		49	128			80	66				
06:15		45	122			131	61				
06:30		59	148			72	37				
06:45		54	128	207	526	117	58	400	222	607	748
07:00		55	128			101	37				
07:15		58	84			106	33				
07:30		49	90			122	38				
07:45		**75**	84	237	386	**161**	42	490	150	727	536
08:00		**70**	79			**106**	32				
08:15		**59**	75			**119**	33				
08:30		**50**	66			**127**	28				
08:45		37	59	216	279	129	27	481	120	697	399
09:00		44	60			104	24				
09:15		41	48			95	22				
09:30		43	57			86	14				
09:45		42	46	170	211	82	17	367	77	537	288
10:00		37	37			78	30				
10:15		39	29			67	18				
10:30		48	44			115	12				
10:45		50	33	174	143	72	12	332	72	506	215
11:00		47	34			70	14				
11:15		51	26			58	11				
11:30		60	11			57	13				
11:45		57	18	215	89	56	8	241	46	456	135
Total		1591	4765	1591	4765	3384	2445	3384	2445	4975	7210
Combined Total		6356		6356		5829		5829		12185	
AM Peak	-	07:45	-	-	-	07:45	-	-	-	-	-
Vol.	-	254	-	-	-	513	-	-	-	-	-
P.H.F.		0.847				0.797					
PM Peak	-	-	04:15	-	-	-	03:45	-	-	-	-
Vol.	-	-	706	-	-	-	334	-	-	-	-
P.H.F.			0.964				0.938				
Percentage		25.0%	75.0%			58.1%	41.9%				
ADT/AADT		ADT 12,185		AADT 12,185							

Counts Unlimited, Inc.

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City of Riverside
La Sierra Avenue
N/ El Sobrante Road
24 Hour Directional Volume Count

RIV003
Site Code: 051-23396

Start Time	8/17/23 Thu	Northbound Morning	Northbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Southbound Morning	Southbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Combined Totals Morning	Combined Totals Afternoon
12:00		19	85			22	76				
12:15		24	90			17	80				
12:30		21	73			19	124				
12:45		19	78	83	326	16	97	74	377	157	703
01:00		9	99			5	108				
01:15		8	110			9	122				
01:30		7	96			8	123				
01:45		9	107	33	412	9	141	31	494	64	906
02:00		12	93			10	204				
02:15		7	111			5	146				
02:30		21	106			20	188				
02:45		22	115	62	425	12	266	47	804	109	1229
03:00		16	101			9	205				
03:15		31	126			8	320				
03:30		30	111			10	263				
03:45		49	134	126	472	12	316	39	1104	165	1576
04:00		70	**134**			17	238				
04:15		92	**126**			12	**306**				
04:30		121	**126**			24	**387**				
04:45		103	**142**	386	528	25	**406**	78	1337	464	1865
05:00		122	114			42	**287**				
05:15		100	97			52	292				
05:30		102	132			79	260				
05:45		118	104	442	447	87	219	260	1058	702	1505
06:00		87	118			81	206				
06:15		113	89			75	167				
06:30		98	66			87	180				
06:45		128	71	426	344	77	157	320	710	746	1054
07:00		137	57			86	125				
07:15		115	71			75	105				
07:30		125	62			75	106				
07:45		**163**	59	540	249	108	107	344	443	884	692
08:00		**143**	50			95	91				
08:15		**108**	45			65	102				
08:30		**154**	52			79	71				
08:45		151	43	556	190	59	73	298	337	854	527
09:00		130	45			61	77				
09:15		124	30			72	55				
09:30		104	33			67	62				
09:45		101	23	459	131	75	55	275	249	734	380
10:00		96	40			57	40				
10:15		87	23			67	36				
10:30		123	21			**95**	48				
10:45		96	15	402	99	**76**	38	295	162	697	261
11:00		96	19			**91**	34				
11:15		85	14			**101**	31				
11:30		72	22			83	17				
11:45		74	12	327	67	79	26	354	108	681	175
Total		3842	3690	3842	3690	2415	7183	2415	7183	6257	10873
Combined Total		7532		7532		9598		9598		17130	
AM Peak	-	07:45	-	-	-	10:30	-	-	-	-	-
Vol.	-	568	-	-	-	363	-	-	-	-	-
P.H.F.		0.871				0.840					
PM Peak	-	-	04:00	-	-	-	04:15	-	-	-	-
Vol.	-	-	528	-	-	-	1386	-	-	-	-
P.H.F.			0.930				0.853				
Percentage		51.0%	49.0%			25.2%	74.8%				
ADT/AADT		ADT 17,130		AADT 17,130							

Counts Unlimited, Inc.

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City of Riverside
La Sierra Avenue
S/ Indiana Avenue
24 Hour Directional Volume Count

RIV001
Site Code: 051-23396

Start Time	8/17/23 Thu	Northbound Morning	Northbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Southbound Morning	Southbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Combined Totals Morning	Combined Totals Afternoon
12:00		33	205			53	195				
12:15		34	189			41	209				
12:30		32	235			34	256				
12:45		28	175	127	804	35	215	163	875	290	1679
01:00		28	156			22	240				
01:15		20	212			40	280				
01:30		17	181			27	269				
01:45		18	209	83	758	26	303	115	1092	198	1850
02:00		23	217			16	292				
02:15		16	208			15	303				
02:30		33	195			27	346				
02:45		25	226	97	846	18	388	76	1329	173	2175
03:00		36	**385**			22	324				
03:15		47	**334**			18	350				
03:30		58	**241**			15	349				
03:45		68	**235**	209	1195	20	415	75	1438	284	2633
04:00		96	239			21	373				
04:15		163	240			24	408				
04:30		159	252			34	**427**				
04:45		187	225	605	956	34	**401**	113	1609	718	2565
05:00		210	226			59	**425**				
05:15		168	234			69	**426**				
05:30		222	238			68	385				
05:45		229	225	829	923	94	383	290	1619	1119	2542
06:00		212	209			106	351				
06:15		250	203			111	351				
06:30		268	199			133	356				
06:45		296	196	1026	807	120	321	470	1379	1496	2186
07:00		325	200			156	260				
07:15		338	216			162	283				
07:30		**332**	182			**249**	293				
07:45		**420**	188	1415	786	**315**	284	882	1120	2297	1906
08:00		**423**	154			**264**	264				
08:15		**382**	139			**183**	266				
08:30		310	135			171	261				
08:45		333	109	1448	537	146	208	764	999	2212	1536
09:00		251	101			137	206				
09:15		226	98			141	186				
09:30		256	95			147	160				
09:45		278	75	1011	369	157	141	582	693	1593	1062
10:00		289	88			129	123				
10:15		247	57			136	135				
10:30		222	47			192	109				
10:45		238	63	996	255	181	118	638	485	1634	740
11:00		241	48			186	69				
11:15		224	36			193	83				
11:30		218	41			204	71				
11:45		190	35	873	160	222	50	805	273	1678	433
Total		8719	8396	8719	8396	4973	12911	4973	12911	13692	21307
Combined Total		17115		17115		17884		17884		34999	
AM Peak	-	07:30	-	-	-	07:30	-	-	-	-	-
Vol.	-	1557	-	-	-	1011	-	-	-	-	-
P.H.F.		0.920				0.802					
PM Peak	-	-	03:00	-	-	-	04:30	-	-	-	-
Vol.	-	-	1195	-	-	-	1679	-	-	-	-
P.H.F.			0.776				0.983				
Percentage		50.9%	49.1%			27.8%	72.2%				
ADT/AADT		ADT 34,999		AADT 34,999							

Counts Unlimited, Inc.

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City of Riverside
Victoria Avenue
E/ La Sierra Avenue
24 Hour Directional Volume Count

RIV002
Site Code: 051-23396

Start Time	8/17/23 Thu	Eastbound Morning	Eastbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Westbound Morning	Westbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Combined Totals Morning	Combined Totals Afternoon
12:00		2	43			3	36				
12:15		0	35			4	45				
12:30		1	37			2	45				
12:45		3	41	6	156	4	51	13	177	19	333
01:00		3	43			1	40				
01:15		3	60			4	59				
01:30		0	39			0	40				
01:45		4	54	10	196	2	51	7	190	17	386
02:00		2	54			2	59				
02:15		1	56			1	48				
02:30		1	65			0	73				
02:45		2	73	6	248	1	**103**	4	283	10	531
03:00		2	66			2	**78**				
03:15		1	89			2	**105**				
03:30		4	79			2	**94**				
03:45		2	**111**	9	345	2	71	8	348	17	693
04:00		3	**82**			3	78				
04:15		1	**82**			7	73				
04:30		6	**91**			10	72				
04:45		9	79	19	334	10	77	30	300	49	634
05:00		7	92			10	80				
05:15		12	85			14	88				
05:30		8	75			20	74				
05:45		9	87	36	339	33	87	77	329	113	668
06:00		11	70			17	53				
06:15		18	55			21	64				
06:30		27	59			34	48				
06:45		36	50	92	234	46	37	118	202	210	436
07:00		33	45			41	35				
07:15		54	50			48	38				
07:30		54	44			60	44				
07:45		**92**	34	233	173	**84**	23	233	140	466	313
08:00		**80**	27			**89**	33				
08:15		**82**	33			**56**	41				
08:30		**65**	18			**62**	22				
08:45		33	20	260	98	61	21	268	117	528	215
09:00		22	14			56	22				
09:15		41	27			43	16				
09:30		34	19			57	27				
09:45		30	14	127	74	42	8	198	73	325	147
10:00		29	11			46	17				
10:15		27	11			58	6				
10:30		32	10			59	7				
10:45		31	9	119	41	43	10	206	40	325	81
11:00		29	6			49	9				
11:15		29	6			54	8				
11:30		39	7			52	13				
11:45		43	6	140	25	38	4	193	34	333	59
Total		1057	2263	1057	2263	1355	2233	1355	2233	2412	4496
Combined Total		3320		3320		3588		3588		6908	
AM Peak	-	07:45	-	-	-	07:45	-	-	-	-	-
Vol.	-	319	-	-	-	291	-	-	-	-	-
P.H.F.		0.867				0.817					
PM Peak	-	-	03:45	-	-	-	02:45	-	-	-	-
Vol.	-	-	366	-	-	-	380	-	-	-	-
P.H.F.			0.824				0.905				
Percentage		31.8%	68.2%			37.8%	62.2%				
ADT/AADT		ADT 6,908		AADT 6,908							

APPENDIX 1.2: SITE ADJACENT QUEUES

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Intersection: 8: El Sobrante Rd. & Street A

Movement	EB	WB	SB
Directions Served	L	TR	LR
Maximum Queue (ft)	56	10	157
Average Queue (ft)	20	0	64
95th Queue (ft)	49	7	120
Link Distance (ft)		5676	1174
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	100		
Storage Blk Time (%)			
Queuing Penalty (veh)			

Network Summary

Network wide Queuing Penalty: 0

Intersection: 8: El Sobrante Rd. & Street A

Movement	EB	WB	SB
Directions Served	L	TR	LR
Maximum Queue (ft)	86	9	124
Average Queue (ft)	37	0	48
95th Queue (ft)	67	5	93
Link Distance (ft)		5674	1174
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	100		
Storage Blk Time (%)	0		
Queuing Penalty (veh)	1		

Network Summary

Network wide Queuing Penalty: 1

APPENDIX 3.1: TRAFFIC COUNTS

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Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
(951) 268-6268

City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 1

Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

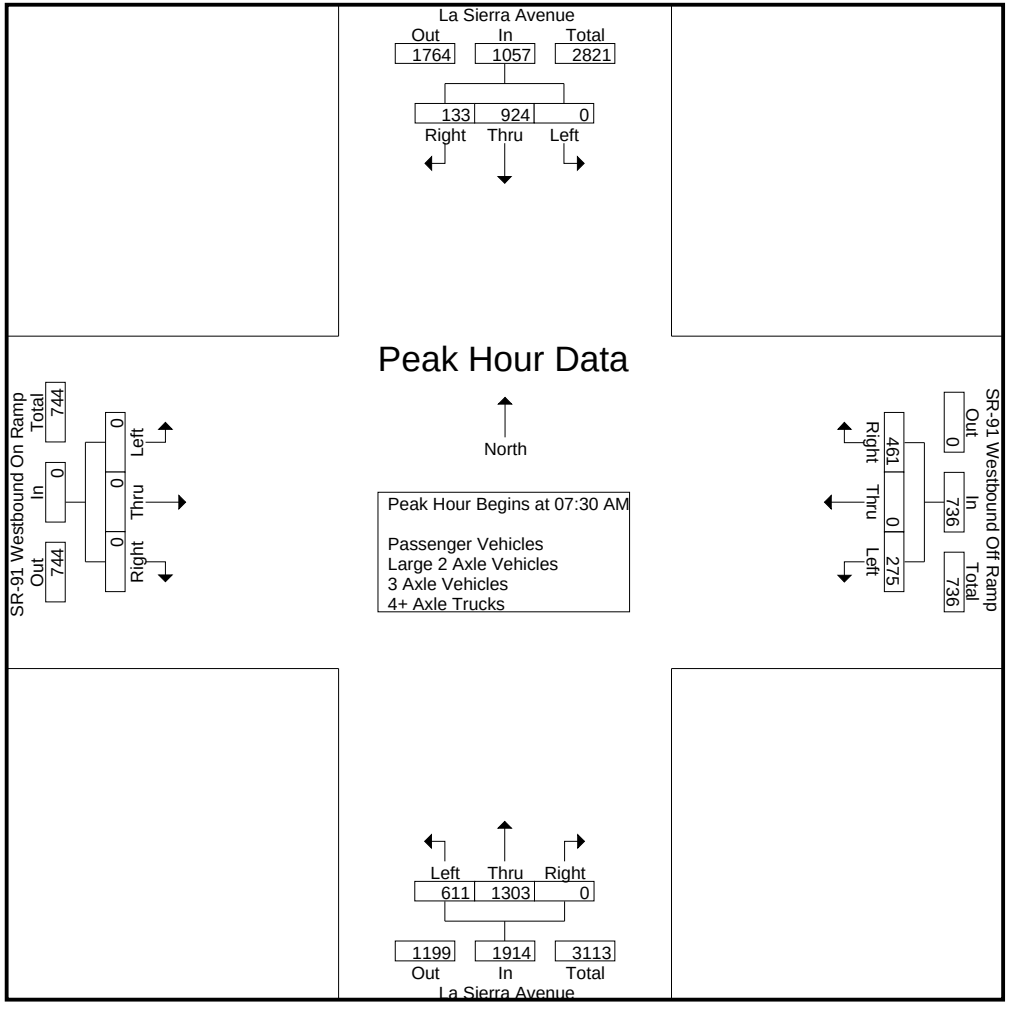
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	135	37	14	172	39	1	66	54	106	128	158	0	0	286	0	0	0	0	0	68	564	632
07:15 AM	0	213	38	12	251	45	0	110	38	155	130	218	0	0	348	0	0	0	0	0	50	754	804
07:30 AM	0	230	40	18	270	53	0	111	56	164	128	259	0	0	387	0	0	0	0	0	74	821	895
07:45 AM	0	278	22	4	300	58	0	109	34	167	165	341	0	0	506	0	0	0	0	0	38	973	1011
Total	0	856	137	48	993	195	1	396	182	592	551	976	0	0	1527	0	0	0	0	0	230	3112	3342
08:00 AM	0	235	38	4	273	90	0	113	35	203	149	350	0	0	499	0	0	0	0	0	39	975	1014
08:15 AM	0	181	33	16	214	74	0	128	44	202	169	353	0	0	522	0	0	0	0	0	60	938	998
08:30 AM	0	176	40	9	216	43	1	128	71	172	130	227	0	0	357	0	0	0	0	0	80	745	825
08:45 AM	0	154	25	12	179	53	0	110	71	163	139	212	0	0	351	0	0	0	0	0	83	693	776
Total	0	746	136	41	882	260	1	479	221	740	587	1142	0	0	1729	0	0	0	0	0	262	3351	3613
Grand Total	0	1602	273	89	1875	455	2	875	403	1332	1138	2118	0	0	3256	0	0	0	0	0	492	6463	6955
Apprch %	0	85.4	14.6			34.2	0.2	65.7			35	65	0			0	0	0					
Total %	0	24.8	4.2		29	7	0	13.5		20.6	17.6	32.8	0		50.4	0	0	0		0	7.1	92.9	
Passenger Vehicles	0	1558	263		1906	441	2	857		1691	1110	2072	0		3182	0	0	0		0	0	0	6779
% Passenger Vehicles	0	97.3	96.3	95.5	97	96.9	100	97.9	97	97.5	97.5	97.8	0	0	97.7	0	0	0	0	0	0	0	97.5
Large 2 Axle Vehicles	0	25	6		33	7	0	14		30	13	30	0		43	0	0	0		0	0	0	106
% Large 2 Axle Vehicles	0	1.6	2.2	2.2	1.7	1.5	0	1.6	2.2	1.7	1.1	1.4	0	0	1.3	0	0	0	0	0	0	0	1.5
3 Axle Vehicles	0	6	0		7	4	0	1		6	4	8	0		12	0	0	0		0	0	0	25
% 3 Axle Vehicles	0	0.4	0	1.1	0.4	0.9	0	0.1	0.2	0.3	0.4	0.4	0	0	0.4	0	0	0	0	0	0	0	0.4
4+ Axle Trucks	0	13	4		18	3	0	3		8	11	8	0		19	0	0	0		0	0	0	45
% 4+ Axle Trucks	0	0.8	1.5	1.1	0.9	0.7	0	0.3	0.5	0.5	1	0.4	0	0	0.6	0	0	0	0	0	0	0	0.6

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	0	230	**40**	270	53	0	111	164	128	259	0	387	0	0	0	0	821
07:45 AM	0	**278**	22	**300**	58	0	109	167	165	341	0	506	0	0	0	0	973
08:00 AM	0	235	38	273	**90**	0	113	**203**	149	350	0	499	0	0	0	0	**975**
08:15 AM	0	181	33	214	74	0	**128**	202	**169**	**353**	0	**522**	0	0	0	0	938
Total Volume	0	924	133	1057	275	0	461	736	611	1303	0	1914	0	0	0	0	3707
% App. Total	0	87.4	12.6		37.4	0	62.6		31.9	68.1	0		0	0	0		
PHF	.000	.831	.831	.881	.764	.000	.900	.906	.904	.923	.000	.917	.000	.000	.000	.000	.951

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W AM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:15 AM				07:45 AM				07:30 AM				07:00 AM				
+0 mins.	0	213	38	251	58	0	109	167	128	259	0	387	0	0	0	0	
+15 mins.	0	230	**40**	270	**90**	0	113	**203**	165	341	0	506	0	0	0	0	
+30 mins.	0	**278**	22	**300**	74	0	**128**	202	149	350	0	499	0	0	0	0	
+45 mins.	0	235	38	273	43	**1**	128	172	**169**	**353**	0	**522**	0	0	0	0	
Total Volume	0	956	138	1094	265	1	478	744	611	1303	0	1914	0	0	0	0	
% App. Total	0	87.4	12.6		35.6	0.1	64.2		31.9	68.1	0		0	0	0		
PHF	.000	.860	.863	.912	.736	.250	.934	.916	.904	.923	.000	.917	.000	.000	.000	.000	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W AM
Site Code : 05124383
Start Date : 4/25/2024

Groups Printed- Passenger Vehicles

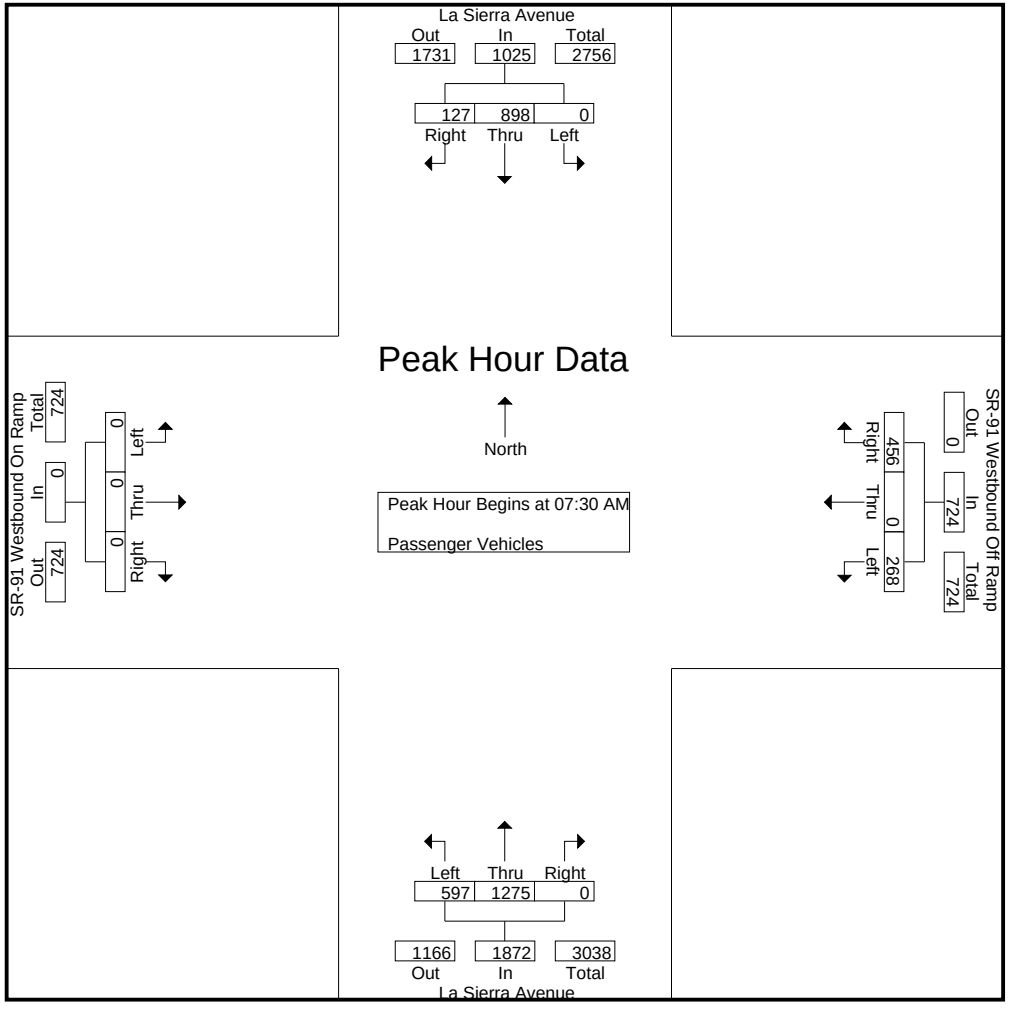
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	133	36	13	169	36	1	63	51	100	122	155	0	0	277	0	0	0	0	0	64	546	610
07:15 AM	0	201	37	12	238	45	0	108	37	153	126	213	0	0	339	0	0	0	0	0	49	730	779
07:30 AM	0	221	39	18	260	52	0	110	56	162	125	251	0	0	376	0	0	0	0	0	74	798	872
07:45 AM	0	273	22	4	295	56	0	106	32	162	161	329	0	0	490	0	0	0	0	0	36	947	983
Total	0	828	134	47	962	189	1	387	176	577	534	948	0	0	1482	0	0	0	0	0	223	3021	3244
08:00 AM	0	228	36	4	264	89	0	113	34	202	146	347	0	0	493	0	0	0	0	0	38	959	997
08:15 AM	0	176	30	14	206	71	0	127	44	198	165	348	0	0	513	0	0	0	0	0	58	917	975
08:30 AM	0	173	40	8	213	41	1	124	70	166	128	221	0	0	349	0	0	0	0	0	78	728	806
08:45 AM	0	153	23	12	176	51	0	106	67	157	137	208	0	0	345	0	0	0	0	0	79	678	757
Total	0	730	129	38	859	252	1	470	215	723	576	1124	0	0	1700	0	0	0	0	0	253	3282	3535
Grand Total	0	1558	263	85	1821	441	2	857	391	1300	1110	2072	0	0	3182	0	0	0	0	0	476	6303	6779
Apprch %	0	85.6	14.4			33.9	0.2	65.9			34.9	65.1	0			0	0	0					
Total %	0	24.7	4.2		28.9	7	0	13.6		20.6	17.6	32.9	0		50.5	0	0	0		0	7	93	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	0	221	**39**	260	52	0	110	162	125	251	0	376	0	0	0	0	798
07:45 AM	0	**273**	22	**295**	56	0	106	162	161	329	0	490	0	0	0	0	947
08:00 AM	0	228	36	264	**89**	0	113	**202**	146	347	0	493	0	0	0	0	**959**
08:15 AM	0	176	30	206	71	0	**127**	198	**165**	**348**	0	**513**	0	0	0	0	917
Total Volume	0	898	127	1025	268	0	456	724	597	1275	0	1872	0	0	0	0	3621
% App. Total	0	87.6	12.4		37	0	63		31.9	68.1	0		0	0	0		
PHF	.000	.822	.814	.869	.753	.000	.898	.896	.905	.916	.000	.912	.000	.000	.000	.000	.944

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W AM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	0	221	**39**	260	52	0	110	162	125	251	0	376	0	0	0	0	
+15 mins.	0	**273**	22	**295**	56	0	106	162	161	329	0	490	0	0	0	0	
+30 mins.	0	228	36	264	**89**	0	113	**202**	146	347	0	493	0	0	0	0	
+45 mins.	0	176	30	206	71	0	**127**	198	**165**	**348**	0	**513**	0	0	0	0	
Total Volume	0	898	127	1025	268	0	456	724	597	1275	0	1872	0	0	0	0	
% App. Total	0	87.6	12.4		37	0	63		31.9	68.1	0		0	0	0		
PHF	.000	.822	.814	.869	.753	.000	.898	.896	.905	.916	.000	.912	.000	.000	.000	.000	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 1

Groups Printed- Large 2 Axle Vehicles

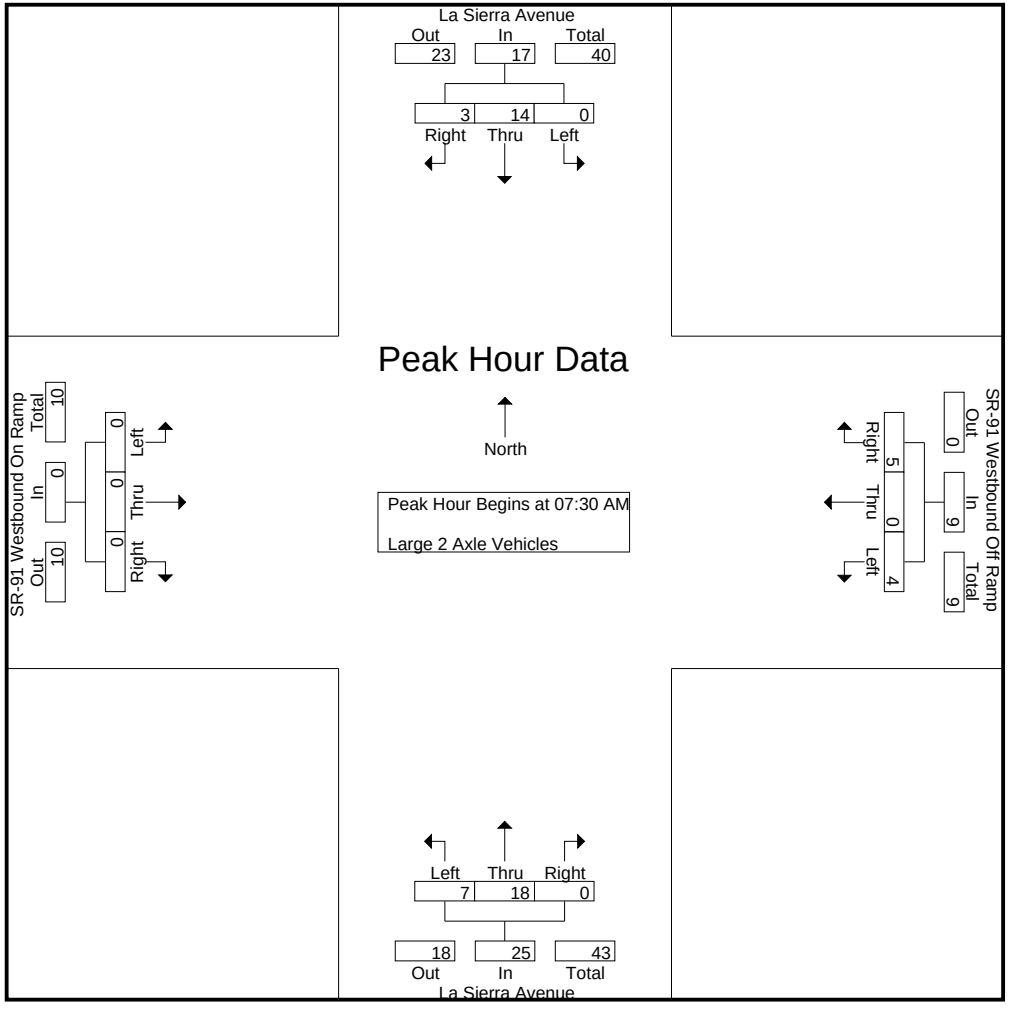
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	2	0	0	2	3	0	1	1	4	1	2	0	0	3	0	0	0	0	0	1	9	10
07:15 AM	0	7	1	0	8	0	0	0	0	0	2	4	0	0	6	0	0	0	0	0	0	14	14
07:30 AM	0	3	0	0	3	1	0	1	0	2	2	6	0	0	8	0	0	0	0	0	0	13	13
07:45 AM	0	3	0	0	3	2	0	3	2	5	2	9	0	0	11	0	0	0	0	0	2	19	21
Total	0	15	1	0	16	6	0	5	3	11	7	21	0	0	28	0	0	0	0	0	3	55	58
08:00 AM	0	5	1	0	6	0	0	0	1	0	3	1	0	0	4	0	0	0	0	0	1	10	11
08:15 AM	0	3	2	2	5	1	0	1	0	2	0	2	0	0	2	0	0	0	0	0	2	9	11
08:30 AM	0	1	0	0	1	0	0	4	1	4	1	4	0	0	5	0	0	0	0	0	1	10	11
08:45 AM	0	1	2	0	3	0	0	4	4	4	2	2	0	0	4	0	0	0	0	0	4	11	15
Total	0	10	5	2	15	1	0	9	6	10	6	9	0	0	15	0	0	0	0	0	8	40	48
Grand Total	0	25	6	2	31	7	0	14	9	21	13	30	0	0	43	0	0	0	0	0	11	95	106
Apprch %	0	80.6	19.4			33.3	0	66.7			30.2	69.8	0			0	0	0					
Total %	0	26.3	6.3		32.6	7.4	0	14.7		22.1	13.7	31.6	0		45.3	0	0	0		0	10.4	89.6	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	0	3	0	3	1	0	1	2	2	6	0	8	0	0	0	0	13
07:45 AM	0	3	0	3	**2**	0	**3**	**5**	2	**9**	0	**11**	0	0	0	0	**19**
08:00 AM	0	**5**	1	**6**	0	0	0	0	**3**	1	0	4	0	0	0	0	10
08:15 AM	0	3	**2**	5	1	0	1	2	0	2	0	2	0	0	0	0	9
Total Volume	0	14	3	17	4	0	5	9	7	18	0	25	0	0	0	0	51
% App. Total	0	82.4	17.6		44.4	0	55.6		28	72	0		0	0	0		
PHF	.000	.700	.375	.708	.500	.000	.417	.450	.583	.500	.000	.568	.000	.000	.000	.000	.671

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W AM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	0	3	0	3	1	0	1	2	2	6	0	8	0	0	0	0	
+15 mins.	0	3	0	3	**2**	0	**3**	**5**	2	**9**	0	**11**	0	0	0	0	
+30 mins.	0	**5**	1	**6**	0	0	0	0	**3**	1	0	4	0	0	0	0	
+45 mins.	0	3	**2**	5	1	0	1	2	0	2	0	2	0	0	0	0	
Total Volume	0	14	3	17	4	0	5	9	7	18	0	25	0	0	0	0	
% App. Total	0	82.4	17.6		44.4	0	55.6		28	72	0		0	0	0		
PHF	.000	.700	.375	.708	.500	.000	.417	.450	.583	.500	.000	.568	.000	.000	.000	.000	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 1

Groups Printed- 3 Axle Vehicles

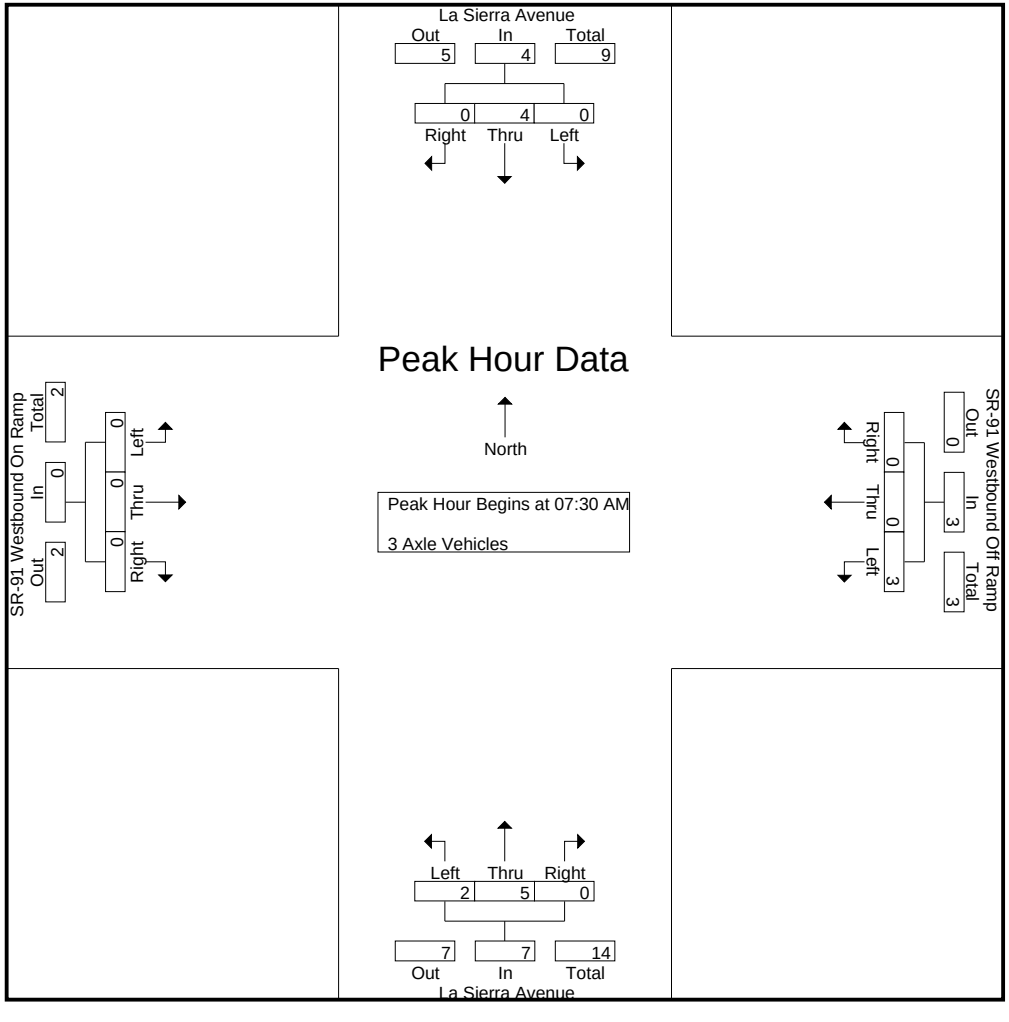
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	0	0	0	0	0	1	1	1	2	1	0	0	3	0	0	0	0	0	1	4	5
07:15 AM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
07:30 AM	0	4	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	4
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	2	2
Total	0	5	0	0	5	0	0	1	1	1	2	3	0	0	5	0	0	0	0	0	1	11	12
08:00 AM	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	1
08:15 AM	0	0	0	0	0	2	0	0	0	2	2	3	0	0	5	0	0	0	0	0	0	7	7
08:30 AM	0	1	0	1	1	1	0	0	0	1	0	2	0	0	2	0	0	0	0	0	1	4	5
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	1	0	1	1	4	0	0	0	4	2	5	0	0	7	0	0	0	0	0	1	12	13
Grand Total	0	6	0	1	6	4	0	1	1	5	4	8	0	0	12	0	0	0	0	0	2	23	25
Apprch %	0	100	0			80	0	20			33.3	66.7	0			0	0	0					
Total %	0	26.1	0		26.1	17.4	0	4.3		21.7	17.4	34.8	0		52.2	0	0	0		0	8	92	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	0	**4**	0	**4**	0	0	0	0	0	0	0	0	0	0	0	0	4
07:45 AM	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	2
08:00 AM	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	1
08:15 AM	0	0	0	0	**2**	0	0	**2**	**2**	**3**	0	**5**	0	0	0	0	**7**
Total Volume	0	4	0	4	3	0	0	3	2	5	0	7	0	0	0	0	14
% App. Total	0	100	0		100	0	0		28.6	71.4	0		0	0	0		
PHF	.000	.250	.000	.250	.375	.000	.000	.375	.250	.417	.000	.350	.000	.000	.000	.000	.500

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W AM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	0	**4**	0	**4**	0	0	0	0	0	0	0	0	0	0	0	0	
+15 mins.	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	
+30 mins.	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	
+45 mins.	0	0	0	0	**2**	0	0	**2**	**2**	**3**	0	**5**	0	0	0	0	
Total Volume	0	4	0	4	3	0	0	3	2	5	0	7	0	0	0	0	
% App. Total	0	100	0		100	0	0		28.6	71.4	0		0	0	0		
PHF	.000	.250	.000	.250	.375	.000	.000	.375	.250	.417	.000	.350	.000	.000	.000	.000	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 1

Groups Printed- 4+ Axle Trucks

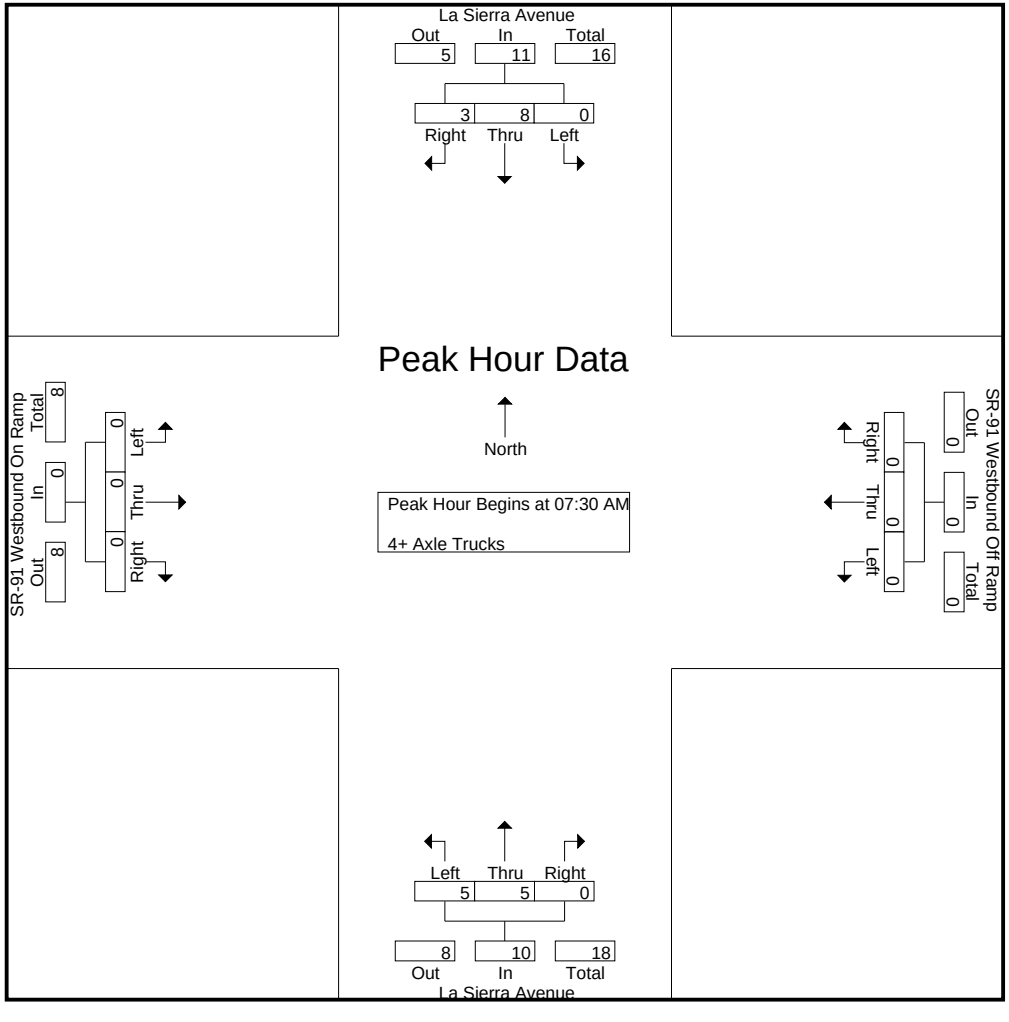
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	1	1	1	0	0	1	1	1	3	0	0	0	3	0	0	0	0	0	2	5	7
07:15 AM	0	4	0	0	4	0	0	2	1	2	2	1	0	0	3	0	0	0	0	0	1	9	10
07:30 AM	0	2	1	0	3	0	0	0	0	0	1	2	0	0	3	0	0	0	0	0	0	6	6
07:45 AM	0	2	0	0	2	0	0	0	0	0	2	1	0	0	3	0	0	0	0	0	0	5	5
Total	0	8	2	1	10	0	0	3	2	3	8	4	0	0	12	0	0	0	0	0	3	25	28
08:00 AM	0	2	1	0	3	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	5	5
08:15 AM	0	2	1	0	3	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	0	5	5
08:30 AM	0	1	0	0	1	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0	0	3	3
08:45 AM	0	0	0	0	0	2	0	0	0	2	0	2	0	0	2	0	0	0	0	0	0	4	4
Total	0	5	2	0	7	3	0	0	0	3	3	4	0	0	7	0	0	0	0	0	0	17	17
Grand Total	0	13	4	1	17	3	0	3	2	6	11	8	0	0	19	0	0	0	0	0	3	42	45
Apprch %	0	76.5	23.5			50	0	50			57.9	42.1	0			0	0	0					
Total %	0	31	9.5		40.5	7.1	0	7.1		14.3	26.2	19	0		45.2	0	0	0		0	6.7	93.3	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	0	**2**	**1**	**3**	0	0	0	0	1	**2**	0	**3**	0	0	0	0	**6**
07:45 AM	0	2	0	2	0	0	0	0	**2**	1	0	3	0	0	0	0	5
08:00 AM	0	2	1	3	0	0	0	0	0	2	0	2	0	0	0	0	5
08:15 AM	0	2	1	3	0	0	0	0	2	0	0	2	0	0	0	0	5
Total Volume	0	8	3	11	0	0	0	0	5	5	0	10	0	0	0	0	21
% App. Total	0	72.7	27.3		0	0	0		50	50	0		0	0	0		
PHF	.000	1.00	.750	.917	.000	.000	.000	.000	.625	.625	.000	.833	.000	.000	.000	.000	.875

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W AM
Site Code : 05124383
Start Date : 4/25/2024
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Site Code : 05124383
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	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	0	**2**	**1**	**3**	0	0	0	0	1	**2**	0	**3**	0	0	0	0	
+15 mins.	0	2	0	2	0	0	0	0	**2**	1	0	3	0	0	0	0	
+30 mins.	0	2	1	3	0	0	0	0	0	2	0	2	0	0	0	0	
+45 mins.	0	2	1	3	0	0	0	0	2	0	0	2	0	0	0	0	
Total Volume	0	8	3	11	0	0	0	0	5	5	0	10	0	0	0	0	
% App. Total	0	72.7	27.3		0	0	0		50	50	0		0	0	0		
PHF	.000	1.000	.750	.917	.000	.000	.000	.000	.625	.625	.000	.833	.000	.000	.000	.000	

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File Name : 01_RIV_LS_91W PM
Site Code : 05124383
Start Date : 4/25/2024
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Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

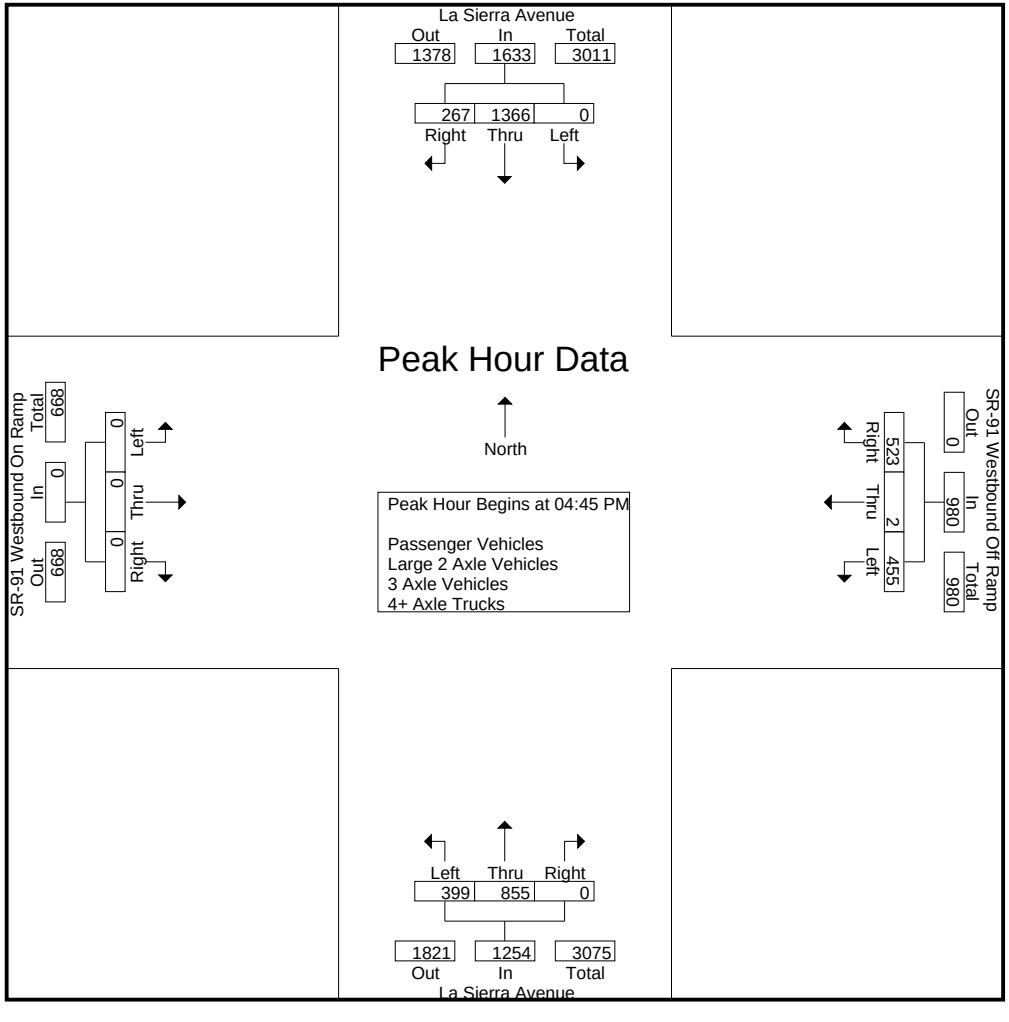
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	346	62	18	408	97	0	87	47	184	107	215	0	0	322	0	0	0	0	0	65	914	979
04:15 PM	0	299	47	19	346	60	0	85	48	145	113	207	0	0	320	0	0	0	0	0	67	811	878
04:30 PM	0	320	86	15	406	82	0	98	44	180	98	218	0	0	316	0	0	0	0	0	59	902	961
04:45 PM	0	286	61	9	347	105	0	118	53	223	110	199	0	0	309	0	0	0	0	0	62	879	941
Total	0	1251	256	61	1507	344	0	388	192	732	428	839	0	0	1267	0	0	0	0	0	253	3506	3759
05:00 PM	0	366	71	17	437	117	2	148	46	267	90	208	0	0	298	0	0	0	0	0	63	1002	1065
05:15 PM	0	363	73	23	436	121	0	135	54	256	101	205	0	0	306	0	0	0	0	0	77	998	1075
05:30 PM	0	351	62	23	413	112	0	122	52	234	98	243	0	0	341	0	0	0	0	0	75	988	1063
05:45 PM	0	274	50	20	324	108	0	121	55	229	83	242	0	0	325	0	0	0	0	0	75	878	953
Total	0	1354	256	83	1610	458	2	526	207	986	372	898	0	0	1270	0	0	0	0	0	290	3866	4156
Grand Total	0	2605	512	144	3117	802	2	914	399	1718	800	1737	0	0	2537	0	0	0	0	0	543	7372	7915
Apprch %	0	83.6	16.4			46.7	0.1	53.2			31.5	68.5	0			0	0	0					
Total %	0	35.3	6.9		42.3	10.9	0	12.4		23.3	10.9	23.6	0		34.4	0	0	0		0	6.9	93.1	
Passenger Vehicles	0	2571	511		3226	791	2	910		2100	779	1716	0		2495	0	0	0		0	0	0	7821
% Passenger Vehicles	0	98.7	99.8	100	98.9	98.6	100	99.6	99.5	99.2	97.4	98.8	0	0	98.3	0	0	0	0	0	0	0	98.8
Large 2 Axle Vehicles	0	20	1		21	6	0	1		7	10	15	0		25	0	0	0		0	0	0	53
% Large 2 Axle Vehicles	0	0.8	0.2	0	0.6	0.7	0	0.1	0	0.3	1.2	0.9	0	0	1	0	0	0	0	0	0	0	0.7
3 Axle Vehicles	0	5	0		5	1	0	0		1	3	2	0		5	0	0	0		0	0	0	11
% 3 Axle Vehicles	0	0.2	0	0	0.2	0.1	0	0	0	0	0.4	0.1	0	0	0.2	0	0	0	0	0	0	0	0.1
4+ Axle Trucks	0	9	0		9	4	0	3		9	8	4	0		12	0	0	0		0	0	0	30
% 4+ Axle Trucks	0	0.3	0	0	0.3	0.5	0	0.3	0.5	0.4	1	0.2	0	0	0.5	0	0	0	0	0	0	0	0.4

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	0	286	61	347	105	0	118	223	**110**	199	0	309	0	0	0	0	879
05:00 PM	0	**366**	71	**437**	117	**2**	**148**	**267**	90	208	0	298	0	0	0	0	**1002**
05:15 PM	0	363	**73**	436	**121**	0	135	256	101	205	0	306	0	0	0	0	998
05:30 PM	0	351	62	413	112	0	122	234	98	**243**	0	**341**	0	0	0	0	988
Total Volume	0	1366	267	1633	455	2	523	980	399	855	0	1254	0	0	0	0	3867
% App. Total	0	83.6	16.4		46.4	0.2	53.4		31.8	68.2	0		0	0	0		
PHF	.000	.933	.914	.934	.940	.250	.883	.918	.907	.880	.000	.919	.000	.000	.000	.000	.965

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City of Riverside
N/S: La Sierra Avenue
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Weather: Clear

File Name : 01_RIV_LS_91W PM
Site Code : 05124383
Start Date : 4/25/2024
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	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				05:00 PM				05:00 PM				04:00 PM				
+0 mins.	0	286	61	347	117	**2**	**148**	**267**	90	208	0	298	0	0	0	0	
+15 mins.	0	**366**	71	**437**	**121**	0	135	256	**101**	205	0	306	0	0	0	0	
+30 mins.	0	363	**73**	436	112	0	122	234	98	**243**	0	**341**	0	0	0	0	
+45 mins.	0	351	62	413	108	0	121	229	83	242	0	325	0	0	0	0	
Total Volume	0	1366	267	1633	458	2	526	986	372	898	0	1270	0	0	0	0	
% App. Total	0	83.6	16.4		46.5	0.2	53.3		29.3	70.7	0		0	0	0		
PHF	.000	.933	.914	.934	.946	.250	.889	.923	.921	.924	.000	.931	.000	.000	.000	.000	

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Groups Printed- Passenger Vehicles

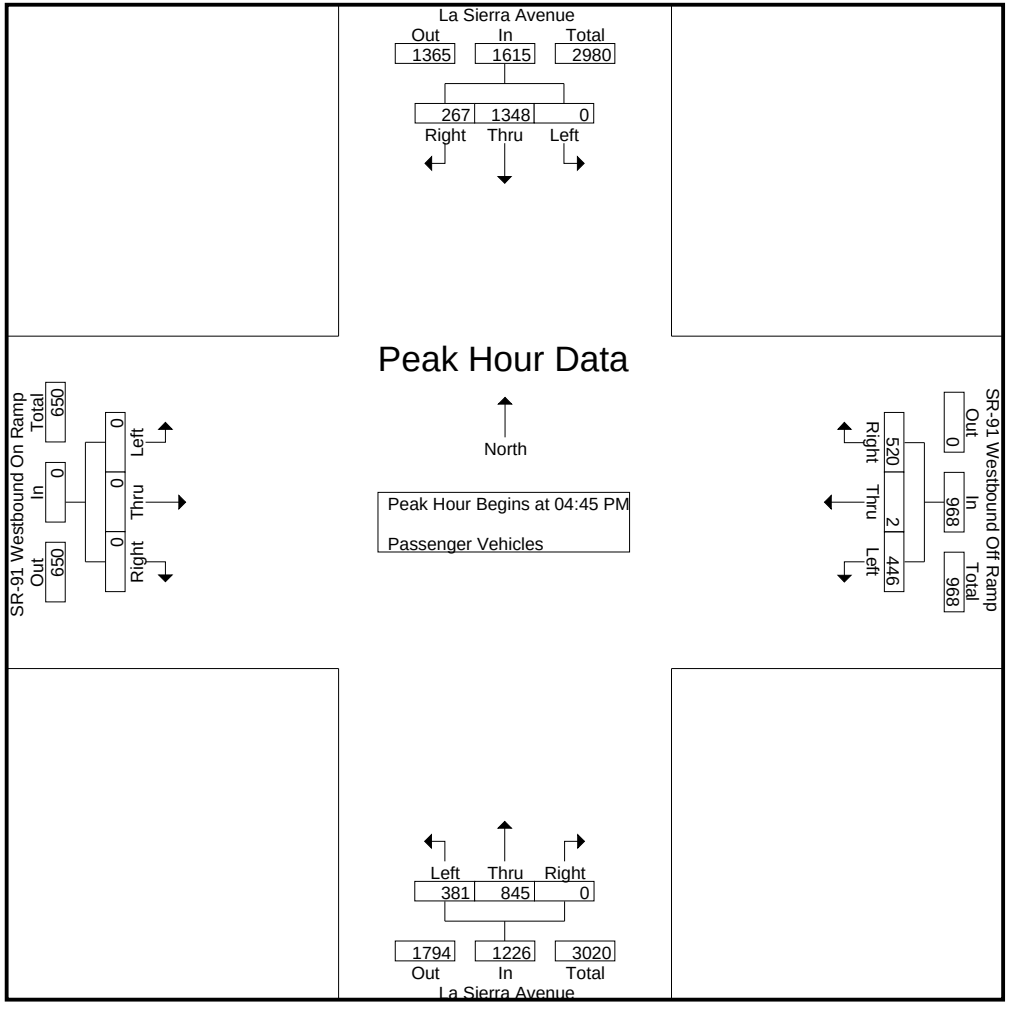
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	340	62	18	402	97	0	86	46	183	106	213	0	0	319	0	0	0	0	0	64	904	968
04:15 PM	0	293	46	19	339	60	0	85	48	145	113	206	0	0	319	0	0	0	0	0	67	803	870
04:30 PM	0	319	86	15	405	82	0	98	44	180	96	213	0	0	309	0	0	0	0	0	59	894	953
04:45 PM	0	283	61	9	344	102	0	117	52	219	103	197	0	0	300	0	0	0	0	0	61	863	924
Total	0	1235	255	61	1490	341	0	386	190	727	418	829	0	0	1247	0	0	0	0	0	251	3464	3715
05:00 PM	0	362	71	17	433	115	2	147	46	264	86	206	0	0	292	0	0	0	0	0	63	989	1052
05:15 PM	0	356	73	23	429	117	0	135	54	252	96	202	0	0	298	0	0	0	0	0	77	979	1056
05:30 PM	0	347	62	23	409	112	0	121	52	233	96	240	0	0	336	0	0	0	0	0	75	978	1053
05:45 PM	0	271	50	20	321	106	0	121	55	227	83	239	0	0	322	0	0	0	0	0	75	870	945
Total	0	1336	256	83	1592	450	2	524	207	976	361	887	0	0	1248	0	0	0	0	0	290	3816	4106
Grand Total	0	2571	511	144	3082	791	2	910	397	1703	779	1716	0	0	2495	0	0	0	0	0	541	7280	7821
Apprch %	0	83.4	16.6			46.4	0.1	53.4			31.2	68.8	0			0	0	0					
Total %	0	35.3	7		42.3	10.9	0	12.5		23.4	10.7	23.6	0		34.3	0	0	0		0	6.9	93.1	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	0	283	61	344	102	0	117	219	**103**	197	0	300	0	0	0	0	863
05:00 PM	0	**362**	71	**433**	115	**2**	**147**	**264**	86	206	0	292	0	0	0	0	**989**
05:15 PM	0	356	**73**	429	**117**	0	135	252	96	202	0	298	0	0	0	0	979
05:30 PM	0	347	62	409	112	0	121	233	96	**240**	0	**336**	0	0	0	0	978
Total Volume	0	1348	267	1615	446	2	520	968	381	845	0	1226	0	0	0	0	3809
% App. Total	0	83.5	16.5		46.1	0.2	53.7		31.1	68.9	0		0	0	0		
PHF	.000	.931	.914	.932	.953	.250	.884	.917	.925	.880	.000	.912	.000	.000	.000	.000	.963

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Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:45 PM				04:45 PM				04:45 PM				
+0 mins.	0	283	61	344	102	0	117	219	**103**	197	0	300	0	0	0	0	
+15 mins.	0	**362**	71	**433**	115	**2**	**147**	**264**	86	206	0	292	0	0	0	0	
+30 mins.	0	356	**73**	429	**117**	0	135	252	96	202	0	298	0	0	0	0	
+45 mins.	0	347	62	409	112	0	121	233	96	**240**	0	**336**	0	0	0	0	
Total Volume	0	1348	267	1615	446	2	520	968	381	845	0	1226	0	0	0	0	
% App. Total	0	83.5	16.5		46.1	0.2	53.7		31.1	68.9	0		0	0	0		
PHF	.000	.931	.914	.932	.953	.250	.884	.917	.925	.880	.000	.912	.000	.000	.000	.000	

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Groups Printed- Large 2 Axle Vehicles

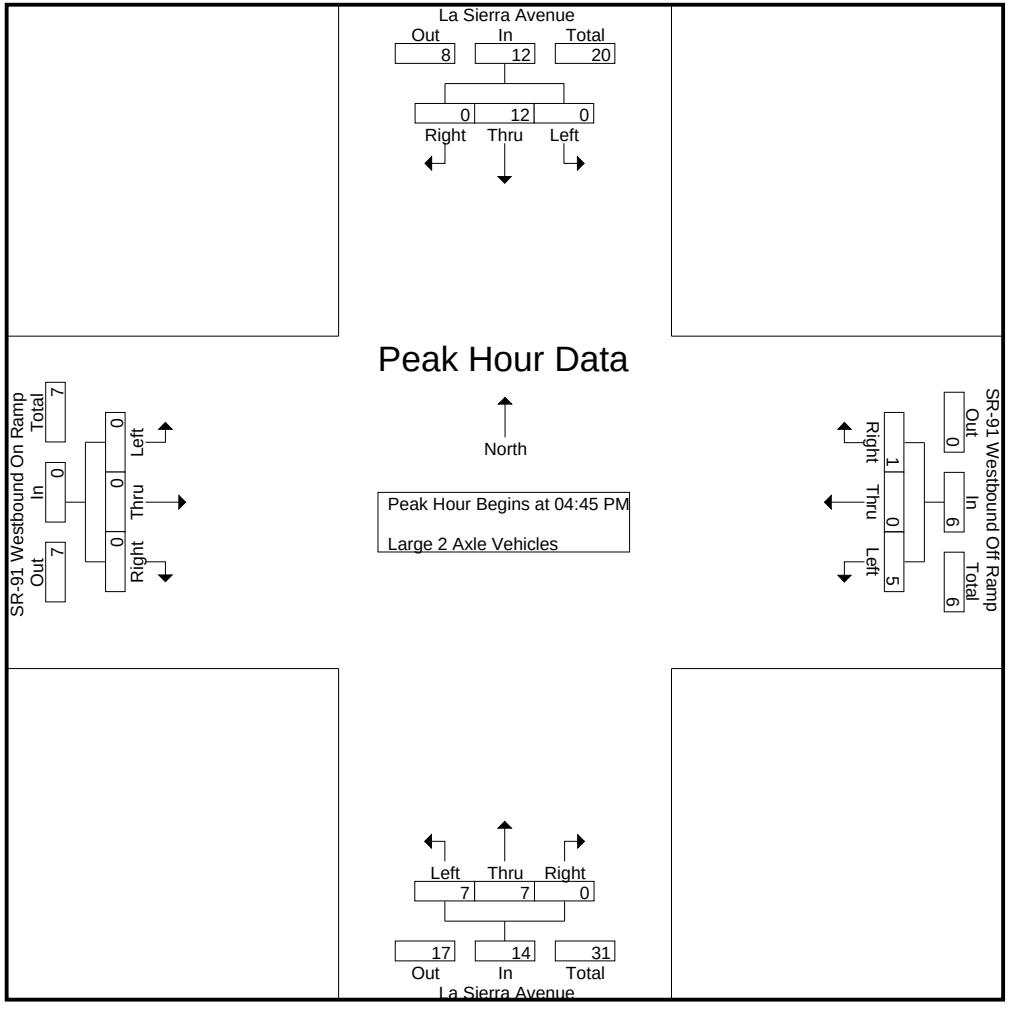
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	2	0	0	2	0	0	0	0	0	1	2	0	0	3	0	0	0	0	0	0	5	5
04:15 PM	0	3	1	0	4	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	5	5
04:30 PM	0	0	0	0	0	0	0	0	0	0	2	2	0	0	4	0	0	0	0	0	0	4	4
04:45 PM	0	1	0	0	1	1	0	0	0	1	5	2	0	0	7	0	0	0	0	0	0	9	9
Total	0	6	1	0	7	1	0	0	0	1	8	7	0	0	15	0	0	0	0	0	0	23	23
05:00 PM	0	4	0	0	4	1	0	0	0	1	0	2	0	0	2	0	0	0	0	0	0	7	7
05:15 PM	0	5	0	0	5	3	0	0	0	3	1	1	0	0	2	0	0	0	0	0	0	10	10
05:30 PM	0	2	0	0	2	0	0	1	0	1	1	2	0	0	3	0	0	0	0	0	0	6	6
05:45 PM	0	3	0	0	3	1	0	0	0	1	0	3	0	0	3	0	0	0	0	0	0	7	7
Total	0	14	0	0	14	5	0	1	0	6	2	8	0	0	10	0	0	0	0	0	0	30	30
Grand Total	0	20	1	0	21	6	0	1	0	7	10	15	0	0	25	0	0	0	0	0	0	53	53
Apprch %	0	95.2	4.8			85.7	0	14.3			40	60	0			0	0	0					
Total %	0	37.7	1.9		39.6	11.3	0	1.9		13.2	18.9	28.3	0		47.2	0	0	0		0	0	100	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	0	1	0	1	1	0	0	1	**5**	**2**	0	**7**	0	0	0	0	9
05:00 PM	0	4	0	4	1	0	0	1	0	2	0	2	0	0	0	0	7
05:15 PM	0	**5**	0	**5**	**3**	0	0	**3**	1	1	0	2	0	0	0	0	**10**
05:30 PM	0	2	0	2	0	0	**1**	1	1	2	0	3	0	0	0	0	6
Total Volume	0	12	0	12	5	0	1	6	7	7	0	14	0	0	0	0	32
% App. Total	0	100	0		83.3	0	16.7		50	50	0		0	0	0		
PHF	.000	.600	.000	.600	.417	.000	.250	.500	.350	.875	.000	.500	.000	.000	.000	.000	.800

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
(951) 268-6268

City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W PM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W PM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:45 PM				04:45 PM				04:45 PM				
+0 mins.	0	1	0	1	1	0	0	1	**5**	**2**	0	**7**	0	0	0	0	
+15 mins.	0	4	0	4	1	0	0	1	0	2	0	2	0	0	0	0	
+30 mins.	0	**5**	0	**5**	**3**	0	0	**3**	1	1	0	2	0	0	0	0	
+45 mins.	0	2	0	2	0	0	**1**	1	1	2	0	3	0	0	0	0	
Total Volume	0	12	0	12	5	0	1	6	7	7	0	14	0	0	0	0	
% App. Total	0	100	0		83.3	0	16.7		50	50	0		0	0	0		
PHF	.000	.600	.000	.600	.417	.000	.250	.500	.350	.875	.000	.500	.000	.000	.000	.000	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W PM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 1

Groups Printed- 3 Axle Vehicles

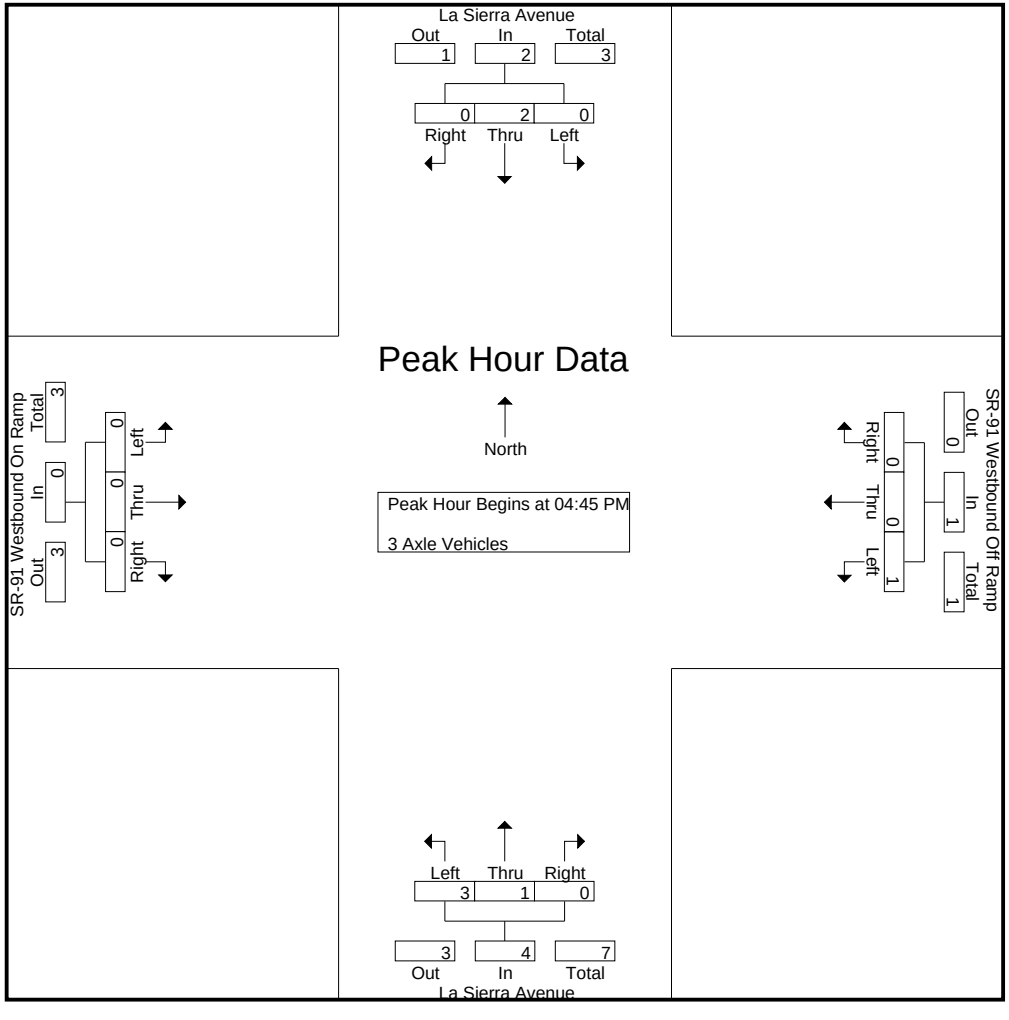
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
04:15 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
04:30 PM	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	2	2
04:45 PM	0	1	0	0	1	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	2	2
Total	0	4	0	0	4	0	0	0	0	0	1	1	0	0	2	0	0	0	0	0	0	6	6
05:00 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	1	1
05:15 PM	0	0	0	0	0	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0	0	2	2
05:30 PM	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	2	2
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	1	0	0	1	1	0	0	0	1	2	1	0	0	3	0	0	0	0	0	0	5	5
Grand Total	0	5	0	0	5	1	0	0	0	1	3	2	0	0	5	0	0	0	0	0	0	11	11
Apprch %	0	100	0			100	0	0			60	40	0			0	0	0					
Total %	0	45.5	0		45.5	9.1	0	0		9.1	27.3	18.2	0		45.5	0	0	0		0	0	100	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	0	**1**	0	**1**	0	0	0	0	**1**	0	0	**1**	0	0	0	0	**2**
05:00 PM	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	1
05:15 PM	0	0	0	0	**1**	0	0	**1**	1	0	0	1	0	0	0	0	2
05:30 PM	0	1	0	1	0	0	0	0	0	**1**	0	1	0	0	0	0	2
Total Volume	0	2	0	2	1	0	0	1	3	1	0	4	0	0	0	0	7
% App. Total	0	100	0		100	0	0		75	25	0		0	0	0		
PHF	.000	.500	.000	.500	.250	.000	.000	.250	.750	.250	.000	1.00	.000	.000	.000	.000	.875

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
(951) 268-6268

City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W PM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W PM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:45 PM				04:45 PM				04:45 PM				
+0 mins.	0	**1**	0	**1**	0	0	0	0	**1**	0	0	**1**	0	0	0	0	
+15 mins.	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	
+30 mins.	0	0	0	0	**1**	0	0	**1**	1	0	0	1	0	0	0	0	
+45 mins.	0	1	0	1	0	0	0	0	0	**1**	0	1	0	0	0	0	
Total Volume	0	2	0	2	1	0	0	1	3	1	0	4	0	0	0	0	
% App. Total	0	100	0		100	0	0		75	25	0		0	0	0		
PHF	.000	.500	.000	.500	.250	.000	.000	.250	.750	.250	.000	1.000	.000	.000	.000	.000	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W PM
Site Code : 05124383
Start Date : 4/25/2024

Groups Printed- 4+ Axle Trucks

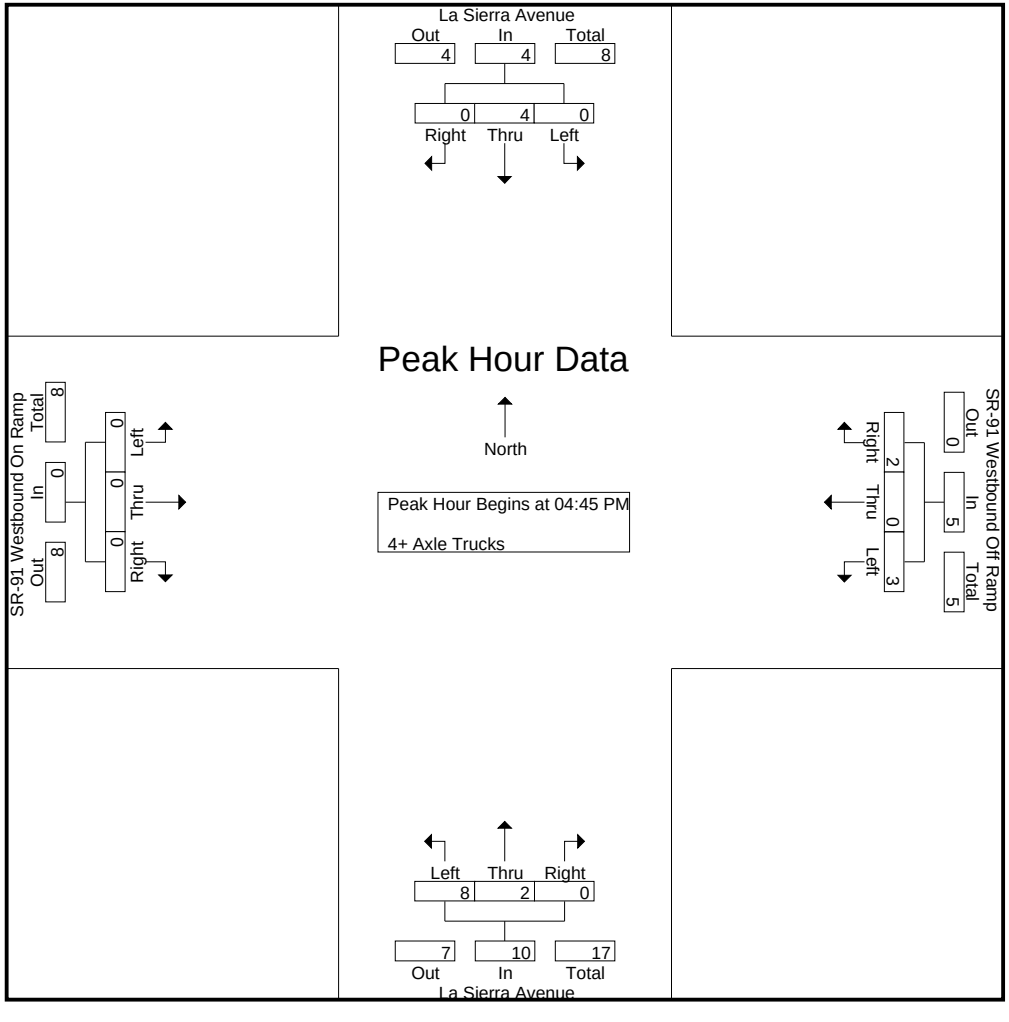
	La Sierra Avenue Southbound					SR-91 Westbound Off Ramp Westbound					La Sierra Avenue Northbound					SR-91 Westbound On Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	3	0	0	3	0	0	1	1	1	0	0	0	0	0	0	0	0	0	0	1	4	5
04:15 PM	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	2	2
04:45 PM	0	1	0	0	1	2	0	1	1	3	1	0	0	0	1	0	0	0	0	0	1	5	6
Total	0	6	0	0	6	2	0	2	2	4	1	2	0	0	3	0	0	0	0	0	2	13	15
05:00 PM	0	0	0	0	0	1	0	1	0	2	3	0	0	0	3	0	0	0	0	0	0	5	5
05:15 PM	0	2	0	0	2	0	0	0	0	0	3	2	0	0	5	0	0	0	0	0	0	7	7
05:30 PM	0	1	0	0	1	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	2	2
05:45 PM	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	1
Total	0	3	0	0	3	2	0	1	0	3	7	2	0	0	9	0	0	0	0	0	0	15	15
Grand Total	0	9	0	0	9	4	0	3	2	7	8	4	0	0	12	0	0	0	0	0	2	28	30
Apprch %	0	100	0			57.1	0	42.9			66.7	33.3	0			0	0	0					
Total %	0	32.1	0		32.1	14.3	0	10.7		25	28.6	14.3	0		42.9	0	0	0		0	6.7	93.3	

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	0	1	0	1	**2**	0	**1**	**3**	1	0	0	1	0	0	0	0	5
05:00 PM	0	0	0	0	1	0	1	2	**3**	0	0	3	0	0	0	0	5
05:15 PM	0	**2**	0	**2**	0	0	0	0	3	**2**	0	**5**	0	0	0	0	**7**
05:30 PM	0	1	0	1	0	0	0	0	1	0	0	1	0	0	0	0	2
Total Volume	0	4	0	4	3	0	2	5	8	2	0	10	0	0	0	0	19
% App. Total	0	100	0		60	0	40		80	20	0		0	0	0		
PHF	.000	.500	.000	.500	.375	.000	.500	.417	.667	.250	.000	.500	.000	.000	.000	.000	.679

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(951) 268-6268

City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W PM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Westbound Ramps
Weather: Clear

File Name : 01_RIV_LS_91W PM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Westbound Off Ramp Westbound				La Sierra Avenue Northbound				SR-91 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:45 PM				04:45 PM				04:45 PM				
+0 mins.	0	1	0	1	**2**	0	**1**	**3**	1	0	0	1	0	0	0	0	
+15 mins.	0	0	0	0	1	0	1	2	**3**	0	0	3	0	0	0	0	
+30 mins.	0	**2**	0	**2**	0	0	0	0	3	**2**	0	**5**	0	0	0	0	
+45 mins.	0	1	0	1	0	0	0	0	1	0	0	1	0	0	0	0	
Total Volume	0	4	0	4	3	0	2	5	8	2	0	10	0	0	0	0	
% App. Total	0	100	0		60	0	40		80	20	0		0	0	0		
PHF	.000	.500	.000	.500	.375	.000	.500	.417	.667	.250	.000	.500	.000	.000	.000	.000	

Location: Riverside
N/S: La Sierra Avenue
E/W: SR-91 WB Ramps

Date: 4/25/2024
Day: Thursday

PEDESTRIANS

	North Leg La Sierra Avenue	East Leg SR-91 WB Ramps	South Leg La Sierra Avenue	West Leg SR-91 WB Ramps	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	0	1	0	0	1
7:15 AM	0	0	0	5	5
7:30 AM	0	0	0	1	1
7:45 AM	0	1	0	0	1
8:00 AM	0	1	0	2	3
8:15 AM	0	0	0	1	1
8:30 AM	0	0	0	0	0
8:45 AM	0	3	0	0	3
TOTAL VOLUMES:	0	6	0	9	15

	North Leg La Sierra Avenue	East Leg SR-91 WB Ramps	South Leg La Sierra Avenue	West Leg SR-91 WB Ramps	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	0	0	0	3	3
4:15 PM	0	2	0	3	5
4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	1	1
5:00 PM	0	0	0	2	2
5:15 PM	0	3	0	0	3
5:30 PM	0	0	0	2	2
5:45 PM	0	0	0	2	2
TOTAL VOLUMES:	0	5	0	13	18

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951-268-6268

Location: Riverside
N/S: La Sierra Avenue
E/W: SR-91 WB Ramps

Date: 4/25/2024
Day: Thursday

BICYCLES

	Southbound La Sierra Avenue			Westbound SR-91 WB Ramps			Northbound La Sierra Avenue			Eastbound SR-91 WB Ramps			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	1	0	0	0	0	0	1
7:30 AM	0	2	0	0	0	0	1	0	0	0	0	0	3
7:45 AM	0	2	0	0	0	0	0	0	0	0	0	0	2
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	4	0	0	0	0	2	0	0	0	0	0	6

	Southbound La Sierra Avenue			Westbound SR-91 WB Ramps			Northbound La Sierra Avenue			Eastbound SR-91 WB Ramps			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	1	0	0	0	0	1
4:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	1
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	1	1	0	0	0	2
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	1	0	0	0	0	0	1	0	0	0	0	2
5:45 PM	0	0	0	0	0	0	0	1	0	0	0	0	1
TOTAL VOLUMES:	0	2	0	0	0	0	0	4	1	0	0	0	7

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 1

Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

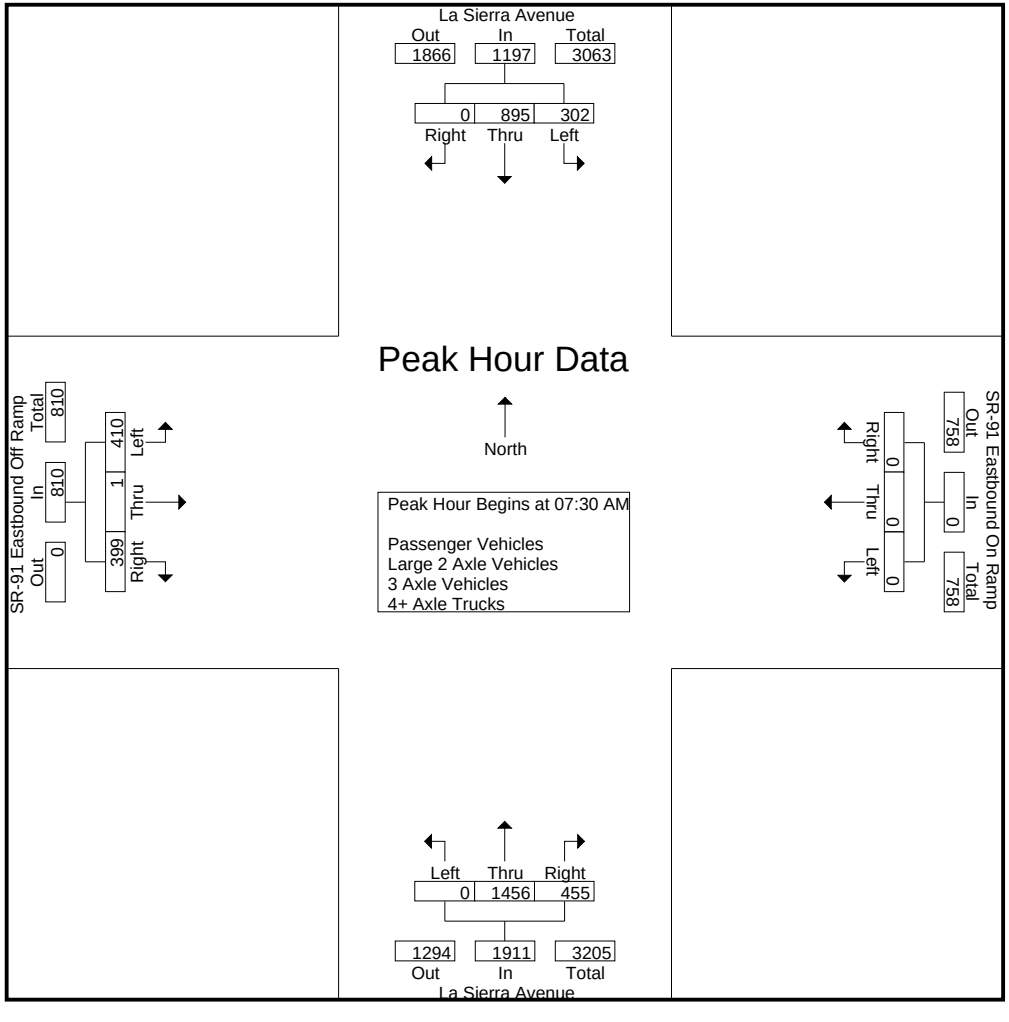
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	87	97	0	0	184	0	0	0	0	0	0	235	141	29	376	54	0	75	51	129	80	689	769
07:15 AM	122	139	0	0	261	0	0	0	0	0	0	275	125	28	400	80	0	71	48	151	76	812	888
07:30 AM	87	184	0	0	271	0	0	0	0	0	0	286	136	38	422	91	1	103	56	195	94	888	982
07:45 AM	79	263	0	0	342	0	0	0	0	0	0	375	98	30	473	115	0	90	53	205	83	1020	1103
Total	375	683	0	0	1058	0	0	0	0	0	0	1171	500	125	1671	340	1	339	208	680	333	3409	3742
08:00 AM	66	255	0	0	321	0	0	0	0	0	0	372	110	41	482	106	0	119	52	225	93	1028	1121
08:15 AM	70	193	0	0	263	0	0	0	0	0	0	423	111	37	534	98	0	87	57	185	94	982	1076
08:30 AM	83	139	0	0	222	0	0	0	0	0	0	291	109	24	400	70	0	96	60	166	84	788	872
08:45 AM	79	133	0	0	212	0	0	0	0	0	0	299	119	23	418	70	1	86	59	157	82	787	869
Total	298	720	0	0	1018	0	0	0	0	0	0	1385	449	125	1834	344	1	388	228	733	353	3585	3938
Grand Total	673	1403	0	0	2076	0	0	0	0	0	0	2556	949	250	3505	684	2	727	436	1413	686	6994	7680
Apprch %	32.4	67.6	0			0	0	0			0	72.9	27.1			48.4	0.1	51.5					
Total %	9.6	20.1	0		29.7	0	0	0		0	0	36.5	13.6		50.1	9.8	0	10.4		20.2	8.9	91.1	
Passenger Vehicles	655	1356	0		2011	0	0	0		0	0	2485	915		3642	676	2	672		1751	0	0	7404
% Passenger Vehicles	97.3	96.7	0	0	96.9	0	0	0	0	0	0	97.2	96.4	96.8	97	98.8	100	92.4	92	94.7	0	0	96.4
Large 2 Axle Vehicles	5	26	0		31	0	0	0		0	0	43	24		72	4	0	26		46	0	0	149
% Large 2 Axle Vehicles	0.7	1.9	0	0	1.5	0	0	0	0	0	0	1.7	2.5	2	1.9	0.6	0	3.6	3.7	2.5	0	0	1.9
3 Axle Vehicles	6	9	0		15	0	0	0		0	0	11	3		15	1	0	10		20	0	0	50
% 3 Axle Vehicles	0.9	0.6	0	0	0.7	0	0	0	0	0	0	0.4	0.3	0.4	0.4	0.1	0	1.4	2.1	1.1	0	0	0.7
4+ Axle Trucks	7	12	0		19	0	0	0		0	0	17	7		26	3	0	19		32	0	0	77
% 4+ Axle Trucks	1	0.9	0	0	0.9	0	0	0	0	0	0	0.7	0.7	0.8	0.7	0.4	0	2.6	2.3	1.7	0	0	1

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	**87**	184	0	271	0	0	0	0	0	286	**136**	422	91	**1**	103	195	888
07:45 AM	79	**263**	0	**342**	0	0	0	0	0	375	98	473	**115**	0	90	205	1020
08:00 AM	66	255	0	321	0	0	0	0	0	372	110	482	106	0	**119**	**225**	**1028**
08:15 AM	70	193	0	263	0	0	0	0	0	**423**	111	**534**	98	0	87	185	982
Total Volume	302	895	0	1197	0	0	0	0	0	1456	455	1911	410	1	399	810	3918
% App. Total	25.2	74.8	0		0	0	0		0	76.2	23.8		50.6	0.1	49.3		
PHF	.868	.851	.000	.875	.000	.000	.000	.000	.000	.861	.836	.895	.891	.250	.838	.900	.953

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
(951) 268-6268

City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:00 AM				07:30 AM				07:30 AM				
+0 mins.	**87**	184	0	271	0	0	0	0	0	286	**136**	422	91	**1**	103	195	
+15 mins.	79	**263**	0	**342**	0	0	0	0	0	375	98	473	**115**	0	90	205	
+30 mins.	66	255	0	321	0	0	0	0	0	372	110	482	106	0	**119**	**225**	
+45 mins.	70	193	0	263	0	0	0	0	0	**423**	111	**534**	98	0	87	185	
Total Volume	302	895	0	1197	0	0	0	0	0	1456	455	1911	410	1	399	810	
% App. Total	25.2	74.8	0		0	0	0		0	76.2	23.8		50.6	0.1	49.3		
PHF	.868	.851	.000	.875	.000	.000	.000	.000	.000	.861	.836	.895	.891	.250	.838	.900	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024

Groups Printed- Passenger Vehicles

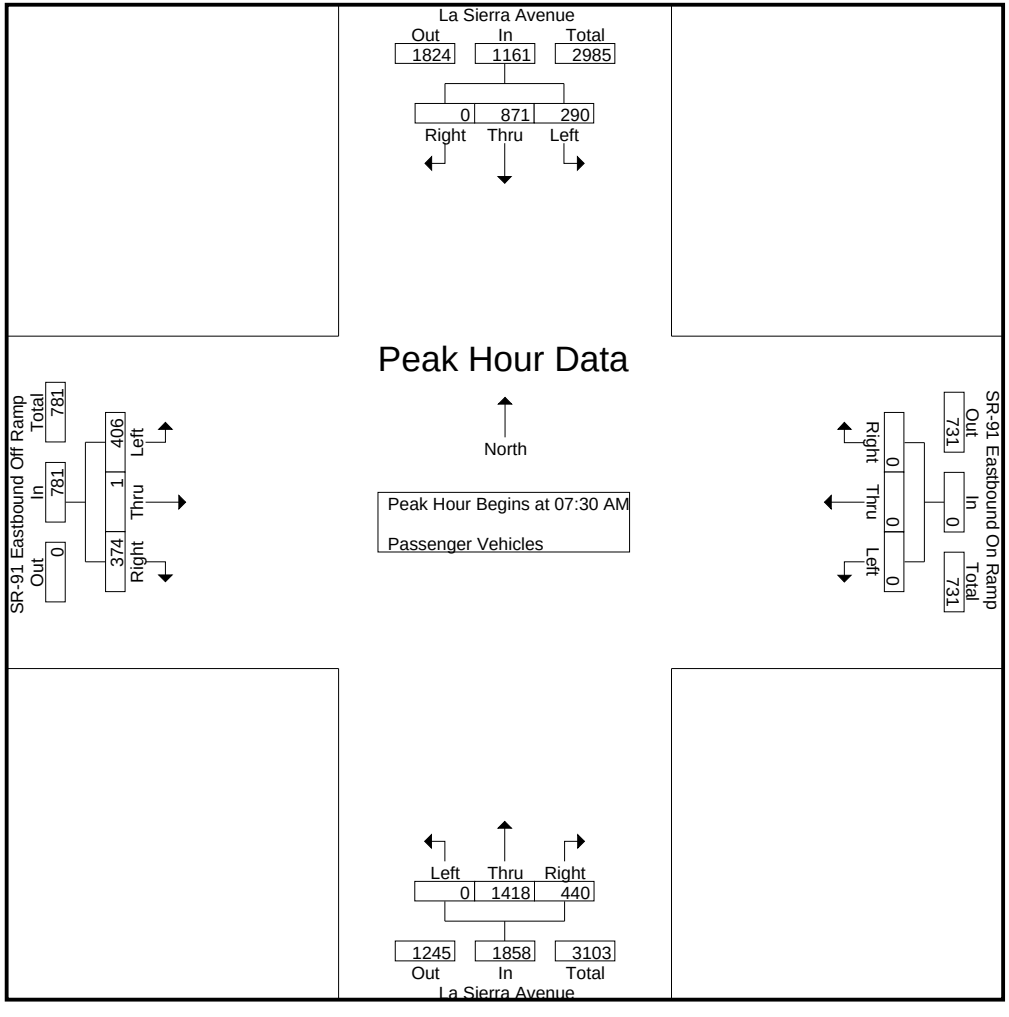
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	85	93	0	0	178	0	0	0	0	0	0	226	138	28	364	54	0	71	47	125	75	667	742
07:15 AM	119	128	0	0	247	0	0	0	0	0	0	265	120	26	385	77	0	63	42	140	68	772	840
07:30 AM	81	179	0	0	260	0	0	0	0	0	0	277	131	38	408	89	1	98	54	188	92	856	948
07:45 AM	78	256	0	0	334	0	0	0	0	0	0	361	96	30	457	113	0	88	52	201	82	992	1074
Total	363	656	0	0	1019	0	0	0	0	0	0	1129	485	122	1614	333	1	320	195	654	317	3287	3604
08:00 AM	63	251	0	0	314	0	0	0	0	0	0	366	103	39	469	106	0	107	46	213	85	996	1081
08:15 AM	68	185	0	0	253	0	0	0	0	0	0	414	110	36	524	98	0	81	53	179	89	956	1045
08:30 AM	82	134	0	0	216	0	0	0	0	0	0	283	105	22	388	69	0	84	50	153	72	757	829
08:45 AM	79	130	0	0	209	0	0	0	0	0	0	293	112	23	405	70	1	80	57	151	80	765	845
Total	292	700	0	0	992	0	0	0	0	0	0	1356	430	120	1786	343	1	352	206	696	326	3474	3800
Grand Total	655	1356	0	0	2011	0	0	0	0	0	0	2485	915	242	3400	676	2	672	401	1350	643	6761	7404
Apprch %	32.6	67.4	0			0	0	0			0	73.1	26.9			50.1	0.1	49.8					
Total %	9.7	20.1	0		29.7	0	0	0		0	0	36.8	13.5		50.3	10	0	9.9		20	8.7	91.3	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	**81**	179	0	260	0	0	0	0	0	277	**131**	408	89	**1**	98	188	856
07:45 AM	78	**256**	0	**334**	0	0	0	0	0	361	96	457	**113**	0	88	201	992
08:00 AM	63	251	0	314	0	0	0	0	0	366	103	469	106	0	**107**	**213**	**996**
08:15 AM	68	185	0	253	0	0	0	0	0	**414**	110	**524**	98	0	81	179	956
Total Volume	290	871	0	1161	0	0	0	0	0	1418	440	1858	406	1	374	781	3800
% App. Total	25	75	0		0	0	0		0	76.3	23.7		52	0.1	47.9		
PHF	.895	.851	.000	.869	.000	.000	.000	.000	.000	.856	.840	.886	.898	.250	.874	.917	.954

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	**81**	179	0	260	0	0	0	0	0	277	**131**	408	89	**1**	98	188	
+15 mins.	78	**256**	0	**334**	0	0	0	0	0	361	96	457	**113**	0	88	201	
+30 mins.	63	251	0	314	0	0	0	0	0	366	103	469	106	0	**107**	**213**	
+45 mins.	68	185	0	253	0	0	0	0	0	**414**	110	**524**	98	0	81	179	
Total Volume	290	871	0	1161	0	0	0	0	0	1418	440	1858	406	1	374	781	
% App. Total	25	75	0		0	0	0		0	76.3	23.7		52	0.1	47.9		
PHF	.895	.851	.000	.869	.000	.000	.000	.000	.000	.856	.840	.886	.898	.250	.874	.917	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024

Groups Printed- Large 2 Axle Vehicles

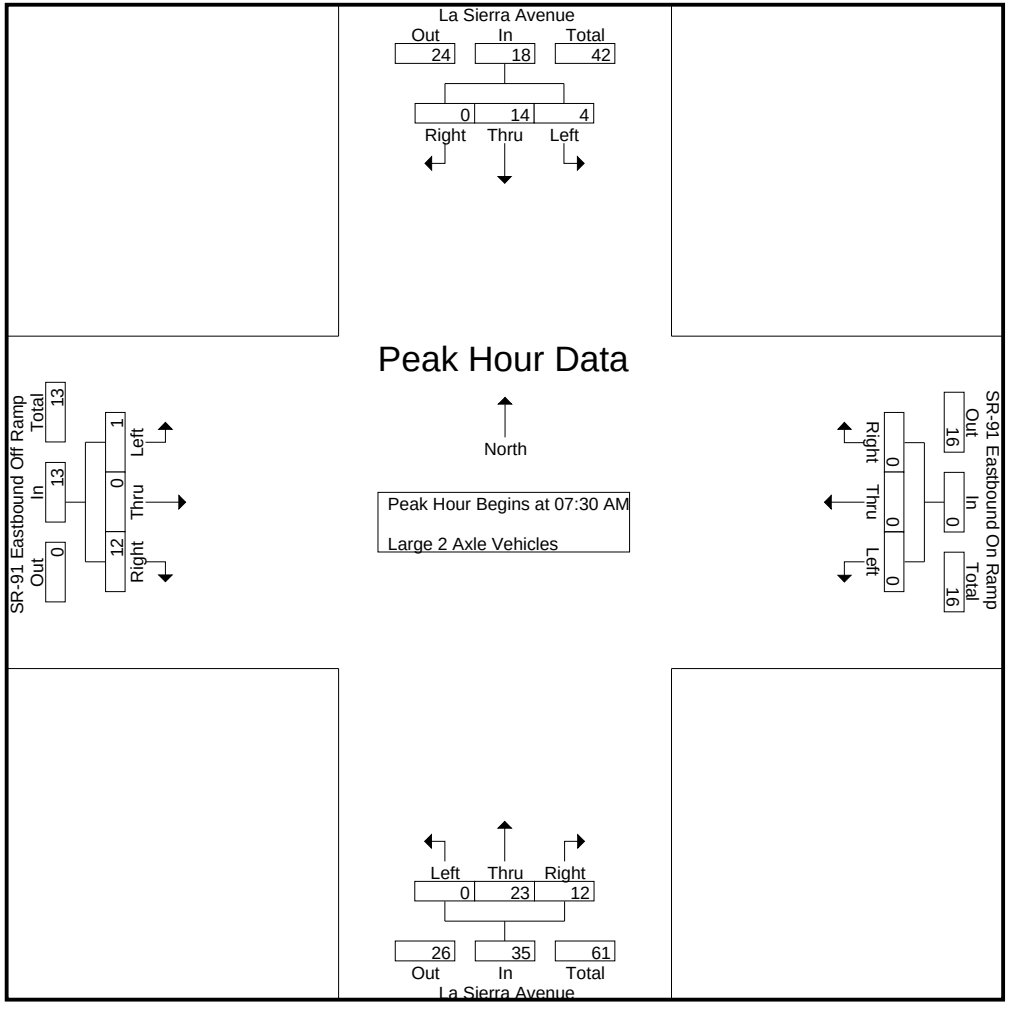
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	1	4	0	0	5	0	0	0	0	0	0	4	1	0	5	0	0	2	2	2	2	12	14
07:15 AM	0	6	0	0	6	0	0	0	0	0	0	7	4	2	11	2	0	5	3	7	5	24	29
07:30 AM	0	4	0	0	4	0	0	0	0	0	0	7	4	0	11	0	0	1	0	1	0	16	16
07:45 AM	0	5	0	0	5	0	0	0	0	0	0	10	1	0	11	1	0	1	0	2	0	18	18
Total	1	19	0	0	20	0	0	0	0	0	0	28	10	2	38	3	0	9	5	12	7	70	77
08:00 AM	3	1	0	0	4	0	0	0	0	0	0	4	6	2	10	0	0	7	4	7	6	21	27
08:15 AM	1	4	0	0	5	0	0	0	0	0	0	2	1	0	3	0	0	3	2	3	2	11	13
08:30 AM	0	2	0	0	2	0	0	0	0	0	0	5	3	1	8	1	0	4	4	5	5	15	20
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	4	4	0	8	0	0	3	1	3	1	11	12
Total	4	7	0	0	11	0	0	0	0	0	0	15	14	3	29	1	0	17	11	18	14	58	72
Grand Total	5	26	0	0	31	0	0	0	0	0	0	43	24	5	67	4	0	26	16	30	21	128	149
Apprch %	16.1	83.9	0			0	0	0			0	64.2	35.8			13.3	0	86.7					
Total %	3.9	20.3	0		24.2	0	0	0		0	0	33.6	18.8		52.3	3.1	0	20.3		23.4	14.1	85.9	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	0	4	0	4	0	0	0	0	0	7	4	**11**	0	0	1	1	16
07:45 AM	0	**5**	0	**5**	0	0	0	0	0	**10**	1	11	**1**	0	1	2	18
08:00 AM	**3**	1	0	4	0	0	0	0	0	4	**6**	10	0	0	**7**	**7**	**21**
08:15 AM	1	4	0	5	0	0	0	0	0	2	1	3	0	0	3	3	11
Total Volume	4	14	0	18	0	0	0	0	0	23	12	35	1	0	12	13	66
% App. Total	22.2	77.8	0		0	0	0		0	65.7	34.3		7.7	0	92.3		
PHF	.333	.700	.000	.900	.000	.000	.000	.000	.000	.575	.500	.795	.250	.000	.429	.464	.786

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(951) 268-6268

City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	0	4	0	4	0	0	0	0	0	7	4	**11**	0	0	1	1	
+15 mins.	0	**5**	0	**5**	0	0	0	0	0	**10**	1	11	**1**	0	1	2	
+30 mins.	**3**	1	0	4	0	0	0	0	0	4	**6**	10	0	0	**7**	**7**	
+45 mins.	1	4	0	5	0	0	0	0	0	2	1	3	0	0	3	3	
Total Volume	4	14	0	18	0	0	0	0	0	23	12	35	1	0	12	13	
% App. Total	22.2	77.8	0		0	0	0		0	65.7	34.3		7.7	0	92.3		
PHF	.333	.700	.000	.900	.000	.000	.000	.000	.000	.575	.500	.795	.250	.000	.429	.464	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024

Groups Printed- 3 Axle Vehicles

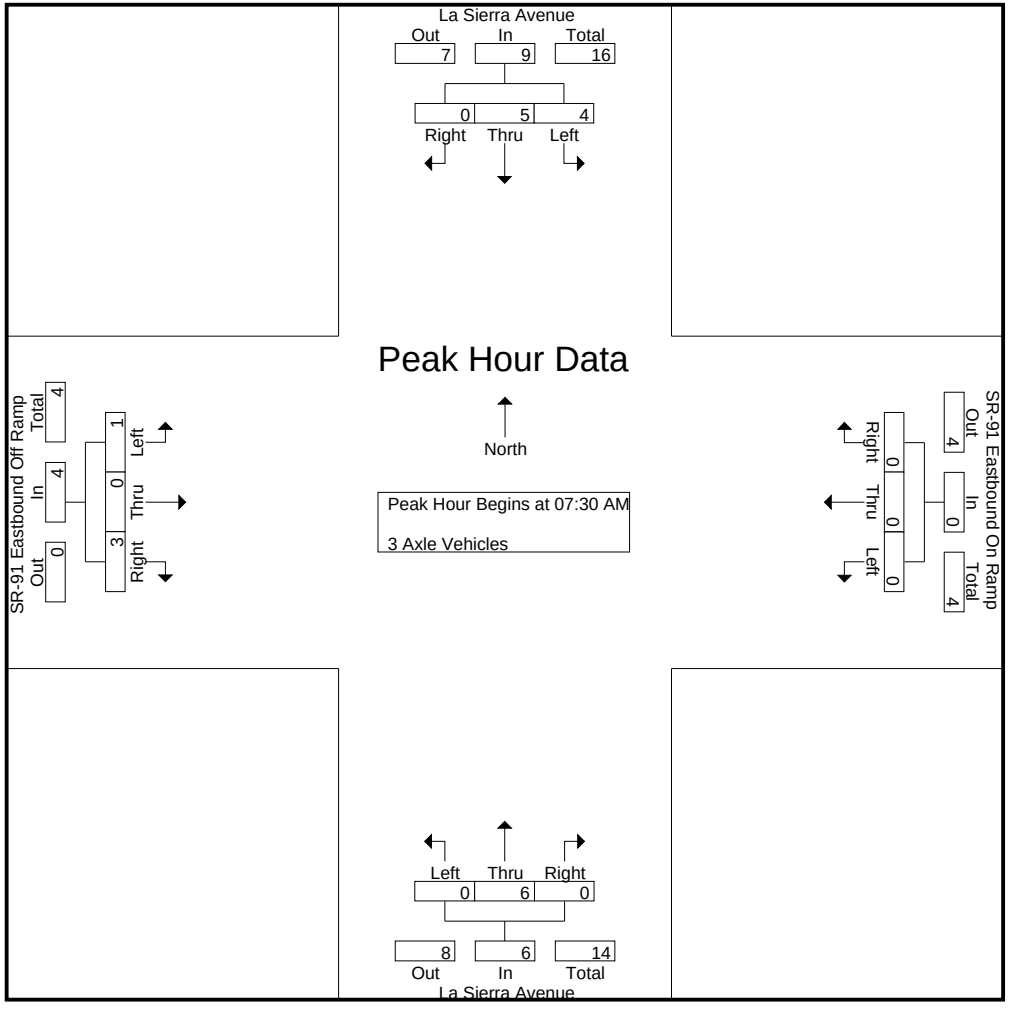
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	1	0	0	0	1	0	0	0	0	0	0	3	1	1	4	0	0	2	2	2	3	7	10
07:15 AM	1	0	0	0	1	0	0	0	0	0	0	0	1	0	1	0	0	1	1	1	1	3	4
07:30 AM	3	1	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	5	5
07:45 AM	1	0	0	0	1	0	0	0	0	0	0	1	0	0	1	1	0	0	0	1	0	3	3
Total	6	1	0	0	7	0	0	0	0	0	0	4	2	1	6	1	0	4	3	5	4	18	22
08:00 AM	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2
08:15 AM	0	2	0	0	2	0	0	0	0	0	0	5	0	0	5	0	0	2	2	2	2	9	11
08:30 AM	0	3	0	0	3	0	0	0	0	0	0	2	0	0	2	0	0	3	3	3	3	8	11
08:45 AM	0	1	0	0	1	0	0	0	0	0	0	0	1	0	1	0	0	1	1	1	1	3	4
Total	0	8	0	0	8	0	0	0	0	0	0	7	1	0	8	0	0	6	6	6	6	22	28
Grand Total	6	9	0	0	15	0	0	0	0	0	0	11	3	1	14	1	0	10	9	11	10	40	50
Apprch %	40	60	0			0	0	0			0	78.6	21.4			9.1	0	90.9					
Total %	15	22.5	0		37.5	0	0	0		0	0	27.5	7.5		35	2.5	0	25		27.5	20	80	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	**3**	1	0	**4**	0	0	0	0	0	0	0	0	0	0	1	1	5
07:45 AM	1	0	0	1	0	0	0	0	0	1	0	1	**1**	0	0	1	3
08:00 AM	0	**2**	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
08:15 AM	0	2	0	2	0	0	0	0	0	**5**	0	**5**	0	0	**2**	**2**	**9**
Total Volume	4	5	0	9	0	0	0	0	0	6	0	6	1	0	3	4	19
% App. Total	44.4	55.6	0		0	0	0		0	100	0		25	0	75		
PHF	.333	.625	.000	.563	.000	.000	.000	.000	.000	.300	.000	.300	.250	.000	.375	.500	.528

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	**3**	1	0	**4**	0	0	0	0	0	0	0	0	0	0	1	1	
+15 mins.	1	0	0	1	0	0	0	0	0	1	0	1	**1**	0	0	1	
+30 mins.	0	**2**	0	2	0	0	0	0	0	0	0	0	0	0	0	0	
+45 mins.	0	2	0	2	0	0	0	0	0	**5**	0	**5**	0	0	**2**	**2**	
Total Volume	4	5	0	9	0	0	0	0	0	6	0	6	1	0	3	4	
% App. Total	44.4	55.6	0		0	0	0		0	100	0		25	0	75		
PHF	.333	.625	.000	.563	.000	.000	.000	.000	.000	.300	.000	.300	.250	.000	.375	.500	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 1

Groups Printed- 4+ Axle Trucks

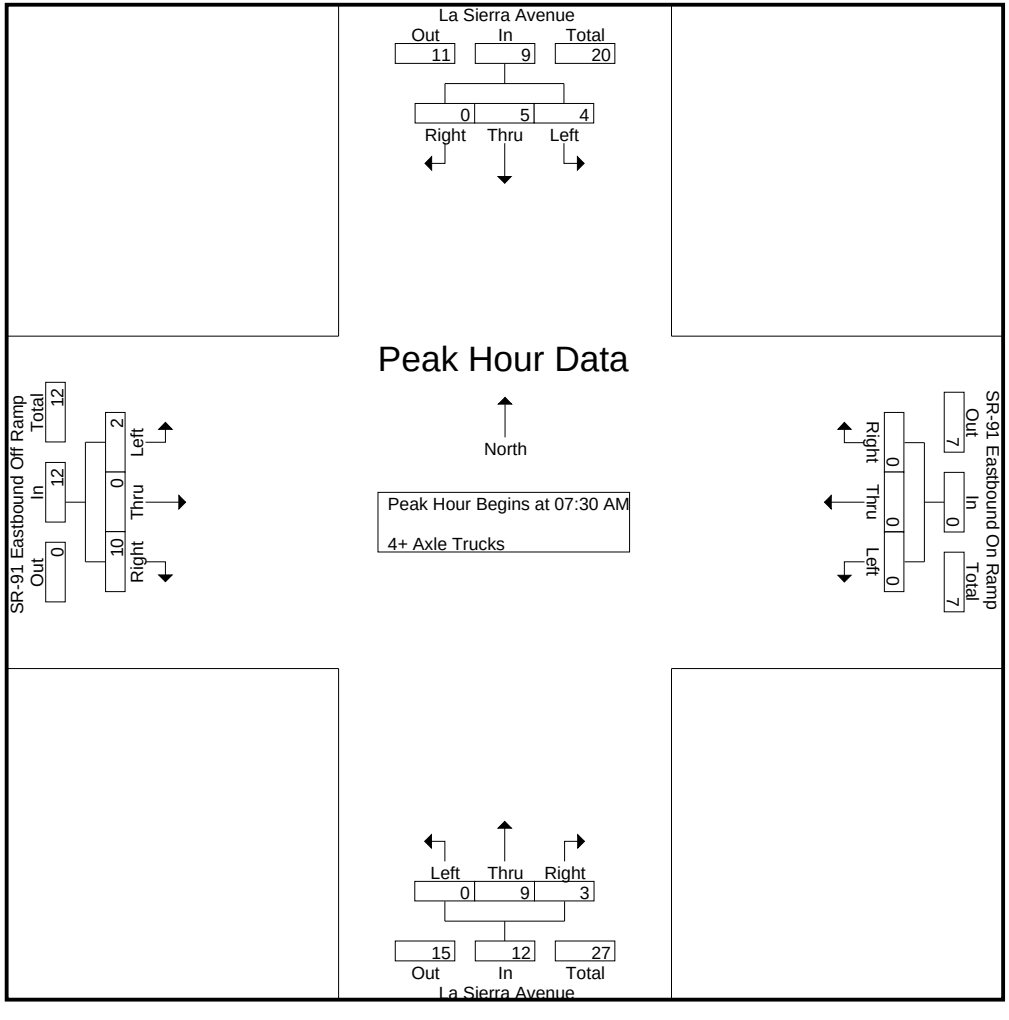
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	2	1	0	3	0	0	0	0	0	0	3	3
07:15 AM	2	5	0	0	7	0	0	0	0	0	0	3	0	0	3	1	0	2	2	3	2	13	15
07:30 AM	3	0	0	0	3	0	0	0	0	0	0	2	1	0	3	2	0	3	2	5	2	11	13
07:45 AM	0	2	0	0	2	0	0	0	0	0	0	3	1	0	4	0	0	1	1	1	1	7	8
Total	5	7	0	0	12	0	0	0	0	0	0	10	3	0	13	3	0	6	5	9	5	34	39
08:00 AM	0	1	0	0	1	0	0	0	0	0	0	2	1	0	3	0	0	5	2	5	2	9	11
08:15 AM	1	2	0	0	3	0	0	0	0	0	0	2	0	1	2	0	0	1	0	1	1	6	7
08:30 AM	1	0	0	0	1	0	0	0	0	0	0	1	1	1	2	0	0	5	3	5	4	8	12
08:45 AM	0	2	0	0	2	0	0	0	0	0	0	2	2	0	4	0	0	2	0	2	0	8	8
Total	2	5	0	0	7	0	0	0	0	0	0	7	4	2	11	0	0	13	5	13	7	31	38
Grand Total	7	12	0	0	19	0	0	0	0	0	0	17	7	2	24	3	0	19	10	22	12	65	77
Apprch %	36.8	63.2	0			0	0	0			0	70.8	29.2			13.6	0	86.4					
Total %	10.8	18.5	0		29.2	0	0	0		0	0	26.2	10.8		36.9	4.6	0	29.2		33.8	15.6	84.4	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	**3**	0	0	**3**	0	0	0	0	0	2	**1**	3	**2**	0	3	**5**	**11**
07:45 AM	0	**2**	0	2	0	0	0	0	0	**3**	1	**4**	0	0	1	1	7
08:00 AM	0	1	0	1	0	0	0	0	0	2	1	3	0	0	**5**	5	9
08:15 AM	1	2	0	3	0	0	0	0	0	2	0	2	0	0	1	1	6
Total Volume	4	5	0	9	0	0	0	0	0	9	3	12	2	0	10	12	33
% App. Total	44.4	55.6	0		0	0	0		0	75	25		16.7	0	83.3		
PHF	.333	.625	.000	.750	.000	.000	.000	.000	.000	.750	.750	.750	.250	.000	.500	.600	.750

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024
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File Name : 02_RIV_LS_91E AM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:30 AM to 08:15 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:30 AM				
+0 mins.	**3**	0	0	**3**	0	0	0	0	0	2	**1**	3	**2**	0	3	**5**	
+15 mins.	0	**2**	0	2	0	0	0	0	0	**3**	1	**4**	0	0	1	1	
+30 mins.	0	1	0	1	0	0	0	0	0	2	1	3	0	0	**5**	5	
+45 mins.	1	2	0	3	0	0	0	0	0	2	0	2	0	0	1	1	
Total Volume	4	5	0	9	0	0	0	0	0	9	3	12	2	0	10	12	
% App. Total	44.4	55.6	0		0	0	0		0	75	25		16.7	0	83.3		
PHF	.333	.625	.000	.750	.000	.000	.000	.000	.000	.750	.750	.750	.250	.000	.500	.600	

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File Name : 02_RIV_LS_91E PM
Site Code : 05124383
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Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

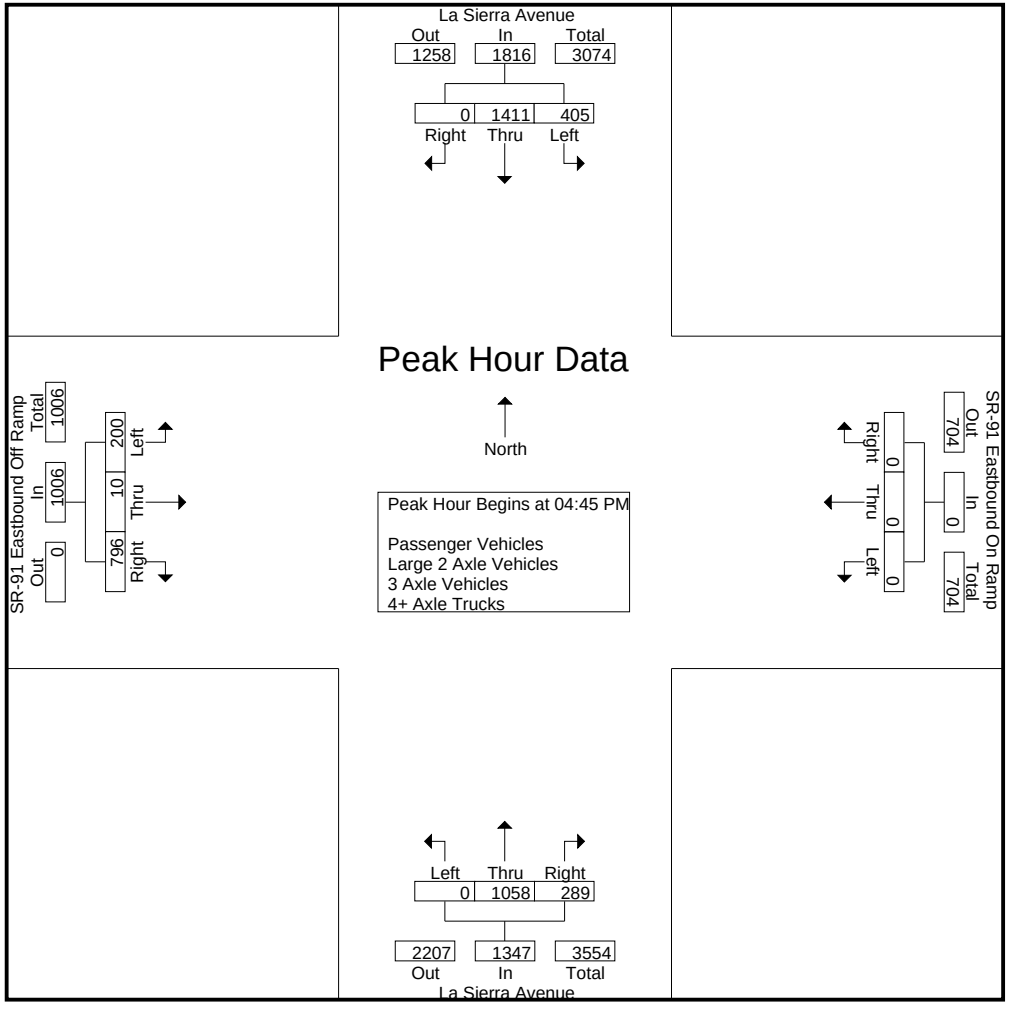
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	116	324	0	0	440	0	0	0	0	0	0	267	92	35	359	43	1	197	82	241	117	1040	1157
04:15 PM	76	266	0	0	342	0	0	0	0	0	0	277	88	41	365	49	4	174	71	227	112	934	1046
04:30 PM	88	304	0	0	392	0	0	0	0	0	0	276	82	41	358	46	10	181	66	237	107	987	1094
04:45 PM	99	306	0	0	405	0	0	0	0	0	0	272	66	27	338	45	3	197	78	245	105	988	1093
Total	379	1200	0	0	1579	0	0	0	0	0	0	1092	328	144	1420	183	18	749	297	950	441	3949	4390
05:00 PM	111	362	0	0	473	0	0	0	0	0	0	289	79	33	368	45	0	190	53	235	86	1076	1162
05:15 PM	92	392	0	0	484	0	0	0	0	0	0	236	64	35	300	54	1	206	55	261	90	1045	1135
05:30 PM	103	351	0	0	454	0	0	0	0	0	0	261	80	40	341	56	6	203	45	265	85	1060	1145
05:45 PM	90	292	0	0	382	0	0	0	0	0	0	241	71	36	312	61	6	189	52	256	88	950	1038
Total	396	1397	0	0	1793	0	0	0	0	0	0	1027	294	144	1321	216	13	788	205	1017	349	4131	4480
Grand Total	775	2597	0	0	3372	0	0	0	0	0	0	2119	622	288	2741	399	31	1537	502	1967	790	8080	8870
Apprch %	23	77	0			0	0	0			0	77.3	22.7			20.3	1.6	78.1					
Total %	9.6	32.1	0		41.7	0	0	0		0	0	26.2	7.7		33.9	4.9	0.4	19		24.3	8.9	91.1	
Passenger Vehicles	764	2568	0		3332	0	0	0		0	0	2076	608		2965	396	30	1503		2419	0	0	8716
% Passenger Vehicles	98.6	98.9	0	0	98.8	0	0	0	0	0	0	98	97.7	97.6	97.9	99.2	96.8	97.8	97.6	98	0	0	98.3
Large 2 Axle Vehicles	6	22	0		28	0	0	0		0	0	25	8		36	2	0	15		22	0	0	86
% Large 2 Axle Vehicles	0.8	0.8	0	0	0.8	0	0	0	0	0	0	1.2	1.3	1	1.2	0.5	0	1	1	0.9	0	0	1
3 Axle Vehicles	3	1	0		4	0	0	0		0	0	3	3		7	1	1	6		9	0	0	20
% 3 Axle Vehicles	0.4	0	0	0	0.1	0	0	0	0	0	0	0.1	0.5	0.3	0.2	0.3	3.2	0.4	0.2	0.4	0	0	0.2
4+ Axle Trucks	2	6	0		8	0	0	0		0	0	15	3		21	0	0	13		19	0	0	48
% 4+ Axle Trucks	0.3	0.2	0	0	0.2	0	0	0	0	0	0	0.7	0.5	1	0.7	0	0	0.8	1.2	0.8	0	0	0.5

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	99	306	0	405	0	0	0	0	0	272	66	338	45	3	197	245	988
05:00 PM	**111**	362	0	473	0	0	0	0	0	**289**	79	**368**	45	0	190	235	**1076**
05:15 PM	92	**392**	0	**484**	0	0	0	0	0	236	64	300	54	1	**206**	261	1045
05:30 PM	103	351	0	454	0	0	0	0	0	261	**80**	341	**56**	**6**	203	**265**	1060
Total Volume	405	1411	0	1816	0	0	0	0	0	1058	289	1347	200	10	796	1006	4169
% App. Total	22.3	77.7	0		0	0	0		0	78.5	21.5		19.9	1	79.1		
PHF	.912	.900	.000	.938	.000	.000	.000	.000	.000	.915	.903	.915	.893	.417	.966	.949	.969

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City of Riverside
N/S: La Sierra Avenue
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Weather: Clear

File Name : 02_RIV_LS_91E PM
Site Code : 05124383
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	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:00 PM				04:15 PM				05:00 PM				
+0 mins.	99	306	0	405	0	0	0	0	0	277	**88**	365	45	0	190	235	
+15 mins.	**111**	362	0	473	0	0	0	0	0	276	82	358	54	1	**206**	261	
+30 mins.	92	**392**	0	**484**	0	0	0	0	0	272	66	338	56	**6**	203	**265**	
+45 mins.	103	351	0	454	0	0	0	0	0	**289**	79	**368**	**61**	6	189	256	
Total Volume	405	1411	0	1816	0	0	0	0	0	1114	315	1429	216	13	788	1017	
% App. Total	22.3	77.7	0		0	0	0		0	78	22		21.2	1.3	77.5		
PHF	.912	.900	.000	.938	.000	.000	.000	.000	.000	.964	.895	.971	.885	.542	.956	.959	

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Groups Printed- Passenger Vehicles

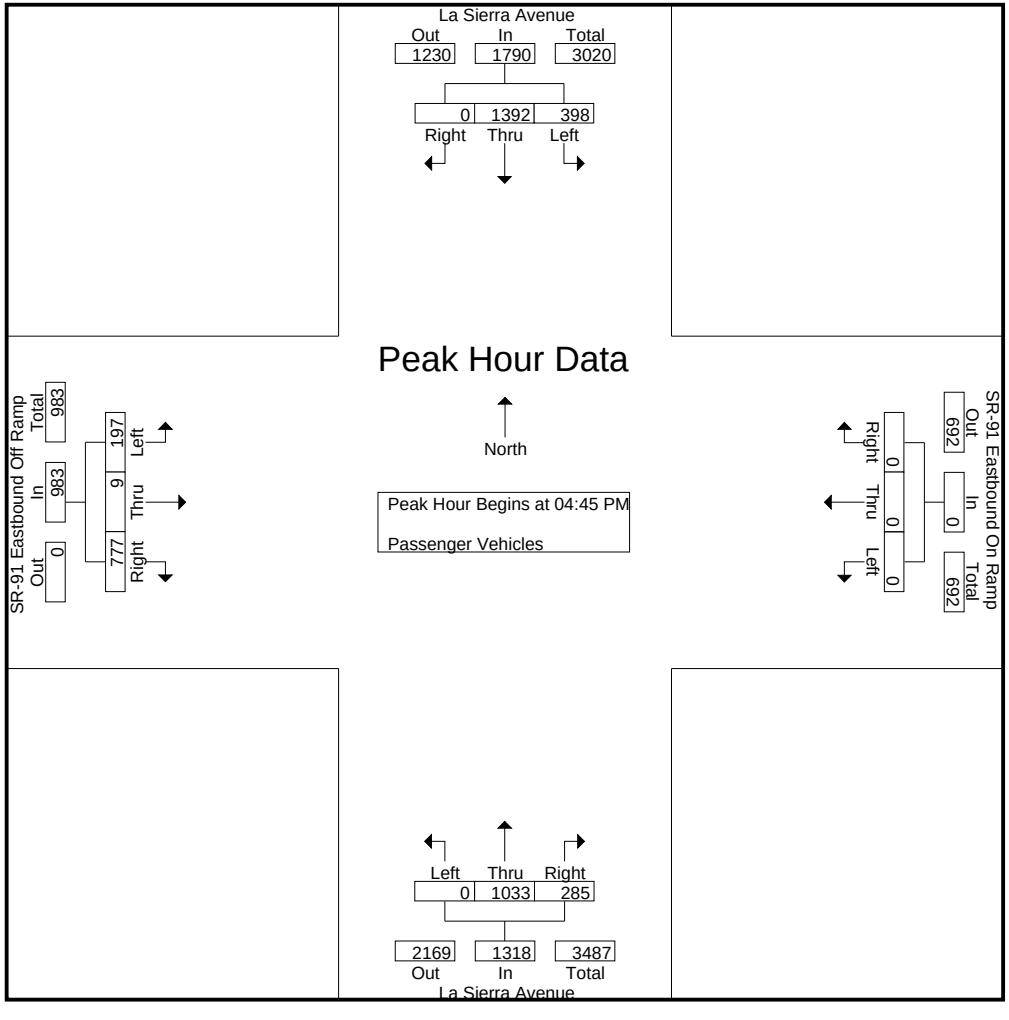
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	115	321	0	0	436	0	0	0	0	0	0	263	90	34	353	43	1	191	81	235	115	1024	1139
04:15 PM	74	264	0	0	338	0	0	0	0	0	0	274	85	39	359	49	4	170	68	223	107	920	1027
04:30 PM	88	303	0	0	391	0	0	0	0	0	0	268	80	40	348	46	10	178	65	234	105	973	1078
04:45 PM	97	303	0	0	400	0	0	0	0	0	0	264	65	27	329	44	3	191	76	238	103	967	1070
Total	374	1191	0	0	1565	0	0	0	0	0	0	1069	320	140	1389	182	18	730	290	930	430	3884	4314
05:00 PM	109	358	0	0	467	0	0	0	0	0	0	281	78	33	359	45	0	189	53	234	86	1060	1146
05:15 PM	91	382	0	0	473	0	0	0	0	0	0	232	63	35	295	52	1	199	52	252	87	1020	1107
05:30 PM	101	349	0	0	450	0	0	0	0	0	0	256	79	39	335	56	5	198	43	259	82	1044	1126
05:45 PM	89	288	0	0	377	0	0	0	0	0	0	238	68	34	306	61	6	187	52	254	86	937	1023
Total	390	1377	0	0	1767	0	0	0	0	0	0	1007	288	141	1295	214	12	773	200	999	341	4061	4402
Grand Total	764	2568	0	0	3332	0	0	0	0	0	0	2076	608	281	2684	396	30	1503	490	1929	771	7945	8716
Apprch %	22.9	77.1	0			0	0	0			0	77.3	22.7			20.5	1.6	77.9					
Total %	9.6	32.3	0		41.9	0	0	0		0	0	26.1	7.7		33.8	5	0.4	18.9		24.3	8.8	91.2	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	97	303	0	400	0	0	0	0	0	264	65	329	44	3	191	238	967
05:00 PM	**109**	358	0	467	0	0	0	0	0	**281**	78	**359**	45	0	189	234	**1060**
05:15 PM	91	**382**	0	**473**	0	0	0	0	0	232	63	295	52	1	**199**	252	1020
05:30 PM	101	349	0	450	0	0	0	0	0	256	**79**	335	**56**	**5**	198	**259**	1044
Total Volume	398	1392	0	1790	0	0	0	0	0	1033	285	1318	197	9	777	983	4091
% App. Total	22.2	77.8	0		0	0	0		0	78.4	21.6		20	0.9	79		
PHF	.913	.911	.000	.946	.000	.000	.000	.000	.000	.919	.902	.918	.879	.450	.976	.949	.965

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File Name : 02_RIV_LS_91E PM
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	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:45 PM				04:45 PM				04:45 PM				
+0 mins.	97	303	0	400	0	0	0	0	0	264	65	329	44	3	191	238	
+15 mins.	**109**	358	0	467	0	0	0	0	0	**281**	78	**359**	45	0	189	234	
+30 mins.	91	**382**	0	**473**	0	0	0	0	0	232	63	295	52	1	**199**	252	
+45 mins.	101	349	0	450	0	0	0	0	0	256	**79**	335	**56**	**5**	198	**259**	
Total Volume	398	1392	0	1790	0	0	0	0	0	1033	285	1318	197	9	777	983	
% App. Total	22.2	77.8	0		0	0	0		0	78.4	21.6		20	0.9	79		
PHF	.913	.911	.000	.946	.000	.000	.000	.000	.000	.919	.902	.918	.879	.450	.976	.949	

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Groups Printed- Large 2 Axle Vehicles

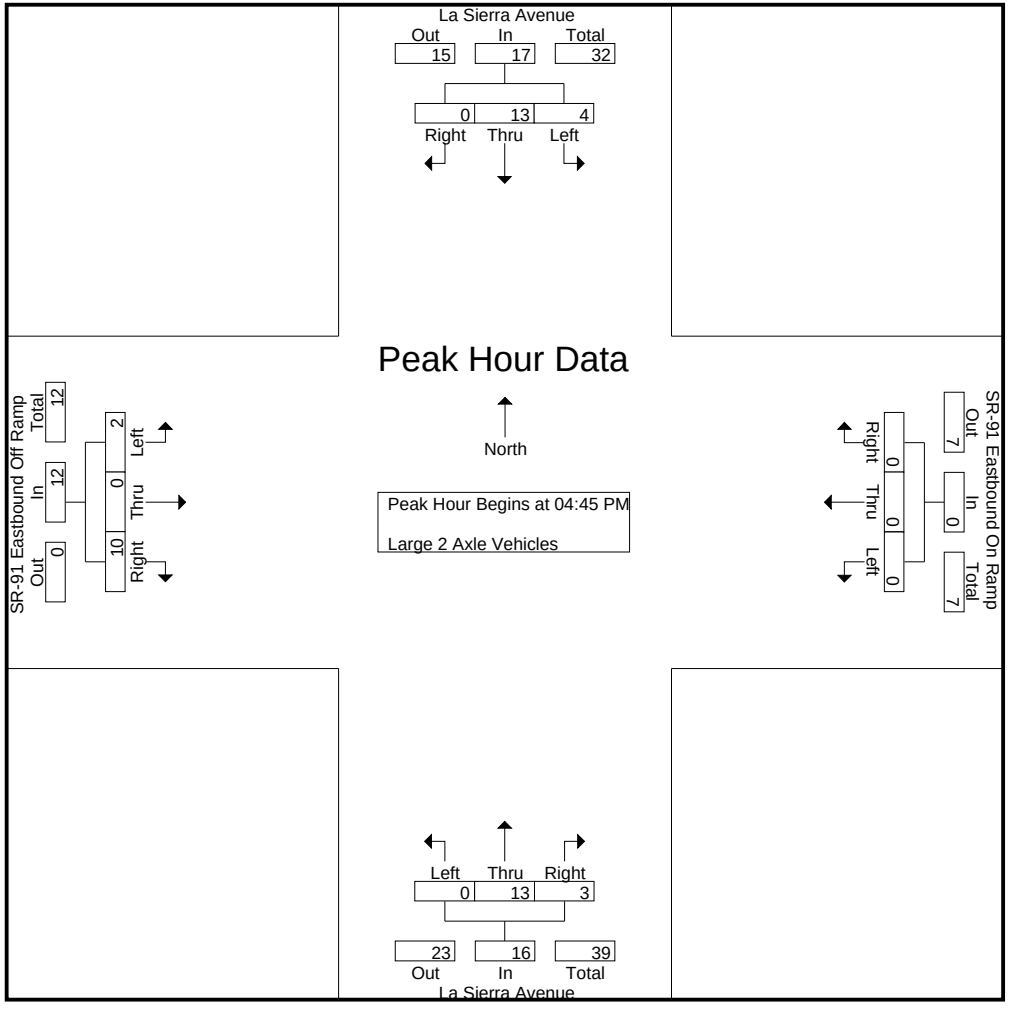
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	3	0	0	3	0	0	0	0	0	0	3	1	0	4	0	0	2	0	2	0	9	9
04:15 PM	1	2	0	0	3	0	0	0	0	0	0	1	1	1	2	0	0	2	1	2	2	7	9
04:30 PM	0	1	0	0	1	0	0	0	0	0	0	5	2	1	7	0	0	1	1	1	2	9	11
04:45 PM	1	1	0	0	2	0	0	0	0	0	0	7	1	0	8	1	0	3	0	4	0	14	14
Total	2	7	0	0	9	0	0	0	0	0	0	16	5	2	21	1	0	8	2	9	4	39	43
05:00 PM	2	3	0	0	5	0	0	0	0	0	0	1	1	0	2	0	0	1	0	1	0	8	8
05:15 PM	0	8	0	0	8	0	0	0	0	0	0	1	1	0	2	1	0	2	1	3	1	13	14
05:30 PM	1	1	0	0	2	0	0	0	0	0	0	4	0	0	4	0	0	4	2	4	2	10	12
05:45 PM	1	3	0	0	4	0	0	0	0	0	0	3	1	1	4	0	0	0	0	0	1	8	9
Total	4	15	0	0	19	0	0	0	0	0	0	9	3	1	12	1	0	7	3	8	4	39	43
Grand Total	6	22	0	0	28	0	0	0	0	0	0	25	8	3	33	2	0	15	5	17	8	78	86
Apprch %	21.4	78.6	0			0	0	0			0	75.8	24.2			11.8	0	88.2					
Total %	7.7	28.2	0		35.9	0	0	0		0	0	32.1	10.3		42.3	2.6	0	19.2		21.8	9.3	90.7	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	1	1	0	2	0	0	0	0	0	**7**	**1**	**8**	**1**	0	3	**4**	**14**
05:00 PM	**2**	3	0	5	0	0	0	0	0	1	1	2	0	0	1	1	8
05:15 PM	0	**8**	0	**8**	0	0	0	0	0	1	1	2	1	0	2	3	13
05:30 PM	1	1	0	2	0	0	0	0	0	4	0	4	0	0	**4**	4	10
Total Volume	4	13	0	17	0	0	0	0	0	13	3	16	2	0	10	12	45
% App. Total	23.5	76.5	0		0	0	0		0	81.2	18.8		16.7	0	83.3		
PHF	.500	.406	.000	.531	.000	.000	.000	.000	.000	.464	.750	.500	.500	.000	.625	.750	.804

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
(951) 268-6268

City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E PM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E PM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:45 PM				04:45 PM				04:45 PM				
+0 mins.	1	1	0	2	0	0	0	0	0	**7**	**1**	**8**	**1**	0	3	**4**	
+15 mins.	**2**	3	0	5	0	0	0	0	0	1	1	2	0	0	1	1	
+30 mins.	0	**8**	0	**8**	0	0	0	0	0	1	1	2	1	0	2	3	
+45 mins.	1	1	0	2	0	0	0	0	0	4	0	4	0	0	**4**	4	
Total Volume	4	13	0	17	0	0	0	0	0	13	3	16	2	0	10	12	
% App. Total	23.5	76.5	0		0	0	0		0	81.2	18.8		16.7	0	83.3		
PHF	.500	.406	.000	.531	.000	.000	.000	.000	.000	.464	.750	.500	.500	.000	.625	.750	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E PM
Site Code : 05124383
Start Date : 4/25/2024

Groups Printed- 3 Axle Vehicles

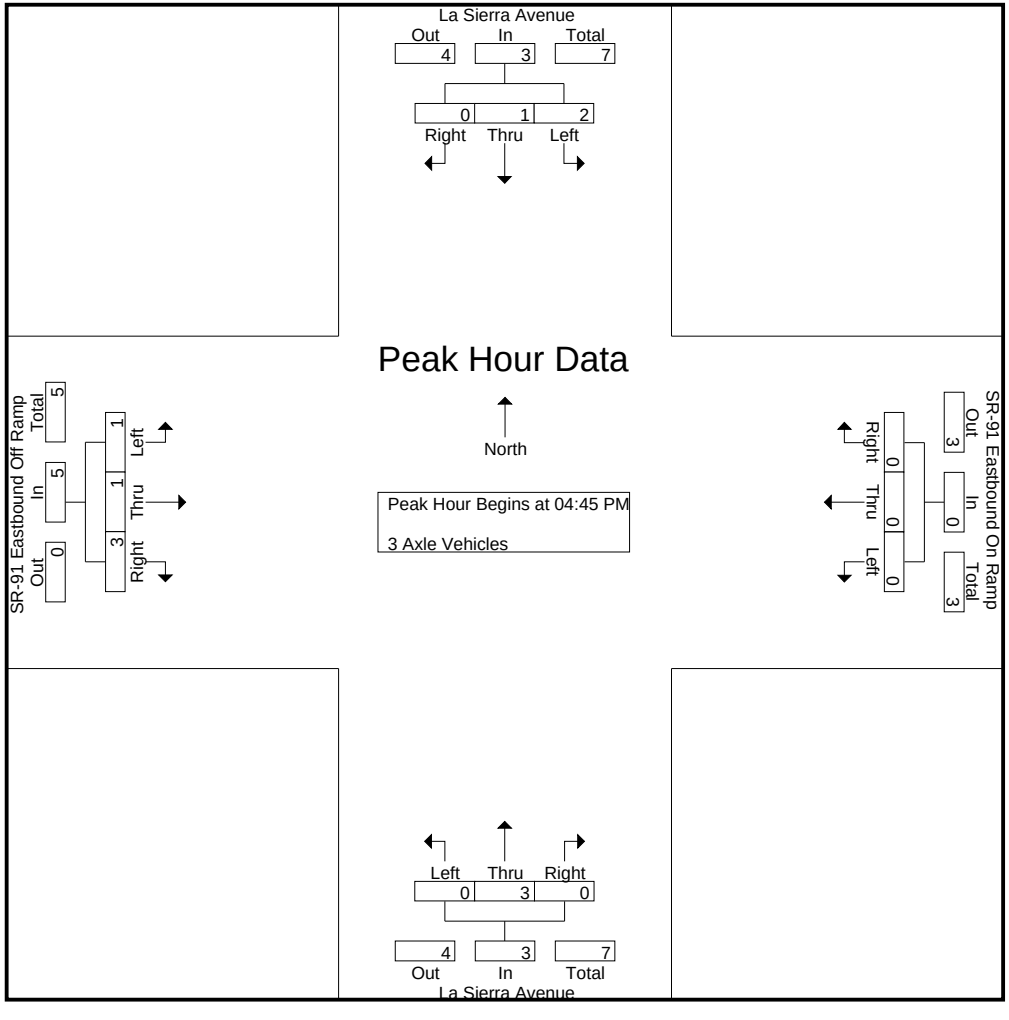
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	1	1
04:15 PM	1	0	0	0	1	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	2	2
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	0	2	2
04:45 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	1	2	3
Total	2	0	0	0	2	0	0	0	0	0	0	0	1	0	1	0	0	4	1	4	1	7	8
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3	0	0	0	0	0	0	3	3
05:15 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1	0	2	0	3	0	4	4
05:30 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	2	2
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	2	1	2	0	0	0	0	0	1	2	3
Total	1	1	0	0	2	0	0	0	0	0	0	3	2	1	5	1	1	2	0	4	1	11	12
Grand Total	3	1	0	0	4	0	0	0	0	0	0	3	3	1	6	1	1	6	1	8	2	18	20
Apprch %	75	25	0			0	0	0			0	50	50			12.5	12.5	75					
Total %	16.7	5.6	0		22.2	0	0	0		0	0	16.7	16.7		33.3	5.6	5.6	33.3		44.4	10	90	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	**1**	0	0	**1**	0	0	0	0	0	0	0	0	0	0	1	1	2
05:00 PM	0	0	0	0	0	0	0	0	0	**3**	0	**3**	0	0	0	0	3
05:15 PM	0	**1**	0	1	0	0	0	0	0	0	0	0	**1**	0	**2**	**3**	**4**
05:30 PM	1	0	0	1	0	0	0	0	0	0	0	0	0	**1**	0	1	2
Total Volume	2	1	0	3	0	0	0	0	0	3	0	3	1	1	3	5	11
% App. Total	66.7	33.3	0		0	0	0		0	100	0		20	20	60		
PHF	.500	.250	.000	.750	.000	.000	.000	.000	.000	.250	.000	.250	.250	.250	.375	.417	.688

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PO Box 1178
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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E PM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
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File Name : 02_RIV_LS_91E PM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:45 PM				04:45 PM				04:45 PM				
+0 mins.	**1**	0	0	**1**	0	0	0	0	0	0	0	0	0	0	1	1	
+15 mins.	0	0	0	0	0	0	0	0	0	**3**	0	**3**	0	0	0	0	
+30 mins.	0	**1**	0	1	0	0	0	0	0	0	0	0	**1**	0	**2**	**3**	
+45 mins.	1	0	0	1	0	0	0	0	0	0	0	0	0	**1**	0	1	
Total Volume	2	1	0	3	0	0	0	0	0	3	0	3	1	1	3	5	
% App. Total	66.7	33.3	0		0	0	0		0	100	0		20	20	60		
PHF	.500	.250	.000	.750	.000	.000	.000	.000	.000	.250	.000	.250	.250	.250	.375	.417	

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E PM
Site Code : 05124383
Start Date : 4/25/2024

Groups Printed- 4+ Axle Trucks

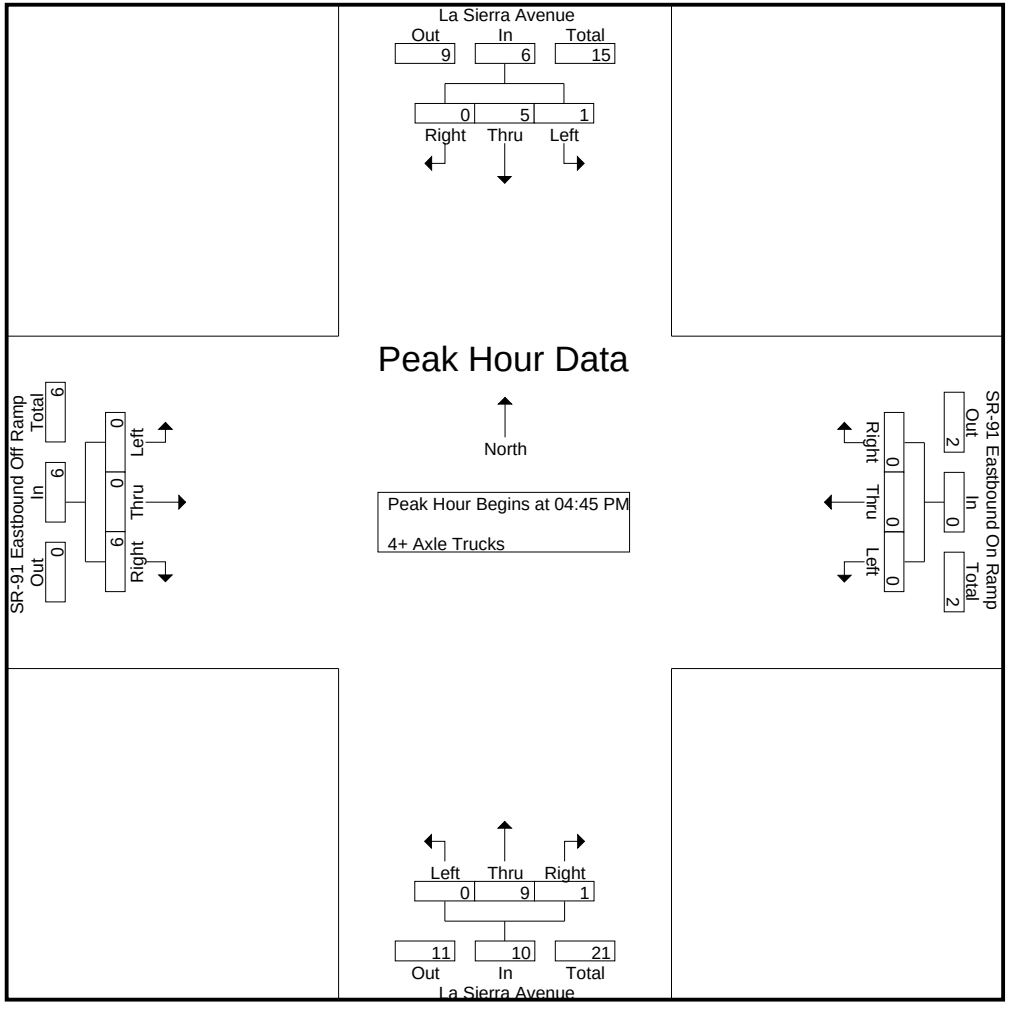
	La Sierra Avenue Southbound					SR-91 Eastbound On Ramp Westbound					La Sierra Avenue Northbound					SR-91 Eastbound Off Ramp Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	1	0	0	0	1	0	0	0	0	0	0	1	1	1	2	0	0	3	1	3	2	6	8
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	2	1	1	3	0	0	2	2	2	3	5	8
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3	0	0	0	0	0	0	3	3
04:45 PM	0	2	0	0	2	0	0	0	0	0	0	1	0	0	1	0	0	2	1	2	1	5	6
Total	1	2	0	0	3	0	0	0	0	0	0	7	2	2	9	0	0	7	4	7	6	19	25
05:00 PM	0	1	0	0	1	0	0	0	0	0	0	4	0	0	4	0	0	0	0	0	0	5	5
05:15 PM	1	1	0	0	2	0	0	0	0	0	0	3	0	0	3	0	0	3	2	3	2	8	10
05:30 PM	0	1	0	0	1	0	0	0	0	0	0	1	1	1	2	0	0	1	0	1	1	4	5
05:45 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	0	3	3
Total	1	4	0	0	5	0	0	0	0	0	0	8	1	1	9	0	0	6	2	6	3	20	23
Grand Total	2	6	0	0	8	0	0	0	0	0	0	15	3	3	18	0	0	13	6	13	9	39	48
Apprch %	25	75	0			0	0	0			0	83.3	16.7			0	0	100					
Total %	5.1	15.4	0		20.5	0	0	0		0	0	38.5	7.7		46.2	0	0	33.3		33.3	18.8	81.2	

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	0	**2**	0	**2**	0	0	0	0	0	1	0	1	0	0	2	2	5
05:00 PM	0	1	0	1	0	0	0	0	0	**4**	0	**4**	0	0	0	0	5
05:15 PM	**1**	1	0	2	0	0	0	0	0	3	0	3	0	0	**3**	**3**	**8**
05:30 PM	0	1	0	1	0	0	0	0	0	1	**1**	2	0	0	1	1	4
Total Volume	1	5	0	6	0	0	0	0	0	9	1	10	0	0	6	6	22
% App. Total	16.7	83.3	0		0	0	0		0	90	10		0	0	100		
PHF	.250	.625	.000	.750	.000	.000	.000	.000	.000	.563	.250	.625	.000	.000	.500	.500	.688

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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E PM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
E/W: SR-91 Eastbound Ramps
Weather: Clear

File Name : 02_RIV_LS_91E PM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				SR-91 Eastbound On Ramp Westbound				La Sierra Avenue Northbound				SR-91 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:45 PM to 05:30 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:45 PM				04:45 PM				04:45 PM				
+0 mins.	0	**2**	0	**2**	0	0	0	0	0	1	0	1	0	0	2	2	
+15 mins.	0	1	0	1	0	0	0	0	0	**4**	0	**4**	0	0	0	0	
+30 mins.	**1**	1	0	2	0	0	0	0	0	3	0	3	0	0	**3**	**3**	
+45 mins.	0	1	0	1	0	0	0	0	0	1	**1**	2	0	0	1	1	
Total Volume	1	5	0	6	0	0	0	0	0	9	1	10	0	0	6	6	
% App. Total	16.7	83.3	0		0	0	0		0	90	10		0	0	100		
PHF	.250	.625	.000	.750	.000	.000	.000	.000	.000	.563	.250	.625	.000	.000	.500	.500	

Location: Riverside
N/S: La Sierra Avenue
E/W: SR-91 EB Ramps

Date: 4/25/2024
Day: Thursday

PEDESTRIANS

	North Leg La Sierra Avenue	East Leg SR-91 EB Ramps	South Leg La Sierra Avenue	West Leg SR-91 EB Ramps	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	0	0	0	0	0
7:15 AM	0	0	0	2	2
7:30 AM	0	0	0	4	4
7:45 AM	0	1	0	0	1
8:00 AM	0	2	0	2	4
8:15 AM	0	0	0	1	1
8:30 AM	0	2	0	0	2
8:45 AM	0	2	0	0	2
TOTAL VOLUMES:	0	7	0	9	16

	North Leg La Sierra Avenue	East Leg SR-91 EB Ramps	South Leg La Sierra Avenue	West Leg SR-91 EB Ramps	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	0	0	0	3	3
4:15 PM	0	2	0	3	5
4:30 PM	0	2	0	2	4
4:45 PM	0	0	0	1	1
5:00 PM	0	2	0	2	4
5:15 PM	0	1	0	1	2
5:30 PM	0	0	0	0	0
5:45 PM	0	1	0	1	2
TOTAL VOLUMES:	0	8	0	13	21

Location: Riverside
N/S: La Sierra Avenue
E/W: SR-91 EB Ramps

Date: 4/25/2024
Day: Thursday

BICYCLES

	Southbound La Sierra Avenue			Westbound SR-91 EB Ramps			Northbound La Sierra Avenue			Eastbound SR-91 EB Ramps			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	1
7:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	1
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	2	0	0	0	0	0	0	0	0	0	0	2

	Southbound La Sierra Avenue			Westbound SR-91 EB Ramps			Northbound La Sierra Avenue			Eastbound SR-91 EB Ramps			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	1
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	3	0	0	0	0	3
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	1	0	0	0	0	1
5:45 PM	0	1	0	0	0	0	0	1	0	0	0	0	2
TOTAL VOLUMES:	0	2	0	0	0	0	0	5	0	0	0	0	7

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City of Riverside
N/S: La Sierra Avenue
E/W: Indiana Avenue
Weather: Clear

File Name : 03_RIV_LS_Ind AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 1

Groups Printed- Total Volume

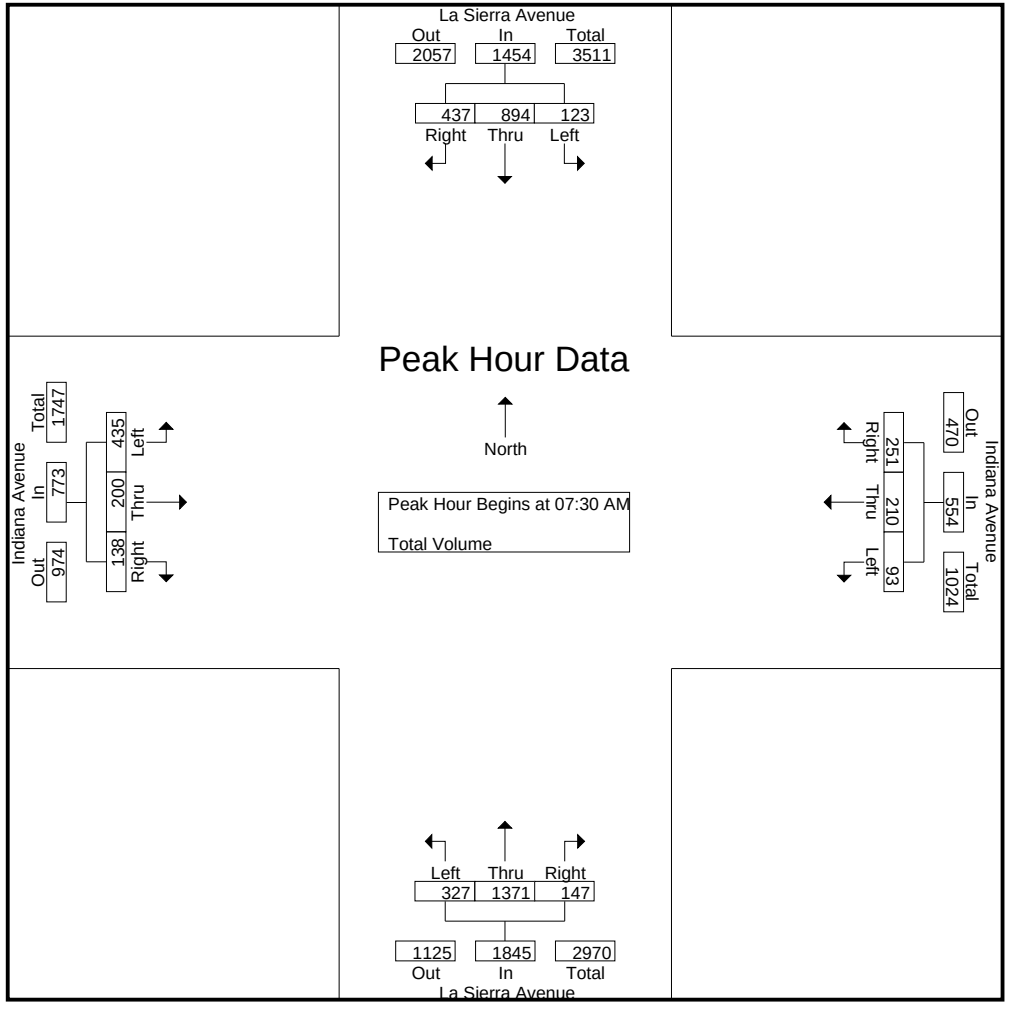
	La Sierra Avenue Southbound					Indiana Avenue Westbound					La Sierra Avenue Northbound					Indiana Avenue Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	18	108	34	13	160	13	27	55	18	95	47	261	10	4	318	76	22	28	11	126	46	699	745
07:15 AM	32	109	65	30	206	16	21	60	25	97	66	316	19	7	401	79	25	17	7	121	69	825	894
07:30 AM	36	192	78	39	306	26	37	82	24	145	56	293	37	16	386	94	47	27	9	168	88	1005	1093
07:45 AM	29	265	93	45	387	29	40	55	14	124	81	335	36	14	452	112	45	31	11	188	84	1151	1235
Total	115	674	270	127	1059	84	125	252	81	461	250	1205	102	41	1557	361	139	103	38	603	287	3680	3967
08:00 AM	25	272	141	62	438	24	67	61	20	152	90	349	33	12	472	123	50	40	13	213	107	1275	1382
08:15 AM	33	165	125	59	323	14	66	53	15	133	100	394	41	16	535	106	58	40	15	204	105	1195	1300
08:30 AM	30	149	62	31	241	18	56	60	21	134	58	221	16	4	295	130	73	56	18	259	74	929	1003
08:45 AM	39	147	52	25	238	16	16	49	20	81	32	273	20	6	325	94	28	31	12	153	63	797	860
Total	127	733	380	177	1240	72	205	223	76	500	280	1237	110	38	1627	453	209	167	58	829	349	4196	4545
Grand Total	242	1407	650	304	2299	156	330	475	157	961	530	2442	212	79	3184	814	348	270	96	1432	636	7876	8512
Apprch %	10.5	61.2	28.3			16.2	34.3	49.4			16.6	76.7	6.7			56.8	24.3	18.9			7.5	92.5	
Total %	3.1	17.9	8.3		29.2	2	4.2	6		12.2	6.7	31	2.7		40.4	10.3	4.4	3.4		18.2			

	La Sierra Avenue Southbound				Indiana Avenue Westbound				La Sierra Avenue Northbound				Indiana Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	**36**	192	78	306	26	37	**82**	145	56	293	37	386	94	47	27	168	1005
07:45 AM	29	265	93	387	**29**	40	55	124	81	335	36	452	112	45	31	188	1151
08:00 AM	25	**272**	**141**	**438**	24	**67**	61	**152**	90	349	33	472	**123**	50	**40**	**213**	**1275**
08:15 AM	33	165	125	323	14	66	53	133	**100**	**394**	**41**	**535**	106	**58**	40	204	1195
Total Volume	123	894	437	1454	93	210	251	554	327	1371	147	1845	435	200	138	773	4626
% App. Total	8.5	61.5	30.1		16.8	37.9	45.3		17.7	74.3	8		56.3	25.9	17.9		
PHF	.854	.822	.775	.830	.802	.784	.765	.911	.818	.870	.896	.862	.884	.862	.863	.907	.907

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City of Riverside
N/S: La Sierra Avenue
E/W: Indiana Avenue
Weather: Clear

File Name : 03_RIV_LS_Ind AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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N/S: La Sierra Avenue
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File Name : 03_RIV_LS_Ind AM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				Indiana Avenue Westbound				La Sierra Avenue Northbound				Indiana Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:30 AM				07:30 AM				07:30 AM				07:45 AM				
+0 mins.	**36**	192	78	306	26	37	**82**	145	56	293	37	386	112	45	31	188	
+15 mins.	29	265	93	387	**29**	40	55	124	81	335	36	452	123	50	40	213	
+30 mins.	25	**272**	**141**	**438**	24	**67**	61	**152**	90	349	33	472	106	58	40	204	
+45 mins.	33	165	125	323	14	66	53	133	**100**	**394**	**41**	**535**	**130**	**73**	**56**	**259**	
Total Volume	123	894	437	1454	93	210	251	554	327	1371	147	1845	471	226	167	864	
% App. Total	8.5	61.5	30.1		16.8	37.9	45.3		17.7	74.3	8		54.5	26.2	19.3		
PHF	.854	.822	.775	.830	.802	.784	.765	.911	.818	.870	.896	.862	.906	.774	.746	.834	

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City of Riverside
N/S: La Sierra Avenue
E/W: Indiana Avenue
Weather: Clear

File Name : 03_RIV_LS_IndPM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 1

Groups Printed- Total Volume

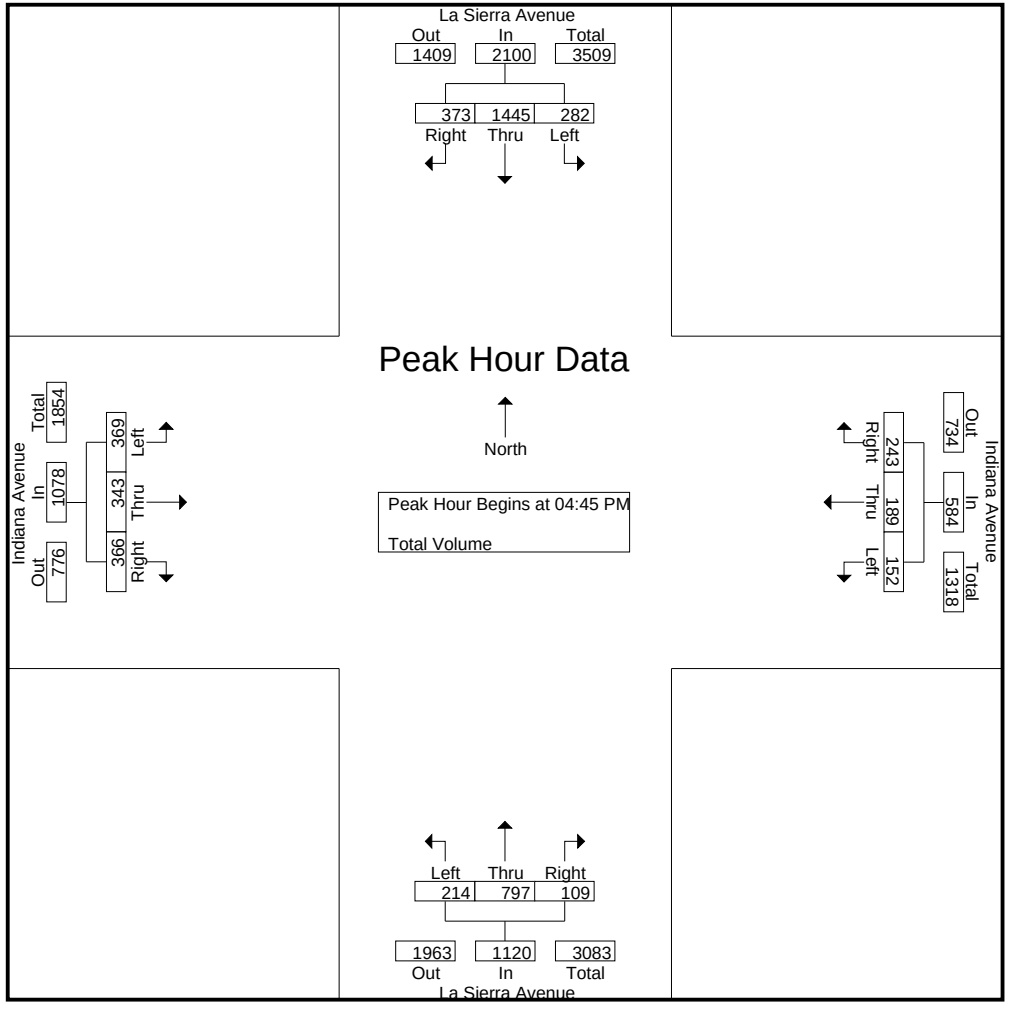
	La Sierra Avenue Southbound					Indiana Avenue Westbound					La Sierra Avenue Northbound					Indiana Avenue Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	72	348	95	24	515	21	51	68	15	140	59	199	30	13	288	112	104	95	21	311	73	1254	1327
04:15 PM	61	310	74	14	445	34	46	49	13	129	47	215	23	9	285	89	84	102	38	275	74	1134	1208
04:30 PM	66	350	74	16	490	47	61	63	15	171	37	224	23	4	284	74	80	105	36	259	71	1204	1275
04:45 PM	71	344	92	24	507	31	45	50	15	126	53	204	23	8	280	93	77	83	18	253	65	1166	1231
Total	270	1352	335	78	1957	133	203	230	58	566	196	842	99	34	1137	368	345	385	113	1098	283	4758	5041
05:00 PM	69	367	106	27	542	53	67	75	16	195	67	194	24	12	285	103	99	109	25	311	80	1333	1413
05:15 PM	57	389	93	20	539	25	43	47	12	115	43	188	30	14	261	76	88	95	17	259	63	1174	1237
05:30 PM	85	345	82	19	512	43	34	71	11	148	51	211	32	16	294	97	79	79	19	255	65	1209	1274
05:45 PM	97	372	98	30	567	42	18	51	11	111	38	192	36	15	266	79	63	50	11	192	67	1136	1203
Total	308	1473	379	96	2160	163	162	244	50	569	199	785	122	57	1106	355	329	333	72	1017	275	4852	5127
Grand Total	578	2825	714	174	4117	296	365	474	108	1135	395	1627	221	91	2243	723	674	718	185	2115	558	9610	10168
Apprch %	14	68.6	17.3			26.1	32.2	41.8			17.6	72.5	9.9			34.2	31.9	33.9					
Total %	6	29.4	7.4		42.8	3.1	3.8	4.9		11.8	4.1	16.9	2.3		23.3	7.5	7	7.5		22	5.5	94.5	

	La Sierra Avenue Southbound				Indiana Avenue Westbound				La Sierra Avenue Northbound				Indiana Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	71	344	92	507	31	45	50	126	53	204	23	280	93	77	83	253	1166
05:00 PM	69	367	**106**	**542**	**53**	**67**	**75**	**195**	**67**	194	24	285	**103**	**99**	**109**	**311**	**1333**
05:15 PM	57	**389**	93	539	25	43	47	115	43	188	30	261	76	88	95	259	1174
05:30 PM	**85**	345	82	512	43	34	71	148	51	**211**	**32**	**294**	97	79	79	255	1209
Total Volume	282	1445	373	2100	152	189	243	584	214	797	109	1120	369	343	366	1078	4882
% App. Total	13.4	68.8	17.8		26	32.4	41.6		19.1	71.2	9.7		34.2	31.8	34		
PHF	.829	.929	.880	.969	.717	.705	.810	.749	.799	.944	.852	.952	.896	.866	.839	.867	.916

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City of Riverside
N/S: La Sierra Avenue
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Weather: Clear

File Name : 03_RIV_LS_IndPM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



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City of Riverside
N/S: La Sierra Avenue
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File Name : 03_RIV_LS_IndPM
Site Code : 05124383
Start Date : 4/25/2024

	La Sierra Avenue Southbound				Indiana Avenue Westbound				La Sierra Avenue Northbound				Indiana Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	05:00 PM				04:15 PM				04:00 PM				04:00 PM				
+0 mins.	69	367	**106**	542	34	46	49	129	**59**	199	**30**	**288**	**112**	**104**	95	**311**	
+15 mins.	57	**389**	93	539	47	61	63	171	47	215	23	285	89	84	102	275	
+30 mins.	85	345	82	512	31	45	50	126	37	**224**	23	284	74	80	**105**	259	
+45 mins.	**97**	372	98	**567**	**53**	**67**	**75**	**195**	53	204	23	280	93	77	83	253	
Total Volume	308	1473	379	2160	165	219	237	621	196	842	99	1137	368	345	385	1098	
% App. Total	14.3	68.2	17.5		26.6	35.3	38.2		17.2	74.1	8.7		33.5	31.4	35.1		
PHF	.794	.947	.894	.952	.778	.817	.790	.796	.831	.940	.825	.987	.821	.829	.917	.883	

Location: Riverside
N/S: La Sierra Avenue
E/W: Indiana Avenue

Date: 4/25/2024
Day: Thursday

PEDESTRIANS

	North Leg La Sierra Avenue	East Leg Indiana Avenue	South Leg La Sierra Avenue	West Leg Indiana Avenue	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	1	0	0	2	3
7:15 AM	0	0	2	4	6
7:30 AM	0	1	0	0	1
7:45 AM	1	1	0	5	7
8:00 AM	1	0	1	3	5
8:15 AM	1	0	0	0	1
8:30 AM	1	1	1	1	4
8:45 AM	0	0	0	0	0
TOTAL VOLUMES:	5	3	4	15	27

	North Leg La Sierra Avenue	East Leg Indiana Avenue	South Leg La Sierra Avenue	West Leg Indiana Avenue	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	1	3	7	1	12
4:15 PM	0	0	0	4	4
4:30 PM	2	0	1	5	8
4:45 PM	0	0	0	5	5
5:00 PM	1	2	4	3	10
5:15 PM	2	0	2	1	5
5:30 PM	0	4	3	4	11
5:45 PM	0	0	0	5	5
TOTAL VOLUMES:	6	9	17	28	60

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Location: Riverside
N/S: La Sierra Avenue
E/W: Indiana Avenue

Date: 4/25/2024
Day: Thursday

BICYCLES

	Southbound La Sierra Avenue			Westbound Indiana Avenue			Northbound La Sierra Avenue			Eastbound Indiana Avenue			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	1	1	0	0	0	0	0	0	0	0	0	0	2
7:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	1
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	1	2	0	0	0	0	0	0	0	0	0	0	3

	Southbound La Sierra Avenue			Westbound Indiana Avenue			Northbound La Sierra Avenue			Eastbound Indiana Avenue			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	1	0	0	0	0	0	0	0	0	0	1	0	2
5:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	1
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	1	0	0	0	0	0	0	0	0	0	0	0	1
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	2	0	0	0	1	0	0	0	0	0	1	0	4

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City of Riverside
N/S: La Sierra Avenue
E/W: Victoria Avenue
Weather: Clear

File Name : 04_RIV_LS_Vic AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 1

Groups Printed- Total Volume

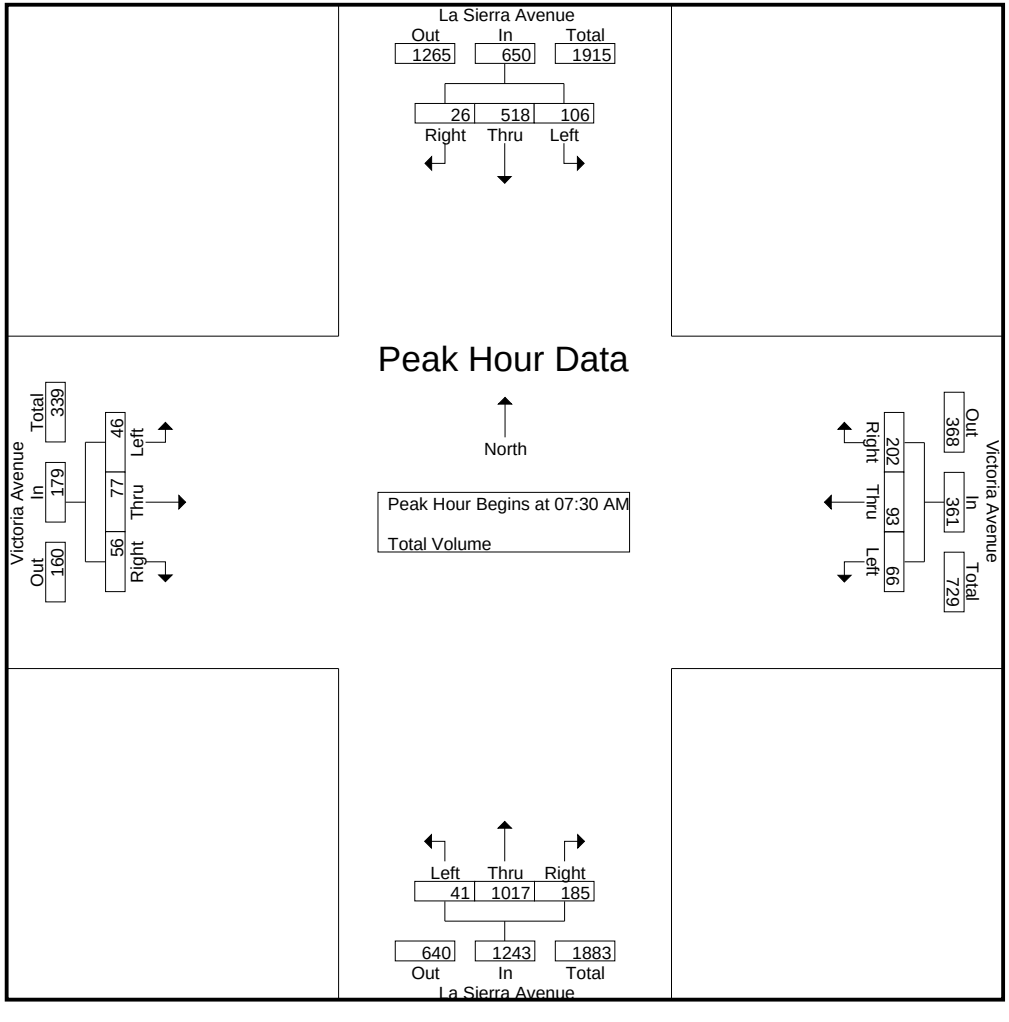
	La Sierra Avenue Southbound					Victoria Avenue Westbound					La Sierra Avenue Northbound					Victoria Avenue Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	12	89	3	0	104	12	15	28	12	55	9	241	21	8	271	11	9	7	7	27	27	457	484
07:15 AM	12	95	7	0	114	7	14	34	14	55	7	275	37	4	319	25	14	8	6	47	24	535	559
07:30 AM	19	108	4	0	131	16	25	37	19	78	7	262	43	13	312	8	20	16	11	44	43	565	608
07:45 AM	27	124	5	1	156	11	25	45	16	81	13	255	46	4	314	9	20	14	5	43	26	594	620
Total	70	416	19	1	505	46	79	144	61	269	36	1033	147	29	1216	53	63	45	29	161	120	2151	2271
08:00 AM	33	150	10	0	193	24	22	83	25	129	7	243	53	6	303	17	24	17	11	58	42	683	725
08:15 AM	27	136	7	0	170	15	21	37	16	73	14	257	43	3	314	12	13	9	8	34	27	591	618
08:30 AM	20	131	7	1	158	10	20	34	17	64	5	204	21	1	230	12	14	4	3	30	22	482	504
08:45 AM	16	108	5	1	129	13	8	28	14	49	4	227	18	1	249	19	14	3	2	36	18	463	481
Total	96	525	29	2	650	62	71	182	72	315	30	931	135	11	1096	60	65	33	24	158	109	2219	2328
Grand Total	166	941	48	3	1155	108	150	326	133	584	66	1964	282	40	2312	113	128	78	53	319	229	4370	4599
Apprch %	14.4	81.5	4.2			18.5	25.7	55.8			2.9	84.9	12.2			35.4	40.1	24.5					
Total %	3.8	21.5	1.1		26.4	2.5	3.4	7.5		13.4	1.5	44.9	6.5		52.9	2.6	2.9	1.8		7.3	5	95	

	La Sierra Avenue Southbound				Victoria Avenue Westbound				La Sierra Avenue Northbound				Victoria Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	19	108	4	131	16	**25**	37	78	7	**262**	43	312	8	20	16	44	565
07:45 AM	27	124	5	156	11	25	45	81	13	255	46	**314**	9	20	14	43	594
08:00 AM	**33**	**150**	**10**	**193**	**24**	22	**83**	**129**	7	243	**53**	303	**17**	**24**	**17**	**58**	**683**
08:15 AM	27	136	7	170	15	21	37	73	**14**	257	43	314	12	13	9	34	591
Total Volume	106	518	26	650	66	93	202	361	41	1017	185	1243	46	77	56	179	2433
% App. Total	16.3	79.7	4		18.3	25.8	56		3.3	81.8	14.9		25.7	43	31.3		
PHF	.803	.863	.650	.842	.688	.930	.608	.700	.732	.970	.873	.990	.676	.802	.824	.772	.891

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City of Riverside
N/S: La Sierra Avenue
E/W: Victoria Avenue
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File Name : 04_RIV_LS_Vic AM
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Page No : 2



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	La Sierra Avenue Southbound				Victoria Avenue Westbound				La Sierra Avenue Northbound				Victoria Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	07:45 AM				07:30 AM				07:15 AM				07:15 AM				
+0 mins.	27	124	5	156	16	**25**	37	78	7	**275**	37	**319**	**25**	14	8	47	
+15 mins.	**33**	**150**	**10**	**193**	11	25	45	81	7	262	43	312	8	20	16	44	
+30 mins.	27	136	7	170	**24**	22	**83**	**129**	**13**	255	46	314	9	20	14	43	
+45 mins.	20	131	7	158	15	21	37	73	7	243	**53**	303	17	**24**	**17**	**58**	
Total Volume	107	541	29	677	66	93	202	361	34	1035	179	1248	59	78	55	192	
% App. Total	15.8	79.9	4.3		18.3	25.8	56		2.7	82.9	14.3		30.7	40.6	28.6		
PHF	.811	.902	.725	.877	.688	.930	.608	.700	.654	.941	.844	.978	.590	.813	.809	.828	

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Groups Printed- Total Volume

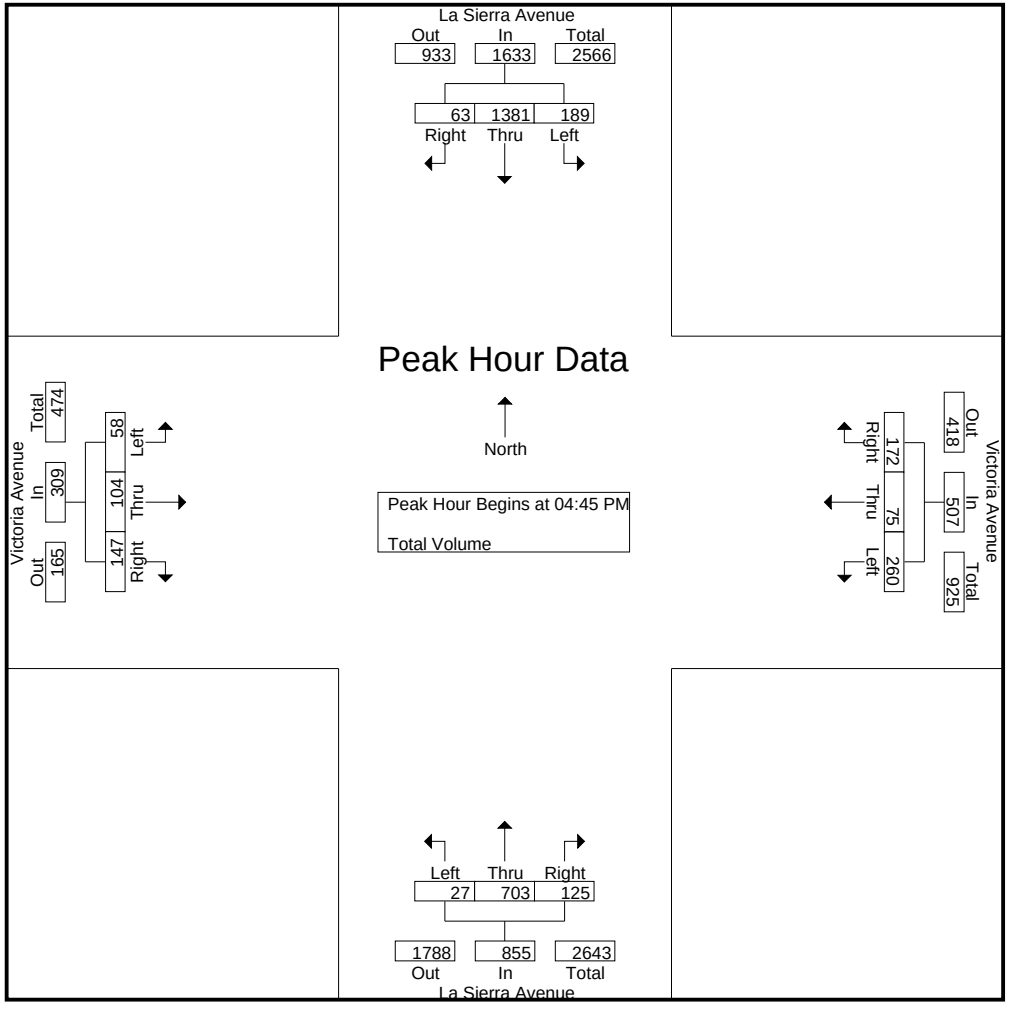
	La Sierra Avenue Southbound					Victoria Avenue Westbound					La Sierra Avenue Northbound					Victoria Avenue Eastbound							
Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	50	340	3	1	393	50	20	27	8	97	7	190	54	10	251	16	22	39	8	77	27	818	845
04:15 PM	29	325	14	5	368	56	18	29	15	103	5	193	41	7	239	5	30	23	6	58	33	768	801
04:30 PM	51	331	13	3	395	51	20	39	21	110	6	183	36	8	225	11	28	34	18	73	50	803	853
04:45 PM	48	341	15	5	404	63	23	43	16	129	7	178	38	13	223	13	33	28	4	74	38	830	868
Total	178	1337	45	14	1560	220	81	138	60	439	25	744	169	38	938	45	113	124	36	282	148	3219	3367
05:00 PM	39	333	12	3	384	74	14	35	10	123	6	155	28	9	189	14	29	48	15	91	37	787	824
05:15 PM	43	389	15	8	447	63	17	41	17	121	5	187	38	7	230	16	19	45	10	80	42	878	920
05:30 PM	59	318	21	5	398	60	21	53	24	134	9	183	21	9	213	15	23	26	8	64	46	809	855
05:45 PM	45	309	20	4	374	54	11	29	3	94	1	147	28	4	176	9	24	23	8	56	19	700	719
Total	186	1349	68	20	1603	251	63	158	54	472	21	672	115	29	808	54	95	142	41	291	144	3174	3318
Grand Total	364	2686	113	34	3163	471	144	296	114	911	46	1416	284	67	1746	99	208	266	77	573	292	6393	6685
Apprch %	11.5	84.9	3.6			51.7	15.8	32.5			2.6	81.1	16.3			17.3	36.3	46.4					
Total %	5.7	42	1.8		49.5	7.4	2.3	4.6		14.2	0.7	22.1	4.4		27.3	1.5	3.3	4.2		9	4.4	95.6	

	La Sierra Avenue Southbound				Victoria Avenue Westbound				La Sierra Avenue Northbound				Victoria Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	48	341	15	404	63	**23**	43	129	7	178	**38**	223	13	**33**	28	74	830
05:00 PM	39	333	12	384	**74**	14	35	123	6	155	28	189	14	29	**48**	**91**	787
05:15 PM	43	**389**	15	**447**	63	17	41	121	5	**187**	38	**230**	**16**	19	45	80	**878**
05:30 PM	**59**	318	**21**	398	60	21	**53**	**134**	**9**	183	21	213	15	23	26	64	809
Total Volume	189	1381	63	1633	260	75	172	507	27	703	125	855	58	104	147	309	3304
% App. Total	11.6	84.6	3.9		51.3	14.8	33.9		3.2	82.2	14.6		18.8	33.7	47.6		
PHF	.801	.888	.750	.913	.878	.815	.811	.946	.750	.940	.822	.929	.906	.788	.766	.849	.941

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	La Sierra Avenue Southbound				Victoria Avenue Westbound				La Sierra Avenue Northbound				Victoria Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Each Approach Begins at:																	
	04:45 PM				04:45 PM				04:00 PM				04:30 PM				
+0 mins.	48	341	15	404	63	**23**	43	129	**7**	190	**54**	**251**	11	28	34	73	
+15 mins.	39	333	12	384	**74**	14	35	123	5	**193**	41	239	13	**33**	28	74	
+30 mins.	43	**389**	15	**447**	63	17	41	121	6	183	36	225	14	29	**48**	**91**	
+45 mins.	**59**	318	**21**	398	60	21	**53**	**134**	7	178	38	223	**16**	19	45	80	
Total Volume	189	1381	63	1633	260	75	172	507	25	744	169	938	54	109	155	318	
% App. Total	11.6	84.6	3.9		51.3	14.8	33.9		2.7	79.3	18		17	34.3	48.7		
PHF	.801	.888	.750	.913	.878	.815	.811	.946	.893	.964	.782	.934	.844	.826	.807	.874	

Location: Riverside
N/S: La Sierra Avenue
E/W: Victoria Avenue

Date: 4/25/2024
Day: Thursday

PEDESTRIANS

	North Leg La Sierra Avenue	East Leg Victoria Avenue	South Leg La Sierra Avenue	West Leg Victoria Avenue	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	2	0	0	0	2
7:15 AM	0	0	0	0	0
7:30 AM	0	0	0	0	0
7:45 AM	1	0	0	0	1
8:00 AM	0	0	0	0	0
8:15 AM	3	0	0	0	3
8:30 AM	0	0	0	0	0
8:45 AM	3	0	0	0	3
TOTAL VOLUMES:	9	0	0	0	9

	North Leg La Sierra Avenue	East Leg Victoria Avenue	South Leg La Sierra Avenue	West Leg Victoria Avenue	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	1	0	0	0	1
4:15 PM	0	0	0	0	0
4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0
5:15 PM	0	0	0	0	0
5:30 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0
TOTAL VOLUMES:	1	0	0	0	1

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Location: Riverside
N/S: La Sierra Avenue
E/W: Victoria Avenue

Date: 4/25/2024
Day: Thursday

BICYCLES

	Southbound La Sierra Avenue			Westbound Victoria Avenue			Northbound La Sierra Avenue			Eastbound Victoria Avenue			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	1	1	0	0	0	0	0	0	0	2
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	1	0	1
7:45 AM	0	0	0	0	1	0	0	0	0	0	0	0	1
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	1	0	0	0	0	0	0	0	1
8:45 AM	0	0	0	0	3	0	1	0	0	0	4	0	8
TOTAL VOLUMES:	0	0	0	1	6	0	1	0	0	0	5	0	13

	Southbound La Sierra Avenue			Westbound Victoria Avenue			Northbound La Sierra Avenue			Eastbound Victoria Avenue			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	2	0	2
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	1	0	0	0	0	0	0	1	2
4:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	1
5:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	1
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	1	0	0	0	0	0	0	1	2
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	3	0	0	0	0	0	3	2	8

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City of Riverside
N/S: La Sierra Avenue
E/W: McAllister Parkway
Weather: Clear

File Name : 05_RIV_LS_McA AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 1

Groups Printed- Total Volume

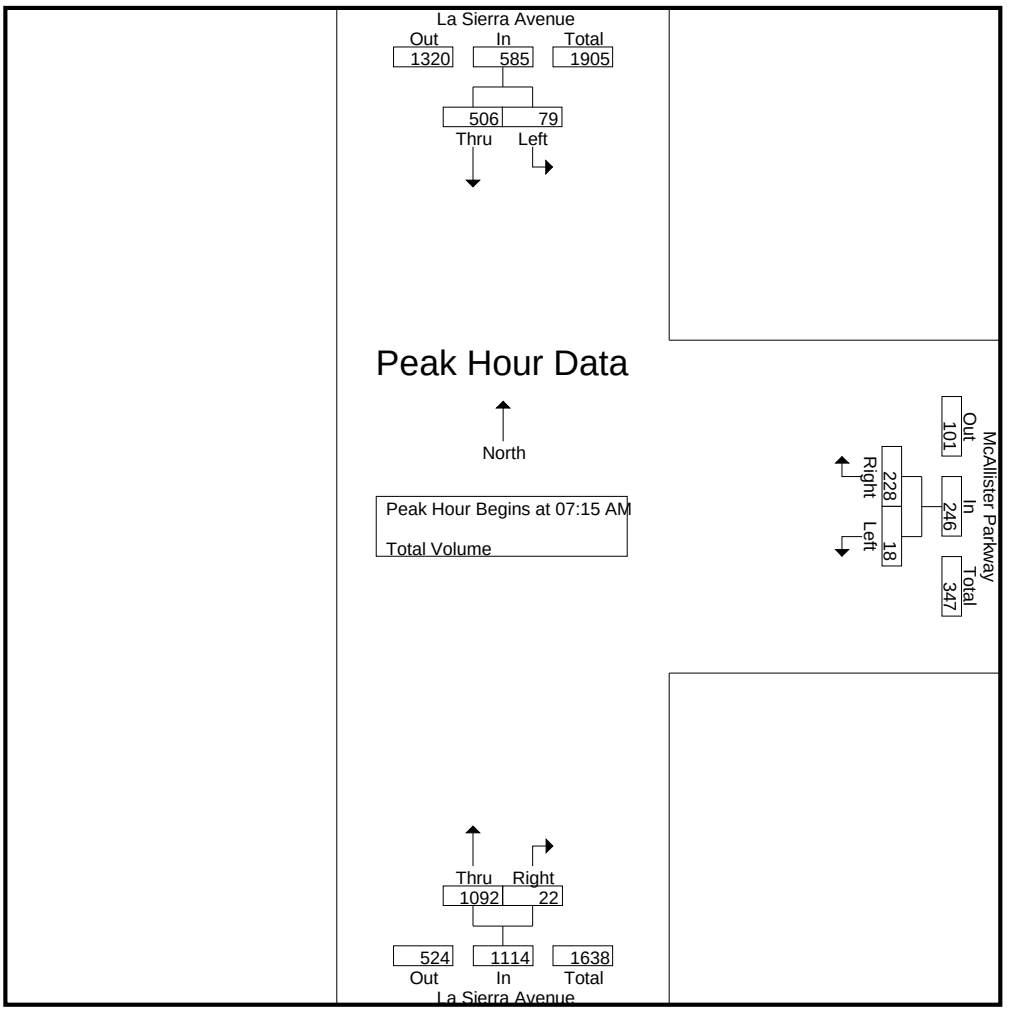
	La Sierra Avenue Southbound			McAllister Parkway Westbound				La Sierra Avenue Northbound						
Start Time	Left	Thru	App. Total	Left	Right	RTOR	App. Total	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	13	97	110	2	44	34	46	227	3	1	230	35	386	421
07:15 AM	11	95	106	3	60	26	63	282	3	0	285	26	454	480
07:30 AM	18	120	138	2	66	30	68	244	2	0	246	30	452	482
07:45 AM	24	134	158	8	54	24	62	263	5	0	268	24	488	512
Total	66	446	512	15	224	114	239	1016	13	1	1029	115	1780	1895
08:00 AM	26	157	183	5	48	21	53	303	12	3	315	24	551	575
08:15 AM	31	125	156	1	53	30	54	239	4	0	243	30	453	483
08:30 AM	31	107	138	3	35	22	38	198	2	1	200	23	376	399
08:45 AM	21	109	130	0	40	23	40	229	2	0	231	23	401	424
Total	109	498	607	9	176	96	185	969	20	4	989	100	1781	1881
Grand Total	175	944	1119	24	400	210	424	1985	33	5	2018	215	3561	3776
Apprch %	15.6	84.4		5.7	94.3			98.4	1.6					
Total %	4.9	26.5	31.4	0.7	11.2		11.9	55.7	0.9		56.7	5.7	94.3	

	La Sierra Avenue Southbound			McAllister Parkway Westbound			La Sierra Avenue Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:15 AM										
07:15 AM	11	95	106	3	60	63	282	3	285	454
07:30 AM	18	120	138	2	**66**	**68**	244	2	246	452
07:45 AM	24	134	158	**8**	54	62	263	5	268	488
08:00 AM	**26**	**157**	**183**	5	48	53	**303**	**12**	**315**	**551**
Total Volume	79	506	585	18	228	246	1092	22	1114	1945
% App. Total	13.5	86.5		7.3	92.7		98	2		
PHF	.760	.806	.799	.563	.864	.904	.901	.458	.884	.882

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City of Riverside
N/S: La Sierra Avenue
E/W: McAllister Parkway
Weather: Clear

File Name : 05_RIV_LS_McA AM
Site Code : 05124383
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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:30 AM			07:15 AM			07:15 AM		
+0 mins.	18	120	138	3	60	63	282	3	285
+15 mins.	24	134	158	2	**66**	**68**	244	2	246
+30 mins.	26	**157**	**183**	**8**	54	62	263	5	268
+45 mins.	**31**	125	156	5	48	53	**303**	**12**	**315**
Total Volume	99	536	635	18	228	246	1092	22	1114
% App. Total	15.6	84.4		7.3	92.7		98	2	
PHF	.798	.854	.867	.563	.864	.904	.901	.458	.884

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City of Riverside
N/S: La Sierra Avenue
E/W: McAllister Parkway
Weather: Clear

File Name : 05_RIV_LS_McA PM
Site Code : 05124383
Start Date : 4/25/2024
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Groups Printed- Total Volume

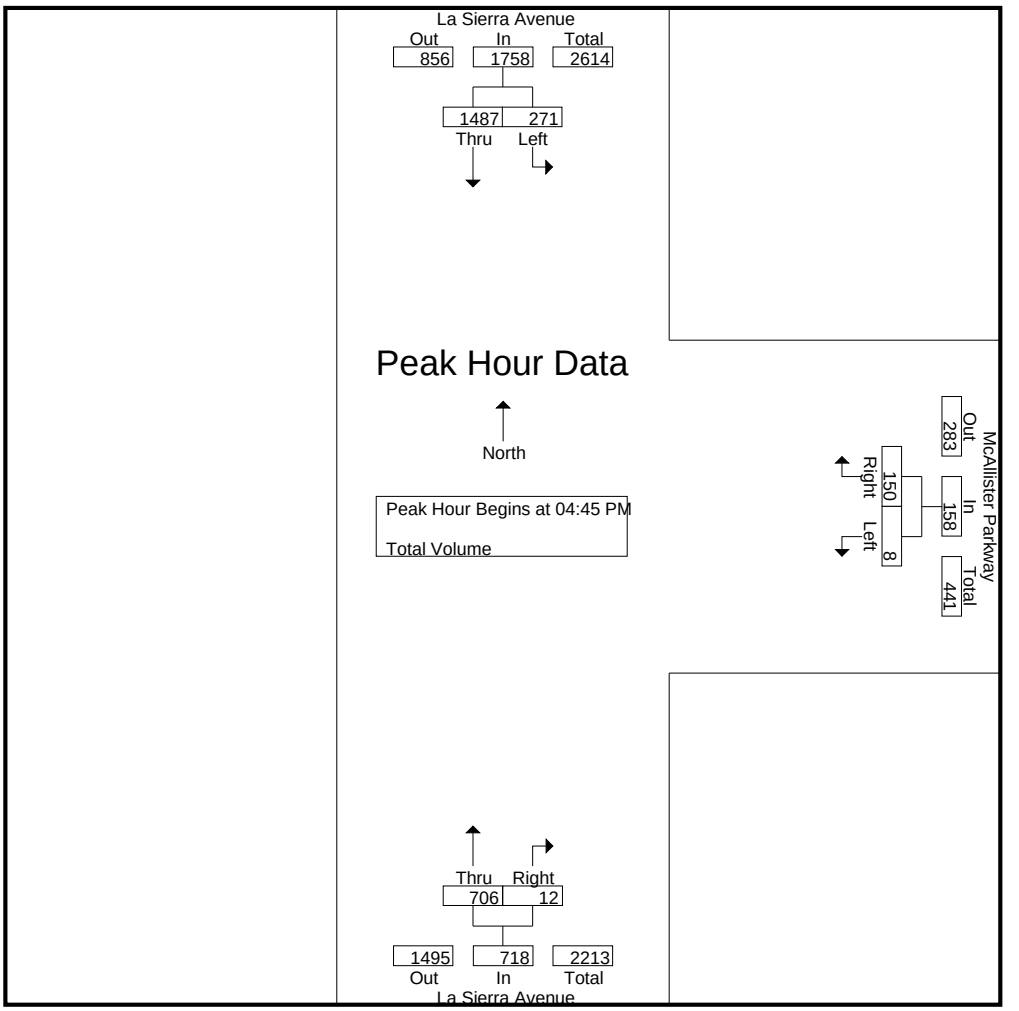
	La Sierra Avenue Southbound			McAllister Parkway Westbound				La Sierra Avenue Northbound						
Start Time	Left	Thru	App. Total	Left	Right	RTOR	App. Total	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	69	364	433	1	35	24	36	210	4	0	214	24	683	707
04:15 PM	76	318	394	4	35	17	39	200	2	0	202	17	635	652
04:30 PM	53	353	406	2	34	22	36	183	3	0	186	22	628	650
04:45 PM	74	340	414	2	32	20	34	190	0	0	190	20	638	658
Total	272	1375	1647	9	136	83	145	783	9	0	792	83	2584	2667
05:00 PM	61	401	462	2	35	24	37	162	5	0	167	24	666	690
05:15 PM	67	406	473	3	41	30	44	174	2	0	176	30	693	723
05:30 PM	69	340	409	1	42	26	43	180	5	0	185	26	637	663
05:45 PM	60	333	393	3	39	25	42	126	3	0	129	25	564	589
Total	257	1480	1737	9	157	105	166	642	15	0	657	105	2560	2665
Grand Total	529	2855	3384	18	293	188	311	1425	24	0	1449	188	5144	5332
Apprch %	15.6	84.4		5.8	94.2			98.3	1.7					
Total %	10.3	55.5	65.8	0.3	5.7		6	27.7	0.5		28.2	3.5	96.5	

	La Sierra Avenue Southbound			McAllister Parkway Westbound			La Sierra Avenue Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 04:45 PM										
04:45 PM	**74**	340	414	2	32	34	**190**	0	**190**	638
05:00 PM	61	401	462	2	35	37	162	**5**	167	666
05:15 PM	67	**406**	**473**	**3**	41	**44**	174	2	176	**693**
05:30 PM	69	340	409	1	**42**	43	180	5	185	637
Total Volume	271	1487	1758	8	150	158	706	12	718	2634
% App. Total	15.4	84.6		5.1	94.9		98.3	1.7		
PHF	.916	.916	.929	.667	.893	.898	.929	.600	.945	.950

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City of Riverside
N/S: La Sierra Avenue
E/W: McAllister Parkway
Weather: Clear

File Name : 05_RIV_LS_McA PM
Site Code : 05124383
Start Date : 4/25/2024
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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:45 PM			05:00 PM			04:00 PM		
+0 mins.	**74**	340	414	2	35	37	**210**	**4**	**214**
+15 mins.	61	401	462	**3**	41	**44**	200	2	202
+30 mins.	67	**406**	**473**	1	**42**	43	183	3	186
+45 mins.	69	340	409	3	39	42	190	0	190
Total Volume	271	1487	1758	9	157	166	783	9	792
% App. Total	15.4	84.6		5.4	94.6		98.9	1.1	
PHF	.916	.916	.929	.750	.935	.943	.932	.563	.925

Location: Riverside
N/S: La Sierra Avenue
E/W: McAllister Parkway

Date: 4/25/2024
Day: Thursday

PEDESTRIANS

	North Leg La Sierra Avenue	East Leg McAllister Parkway	South Leg La Sierra Avenue	West Leg McAllister Parkway	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	0	0	0	0	0
7:15 AM	0	0	0	0	0
7:30 AM	0	0	0	0	0
7:45 AM	0	0	0	0	0
8:00 AM	0	0	0	0	0
8:15 AM	0	0	0	0	0
8:30 AM	0	0	0	0	0
8:45 AM	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0

	North Leg La Sierra Avenue	East Leg McAllister Parkway	South Leg La Sierra Avenue	West Leg McAllister Parkway	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	0	0	0	0	0
4:15 PM	0	0	0	0	0
4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0
5:15 PM	0	0	0	0	0
5:30 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0

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Location: Riverside
N/S: La Sierra Avenue
E/W: McAllister Parkway

Date: 4/25/2024
Day: Thursday

BICYCLES

	Southbound La Sierra Avenue			Westbound McAllister Parkway			Northbound La Sierra Avenue			Eastbound McAllister Parkway			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	1	0	0	0	0	0	0	1
TOTAL VOLUMES:	0	0	0	0	0	1	0	0	0	0	0	0	1

	Southbound La Sierra Avenue			Westbound McAllister Parkway			Northbound La Sierra Avenue			Eastbound McAllister Parkway			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	1	0	0	0	0	0	0	0	0	1
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	1
5:00 PM	0	0	0	0	0	0	0	0	1	0	0	0	1
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	1	0	0	0	0	0	0	0	0	0	0	0	1
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	1	1	0	1	0	0	0	0	1	0	0	0	4

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City of Riverside
N/S: La Sierra Avenue
E/W: El Sobrante Road
Weather: Clear

File Name : 06_RIV_LS_ElS AM
Site Code : 05124383
Start Date : 4/25/2024
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Groups Printed- Total Volume

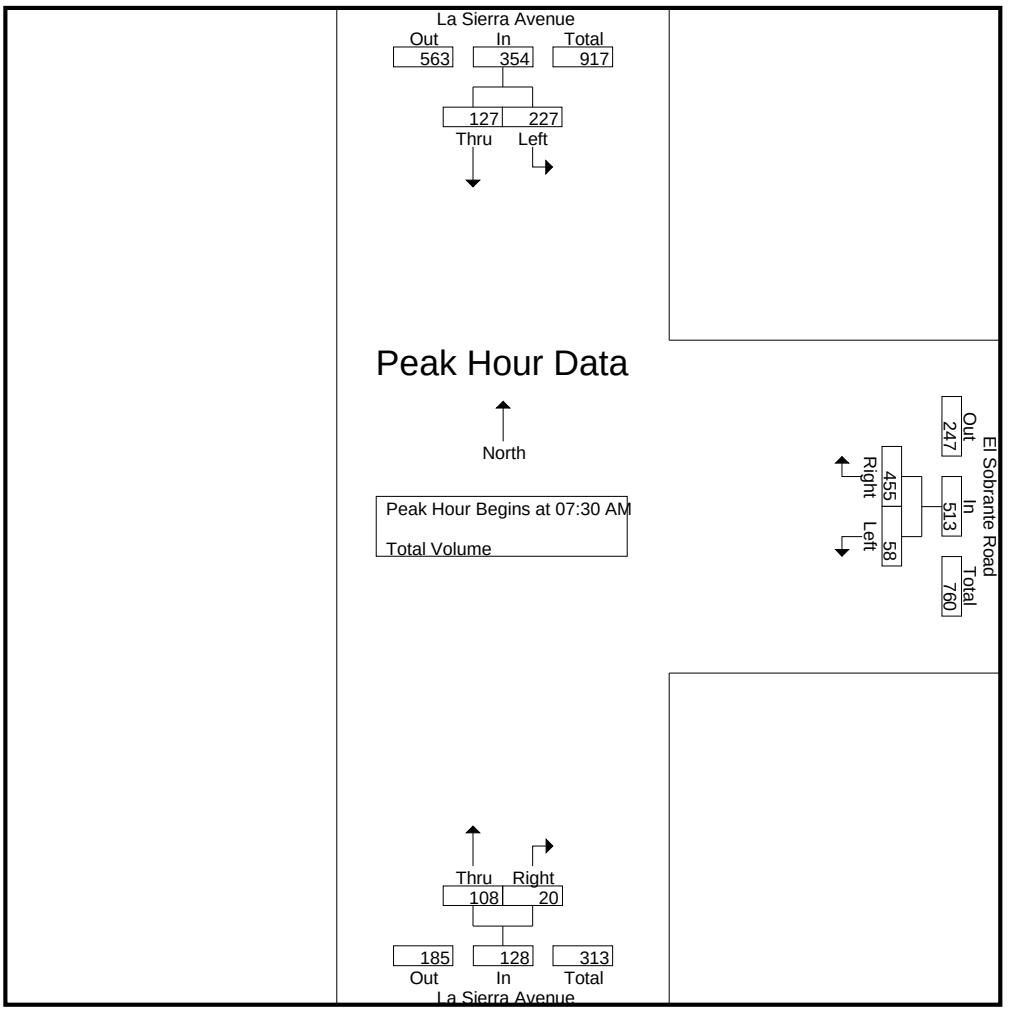
	La Sierra Avenue Southbound			El Sobrante Road Westbound			La Sierra Avenue Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
07:00 AM	48	26	74	14	107	121	25	4	29	224
07:15 AM	45	31	76	24	124	148	28	3	31	255
07:30 AM	51	36	87	14	105	119	32	4	36	242
07:45 AM	55	28	83	18	112	130	22	6	28	241
Total	199	121	320	70	448	518	107	17	124	962
08:00 AM	58	30	88	12	123	135	25	6	31	254
08:15 AM	63	33	96	14	115	129	29	4	33	258
08:30 AM	61	25	86	9	97	106	36	8	44	236
08:45 AM	39	20	59	8	130	138	30	7	37	234
Total	221	108	329	43	465	508	120	25	145	982
Grand Total	420	229	649	113	913	1026	227	42	269	1944
Apprch %	64.7	35.3		11	89		84.4	15.6		
Total %	21.6	11.8	33.4	5.8	47	52.8	11.7	2.2	13.8	

	La Sierra Avenue Southbound			El Sobrante Road Westbound			La Sierra Avenue Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:30 AM										
07:30 AM	51	**36**	87	14	105	119	**32**	4	**36**	242
07:45 AM	55	28	83	**18**	112	130	22	**6**	28	241
08:00 AM	58	30	88	12	**123**	**135**	25	6	31	254
08:15 AM	**63**	33	**96**	14	115	129	29	4	33	**258**
Total Volume	227	127	354	58	455	513	108	20	128	995
% App. Total	64.1	35.9		11.3	88.7		84.4	15.6		
PHF	.901	.882	.922	.806	.925	.950	.844	.833	.889	.964

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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:30 AM			07:15 AM			08:00 AM		
+0 mins.	51	**36**	87	**24**	**124**	**148**	25	6	31
+15 mins.	55	28	83	14	105	119	29	4	33
+30 mins.	58	30	88	18	112	130	**36**	**8**	**44**
+45 mins.	**63**	33	**96**	12	123	135	30	7	37
Total Volume	227	127	354	68	464	532	120	25	145
% App. Total	64.1	35.9		12.8	87.2		82.8	17.2	
PHF	.901	.882	.922	.708	.935	.899	.833	.781	.824

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City of Riverside
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E/W: El Sobrante Road
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File Name : 06_RIV_LS_ElS PM
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Groups Printed- Total Volume

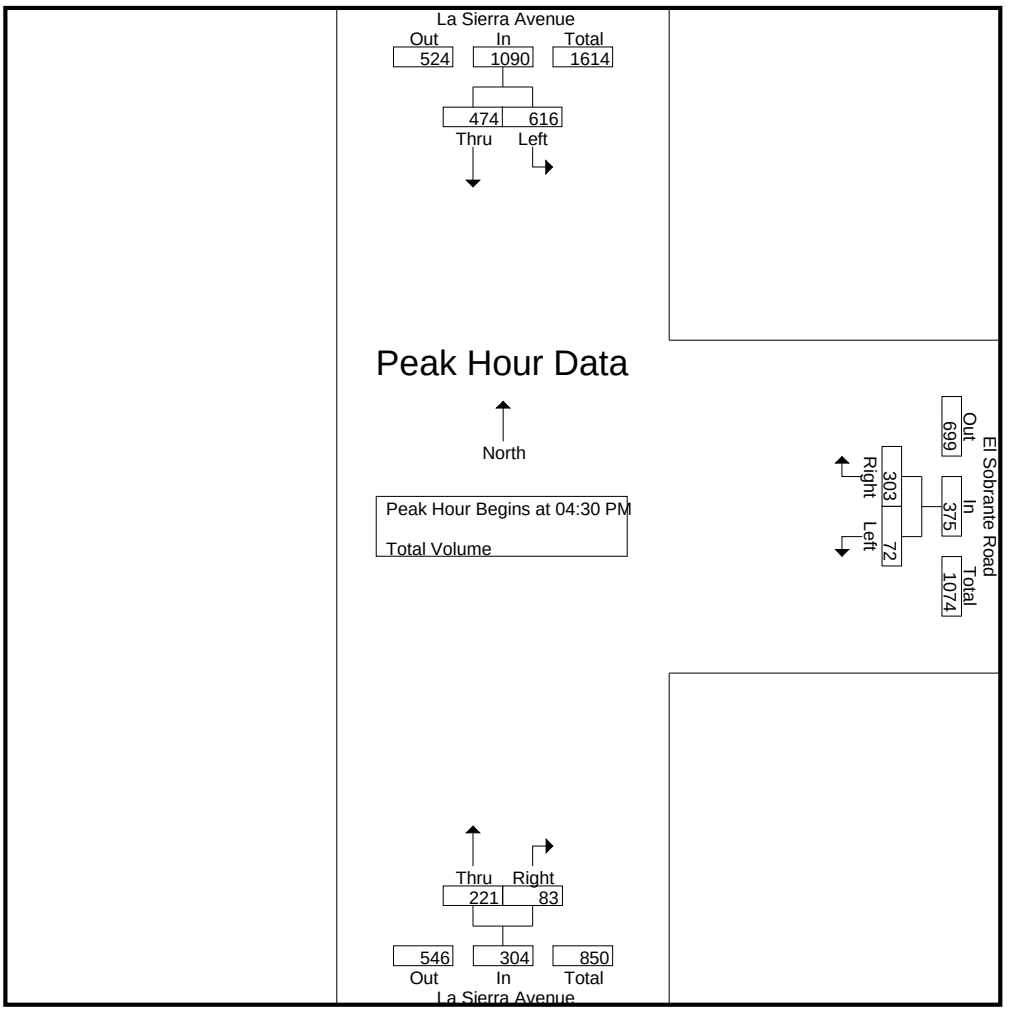
	La Sierra Avenue Southbound			El Sobrante Road Westbound			La Sierra Avenue Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
04:00 PM	126	114	240	20	88	108	62	22	84	432
04:15 PM	146	112	258	19	90	109	60	23	83	450
04:30 PM	142	114	256	18	65	83	65	18	83	422
04:45 PM	145	110	255	23	79	102	68	15	83	440
Total	559	450	1009	80	322	402	255	78	333	1744
05:00 PM	158	122	280	17	68	85	45	26	71	436
05:15 PM	171	128	299	14	91	105	43	24	67	471
05:30 PM	145	102	247	19	71	90	43	25	68	405
05:45 PM	146	100	246	9	52	61	32	16	48	355
Total	620	452	1072	59	282	341	163	91	254	1667
Grand Total	1179	902	2081	139	604	743	418	169	587	3411
Apprch %	56.7	43.3		18.7	81.3		71.2	28.8		
Total %	34.6	26.4	61	4.1	17.7	21.8	12.3	5	17.2	

	La Sierra Avenue Southbound			El Sobrante Road Westbound			La Sierra Avenue Northbound			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 04:30 PM										
04:30 PM	142	114	256	18	65	83	65	18	**83**	422
04:45 PM	145	110	255	**23**	79	102	**68**	15	83	440
05:00 PM	158	122	280	17	68	85	45	**26**	71	436
05:15 PM	**171**	**128**	**299**	14	**91**	**105**	43	24	67	**471**
Total Volume	616	474	1090	72	303	375	221	83	304	1769
% App. Total	56.5	43.5		19.2	80.8		72.7	27.3		
PHF	.901	.926	.911	.783	.832	.893	.813	.798	.916	.939

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City of Riverside
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File Name : 06_RIV_LS_ElS PM
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Start Date : 4/25/2024
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:30 PM			04:00 PM			04:00 PM		
+0 mins.	142	114	256	20	88	108	62	22	**84**
+15 mins.	145	110	255	19	**90**	**109**	60	**23**	83
+30 mins.	158	122	280	18	65	83	65	18	83
+45 mins.	**171**	**128**	**299**	**23**	79	102	**68**	15	83
Total Volume	616	474	1090	80	322	402	255	78	333
% App. Total	56.5	43.5		19.9	80.1		76.6	23.4	
PHF	.901	.926	.911	.870	.894	.922	.938	.848	.991

Location: Riverside
N/S: La Sierra Avenue
E/W: El Sobrante Road

Date: 4/25/2024
Day: Thursday

PEDESTRIANS

	North Leg La Sierra Avenue	East Leg El Sobrante Road	South Leg La Sierra Avenue	West Leg El Sobrante Road	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	0	0	0	0	0
7:15 AM	0	0	0	0	0
7:30 AM	0	0	0	0	0
7:45 AM	0	0	0	0	0
8:00 AM	0	0	0	0	0
8:15 AM	0	0	0	0	0
8:30 AM	0	0	0	0	0
8:45 AM	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0

	North Leg La Sierra Avenue	East Leg El Sobrante Road	South Leg La Sierra Avenue	West Leg El Sobrante Road	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	0	0	0	0	0
4:15 PM	0	0	0	0	0
4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0
5:15 PM	0	0	0	0	0
5:30 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
951-268-6268

Location: Riverside
N/S: La Sierra Avenue
E/W: El Sobrante Road

Date: 4/25/2024
Day: Thursday

BICYCLES

	Southbound La Sierra Avenue			Westbound El Sobrante Road			Northbound La Sierra Avenue			Eastbound El Sobrante Road			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0	0	0	0	0	0	0	0	0

	Southbound La Sierra Avenue			Westbound El Sobrante Road			Northbound La Sierra Avenue			Eastbound El Sobrante Road			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0	0	0	0	0	0	0	0	0

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Corona, CA 92878
(951) 268-6268

City of Riverside
N/S: McAllister Street
E/W: El Sobrante Road
Weather: Clear

File Name : 07_RIV_McA_ElS AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 1

Groups Printed- Total Volume

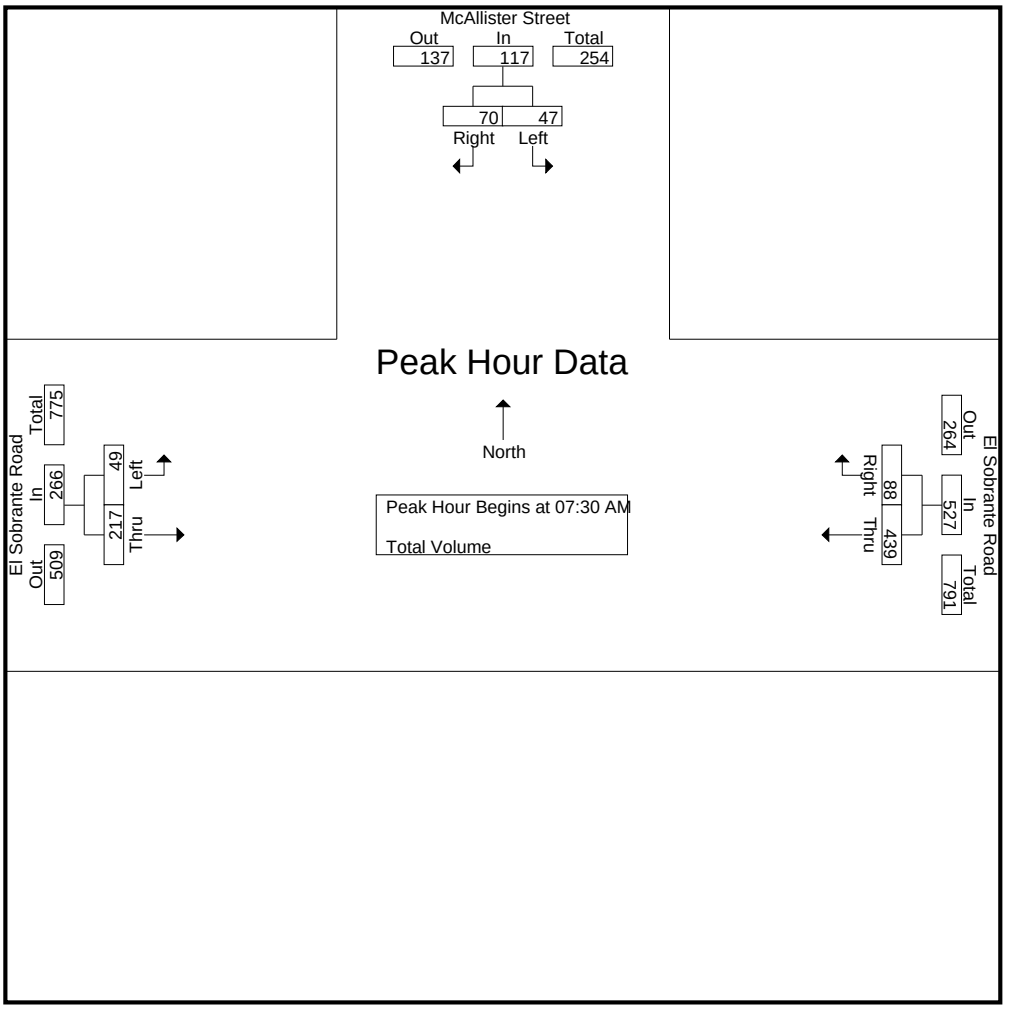
	McAllister Street Southbound				El Sobrante Road Westbound				El Sobrante Road Eastbound					
Start Time	Left	Right	RTOR	App. Total	Thru	Right	RTOR	App. Total	Left	Thru	App. Total	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	10	22	9	32	104	13	1	117	9	47	56	10	205	215
07:15 AM	2	34	11	36	106	8	0	114	10	42	52	11	202	213
07:30 AM	5	17	6	22	114	13	1	127	13	45	58	7	207	214
07:45 AM	13	14	4	27	106	28	5	134	9	61	70	9	231	240
Total	30	87	30	117	430	62	7	492	41	195	236	37	845	882
08:00 AM	16	19	3	35	103	32	4	135	15	51	66	7	236	243
08:15 AM	13	20	5	33	116	15	0	131	12	60	72	5	236	241
08:30 AM	3	17	4	20	95	8	1	103	15	55	70	5	193	198
08:45 AM	5	14	3	19	110	11	0	121	8	41	49	3	189	192
Total	37	70	15	107	424	66	5	490	50	207	257	20	854	874
Grand Total	67	157	45	224	854	128	12	982	91	402	493	57	1699	1756
Apprch %	29.9	70.1			87	13			18.5	81.5				
Total %	3.9	9.2		13.2	50.3	7.5		57.8	5.4	23.7	29	3.2	96.8	

	McAllister Street Southbound			El Sobrante Road Westbound			El Sobrante Road Eastbound			
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:30 AM										
07:30 AM	5	17	22	114	13	127	13	45	58	207
07:45 AM	13	14	27	106	28	134	9	**61**	70	231
08:00 AM	**16**	19	**35**	103	**32**	**135**	**15**	51	66	**236**
08:15 AM	13	**20**	33	**116**	15	131	12	60	**72**	236
Total Volume	47	70	117	439	88	527	49	217	266	910
% App. Total	40.2	59.8		83.3	16.7		18.4	81.6		
PHF	.734	.875	.836	.946	.688	.976	.817	.889	.924	.964

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City of Riverside
N/S: McAllister Street
E/W: El Sobrante Road
Weather: Clear

File Name : 07_RIV_McA_EIS AM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:15 AM			07:30 AM			07:45 AM		
+0 mins.	2	**34**	**36**	114	13	127	9	**61**	70
+15 mins.	5	17	22	106	28	134	**15**	51	66
+30 mins.	13	14	27	103	**32**	**135**	12	60	**72**
+45 mins.	**16**	19	35	**116**	15	131	15	55	70
Total Volume	36	84	120	439	88	527	51	227	278
% App. Total	30	70		83.3	16.7		18.3	81.7	
PHF	.563	.618	.833	.946	.688	.976	.850	.930	.965

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City of Riverside
N/S: McAllister Street
E/W: El Sobrante Road
Weather: Clear

File Name : 07_RIV_McA_EIS PM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 1

Groups Printed- Total Volume

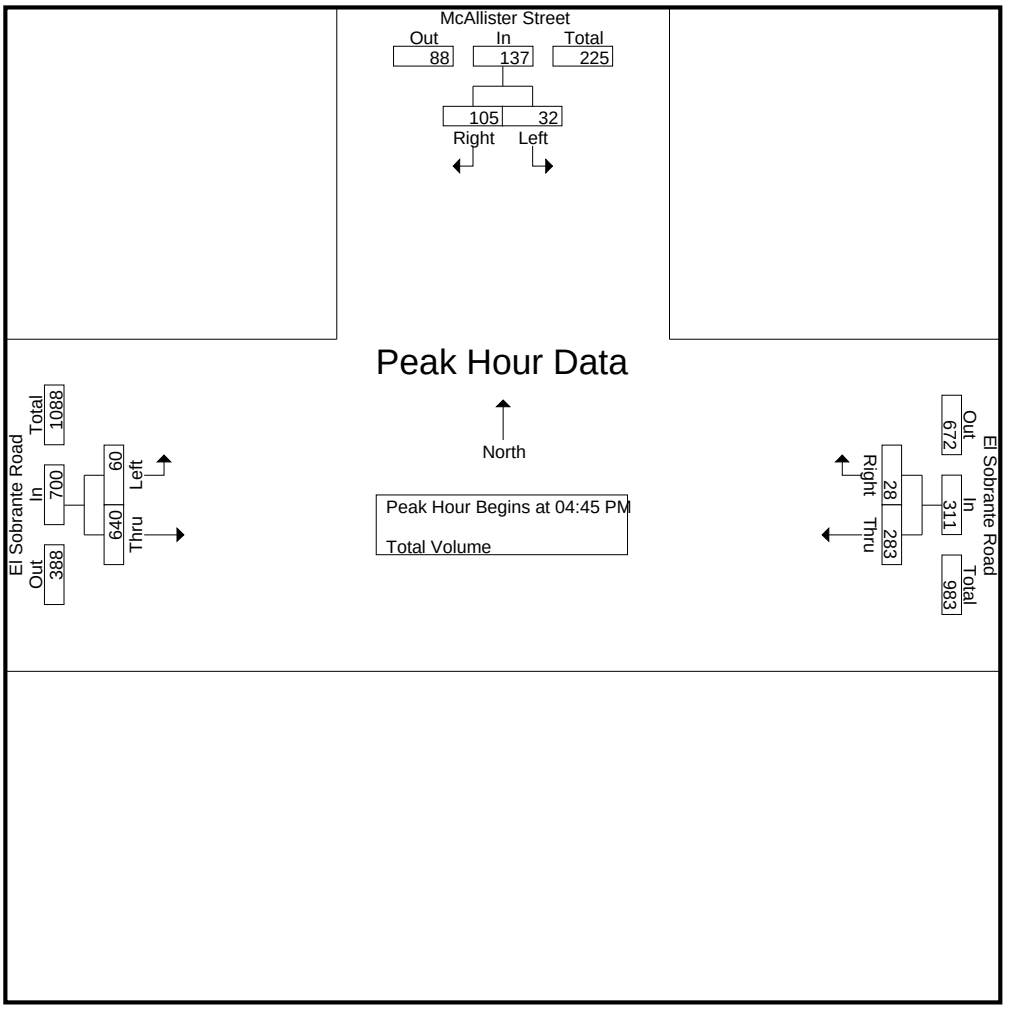
	McAllister Street Southbound				El Sobrante Road Westbound				El Sobrante Road Eastbound					
Start Time	Left	Right	RTOR	App. Total	Thru	Right	RTOR	App. Total	Left	Thru	App. Total	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	9	35	10	44	93	10	2	103	15	125	140	12	287	299
04:15 PM	5	19	4	24	75	5	1	80	20	144	164	5	268	273
04:30 PM	9	20	5	29	61	8	0	69	19	144	163	5	261	266
04:45 PM	9	27	5	36	71	10	3	81	11	143	154	8	271	279
Total	32	101	24	133	300	33	6	333	65	556	621	30	1087	1117
05:00 PM	9	35	10	44	67	4	0	71	20	159	179	10	294	304
05:15 PM	9	22	6	31	83	8	1	91	12	173	185	7	307	314
05:30 PM	5	21	2	26	62	6	0	68	17	165	182	2	276	278
05:45 PM	5	16	5	21	55	3	0	58	14	141	155	5	234	239
Total	28	94	23	122	267	21	1	288	63	638	701	24	1111	1135
Grand Total	60	195	47	255	567	54	7	621	128	1194	1322	54	2198	2252
Apprch %	23.5	76.5			91.3	8.7			9.7	90.3				
Total %	2.7	8.9		11.6	25.8	2.5		28.3	5.8	54.3	60.1	2.4	97.6	

	McAllister Street Southbound			El Sobrante Road Westbound			El Sobrante Road Eastbound			
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 04:45 PM										
04:45 PM	**9**	27	36	71	**10**	81	11	143	154	271
05:00 PM	9	**35**	**44**	67	4	71	**20**	159	179	294
05:15 PM	9	22	31	**83**	8	**91**	12	**173**	**185**	**307**
05:30 PM	5	21	26	62	6	68	17	165	182	276
Total Volume	32	105	137	283	28	311	60	640	700	1148
% App. Total	23.4	76.6		91	9		8.6	91.4		
PHF	.889	.750	.778	.852	.700	.854	.750	.925	.946	.935

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City of Riverside
N/S: McAllister Street
E/W: El Sobrante Road
Weather: Clear

File Name : 07_RIV_McA_EIS PM
Site Code : 05124383
Start Date : 4/25/2024
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:30 PM			04:00 PM			05:00 PM		
+0 mins.	**9**	20	29	**93**	**10**	**103**	**20**	159	179
+15 mins.	9	27	36	75	5	80	12	**173**	**185**
+30 mins.	9	**35**	**44**	61	8	69	17	165	182
+45 mins.	9	22	31	71	10	81	14	141	155
Total Volume	36	104	140	300	33	333	63	638	701
% App. Total	25.7	74.3		90.1	9.9		9	91	
PHF	1.000	.743	.795	.806	.825	.808	.788	.922	.947

Location: Riverside
N/S: McAllister Street
E/W: El Sobrante Road

Date: 4/25/2024
Day: Thursday

PEDESTRIANS

	North Leg McAllister Street	East Leg El Sobrante Road	South Leg Dead End	West Leg El Sobrante Road	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	0	0	0	0	0
7:15 AM	0	0	0	0	0
7:30 AM	0	0	0	0	0
7:45 AM	0	0	0	0	0
8:00 AM	0	0	0	0	0
8:15 AM	0	0	0	0	0
8:30 AM	0	0	0	0	0
8:45 AM	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0

	North Leg McAllister Street	East Leg El Sobrante Road	South Leg Dead End	West Leg El Sobrante Road	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	0	0	0	0	0
4:15 PM	0	0	0	0	0
4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0
5:15 PM	0	0	0	0	0
5:30 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0

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Corona, CA 92878
951-268-6268

Location: Riverside
N/S: McAllister Street
E/W: El Sobrante Road

Date: 4/25/2024
Day: Thursday

BICYCLES

	Southbound McAllister Street			Westbound El Sobrante Road			Northbound Dead End			Eastbound El Sobrante Road			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0	0	0	0	0	0	0	0	0

	Southbound McAllister Street			Westbound El Sobrante Road			Northbound Dead End			Eastbound El Sobrante Road			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0	0	0	0	0	0	0	0	0

Counts Unlimited, Inc.
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Counts Unlimited, Inc.

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County of Riverside
El Sobrante Road
E/ McAllister Street
24 Hour Directional Volume Count

CRV005
Site Code: 051-24383

Start	4/25/2024	Eastbound		Hour Totals		Westbound		Hour Totals		Combined Totals	
Time	Thu	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
12:00		19	64			2	67				
12:15		15	70			2	47				
12:30		9	82			3	89				
12:45		5	76	48	292	4	86	11	289	59	581
01:00		10	76			4	71				
01:15		13	68			3	62				
01:30		9	88			3	87				
01:45		5	102	37	334	6	94	16	314	53	648
02:00		5	84			6	82				
02:15		6	94			9	79				
02:30		10	131			7	71				
02:45		6	142	27	451	1	**121**	23	353	50	804
03:00		4	137			15	**101**				
03:15		5	142			27	**98**				
03:30		5	127			25	**97**				
03:45		7	132	21	538	46	94	113	390	134	928
04:00		6	134			64	101				
04:15		8	149			79	79				
04:30		10	153			103	69				
04:45		11	**152**	35	588	86	78	332	327	367	915
05:00		17	**168**			91	71				
05:15		31	**182**			73	90				
05:30		41	**170**			106	68				
05:45		35	146	124	666	77	58	347	287	471	953
06:00		44	122			76	61				
06:15		65	139			103	51				
06:30		54	133			87	43				
06:45		47	126	210	520	93	38	359	193	569	713
07:00		57	97			116	47				
07:15		44	86			114	37				
07:30		50	84			**126**	32				
07:45		**74**	70	225	337	**129**	28	485	144	710	481
08:00		**67**	61			**131**	28				
08:15		**73**	66			**132**	28				
08:30		**58**	54			102	28				
08:45		46	66	244	247	121	24	486	108	730	355
09:00		41	54			99	23				
09:15		41	54			90	23				
09:30		42	54			91	20				
09:45		44	45	168	207	106	21	386	87	554	294
10:00		43	40			82	29				
10:15		49	31			78	17				
10:30		53	25			78	11				
10:45		47	30	192	126	80	7	318	64	510	190
11:00		49	25			72	6				
11:15		51	27			60	11				
11:30		47	15			72	7				
11:45		73	20	220	87	57	6	261	30	481	117
Total		1551	4393	1551	4393	3137	2586	3137	2586	4688	6979
Combined Total		5944		5944		5723		5723		11667	
AM Peak	-	07:45	-	-	-	07:30	-	-	-	-	-
Vol.	-	272	-	-	-	518	-	-	-	-	-
P.H.F.		0.919				0.981					
PM Peak	-	-	04:45	-	-	-	02:45	-	-	-	-
Vol.	-	-	672	-	-	-	417	-	-	-	-
P.H.F.			0.923				0.862				
Percentage		26.1%	73.9%			54.8%	45.2%				
ADT/AADT		ADT 11,667		AADT 11,667							

Counts Unlimited, Inc.

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County of Riverside
La Sierra Avenue
N/ El Sobrante Road
24 Hour Directional Volume Count

CRV003
Site Code: 051-24383

Start Time	4/25/2024 Thu	Northbound Morning	Northbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Southbound Morning	Southbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Combined Totals Morning	Combined Totals Afternoon
12:00		8	99			23	106				
12:15		3	74			16	81				
12:30		9	106			2	116				
12:45		6	101	26	380	8	96	49	399	75	779
01:00		4	85			10	100				
01:15		3	83			24	98				
01:30		6	124			9	108				
01:45		8	113	21	405	4	153	47	459	68	864
02:00		6	109			6	121				
02:15		8	132			7	128				
02:30		11	129			12	170				
02:45		13	**171**	38	541	6	166	31	585	69	1126
03:00		16	**133**			5	192				
03:15		27	**162**			7	250				
03:30		24	**162**			8	221				
03:45		44	145	111	602	7	213	27	876	138	1478
04:00		67	150			8	240				
04:15		83	150			18	258				
04:30		99	130			15	**256**				
04:45		100	147	349	577	13	**255**	54	1009	403	1586
05:00		92	113			29	**280**				
05:15		86	134			50	**299**				
05:30		103	114			66	248				
05:45		81	84	362	445	76	246	221	1073	583	1518
06:00		85	92			85	181				
06:15		94	87			73	218				
06:30		97	84			94	166				
06:45		107	79	383	342	79	138	331	703	714	1045
07:00		132	62			74	112				
07:15		152	69			76	112				
07:30		137	53			**87**	103				
07:45		134	43	555	227	**83**	90	320	417	875	644
08:00		**148**	43			**88**	65				
08:15		**144**	39			**96**	81				
08:30		**133**	47			86	72				
08:45		**160**	37	585	166	59	74	329	292	914	458
09:00		117	35			92	64				
09:15		128	30			60	61				
09:30		111	37			59	70				
09:45		130	30	486	132	72	56	283	251	769	383
10:00		88	31			60	45				
10:15		84	30			72	38				
10:30		114	15			80	35				
10:45		102	24	388	100	65	31	277	149	665	249
11:00		85	9			83	29				
11:15		80	17			81	32				
11:30		92	7			76	21				
11:45		72	9	329	42	87	23	327	105	656	147
Total		3633	3959	3633	3959	2296	6318	2296	6318	5929	10277
Combined Total		7592		7592		8614		8614		16206	
AM Peak	-	08:00	-	-	-	07:30	-	-	-	-	-
Vol.	-	585	-	-	-	354	-	-	-	-	-
P.H.F.		0.914				0.922					
PM Peak	-	-	02:45	-	-	-	04:30	-	-	-	-
Vol.	-	-	628	-	-	-	1090	-	-	-	-
P.H.F.			0.918				0.911				
Percentage		47.9%	52.1%			26.7%	73.3%				
ADT/AADT		ADT 16,206		AADT 16,206							

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County of Riverside
McAllister Street
N/ El Sobrante Road
24 Hour Directional Volume Count

CRV004
Site Code: 051-24383

Start Time	4/25/2024 Thu	Northbound Morning	Northbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Southbound Morning	Southbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Combined Totals Morning	Combined Totals Afternoon
12:00		0	14			0	16				
12:15		0	16			1	12				
12:30		1	12			0	11				
12:45		0	12	1	54	0	16	1	55	2	109
01:00		1	14			0	10				
01:15		0	11			0	7				
01:30		1	12			0	14				
01:45		0	16	2	53	1	14	1	45	3	98
02:00		0	25			0	18				
02:15		0	15			0	17				
02:30		0	22			0	**39**				
02:45		0	17	0	79	1	**33**	1	107	1	186
03:00		0	20			0	**25**				
03:15		1	16			0	**19**				
03:30		1	**27**			0	32				
03:45		1	**24**	3	87	1	27	1	103	4	190
04:00		0	**23**			0	34				
04:15		2	**24**			2	20				
04:30		2	27			1	24				
04:45		2	18	6	92	1	31	4	109	10	201
05:00		1	24			2	34				
05:15		2	19			1	25				
05:30		0	23			8	24				
05:45		5	17	8	83	9	16	20	99	28	182
06:00		12	12			10	18				
06:15		15	25			14	7				
06:30		24	14			9	14				
06:45		22	6	73	57	7	13	40	52	113	109
07:00		21	16			24	12				
07:15		18	8			25	8				
07:30		**25**	7			**16**	12				
07:45		**32**	9	96	40	**23**	7	88	39	184	79
08:00		**43**	3			**32**	8				
08:15		**27**	3			**28**	7				
08:30		22	7			16	5				
08:45		19	3	111	16	16	9	92	29	203	45
09:00		15	2			13	3				
09:15		10	4			14	8				
09:30		9	8			13	5				
09:45		14	4	48	18	11	3	51	19	99	37
10:00		13	6			9	0				
10:15		16	3			12	1				
10:30		19	2			11	0				
10:45		12	1	60	12	12	3	44	4	104	16
11:00		16	0			16	1				
11:15		13	1			11	2				
11:30		17	3			15	1				
11:45		10	3	56	7	13	2	55	6	111	13
Total		464	598	464	598	398	667	398	667	862	1265
Combined Total		1062		1062		1065		1065		2127	
AM Peak	-	07:30	-	-	-	07:30	-	-	-	-	-
Vol.	-	127	-	-	-	99	-	-	-	-	-
P.H.F.		0.738				0.773					
PM Peak	-	-	03:30	-	-	-	02:30	-	-	-	-
Vol.	-	-	98	-	-	-	116	-	-	-	-
P.H.F.			0.907				0.744				
Percentage		43.7%	56.3%			37.4%	62.6%				
ADT/AADT		ADT 2,127		AADT 2,127							

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County of Riverside
Travertine Drive
S/ Sweet Avenue
24 Hour Directional Volume Count

CRV006
Site Code: 051-24383

Start Time	4/25/2024 Thu	Northbound Morning	Northbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Southbound Morning	Southbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Combined Totals Morning	Combined Totals Afternoon
12:00		1	4			0	7				
12:15		0	8			0	3				
12:30		3	2			1	6				
12:45		0	1	4	15	1	4	2	20	6	35
01:00		0	6			2	5				
01:15		0	7			0	6				
01:30		0	6			0	6				
01:45		0	6	0	25	1	5	3	22	3	47
02:00		0	**6**			0	8				
02:15		0	**13**			0	9				
02:30		0	**3**			1	8				
02:45		0	**12**	0	34	0	8	1	33	1	67
03:00		0	6			0	12				
03:15		0	7			0	6				
03:30		0	3			0	6				
03:45		0	4	0	20	0	5	0	29	0	49
04:00		2	10			0	5				
04:15		3	8			1	11				
04:30		3	5			1	6				
04:45		3	6	11	29	0	10	2	32	13	61
05:00		3	9			0	**11**				
05:15		3	5			1	**11**				
05:30		2	2			0	**8**				
05:45		4	8	12	24	0	**12**	1	42	13	66
06:00		5	2			1	6				
06:15		3	3			2	5				
06:30		3	4			1	14				
06:45		9	6	20	15	2	10	6	35	26	50
07:00		4	5			4	11				
07:15		**15**	6			3	5				
07:30		**15**	3			2	5				
07:45		**15**	1	49	15	**5**	7	14	28	63	43
08:00		**8**	2			**12**	3				
08:15		1	0			**10**	7				
08:30		11	4			**5**	7				
08:45		4	4	24	10	1	12	28	29	52	39
09:00		7	2			2	8				
09:15		4	5			3	2				
09:30		7	2			1	6				
09:45		5	1	23	10	2	5	8	21	31	31
10:00		4	0			2	5				
10:15		2	1			6	0				
10:30		7	1			1	1				
10:45		6	0	19	2	1	2	10	8	29	10
11:00		1	3			3	2				
11:15		5	0			4	1				
11:30		6	1			2	1				
11:45		5	3	17	7	2	1	11	5	28	12
Total		179	206	179	206	86	304	86	304	265	510
Combined Total		385		385		390		390		775	
AM Peak	-	07:15	-	-	-	07:45	-	-	-	-	-
Vol.	-	53	-	-	-	32	-	-	-	-	-
P.H.F.		0.883				0.667					
PM Peak	-	-	02:00	-	-	-	05:00	-	-	-	-
Vol.	-	-	34	-	-	-	42	-	-	-	-
P.H.F.			0.654				0.875				
Percentage		46.5%	53.5%			22.1%	77.9%				
ADT/AADT		ADT 775		AADT 775							

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CIty of Riverside
Indiana Avenue
W/ La Sierra Avenue
24 Hour Directional Volume Count

RIV001
Site Code: 051-24383

Start Time	4/25/2024 Thu	Eastbound Morning	Eastbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Westbound Morning	Westbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Combined Totals Morning	Combined Totals Afternoon
12:00		11	138			9	108				
12:15		5	139			8	106				
12:30		10	166			16	107				
12:45		8	170	34	613	6	106	39	427	73	1040
01:00		10	140			6	124				
01:15		4	128			7	145				
01:30		6	153			8	130				
01:45		4	139	24	560	7	144	28	543	52	1103
02:00		16	163			9	140				
02:15		3	206			8	142				
02:30		8	214			5	162				
02:45		8	236	35	819	4	171	26	615	61	1434
03:00		10	**301**			3	157				
03:15		14	**281**			8	**222**				
03:30		8	**322**			7	**186**				
03:45		18	**371**	50	1275	10	**180**	28	745	78	2020
04:00		26	290			11	**181**				
04:15		24	237			23	153				
04:30		51	223			18	156				
04:45		33	235	134	985	30	166	82	656	216	1641
05:00		32	286			42	213				
05:15		51	242			44	159				
05:30		66	236			59	148				
05:45		63	181	212	945	79	124	224	644	436	1589
06:00		79	187			67	126				
06:15		77	219			57	157				
06:30		79	174			60	159				
06:45		77	145	312	725	92	113	276	555	588	1280
07:00		115	125			95	108				
07:15		114	119			122	105				
07:30		159	118			132	117				
07:45		**177**	93	565	455	**169**	90	518	420	1083	875
08:00		**200**	105			**236**	103				
08:15		**189**	99			**232**	85				
08:30		**241**	99			**145**	83				
08:45		141	80	771	383	75	69	688	340	1459	723
09:00		123	73			86	67				
09:15		97	80			118	81				
09:30		105	68			90	78				
09:45		118	54	443	275	97	48	391	274	834	549
10:00		122	52			96	36				
10:15		129	38			107	47				
10:30		102	40			89	37				
10:45		134	26	487	156	95	34	387	154	874	310
11:00		120	39			106	25				
11:15		101	24			100	28				
11:30		138	23			123	21				
11:45		163	16	522	102	108	24	437	98	959	200
Total		3589	7293	3589	7293	3124	5471	3124	5471	6713	12764
Combined Total		10882		10882		8595		8595		19477	
AM Peak	-	07:45	-	-	-	07:45	-	-	-	-	-
Vol.	-	807	-	-	-	782	-	-	-	-	-
P.H.F.		0.837				0.828					
PM Peak	-	-	03:00	-	-	-	03:15	-	-	-	-
Vol.	-	-	1275	-	-	-	769	-	-	-	-
P.H.F.			0.859				0.866				
Percentage		33.0%	67.0%			36.3%	63.7%				
ADT/AADT		ADT 19,477		AADT 19,477							

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CIty of Riverside
La Sierra Avenue
S/ Victoria Avenue
24 Hour Directional Volume Count

RIV002
Site Code: 051-24383

Start Time	4/25/2024 Thu	Northbound Morning	Northbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Southbound Morning	Southbound Afternoon	Hour Totals Morning	Hour Totals Afternoon	Combined Totals Morning	Combined Totals Afternoon
12:00		15	212			51	198				
12:15		10	173			25	195				
12:30		13	193			19	222				
12:45		13	222	51	800	18	224	113	839	164	1639
01:00		8	155			17	196				
01:15		8	189			23	200				
01:30		7	203			11	213				
01:45		13	230	36	777	16	269	67	878	103	1655
02:00		15	204			11	223				
02:15		11	216			17	253				
02:30		17	228			16	304				
02:45		17	**299**	60	947	9	269	53	1049	113	1996
03:00		21	**244**			9	327				
03:15		36	**277**			11	361				
03:30		38	**251**			14	381				
03:45		53	242	148	1014	9	361	43	1430	191	2444
04:00		79	251			19	429				
04:15		122	239			13	404				
04:30		148	225			17	**416**				
04:45		148	223	497	938	26	**432**	75	1681	572	2619
05:00		156	189			31	**455**				
05:15		151	230			56	**497**				
05:30		162	213			72	404				
05:45		155	176	624	808	77	386	236	1742	860	2550
06:00		195	168			99	331				
06:15		192	196			87	276				
06:30		227	162			120	303				
06:45		227	137	841	663	99	275	405	1185	1246	1848
07:00		271	132			108	253				
07:15		**319**	126			110	242				
07:30		**312**	105			140	246				
07:45		**314**	105	1216	468	149	229	507	970	1723	1438
08:00		**303**	99			191	193				
08:15		314	89			160	225				
08:30		230	93			145	208				
08:45		249	77	1096	358	124	178	620	804	1716	1162
09:00		264	66			129	145				
09:15		255	72			108	151				
09:30		207	55			122	158				
09:45		248	59	974	252	140	119	499	573	1473	825
10:00		189	57			131	107				
10:15		193	55			134	92				
10:30		227	29			134	87				
10:45		196	38	805	179	142	68	541	354	1346	533
11:00		172	28			**167**	82				
11:15		193	37			**156**	62				
11:30		189	20			**150**	56				
11:45		167	18	721	103	**180**	42	653	242	1374	345
Total		7069	7307	7069	7307	3812	11747	3812	11747	10881	19054
Combined Total		14376		14376		15559		15559		29935	
AM Peak	-	07:15	-	-	-	11:00	-	-	-	-	-
Vol.	-	1248	-	-	-	653	-	-	-	-	-
P.H.F.		0.978				0.855					
PM Peak	-	-	02:45	-	-	-	04:30	-	-	-	-
Vol.	-	-	1071	-	-	-	1800	-	-	-	-
P.H.F.			0.895				0.905				
Percentage		49.2%	50.8%			24.5%	75.5%				
ADT/AADT		ADT 29,935		AADT 29,935							

APPENDIX 3.2: EXISTING (2024) CONDITIONS INTERSECTION OPERATIONS ANALYSIS WORKSHEETS

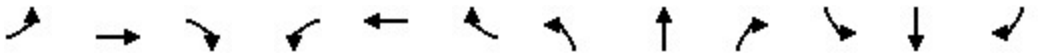
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Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Configurations							
Traffic Volume (vph)	275	0	461	611	1303	924	133
Future Volume (vph)	275	0	461	611	1303	924	133
Turn Type	Split	NA	Perm	Prot	NA	NA	Perm
Protected Phases	8	8		5	2	6	
Permitted Phases			8				6
Detector Phase	8	8	8	5	2	6	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	9.0	26.5	29.5	29.5
Total Split (s)	37.0	37.0	37.0	30.0	63.0	33.0	33.0
Total Split (%)	37.0%	37.0%	37.0%	30.0%	63.0%	33.0%	33.0%
Yellow Time (s)	4.0	4.0	4.0	3.0	4.5	4.5	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	4.0	5.5	5.5	5.5
Lead/Lag				Lead		Lag	Lag
Lead-Lag Optimize?				Yes		Yes	Yes
Recall Mode	None	None	None	None	C-Min	C-Min	C-Min
Act Effct Green (s)	21.2	21.2	21.2	22.5	68.3	41.8	41.8
Actuated g/C Ratio	0.21	0.21	0.21	0.23	0.68	0.42	0.42
v/c Ratio	0.73	0.68	0.67	0.82	0.39	0.45	0.19
Control Delay (s/veh)	48.0	34.5	33.5	42.0	13.5	23.8	6.2
Queue Delay	0.0	0.0	0.0	0.0	0.3	0.0	0.0
Total Delay (s/veh)	48.0	34.5	33.5	42.0	13.8	23.8	6.2
LOS	D	C	C	D	B	C	A
Approach Delay (s/veh)		38.7			22.8	21.6	
Approach LOS		D			C	C	

Intersection Summary	
Cycle Length: 100	
Actuated Cycle Length: 100	
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow, Master Intersection	
Natural Cycle: 60	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.82	
Intersection Signal Delay (s/veh): 25.6	Intersection LOS: C
Intersection Capacity Utilization 64.6%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 1: La Sierra Av. & SR-91 WB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	275	0	461	611	1303	0	0	924	133
Future Volume (veh/h)	0	0	0	275	0	461	611	1303	0	0	924	133
Initial Q (Qb), veh				0	0	0	0	0	0	0	0	0
Lane Width Adj.				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach					No			No			No	
Adj Sat Flow, veh/h/ln				1856	1900	1885	1870	1870	0	0	1856	1826
Adj Flow Rate, veh/h				380	0	198	630	1343	0	0	953	94
Peak Hour Factor				0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %				3	0	1	2	2	0	0	3	5
Cap, veh/h				543	0	245	692	3786	0	0	2539	763
Arrive On Green				0.15	0.00	0.15	0.40	1.00	0.00	0.00	0.50	0.50
Sat Flow, veh/h				3534	0	1598	3456	5274	0	0	5233	1522
Grp Volume(v), veh/h				380	0	198	630	1343	0	0	953	94
Grp Sat Flow(s),veh/h/ln				1767	0	1598	1728	1702	0	0	1689	1522
Q Serve(g_s), s				10.2	0.0	12.0	17.2	0.0	0.0	0.0	11.6	3.3
Cycle Q Clear(g_c), s				10.2	0.0	12.0	17.2	0.0	0.0	0.0	11.6	3.3
Prop In Lane				1.00		1.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h				543	0	245	692	3786	0	0	2539	763
V/C Ratio(X)				0.70	0.00	0.81	0.91	0.35	0.00	0.00	0.38	0.12
Avail Cap(c_a), veh/h				1131	0	511	898	3786	0	0	2539	763
HCM Platoon Ratio				1.00	1.00	1.00	2.00	2.00	1.00	1.00	1.00	1.00
Upstream Filter(I)				1.00	0.00	1.00	0.73	0.73	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh				40.1	0.0	40.9	29.1	0.0	0.0	0.0	15.3	13.3
Incr Delay (d2), s/veh				1.2	0.0	4.7	7.4	0.2	0.0	0.0	0.4	0.3
Initial Q Delay(d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln				4.3	0.0	4.8	5.8	0.1	0.0	0.0	4.1	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh				41.4	0.0	45.6	36.5	0.2	0.0	0.0	15.7	13.6
LnGrp LOS				D		D	D	A			B	B
Approach Vol, veh/h					578			1973			1047	
Approach Delay, s/veh					42.8			11.8			15.5	
Approach LOS					D			B			B	

Timer - Assigned Phs	2	5	6	8
Phs Duration (G+Y+Rc), s	79.7	24.0	55.6	20.3
Change Period (Y+Rc), s	5.5	4.0	5.5	5.0
Max Green Setting (Gmax), s	57.5	26.0	27.5	32.0
Max Q Clear Time (g_c+I1), s	2.0	19.2	13.6	14.0
Green Ext Time (p_c), s	12.3	0.8	5.4	1.4

Intersection Summary	
HCM 7th Control Delay, s/veh	17.9
HCM 7th LOS	B

Notes

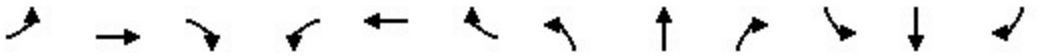
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	410	1	399	1456	455	302	895
Future Volume (vph)	410	1	399	1456	455	302	895
Turn Type	Split	NA	Perm	NA	Perm	Prot	NA
Protected Phases	4	4		2		1	6
Permitted Phases			4		2		
Detector Phase	4	4	4	2	2	1	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	29.5	29.5	9.0	26.5
Total Split (s)	37.0	37.0	37.0	45.0	45.0	18.0	63.0
Total Split (%)	37.0%	37.0%	37.0%	45.0%	45.0%	18.0%	63.0%
Yellow Time (s)	4.0	4.0	4.0	4.5	4.5	3.0	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.5	5.5	4.0	5.5
Lead/Lag				Lag	Lag	Lead	
Lead-Lag Optimize?				Yes	Yes	Yes	
Recall Mode	None	None	None	C-Min	C-Min	None	C-Min
Act Effct Green (s)	23.3	23.3	23.3	48.9	48.9	13.3	66.2
Actuated g/C Ratio	0.23	0.23	0.23	0.49	0.49	0.13	0.66
v/c Ratio	0.75	0.72	0.62	0.62	0.48	0.70	0.28
Control Delay (s/veh)	46.7	37.2	23.2	21.5	3.7	48.8	1.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	46.7	37.2	23.2	21.5	3.7	48.8	1.8
LOS	D	D	C	C	A	D	A
Approach Delay (s/veh)		36.1		17.2			13.7
Approach LOS		D		B			B

Intersection Summary	
Cycle Length: 100	
Actuated Cycle Length: 100	
Offset: 28 (28%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow	
Natural Cycle: 60	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.75	
Intersection Signal Delay (s/veh): 20.1	Intersection LOS: C
Intersection Capacity Utilization 64.6%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 2: La Sierra Av. & SR-91 EB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	410	1	399	0	0	0	0	1456	455	302	895	0
Future Volume (veh/h)	410	1	399	0	0	0	0	1456	455	302	895	0
Initial Q (Qb), veh	0	0	0				0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No						No			No	
Adj Sat Flow, veh/h/ln	1885	1900	1811				0	1856	1856	1841	1856	0
Adj Flow Rate, veh/h	486	0	126				0	1517	322	315	932	0
Peak Hour Factor	0.96	0.96	0.96				0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	1	0	6				0	3	3	4	3	0
Cap, veh/h	594	0	254				0	2914	903	389	3696	0
Arrive On Green	0.17	0.00	0.17				0.00	0.58	0.58	0.04	0.24	0.00
Sat Flow, veh/h	3591	0	1535				0	5233	1570	3401	5233	0
Grp Volume(v), veh/h	486	0	126				0	1517	322	315	932	0
Grp Sat Flow(s),veh/h/ln	1795	0	1535				0	1689	1570	1700	1689	0
Q Serve(g_s), s	13.1	0.0	7.5				0.0	18.2	11.0	9.2	14.9	0.0
Cycle Q Clear(g_c), s	13.1	0.0	7.5				0.0	18.2	11.0	9.2	14.9	0.0
Prop In Lane	1.00		1.00				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	594	0	254				0	2914	903	389	3696	0
V/C Ratio(X)	0.82	0.00	0.50				0.00	0.52	0.36	0.81	0.25	0.00
Avail Cap(c_a), veh/h	1149	0	491				0	2914	903	476	3696	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	0.33	0.33	1.00
Upstream Filter(I)	1.00	0.00	1.00				0.00	0.57	0.57	0.85	0.85	0.00
Uniform Delay (d), s/veh	40.3	0.0	37.9				0.0	12.9	11.3	47.0	15.9	0.0
Incr Delay (d2), s/veh	2.1	0.0	1.1				0.0	0.4	0.6	6.7	0.1	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.7	0.0	2.8				0.0	6.0	3.5	4.4	6.6	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	42.4	0.0	39.1				0.0	13.3	12.0	53.8	16.0	0.0
LnGrp LOS	D		D					B	B	D	B	
Approach Vol, veh/h		612						1839			1247	
Approach Delay, s/veh		41.7						13.0			25.6	
Approach LOS		D						B			C	

Timer - Assigned Phs	1	2		4		6						
Phs Duration (G+Y+Rc), s	15.4	63.0		21.5		78.5						
Change Period (Y+Rc), s	4.0	5.5		5.0		5.5						
Max Green Setting (Gmax), s	14.0	39.5		32.0		57.5						
Max Q Clear Time (g_c+I1), s	11.2	20.2		15.1		16.9						
Green Ext Time (p_c), s	0.2	11.3		1.5		7.1						

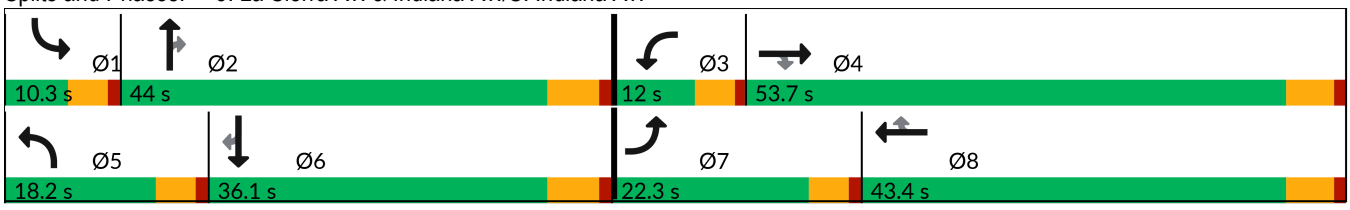
Intersection Summary			
HCM 7th Control Delay, s/veh		22.0	
HCM 7th LOS		C	

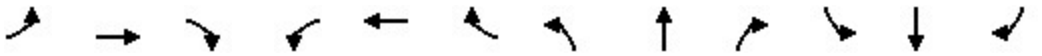
Notes
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	435	200	138	93	210	251	327	1371	147	123	894	437
Future Volume (vph)	435	200	138	93	210	251	327	1371	147	123	894	437
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4			8			2			6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	9.6	43.4	43.4	9.6	43.4	43.4	9.6	37.8	37.8	9.6	32.8	32.8
Total Split (s)	22.3	53.7	53.7	12.0	43.4	43.4	18.2	44.0	44.0	10.3	36.1	36.1
Total Split (%)	18.6%	44.8%	44.8%	10.0%	36.2%	36.2%	15.2%	36.7%	36.7%	8.6%	30.1%	30.1%
Yellow Time (s)	3.6	4.4	4.4	3.6	4.4	4.4	3.6	4.8	4.8	3.6	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.4	5.4	4.6	5.4	5.4	4.6	5.8	5.8	4.6	5.8	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	17.3	26.7	26.7	6.8	16.1	16.1	13.4	38.0	38.0	5.8	30.4	30.4
Actuated g/C Ratio	0.18	0.27	0.27	0.07	0.16	0.16	0.14	0.39	0.39	0.06	0.31	0.31
v/c Ratio	0.79	0.23	0.28	0.43	0.40	0.66	0.76	0.76	0.23	0.67	0.62	0.59
Control Delay (s/veh)	50.3	27.5	5.4	52.2	37.7	19.8	53.9	30.6	5.1	64.2	32.4	6.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	50.3	27.5	5.4	52.2	37.7	19.8	53.9	30.6	5.1	64.2	32.4	6.5
LOS	D	C	A	D	D	B	D	C	A	E	C	A
Approach Delay (s/veh)		36.4			32.0			32.6			27.3	
Approach LOS		D			C			C			C	

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 97.9	
Natural Cycle: 115	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.79	
Intersection Signal Delay (s/veh): 31.5	Intersection LOS: C
Intersection Capacity Utilization 70.6%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 3: La Sierra Av. & Indiana Av./S. Indiana Av.





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	435	200	138	93	210	251	327	1371	147	123	894	437
Future Volume (veh/h)	435	200	138	93	210	251	327	1371	147	123	894	437
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		0.99	1.00		1.00	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	478	220	99	102	231	196	359	1507	98	135	982	255
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	556	1010	450	175	619	275	433	1958	607	201	1615	491
Arrive On Green	0.16	0.28	0.28	0.05	0.17	0.17	0.13	0.38	0.38	0.06	0.32	0.32
Sat Flow, veh/h	3456	3554	1583	3456	3554	1577	3456	5106	1583	3456	5106	1551
Grp Volume(v), veh/h	478	220	99	102	231	196	359	1507	98	135	982	255
Grp Sat Flow(s),veh/h/ln	1728	1777	1583	1728	1777	1577	1728	1702	1583	1728	1702	1551
Q Serve(g_s), s	12.3	4.3	4.4	2.6	5.2	10.7	9.3	23.6	3.7	3.5	14.9	12.3
Cycle Q Clear(g_c), s	12.3	4.3	4.4	2.6	5.2	10.7	9.3	23.6	3.7	3.5	14.9	12.3
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	556	1010	450	175	619	275	433	1958	607	201	1615	491
V/C Ratio(X)	0.86	0.22	0.22	0.58	0.37	0.71	0.83	0.77	0.16	0.67	0.61	0.52
Avail Cap(c_a), veh/h	670	1880	838	280	1479	656	515	2137	662	216	1695	515
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.3	24.9	24.9	42.4	33.3	35.6	39.0	24.6	18.5	42.1	26.4	25.5
Incr Delay (d2), s/veh	8.3	0.1	0.2	1.1	0.4	3.4	8.1	1.8	0.2	5.5	0.7	1.2
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.6	1.8	1.6	1.1	2.2	4.2	4.2	8.9	1.3	1.6	5.7	4.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	45.6	25.0	25.2	43.5	33.7	39.0	47.0	26.4	18.7	47.6	27.1	26.7
LnGrp LOS	D	C	C	D	C	D	D	C	B	D	C	C
Approach Vol, veh/h		797			529			1964			1372	
Approach Delay, s/veh		37.4			37.5			29.8			29.1	
Approach LOS		D			D			C			C	

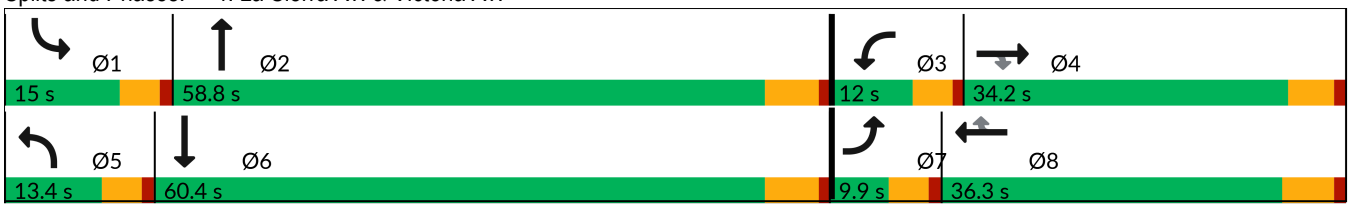
Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	9.9	40.8	9.2	31.3	16.0	34.7	19.3	21.3
Change Period (Y+Rc), s	4.6	5.8	4.6	5.4	4.6	5.8	4.6	5.4
Max Green Setting (Gmax), s	5.7	38.2	7.4	48.3	13.6	30.3	17.7	38.0
Max Q Clear Time (g_c+I1), s	5.5	25.6	4.6	6.4	11.3	16.9	14.3	12.7
Green Ext Time (p_c), s	0.0	9.4	0.0	1.7	0.2	7.7	0.4	2.0

Intersection Summary	
HCM 7th Control Delay, s/veh	31.8
HCM 7th LOS	C

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations										
Traffic Volume (vph)	46	77	56	66	93	202	41	1017	106	518
Future Volume (vph)	46	77	56	66	93	202	41	1017	106	518
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Prot	NA
Protected Phases	7	4		3	8		5	2	1	6
Permitted Phases			4			8				
Detector Phase	7	4	4	3	8	8	5	2	1	6
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	5.0	10.0
Minimum Split (s)	9.6	34.1	34.1	9.6	33.8	33.8	9.6	35.8	9.6	33.8
Total Split (s)	9.9	34.2	34.2	12.0	36.3	36.3	13.4	58.8	15.0	60.4
Total Split (%)	8.3%	28.5%	28.5%	10.0%	30.3%	30.3%	11.2%	49.0%	12.5%	50.3%
Yellow Time (s)	3.6	4.1	4.1	3.6	4.8	4.8	3.6	4.8	3.6	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.1	5.1	4.6	5.8	5.8	4.6	5.8	4.6	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	None	Min
Act Effct Green (s)	5.5	13.9	13.9	7.2	14.8	14.8	7.0	45.8	9.9	53.7
Actuated g/C Ratio	0.06	0.15	0.15	0.08	0.16	0.16	0.07	0.48	0.10	0.57
v/c Ratio	0.26	0.32	0.20	0.56	0.36	0.57	0.35	0.80	0.65	0.31
Control Delay (s/veh)	52.8	42.8	2.5	64.6	42.2	15.3	55.3	26.0	62.6	13.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	52.8	42.8	2.5	64.6	42.2	15.3	55.3	26.0	62.6	13.7
LOS	D	D	A	E	D	B	E	C	E	B
Approach Delay (s/veh)		32.8			31.2			27.0		21.7
Approach LOS		C			C			C		C

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 94.8	
Natural Cycle: 100	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.80	
Intersection Signal Delay (s/veh): 26.6	Intersection LOS: C
Intersection Capacity Utilization 65.9%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 4: La Sierra Av. & Victoria Av.



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	46	77	56	66	93	202	41	1017	185	106	518	26
Future Volume (veh/h)	46	77	56	66	93	202	41	1017	185	106	518	26
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		0.98	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	52	87	24	74	104	142	46	1143	179	119	582	28
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	144	242	202	95	264	218	70	1497	234	150	1833	88
Arrive On Green	0.04	0.13	0.13	0.05	0.14	0.14	0.04	0.49	0.49	0.08	0.53	0.53
Sat Flow, veh/h	3456	1870	1563	1781	1870	1550	1781	3080	481	1781	3451	166
Grp Volume(v), veh/h	52	87	24	74	104	142	46	658	664	119	299	311
Grp Sat Flow(s),veh/h/ln	1728	1870	1563	1781	1870	1550	1781	1777	1784	1781	1777	1841
Q Serve(g_s), s	1.2	3.6	1.1	3.5	4.3	7.3	2.1	25.4	25.7	5.5	8.0	8.0
Cycle Q Clear(g_c), s	1.2	3.6	1.1	3.5	4.3	7.3	2.1	25.4	25.7	5.5	8.0	8.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.27	1.00		0.09
Lane Grp Cap(c), veh/h	144	242	202	95	264	218	70	864	867	150	944	978
V/C Ratio(X)	0.36	0.36	0.12	0.78	0.39	0.65	0.66	0.76	0.77	0.79	0.32	0.32
Avail Cap(c_a), veh/h	217	646	540	157	677	561	186	1118	1123	220	1152	1193
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	39.3	33.5	32.4	39.4	32.9	34.2	39.9	17.7	17.7	37.8	11.1	11.1
Incr Delay (d2), s/veh	0.6	0.9	0.3	5.1	1.0	3.2	3.9	2.8	2.9	6.7	0.3	0.3
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	1.6	0.4	1.6	1.9	2.8	1.0	9.5	9.6	2.6	2.7	2.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	39.8	34.4	32.7	44.4	33.9	37.5	43.8	20.4	20.6	44.5	11.4	11.4
LnGrp LOS	D	C	C	D	C	D	D	C	C	D	B	B
Approach Vol, veh/h		163			320			1368			729	
Approach Delay, s/veh		35.9			37.9			21.3			16.8	
Approach LOS		D			D			C			B	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	11.7	46.7	9.1	16.7	7.9	50.5	8.1	17.7
Change Period (Y+Rc), s	4.6	5.8	4.6	* 5.8	4.6	5.8	4.6	5.8
Max Green Setting (Gmax), s	10.4	53.0	7.4	* 29	8.8	54.6	5.3	30.5
Max Q Clear Time (g_c+I1), s	7.5	27.7	5.5	5.6	4.1	10.0	3.2	9.3
Green Ext Time (p_c), s	0.0	13.3	0.0	0.5	0.0	5.6	0.0	0.9

Intersection Summary	
HCM 7th Control Delay, s/veh	23.0
HCM 7th LOS	C

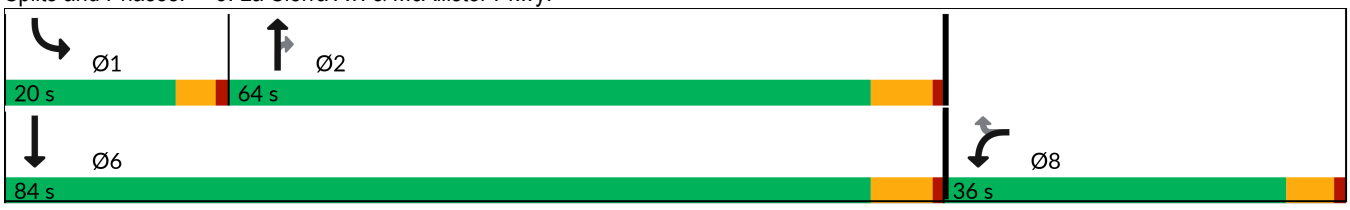
Notes

* HCM 7th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	18	228	1092	22	79	506
Future Volume (vph)	18	228	1092	22	79	506
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	10.0
Minimum Split (s)	33.4	33.4	27.5	27.5	9.6	16.5
Total Split (s)	36.0	36.0	64.0	64.0	20.0	84.0
Total Split (%)	30.0%	30.0%	53.3%	53.3%	16.7%	70.0%
Yellow Time (s)	4.4	4.4	5.5	5.5	3.6	5.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.4	6.5	6.5	4.6	6.5
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	Min	Min	None	Min
Act Effct Green (s)	13.4	13.4	36.1	36.1	9.1	46.9
Actuated g/C Ratio	0.18	0.18	0.49	0.49	0.12	0.64
v/c Ratio	0.06	0.52	0.71	0.03	0.41	0.25
Control Delay (s/veh)	31.5	8.7	18.2	6.6	41.9	5.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	31.5	8.7	18.2	6.6	41.9	5.8
LOS	C	A	B	A	D	A
Approach Delay (s/veh)	10.4		18.0			10.7
Approach LOS	B		B			B

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 73.6	
Natural Cycle: 80	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.71	
Intersection Signal Delay (s/veh): 14.8	Intersection LOS: B
Intersection Capacity Utilization 56.6%	ICU Level of Service B
Analysis Period (min) 15	

Splits and Phases: 5: La Sierra Av. & McAllister Pkwy.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	18	228	1092	22	79	506
Future Volume (veh/h)	18	228	1092	22	79	506
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	20	138	1227	22	89	569
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	266	237	1856	828	115	2347
Arrive On Green	0.15	0.15	0.52	0.52	0.06	0.66
Sat Flow, veh/h	1781	1585	3647	1585	1781	3647
Grp Volume(v), veh/h	20	138	1227	22	89	569
Grp Sat Flow(s),veh/h/ln	1781	1585	1777	1585	1781	1777
Q Serve(g_s), s	0.6	5.1	15.8	0.4	3.1	4.1
Cycle Q Clear(g_c), s	0.6	5.1	15.8	0.4	3.1	4.1
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	266	237	1856	828	115	2347
V/C Ratio(X)	0.08	0.58	0.66	0.03	0.77	0.24
Avail Cap(c_a), veh/h	871	775	3265	1456	438	4400
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	22.9	24.8	10.9	7.2	28.8	4.3
Incr Delay (d2), s/veh	0.1	2.3	0.6	0.0	4.1	0.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	1.9	4.2	0.1	1.3	0.7
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	23.0	27.1	11.5	7.3	32.9	4.4
LnGrp LOS	C	C	B	A	C	A
Approach Vol, veh/h	158		1249			658
Approach Delay, s/veh	26.5		11.4			8.2
Approach LOS	C		B			A

Timer - Assigned Phs	1	2				6	8
Phs Duration (G+Y+Rc), s	8.6	39.2				47.8	14.8
Change Period (Y+Rc), s	4.6	6.5				6.5	5.4
Max Green Setting (Gmax), s	15.4	57.5				77.5	30.6
Max Q Clear Time (g_c+I1), s	5.1	17.8				6.1	7.1
Green Ext Time (p_c), s	0.1	14.9				5.5	0.4

Intersection Summary	
HCM 7th Control Delay, s/veh	11.6
HCM 7th LOS	B

Intersection	
Intersection Delay, s/veh	16.3
Intersection LOS	C

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	58	455	108	20	227	127
Future Vol, veh/h	58	455	108	20	227	127
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	61	479	114	21	239	134
Number of Lanes	1	0	2	0	1	1

Approach	WB	NB	SB
Opposing Approach		SB	NB
Opposing Lanes	0	2	2
Conflicting Approach Left	NB		WB
Conflicting Lanes Left	2	0	1
Conflicting Approach Right	SB	WB	
Conflicting Lanes Right	2	1	0
HCM Control Delay, s/veh	9.8	10.1	13.4
HCM LOS	C	B	B

Lane	NBLn1	NBLn2	WBLn1	SBLn1	SBLn2
Vol Left, %	0%	0%	11%	100%	0%
Vol Thru, %	100%	64%	0%	0%	100%
Vol Right, %	0%	36%	89%	0%	0%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	72	56	513	227	127
LT Vol	0	0	58	227	0
Through Vol	72	36	0	0	127
RT Vol	0	20	455	0	0
Lane Flow Rate	76	59	540	239	134
Geometry Grp	5	5	2	5	5
Degree of Util (X)	0.139	0.104	0.73	0.445	0.23
Departure Headway (Hd)	6.597	6.342	4.868	6.708	6.199
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	542	564	745	538	579
Service Time	4.357	4.101	2.868	4.456	3.946
HCM Lane V/C Ratio	0.14	0.105	0.725	0.444	0.231
HCM Control Delay, s/veh	10.4	9.8	19.8	14.8	10.8
HCM Lane LOS	B	A	C	B	B
HCM 95th-tile Q	0.5	0.3	6.4	2.3	0.9

Intersection	
Intersection Delay, s/veh	10.7
Intersection LOS	B

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	49	217	439	88	47	70
Future Vol, veh/h	49	217	439	88	47	70
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	51	226	457	92	49	73
Number of Lanes	1	1	2	0	1	0

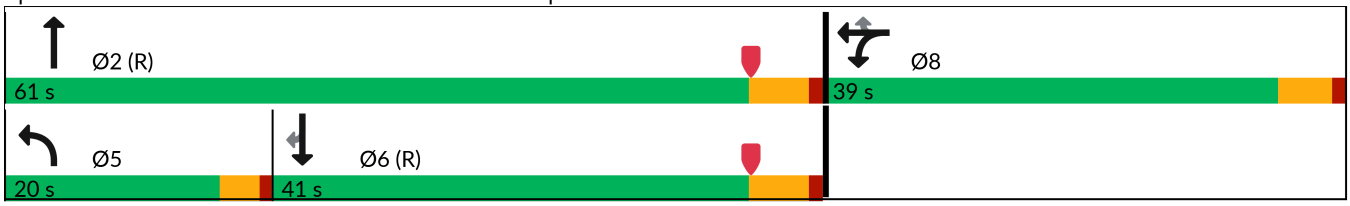
Approach	EB	WB	SB
Opposing Approach	WB	EB	
Opposing Lanes	2	2	0
Conflicting Approach Left	SB		WB
Conflicting Lanes Left	1	0	2
Conflicting Approach Right		SB	EB
Conflicting Lanes Right	0	1	2
HCM Control Delay, s/veh	10.6	11	9.6
HCM LOS	B	B	A

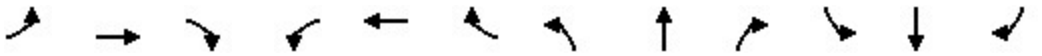
Lane	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1
Vol Left, %	100%	0%	0%	0%	40%
Vol Thru, %	0%	100%	100%	62%	0%
Vol Right, %	0%	0%	0%	38%	60%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	49	217	293	234	117
LT Vol	49	0	0	0	47
Through Vol	0	217	293	146	0
RT Vol	0	0	0	88	70
Lane Flow Rate	51	226	305	244	122
Geometry Grp	5	5	5	5	2
Degree of Util (X)	0.084	0.338	0.435	0.33	0.182
Departure Headway (Hd)	5.891	5.387	5.131	4.867	5.371
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	605	665	698	735	663
Service Time	3.658	3.154	2.887	2.622	3.44
HCM Lane V/C Ratio	0.084	0.34	0.437	0.332	0.184
HCM Control Delay, s/veh	9.2	10.9	11.8	10	9.6
HCM Lane LOS	A	B	B	A	A
HCM 95th-tile Q	0.3	1.5	2.2	1.4	0.7

Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Configurations							
Traffic Volume (vph)	458	2	523	372	898	1354	267
Future Volume (vph)	458	2	523	372	898	1354	267
Turn Type	Split	NA	Perm	Prot	NA	NA	Perm
Protected Phases	8	8		5	2	6	
Permitted Phases			8				6
Detector Phase	8	8	8	5	2	6	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	9.0	26.5	29.5	29.5
Total Split (s)	39.0	39.0	39.0	20.0	61.0	41.0	41.0
Total Split (%)	39.0%	39.0%	39.0%	20.0%	61.0%	41.0%	41.0%
Yellow Time (s)	4.0	4.0	4.0	3.0	4.5	4.5	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	4.0	5.5	5.5	5.5
Lead/Lag				Lead		Lag	Lag
Lead-Lag Optimize?				Yes		Yes	Yes
Recall Mode	None	None	None	None	C-Min	C-Min	C-Min
Act Effct Green (s)	26.8	26.8	26.8	14.8	62.7	44.0	44.0
Actuated g/C Ratio	0.27	0.27	0.27	0.15	0.63	0.44	0.44
v/c Ratio	0.79	0.70	0.65	0.76	0.29	0.62	0.34
Control Delay (s/veh)	46.2	30.3	25.5	38.5	21.5	24.6	8.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	46.2	30.3	25.5	38.5	21.5	24.6	8.1
LOS	D	C	C	D	C	C	A
Approach Delay (s/veh)		34.3			26.5	21.9	
Approach LOS		C			C	C	

Intersection Summary	
Cycle Length: 100	
Actuated Cycle Length: 100	
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow, Master Intersection	
Natural Cycle: 60	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.79	
Intersection Signal Delay (s/veh): 26.5	Intersection LOS: C
Intersection Capacity Utilization 68.9%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 1: La Sierra Av. & SR-91 WB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	458	2	523	372	898	0	0	1354	267
Future Volume (veh/h)	0	0	0	458	2	523	372	898	0	0	1354	267
Initial Q (Qb), veh				0	0	0	0	0	0	0	0	0
Lane Width Adj.				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach					No			No			No	
Adj Sat Flow, veh/h/ln				1870	1900	1900	1856	1885	0	0	1885	1900
Adj Flow Rate, veh/h				574	0	218	384	926	0	0	1396	189
Peak Hour Factor				0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %				2	0	0	3	1	0	0	1	0
Cap, veh/h				692	0	313	444	3607	0	0	2734	842
Arrive On Green				0.19	0.00	0.19	0.26	1.00	0.00	0.00	0.53	0.53
Sat Flow, veh/h				3563	0	1610	3428	5316	0	0	5316	1585
Grp Volume(v), veh/h				574	0	218	384	926	0	0	1396	189
Grp Sat Flow(s),veh/h/ln				1781	0	1610	1714	1716	0	0	1716	1585
Q Serve(g_s), s				15.5	0.0	12.6	10.7	0.0	0.0	0.0	17.5	6.3
Cycle Q Clear(g_c), s				15.5	0.0	12.6	10.7	0.0	0.0	0.0	17.5	6.3
Prop In Lane				1.00		1.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h				692	0	313	444	3607	0	0	2734	842
V/C Ratio(X)				0.83	0.00	0.70	0.86	0.26	0.00	0.00	0.51	0.22
Avail Cap(c_a), veh/h				1211	0	547	549	3607	0	0	2734	842
HCM Platoon Ratio				1.00	1.00	1.00	2.00	2.00	1.00	1.00	1.00	1.00
Upstream Filter(I)				1.00	0.00	1.00	0.81	0.81	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh				38.7	0.0	37.6	36.2	0.0	0.0	0.0	15.1	12.5
Incr Delay (d2), s/veh				2.0	0.0	2.1	8.2	0.1	0.0	0.0	0.7	0.6
Initial Q Delay(d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln				6.6	0.0	4.9	4.2	0.0	0.0	0.0	6.2	2.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh				40.7	0.0	39.6	44.4	0.1	0.0	0.0	15.8	13.1
LnGrp LOS				D		D	D	A			B	B
Approach Vol, veh/h					792			1310			1585	
Approach Delay, s/veh					40.4			13.1			15.4	
Approach LOS					D			B			B	

Timer - Assigned Phs	2		5	6		8						
Phs Duration (G+Y+Rc), s	75.6		17.0	58.6		24.4						
Change Period (Y+Rc), s	5.5		4.0	5.5		5.0						
Max Green Setting (Gmax), s	55.5		16.0	35.5		34.0						
Max Q Clear Time (g_c+I1), s	2.0		12.7	19.5		17.5						
Green Ext Time (p_c), s	7.1		0.3	8.9		1.9						

Intersection Summary	
HCM 7th Control Delay, s/veh	20.0
HCM 7th LOS	B

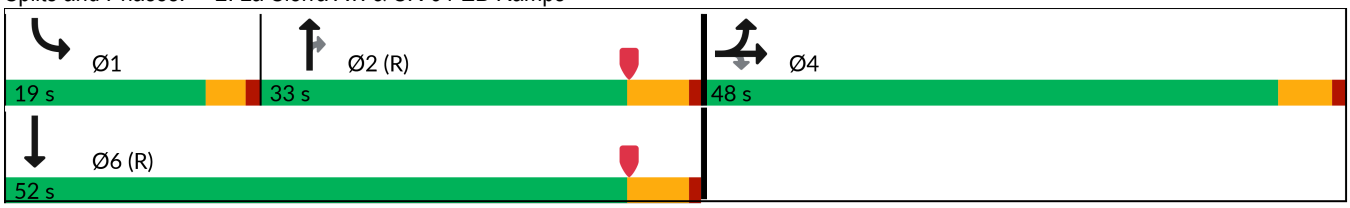
Notes

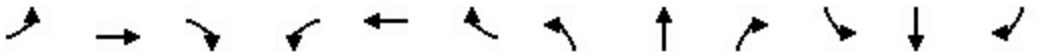
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	200	10	796	1058	289	405	1411
Future Volume (vph)	200	10	796	1058	289	405	1411
Turn Type	Split	NA	Perm	NA	Perm	Prot	NA
Protected Phases	4	4		2		1	6
Permitted Phases			4		2		
Detector Phase	4	4	4	2	2	1	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	29.5	29.5	9.0	26.5
Total Split (s)	48.0	48.0	48.0	33.0	33.0	19.0	52.0
Total Split (%)	48.0%	48.0%	48.0%	33.0%	33.0%	19.0%	52.0%
Yellow Time (s)	4.0	4.0	4.0	4.5	4.5	3.0	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.5	5.5	4.0	5.5
Lead/Lag				Lag	Lag	Lead	
Lead-Lag Optimize?				Yes	Yes	Yes	
Recall Mode	None	None	None	C-Min	C-Min	None	C-Min
Act Effct Green (s)	34.5	34.5	34.5	35.4	35.4	15.6	55.0
Actuated g/C Ratio	0.35	0.35	0.35	0.35	0.35	0.16	0.55
v/c Ratio	0.32	0.83	0.75	0.60	0.40	0.77	0.51
Control Delay (s/veh)	24.1	42.6	32.0	29.9	5.2	30.0	9.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.3
Total Delay (s/veh)	24.1	42.6	32.0	29.9	5.2	30.0	10.0
LOS	C	D	C	C	A	C	A
Approach Delay (s/veh)		34.9		24.6			14.4
Approach LOS		C		C			B

Intersection Summary	
Cycle Length: 100	
Actuated Cycle Length: 100	
Offset: 28 (28%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow	
Natural Cycle: 70	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.83	
Intersection Signal Delay (s/veh): 22.7	Intersection LOS: C
Intersection Capacity Utilization 68.9%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 2: La Sierra Av. & SR-91 EB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	200	10	796	0	0	0	0	1058	289	405	1411	0
Future Volume (veh/h)	200	10	796	0	0	0	0	1058	289	405	1411	0
Initial Q (Qb), veh	0	0	0				0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		0.98	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No						No			No	
Adj Sat Flow, veh/h/ln	1870	1752	1856				0	1870	1885	1870	1885	0
Adj Flow Rate, veh/h	139	0	652				0	1080	157	413	1440	0
Peak Hour Factor	0.98	0.98	0.98				0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	2	10	3				0	2	1	2	1	0
Cap, veh/h	430	0	759				0	2418	745	485	3365	0
Arrive On Green	0.24	0.00	0.24				0.00	0.47	0.47	0.05	0.22	0.00
Sat Flow, veh/h	1781	0	3145				0	5274	1572	3456	5316	0
Grp Volume(v), veh/h	139	0	652				0	1080	157	413	1440	0
Grp Sat Flow(s),veh/h/ln	1781	0	1572				0	1702	1572	1728	1716	0
Q Serve(g_s), s	6.4	0.0	19.8				0.0	14.1	5.8	11.9	24.2	0.0
Cycle Q Clear(g_c), s	6.4	0.0	19.8				0.0	14.1	5.8	11.9	24.2	0.0
Prop In Lane	1.00		1.00				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	430	0	759				0	2418	745	485	3365	0
V/C Ratio(X)	0.32	0.00	0.86				0.00	0.45	0.21	0.85	0.43	0.00
Avail Cap(c_a), veh/h	766	0	1352				0	2418	745	518	3365	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	0.33	0.33	1.00
Upstream Filter(I)	1.00	0.00	1.00				0.00	0.78	0.78	0.71	0.71	0.00
Uniform Delay (d), s/veh	31.2	0.0	36.3				0.0	17.6	15.4	46.7	23.1	0.0
Incr Delay (d2), s/veh	0.3	0.0	2.2				0.0	0.5	0.5	8.8	0.3	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.7	0.0	7.4				0.0	5.2	2.0	6.0	11.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	31.5	0.0	38.6				0.0	18.0	15.9	55.5	23.3	0.0
LnGrp LOS	C		D					B	B	E	C	
Approach Vol, veh/h		791						1237			1853	
Approach Delay, s/veh		37.3						17.8			30.5	
Approach LOS		D						B			C	

Timer - Assigned Phs	1	2		4		6						
Phs Duration (G+Y+Rc), s	18.0	52.9		29.1		70.9						
Change Period (Y+Rc), s	4.0	5.5		5.0		5.5						
Max Green Setting (Gmax), s	15.0	27.5		43.0		46.5						
Max Q Clear Time (g_c+I1), s	13.9	16.1		21.8		26.2						
Green Ext Time (p_c), s	0.2	5.6		2.3		9.8						

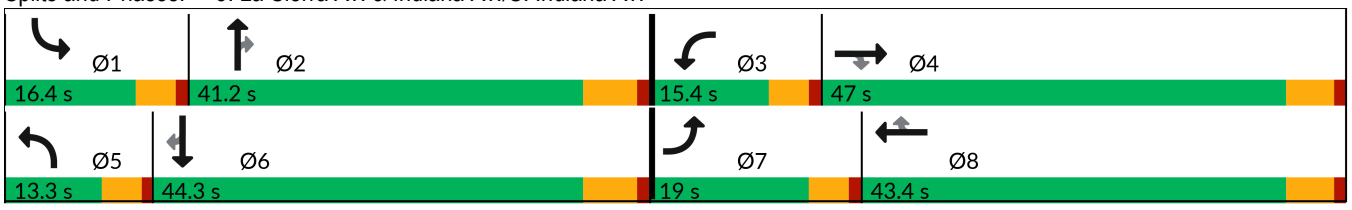
Intersection Summary	
HCM 7th Control Delay, s/veh	27.8
HCM 7th LOS	C

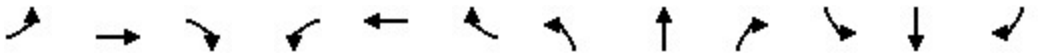
Notes
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	369	343	366	152	189	243	214	797	109	282	1445	373
Future Volume (vph)	369	343	366	152	189	243	214	797	109	282	1445	373
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4			8			2			6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	9.6	43.4	43.4	9.6	43.4	43.4	9.6	37.8	37.8	9.6	32.8	32.8
Total Split (s)	19.0	47.0	47.0	15.4	43.4	43.4	13.3	41.2	41.2	16.4	44.3	44.3
Total Split (%)	15.8%	39.2%	39.2%	12.8%	36.2%	36.2%	11.1%	34.3%	34.3%	13.7%	36.9%	36.9%
Yellow Time (s)	3.6	4.4	4.4	3.6	4.4	4.4	3.6	4.8	4.8	3.6	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.4	5.4	4.6	5.4	5.4	4.6	5.8	5.8	4.6	5.8	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	14.5	24.6	24.6	8.9	18.9	18.9	8.8	36.0	36.0	11.7	38.9	38.9
Actuated g/C Ratio	0.14	0.24	0.24	0.09	0.19	0.19	0.09	0.35	0.35	0.12	0.38	0.38
v/c Ratio	0.82	0.44	0.77	0.55	0.31	0.60	0.79	0.48	0.19	0.78	0.81	0.48
Control Delay (s/veh)	58.3	33.8	28.1	53.4	35.7	16.9	66.5	28.2	6.2	59.9	33.6	4.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	58.3	33.8	28.1	53.4	35.7	16.9	66.5	28.2	6.2	59.9	33.6	4.9
LOS	E	C	C	D	D	B	E	C	A	E	C	A
Approach Delay (s/veh)		40.3			32.5			33.4			32.0	
Approach LOS		D			C			C			C	

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 101.7	
Natural Cycle: 115	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.82	
Intersection Signal Delay (s/veh): 34.2	Intersection LOS: C
Intersection Capacity Utilization 72.1%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 3: La Sierra Av. & Indiana Av./S. Indiana Av.





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	369	343	366	152	189	243	214	797	109	282	1445	373
Future Volume (veh/h)	369	343	366	152	189	243	214	797	109	282	1445	373
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		0.98	1.00		0.99	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	401	373	312	165	205	205	233	866	64	307	1571	307
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	467	937	408	233	696	305	298	1807	558	374	1918	582
Arrive On Green	0.14	0.26	0.26	0.07	0.20	0.20	0.09	0.35	0.35	0.11	0.38	0.38
Sat Flow, veh/h	3456	3554	1549	3456	3554	1557	3456	5106	1577	3456	5106	1548
Grp Volume(v), veh/h	401	373	312	165	205	205	233	866	64	307	1571	307
Grp Sat Flow(s),veh/h/ln	1728	1777	1549	1728	1777	1557	1728	1702	1577	1728	1702	1548
Q Serve(g_s), s	11.2	8.5	18.3	4.6	4.9	12.0	6.5	13.0	2.7	8.6	27.3	15.2
Cycle Q Clear(g_c), s	11.2	8.5	18.3	4.6	4.9	12.0	6.5	13.0	2.7	8.6	27.3	15.2
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	467	937	408	233	696	305	298	1807	558	374	1918	582
V/C Ratio(X)	0.86	0.40	0.76	0.71	0.29	0.67	0.78	0.48	0.11	0.82	0.82	0.53
Avail Cap(c_a), veh/h	505	1500	654	379	1371	601	305	1835	567	414	1995	605
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	41.7	29.8	33.4	45.0	33.8	36.7	44.1	24.8	21.4	43.0	27.7	24.0
Incr Delay (d2), s/veh	12.2	0.3	3.0	1.5	0.2	2.6	11.0	0.3	0.1	10.3	2.9	1.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.4	3.5	6.9	2.0	2.1	4.6	3.1	4.9	1.0	4.0	10.7	5.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	53.9	30.1	36.5	46.5	34.0	39.3	55.1	25.0	21.6	53.3	30.6	25.0
LnGrp LOS	D	C	D	D	C	D	E	C	C	D	C	C
Approach Vol, veh/h		1086			575			1163			2185	
Approach Delay, s/veh		40.7			39.5			30.9			33.0	
Approach LOS		D			D			C			C	

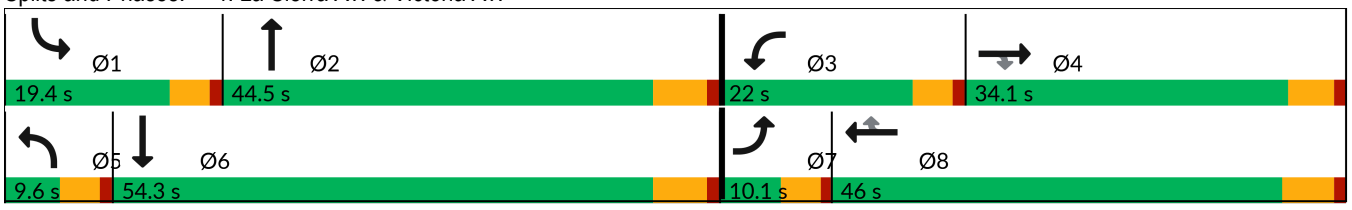
Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	15.3	40.7	11.2	31.4	13.1	42.8	17.9	24.7
Change Period (Y+Rc), s	4.6	5.8	4.6	5.4	4.6	5.8	4.6	5.4
Max Green Setting (Gmax), s	11.8	35.4	10.8	41.6	8.7	38.5	14.4	38.0
Max Q Clear Time (g_c+I1), s	10.6	15.0	6.6	20.3	8.5	29.3	13.2	14.0
Green Ext Time (p_c), s	0.1	7.8	0.1	3.4	0.0	7.7	0.1	1.9

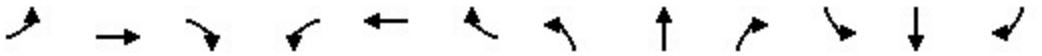
Intersection Summary	
HCM 7th Control Delay, s/veh	34.9
HCM 7th LOS	C

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations										
Traffic Volume (vph)	58	104	147	260	75	172	27	703	189	1381
Future Volume (vph)	58	104	147	260	75	172	27	703	189	1381
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Prot	NA
Protected Phases	7	4		3	8		5	2	1	6
Permitted Phases			4			8				
Detector Phase	7	4	4	3	8	8	5	2	1	6
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	5.0	10.0
Minimum Split (s)	9.6	34.1	34.1	9.6	33.8	33.8	9.6	35.8	9.6	33.8
Total Split (s)	10.1	34.1	34.1	22.0	46.0	46.0	9.6	44.5	19.4	54.3
Total Split (%)	8.4%	28.4%	28.4%	18.3%	38.3%	38.3%	8.0%	37.1%	16.2%	45.3%
Yellow Time (s)	3.6	4.1	4.1	3.6	4.8	4.8	3.6	4.8	3.6	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.1	5.1	4.6	5.8	5.8	4.6	5.8	4.6	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	None	Min
Act Effct Green (s)	5.4	14.2	14.2	17.6	27.8	27.8	5.1	35.3	14.4	49.0
Actuated g/C Ratio	0.05	0.14	0.14	0.17	0.27	0.27	0.05	0.35	0.14	0.48
v/c Ratio	0.34	0.43	0.41	0.91	0.16	0.33	0.33	0.73	0.80	0.91
Control Delay (s/veh)	55.1	45.5	5.7	76.8	30.9	6.1	61.4	33.3	68.1	35.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	55.1	45.5	5.7	76.8	30.9	6.1	61.4	33.3	68.1	35.0
LOS	E	D	A	E	C	A	E	C	E	D
Approach Delay (s/veh)		28.4			46.0			34.2		38.8
Approach LOS		C			D			C		D

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 101.9	
Natural Cycle: 120	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.91	
Intersection Signal Delay (s/veh): 37.8	Intersection LOS: D
Intersection Capacity Utilization 78.9%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 4: La Sierra Av. & Victoria Av.





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	58	104	147	260	75	172	27	703	125	189	1381	63
Future Volume (veh/h)	58	104	147	260	75	172	27	703	125	189	1381	63
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		0.99	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	62	111	117	277	80	112	29	748	93	201	1469	45
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	145	195	162	309	440	368	50	1193	148	234	1684	51
Arrive On Green	0.04	0.10	0.10	0.17	0.24	0.24	0.03	0.38	0.38	0.13	0.48	0.48
Sat Flow, veh/h	3456	1870	1559	1781	1870	1563	1781	3181	395	1781	3520	108
Grp Volume(v), veh/h	62	111	117	277	80	112	29	418	423	201	740	774
Grp Sat Flow(s),veh/h/ln	1728	1870	1559	1781	1870	1563	1781	1777	1799	1781	1777	1851
Q Serve(g_s), s	1.7	5.4	7.0	14.6	3.3	5.7	1.5	18.5	18.5	10.6	35.8	36.0
Cycle Q Clear(g_c), s	1.7	5.4	7.0	14.6	3.3	5.7	1.5	18.5	18.5	10.6	35.8	36.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.22	1.00		0.06
Lane Grp Cap(c), veh/h	145	195	162	309	440	368	50	666	675	234	850	885
V/C Ratio(X)	0.43	0.57	0.72	0.90	0.18	0.30	0.58	0.63	0.63	0.86	0.87	0.87
Avail Cap(c_a), veh/h	198	564	470	322	782	654	93	715	724	274	896	934
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	44.9	41.0	41.7	38.9	29.4	30.3	46.2	24.5	24.6	40.9	22.4	22.5
Incr Delay (d2), s/veh	0.7	2.6	5.9	24.6	0.2	0.5	3.9	1.9	1.9	18.5	9.4	9.2
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.7	2.6	2.8	8.2	1.4	2.1	0.7	7.5	7.6	5.7	15.4	16.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	45.7	43.6	47.6	63.5	29.6	30.7	50.1	26.5	26.4	59.4	31.8	31.7
LnGrp LOS	D	D	D	E	C	C	D	C	C	E	C	C
Approach Vol, veh/h		290			469			870			1715	
Approach Delay, s/veh		45.7			49.9			27.2			35.0	
Approach LOS		D			D			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	17.2	41.9	21.3	15.8	7.3	51.8	8.6	28.4
Change Period (Y+Rc), s	4.6	5.8	4.6	* 5.8	4.6	5.8	4.6	5.8
Max Green Setting (Gmax), s	14.8	38.7	17.4	* 29	5.0	48.5	5.5	40.2
Max Q Clear Time (g_c+I1), s	12.6	20.5	16.6	9.0	3.5	38.0	3.7	7.7
Green Ext Time (p_c), s	0.1	6.5	0.0	0.9	0.0	8.0	0.0	0.7

Intersection Summary	
HCM 7th Control Delay, s/veh	36.0
HCM 7th LOS	D

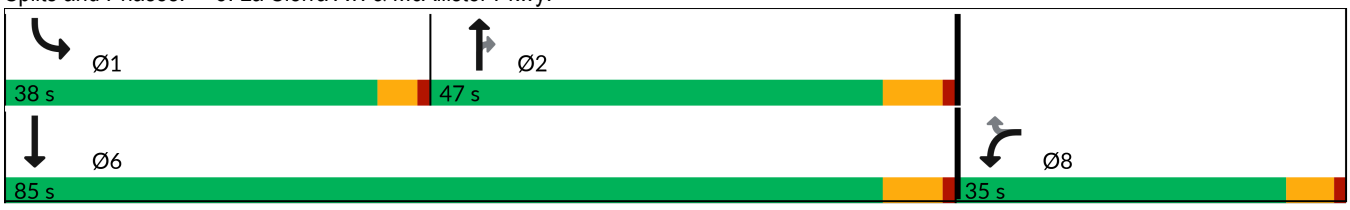
Notes

* HCM 7th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	8	150	706	12	271	1487
Future Volume (vph)	8	150	706	12	271	1487
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	10.0
Minimum Split (s)	33.4	33.4	27.5	27.5	9.6	16.5
Total Split (s)	35.0	35.0	47.0	47.0	38.0	85.0
Total Split (%)	29.2%	29.2%	39.2%	39.2%	31.7%	70.8%
Yellow Time (s)	4.4	4.4	5.5	5.5	3.6	5.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.4	6.5	6.5	4.6	6.5
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	Min	Min	None	Min
Act Effct Green (s)	13.0	13.0	24.3	24.3	17.4	46.7
Actuated g/C Ratio	0.18	0.18	0.34	0.34	0.24	0.64
v/c Ratio	0.03	0.38	0.63	0.02	0.68	0.69
Control Delay (s/veh)	29.8	8.6	24.2	10.9	35.7	10.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	29.8	8.6	24.2	10.9	35.7	10.4
LOS	C	A	C	B	D	B
Approach Delay (s/veh)	9.8		23.9			14.3
Approach LOS	A		C			B

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 72.5	
Natural Cycle: 80	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.69	
Intersection Signal Delay (s/veh): 16.7	Intersection LOS: B
Intersection Capacity Utilization 59.4%	ICU Level of Service B
Analysis Period (min) 15	

Splits and Phases: 5: La Sierra Av. & McAllister Pkwy.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	8	150	706	12	271	1487
Future Volume (veh/h)	8	150	706	12	271	1487
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		0.98	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	9	54	751	13	288	1582
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	191	170	1531	668	343	2482
Arrive On Green	0.11	0.11	0.43	0.43	0.19	0.70
Sat Flow, veh/h	1781	1585	3647	1552	1781	3647
Grp Volume(v), veh/h	9	54	751	13	288	1582
Grp Sat Flow(s),veh/h/ln	1781	1585	1777	1552	1781	1777
Q Serve(g_s), s	0.3	1.9	9.3	0.3	9.5	14.8
Cycle Q Clear(g_c), s	0.3	1.9	9.3	0.3	9.5	14.8
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	191	170	1531	668	343	2482
V/C Ratio(X)	0.05	0.32	0.49	0.02	0.84	0.64
Avail Cap(c_a), veh/h	861	766	2350	1026	971	4554
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	24.5	25.3	12.6	10.0	23.8	5.0
Incr Delay (d2), s/veh	0.1	1.1	0.3	0.0	2.1	0.4
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.1	0.7	2.8	0.1	3.5	1.7
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	24.6	26.3	12.9	10.0	26.0	5.4
LnGrp LOS	C	C	B	B	C	A
Approach Vol, veh/h	63		764			1870
Approach Delay, s/veh	26.1		12.9			8.6
Approach LOS	C		B			A

Timer - Assigned Phs	1	2				6	8
Phs Duration (G+Y+Rc), s	16.4	32.9				49.3	12.0
Change Period (Y+Rc), s	4.6	6.5				6.5	5.4
Max Green Setting (Gmax), s	33.4	40.5				78.5	29.6
Max Q Clear Time (g_c+I1), s	11.5	11.3				16.8	3.9
Green Ext Time (p_c), s	0.3	7.1				26.0	0.1

Intersection Summary	
HCM 7th Control Delay, s/veh	10.2
HCM 7th LOS	B

Intersection	
Intersection Delay, s/veh	75.3
Intersection LOS	F

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	72	303	221	83	616	474
Future Vol, veh/h	72	303	221	83	616	474
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	77	326	238	89	662	510
Number of Lanes	1	0	2	0	1	1

Approach	WB	NB	SB
Opposing Approach		SB	NB
Opposing Lanes	0	2	2
Conflicting Approach Left	NB		WB
Conflicting Lanes Left	2	0	1
Conflicting Approach Right	SB	WB	
Conflicting Lanes Right	2	1	0
HCM Control Delay, s/veh	20.4	13.4	111.4
HCM LOS	C	B	F

Lane	NBLn1	NBLn2	WBLn1	SBLn1	SBLn2
Vol Left, %	0%	0%	19%	100%	0%
Vol Thru, %	100%	47%	0%	0%	100%
Vol Right, %	0%	53%	81%	0%	0%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	147	157	375	616	474
LT Vol	0	0	72	616	0
Through Vol	147	74	0	0	474
RT Vol	0	83	303	0	0
Lane Flow Rate	158	168	403	662	510
Geometry Grp	5	5	2	5	5
Degree of Util (X)	0.319	0.322	0.664	1.281	0.914
Departure Headway (Hd)	7.538	7.157	6.128	6.964	6.453
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	480	505	593	523	566
Service Time	5.238	4.857	4.128	4.679	4.168
HCM Lane V/C Ratio	0.329	0.333	0.68	1.266	0.901
HCM Control Delay, s/veh	13.7	13.2	20.4	162.5	44.9
HCM Lane LOS	B	B	C	F	E
HCM 95th-tile Q	1.4	1.4	4.9	27.2	11.1

Intersection	
Intersection Delay, s/veh	39.4
Intersection LOS	E

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	60	640	283	28	32	105
Future Vol, veh/h	60	640	283	28	32	105
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	64	681	301	30	34	112
Number of Lanes	1	1	2	0	1	0

Approach	EB	WB	SB
Opposing Approach	WB	EB	
Opposing Lanes	2	2	0
Conflicting Approach Left	SB		WB
Conflicting Lanes Left	1	0	2
Conflicting Approach Right		SB	EB
Conflicting Lanes Right	0	1	2
HCM Control Delay, s/veh	57.8	10.6	10.9
HCM LOS	F	B	B

Lane	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1
Vol Left, %	100%	0%	0%	0%	23%
Vol Thru, %	0%	100%	100%	77%	0%
Vol Right, %	0%	0%	0%	23%	77%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	60	640	189	122	137
LT Vol	60	0	0	0	32
Through Vol	0	640	189	94	0
RT Vol	0	0	0	28	105
Lane Flow Rate	64	681	201	130	146
Geometry Grp	5	5	5	5	2
Degree of Util (X)	0.105	1.019	0.317	0.2	0.238
Departure Headway (Hd)	5.894	5.389	5.827	5.664	6.041
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	611	681	620	638	598
Service Time	3.594	3.089	3.527	3.364	4.041
HCM Lane V/C Ratio	0.105	1	0.324	0.204	0.244
HCM Control Delay, s/veh	9.3	62.3	11.2	9.8	10.9
HCM Lane LOS	A	F	B	A	B
HCM 95th-tile Q	0.4	16.8	1.4	0.7	0.9

APPENDIX 3.3: EXISTING (2024) CONDITIONS TRAFFIC SIGNAL WARRANT ANALYSIS WORKSHEETS

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Figure 4C-4. Warrant 3, Peak Hour (70% Factor)

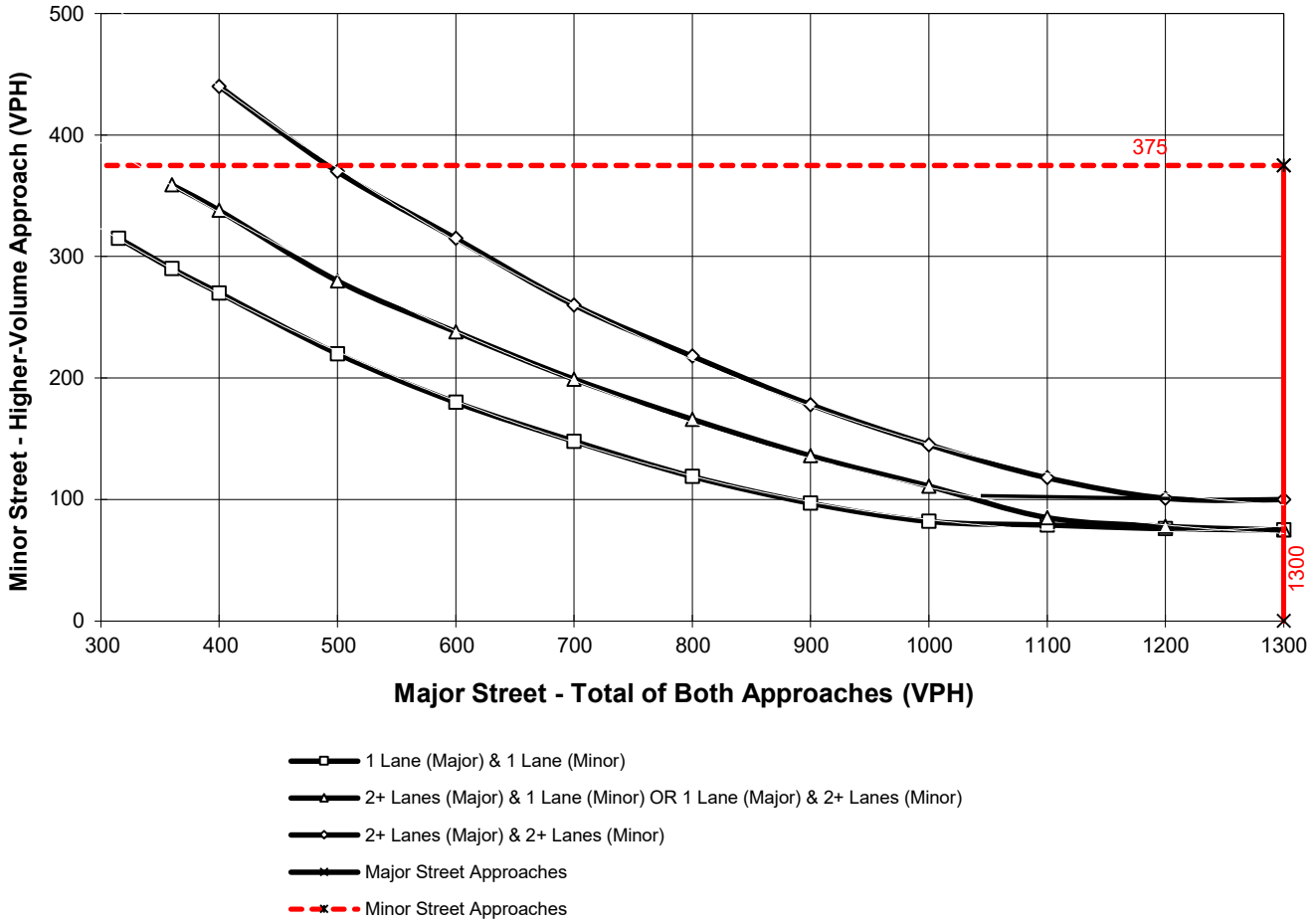
(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 64 km/h OR ABOVE 40 mph ON MAJOR STREET)

Traffic Conditions = **Existing (2024) Conditions - Weekday PM Peak Hour**

Major Street Name = **La Sierra Av.** Total of Both Approaches (VPH) = **1394**
Number of Approach Lanes Major Street = **1**

Minor Street Name = **El Sobrante Rd.** High Volume Approach (VPH) = **375**
Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



*Note: 100 vph applies as the lower threshold for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold for a minor-street approach with one lane

Figure 4C-4. Warrant 3, Peak Hour (70% Factor)

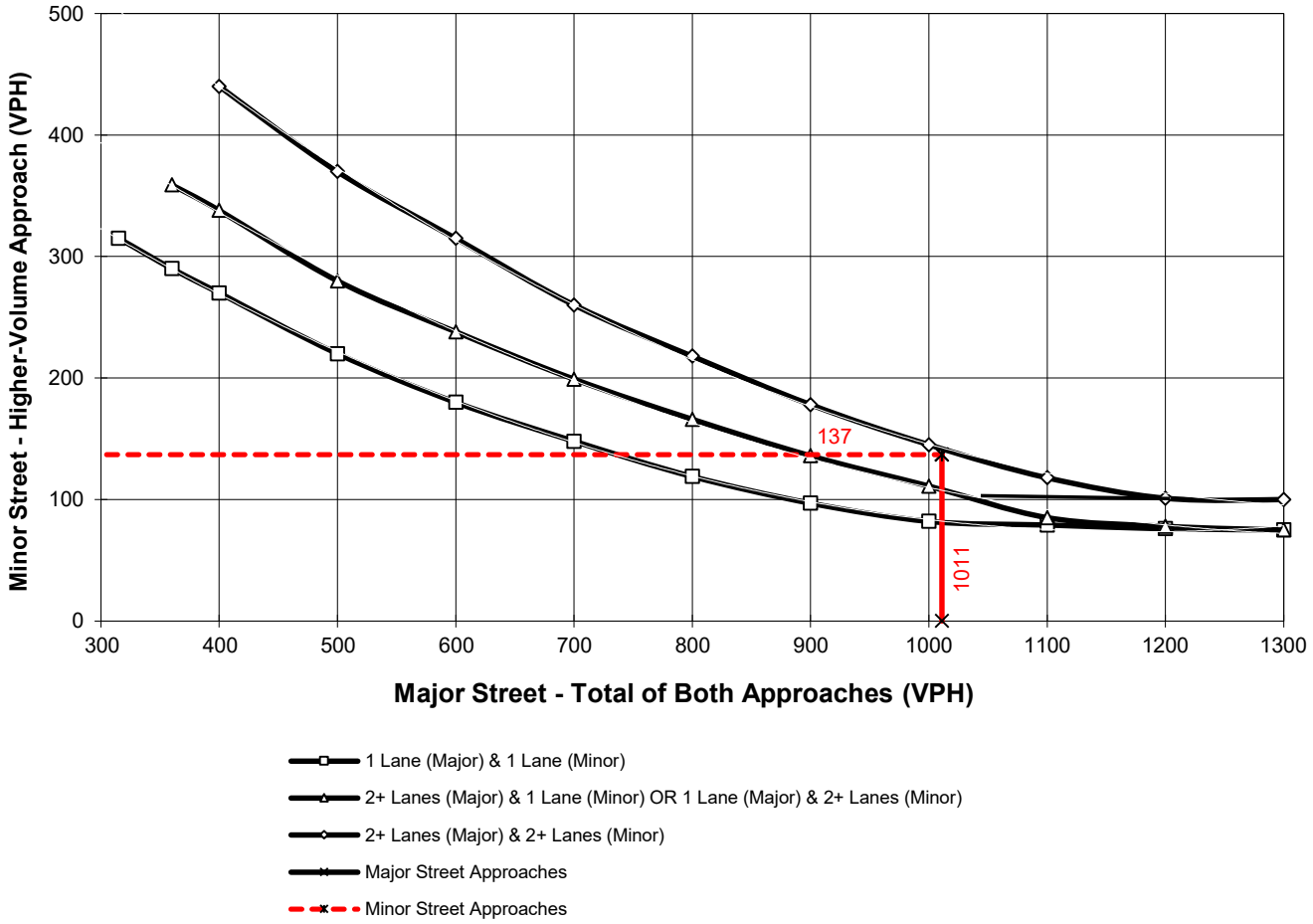
(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 64 km/h OR ABOVE 40 mph ON MAJOR STREET)

Traffic Conditions = **Existing (2024) Conditions - Weekday PM Peak Hour**

Major Street Name = **El Sobrante Rd.** Total of Both Approaches (VPH) = **1011**
Number of Approach Lanes Major Street = **1**

Minor Street Name = **McAllister St.** High Volume Approach (VPH) = **137**
Number of Approach Lanes Minor Street = **1**

WARRANTED FOR A SIGNAL



*Note: 100 vph applies as the lower threshold for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold for a minor-street approach with one lane

APPENDIX 3.4: EXISTING (2024) CONDITIONS QUEUING ANALYSIS WORKSHEETS

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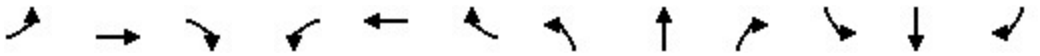
Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	256	251	252	630	1343	953	137
v/c Ratio	0.73	0.68	0.67	0.82	0.39	0.45	0.19
Control Delay (s/veh)	48.0	34.5	33.5	42.0	13.5	23.8	6.2
Queue Delay	0.0	0.0	0.0	0.0	0.3	0.0	0.0
Total Delay (s/veh)	48.0	34.5	33.5	42.0	13.8	23.8	6.2
Queue Length 50th (ft)	161	116	111	221	165	156	3
Queue Length 95th (ft)	226	187	180	271	313	245	49
Internal Link Dist (ft)		1180			412	935	
Turn Bay Length (ft)	500		500	155			140
Base Capacity (vph)	532	519	534	904	3473	2103	704
Starvation Cap Reductn	0	0	0	0	1339	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.48	0.48	0.47	0.70	0.63	0.45	0.19

Intersection Summary



Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	295	283	266	1517	474	315	932
v/c Ratio	0.75	0.72	0.62	0.62	0.48	0.70	0.28
Control Delay (s/veh)	46.7	37.2	23.2	21.5	3.7	48.8	1.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	46.7	37.2	23.2	21.5	3.7	48.8	1.8
Queue Length 50th (ft)	185	145	83	252	0	30	2
Queue Length 95th (ft)	252	218	153	359	61	120	29
Internal Link Dist (ft)		1416		886			412
Turn Bay Length (ft)			450		415	155	
Base Capacity (vph)	543	520	547	2463	997	483	3333
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.54	0.54	0.49	0.62	0.48	0.65	0.28

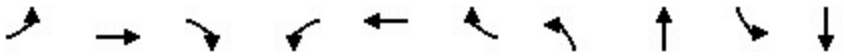
Intersection Summary



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	478	220	152	102	231	276	359	1507	162	135	982	480
v/c Ratio	0.79	0.23	0.28	0.43	0.40	0.66	0.76	0.76	0.23	0.67	0.62	0.59
Control Delay (s/veh)	50.3	27.5	5.4	52.2	37.7	19.8	53.9	30.6	5.1	64.2	32.4	6.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	50.3	27.5	5.4	52.2	37.7	19.8	53.9	30.6	5.1	64.2	32.4	6.5
Queue Length 50th (ft)	138	56	0	30	68	46	106	268	0	41	176	0
Queue Length 95th (ft)	#294	84	41	70	101	122	#234	#508	49	#111	312	91
Internal Link Dist (ft)		2549			2428			2703			886	
Turn Bay Length (ft)	205		205	205		130	200		305	355		535
Base Capacity (vph)	628	1768	857	262	1391	730	483	2009	715	202	1594	813
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.76	0.12	0.18	0.39	0.17	0.38	0.74	0.75	0.23	0.67	0.62	0.59

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	52	87	63	74	104	227	46	1351	119	611
v/c Ratio	0.26	0.32	0.20	0.56	0.36	0.57	0.35	0.80	0.65	0.31
Control Delay (s/veh)	52.8	42.8	2.5	64.6	42.2	15.3	55.3	26.0	62.6	13.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	52.8	42.8	2.5	64.6	42.2	15.3	55.3	26.0	62.6	13.7
Queue Length 50th (ft)	15	51	0	45	61	22	28	333	72	103
Queue Length 95th (ft)	41	98	7	#129	111	89	73	578	#187	202
Internal Link Dist (ft)		810			1637			2644		1335
Turn Bay Length (ft)	130			205		200	215		210	
Base Capacity (vph)	200	598	575	144	626	647	171	2030	203	2150
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.26	0.15	0.11	0.51	0.17	0.35	0.27	0.67	0.59	0.28

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	20	256	1227	25	89	569
v/c Ratio	0.06	0.52	0.71	0.03	0.41	0.25
Control Delay (s/veh)	31.5	8.7	18.2	6.6	41.9	5.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	31.5	8.7	18.2	6.6	41.9	5.8
Queue Length 50th (ft)	7	0	198	1	36	39
Queue Length 95th (ft)	32	62	436	16	114	107
Internal Link Dist (ft)	3339		1159			2644
Turn Bay Length (ft)	150			200	300	
Base Capacity (vph)	820	871	2815	1263	412	3302
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.02	0.29	0.44	0.02	0.22	0.17

Intersection Summary

Intersection: 1: La Sierra Av. & SR-91 WB Ramps

Movement	WB	WB	WB	NB	NB	NB	NB	NB	SB	SB	SB	SB
Directions Served	L	LTR	R	L	L	T	T	T	T	T	T	R
Maximum Queue (ft)	225	327	270	245	347	262	263	254	402	339	198	138
Average Queue (ft)	100	208	154	189	220	168	168	165	256	168	96	42
95th Queue (ft)	213	291	257	281	323	252	249	241	366	312	167	91
Link Distance (ft)		1183			444	444	444	444	949	949	949	
Upstream Blk Time (%)					0							
Queuing Penalty (veh)					1							
Storage Bay Dist (ft)	500		500	155								140
Storage Blk Time (%)				9	20						3	
Queuing Penalty (veh)				27	61						3	

Intersection: 2: La Sierra Av. & SR-91 EB Ramps

Movement	EB	EB	EB	NB	NB	NB	NB	SB	SB	SB	SB	SB
Directions Served	L	LTR	R	T	T	T	R	L	L	T	T	T
Maximum Queue (ft)	372	489	417	435	362	336	214	192	204	198	176	174
Average Queue (ft)	55	293	209	246	193	181	84	94	119	31	38	62
95th Queue (ft)	260	451	386	392	323	294	163	164	179	109	109	126
Link Distance (ft)	1430	1430		848	848	848				444	444	444
Upstream Blk Time (%)										0	0	
Queuing Penalty (veh)										0	0	
Storage Bay Dist (ft)			450				415	155	155			
Storage Blk Time (%)		2	0					1	9	0		
Queuing Penalty (veh)		3	0					2	26	0		

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	L	T	T	R	L	L	T	T	R	L	L
Maximum Queue (ft)	234	260	328	193	85	66	82	112	191	180	201	256
Average Queue (ft)	138	168	79	53	33	20	41	56	55	93	105	130
95th Queue (ft)	234	252	223	137	64	50	78	96	126	164	166	217
Link Distance (ft)			2554	2554				2432	2432			
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	205	205			205	205	205			130	200	200
Storage Blk Time (%)	1	5	1						0	5	0	0
Queuing Penalty (veh)	1	5	4						0	5	0	1

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	NB	NB	NB	NB	SB	SB	SB	SB	SB	SB
Directions Served	T	T	T	R	L	L	T	T	T	R
Maximum Queue (ft)	294	307	363	146	75	103	212	223	216	257
Average Queue (ft)	174	183	210	31	30	54	136	143	130	120
95th Queue (ft)	277	287	331	91	63	92	201	208	200	222
Link Distance (ft)	2691	2691	2691				848	848	848	
Upstream Blk Time (%)										
Queuing Penalty (veh)										
Storage Bay Dist (ft)				305	355	355				535
Storage Blk Time (%)	6		2							
Queuing Penalty (veh)	18		2							

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	L	T	R	L	T	R	L	T	TR	L	T
Maximum Queue (ft)	36	83	128	63	110	138	157	170	349	374	163	179
Average Queue (ft)	8	35	51	26	46	58	69	30	178	205	72	67
95th Queue (ft)	30	71	100	55	89	116	124	88	295	331	134	157
Link Distance (ft)			843	843		1670			2640	2640		1333
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	130	130			205		200	215			210	
Storage Blk Time (%)		0	1				0		4			0
Queuing Penalty (veh)		0	0				0		2			0

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	SB	B20	B20
Directions Served	TR	T	T
Maximum Queue (ft)	198	6	37
Average Queue (ft)	80	0	2
95th Queue (ft)	168	5	16
Link Distance (ft)	1333	2691	2691
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 5: La Sierra Av. & McAllister Pkwy.

Movement	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	R	T	T	R	L	T	T
Maximum Queue (ft)	41	180	228	216	30	112	113	117
Average Queue (ft)	10	67	116	94	5	49	28	46
95th Queue (ft)	32	133	194	183	23	91	83	104
Link Distance (ft)		3360	1207	1207			2640	2640
Upstream Blk Time (%)								
Queuing Penalty (veh)								
Storage Bay Dist (ft)	150				200	300		
Storage Blk Time (%)		1		0				
Queuing Penalty (veh)		0		0				

Network Summary

Network wide Queuing Penalty: 163



Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	354	336	323	384	926	1396	275
v/c Ratio	0.79	0.70	0.65	0.76	0.29	0.62	0.34
Control Delay (s/veh)	46.2	30.3	25.5	38.5	21.5	24.6	8.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	46.2	30.3	25.5	38.5	21.5	24.6	8.1
Queue Length 50th (ft)	217	148	118	133	210	251	29
Queue Length 95th (ft)	300	235	197	176	251	342	96
Internal Link Dist (ft)		1180			412	935	
Turn Bay Length (ft)	500		500	155			140
Base Capacity (vph)	571	583	600	553	3221	2257	806
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.62	0.58	0.54	0.69	0.29	0.62	0.34

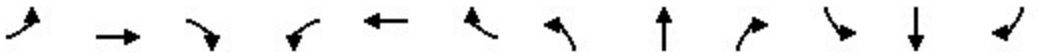
Intersection Summary



Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	184	420	422	1080	295	413	1440
v/c Ratio	0.32	0.83	0.75	0.60	0.40	0.77	0.51
Control Delay (s/veh)	24.1	42.6	32.0	29.9	5.2	30.0	9.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.3
Total Delay (s/veh)	24.1	42.6	32.0	29.9	5.2	30.0	10.0
Queue Length 50th (ft)	88	253	205	212	0	72	198
Queue Length 95th (ft)	128	346	290	284	62	#188	281
Internal Link Dist (ft)		1416		886			412
Turn Bay Length (ft)			450		415	155	
Base Capacity (vph)	722	628	681	1802	746	550	2825
Starvation Cap Reductn	0	0	0	0	0	0	618
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.25	0.67	0.62	0.60	0.40	0.75	0.65

Intersection Summary

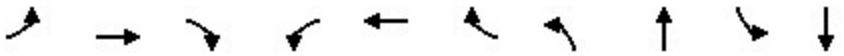
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	401	373	398	165	205	264	233	866	118	307	1571	405
v/c Ratio	0.82	0.44	0.77	0.55	0.31	0.60	0.79	0.48	0.19	0.78	0.81	0.48
Control Delay (s/veh)	58.3	33.8	28.1	53.4	35.7	16.9	66.5	28.2	6.2	59.9	33.6	4.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	58.3	33.8	28.1	53.4	35.7	16.9	66.5	28.2	6.2	59.9	33.6	4.9
Queue Length 50th (ft)	126	107	125	51	60	43	75	151	0	97	312	0
Queue Length 95th (ft)	#263	151	236	101	91	116	#174	254	44	#207	#542	72
Internal Link Dist (ft)		2549			2428			2703			886	
Turn Bay Length (ft)	205		205	205		130	200		305	355		535
Base Capacity (vph)	490	1461	753	368	1335	704	296	1799	626	402	1943	836
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.82	0.26	0.53	0.45	0.15	0.38	0.79	0.48	0.19	0.76	0.81	0.48

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	62	111	156	277	80	183	29	881	201	1536
v/c Ratio	0.34	0.43	0.41	0.91	0.16	0.33	0.33	0.73	0.80	0.91
Control Delay (s/veh)	55.1	45.5	5.7	76.8	30.9	6.1	61.4	33.3	68.1	35.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	55.1	45.5	5.7	76.8	30.9	6.1	61.4	33.3	68.1	35.0
Queue Length 50th (ft)	20	70	0	180	43	0	19	243	129	476
Queue Length 95th (ft)	48	121	30	#412	81	50	55	397	#301	#841
Internal Link Dist (ft)		810			1637			2644		1335
Turn Bay Length (ft)	130			205		200	215		210	
Base Capacity (vph)	187	535	586	305	742	732	87	1338	259	1692
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.33	0.21	0.27	0.91	0.11	0.25	0.33	0.66	0.78	0.91

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	9	160	751	13	288	1582
v/c Ratio	0.03	0.38	0.63	0.02	0.68	0.69
Control Delay (s/veh)	29.8	8.6	24.2	10.9	35.7	10.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	29.8	8.6	24.2	10.9	35.7	10.4
Queue Length 50th (ft)	3	0	127	0	102	166
Queue Length 95th (ft)	19	53	306	14	280	431
Internal Link Dist (ft)	3339		1159			2644
Turn Bay Length (ft)	150			200	300	
Base Capacity (vph)	777	785	2127	936	877	3317
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.01	0.20	0.35	0.01	0.33	0.48

Intersection Summary

Intersection: 1: La Sierra Av. & SR-91 WB Ramps

Movement	WB	WB	WB	NB	NB	NB	NB	NB	SB	SB	SB	SB
Directions Served	L	LTR	R	L	L	T	T	T	T	T	T	R
Maximum Queue (ft)	264	364	316	204	224	237	221	231	667	601	281	188
Average Queue (ft)	157	243	184	117	145	148	146	151	422	339	151	59
95th Queue (ft)	253	323	288	179	205	220	218	228	614	543	253	131
Link Distance (ft)		1183			444	444	444	444	949	949	949	
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	500		500	155								140
Storage Blk Time (%)				1	6						5	
Queuing Penalty (veh)				3	11						15	

Intersection: 2: La Sierra Av. & SR-91 EB Ramps

Movement	EB	EB	EB	NB	NB	NB	NB	SB	SB	SB	SB	SB
Directions Served	L	LTR	R	T	T	T	R	L	L	T	T	T
Maximum Queue (ft)	224	373	359	338	315	320	137	187	227	337	277	285
Average Queue (ft)	14	250	203	198	174	183	64	105	134	99	94	122
95th Queue (ft)	94	343	310	319	279	283	109	177	204	225	187	215
Link Distance (ft)	1430	1430		849	849	849				444	444	444
Upstream Blk Time (%)										0		
Queuing Penalty (veh)										0		
Storage Bay Dist (ft)			450				415	155	155			
Storage Blk Time (%)								0	9	1		
Queuing Penalty (veh)								2	42	5		

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	L	T	T	R	L	L	T	T	R	L	L
Maximum Queue (ft)	234	263	386	308	255	93	114	116	112	136	151	202
Average Queue (ft)	133	165	138	114	128	43	63	59	46	66	72	89
95th Queue (ft)	235	264	334	240	222	83	105	103	90	119	128	153
Link Distance (ft)			2554	2554				2405	2405			
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	205	205			205	205	205			130	200	200
Storage Blk Time (%)	3	8	2	1	1				0	1	0	0
Queuing Penalty (veh)	6	14	8	2	2				0	1	0	1

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	NB	NB	NB	NB	SB	SB	SB	SB	SB	SB
Directions Served	T	T	T	R	L	L	T	T	T	R
Maximum Queue (ft)	237	239	251	86	173	220	337	364	393	195
Average Queue (ft)	112	123	143	27	94	108	216	231	239	81
95th Queue (ft)	200	205	227	63	156	179	328	340	358	157
Link Distance (ft)	2691	2691	2691				849	849	849	
Upstream Blk Time (%)										
Queuing Penalty (veh)										
Storage Bay Dist (ft)				305	355	355				535
Storage Blk Time (%)	1		0				0			
Queuing Penalty (veh)	3		0				0			

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	L	T	R	L	T	R	L	T	TR	L	T
Maximum Queue (ft)	55	101	147	135	252	175	121	188	290	338	275	510
Average Queue (ft)	11	41	68	67	153	52	52	25	146	174	165	281
95th Queue (ft)	38	83	128	116	233	117	96	90	256	299	286	476
Link Distance (ft)			843	843		1670			2640	2640		1333
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	130	130			205		200	215			210	
Storage Blk Time (%)		0	1		4	0			3		2	16
Queuing Penalty (veh)		0	1		9	0			1		14	31

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	SB	B20	B20
Directions Served	TR	T	T
Maximum Queue (ft)	526	228	608
Average Queue (ft)	291	16	41
95th Queue (ft)	485	177	431
Link Distance (ft)	1333	2691	2691
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 5: La Sierra Av. & McAllister Pkwy.

Movement	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	R	T	T	R	L	T	T
Maximum Queue (ft)	29	103	206	189	30	285	296	275
Average Queue (ft)	4	34	99	78	4	142	65	82
95th Queue (ft)	19	70	180	157	19	235	189	201
Link Distance (ft)		3360	1207	1207			2640	2640
Upstream Blk Time (%)								
Queuing Penalty (veh)								
Storage Bay Dist (ft)	150				200	300		
Storage Blk Time (%)		0		0		0	0	
Queuing Penalty (veh)		0		0		3	0	

Network Summary

Network wide Queuing Penalty: 171

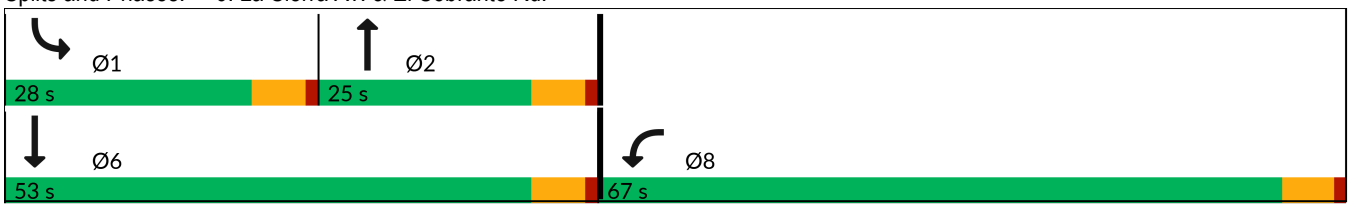
APPENDIX 3.5: EXISTING (2024) CONDITIONS INTERSECTION OPERATIONS ANALYSIS WORKSHEETS WITH IMPROVEMENTS

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Lane Group	WBL	NBT	SBL	SBT
Lane Configurations				
Traffic Volume (vph)	58	108	227	127
Future Volume (vph)	58	108	227	127
Turn Type	Prot	NA	Prot	NA
Protected Phases	8	2	1	6
Permitted Phases				
Detector Phase	8	2	1	6
Switch Phase				
Minimum Initial (s)	5.0	10.0	5.0	10.0
Minimum Split (s)	23.8	23.8	10.8	23.8
Total Split (s)	67.0	25.0	28.0	53.0
Total Split (%)	55.8%	20.8%	23.3%	44.2%
Yellow Time (s)	4.8	4.8	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.8	5.8	5.8	5.8
Lead/Lag		Lag	Lead	
Lead-Lag Optimize?		Yes	Yes	
Recall Mode	None	None	None	None
Act Effct Green (s)	9.9	12.3	13.2	22.9
Actuated g/C Ratio	0.20	0.25	0.27	0.47
v/c Ratio	0.76	0.15	0.50	0.15
Control Delay (s/veh)	11.8	17.6	22.6	7.5
Queue Delay	0.0	0.0	0.0	0.0
Total Delay (s/veh)	11.8	17.6	22.6	7.5
LOS	B	B	C	A
Approach Delay (s/veh)	11.8	17.6		17.1
Approach LOS	B	B		B

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 49	
Natural Cycle: 65	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.76	
Intersection Signal Delay (s/veh): 14.4	Intersection LOS: B
Intersection Capacity Utilization 66.7%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 6: La Sierra Av. & El Sobrante Rd.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	58	455	108	20	227	127
Future Volume (veh/h)	58	455	108	20	227	127
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	61	268	114	16	239	134
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	77	336	563	78	312	906
Arrive On Green	0.26	0.26	0.18	0.18	0.17	0.48
Sat Flow, veh/h	299	1315	3230	432	1781	1870
Grp Volume(v), veh/h	330	0	64	66	239	134
Grp Sat Flow(s),veh/h/ln	1619	0	1777	1793	1781	1870
Q Serve(g_s), s	8.5	0.0	1.4	1.4	5.7	1.8
Cycle Q Clear(g_c), s	8.5	0.0	1.4	1.4	5.7	1.8
Prop In Lane	0.18	0.81		0.24	1.00	
Lane Grp Cap(c), veh/h	414	0	319	322	312	906
V/C Ratio(X)	0.80	0.00	0.20	0.21	0.77	0.15
Avail Cap(c_a), veh/h	2220	0	764	771	886	1978
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	15.5	0.0	15.6	15.6	17.5	6.4
Incr Delay (d2), s/veh	3.5	0.0	0.3	0.3	4.0	0.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.8	0.0	0.4	0.5	2.1	0.4
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	19.1	0.0	15.9	15.9	21.5	6.5
LnGrp LOS	B		B	B	C	A
Approach Vol, veh/h	330		130			373
Approach Delay, s/veh	19.1		15.9			16.1
Approach LOS	B		B			B

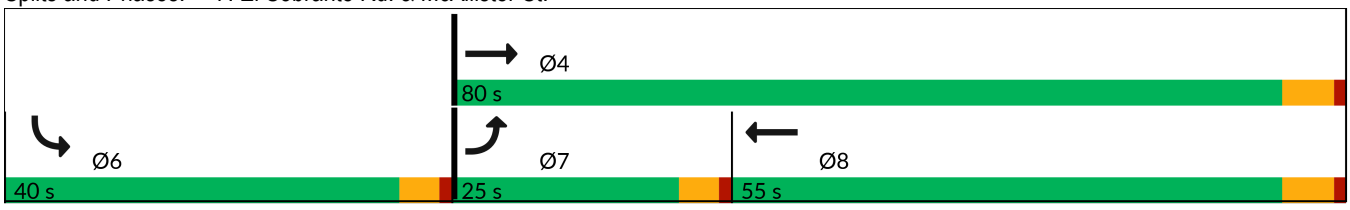
Timer - Assigned Phs	1	2				6	8
Phs Duration (G+Y+Rc), s	13.6	13.8				27.4	17.2
Change Period (Y+Rc), s	5.8	5.8				5.8	5.8
Max Green Setting (Gmax), s	22.2	19.2				47.2	61.2
Max Q Clear Time (g_c+I1), s	7.7	3.4				3.8	10.5
Green Ext Time (p_c), s	0.5	0.4				0.6	1.1

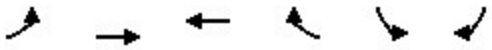
Intersection Summary	
HCM 7th Control Delay, s/veh	17.2
HCM 7th LOS	B

Lane Group	EBL	EBT	WBT	SBL
Lane Configurations				
Traffic Volume (vph)	49	217	439	47
Future Volume (vph)	49	217	439	47
Turn Type	Prot	NA	NA	Prot
Protected Phases	7	4	8	6
Permitted Phases				
Detector Phase	7	4	8	6
Switch Phase				
Minimum Initial (s)	5.0	10.0	10.0	5.0
Minimum Split (s)	10.8	23.8	23.8	23.8
Total Split (s)	25.0	80.0	55.0	40.0
Total Split (%)	20.8%	66.7%	45.8%	33.3%
Yellow Time (s)	3.6	4.8	4.8	3.6
All-Red Time (s)	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.8	5.8	4.6
Lead/Lag	Lead		Lag	
Lead-Lag Optimize?	Yes		Yes	
Recall Mode	None	None	None	None
Act Effct Green (s)	6.9	19.9	16.6	8.3
Actuated g/C Ratio	0.20	0.58	0.49	0.24
v/c Ratio	0.14	0.21	0.32	0.27
Control Delay (s/veh)	18.6	6.4	10.4	9.9
Queue Delay	0.0	0.0	0.0	0.0
Total Delay (s/veh)	18.6	6.4	10.4	9.9
LOS	B	A	B	A
Approach Delay (s/veh)		8.6	10.4	9.9
Approach LOS		A	B	A

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 34.2	
Natural Cycle: 60	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.32	
Intersection Signal Delay (s/veh): 9.8	Intersection LOS: A
Intersection Capacity Utilization 38.5%	ICU Level of Service A
Analysis Period (min) 15	

Splits and Phases: 7: El Sobrante Rd. & McAllister St.





Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	49	217	439	88	47	70
Future Volume (veh/h)	49	217	439	88	47	70
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	51	226	457	92	49	73
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	103	1017	979	196	71	106
Arrive On Green	0.06	0.54	0.33	0.33	0.11	0.11
Sat Flow, veh/h	1781	1870	3044	590	661	985
Grp Volume(v), veh/h	51	226	274	275	123	0
Grp Sat Flow(s),veh/h/ln	1781	1870	1777	1764	1660	0
Q Serve(g_s), s	0.8	1.9	3.6	3.7	2.1	0.0
Cycle Q Clear(g_c), s	0.8	1.9	3.6	3.7	2.1	0.0
Prop In Lane	1.00			0.33	0.40	0.59
Lane Grp Cap(c), veh/h	103	1017	590	585	178	0
V/C Ratio(X)	0.50	0.22	0.46	0.47	0.69	0.00
Avail Cap(c_a), veh/h	1219	4655	2933	2912	1971	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	13.6	3.5	7.9	7.9	12.8	0.0
Incr Delay (d2), s/veh	1.4	0.1	0.6	0.6	1.8	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	0.1	0.7	0.7	0.6	0.0
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	15.0	3.6	8.4	8.5	14.6	0.0
LnGrp LOS	B	A	A	A	B	
Approach Vol, veh/h		277	549		123	
Approach Delay, s/veh		5.7	8.5		14.6	
Approach LOS		A	A		B	

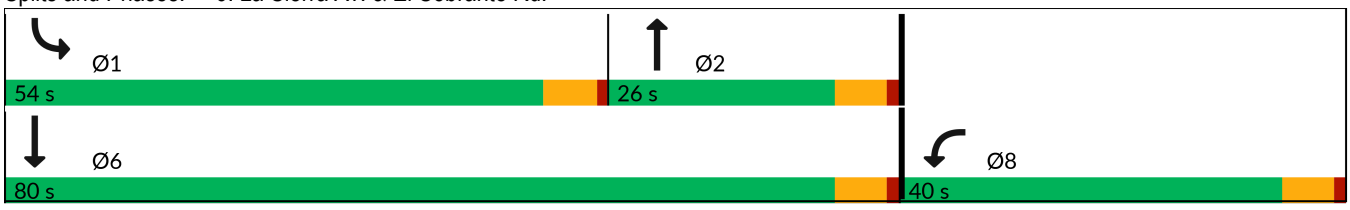
Timer - Assigned Phs	4	6	7	8
Phs Duration (G+Y+Rc), s	22.0	7.8	6.3	15.7
Change Period (Y+Rc), s	5.8	4.6	4.6	5.8
Max Green Setting (Gmax), s	74.2	35.4	20.4	49.2
Max Q Clear Time (g_c+I1), s	3.9	4.1	2.8	5.7
Green Ext Time (p_c), s	1.2	0.2	0.0	3.2

Intersection Summary	
HCM 7th Control Delay, s/veh	8.5
HCM 7th LOS	A

Lane Group	WBL	NBT	SBL	SBT
Lane Configurations				
Traffic Volume (vph)	72	221	616	474
Future Volume (vph)	72	221	616	474
Turn Type	Prot	NA	Prot	NA
Protected Phases	8	2	1	6
Permitted Phases				
Detector Phase	8	2	1	6
Switch Phase				
Minimum Initial (s)	5.0	10.0	5.0	10.0
Minimum Split (s)	23.8	23.8	10.8	23.8
Total Split (s)	40.0	26.0	54.0	80.0
Total Split (%)	33.3%	21.7%	45.0%	66.7%
Yellow Time (s)	4.8	4.8	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.8	5.8	5.8	5.8
Lead/Lag		Lag	Lead	
Lead-Lag Optimize?		Yes	Yes	
Recall Mode	None	None	None	None
Act Effct Green (s)	20.3	14.3	40.6	61.1
Actuated g/C Ratio	0.22	0.15	0.43	0.65
v/c Ratio	0.81	0.59	0.86	0.42
Control Delay (s/veh)	33.6	39.7	38.6	10.0
Queue Delay	0.0	0.0	0.0	0.0
Total Delay (s/veh)	33.6	39.7	38.6	10.0
LOS	C	D	D	B
Approach Delay (s/veh)	33.6	39.7		26.2
Approach LOS	C	D		C

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 93.6	
Natural Cycle: 90	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.86	
Intersection Signal Delay (s/veh): 30.1	Intersection LOS: C
Intersection Capacity Utilization 80.1%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 6: La Sierra Av. & El Sobrante Rd.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	72	303	221	83	616	474
Future Volume (veh/h)	72	303	221	83	616	474
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	77	192	238	67	662	510
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	92	230	395	109	728	1189
Arrive On Green	0.20	0.20	0.14	0.14	0.41	0.64
Sat Flow, veh/h	467	1164	2847	758	1781	1870
Grp Volume(v), veh/h	270	0	152	153	662	510
Grp Sat Flow(s),veh/h/ln	1637	0	1777	1734	1781	1870
Q Serve(g_s), s	11.0	0.0	5.6	5.8	24.3	9.5
Cycle Q Clear(g_c), s	11.0	0.0	5.6	5.8	24.3	9.5
Prop In Lane	0.29	0.71		0.44	1.00	
Lane Grp Cap(c), veh/h	323	0	255	249	728	1189
V/C Ratio(X)	0.83	0.00	0.60	0.62	0.91	0.43
Avail Cap(c_a), veh/h	805	0	516	504	1235	1996
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	26.8	0.0	27.9	28.0	19.3	6.3
Incr Delay (d2), s/veh	5.6	0.0	2.2	2.5	5.9	0.2
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.4	0.0	2.2	2.3	9.0	2.2
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	32.5	0.0	30.1	30.5	25.2	6.6
LnGrp LOS	C		C	C	C	A
Approach Vol, veh/h	270		305			1172
Approach Delay, s/veh	32.5		30.3			17.1
Approach LOS	C		C			B

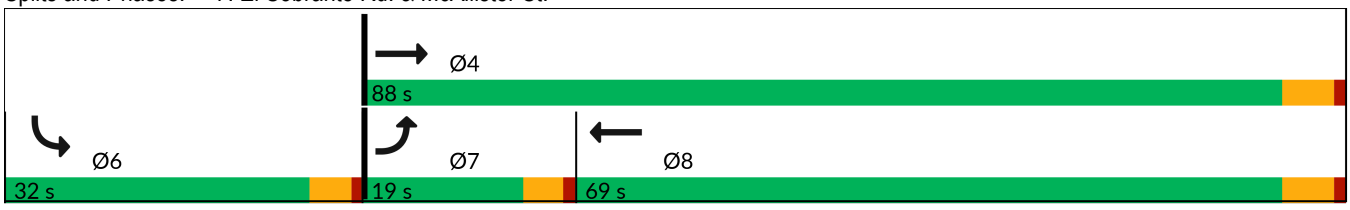
Timer - Assigned Phs	1	2				6	8
Phs Duration (G+Y+Rc), s	34.2	15.8				50.0	19.5
Change Period (Y+Rc), s	5.8	5.8				5.8	5.8
Max Green Setting (Gmax), s	48.2	20.2				74.2	34.2
Max Q Clear Time (g_c+I1), s	26.3	7.8				11.5	13.0
Green Ext Time (p_c), s	2.1	1.1				3.0	0.8

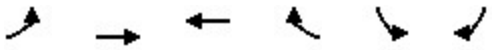
Intersection Summary	
HCM 7th Control Delay, s/veh	21.8
HCM 7th LOS	C

Lane Group	EBL	EBT	WBT	SBL
Lane Configurations				
Traffic Volume (vph)	60	640	283	32
Future Volume (vph)	60	640	283	32
Turn Type	Prot	NA	NA	Prot
Protected Phases	7	4	8	6
Permitted Phases				
Detector Phase	7	4	8	6
Switch Phase				
Minimum Initial (s)	5.0	10.0	10.0	5.0
Minimum Split (s)	10.8	23.8	23.8	23.8
Total Split (s)	19.0	88.0	69.0	32.0
Total Split (%)	15.8%	73.3%	57.5%	26.7%
Yellow Time (s)	3.6	4.8	4.8	3.6
All-Red Time (s)	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.8	5.8	4.6
Lead/Lag	Lead		Lag	
Lead-Lag Optimize?	Yes		Yes	
Recall Mode	None	None	None	None
Act Effct Green (s)	7.0	23.1	19.3	7.8
Actuated g/C Ratio	0.19	0.63	0.52	0.21
v/c Ratio	0.19	0.58	0.18	0.33
Control Delay (s/veh)	19.3	9.4	9.6	8.4
Queue Delay	0.0	0.0	0.0	0.0
Total Delay (s/veh)	19.3	9.4	9.6	8.4
LOS	B	A	A	A
Approach Delay (s/veh)		10.3	9.6	8.4
Approach LOS		B	A	A

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 36.8	
Natural Cycle: 60	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.58	
Intersection Signal Delay (s/veh): 9.9	Intersection LOS: A
Intersection Capacity Utilization 50.6%	ICU Level of Service A
Analysis Period (min) 15	

Splits and Phases: 7: El Sobrante Rd. & McAllister St.





Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	60	640	283	28	32	105
Future Volume (veh/h)	60	640	283	28	32	105
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	64	681	301	30	34	112
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	123	1001	1021	101	44	146
Arrive On Green	0.07	0.54	0.31	0.31	0.12	0.12
Sat Flow, veh/h	1781	1870	3359	323	377	1241
Grp Volume(v), veh/h	64	681	163	168	147	0
Grp Sat Flow(s),veh/h/ln	1781	1870	1777	1812	1628	0
Q Serve(g_s), s	1.0	8.0	2.1	2.1	2.6	0.0
Cycle Q Clear(g_c), s	1.0	8.0	2.1	2.1	2.6	0.0
Prop In Lane	1.00			0.18	0.23	0.76
Lane Grp Cap(c), veh/h	123	1001	555	566	192	0
V/C Ratio(X)	0.52	0.68	0.29	0.30	0.77	0.00
Avail Cap(c_a), veh/h	856	5132	3749	3823	1489	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	13.5	5.1	7.8	7.8	12.8	0.0
Incr Delay (d2), s/veh	1.3	0.8	0.3	0.3	2.4	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	0.6	0.4	0.4	0.7	0.0
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	14.7	5.9	8.1	8.1	15.2	0.0
LnGrp LOS	B	A	A	A	B	
Approach Vol, veh/h		745	331		147	
Approach Delay, s/veh		6.7	8.1		15.2	
Approach LOS		A	A		B	

Timer - Assigned Phs	4	6	7	8
Phs Duration (G+Y+Rc), s	21.8	8.1	6.7	15.2
Change Period (Y+Rc), s	5.8	4.6	4.6	5.8
Max Green Setting (Gmax), s	82.2	27.4	14.4	63.2
Max Q Clear Time (g_c+I1), s	10.0	4.6	3.0	4.1
Green Ext Time (p_c), s	4.9	0.2	0.0	1.8

Intersection Summary	
HCM 7th Control Delay, s/veh	8.1
HCM 7th LOS	A

APPENDIX 5.1: EAP (2027) (PROJECT ONLY) CONDITIONS INTERSECTION OPERATIONS ANALYSIS WORKSHEETS

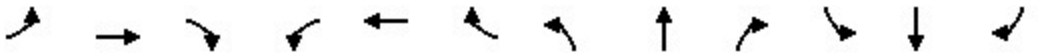
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Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Configurations							
Traffic Volume (vph)	298	0	489	664	1396	985	141
Future Volume (vph)	298	0	489	664	1396	985	141
Turn Type	Split	NA	Perm	Prot	NA	NA	Perm
Protected Phases	8	8		5	2	6	
Permitted Phases			8				6
Detector Phase	8	8	8	5	2	6	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	9.0	26.5	29.5	29.5
Total Split (s)	37.0	37.0	37.0	30.0	63.0	33.0	33.0
Total Split (%)	37.0%	37.0%	37.0%	30.0%	63.0%	33.0%	33.0%
Yellow Time (s)	4.0	4.0	4.0	3.0	4.5	4.5	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	4.0	5.5	5.5	5.5
Lead/Lag				Lead		Lag	Lag
Lead-Lag Optimize?				Yes		Yes	Yes
Recall Mode	None	None	None	None	C-Min	C-Min	C-Min
Act Effct Green (s)	22.5	22.5	22.5	23.6	67.0	39.4	39.4
Actuated g/C Ratio	0.23	0.23	0.23	0.24	0.67	0.39	0.39
v/c Ratio	0.74	0.69	0.67	0.85	0.42	0.51	0.22
Control Delay (s/veh)	47.2	34.6	33.4	42.4	15.7	26.0	7.3
Queue Delay	0.0	0.0	0.0	0.0	0.4	0.0	0.0
Total Delay (s/veh)	47.2	34.6	33.4	42.4	16.1	26.0	7.3
LOS	D	C	C	D	B	C	A
Approach Delay (s/veh)		38.5			24.6	23.6	
Approach LOS		D			C	C	

Intersection Summary	
Cycle Length: 100	
Actuated Cycle Length: 100	
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow, Master Intersection	
Natural Cycle: 65	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.85	
Intersection Signal Delay (s/veh): 27.1	Intersection LOS: C
Intersection Capacity Utilization 68.9%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 1: La Sierra Av. & SR-91 WB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	298	0	489	664	1396	0	0	985	141
Future Volume (veh/h)	0	0	0	298	0	489	664	1396	0	0	985	141
Initial Q (Qb), veh				0	0	0	0	0	0	0	0	0
Lane Width Adj.				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach					No			No			No	
Adj Sat Flow, veh/h/ln				1856	1900	1885	1870	1870	0	0	1856	1826
Adj Flow Rate, veh/h				413	0	216	685	1439	0	0	1015	102
Peak Hour Factor				0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %				3	0	1	2	2	0	0	3	5
Cap, veh/h				585	0	264	743	3725	0	0	2404	722
Arrive On Green				0.17	0.00	0.17	0.43	1.00	0.00	0.00	0.47	0.47
Sat Flow, veh/h				3534	0	1598	3456	5274	0	0	5233	1522
Grp Volume(v), veh/h				413	0	216	685	1439	0	0	1015	102
Grp Sat Flow(s),veh/h/ln				1767	0	1598	1728	1702	0	0	1689	1522
Q Serve(g_s), s				11.0	0.0	13.0	18.7	0.0	0.0	0.0	13.2	3.8
Cycle Q Clear(g_c), s				11.0	0.0	13.0	18.7	0.0	0.0	0.0	13.2	3.8
Prop In Lane				1.00		1.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h				585	0	264	743	3725	0	0	2404	722
V/C Ratio(X)				0.71	0.00	0.82	0.92	0.39	0.00	0.00	0.42	0.14
Avail Cap(c_a), veh/h				1131	0	511	898	3725	0	0	2404	722
HCM Platoon Ratio				1.00	1.00	1.00	2.00	2.00	1.00	1.00	1.00	1.00
Upstream Filter(I)				1.00	0.00	1.00	0.65	0.65	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh				39.4	0.0	40.3	27.7	0.0	0.0	0.0	17.3	14.8
Incr Delay (d2), s/veh				1.2	0.0	4.6	8.4	0.2	0.0	0.0	0.5	0.4
Initial Q Delay(d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln				4.7	0.0	5.2	6.2	0.1	0.0	0.0	4.8	1.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh				40.6	0.0	44.9	36.1	0.2	0.0	0.0	17.8	15.2
LnGrp LOS				D		D	D	A			B	B
Approach Vol, veh/h					629			2124			1117	
Approach Delay, s/veh					42.1			11.8			17.6	
Approach LOS					D			B			B	

Timer - Assigned Phs		2			5	6		8				
Phs Duration (G+Y+Rc), s		78.5			25.5	53.0		21.5				
Change Period (Y+Rc), s		5.5			4.0	5.5		5.0				
Max Green Setting (Gmax), s		57.5			26.0	27.5		32.0				
Max Q Clear Time (g_c+I1), s		2.0			20.7	15.2		15.0				
Green Ext Time (p_c), s		13.8			0.8	5.4		1.5				

Intersection Summary		
HCM 7th Control Delay, s/veh	18.4	
HCM 7th LOS	B	

Notes

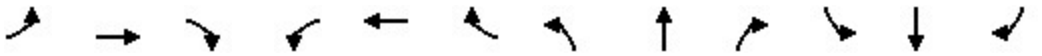
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	435	1	429	1574	499	320	960
Future Volume (vph)	435	1	429	1574	499	320	960
Turn Type	Split	NA	Perm	NA	Perm	Prot	NA
Protected Phases	4	4		2		1	6
Permitted Phases			4		2		
Detector Phase	4	4	4	2	2	1	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	29.5	29.5	9.0	26.5
Total Split (s)	37.0	37.0	37.0	45.0	45.0	18.0	63.0
Total Split (%)	37.0%	37.0%	37.0%	45.0%	45.0%	18.0%	63.0%
Yellow Time (s)	4.0	4.0	4.0	4.5	4.5	3.0	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.5	5.5	4.0	5.5
Lead/Lag				Lag	Lag	Lead	
Lead-Lag Optimize?				Yes	Yes	Yes	
Recall Mode	None	None	None	C-Min	C-Min	None	C-Min
Act Effct Green (s)	24.4	24.4	24.4	47.4	47.4	13.7	65.1
Actuated g/C Ratio	0.24	0.24	0.24	0.47	0.47	0.14	0.65
v/c Ratio	0.76	0.74	0.66	0.69	0.52	0.72	0.30
Control Delay (s/veh)	46.3	37.9	27.9	23.8	3.9	51.6	2.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	46.3	37.9	27.9	23.8	3.9	51.6	2.2
LOS	D	D	C	C	A	D	A
Approach Delay (s/veh)		37.7		19.0			14.5
Approach LOS		D		B			B

Intersection Summary	
Cycle Length: 100	
Actuated Cycle Length: 100	
Offset: 28 (28%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow	
Natural Cycle: 60	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.76	
Intersection Signal Delay (s/veh): 21.5	Intersection LOS: C
Intersection Capacity Utilization 68.9%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 2: La Sierra Av. & SR-91 EB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	435	1	429	0	0	0	0	1574	499	320	960	0
Future Volume (veh/h)	435	1	429	0	0	0	0	1574	499	320	960	0
Initial Q (Qb), veh	0	0	0				0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No						No			No	
Adj Sat Flow, veh/h/ln	1885	1900	1811				0	1856	1856	1841	1856	0
Adj Flow Rate, veh/h	522	0	147				0	1640	368	333	1000	0
Peak Hour Factor	0.96	0.96	0.96				0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	1	0	6				0	3	3	4	3	0
Cap, veh/h	633	0	270				0	2838	879	404	3641	0
Arrive On Green	0.18	0.00	0.18				0.00	0.56	0.56	0.08	0.48	0.00
Sat Flow, veh/h	3591	0	1535				0	5233	1570	3401	5233	0
Grp Volume(v), veh/h	522	0	147				0	1640	368	333	1000	0
Grp Sat Flow(s),veh/h/ln	1795	0	1535				0	1689	1570	1700	1689	0
Q Serve(g_s), s	14.0	0.0	8.7				0.0	21.1	13.5	9.6	11.8	0.0
Cycle Q Clear(g_c), s	14.0	0.0	8.7				0.0	21.1	13.5	9.6	11.8	0.0
Prop In Lane	1.00		1.00				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	633	0	270				0	2838	879	404	3641	0
V/C Ratio(X)	0.83	0.00	0.54				0.00	0.58	0.42	0.82	0.27	0.00
Avail Cap(c_a), veh/h	1149	0	491				0	2838	879	476	3641	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	0.67	0.67	1.00
Upstream Filter(I)	1.00	0.00	1.00				0.00	0.47	0.47	0.81	0.81	0.00
Uniform Delay (d), s/veh	39.7	0.0	37.5				0.0	14.3	12.6	45.0	10.3	0.0
Incr Delay (d2), s/veh	2.1	0.0	1.3				0.0	0.4	0.7	7.7	0.2	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.1	0.0	3.2				0.0	7.1	4.3	4.5	4.2	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	41.8	0.0	38.8				0.0	14.7	13.3	52.7	10.5	0.0
LnGrp LOS	D		D					B	B	D	B	
Approach Vol, veh/h		669						2008			1333	
Approach Delay, s/veh		41.1						14.5			21.0	
Approach LOS		D						B			C	

Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	15.9	61.5		22.6		77.4
Change Period (Y+Rc), s	4.0	5.5		5.0		5.5
Max Green Setting (Gmax), s	14.0	39.5		32.0		57.5
Max Q Clear Time (g_c+I1), s	11.6	23.1		16.0		13.8
Green Ext Time (p_c), s	0.2	11.0		1.6		7.8

Intersection Summary		
HCM 7th Control Delay, s/veh		21.1
HCM 7th LOS		C

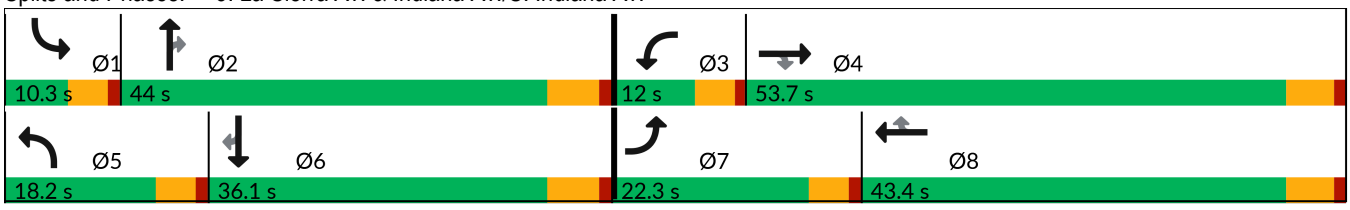
Notes

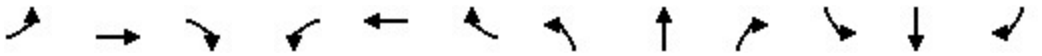
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	462	212	149	101	223	266	354	1500	162	131	965	464
Future Volume (vph)	462	212	149	101	223	266	354	1500	162	131	965	464
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4			8			2			6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	9.6	43.4	43.4	9.6	43.4	43.4	9.6	37.8	37.8	9.6	32.8	32.8
Total Split (s)	22.3	53.7	53.7	12.0	43.4	43.4	18.2	44.0	44.0	10.3	36.1	36.1
Total Split (%)	18.6%	44.8%	44.8%	10.0%	36.2%	36.2%	15.2%	36.7%	36.7%	8.6%	30.1%	30.1%
Yellow Time (s)	3.6	4.4	4.4	3.6	4.4	4.4	3.6	4.8	4.8	3.6	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.4	5.4	4.6	5.4	5.4	4.6	5.8	5.8	4.6	5.8	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	17.9	27.8	27.8	6.9	16.8	16.8	13.7	38.6	38.6	5.8	30.6	30.6
Actuated g/C Ratio	0.18	0.28	0.28	0.07	0.17	0.17	0.14	0.39	0.39	0.06	0.31	0.31
v/c Ratio	0.83	0.24	0.30	0.47	0.41	0.69	0.82	0.84	0.25	0.73	0.68	0.63
Control Delay (s/veh)	53.1	27.6	5.3	53.6	37.9	21.7	58.4	33.7	5.5	69.4	34.2	7.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	53.1	27.6	5.3	53.6	37.9	21.7	58.4	33.7	5.5	69.4	34.2	7.8
LOS	D	C	A	D	D	C	E	C	A	E	C	A
Approach Delay (s/veh)		37.8			33.3			35.7			29.3	
Approach LOS		D			C			D			C	

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 99.6	
Natural Cycle: 115	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.84	
Intersection Signal Delay (s/veh): 33.8	Intersection LOS: C
Intersection Capacity Utilization 73.9%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 3: La Sierra Av. & Indiana Av./S. Indiana Av.





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	462	212	149	101	223	266	354	1500	162	131	965	464
Future Volume (veh/h)	462	212	149	101	223	266	354	1500	162	131	965	464
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	508	233	111	111	245	212	389	1648	114	144	1060	285
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	576	1061	473	172	646	287	455	1950	604	203	1578	479
Arrive On Green	0.17	0.30	0.30	0.05	0.18	0.18	0.13	0.38	0.38	0.06	0.31	0.31
Sat Flow, veh/h	3456	3554	1583	3456	3554	1577	3456	5106	1583	3456	5106	1551
Grp Volume(v), veh/h	508	233	111	111	245	212	389	1648	114	144	1060	285
Grp Sat Flow(s),veh/h/ln	1728	1777	1583	1728	1777	1577	1728	1702	1583	1728	1702	1551
Q Serve(g_s), s	13.9	4.8	5.1	3.1	5.9	12.3	10.7	28.5	4.6	4.0	17.5	15.1
Cycle Q Clear(g_c), s	13.9	4.8	5.1	3.1	5.9	12.3	10.7	28.5	4.6	4.0	17.5	15.1
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	576	1061	473	172	646	287	455	1950	604	203	1578	479
V/C Ratio(X)	0.88	0.22	0.23	0.64	0.38	0.74	0.85	0.85	0.19	0.71	0.67	0.59
Avail Cap(c_a), veh/h	632	1773	790	264	1395	619	485	2014	624	203	1598	485
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	39.4	25.5	25.6	45.2	34.8	37.5	41.1	27.3	19.9	44.7	29.2	28.3
Incr Delay (d2), s/veh	12.1	0.1	0.3	1.5	0.4	3.7	12.4	3.6	0.2	9.3	1.2	2.3
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.6	2.0	1.9	1.3	2.5	4.9	5.1	11.2	1.7	1.9	6.9	5.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	51.5	25.6	25.9	46.7	35.2	41.2	53.5	30.9	20.1	54.0	30.4	30.7
LnGrp LOS	D	C	C	D	D	D	D	C	C	D	C	C
Approach Vol, veh/h		852			568			2151			1489	
Approach Delay, s/veh		41.1			39.7			34.4			32.7	
Approach LOS		D			D			C			C	

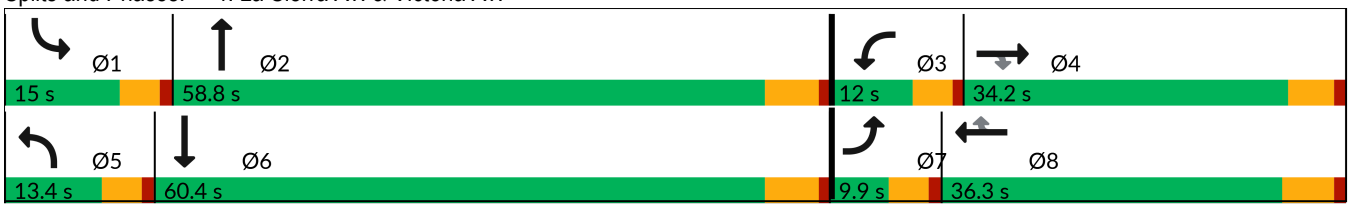
Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	10.3	42.8	9.4	34.3	17.4	35.7	20.7	23.0
Change Period (Y+Rc), s	4.6	5.8	4.6	5.4	4.6	5.8	4.6	5.4
Max Green Setting (Gmax), s	5.7	38.2	7.4	48.3	13.6	30.3	17.7	38.0
Max Q Clear Time (g_c+I1), s	6.0	30.5	5.1	7.1	12.7	19.5	15.9	14.3
Green Ext Time (p_c), s	0.0	6.5	0.0	1.8	0.1	7.0	0.2	2.1

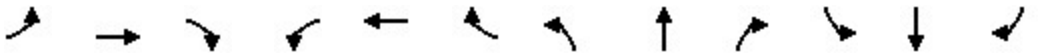
Intersection Summary	
HCM 7th Control Delay, s/veh	35.6
HCM 7th LOS	D

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations										
Traffic Volume (vph)	49	82	59	72	99	214	44	1138	112	570
Future Volume (vph)	49	82	59	72	99	214	44	1138	112	570
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Prot	NA
Protected Phases	7	4		3	8		5	2	1	6
Permitted Phases			4			8				
Detector Phase	7	4	4	3	8	8	5	2	1	6
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	5.0	10.0
Minimum Split (s)	9.6	34.1	34.1	9.6	33.8	33.8	9.6	35.8	9.6	33.8
Total Split (s)	9.9	34.2	34.2	12.0	36.3	36.3	13.4	58.8	15.0	60.4
Total Split (%)	8.3%	28.5%	28.5%	10.0%	30.3%	30.3%	11.2%	49.0%	12.5%	50.3%
Yellow Time (s)	3.6	4.1	4.1	3.6	4.8	4.8	3.6	4.8	3.6	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.1	5.1	4.6	5.8	5.8	4.6	5.8	4.6	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	None	Min
Act Effct Green (s)	5.3	13.8	13.8	7.2	17.1	17.1	7.1	52.3	10.0	57.4
Actuated g/C Ratio	0.05	0.13	0.13	0.07	0.17	0.17	0.07	0.51	0.10	0.55
v/c Ratio	0.32	0.37	0.22	0.66	0.36	0.59	0.41	0.86	0.74	0.34
Control Delay (s/veh)	55.2	45.0	3.2	74.7	42.9	18.5	58.8	29.1	72.6	14.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	55.2	45.0	3.2	74.7	42.9	18.5	58.8	29.1	72.6	14.9
LOS	E	D	A	E	D	B	E	C	E	B
Approach Delay (s/veh)		34.7			35.3			30.1		24.0
Approach LOS		C			D			C		C

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 103.5	
Natural Cycle: 110	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.86	
Intersection Signal Delay (s/veh): 29.6	Intersection LOS: C
Intersection Capacity Utilization 70.5%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 4: La Sierra Av. & Victoria Av.





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	49	82	59	72	99	214	44	1138	202	112	570	28
Future Volume (veh/h)	49	82	59	72	99	214	44	1138	202	112	570	28
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		0.98	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	55	92	27	81	111	155	49	1279	198	126	640	30
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	142	224	187	104	256	212	69	1568	241	156	1925	90
Arrive On Green	0.04	0.12	0.12	0.06	0.14	0.14	0.04	0.51	0.51	0.09	0.56	0.56
Sat Flow, veh/h	3456	1870	1563	1781	1870	1550	1781	3087	475	1781	3456	162
Grp Volume(v), veh/h	55	92	27	81	111	155	49	732	745	126	329	341
Grp Sat Flow(s),veh/h/ln	1728	1870	1563	1781	1870	1550	1781	1777	1785	1781	1777	1841
Q Serve(g_s), s	1.4	4.2	1.4	4.1	5.0	8.8	2.5	31.7	32.4	6.4	9.3	9.3
Cycle Q Clear(g_c), s	1.4	4.2	1.4	4.1	5.0	8.8	2.5	31.7	32.4	6.4	9.3	9.3
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.27	1.00		0.09
Lane Grp Cap(c), veh/h	142	224	187	104	256	212	69	903	907	156	990	1026
V/C Ratio(X)	0.39	0.41	0.14	0.78	0.43	0.73	0.71	0.81	0.82	0.81	0.33	0.33
Avail Cap(c_a), veh/h	199	592	495	143	620	514	170	1024	1029	201	1055	1093
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	43.0	37.5	36.3	42.7	36.4	38.1	43.7	18.9	19.1	41.2	11.1	11.1
Incr Delay (d2), s/veh	0.6	1.2	0.3	11.1	1.2	4.8	4.9	4.9	5.3	12.9	0.3	0.3
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	2.0	0.5	2.1	2.3	3.5	1.2	12.4	12.8	3.2	3.2	3.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	43.6	38.7	36.6	53.8	37.6	42.8	48.6	23.8	24.4	54.1	11.4	11.3
LnGrp LOS	D	D	D	D	D	D	D	C	C	D	B	B
Approach Vol, veh/h		174			347			1526			796	
Approach Delay, s/veh		39.9			43.7			24.9			18.1	
Approach LOS		D			D			C			B	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	12.7	52.5	10.0	16.8	8.2	57.0	8.4	18.4
Change Period (Y+Rc), s	4.6	5.8	4.6	* 5.8	4.6	5.8	4.6	5.8
Max Green Setting (Gmax), s	10.4	53.0	7.4	* 29	8.8	54.6	5.3	30.5
Max Q Clear Time (g_c+I1), s	8.4	34.4	6.1	6.2	4.5	11.3	3.4	10.8
Green Ext Time (p_c), s	0.0	12.3	0.0	0.5	0.0	6.2	0.0	1.0

Intersection Summary	
HCM 7th Control Delay, s/veh	26.2
HCM 7th LOS	C

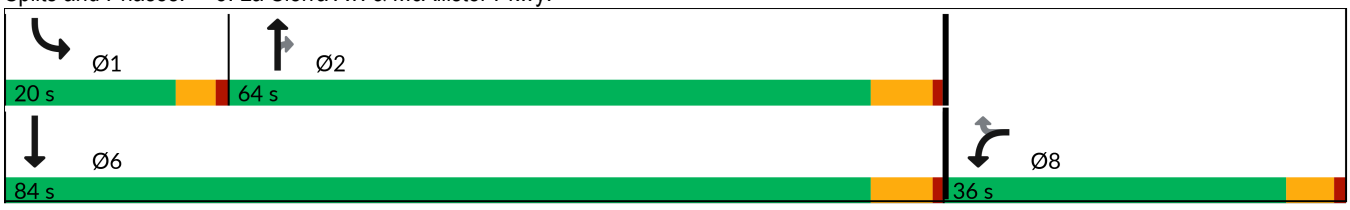
Notes

* HCM 7th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	19	242	1224	23	84	560
Future Volume (vph)	19	242	1224	23	84	560
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	10.0
Minimum Split (s)	33.4	33.4	27.5	27.5	9.6	16.5
Total Split (s)	36.0	36.0	64.0	64.0	20.0	84.0
Total Split (%)	30.0%	30.0%	53.3%	53.3%	16.7%	70.0%
Yellow Time (s)	4.4	4.4	5.5	5.5	3.6	5.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.4	6.5	6.5	4.6	6.5
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	Min	Min	None	Min
Act Effct Green (s)	13.3	13.3	42.7	42.7	9.5	54.0
Actuated g/C Ratio	0.17	0.17	0.53	0.53	0.12	0.67
v/c Ratio	0.07	0.58	0.73	0.03	0.45	0.26
Control Delay (s/veh)	33.9	12.0	18.6	6.8	45.6	5.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	33.9	12.0	18.6	6.8	45.6	5.6
LOS	C	B	B	A	D	A
Approach Delay (s/veh)	13.6		18.4			10.8
Approach LOS	B		B			B

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 80.1	
Natural Cycle: 90	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.73	
Intersection Signal Delay (s/veh): 15.6	Intersection LOS: B
Intersection Capacity Utilization 60.6%	ICU Level of Service B
Analysis Period (min) 15	

Splits and Phases: 5: La Sierra Av. & McAllister Pkwy.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	19	242	1224	23	84	560
Future Volume (veh/h)	19	242	1224	23	84	560
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	21	154	1375	23	94	629
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	248	221	1969	878	122	2447
Arrive On Green	0.14	0.14	0.55	0.55	0.07	0.69
Sat Flow, veh/h	1781	1585	3647	1585	1781	3647
Grp Volume(v), veh/h	21	154	1375	23	94	629
Grp Sat Flow(s),veh/h/ln	1781	1585	1777	1585	1781	1777
Q Serve(g_s), s	0.7	6.4	19.5	0.5	3.6	4.6
Cycle Q Clear(g_c), s	0.7	6.4	19.5	0.5	3.6	4.6
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	248	221	1969	878	122	2447
V/C Ratio(X)	0.08	0.70	0.70	0.03	0.77	0.26
Avail Cap(c_a), veh/h	787	701	2952	1317	396	3978
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	25.9	28.4	11.2	7.0	31.7	4.1
Incr Delay (d2), s/veh	0.1	3.9	0.6	0.0	3.9	0.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	2.5	5.3	0.1	1.5	0.8
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	26.1	32.3	11.9	7.0	35.6	4.2
LnGrp LOS	C	C	B	A	D	A
Approach Vol, veh/h	175		1398			723
Approach Delay, s/veh	31.6		11.8			8.2
Approach LOS	C		B			A

Timer - Assigned Phs	1	2				6	8
Phs Duration (G+Y+Rc), s	9.3	44.8				54.2	15.1
Change Period (Y+Rc), s	4.6	6.5				6.5	5.4
Max Green Setting (Gmax), s	15.4	57.5				77.5	30.6
Max Q Clear Time (g_c+I1), s	5.6	21.5				6.6	8.4
Green Ext Time (p_c), s	0.1	16.9				6.2	0.5

Intersection Summary	
HCM 7th Control Delay, s/veh	12.2
HCM 7th LOS	B

Intersection	
Intersection Delay, s/veh	27.4
Intersection LOS	D

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	72	548	115	24	264	135
Future Vol, veh/h	72	548	115	24	264	135
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	76	577	121	25	278	142
Number of Lanes	1	0	2	0	1	1

Approach	WB	NB	SB
Opposing Approach		SB	NB
Opposing Lanes	0	2	2
Conflicting Approach Left	NB		WB
Conflicting Lanes Left	2	0	1
Conflicting Approach Right	SB	WB	
Conflicting Lanes Right	2	1	0
HCM Control Delay, s/veh	38.4	11	16.1
HCM LOS	E	B	C

Lane	NBLn1	NBLn2	WBLn1	SBLn1	SBLn2
Vol Left, %	0%	0%	12%	100%	0%
Vol Thru, %	100%	61%	0%	0%	100%
Vol Right, %	0%	39%	88%	0%	0%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	77	62	620	264	135
LT Vol	0	0	72	264	0
Through Vol	77	38	0	0	135
RT Vol	0	24	548	0	0
Lane Flow Rate	81	66	653	278	142
Geometry Grp	5	5	2	5	5
Degree of Util (X)	0.16	0.125	0.914	0.551	0.262
Departure Headway (Hd)	7.135	6.858	5.041	7.141	6.629
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	500	520	718	503	540
Service Time	4.914	4.637	3.095	4.901	4.389
HCM Lane V/C Ratio	0.162	0.127	0.909	0.553	0.263
HCM Control Delay, s/veh	11.3	10.6	38.4	18.4	11.7
HCM Lane LOS	B	B	E	C	B
HCM 95th-tile Q	0.6	0.4	12.2	3.3	1

Intersection	
Intersection Delay, s/veh	12.2
Intersection LOS	B

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	52	256	541	96	51	74
Future Vol, veh/h	52	256	541	96	51	74
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	54	267	564	100	53	77
Number of Lanes	1	1	2	0	1	0

Approach	EB	WB	SB
Opposing Approach	WB	EB	
Opposing Lanes	2	2	0
Conflicting Approach Left	SB		WB
Conflicting Lanes Left	1	0	2
Conflicting Approach Right		SB	EB
Conflicting Lanes Right	0	1	2
HCM Control Delay, s/veh	11.7	12.8	10.2
HCM LOS	B	B	B

Lane	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1
Vol Left, %	100%	0%	0%	0%	41%
Vol Thru, %	0%	100%	100%	65%	0%
Vol Right, %	0%	0%	0%	35%	59%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	52	256	361	276	125
LT Vol	52	0	0	0	51
Through Vol	0	256	361	180	0
RT Vol	0	0	0	96	74
Lane Flow Rate	54	267	376	288	130
Geometry Grp	5	5	5	5	2
Degree of Util (X)	0.091	0.412	0.545	0.398	0.204
Departure Headway (Hd)	6.063	5.558	5.221	4.976	5.635
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	586	640	688	717	630
Service Time	3.854	3.349	2.994	2.749	3.725
HCM Lane V/C Ratio	0.092	0.417	0.547	0.402	0.206
HCM Control Delay, s/veh	9.5	12.2	14.2	11	10.2
HCM Lane LOS	A	B	B	B	B
HCM 95th-tile Q	0.3	2	3.3	1.9	0.8

Intersection	
Int Delay, s/veh	1.6

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	27	280	559	4	13	78
Future Vol, veh/h	27	280	559	4	13	78
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	1	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	29	304	608	4	14	85

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	612	0	-	0	973	610
Stage 1	-	-	-	-	610	-
Stage 2	-	-	-	-	363	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	967	-	-	-	280	495
Stage 1	-	-	-	-	542	-
Stage 2	-	-	-	-	704	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	967	-	-	-	271	495
Mov Cap-2 Maneuver	-	-	-	-	394	-
Stage 1	-	-	-	-	526	-
Stage 2	-	-	-	-	704	-

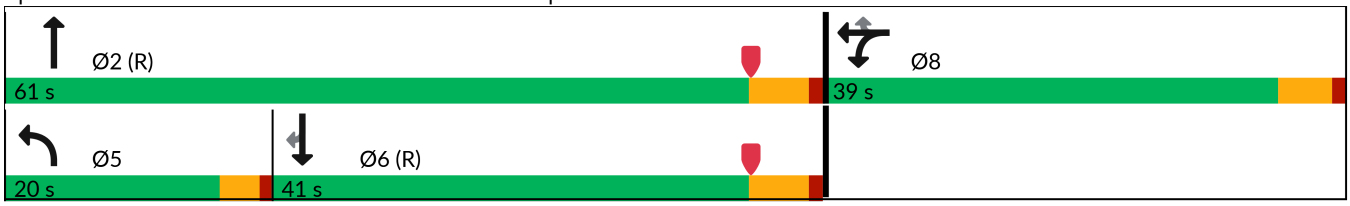
Approach	EB	WB	SB
HCM Control Delay, s/v	0.78	0	14.51
HCM LOS			B

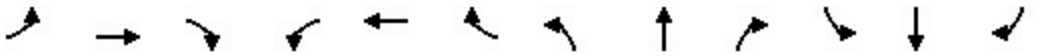
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	967	-	-	-	477
HCM Lane V/C Ratio	0.03	-	-	-	0.207
HCM Control Delay (s/veh)	8.8	-	-	-	14.5
HCM Lane LOS	A	-	-	-	B
HCM 95th %tile Q(veh)	0.1	-	-	-	0.8

Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Configurations							
Traffic Volume (vph)	504	2	555	406	962	1452	283
Future Volume (vph)	504	2	555	406	962	1452	283
Turn Type	Split	NA	Perm	Prot	NA	NA	Perm
Protected Phases	8	8		5	2	6	
Permitted Phases			8				6
Detector Phase	8	8	8	5	2	6	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	9.0	26.5	29.5	29.5
Total Split (s)	39.0	39.0	39.0	20.0	61.0	41.0	41.0
Total Split (%)	39.0%	39.0%	39.0%	20.0%	61.0%	41.0%	41.0%
Yellow Time (s)	4.0	4.0	4.0	3.0	4.5	4.5	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	4.0	5.5	5.5	5.5
Lead/Lag				Lead		Lag	Lag
Lead-Lag Optimize?				Yes		Yes	Yes
Recall Mode	None	None	None	None	C-Min	C-Min	C-Min
Act Effct Green (s)	28.5	28.5	28.5	15.3	61.0	41.7	41.7
Actuated g/C Ratio	0.29	0.29	0.29	0.15	0.61	0.42	0.42
v/c Ratio	0.79	0.74	0.69	0.81	0.32	0.70	0.38
Control Delay (s/veh)	45.3	33.1	28.8	39.6	23.4	27.4	9.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	45.3	33.1	28.8	39.6	23.4	27.4	9.4
LOS	D	C	C	D	C	C	A
Approach Delay (s/veh)		35.9			28.3	24.5	
Approach LOS		D			C	C	

Intersection Summary	
Cycle Length: 100	
Actuated Cycle Length: 100	
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow, Master Intersection	
Natural Cycle: 65	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.81	
Intersection Signal Delay (s/veh): 28.6	Intersection LOS: C
Intersection Capacity Utilization 73.9%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 1: La Sierra Av. & SR-91 WB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	504	2	555	406	962	0	0	1452	283
Future Volume (veh/h)	0	0	0	504	2	555	406	962	0	0	1452	283
Initial Q (Qb), veh				0	0	0	0	0	0	0	0	0
Lane Width Adj.				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach					No			No			No	
Adj Sat Flow, veh/h/ln				1870	1900	1900	1856	1885	0	0	1885	1900
Adj Flow Rate, veh/h				632	0	240	419	992	0	0	1497	206
Peak Hour Factor				0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %				2	0	0	3	1	0	0	1	0
Cap, veh/h				753	0	340	477	3519	0	0	2597	800
Arrive On Green				0.21	0.00	0.21	0.28	1.00	0.00	0.00	0.50	0.50
Sat Flow, veh/h				3563	0	1610	3428	5316	0	0	5316	1585
Grp Volume(v), veh/h				632	0	240	419	992	0	0	1497	206
Grp Sat Flow(s),veh/h/ln				1781	0	1610	1714	1716	0	0	1716	1585
Q Serve(g_s), s				17.0	0.0	13.8	11.7	0.0	0.0	0.0	20.3	7.4
Cycle Q Clear(g_c), s				17.0	0.0	13.8	11.7	0.0	0.0	0.0	20.3	7.4
Prop In Lane				1.00		1.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h				753	0	340	477	3519	0	0	2597	800
V/C Ratio(X)				0.84	0.00	0.71	0.88	0.28	0.00	0.00	0.58	0.26
Avail Cap(c_a), veh/h				1211	0	547	549	3519	0	0	2597	800
HCM Platoon Ratio				1.00	1.00	1.00	2.00	2.00	1.00	1.00	1.00	1.00
Upstream Filter(I)				1.00	0.00	1.00	0.73	0.73	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh				37.8	0.0	36.5	35.3	0.0	0.0	0.0	17.3	14.1
Incr Delay (d2), s/veh				2.4	0.0	2.0	9.6	0.1	0.0	0.0	0.9	0.8
Initial Q Delay(d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln				7.3	0.0	5.4	4.6	0.0	0.0	0.0	7.4	2.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh				40.2	0.0	38.5	44.9	0.1	0.0	0.0	18.2	14.9
LnGrp LOS				D		D	D	A			B	B
Approach Vol, veh/h					872			1411			1703	
Approach Delay, s/veh					39.7			13.4			17.8	
Approach LOS					D			B			B	

Timer - Assigned Phs	2		5	6		8						
Phs Duration (G+Y+Rc), s	73.9		17.9	56.0		26.1						
Change Period (Y+Rc), s	5.5		4.0	5.5		5.0						
Max Green Setting (Gmax), s	55.5		16.0	35.5		34.0						
Max Q Clear Time (g_c+I1), s	2.0		13.7	22.3		19.0						
Green Ext Time (p_c), s	7.8		0.2	8.3		2.1						

Intersection Summary		
HCM 7th Control Delay, s/veh	21.1	
HCM 7th LOS	C	

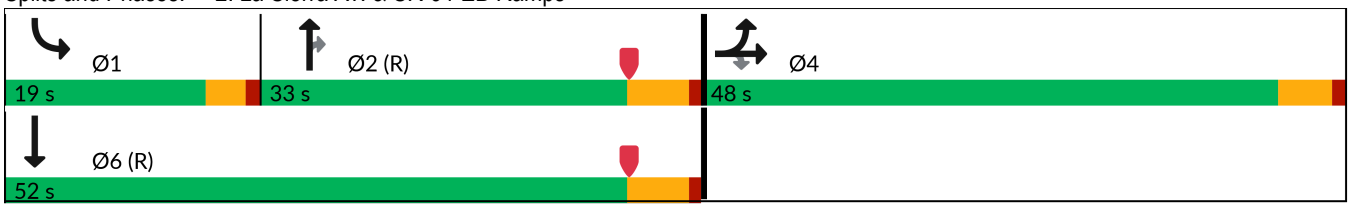
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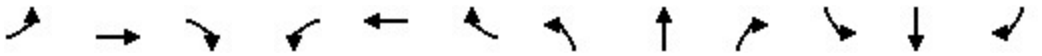
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	212	11	863	1142	318	430	1530
Future Volume (vph)	212	11	863	1142	318	430	1530
Turn Type	Split	NA	Perm	NA	Perm	Prot	NA
Protected Phases	4	4		2		1	6
Permitted Phases			4		2		
Detector Phase	4	4	4	2	2	1	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	29.5	29.5	9.0	26.5
Total Split (s)	48.0	48.0	48.0	33.0	33.0	19.0	52.0
Total Split (%)	48.0%	48.0%	48.0%	33.0%	33.0%	19.0%	52.0%
Yellow Time (s)	4.0	4.0	4.0	4.5	4.5	3.0	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.5	5.5	4.0	5.5
Lead/Lag				Lag	Lag	Lead	
Lead-Lag Optimize?				Yes	Yes	Yes	
Recall Mode	None	None	None	C-Min	C-Min	None	C-Min
Act Effct Green (s)	36.7	36.7	36.7	33.1	33.1	15.7	52.8
Actuated g/C Ratio	0.37	0.37	0.37	0.33	0.33	0.16	0.53
v/c Ratio	0.31	0.85	0.77	0.69	0.44	0.82	0.58
Control Delay (s/veh)	22.8	43.4	32.2	33.1	5.4	32.9	11.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.3
Total Delay (s/veh)	22.8	43.4	32.2	33.1	5.4	32.9	11.4
LOS	C	D	C	C	A	C	B
Approach Delay (s/veh)		35.2		27.1			16.1
Approach LOS		D		C			B

Intersection Summary	
Cycle Length: 100	
Actuated Cycle Length: 100	
Offset: 28 (28%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow	
Natural Cycle: 75	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.85	
Intersection Signal Delay (s/veh): 24.2	Intersection LOS: C
Intersection Capacity Utilization 73.9%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 2: La Sierra Av. & SR-91 EB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	212	11	863	0	0	0	0	1142	318	430	1530	0
Future Volume (veh/h)	212	11	863	0	0	0	0	1142	318	430	1530	0
Initial Q (Qb), veh	0	0	0				0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		0.98	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No						No			No	
Adj Sat Flow, veh/h/ln	1870	1752	1856				0	1870	1885	1870	1885	0
Adj Flow Rate, veh/h	148	0	726				0	1165	186	439	1561	0
Peak Hour Factor	0.98	0.98	0.98				0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	2	10	3				0	2	1	2	1	0
Cap, veh/h	473	0	836				0	2260	696	507	3239	0
Arrive On Green	0.27	0.00	0.27				0.00	0.44	0.44	0.05	0.21	0.00
Sat Flow, veh/h	1781	0	3145				0	5274	1572	3456	5316	0
Grp Volume(v), veh/h	148	0	726				0	1165	186	439	1561	0
Grp Sat Flow(s),veh/h/ln	1781	0	1572				0	1702	1572	1728	1716	0
Q Serve(g_s), s	6.7	0.0	22.0				0.0	16.5	7.5	12.6	26.7	0.0
Cycle Q Clear(g_c), s	6.7	0.0	22.0				0.0	16.5	7.5	12.6	26.7	0.0
Prop In Lane	1.00		1.00				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	473	0	836				0	2260	696	507	3239	0
V/C Ratio(X)	0.31	0.00	0.87				0.00	0.52	0.27	0.87	0.48	0.00
Avail Cap(c_a), veh/h	766	0	1352				0	2260	696	518	3239	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	0.33	0.33	1.00
Upstream Filter(I)	1.00	0.00	1.00				0.00	0.72	0.72	0.63	0.63	0.00
Uniform Delay (d), s/veh	29.4	0.0	35.1				0.0	20.1	17.6	46.6	25.3	0.0
Incr Delay (d2), s/veh	0.3	0.0	3.0				0.0	0.6	0.7	9.3	0.3	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.8	0.0	8.3				0.0	6.1	2.6	6.4	12.2	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	29.7	0.0	38.1				0.0	20.7	18.3	55.9	25.6	0.0
LnGrp LOS	C		D					C	B	E	C	
Approach Vol, veh/h		874						1351			2000	
Approach Delay, s/veh		36.6						20.4			32.2	
Approach LOS		D						C			C	

Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	18.7	49.8		31.6		68.4
Change Period (Y+Rc), s	4.0	5.5		5.0		5.5
Max Green Setting (Gmax), s	15.0	27.5		43.0		46.5
Max Q Clear Time (g_c+I1), s	14.6	18.5		24.0		28.7
Green Ext Time (p_c), s	0.1	5.1		2.5		9.9

Intersection Summary	
HCM 7th Control Delay, s/veh	29.4
HCM 7th LOS	C

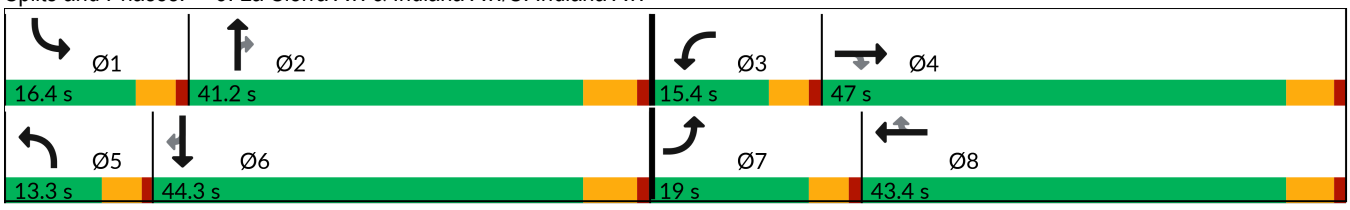
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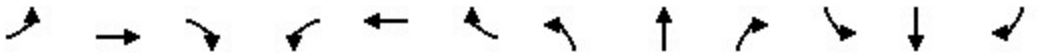
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	392	364	397	168	201	258	232	876	120	299	1584	396
Future Volume (vph)	392	364	397	168	201	258	232	876	120	299	1584	396
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4			8			2			6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	9.6	43.4	43.4	9.6	43.4	43.4	9.6	37.8	37.8	9.6	32.8	32.8
Total Split (s)	19.0	47.0	47.0	15.4	43.4	43.4	13.3	41.2	41.2	16.4	44.3	44.3
Total Split (%)	15.8%	39.2%	39.2%	12.8%	36.2%	36.2%	11.1%	34.3%	34.3%	13.7%	36.9%	36.9%
Yellow Time (s)	3.6	4.4	4.4	3.6	4.4	4.4	3.6	4.8	4.8	3.6	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.4	5.4	4.6	5.4	5.4	4.6	5.8	5.8	4.6	5.8	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	14.5	26.4	26.4	9.2	21.2	21.2	8.8	35.7	35.7	11.9	38.9	38.9
Actuated g/C Ratio	0.14	0.25	0.25	0.09	0.20	0.20	0.08	0.34	0.34	0.11	0.37	0.37
v/c Ratio	0.89	0.44	0.81	0.60	0.30	0.61	0.87	0.54	0.21	0.83	0.91	0.51
Control Delay (s/veh)	66.9	33.5	32.5	55.9	34.9	18.3	77.3	30.4	6.1	64.9	40.2	5.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	66.9	33.5	32.5	55.9	34.9	18.3	77.3	30.4	6.1	64.9	40.2	5.1
LOS	E	C	C	E	C	B	E	C	A	E	D	A
Approach Delay (s/veh)		44.5			33.7			36.9			37.3	
Approach LOS		D			C			D			D	

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 103.9	
Natural Cycle: 115	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.91	
Intersection Signal Delay (s/veh): 38.4	Intersection LOS: D
Intersection Capacity Utilization 76.0%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 3: La Sierra Av. & Indiana Av./S. Indiana Av.





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	392	364	397	168	201	258	232	876	120	299	1584	396
Future Volume (veh/h)	392	364	397	168	201	258	232	876	120	299	1584	396
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		0.98	1.00		0.99	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	426	396	291	183	218	221	252	952	76	325	1722	332
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	485	964	420	249	721	316	293	1765	545	387	1903	577
Arrive On Green	0.14	0.27	0.27	0.07	0.20	0.20	0.08	0.35	0.35	0.11	0.37	0.37
Sat Flow, veh/h	3456	3554	1549	3456	3554	1557	3456	5106	1577	3456	5106	1548
Grp Volume(v), veh/h	426	396	291	183	218	221	252	952	76	325	1722	332
Grp Sat Flow(s),veh/h/ln	1728	1777	1549	1728	1777	1557	1728	1702	1577	1728	1702	1548
Q Serve(g_s), s	12.4	9.4	17.3	5.3	5.3	13.5	7.4	15.4	3.4	9.4	32.7	17.5
Cycle Q Clear(g_c), s	12.4	9.4	17.3	5.3	5.3	13.5	7.4	15.4	3.4	9.4	32.7	17.5
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	485	964	420	249	721	316	293	1765	545	387	1903	577
V/C Ratio(X)	0.88	0.41	0.69	0.74	0.30	0.70	0.86	0.54	0.14	0.84	0.90	0.58
Avail Cap(c_a), veh/h	486	1443	629	364	1318	578	293	1765	545	398	1919	582
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	43.2	30.6	33.5	46.6	34.7	37.9	46.3	27.0	23.0	44.6	30.4	25.7
Incr Delay (d2), s/veh	16.0	0.3	2.1	1.7	0.2	2.8	20.8	0.4	0.2	13.6	6.7	1.7
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.2	3.9	6.5	2.3	2.3	5.2	3.9	5.9	1.2	4.6	13.5	6.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	59.2	30.9	35.6	48.3	34.9	40.8	67.1	27.4	23.2	58.2	37.1	27.4
LnGrp LOS	E	C	D	D	C	D	E	C	C	E	D	C
Approach Vol, veh/h		1113			622			1280			2379	
Approach Delay, s/veh		42.9			40.9			34.9			38.6	
Approach LOS		D			D			C			D	

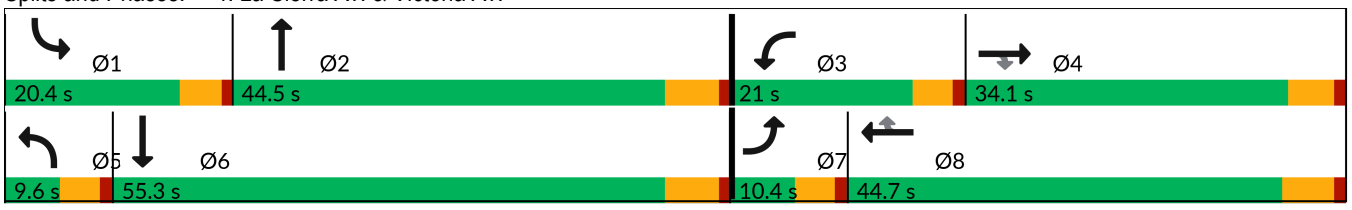
Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	16.1	41.2	12.0	33.2	13.3	44.0	19.0	26.2
Change Period (Y+Rc), s	4.6	5.8	4.6	5.4	4.6	5.8	4.6	5.4
Max Green Setting (Gmax), s	11.8	35.4	10.8	41.6	8.7	38.5	14.4	38.0
Max Q Clear Time (g_c+I1), s	11.4	17.4	7.3	19.3	9.4	34.7	14.4	15.5
Green Ext Time (p_c), s	0.0	8.1	0.1	3.5	0.0	3.5	0.0	2.0

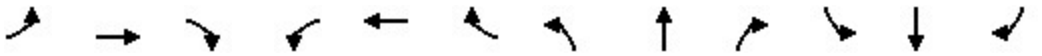
Intersection Summary	
HCM 7th Control Delay, s/veh	38.9
HCM 7th LOS	D

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations										
Traffic Volume (vph)	62	110	156	283	80	183	29	786	201	1533
Future Volume (vph)	62	110	156	283	80	183	29	786	201	1533
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Prot	NA
Protected Phases	7	4		3	8		5	2	1	6
Permitted Phases			4			8				
Detector Phase	7	4	4	3	8	8	5	2	1	6
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	5.0	10.0
Minimum Split (s)	9.6	34.1	34.1	9.6	33.8	33.8	9.6	35.8	9.6	33.8
Total Split (s)	10.4	34.1	34.1	21.0	44.7	44.7	9.6	44.5	20.4	55.3
Total Split (%)	8.7%	28.4%	28.4%	17.5%	37.3%	37.3%	8.0%	37.1%	17.0%	46.1%
Yellow Time (s)	3.6	4.1	4.1	3.6	4.8	4.8	3.6	4.8	3.6	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.1	5.1	4.6	5.8	5.8	4.6	5.8	4.6	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	None	Min
Act Effct Green (s)	5.7	14.4	14.4	16.5	26.7	26.7	5.0	36.4	15.3	50.9
Actuated g/C Ratio	0.06	0.14	0.14	0.16	0.26	0.26	0.05	0.35	0.15	0.49
v/c Ratio	0.35	0.45	0.43	1.06	0.18	0.35	0.36	0.80	0.81	0.98
Control Delay (s/veh)	55.3	46.3	7.0	113.5	32.3	6.3	63.0	35.9	67.9	44.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	55.3	46.3	7.0	113.5	32.3	6.3	63.0	35.9	67.9	44.2
LOS	E	D	A	F	C	A	E	D	E	D
Approach Delay (s/veh)		29.3			65.6			36.8		46.9
Approach LOS		C			E			D		D

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 102.9	
Natural Cycle: 120	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 1.06	
Intersection Signal Delay (s/veh): 45.5	Intersection LOS: D
Intersection Capacity Utilization 84.5%	ICU Level of Service E
Analysis Period (min) 15	

Splits and Phases: 4: La Sierra Av. & Victoria Av.





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	62	110	156	283	80	183	29	786	137	201	1533	67
Future Volume (veh/h)	62	110	156	283	80	183	29	786	137	201	1533	67
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		0.99	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	66	117	76	301	85	115	31	836	93	214	1631	34
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	147	191	159	298	424	354	52	1226	136	246	1742	36
Arrive On Green	0.04	0.10	0.10	0.17	0.23	0.23	0.03	0.57	0.38	0.14	0.73	0.49
Sat Flow, veh/h	3456	1870	1559	1781	1870	1563	1781	3224	359	1781	3560	74
Grp Volume(v), veh/h	66	117	76	301	85	115	31	461	468	214	813	852
Grp Sat Flow(s),veh/h/ln	1728	1870	1559	1781	1870	1563	1781	1777	1806	1781	1777	1857
Q Serve(g_s), s	1.8	5.9	4.5	16.4	3.6	6.0	1.7	17.9	18.7	11.5	38.0	38.7
Cycle Q Clear(g_c), s	1.8	5.9	4.5	16.4	3.6	6.0	1.7	17.9	18.7	11.5	38.0	38.7
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.20	1.00		0.04
Lane Grp Cap(c), veh/h	147	191	159	298	424	354	52	676	687	246	870	909
V/C Ratio(X)	0.45	0.61	0.48	1.01	0.20	0.32	0.60	0.68	0.68	0.87	0.93	0.94
Avail Cap(c_a), veh/h	204	553	461	298	742	620	91	701	713	287	897	938
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.50	1.00	1.00	1.50	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	45.8	42.2	41.6	40.8	30.7	31.6	47.0	16.9	18.6	41.4	11.7	12.2
Incr Delay (d2), s/veh	0.8	3.2	2.2	54.7	0.2	0.5	4.1	2.9	2.9	19.3	16.5	16.4
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.8	2.8	1.8	11.4	1.6	2.2	0.8	5.7	6.4	6.2	9.7	10.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	46.6	45.3	43.8	95.5	30.9	32.2	51.1	19.8	21.5	60.7	28.1	28.7
LnGrp LOS	D	D	D	F	C	C	D	B	C	E	C	C
Approach Vol, veh/h		259			501			960			1879	
Approach Delay, s/veh		45.2			70.0			21.6			32.1	
Approach LOS		D			E			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	18.2	43.1	21.0	15.8	7.5	53.8	8.8	28.0
Change Period (Y+Rc), s	4.6	5.8	4.6	* 5.8	4.6	5.8	4.6	5.8
Max Green Setting (Gmax), s	15.8	38.7	16.4	* 29	5.0	49.5	5.8	38.9
Max Q Clear Time (g_c+I1), s	13.5	20.7	18.4	7.9	3.7	40.7	3.8	8.0
Green Ext Time (p_c), s	0.1	7.2	0.0	0.8	0.0	7.3	0.0	0.8

Intersection Summary	
HCM 7th Control Delay, s/veh	35.5
HCM 7th LOS	D

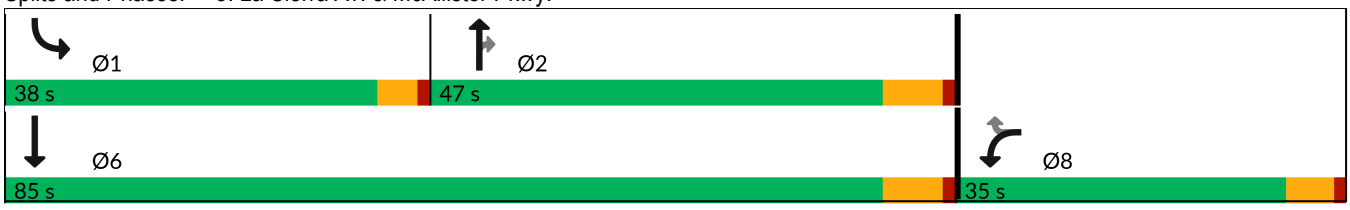
Notes

* HCM 7th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	8	159	793	13	288	1652
Future Volume (vph)	8	159	793	13	288	1652
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	10.0
Minimum Split (s)	33.4	33.4	27.5	27.5	9.6	16.5
Total Split (s)	35.0	35.0	47.0	47.0	38.0	85.0
Total Split (%)	29.2%	29.2%	39.2%	39.2%	31.7%	70.8%
Yellow Time (s)	4.4	4.4	5.5	5.5	3.6	5.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.4	6.5	6.5	4.6	6.5
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	Min	Min	None	Min
Act Effct Green (s)	13.0	13.0	27.5	27.5	19.8	52.2
Actuated g/C Ratio	0.17	0.17	0.35	0.35	0.25	0.67
v/c Ratio	0.03	0.42	0.68	0.03	0.68	0.74
Control Delay (s/veh)	32.6	9.2	25.8	11.5	36.5	11.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	32.6	9.2	25.8	11.5	36.5	11.0
LOS	C	A	C	B	D	B
Approach Delay (s/veh)	10.4		25.6			14.8
Approach LOS	B		C			B

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 77.9	
Natural Cycle: 80	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.74	
Intersection Signal Delay (s/veh): 17.5	Intersection LOS: B
Intersection Capacity Utilization 63.9%	ICU Level of Service B
Analysis Period (min) 15	

Splits and Phases: 5: La Sierra Av. & McAllister Pkwy.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	8	159	793	13	288	1652
Future Volume (veh/h)	8	159	793	13	288	1652
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		0.98	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	9	63	844	14	306	1757
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	191	170	1638	715	355	2576
Arrive On Green	0.11	0.11	0.46	0.46	0.20	0.72
Sat Flow, veh/h	1781	1585	3647	1552	1781	3647
Grp Volume(v), veh/h	9	63	844	14	306	1757
Grp Sat Flow(s),veh/h/ln	1781	1585	1777	1552	1781	1777
Q Serve(g_s), s	0.3	2.6	11.9	0.3	11.8	19.0
Cycle Q Clear(g_c), s	0.3	2.6	11.9	0.3	11.8	19.0
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	191	170	1638	715	355	2576
V/C Ratio(X)	0.05	0.37	0.52	0.02	0.86	0.68
Avail Cap(c_a), veh/h	745	663	2033	888	841	3941
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	28.4	29.4	13.5	10.4	27.4	5.3
Incr Delay (d2), s/veh	0.1	1.3	0.4	0.0	2.5	0.5
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.1	1.0	3.7	0.1	4.6	2.6
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	28.5	30.7	13.8	10.4	29.9	5.8
LnGrp LOS	C	C	B	B	C	A
Approach Vol, veh/h	72		858			2063
Approach Delay, s/veh	30.5		13.8			9.3
Approach LOS	C		B			A

Timer - Assigned Phs	1	2			6	8
Phs Duration (G+Y+Rc), s	18.7	39.1			57.8	13.0
Change Period (Y+Rc), s	4.6	6.5			6.5	5.4
Max Green Setting (Gmax), s	33.4	40.5			78.5	29.6
Max Q Clear Time (g_c+I1), s	13.8	13.9			21.0	4.6
Green Ext Time (p_c), s	0.4	7.9			30.3	0.2

Intersection Summary	
HCM 7th Control Delay, s/veh	11.1
HCM 7th LOS	B

Intersection	
Intersection Delay, s/veh	135.5
Intersection LOS	F

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	82	366	235	99	728	503
Future Vol, veh/h	82	366	235	99	728	503
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	88	394	253	106	783	541
Number of Lanes	1	0	2	0	1	1

Approach	WB	NB	SB
Opposing Approach		SB	NB
Opposing Lanes	0	2	2
Conflicting Approach Left	NB		WB
Conflicting Lanes Left	2	0	1
Conflicting Approach Right	SB	WB	
Conflicting Lanes Right	2	1	0
HCM Control Delay, s/veh	30.6	14.8	206.4
HCM LOS	D	B	F

Lane	NBLn1	NBLn2	WBLn1	SBLn1	SBLn2
Vol Left, %	0%	0%	18%	100%	0%
Vol Thru, %	100%	44%	0%	0%	100%
Vol Right, %	0%	56%	82%	0%	0%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	157	177	448	728	503
LT Vol	0	0	82	728	0
Through Vol	157	78	0	0	503
RT Vol	0	99	366	0	0
Lane Flow Rate	168	191	482	783	541
Geometry Grp	5	5	2	5	5
Degree of Util (X)	0.352	0.378	0.815	1.6	1.029
Departure Headway (Hd)	7.916	7.513	6.09	7.36	6.847
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	457	482	593	501	530
Service Time	5.616	5.213	4.132	5.108	4.595
HCM Lane V/C Ratio	0.368	0.396	0.813	1.563	1.021
HCM Control Delay, s/veh	14.9	14.7	30.6	298.6	73
HCM Lane LOS	B	B	D	F	F
HCM 95th-tile Q	1.6	1.7	8.2	43.2	15.2

Intersection	
Intersection Delay, s/veh	86.6
Intersection LOS	F

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	64	764	350	32	38	111
Future Vol, veh/h	64	764	350	32	38	111
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	68	813	372	34	40	118
Number of Lanes	1	1	2	0	1	0

Approach	EB	WB	SB
Opposing Approach	WB	EB	
Opposing Lanes	2	2	0
Conflicting Approach Left	SB		WB
Conflicting Lanes Left	1	0	2
Conflicting Approach Right		SB	EB
Conflicting Lanes Right	0	1	2
HCM Control Delay, s/veh	54.5	11.9	11.8
HCM LOS	F	B	B

Lane	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1
Vol Left, %	100%	0%	0%	0%	26%
Vol Thru, %	0%	100%	100%	78%	0%
Vol Right, %	0%	0%	0%	22%	74%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	64	764	233	149	149
LT Vol	64	0	0	0	38
Through Vol	0	764	233	117	0
RT Vol	0	0	0	32	111
Lane Flow Rate	68	813	248	158	159
Geometry Grp	5	5	5	5	2
Degree of Util (X)	0.113	1.253	0.396	0.246	0.266
Departure Headway (Hd)	5.956	5.551	6.084	5.93	6.503
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	596	658	596	609	556
Service Time	3.756	3.251	3.784	3.63	4.503
HCM Lane V/C Ratio	0.114	1.236	0.416	0.259	0.286
HCM Control Delay, s/veh	9.5	145	12.7	10.6	11.8
HCM Lane LOS	A	F	B	B	B
HCM 95th-tile Q	0.4	30.5	1.9	1	1.1

Intersection	
Int Delay, s/veh	1.2

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	89	713	330	15	9	53
Future Vol, veh/h	89	713	330	15	9	53
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	1	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	97	775	359	16	10	58

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	375	0	-	0	1335	367
Stage 1	-	-	-	-	367	-
Stage 2	-	-	-	-	968	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1183	-	-	-	169	678
Stage 1	-	-	-	-	701	-
Stage 2	-	-	-	-	368	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	1183	-	-	-	156	678
Mov Cap-2 Maneuver	-	-	-	-	277	-
Stage 1	-	-	-	-	644	-
Stage 2	-	-	-	-	368	-

Approach	EB	WB	SB
HCM Control Delay, s/v	0.92	0	12.3
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1183	-	-	-	561
HCM Lane V/C Ratio	0.082	-	-	-	0.12
HCM Control Delay (s/veh)	8.3	-	-	-	12.3
HCM Lane LOS	A	-	-	-	B
HCM 95th %tile Q(veh)	0.3	-	-	-	0.4

APPENDIX 5.2: EAP (2027) (PROJECT PLUS HIGHLAND GROVE II PLUS GREENTREE) CONDITIONS INTERSECTION OPERATIONS ANALYSIS WORKSHEETS

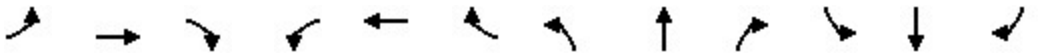
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Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Configurations							
Traffic Volume (vph)	306	0	489	687	1414	992	141
Future Volume (vph)	306	0	489	687	1414	992	141
Turn Type	Split	NA	Perm	Prot	NA	NA	Perm
Protected Phases	8	8		5	2	6	
Permitted Phases			8				6
Detector Phase	8	8	8	5	2	6	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	9.0	26.5	29.5	29.5
Total Split (s)	37.0	37.0	37.0	30.0	63.0	33.0	33.0
Total Split (%)	37.0%	37.0%	37.0%	30.0%	63.0%	33.0%	33.0%
Yellow Time (s)	4.0	4.0	4.0	3.0	4.5	4.5	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	4.0	5.5	5.5	5.5
Lead/Lag				Lead		Lag	Lag
Lead-Lag Optimize?				Yes		Yes	Yes
Recall Mode	None	None	None	None	C-Min	C-Min	C-Min
Act Effct Green (s)	22.9	22.9	22.9	24.1	66.6	38.5	38.5
Actuated g/C Ratio	0.23	0.23	0.23	0.24	0.67	0.39	0.39
v/c Ratio	0.74	0.69	0.66	0.86	0.43	0.53	0.22
Control Delay (s/veh)	47.1	34.0	32.6	41.8	16.7	26.8	7.5
Queue Delay	0.0	0.0	0.0	0.0	0.5	0.0	0.0
Total Delay (s/veh)	47.1	34.0	32.6	41.8	17.2	26.8	7.5
LOS	D	C	C	D	B	C	A
Approach Delay (s/veh)		38.1			25.3	24.4	
Approach LOS		D			C	C	

Intersection Summary	
Cycle Length: 100	
Actuated Cycle Length: 100	
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow, Master Intersection	
Natural Cycle: 65	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.86	
Intersection Signal Delay (s/veh): 27.5	Intersection LOS: C
Intersection Capacity Utilization 70.4%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 1: La Sierra Av. & SR-91 WB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	306	0	489	687	1414	0	0	992	141
Future Volume (veh/h)	0	0	0	306	0	489	687	1414	0	0	992	141
Initial Q (Qb), veh				0	0	0	0	0	0	0	0	0
Lane Width Adj.				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach					No			No			No	
Adj Sat Flow, veh/h/ln				1856	1900	1885	1870	1870	0	0	1856	1826
Adj Flow Rate, veh/h				420	0	217	708	1458	0	0	1023	102
Peak Hour Factor				0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %				3	0	1	2	2	0	0	3	5
Cap, veh/h				587	0	266	764	3721	0	0	2369	712
Arrive On Green				0.17	0.00	0.17	0.44	1.00	0.00	0.00	0.47	0.47
Sat Flow, veh/h				3534	0	1598	3456	5274	0	0	5233	1522
Grp Volume(v), veh/h				420	0	217	708	1458	0	0	1023	102
Grp Sat Flow(s),veh/h/ln				1767	0	1598	1728	1702	0	0	1689	1522
Q Serve(g_s), s				11.2	0.0	13.1	19.4	0.0	0.0	0.0	13.5	3.8
Cycle Q Clear(g_c), s				11.2	0.0	13.1	19.4	0.0	0.0	0.0	13.5	3.8
Prop In Lane				1.00		1.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h				587	0	266	764	3721	0	0	2369	712
V/C Ratio(X)				0.71	0.00	0.82	0.93	0.39	0.00	0.00	0.43	0.14
Avail Cap(c_a), veh/h				1131	0	511	898	3721	0	0	2369	712
HCM Platoon Ratio				1.00	1.00	1.00	2.00	2.00	1.00	1.00	1.00	1.00
Upstream Filter(I)				1.00	0.00	1.00	0.62	0.62	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh				39.4	0.0	40.2	27.1	0.0	0.0	0.0	17.8	15.2
Incr Delay (d2), s/veh				1.2	0.0	4.6	8.8	0.2	0.0	0.0	0.6	0.4
Initial Q Delay(d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln				4.8	0.0	5.3	6.3	0.1	0.0	0.0	4.9	1.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh				40.7	0.0	44.8	36.0	0.2	0.0	0.0	18.3	15.6
LnGrp LOS				D		D	D	A			B	B
Approach Vol, veh/h					637			2166			1125	
Approach Delay, s/veh					42.1			11.9			18.1	
Approach LOS					D			B			B	

Timer - Assigned Phs	2		5	6		8						
Phs Duration (G+Y+Rc), s	78.4		26.1	52.3		21.6						
Change Period (Y+Rc), s	5.5		4.0	5.5		5.0						
Max Green Setting (Gmax), s	57.5		26.0	27.5		32.0						
Max Q Clear Time (g_c+I1), s	2.0		21.4	15.5		15.1						
Green Ext Time (p_c), s	14.1		0.8	5.4		1.5						

Intersection Summary		
HCM 7th Control Delay, s/veh	18.6	
HCM 7th LOS	B	

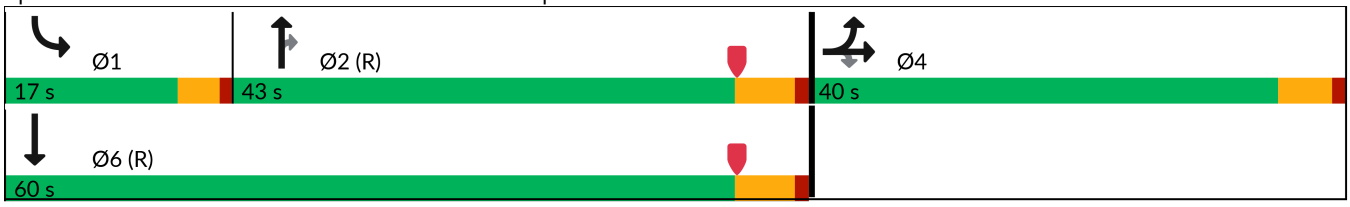
Notes
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	435	1	437	1615	522	320	974
Future Volume (vph)	435	1	437	1615	522	320	974
Turn Type	Split	NA	Perm	NA	Perm	Prot	NA
Protected Phases	4	4		2		1	6
Permitted Phases			4		2		
Detector Phase	4	4	4	2	2	1	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	29.5	29.5	9.0	26.5
Total Split (s)	40.0	40.0	40.0	43.0	43.0	17.0	60.0
Total Split (%)	40.0%	40.0%	40.0%	43.0%	43.0%	17.0%	60.0%
Yellow Time (s)	4.0	4.0	4.0	4.5	4.5	3.0	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.5	5.5	4.0	5.5
Lead/Lag				Lag	Lag	Lead	
Lead-Lag Optimize?				Yes	Yes	Yes	
Recall Mode	None	None	None	C-Min	C-Min	None	C-Min
Act Effct Green (s)	25.1	25.1	25.1	46.6	46.6	13.8	64.4
Actuated g/C Ratio	0.25	0.25	0.25	0.47	0.47	0.14	0.64
v/c Ratio	0.74	0.72	0.67	0.72	0.54	0.71	0.31
Control Delay (s/veh)	44.7	35.1	29.7	25.3	4.2	50.8	2.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	44.7	35.1	29.7	25.3	4.2	50.8	2.5
LOS	D	D	C	C	A	D	A
Approach Delay (s/veh)		36.7		20.2			14.4
Approach LOS		D		C			B

Intersection Summary

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 28 (28%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow
Natural Cycle: 60
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.74
Intersection Signal Delay (s/veh): 21.8 — Intersection LOS: C
Intersection Capacity Utilization 70.4% — ICU Level of Service C
Analysis Period (min) 15

Splits and Phases: 2: La Sierra Av. & SR-91 EB Ramps



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	435	1	437	0	0	0	0	1615	522	320	974	0
Future Volume (veh/h)	435	1	437	0	0	0	0	1615	522	320	974	0
Initial Q (Qb), veh	0	0	0				0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No						No			No	
Adj Sat Flow, veh/h/ln	1885	1900	1811				0	1856	1856	1841	1856	0
Adj Flow Rate, veh/h	524	0	152				0	1682	392	333	1015	0
Peak Hour Factor	0.96	0.96	0.96				0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	1	0	6				0	3	3	4	3	0
Cap, veh/h	637	0	272				0	2833	878	403	3636	0
Arrive On Green	0.18	0.00	0.18				0.00	0.56	0.56	0.04	0.24	0.00
Sat Flow, veh/h	3591	0	1535				0	5233	1570	3401	5233	0
Grp Volume(v), veh/h	524	0	152				0	1682	392	333	1015	0
Grp Sat Flow(s),veh/h/ln	1795	0	1535				0	1689	1570	1700	1689	0
Q Serve(g_s), s	14.1	0.0	9.0				0.0	21.9	14.7	9.7	16.4	0.0
Cycle Q Clear(g_c), s	14.1	0.0	9.0				0.0	21.9	14.7	9.7	16.4	0.0
Prop In Lane	1.00		1.00				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	637	0	272				0	2833	878	403	3636	0
V/C Ratio(X)	0.82	0.00	0.56				0.00	0.59	0.45	0.83	0.28	0.00
Avail Cap(c_a), veh/h	1257	0	537				0	2833	878	442	3636	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	0.33	0.33	1.00
Upstream Filter(I)	1.00	0.00	1.00				0.00	0.43	0.43	0.79	0.79	0.00
Uniform Delay (d), s/veh	39.6	0.0	37.6				0.0	14.5	12.9	47.0	17.0	0.0
Incr Delay (d2), s/veh	2.1	0.0	1.3				0.0	0.4	0.7	8.8	0.2	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.1	0.0	3.4				0.0	7.4	4.7	4.7	7.3	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	41.7	0.0	38.9				0.0	14.9	13.7	55.8	17.2	0.0
LnGrp LOS	D		D					B	B	E	B	
Approach Vol, veh/h		676						2074			1348	
Approach Delay, s/veh		41.1						14.7			26.7	
Approach LOS		D						B			C	

Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	15.9	61.4		22.7		77.3
Change Period (Y+Rc), s	4.0	5.5		5.0		5.5
Max Green Setting (Gmax), s	13.0	37.5		35.0		54.5
Max Q Clear Time (g_c+I1), s	11.7	23.9		16.1		18.4
Green Ext Time (p_c), s	0.1	9.8		1.7		7.8

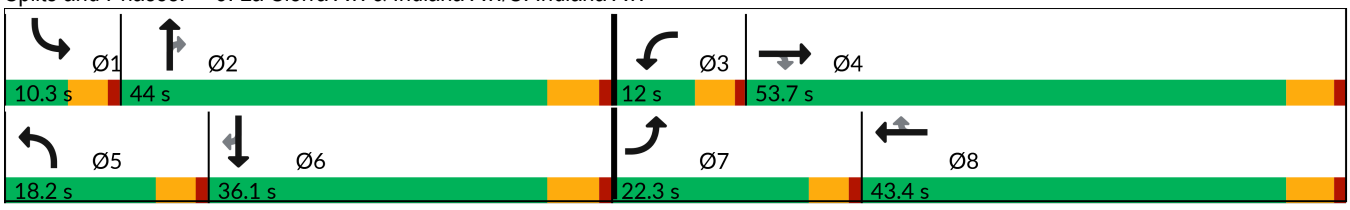
Intersection Summary	
HCM 7th Control Delay, s/veh	23.0
HCM 7th LOS	C

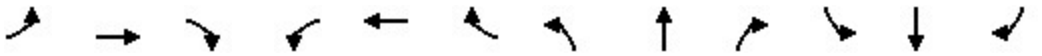
Notes
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	462	212	153	104	223	266	365	1563	171	131	988	464
Future Volume (vph)	462	212	153	104	223	266	365	1563	171	131	988	464
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4			8			2			6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	9.6	43.4	43.4	9.6	43.4	43.4	9.6	37.8	37.8	9.6	32.8	32.8
Total Split (s)	22.3	53.7	53.7	12.0	43.4	43.4	18.2	44.0	44.0	10.3	36.1	36.1
Total Split (%)	18.6%	44.8%	44.8%	10.0%	36.2%	36.2%	15.2%	36.7%	36.7%	8.6%	30.1%	30.1%
Yellow Time (s)	3.6	4.4	4.4	3.6	4.4	4.4	3.6	4.8	4.8	3.6	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.4	5.4	4.6	5.4	5.4	4.6	5.8	5.8	4.6	5.8	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	17.9	27.8	27.8	6.9	16.8	16.8	13.7	38.6	38.6	5.8	30.6	30.6
Actuated g/C Ratio	0.18	0.28	0.28	0.07	0.17	0.17	0.14	0.39	0.39	0.06	0.31	0.31
v/c Ratio	0.83	0.24	0.30	0.48	0.41	0.69	0.85	0.87	0.26	0.73	0.70	0.63
Control Delay (s/veh)	53.1	27.6	5.2	53.9	37.9	21.7	60.6	35.4	6.0	69.4	34.5	7.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	53.1	27.6	5.2	53.9	37.9	21.7	60.6	35.4	6.0	69.4	34.5	7.8
LOS	D	C	A	D	D	C	E	D	A	E	C	A
Approach Delay (s/veh)		37.7			33.4			37.4			29.6	
Approach LOS		D			C			D			C	

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 99.6	
Natural Cycle: 115	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.87	
Intersection Signal Delay (s/veh): 34.6	Intersection LOS: C
Intersection Capacity Utilization 75.1%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 3: La Sierra Av. & Indiana Av./S. Indiana Av.





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	462	212	153	104	223	266	365	1563	171	131	988	464
Future Volume (veh/h)	462	212	153	104	223	266	365	1563	171	131	988	464
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	508	233	115	114	245	212	401	1718	124	144	1086	285
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	575	1056	470	176	645	286	465	1963	608	202	1574	478
Arrive On Green	0.17	0.30	0.30	0.05	0.18	0.18	0.13	0.38	0.38	0.06	0.31	0.31
Sat Flow, veh/h	3456	3554	1583	3456	3554	1577	3456	5106	1583	3456	5106	1551
Grp Volume(v), veh/h	508	233	115	114	245	212	401	1718	124	144	1086	285
Grp Sat Flow(s),veh/h/ln	1728	1777	1583	1728	1777	1577	1728	1702	1583	1728	1702	1551
Q Serve(g_s), s	14.0	4.8	5.4	3.2	5.9	12.4	11.1	30.4	5.1	4.0	18.2	15.2
Cycle Q Clear(g_c), s	14.0	4.8	5.4	3.2	5.9	12.4	11.1	30.4	5.1	4.0	18.2	15.2
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	575	1056	470	176	645	286	465	1963	608	202	1574	478
V/C Ratio(X)	0.88	0.22	0.24	0.65	0.38	0.74	0.86	0.88	0.20	0.71	0.69	0.60
Avail Cap(c_a), veh/h	627	1760	784	262	1385	615	482	2000	620	202	1586	482
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	39.7	25.8	26.0	45.4	35.1	37.8	41.3	27.8	20.0	45.1	29.6	28.6
Incr Delay (d2), s/veh	12.4	0.1	0.3	1.5	0.4	3.8	13.7	4.8	0.2	9.7	1.4	2.4
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.7	2.0	2.0	1.4	2.5	4.9	5.4	12.2	1.8	1.9	7.2	5.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	52.1	25.9	26.2	46.9	35.5	41.5	55.0	32.6	20.3	54.8	31.1	31.0
LnGrp LOS	D	C	C	D	D	D	D	C	C	D	C	C
Approach Vol, veh/h		856			571			2243			1515	
Approach Delay, s/veh		41.5			40.0			35.9			33.3	
Approach LOS		D			D			D			C	

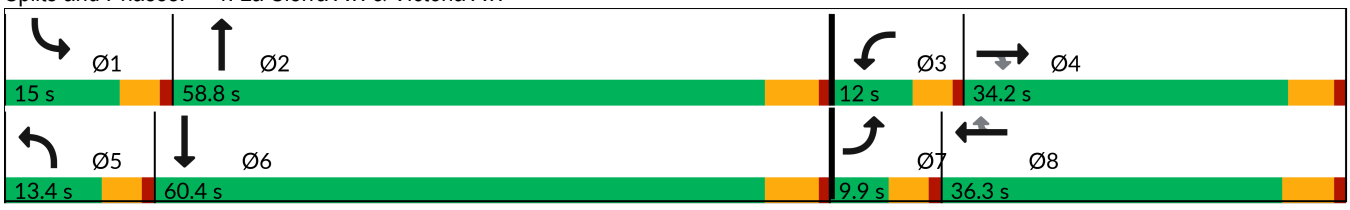
Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	10.3	43.3	9.6	34.4	17.7	35.9	20.8	23.1
Change Period (Y+Rc), s	4.6	5.8	4.6	5.4	4.6	5.8	4.6	5.4
Max Green Setting (Gmax), s	5.7	38.2	7.4	48.3	13.6	30.3	17.7	38.0
Max Q Clear Time (g_c+I1), s	6.0	32.4	5.2	7.4	13.1	20.2	16.0	14.4
Green Ext Time (p_c), s	0.0	5.1	0.0	1.8	0.1	6.8	0.2	2.1

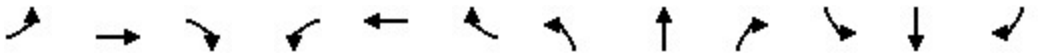
Intersection Summary	
HCM 7th Control Delay, s/veh	36.5
HCM 7th LOS	D

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations										
Traffic Volume (vph)	49	82	59	75	99	214	44	1220	112	600
Future Volume (vph)	49	82	59	75	99	214	44	1220	112	600
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Prot	NA
Protected Phases	7	4		3	8		5	2	1	6
Permitted Phases			4			8				
Detector Phase	7	4	4	3	8	8	5	2	1	6
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	5.0	10.0
Minimum Split (s)	9.6	34.1	34.1	9.6	33.8	33.8	9.6	35.8	9.6	33.8
Total Split (s)	9.9	34.2	34.2	12.0	36.3	36.3	13.4	58.8	15.0	60.4
Total Split (%)	8.3%	28.5%	28.5%	10.0%	30.3%	30.3%	11.2%	49.0%	12.5%	50.3%
Yellow Time (s)	3.6	4.1	4.1	3.6	4.8	4.8	3.6	4.8	3.6	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.1	5.1	4.6	5.8	5.8	4.6	5.8	4.6	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	None	Min
Act Effct Green (s)	5.3	13.9	13.9	7.2	17.2	17.2	7.1	53.3	10.0	58.3
Actuated g/C Ratio	0.05	0.13	0.13	0.07	0.16	0.16	0.07	0.51	0.10	0.56
v/c Ratio	0.32	0.37	0.22	0.69	0.36	0.60	0.41	0.91	0.75	0.36
Control Delay (s/veh)	55.3	45.1	3.2	77.5	43.0	19.4	59.1	32.8	73.7	15.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	55.3	45.1	3.2	77.5	43.0	19.4	59.1	32.8	73.7	15.1
LOS	E	D	A	E	D	B	E	C	E	B
Approach Delay (s/veh)		34.8			36.7			33.6		24.0
Approach LOS		C			D			C		C

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 104.6	
Natural Cycle: 120	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.91	
Intersection Signal Delay (s/veh): 31.6	Intersection LOS: C
Intersection Capacity Utilization 73.0%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 4: La Sierra Av. & Victoria Av.





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	49	82	59	75	99	214	44	1220	211	112	600	28
Future Volume (veh/h)	49	82	59	75	99	214	44	1220	211	112	600	28
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		0.98	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	55	92	27	84	111	155	49	1371	208	126	674	30
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	139	216	181	107	254	210	68	1608	241	156	1969	88
Arrive On Green	0.04	0.12	0.12	0.06	0.14	0.14	0.04	0.52	0.52	0.09	0.57	0.57
Sat Flow, veh/h	3456	1870	1563	1781	1870	1550	1781	3098	465	1781	3465	154
Grp Volume(v), veh/h	55	92	27	84	111	155	49	780	799	126	345	359
Grp Sat Flow(s),veh/h/ln	1728	1870	1563	1781	1870	1550	1781	1777	1787	1781	1777	1843
Q Serve(g_s), s	1.5	4.4	1.5	4.4	5.2	9.2	2.6	36.0	37.1	6.6	10.0	10.0
Cycle Q Clear(g_c), s	1.5	4.4	1.5	4.4	5.2	9.2	2.6	36.0	37.1	6.6	10.0	10.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.26	1.00		0.08
Lane Grp Cap(c), veh/h	139	216	181	107	254	210	68	922	927	156	1010	1047
V/C Ratio(X)	0.40	0.43	0.15	0.78	0.44	0.74	0.72	0.85	0.86	0.81	0.34	0.34
Avail Cap(c_a), veh/h	192	570	476	138	597	495	164	986	991	194	1016	1053
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	44.7	39.3	38.0	44.3	37.9	39.6	45.4	19.7	20.0	42.8	11.0	11.1
Incr Delay (d2), s/veh	0.7	1.3	0.4	14.7	1.2	5.0	5.3	7.0	7.8	14.8	0.3	0.3
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	2.0	0.6	2.3	2.4	3.6	1.2	14.6	15.3	3.4	3.5	3.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	45.4	40.6	38.4	59.0	39.1	44.6	50.8	26.7	27.8	57.6	11.3	11.3
LnGrp LOS	D	D	D	E	D	D	D	C	C	E	B	B
Approach Vol, veh/h		174			350			1628			830	
Approach Delay, s/veh		41.8			46.3			28.0			18.4	
Approach LOS		D			D			C			B	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	13.0	55.4	10.4	16.8	8.2	60.1	8.4	18.8
Change Period (Y+Rc), s	4.6	5.8	4.6	* 5.8	4.6	5.8	4.6	5.8
Max Green Setting (Gmax), s	10.4	53.0	7.4	* 29	8.8	54.6	5.3	30.5
Max Q Clear Time (g_c+I1), s	8.6	39.1	6.4	6.4	4.6	12.0	3.5	11.2
Green Ext Time (p_c), s	0.0	10.4	0.0	0.5	0.0	6.6	0.0	1.0

Intersection Summary	
HCM 7th Control Delay, s/veh	28.2
HCM 7th LOS	C

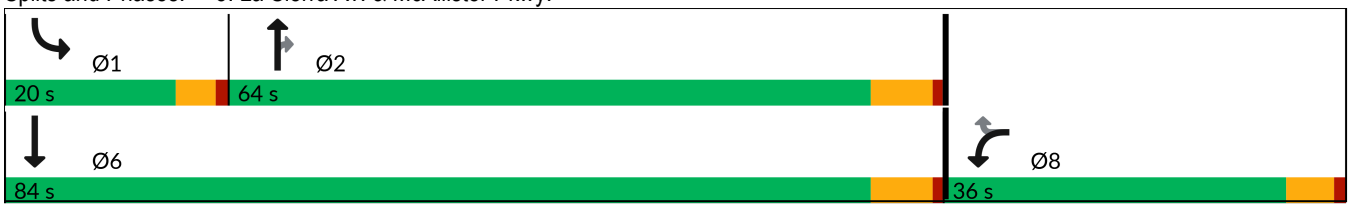
Notes

* HCM 7th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	19	242	1315	23	84	592
Future Volume (vph)	19	242	1315	23	84	592
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	10.0
Minimum Split (s)	33.4	33.4	27.5	27.5	9.6	16.5
Total Split (s)	36.0	36.0	64.0	64.0	20.0	84.0
Total Split (%)	30.0%	30.0%	53.3%	53.3%	16.7%	70.0%
Yellow Time (s)	4.4	4.4	5.5	5.5	3.6	5.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.4	6.5	6.5	4.6	6.5
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	Min	Min	None	Min
Act Effct Green (s)	13.4	13.4	46.9	46.9	9.5	58.2
Actuated g/C Ratio	0.16	0.16	0.56	0.56	0.11	0.69
v/c Ratio	0.07	0.60	0.75	0.03	0.47	0.27
Control Delay (s/veh)	35.5	13.2	19.1	7.1	48.4	5.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	35.5	13.2	19.1	7.1	48.4	5.5
LOS	D	B	B	A	D	A
Approach Delay (s/veh)	14.8		18.9			10.8
Approach LOS	B		B			B

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 84.3	
Natural Cycle: 90	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.75	
Intersection Signal Delay (s/veh): 16.0	Intersection LOS: B
Intersection Capacity Utilization 63.1%	ICU Level of Service B
Analysis Period (min) 15	

Splits and Phases: 5: La Sierra Av. & McAllister Pkwy.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	19	242	1315	23	84	592
Future Volume (veh/h)	19	242	1315	23	84	592
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	21	154	1478	23	94	665
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	236	210	2040	910	121	2505
Arrive On Green	0.13	0.13	0.57	0.57	0.07	0.71
Sat Flow, veh/h	1781	1585	3647	1585	1781	3647
Grp Volume(v), veh/h	21	154	1478	23	94	665
Grp Sat Flow(s),veh/h/ln	1781	1585	1777	1585	1781	1777
Q Serve(g_s), s	0.8	6.8	22.2	0.5	3.8	5.0
Cycle Q Clear(g_c), s	0.8	6.8	22.2	0.5	3.8	5.0
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	236	210	2040	910	121	2505
V/C Ratio(X)	0.09	0.73	0.72	0.03	0.77	0.27
Avail Cap(c_a), veh/h	744	662	2788	1244	374	3758
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	27.9	30.5	11.4	6.7	33.6	3.9
Incr Delay (d2), s/veh	0.2	4.9	0.8	0.0	3.9	0.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	2.7	6.1	0.1	1.6	0.8
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	28.1	35.4	12.2	6.8	37.5	4.0
LnGrp LOS	C	D	B	A	D	A
Approach Vol, veh/h	175		1501			759
Approach Delay, s/veh	34.5		12.1			8.2
Approach LOS	C		B			A

Timer - Assigned Phs	1	2			6	8
Phs Duration (G+Y+Rc), s	9.6	48.6			58.2	15.1
Change Period (Y+Rc), s	4.6	6.5			6.5	5.4
Max Green Setting (Gmax), s	15.4	57.5			77.5	30.6
Max Q Clear Time (g_c+I1), s	5.8	24.2			7.0	8.8
Green Ext Time (p_c), s	0.1	17.8			6.7	0.5

Intersection Summary	
HCM 7th Control Delay, s/veh	12.5
HCM 7th LOS	B

Intersection	
Intersection Delay, s/veh	56.5
Intersection LOS	F

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	86	639	115	29	296	135
Future Vol, veh/h	86	639	115	29	296	135
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	91	673	121	31	312	142
Number of Lanes	1	0	2	0	1	1

Approach	WB	NB	SB
Opposing Approach		SB	NB
Opposing Lanes	0	2	2
Conflicting Approach Left	NB		WB
Conflicting Lanes Left	2	0	1
Conflicting Approach Right	SB	WB	
Conflicting Lanes Right	2	1	0
HCM Control Delay, s/veh	87.6	11.6	19.1
HCM LOS	F	B	C

Lane	NBLn1	NBLn2	WBLn1	SBLn1	SBLn2
Vol Left, %	0%	0%	12%	100%	0%
Vol Thru, %	100%	57%	0%	0%	100%
Vol Right, %	0%	43%	88%	0%	0%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	77	67	725	296	135
LT Vol	0	0	86	296	0
Through Vol	77	38	0	0	135
RT Vol	0	29	639	0	0
Lane Flow Rate	81	71	763	312	142
Geometry Grp	5	5	2	5	5
Degree of Util (X)	0.162	0.138	1.104	0.622	0.264
Departure Headway (Hd)	7.737	7.424	5.206	7.635	7.121
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	466	486	702	477	507
Service Time	5.437	5.124	3.208	5.335	4.821
HCM Lane V/C Ratio	0.174	0.146	1.087	0.654	0.28
HCM Control Delay, s/veh	11.9	11.3	87.6	22.1	12.4
HCM Lane LOS	B	B	F	C	B
HCM 95th-tile Q	0.6	0.5	22	4.2	1.1

Intersection	
Intersection Delay, s/veh	14.9
Intersection LOS	B

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	52	293	646	123	60	74
Future Vol, veh/h	52	293	646	123	60	74
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	54	305	673	128	63	77
Number of Lanes	1	1	2	0	1	0

Approach	EB	WB	SB
Opposing Approach	WB	EB	
Opposing Lanes	2	2	0
Conflicting Approach Left	SB		WB
Conflicting Lanes Left	1	0	2
Conflicting Approach Right		SB	EB
Conflicting Lanes Right	0	1	2
HCM Control Delay, s/veh	13.7	16.2	10.9
HCM LOS	B	C	B

Lane	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1
Vol Left, %	100%	0%	0%	0%	45%
Vol Thru, %	0%	100%	100%	64%	0%
Vol Right, %	0%	0%	0%	36%	55%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	52	293	431	338	134
LT Vol	52	0	0	0	60
Through Vol	0	293	431	215	0
RT Vol	0	0	0	123	74
Lane Flow Rate	54	305	449	352	140
Geometry Grp	5	5	5	5	2
Degree of Util (X)	0.096	0.499	0.677	0.507	0.234
Departure Headway (Hd)	6.393	5.887	5.431	5.174	6.027
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	561	613	668	701	596
Service Time	4.127	3.621	3.131	2.874	4.059
HCM Lane V/C Ratio	0.096	0.498	0.672	0.502	0.235
HCM Control Delay, s/veh	9.8	14.4	18.7	13.1	10.9
HCM Lane LOS	A	B	C	B	B
HCM 95th-tile Q	0.3	2.8	5.3	2.9	0.9

Intersection	
Int Delay, s/veh	5.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	73	280	559	11	31	210
Future Vol, veh/h	73	280	559	11	31	210
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	1	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	79	304	608	12	34	228

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	620	0	-	0	1077	614
Stage 1	-	-	-	-	614	-
Stage 2	-	-	-	-	463	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	961	-	-	-	243	492
Stage 1	-	-	-	-	540	-
Stage 2	-	-	-	-	634	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	961	-	-	-	223	492
Mov Cap-2 Maneuver	-	-	-	-	353	-
Stage 1	-	-	-	-	496	-
Stage 2	-	-	-	-	634	-

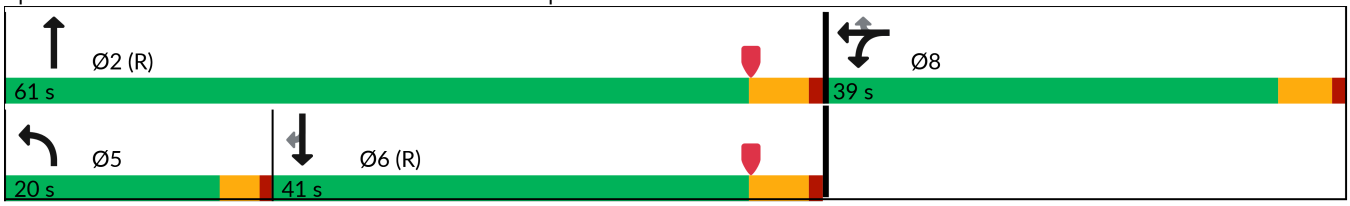
Approach	EB	WB	SB
HCM Control Delay, s/v	1.88	0	22.01
HCM LOS			C

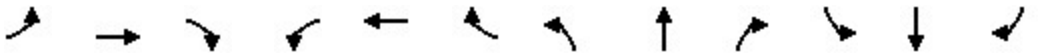
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	961	-	-	-	468
HCM Lane V/C Ratio	0.083	-	-	-	0.559
HCM Control Delay (s/veh)	9.1	-	-	-	22
HCM Lane LOS	A	-	-	-	C
HCM 95th %tile Q(veh)	0.3	-	-	-	3.4

Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Configurations							
Traffic Volume (vph)	530	2	555	422	974	1473	283
Future Volume (vph)	530	2	555	422	974	1473	283
Turn Type	Split	NA	Perm	Prot	NA	NA	Perm
Protected Phases	8	8		5	2	6	
Permitted Phases			8				6
Detector Phase	8	8	8	5	2	6	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	9.0	26.5	29.5	29.5
Total Split (s)	39.0	39.0	39.0	20.0	61.0	41.0	41.0
Total Split (%)	39.0%	39.0%	39.0%	20.0%	61.0%	41.0%	41.0%
Yellow Time (s)	4.0	4.0	4.0	3.0	4.5	4.5	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	4.0	5.5	5.5	5.5
Lead/Lag				Lead		Lag	Lag
Lead-Lag Optimize?				Yes		Yes	Yes
Recall Mode	None	None	None	None	C-Min	C-Min	C-Min
Act Effct Green (s)	28.8	28.8	28.8	15.5	60.7	41.2	41.2
Actuated g/C Ratio	0.29	0.29	0.29	0.16	0.61	0.41	0.41
v/c Ratio	0.80	0.76	0.69	0.82	0.32	0.72	0.38
Control Delay (s/veh)	45.5	35.9	29.4	40.4	24.0	28.2	9.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	45.5	35.9	29.4	40.4	24.0	28.2	9.7
LOS	D	D	C	D	C	C	A
Approach Delay (s/veh)		37.2			28.9	25.2	
Approach LOS		D			C	C	

Intersection Summary	
Cycle Length: 100	
Actuated Cycle Length: 100	
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow, Master Intersection	
Natural Cycle: 65	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.82	
Intersection Signal Delay (s/veh): 29.5	Intersection LOS: C
Intersection Capacity Utilization 75.9%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 1: La Sierra Av. & SR-91 WB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	530	2	555	422	974	0	0	1473	283
Future Volume (veh/h)	0	0	0	530	2	555	422	974	0	0	1473	283
Initial Q (Qb), veh				0	0	0	0	0	0	0	0	0
Lane Width Adj.				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach					No			No			No	
Adj Sat Flow, veh/h/ln				1870	1900	1900	1856	1885	0	0	1885	1900
Adj Flow Rate, veh/h				658	0	240	435	1004	0	0	1519	206
Peak Hour Factor				0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %				2	0	0	3	1	0	0	1	0
Cap, veh/h				779	0	352	491	3480	0	0	2537	781
Arrive On Green				0.22	0.00	0.22	0.29	1.00	0.00	0.00	0.49	0.49
Sat Flow, veh/h				3563	0	1610	3428	5316	0	0	5316	1584
Grp Volume(v), veh/h				658	0	240	435	1004	0	0	1519	206
Grp Sat Flow(s),veh/h/ln				1781	0	1610	1714	1716	0	0	1716	1584
Q Serve(g_s), s				17.7	0.0	13.7	12.1	0.0	0.0	0.0	21.2	7.6
Cycle Q Clear(g_c), s				17.7	0.0	13.7	12.1	0.0	0.0	0.0	21.2	7.6
Prop In Lane				1.00		1.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h				779	0	352	491	3480	0	0	2537	781
V/C Ratio(X)				0.84	0.00	0.68	0.89	0.29	0.00	0.00	0.60	0.26
Avail Cap(c_a), veh/h				1211	0	547	549	3480	0	0	2537	781
HCM Platoon Ratio				1.00	1.00	1.00	2.00	2.00	1.00	1.00	1.00	1.00
Upstream Filter(I)				1.00	0.00	1.00	0.70	0.70	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh				37.4	0.0	35.9	34.9	0.0	0.0	0.0	18.2	14.8
Incr Delay (d2), s/veh				2.8	0.0	1.7	10.2	0.1	0.0	0.0	1.1	0.8
Initial Q Delay(d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln				7.6	0.0	5.3	4.8	0.0	0.0	0.0	7.8	2.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh				40.2	0.0	37.6	45.1	0.1	0.0	0.0	19.3	15.6
LnGrp LOS				D		D	D	A			B	B
Approach Vol, veh/h					898			1439			1725	
Approach Delay, s/veh					39.5			13.7			18.8	
Approach LOS					D			B			B	

Timer - Assigned Phs	2		5	6		8						
Phs Duration (G+Y+Rc), s	73.1		18.3	54.8		26.9						
Change Period (Y+Rc), s	5.5		4.0	5.5		5.0						
Max Green Setting (Gmax), s	55.5		16.0	35.5		34.0						
Max Q Clear Time (g_c+I1), s	2.0		14.1	23.2		19.7						
Green Ext Time (p_c), s	8.0		0.2	8.0		2.2						

Intersection Summary		
HCM 7th Control Delay, s/veh	21.6	
HCM 7th LOS	C	

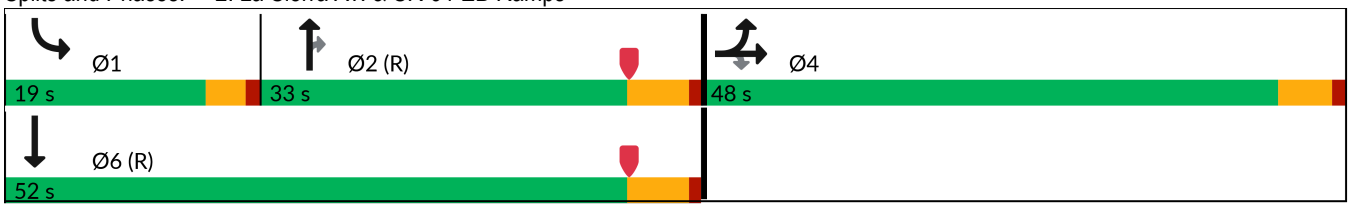
Notes

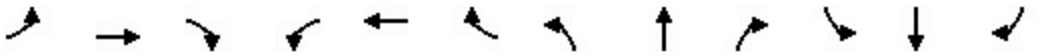
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	212	11	889	1169	334	430	1577
Future Volume (vph)	212	11	889	1169	334	430	1577
Turn Type	Split	NA	Perm	NA	Perm	Prot	NA
Protected Phases	4	4		2		1	6
Permitted Phases			4		2		
Detector Phase	4	4	4	2	2	1	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	29.5	29.5	9.0	26.5
Total Split (s)	48.0	48.0	48.0	33.0	33.0	19.0	52.0
Total Split (%)	48.0%	48.0%	48.0%	33.0%	33.0%	19.0%	52.0%
Yellow Time (s)	4.0	4.0	4.0	4.5	4.5	3.0	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.5	5.5	4.0	5.5
Lead/Lag				Lag	Lag	Lead	
Lead-Lag Optimize?				Yes	Yes	Yes	
Recall Mode	None	None	None	C-Min	C-Min	None	C-Min
Act Effct Green (s)	37.4	37.4	37.4	32.5	32.5	15.6	52.1
Actuated g/C Ratio	0.37	0.37	0.37	0.33	0.33	0.16	0.52
v/c Ratio	0.31	0.86	0.79	0.72	0.46	0.82	0.60
Control Delay (s/veh)	22.4	44.1	32.7	34.2	5.5	33.0	11.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.4
Total Delay (s/veh)	22.4	44.1	32.7	34.2	5.5	33.0	12.1
LOS	C	D	C	C	A	C	B
Approach Delay (s/veh)		35.6		27.8			16.5
Approach LOS		D		C			B

Intersection Summary	
Cycle Length: 100	
Actuated Cycle Length: 100	
Offset: 28 (28%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow	
Natural Cycle: 80	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.86	
Intersection Signal Delay (s/veh): 24.8	Intersection LOS: C
Intersection Capacity Utilization 75.9%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 2: La Sierra Av. & SR-91 EB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	212	11	889	0	0	0	0	1169	334	430	1577	0
Future Volume (veh/h)	212	11	889	0	0	0	0	1169	334	430	1577	0
Initial Q (Qb), veh	0	0	0				0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		0.98	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No						No			No	
Adj Sat Flow, veh/h/ln	1870	1752	1856				0	1870	1885	1870	1885	0
Adj Flow Rate, veh/h	148	0	752				0	1193	203	439	1609	0
Peak Hour Factor	0.98	0.98	0.98				0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	2	10	3				0	2	1	2	1	0
Cap, veh/h	488	0	862				0	2216	682	507	3195	0
Arrive On Green	0.27	0.00	0.27				0.00	0.43	0.43	0.05	0.20	0.00
Sat Flow, veh/h	1781	0	3145				0	5274	1572	3456	5316	0
Grp Volume(v), veh/h	148	0	752				0	1193	203	439	1609	0
Grp Sat Flow(s),veh/h/ln	1781	0	1572				0	1702	1572	1728	1716	0
Q Serve(g_s), s	6.6	0.0	22.8				0.0	17.3	8.4	12.6	27.7	0.0
Cycle Q Clear(g_c), s	6.6	0.0	22.8				0.0	17.3	8.4	12.6	27.7	0.0
Prop In Lane	1.00		1.00				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	488	0	862				0	2216	682	507	3195	0
V/C Ratio(X)	0.30	0.00	0.87				0.00	0.54	0.30	0.87	0.50	0.00
Avail Cap(c_a), veh/h	766	0	1352				0	2216	682	518	3195	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	0.33	0.33	1.00
Upstream Filter(I)	1.00	0.00	1.00				0.00	0.66	0.66	0.60	0.60	0.00
Uniform Delay (d), s/veh	28.7	0.0	34.6				0.0	20.9	18.4	46.6	26.1	0.0
Incr Delay (d2), s/veh	0.3	0.0	3.4				0.0	0.6	0.7	8.9	0.3	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.7	0.0	8.6				0.0	6.4	3.0	6.4	12.6	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	29.0	0.0	38.0				0.0	21.5	19.1	55.5	26.4	0.0
LnGrp LOS	C		D					C	B	E	C	
Approach Vol, veh/h		900						1396			2048	
Approach Delay, s/veh		36.6						21.2			32.7	
Approach LOS		D						C			C	

Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	18.7	48.9		32.4		67.6
Change Period (Y+Rc), s	4.0	5.5		5.0		5.5
Max Green Setting (Gmax), s	15.0	27.5		43.0		46.5
Max Q Clear Time (g_c+I1), s	14.6	19.3		24.8		29.7
Green Ext Time (p_c), s	0.1	4.9		2.6		9.9

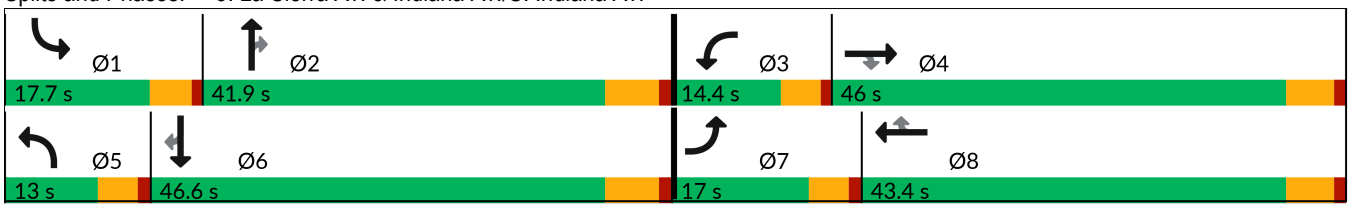
Intersection Summary	
HCM 7th Control Delay, s/veh	29.8
HCM 7th LOS	C

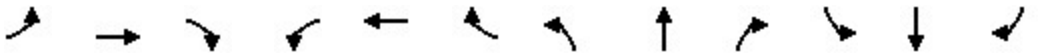
Notes
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	392	364	409	179	201	258	239	919	126	299	1657	396
Future Volume (vph)	392	364	409	179	201	258	239	919	126	299	1657	396
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4			8			2			6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	9.6	43.4	43.4	9.6	43.4	43.4	9.6	37.8	37.8	9.6	32.8	32.8
Total Split (s)	17.0	46.0	46.0	14.4	43.4	43.4	13.0	41.9	41.9	17.7	46.6	46.6
Total Split (%)	14.2%	38.3%	38.3%	12.0%	36.2%	36.2%	10.8%	34.9%	34.9%	14.8%	38.8%	38.8%
Yellow Time (s)	3.6	4.4	4.4	3.6	4.4	4.4	3.6	4.8	4.8	3.6	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.4	5.4	4.6	5.4	5.4	4.6	5.8	5.8	4.6	5.8	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	12.5	27.3	27.3	9.1	23.9	23.9	8.5	37.0	37.0	12.6	41.2	41.2
Actuated g/C Ratio	0.12	0.26	0.26	0.09	0.22	0.22	0.08	0.35	0.35	0.12	0.39	0.39
v/c Ratio	1.06	0.44	0.84	0.67	0.27	0.57	0.96	0.57	0.22	0.80	0.92	0.50
Control Delay (s/veh)	107.8	34.0	36.9	60.8	33.8	17.5	95.0	31.4	5.1	62.8	41.1	4.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	107.8	34.0	36.9	60.8	33.8	17.5	95.0	31.4	5.1	62.8	41.1	4.9
LOS	F	C	D	E	C	B	F	C	A	E	D	A
Approach Delay (s/veh)		59.8			34.8			40.6			37.8	
Approach LOS		E			C			D			D	

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 106.6	
Natural Cycle: 115	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 1.06	
Intersection Signal Delay (s/veh): 42.8	Intersection LOS: D
Intersection Capacity Utilization 78.0%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 3: La Sierra Av. & Indiana Av./S. Indiana Av.





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	392	364	409	179	201	258	239	919	126	299	1657	396
Future Volume (veh/h)	392	364	409	179	201	258	239	919	126	299	1657	396
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		0.98	1.00		1.00	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	426	396	293	195	218	215	260	999	77	325	1801	305
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	418	890	388	260	728	319	283	1852	572	390	2010	610
Arrive On Green	0.12	0.25	0.25	0.08	0.20	0.20	0.08	0.36	0.36	0.11	0.39	0.39
Sat Flow, veh/h	3456	3554	1548	3456	3554	1558	3456	5106	1577	3456	5106	1549
Grp Volume(v), veh/h	426	396	293	195	218	215	260	999	77	325	1801	305
Grp Sat Flow(s),veh/h/ln	1728	1777	1548	1728	1777	1558	1728	1702	1577	1728	1702	1549
Q Serve(g_s), s	12.4	9.6	18.0	5.7	5.3	13.1	7.7	15.9	3.4	9.5	33.9	15.3
Cycle Q Clear(g_c), s	12.4	9.6	18.0	5.7	5.3	13.1	7.7	15.9	3.4	9.5	33.9	15.3
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	418	890	388	260	728	319	283	1852	572	390	2010	610
V/C Ratio(X)	1.02	0.44	0.76	0.75	0.30	0.67	0.92	0.54	0.13	0.83	0.90	0.50
Avail Cap(c_a), veh/h	418	1406	612	330	1316	577	283	1852	572	441	2030	616
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	45.1	32.4	35.6	46.5	34.6	37.6	46.8	25.9	21.9	44.6	29.1	23.5
Incr Delay (d2), s/veh	49.3	0.3	3.0	4.9	0.2	2.5	32.5	0.4	0.2	10.5	5.8	0.9
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	8.0	4.1	6.8	2.6	2.3	5.0	4.5	6.1	1.2	4.5	13.8	5.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	94.4	32.8	38.6	51.4	34.8	40.1	79.3	26.3	22.1	55.1	34.9	24.4
LnGrp LOS	F	C	D	D	C	D	E	C	C	E	C	C
Approach Vol, veh/h		1115			628			1336			2431	
Approach Delay, s/veh		57.8			41.8			36.4			36.3	
Approach LOS		E			D			D			D	

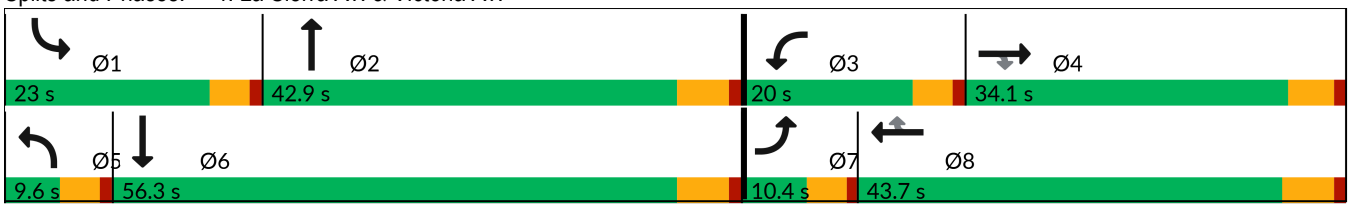
Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	16.2	43.0	12.3	31.1	13.0	46.2	17.0	26.4
Change Period (Y+Rc), s	4.6	5.8	4.6	5.4	4.6	5.8	4.6	5.4
Max Green Setting (Gmax), s	13.1	36.1	9.8	40.6	8.4	40.8	12.4	38.0
Max Q Clear Time (g_c+I1), s	11.5	17.9	7.7	20.0	9.7	35.9	14.4	15.1
Green Ext Time (p_c), s	0.1	8.6	0.1	3.4	0.0	4.5	0.0	2.0

Intersection Summary	
HCM 7th Control Delay, s/veh	41.3
HCM 7th LOS	D

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations										
Traffic Volume (vph)	62	110	156	294	80	183	29	842	201	1628
Future Volume (vph)	62	110	156	294	80	183	29	842	201	1628
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Prot	NA
Protected Phases	7	4		3	8		5	2	1	6
Permitted Phases			4			8				
Detector Phase	7	4	4	3	8	8	5	2	1	6
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	5.0	10.0
Minimum Split (s)	9.6	34.1	34.1	9.6	33.8	33.8	9.6	35.8	9.6	33.8
Total Split (s)	10.4	34.1	34.1	20.0	43.7	43.7	9.6	42.9	23.0	56.3
Total Split (%)	8.7%	28.4%	28.4%	16.7%	36.4%	36.4%	8.0%	35.8%	19.2%	46.9%
Yellow Time (s)	3.6	4.1	4.1	3.6	4.8	4.8	3.6	4.8	3.6	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.1	5.1	4.6	5.8	5.8	4.6	5.8	4.6	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	None	Min
Act Effct Green (s)	5.6	14.4	14.4	15.5	25.7	25.7	5.0	37.1	15.8	52.1
Actuated g/C Ratio	0.05	0.14	0.14	0.15	0.25	0.25	0.05	0.36	0.15	0.51
v/c Ratio	0.35	0.45	0.43	1.18	0.18	0.36	0.36	0.83	0.79	1.01
Control Delay (s/veh)	55.4	46.4	7.0	151.9	33.2	6.5	63.1	37.9	64.0	51.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	55.4	46.4	7.0	151.9	33.2	6.5	63.1	37.9	64.0	51.9
LOS	E	D	A	F	C	A	E	D	E	D
Approach Delay (s/veh)		29.4			87.1			38.7		53.2
Approach LOS		C			F			D		D

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 103.1	
Natural Cycle: 120	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 1.18	
Intersection Signal Delay (s/veh): 52.2	Intersection LOS: D
Intersection Capacity Utilization 87.8%	ICU Level of Service E
Analysis Period (min) 15	

Splits and Phases: 4: La Sierra Av. & Victoria Av.



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	62	110	156	294	80	183	29	842	143	201	1628	67
Future Volume (veh/h)	62	110	156	294	80	183	29	842	143	201	1628	67
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		0.99	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	66	117	76	313	85	115	31	896	72	214	1732	28
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	146	189	158	277	401	335	52	1314	106	247	1804	29
Arrive On Green	0.04	0.10	0.10	0.16	0.21	0.21	0.03	0.59	0.39	0.14	0.76	0.50
Sat Flow, veh/h	3456	1870	1559	1781	1870	1563	1781	3331	268	1781	3579	58
Grp Volume(v), veh/h	66	117	76	313	85	115	31	478	490	214	858	902
Grp Sat Flow(s),veh/h/ln	1728	1870	1559	1781	1870	1563	1781	1777	1822	1781	1777	1860
Q Serve(g_s), s	1.8	5.9	4.6	15.4	3.7	6.2	1.7	18.2	18.9	11.6	42.3	43.1
Cycle Q Clear(g_c), s	1.8	5.9	4.6	15.4	3.7	6.2	1.7	18.2	18.9	11.6	42.3	43.1
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.15	1.00		0.03
Lane Grp Cap(c), veh/h	146	189	158	277	401	335	52	701	719	247	896	938
V/C Ratio(X)	0.45	0.62	0.48	1.13	0.21	0.34	0.60	0.68	0.68	0.87	0.96	0.96
Avail Cap(c_a), veh/h	203	548	457	277	716	599	90	701	719	331	907	949
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.50	1.00	1.00	1.50	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	46.3	42.6	42.0	41.8	32.0	33.0	47.5	16.0	17.2	41.7	11.1	11.6
Incr Delay (d2), s/veh	0.8	3.3	2.3	93.4	0.3	0.6	4.1	3.0	2.9	13.5	20.4	20.5
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.8	2.9	1.8	13.6	1.6	2.3	0.8	5.7	6.3	5.8	10.7	11.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	47.1	45.9	44.3	135.2	32.2	33.6	51.6	19.0	20.2	55.2	31.6	32.1
LnGrp LOS	D	D	D	F	C	C	D	B	C	E	C	C
Approach Vol, veh/h		259			513			999			1974	
Approach Delay, s/veh		45.7			95.3			20.6			34.4	
Approach LOS		D			F			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	18.3	44.8	20.0	15.8	7.5	55.7	8.8	27.0
Change Period (Y+Rc), s	4.6	5.8	4.6	* 5.8	4.6	5.8	4.6	5.8
Max Green Setting (Gmax), s	18.4	37.1	15.4	* 29	5.0	50.5	5.8	37.9
Max Q Clear Time (g_c+I1), s	13.6	20.9	17.4	7.9	3.7	45.1	3.8	8.2
Green Ext Time (p_c), s	0.1	7.1	0.0	0.8	0.0	4.8	0.0	0.8

Intersection Summary	
HCM 7th Control Delay, s/veh	39.8
HCM 7th LOS	D

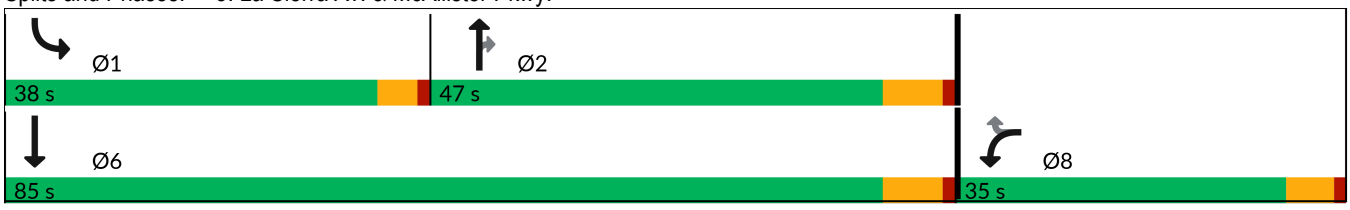
Notes

* HCM 7th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	8	159	855	13	288	1757
Future Volume (vph)	8	159	855	13	288	1757
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	10.0
Minimum Split (s)	33.4	33.4	27.5	27.5	9.6	16.5
Total Split (s)	35.0	35.0	47.0	47.0	38.0	85.0
Total Split (%)	29.2%	29.2%	39.2%	39.2%	31.7%	70.8%
Yellow Time (s)	4.4	4.4	5.5	5.5	3.6	5.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.4	6.5	6.5	4.6	6.5
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	Min	Min	None	Min
Act Effct Green (s)	13.0	13.0	31.1	31.1	20.6	56.6
Actuated g/C Ratio	0.16	0.16	0.38	0.38	0.25	0.69
v/c Ratio	0.03	0.43	0.68	0.02	0.69	0.77
Control Delay (s/veh)	34.0	9.5	25.8	12.0	38.3	11.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	34.0	9.5	25.8	12.0	38.3	11.5
LOS	C	A	C	B	D	B
Approach Delay (s/veh)	10.8		25.6			15.3
Approach LOS	B		C			B

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 82.1	
Natural Cycle: 90	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.77	
Intersection Signal Delay (s/veh): 18.0	Intersection LOS: B
Intersection Capacity Utilization 66.8%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 5: La Sierra Av. & McAllister Pkwy.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	8	159	855	13	288	1757
Future Volume (veh/h)	8	159	855	13	288	1757
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		0.98	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	9	63	910	14	306	1869
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	183	163	1718	750	351	2633
Arrive On Green	0.10	0.10	0.48	0.48	0.20	0.74
Sat Flow, veh/h	1781	1585	3647	1552	1781	3647
Grp Volume(v), veh/h	9	63	910	14	306	1869
Grp Sat Flow(s),veh/h/ln	1781	1585	1777	1552	1781	1777
Q Serve(g_s), s	0.3	2.8	13.5	0.4	12.7	21.9
Cycle Q Clear(g_c), s	0.3	2.8	13.5	0.4	12.7	21.9
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	183	163	1718	750	351	2633
V/C Ratio(X)	0.05	0.39	0.53	0.02	0.87	0.71
Avail Cap(c_a), veh/h	692	616	1890	826	781	3664
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	30.8	31.9	13.7	10.3	29.6	5.4
Incr Delay (d2), s/veh	0.1	1.5	0.4	0.0	2.7	0.5
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.1	1.1	4.3	0.1	5.1	3.1
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	30.9	33.4	14.0	10.3	32.3	5.9
LnGrp LOS	C	C	B	B	C	A
Approach Vol, veh/h	72		924			2175
Approach Delay, s/veh	33.1		14.0			9.6
Approach LOS	C		B			A

Timer - Assigned Phs	1	2			6	8
Phs Duration (G+Y+Rc), s	19.6	43.3			62.9	13.2
Change Period (Y+Rc), s	4.6	6.5			6.5	5.4
Max Green Setting (Gmax), s	33.4	40.5			78.5	29.6
Max Q Clear Time (g_c+I1), s	14.7	15.5			23.9	4.8
Green Ext Time (p_c), s	0.4	8.5			32.5	0.2

Intersection Summary	
HCM 7th Control Delay, s/veh	11.4
HCM 7th LOS	B

Intersection	
Intersection Delay, s/veh	201.4
Intersection LOS	F

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	91	428	235	115	833	503
Future Vol, veh/h	91	428	235	115	833	503
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	98	460	253	124	896	541
Number of Lanes	1	0	2	0	1	1

Approach	WB	NB	SB
Opposing Approach		SB	NB
Opposing Lanes	0	2	2
Conflicting Approach Left	NB		WB
Conflicting Lanes Left	2	0	1
Conflicting Approach Right	SB	WB	
Conflicting Lanes Right	2	1	0
HCM Control Delay, s/veh	50	15.6	308.9
HCM LOS	E	C	F

Lane	NBLn1	NBLn2	WBLn1	SBLn1	SBLn2
Vol Left, %	0%	0%	18%	100%	0%
Vol Thru, %	100%	41%	0%	0%	100%
Vol Right, %	0%	59%	82%	0%	0%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	157	193	519	833	503
LT Vol	0	0	91	833	0
Through Vol	157	78	0	0	503
RT Vol	0	115	428	0	0
Lane Flow Rate	168	208	558	896	541
Geometry Grp	5	5	2	5	5
Degree of Util (X)	0.362	0.423	0.95	1.921	1.083
Departure Headway (Hd)	8.099	7.668	6.126	7.722	7.207
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	446	472	603	475	503
Service Time	5.799	5.368	4.081	5.471	4.956
HCM Lane V/C Ratio	0.377	0.441	0.925	1.886	1.076
HCM Control Delay, s/veh	15.3	15.9	50	440.5	91
HCM Lane LOS	C	C	E	F	F
HCM 95th-tile Q	1.6	2.1	12.8	59	17

Intersection	
Intersection Delay, s/veh	151.1
Intersection LOS	F

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	64	885	422	50	69	111
Future Vol, veh/h	64	885	422	50	69	111
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	68	941	449	53	73	118
Number of Lanes	1	1	2	0	1	0

Approach	EB	WB	SB
Opposing Approach	WB	EB	
Opposing Lanes	2	2	0
Conflicting Approach Left	SB		WB
Conflicting Lanes Left	1	0	2
Conflicting Approach Right		SB	EB
Conflicting Lanes Right	0	1	2
HCM Control Delay, s/veh	245.4	14	13.6
HCM LOS	F	B	B

Lane	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1
Vol Left, %	100%	0%	0%	0%	38%
Vol Thru, %	0%	100%	100%	74%	0%
Vol Right, %	0%	0%	0%	26%	62%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	64	885	281	191	180
LT Vol	64	0	0	0	69
Through Vol	0	885	281	141	0
RT Vol	0	0	0	50	111
Lane Flow Rate	68	941	299	203	191
Geometry Grp	5	5	5	5	2
Degree of Util (X)	0.12	1.529	0.493	0.324	0.333
Departure Headway (Hd)	6.354	5.847	6.493	6.306	7.104
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	568	631	560	574	509
Service Time	4.054	3.547	4.193	4.006	5.104
HCM Lane V/C Ratio	0.12	1.491	0.534	0.354	0.375
HCM Control Delay, s/veh	9.9	262.4	15.3	12	13.6
HCM Lane LOS	A	F	C	B	B
HCM 95th-tile Q	0.4	48.1	2.7	1.4	1.4

Intersection	
Int Delay, s/veh	3.2

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	241	713	330	36	21	143
Future Vol, veh/h	241	713	330	36	21	143
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	1	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	262	775	359	39	23	155

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	398	0	-	0	1677	378
Stage 1	-	-	-	-	378	-
Stage 2	-	-	-	-	1299	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1161	-	-	-	105	668
Stage 1	-	-	-	-	693	-
Stage 2	-	-	-	-	256	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	1161	-	-	-	81	668
Mov Cap-2 Maneuver	-	-	-	-	189	-
Stage 1	-	-	-	-	536	-
Stage 2	-	-	-	-	256	-

Approach	EB	WB	SB
HCM Control Delay, s/v	2.27	0	15.98
HCM LOS			C

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1161	-	-	-	505
HCM Lane V/C Ratio	0.226	-	-	-	0.353
HCM Control Delay (s/veh)	9	-	-	-	16
HCM Lane LOS	A	-	-	-	C
HCM 95th %tile Q(veh)	0.9	-	-	-	1.6

APPENDIX 5.3: EAP (2027) (PROJECT ONLY) CONDITIONS TRAFFIC SIGNAL WARRANT ANALYSIS WORKSHEETS

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Figure 4C-103 (CA). Traffic Signal Warrants Worksheet
(Average Traffic Estimate Form)

DIST	CO	RTE	PM

TRAFFIC CONDITIONS **2027 Project Only**

CALC JB DATE 06/25/24

CHK JB DATE 06/25/24

Jurisdiction: **County of Riverside**

Major Street: **El Sobrante Rd.**

Minor Street: **Street A**

Critical Approach Speed (Major) 45 mph

Critical Approach Speed (Minor) 25 mph

Major Street Approach Lanes = **1** lane

Minor Street Approach Lanes **1** lane

Major Street Future ADT = **13,208** vpd

Minor Street Future ADT = **827** vpd

Speed limit or critical speed on major street traffic > 64 km/h (40 mph); ..... √

or

In built up area of isolated community of < 10,000 population .....

RURAL (R)

(Based on Estimated Average Daily Traffic - See Note)

URBAN RURAL

XX

CONDITION A - Minimum Vehicular Volume

Satisfied Not Satisfied

XX

Number of lanes for moving traffic on each approach

Major Street	Minor Street	Minimum Requirements EADT: Vehicles Per Day on Major Street (Total of Both Approaches) Urban	Rural	Vehicles Per Day on Higher-Volume Minor Street Approach (One Direction Only) Urban	Rural
1 **13,208**	*1* **827**	8,000	5,600 *	2,400	1,680
2 +	*1*	9,600	6,720	2,400	1,680
2 +	*2* +	9,600	6,720	3,200	2,240
1	*2* +	8,000	5,600	3,200	2,240

CONDITION B - Interruption of Continuous Traffic

Satisfied Not Satisfied

XX

Number of lanes for moving traffic on each approach

Major Street	Minor Street	Vehicles Per Day on Major Street (Total of Both Approaches) Urban	Rural	Vehicles Per Day on Higher-Volume Minor Street Approach (One Direction Only) Urban	Rural
1 **13,208**	*1* **827**	12,000	8,400 *	1,200	850
2 +	*1*	14,400	10,080	1,200	850
2 +	*2* +	14,400	10,080	1,600	1,120
1	*2* +	12,000	8,400	1,600	1,120

Combination of CONDITIONS A + B

Satisfied Not Satisfied

XX

No one condition satisfied, but following conditions fulfilled 80% of more ..... **A** 49% **B** 97%

2 CONDITIONS 80%

2 CONDITIONS 80%

Note: To be used only for NEW INTERSECTIONS or other locations where it is not reasonable to count actual traffic volumes.

The satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic control signal.

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APPENDIX 5.4: EAP (2027) (PROJECT PLUS HIGHLAND GROVE II PLUS GREENTREE) CONDITIONS TRAFFIC SIGNAL WARRANT ANALYSIS WORKSHEETS

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Figure 4C-103 (CA). Traffic Signal Warrants Worksheet
(Average Traffic Estimate Form)

DIST	CO	RTE	PM	TRAFFIC CONDITIONS	2027 P+HG4+GT
				CALC JB	DATE 06/25/24
				CHK JB	DATE 06/25/24

Jurisdiction: **County of Riverside**
Major Street: **El Sobrante Rd.**
Minor Street: **Street A**
Critical Approach Speed (Major) 45 mph
Critical Approach Speed (Minor) 25 mph

Major Street Approach Lanes = **1** lane
Minor Street Approach Lanes **1** lane

Major Street Future ADT = **14,581** vpd
Minor Street Future ADT = **2,200** vpd

Speed limit or critical speed on major street traffic > 64 km/h (40 mph); ..... √
or
In built up area of isolated community of < 10,000 population .....

RURAL (R)

(Based on Estimated Average Daily Traffic - See Note)

URBAN	RURAL	Minimum Requirements EADT			
	XX				
CONDITION A - Minimum Vehicular Volume		Vehicles Per Day on Major Street (Total of Both Approaches)		Vehicles Per Day on Higher-Volume Minor Street Approach (One Direction Only)	
Satisfied	Not Satisfied	Urban	Rural	Urban	Rural
XX					
Number of lanes for moving traffic on each approach					
Major Street	Minor Street				
1 **14,581**	*1* **2,200**	8,000	5,600 *	2,400	1,680 *
2 +	*1*	9,600	6,720	2,400	1,680
2 +	*2* +	9,600	6,720	3,200	2,240
1	*2* +	8,000	5,600	3,200	2,240
CONDITION B - Interruption of Continuous Traffic		Vehicles Per Day on Major Street (Total of Both Approaches)		Vehicles Per Day on Higher-Volume Minor Street Approach (One Direction Only)	
Satisfied	Not Satisfied	Urban	Rural	Urban	Rural
XX					
Number of lanes for moving traffic on each approach					
Major Street	Minor Street				
1 **14,581**	*1* **2,200**	12,000	8,400 *	1,200	850 *
2 +	*1*	14,400	10,080	1,200	850
2 +	*2* +	14,400	10,080	1,600	1,120
1	*2* +	12,000	8,400	1,600	1,120
Combination of CONDITIONS A + B					
Satisfied	Not Satisfied				
XX					
No one condition satisfied, but following conditions fulfilled 80% of more	A / B	2 CONDITIONS 80%		2 CONDITIONS 80%	
	A: 100% / B: 100%				

Note: To be used only for NEW INTERSECTIONS or other locations where it is not reasonable to count actual traffic volumes.

The satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic control signal.

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APPENDIX 5.5: EAP (2027) (PROJECT ONLY) CONDITIONS QUEUING ANALYSIS WORKSHEETS

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1: La Sierra Av. & SR-91 WB Ramps



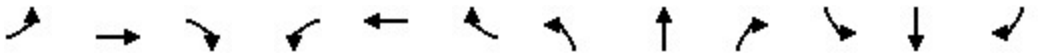
Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	276	268	267	685	1439	1015	145
v/c Ratio	0.74	0.69	0.67	0.85	0.42	0.51	0.22
Control Delay (s/veh)	47.2	34.6	33.4	41.6	16.1	26.0	7.3
Queue Delay	0.0	0.0	0.0	0.0	0.5	0.0	0.0
Total Delay (s/veh)	47.2	34.6	33.4	41.6	16.6	26.0	7.3
Queue Length 50th (ft)	173	127	121	240	192	177	7
Queue Length 95th (ft)	238	202	189	292	373	265	55
Internal Link Dist (ft)		1180			412	935	
Turn Bay Length (ft)	500		500	155			140
Base Capacity (vph)	532	519	534	901	3405	1985	671
Starvation Cap Reductn	0	0	0	0	1327	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.52	0.52	0.50	0.76	0.69	0.51	0.22

Intersection Summary



Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	313	302	286	1640	520	333	1000
v/c Ratio	0.74	0.72	0.66	0.69	0.52	0.72	0.31
Control Delay (s/veh)	44.9	36.3	28.6	24.5	4.0	49.4	2.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	44.9	36.3	28.6	24.5	4.0	49.4	2.4
Queue Length 50th (ft)	193	154	113	298	0	44	4
Queue Length 95th (ft)	261	230	185	415	67	118	35
Internal Link Dist (ft)		1416		886			412
Turn Bay Length (ft)			450		415	155	
Base Capacity (vph)	577	550	556	2363	1000	492	3257
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.54	0.55	0.51	0.69	0.52	0.68	0.31

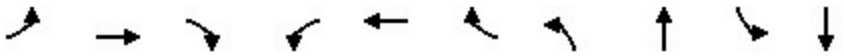
Intersection Summary



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	508	233	164	111	245	292	389	1648	178	144	1060	510
v/c Ratio	0.83	0.24	0.30	0.47	0.41	0.69	0.82	0.84	0.25	0.73	0.68	0.63
Control Delay (s/veh)	53.1	27.6	5.3	53.6	37.9	21.7	58.4	33.7	5.5	69.4	34.2	7.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	53.1	27.6	5.3	53.6	37.9	21.7	58.4	33.7	5.5	69.4	34.2	7.8
Queue Length 50th (ft)	152	60	0	33	73	56	118	315	3	44	200	10
Queue Length 95th (ft)	#321	88	42	74	106	137	#261	#592	55	#121	341	120
Internal Link Dist (ft)		2549			2428			2703			886	
Turn Bay Length (ft)	205		205	205		130	200		305	355		535
Base Capacity (vph)	615	1732	848	257	1362	719	473	1968	709	198	1561	811
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.83	0.13	0.19	0.43	0.18	0.41	0.82	0.84	0.25	0.73	0.68	0.63

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	55	92	66	81	111	240	49	1506	126	671
v/c Ratio	0.32	0.37	0.22	0.66	0.36	0.59	0.41	0.86	0.74	0.34
Control Delay (s/veh)	55.2	45.0	3.2	74.7	42.9	18.5	58.8	29.1	72.6	14.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	55.2	45.0	3.2	74.7	42.9	18.5	58.8	29.1	72.6	14.9
Queue Length 50th (ft)	18	57	0	52	68	38	31	404	80	117
Queue Length 95th (ft)	43	102	9	#144	118	110	77	#743	#202	224
Internal Link Dist (ft)		810			1637			2644		1335
Turn Bay Length (ft)	130			205		200	215		210	
Base Capacity (vph)	176	527	520	127	552	585	151	1793	179	1950
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.31	0.17	0.13	0.64	0.20	0.41	0.32	0.84	0.70	0.34

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	21	272	1375	26	94	629
v/c Ratio	0.07	0.58	0.73	0.03	0.45	0.26
Control Delay (s/veh)	33.9	12.0	18.6	6.8	45.6	5.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	33.9	12.0	18.6	6.8	45.6	5.6
Queue Length 50th (ft)	9	10	245	1	43	44
Queue Length 95th (ft)	33	83	524	18	119	121
Internal Link Dist (ft)	3339		1159			2644
Turn Bay Length (ft)	150			200	300	
Base Capacity (vph)	728	797	2639	1185	366	3226
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.34	0.52	0.02	0.26	0.19

Intersection Summary

Intersection: 1: La Sierra Av. & SR-91 WB Ramps

Movement	WB	WB	WB	NB	NB	NB	NB	NB	SB	SB	SB	SB
Directions Served	L	LTR	R	L	L	T	T	T	T	T	T	R
Maximum Queue (ft)	266	320	272	244	384	266	267	274	444	384	264	172
Average Queue (ft)	109	222	167	207	239	191	188	191	296	220	118	52
95th Queue (ft)	227	308	266	294	339	267	264	263	442	383	207	125
Link Distance (ft)		1183			444	444	444	444	949	949	949	
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	500		500	155								140
Storage Blk Time (%)				9	20						4	
Queuing Penalty (veh)				29	68						6	

Intersection: 2: La Sierra Av. & SR-91 EB Ramps

Movement	EB	EB	EB	NB	NB	NB	NB	SB	SB	SB	SB	SB
Directions Served	L	LTR	R	T	T	T	R	L	L	T	T	T
Maximum Queue (ft)	335	491	439	732	718	688	389	192	219	157	156	185
Average Queue (ft)	47	306	233	494	434	415	215	104	131	29	36	66
95th Queue (ft)	199	434	387	844	826	811	511	172	193	111	105	140
Link Distance (ft)	1430	1430		848	848	848				444	444	444
Upstream Blk Time (%)				0	0	0				0	0	
Queuing Penalty (veh)				3	3	3				0	0	
Storage Bay Dist (ft)			450				415	155	155			
Storage Blk Time (%)		1	0			14	0	2	12	1		
Queuing Penalty (veh)		1	0			71	0	7	37	2		

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	L	T	T	R	L	L	T	T	R	L	L
Maximum Queue (ft)	235	264	545	444	113	76	100	118	206	187	219	275
Average Queue (ft)	172	202	181	100	41	25	46	61	61	109	124	181
95th Queue (ft)	266	295	528	366	84	60	84	104	142	184	191	304
Link Distance (ft)			2554	2554				2432	2432			
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	205	205			205	205	205			130	200	200
Storage Blk Time (%)	5	18	2						0	7	0	2
Queuing Penalty (veh)	6	19	11						1	8	2	10

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	NB	NB	NB	NB	SB	SB	SB	SB	SB	SB		
Directions Served	T	T	T	R	L	L	T	T	T	R		
Maximum Queue (ft)	518	513	565	280	95	93	260	270	263	231		
Average Queue (ft)	273	268	300	65	36	53	146	158	151	117		
95th Queue (ft)	540	533	570	242	77	90	238	243	237	215		
Link Distance (ft)	2691	2691	2691				848	848	848			
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)				305	355	355				535		
Storage Blk Time (%)	22		11									
Queuing Penalty (veh)	76		18									

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	L	T	R	L	T	R	L	T	TR	L	T
Maximum Queue (ft)	39	80	154	66	132	137	203	245	486	506	137	205
Average Queue (ft)	10	35	56	28	53	60	76	48	235	266	73	76
95th Queue (ft)	33	71	114	54	111	115	146	156	416	451	125	172
Link Distance (ft)			843	843		1670			2640	2640		1333
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	130	130			205		200	215			210	
Storage Blk Time (%)			1				0		11			0
Queuing Penalty (veh)			0				1		5			0

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	SB	B20	B20
Directions Served	TR	T	T
Maximum Queue (ft)	224	15	60
Average Queue (ft)	92	1	3
95th Queue (ft)	187	8	31
Link Distance (ft)	1333	2691	2691
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 5: La Sierra Av. & McAllister Pkwy.

Movement	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	R	T	T	R	L	T	T
Maximum Queue (ft)	50	184	269	269	35	117	123	157
Average Queue (ft)	10	73	140	120	9	56	38	55
95th Queue (ft)	33	135	239	218	31	109	100	126
Link Distance (ft)		3360	1207	1207			2640	2640
Upstream Blk Time (%)								
Queuing Penalty (veh)								
Storage Bay Dist (ft)	150				200	300		
Storage Blk Time (%)		1		1				
Queuing Penalty (veh)		0		0				

Network Summary

Network wide Queuing Penalty: 386



Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	380	365	349	419	992	1497	292
v/c Ratio	0.79	0.74	0.69	0.81	0.32	0.70	0.38
Control Delay (s/veh)	45.3	33.1	28.8	39.6	23.4	27.4	9.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	45.3	33.1	28.8	39.6	23.4	27.4	9.4
Queue Length 50th (ft)	230	174	145	145	228	295	39
Queue Length 95th (ft)	326	274	235	189	267	375	109
Internal Link Dist (ft)		1180			412	935	
Turn Bay Length (ft)	500		500	155			140
Base Capacity (vph)	571	576	588	552	3134	2144	774
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.67	0.63	0.59	0.76	0.32	0.70	0.38

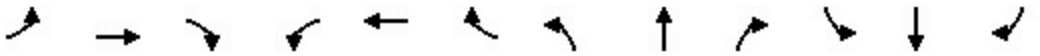
Intersection Summary



Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	194	456	458	1165	324	439	1561
v/c Ratio	0.31	0.85	0.77	0.69	0.44	0.82	0.58
Control Delay (s/veh)	22.8	43.4	32.2	33.1	5.4	32.9	11.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.3
Total Delay (s/veh)	22.8	43.4	32.2	33.1	5.4	32.9	11.4
Queue Length 50th (ft)	89	274	222	250	0	59	222
Queue Length 95th (ft)	134	392	327	311	64	#208	314
Internal Link Dist (ft)		1416		886			412
Turn Bay Length (ft)			450		415	155	
Base Capacity (vph)	722	625	681	1682	735	547	2709
Starvation Cap Reductn	0	0	0	0	0	0	509
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.27	0.73	0.67	0.69	0.44	0.80	0.71

Intersection Summary

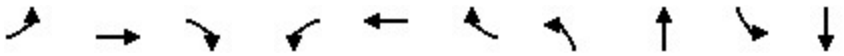
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	426	396	432	183	218	280	252	952	130	325	1722	430
v/c Ratio	0.89	0.44	0.81	0.60	0.30	0.61	0.87	0.54	0.21	0.83	0.91	0.51
Control Delay (s/veh)	66.9	33.5	32.5	55.9	34.9	18.3	77.3	30.4	6.1	64.9	40.2	5.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	66.9	33.5	32.5	55.9	34.9	18.3	77.3	30.4	6.1	64.9	40.2	5.1
Queue Length 50th (ft)	143	116	158	60	64	55	85	182	0	108	385	0
Queue Length 95th (ft)	#286	160	278	110	96	135	#192	283	46	#223	#631	75
Internal Link Dist (ft)		2549			2428			2703			886	
Turn Bay Length (ft)	205		205	205		130	200		305	355		535
Base Capacity (vph)	479	1429	737	360	1306	690	290	1748	619	393	1901	843
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.89	0.28	0.59	0.51	0.17	0.41	0.87	0.54	0.21	0.83	0.91	0.51

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	66	117	166	301	85	195	31	982	214	1702
v/c Ratio	0.32	0.45	0.43	0.98	0.17	0.35	0.36	0.82	0.82	1.00
Control Delay (s/veh)	53.3	46.6	7.0	90.3	32.0	6.2	63.3	38.2	68.5	50.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	53.3	46.6	7.0	90.3	32.0	6.2	63.3	38.2	68.5	50.9
Queue Length 50th (ft)	21	74	0	198	46	0	20	292	137	~634
Queue Length 95th (ft)	49	127	37	#450	86	52	58	#501	#313	#991
Internal Link Dist (ft)		810			1637			2644		1335
Turn Bay Length (ft)	130			205		200	215		210	
Base Capacity (vph)	244	526	579	308	706	713	86	1265	272	1698
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.27	0.22	0.29	0.98	0.12	0.27	0.36	0.78	0.79	1.00

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	9	169	844	14	306	1757
v/c Ratio	0.03	0.42	0.68	0.03	0.68	0.74
Control Delay (s/veh)	32.6	9.2	25.8	11.5	36.5	11.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	32.6	9.2	25.8	11.5	36.5	11.0
Queue Length 50th (ft)	3	0	159	0	119	202
Queue Length 95th (ft)	19	55	361	15	305	522
Internal Link Dist (ft)	3339		1159			2644
Turn Bay Length (ft)	150			200	300	
Base Capacity (vph)	717	742	1967	867	809	3286
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.01	0.23	0.43	0.02	0.38	0.53

Intersection Summary

Intersection: 1: La Sierra Av. & SR-91 WB Ramps

Movement	WB	WB	WB	NB	NB	NB	NB	NB	SB	SB	SB	SB
Directions Served	L	LTR	R	L	L	T	T	T	T	T	T	R
Maximum Queue (ft)	301	374	333	222	250	235	230	236	784	739	353	190
Average Queue (ft)	179	254	195	131	157	160	162	163	519	445	192	87
95th Queue (ft)	288	339	292	202	231	219	222	227	756	690	315	191
Link Distance (ft)		1183			444	444	444	444	949	949	949	
Upstream Blk Time (%)									0			
Queuing Penalty (veh)									0			
Storage Bay Dist (ft)	500		500	155								140
Storage Blk Time (%)				2	9						12	0
Queuing Penalty (veh)				4	17						33	0

Intersection: 2: La Sierra Av. & SR-91 EB Ramps

Movement	EB	EB	EB	NB	NB	NB	NB	SB	SB	SB	SB	SB
Directions Served	L	LTR	R	T	T	T	R	L	L	T	T	T
Maximum Queue (ft)	288	413	391	380	367	370	218	190	212	272	269	286
Average Queue (ft)	26	279	231	219	188	195	75	107	136	104	111	139
95th Queue (ft)	146	379	339	360	317	314	154	172	194	232	221	242
Link Distance (ft)	1430	1430		848	848	848				444	444	444
Upstream Blk Time (%)										0	0	0
Queuing Penalty (veh)										1	0	1
Storage Bay Dist (ft)			450				415	155	155			
Storage Blk Time (%)		0	0			0		1	7	2		
Queuing Penalty (veh)		1	0			0		5	34	10		

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	L	T	T	R	L	L	T	T	R	L	L
Maximum Queue (ft)	234	265	493	441	278	104	117	122	152	169	185	200
Average Queue (ft)	167	200	196	135	144	46	69	58	50	76	94	107
95th Queue (ft)	264	298	427	291	238	92	109	103	104	140	163	173
Link Distance (ft)			2554	2554				2432	2432			
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	205	205			205	205	205			130	200	200
Storage Blk Time (%)	5	16	4	0	2				0	2	0	0
Queuing Penalty (veh)	8	29	17	1	4				0	2	0	1

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	NB	NB	NB	NB	SB	SB	SB	SB	SB	SB		
Directions Served	T	T	T	R	L	L	T	T	T	R		
Maximum Queue (ft)	237	235	276	103	188	396	651	666	674	386		
Average Queue (ft)	136	142	166	39	106	208	408	426	432	167		
95th Queue (ft)	221	221	256	82	165	454	709	729	742	498		
Link Distance (ft)	2691	2691	2691				848	848	848			
Upstream Blk Time (%)							0	1	1			
Queuing Penalty (veh)							3	6	11			
Storage Bay Dist (ft)				305	355	355				535		
Storage Blk Time (%)	2		0			0	19		10	1		
Queuing Penalty (veh)	4		0			0	58		38	5		

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	L	T	R	L	T	R	L	T	TR	L	T
Maximum Queue (ft)	66	121	191	176	289	483	204	103	338	365	275	966
Average Queue (ft)	14	45	75	75	203	179	96	20	189	218	213	562
95th Queue (ft)	45	92	145	142	318	587	262	65	317	350	341	980
Link Distance (ft)			843	843		1670			2640	2640		1333
Upstream Blk Time (%)												1
Queuing Penalty (veh)												6
Storage Bay Dist (ft)	130	130			205		200	215			210	
Storage Blk Time (%)	0	0	2		22	1			6		5	39
Queuing Penalty (veh)	0	0	1		58	5			2		40	78

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	SB	B20	B20
Directions Served	TR	T	T
Maximum Queue (ft)	991	1693	1713
Average Queue (ft)	563	125	134
95th Queue (ft)	986	987	1005
Link Distance (ft)	1333	2691	2691
Upstream Blk Time (%)	1	0	0
Queuing Penalty (veh)	7	0	0
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 5: La Sierra Av. & McAllister Pkwy.

Movement	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	R	T	T	R	L	T	T
Maximum Queue (ft)	33	117	225	186	29	312	219	249
Average Queue (ft)	7	42	115	91	4	154	72	90
95th Queue (ft)	25	84	196	167	20	252	174	207
Link Distance (ft)		3360	1207	1207			2640	2640
Upstream Blk Time (%)								
Queuing Penalty (veh)								
Storage Bay Dist (ft)	150				200	300		
Storage Blk Time (%)		0		0		0		
Queuing Penalty (veh)		0		0		4		

Network Summary

Network wide Queuing Penalty: 494

APPENDIX 5.6: EAP (2027) (PROJECT PLUS HIGHLAND GROVE II PLUS GREENTREE) CONDITIONS QUEUING ANALYSIS WORKSHEETS

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1: La Sierra Av. & SR-91 WB Ramps



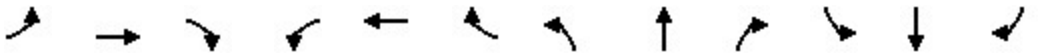
Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	283	269	267	708	1458	1023	145
v/c Ratio	0.74	0.69	0.66	0.86	0.43	0.53	0.22
Control Delay (s/veh)	47.1	34.0	32.6	41.8	16.7	26.8	7.5
Queue Delay	0.0	0.0	0.0	0.0	0.5	0.0	0.0
Total Delay (s/veh)	47.1	34.0	32.6	41.8	17.2	26.8	7.5
Queue Length 50th (ft)	177	127	120	248	197	183	7
Queue Length 95th (ft)	244	201	188	301	379	268	55
Internal Link Dist (ft)		1180			412	935	
Turn Bay Length (ft)	500		500	155			140
Base Capacity (vph)	532	519	534	904	3385	1938	658
Starvation Cap Reductn	0	0	0	0	1315	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.53	0.52	0.50	0.78	0.70	0.53	0.22

Intersection Summary



Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	317	305	287	1682	544	333	1015
v/c Ratio	0.74	0.72	0.67	0.72	0.54	0.71	0.31
Control Delay (s/veh)	44.7	35.1	29.7	25.3	4.2	50.8	2.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	44.7	35.1	29.7	25.3	4.2	50.8	2.5
Queue Length 50th (ft)	195	152	118	312	0	53	5
Queue Length 95th (ft)	263	227	190	430	69	119	36
Internal Link Dist (ft)		1416		886			412
Turn Bay Length (ft)			450		415	155	
Base Capacity (vph)	594	567	564	2344	1009	480	3243
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.53	0.54	0.51	0.72	0.54	0.69	0.31

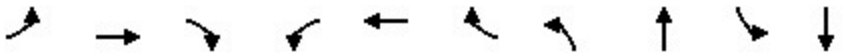
Intersection Summary



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	508	233	168	114	245	292	401	1718	188	144	1086	510
v/c Ratio	0.83	0.24	0.30	0.48	0.41	0.69	0.85	0.87	0.26	0.73	0.70	0.63
Control Delay (s/veh)	53.1	27.6	5.2	53.9	37.9	21.7	60.6	35.4	6.0	69.4	34.5	7.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	53.1	27.6	5.2	53.9	37.9	21.7	60.6	35.4	6.0	69.4	34.5	7.8
Queue Length 50th (ft)	152	60	0	34	73	56	122	335	5	44	206	10
Queue Length 95th (ft)	#321	88	43	76	106	137	#272	#633	61	#121	350	120
Internal Link Dist (ft)		2549			2428			2703			886	
Turn Bay Length (ft)	205		205	205		130	200		305	355		535
Base Capacity (vph)	615	1732	850	257	1362	719	473	1968	710	198	1561	811
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.83	0.13	0.20	0.44	0.18	0.41	0.85	0.87	0.26	0.73	0.70	0.63

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	55	92	66	84	111	240	49	1608	126	705
v/c Ratio	0.32	0.37	0.22	0.69	0.36	0.60	0.41	0.91	0.75	0.36
Control Delay (s/veh)	55.3	45.1	3.2	77.5	43.0	19.4	59.1	32.8	73.7	15.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	55.3	45.1	3.2	77.5	43.0	19.4	59.1	32.8	73.7	15.1
Queue Length 50th (ft)	18	57	0	54	68	41	31	456	80	124
Queue Length 95th (ft)	43	102	9	#152	118	114	77	#829	#202	237
Internal Link Dist (ft)		810			1637			2644		1335
Turn Bay Length (ft)	130			205		200	215		210	
Base Capacity (vph)	174	521	515	125	546	577	149	1773	176	1959
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.32	0.18	0.13	0.67	0.20	0.42	0.33	0.91	0.72	0.36

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	21	272	1478	26	94	665
v/c Ratio	0.07	0.60	0.75	0.03	0.47	0.27
Control Delay (s/veh)	35.5	13.2	19.1	7.1	48.4	5.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	35.5	13.2	19.1	7.1	48.4	5.5
Queue Length 50th (ft)	10	14	276	2	46	47
Queue Length 95th (ft)	33	87	590	18	119	128
Internal Link Dist (ft)	3339		1159			2644
Turn Bay Length (ft)	150			200	300	
Base Capacity (vph)	685	762	2532	1137	344	3134
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.36	0.58	0.02	0.27	0.21

Intersection Summary

Intersection: 1: La Sierra Av. & SR-91 WB Ramps

Movement	WB	WB	WB	NB	NB	NB	NB	NB	SB	SB	SB	SB
Directions Served	L	LTR	R	L	L	T	T	T	T	T	T	R
Maximum Queue (ft)	280	349	291	244	317	288	291	296	560	472	214	150
Average Queue (ft)	113	221	168	212	234	201	205	204	326	240	111	49
95th Queue (ft)	236	300	258	283	309	275	280	279	480	399	190	108
Link Distance (ft)		1183			444	444	444	444	949	949	949	
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	500		500	155								140
Storage Blk Time (%)				10	21						4	0
Queuing Penalty (veh)				34	73						5	0

Intersection: 2: La Sierra Av. & SR-91 EB Ramps

Movement	EB	EB	EB	NB	NB	NB	NB	SB	SB	SB	SB	SB
Directions Served	L	LTR	R	T	T	T	R	L	L	T	T	T
Maximum Queue (ft)	272	444	402	751	723	726	468	182	198	127	90	130
Average Queue (ft)	41	302	223	460	393	367	190	106	130	27	34	62
95th Queue (ft)	177	428	374	746	714	668	451	166	180	80	71	114
Link Distance (ft)	1430	1430		848	848	848				444	444	444
Upstream Blk Time (%)				0	0	0						
Queuing Penalty (veh)				2	1	0						
Storage Bay Dist (ft)			450				415	155	155			
Storage Blk Time (%)		1	0			9	0	2	12			
Queuing Penalty (veh)		2	0			47	0	7	38			

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	L	T	T	R	L	L	T	T	R	L	L
Maximum Queue (ft)	234	265	450	384	107	84	89	122	236	188	211	275
Average Queue (ft)	169	197	157	88	41	29	49	65	71	101	126	181
95th Queue (ft)	272	293	401	251	83	63	82	110	165	177	197	297
Link Distance (ft)			2554	2554				2432	2432			
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	205	205			205	205	205			130	200	200
Storage Blk Time (%)	3	15	2	0					0	7	0	2
Queuing Penalty (veh)	4	16	11	0					1	7	1	8

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	NB	NB	NB	NB	SB	SB	SB	SB	SB	SB		
Directions Served	T	T	T	R	L	L	T	T	T	R		
Maximum Queue (ft)	517	498	529	357	109	133	246	264	251	255		
Average Queue (ft)	262	263	296	63	43	60	143	156	149	119		
95th Queue (ft)	449	436	468	226	87	103	228	245	232	214		
Link Distance (ft)	2691	2691	2691				848	848	848			
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)				305	355	355				535		
Storage Blk Time (%)	17		11									
Queuing Penalty (veh)	64		20									

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	L	T	R	L	T	R	L	T	TR	L	T
Maximum Queue (ft)	46	85	124	64	127	138	163	274	432	488	187	214
Average Queue (ft)	9	36	58	27	63	63	78	53	246	280	80	76
95th Queue (ft)	34	76	109	54	111	117	140	169	414	451	147	179
Link Distance (ft)			843	843		1670			2640	2640		1333
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	130	130			205		200	215			210	
Storage Blk Time (%)			0				0		11		0	0
Queuing Penalty (veh)			0				0		5		0	0

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	SB	B20
Directions Served	TR	T
Maximum Queue (ft)	224	33
Average Queue (ft)	94	1
95th Queue (ft)	196	15
Link Distance (ft)	1333	2691
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 5: La Sierra Av. & McAllister Pkwy.

Movement	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	R	T	T	R	L	T	T
Maximum Queue (ft)	86	237	307	288	31	134	115	151
Average Queue (ft)	13	87	161	142	4	63	34	49
95th Queue (ft)	51	168	270	255	21	118	94	118
Link Distance (ft)		3360	1207	1207			2640	2640
Upstream Blk Time (%)								
Queuing Penalty (veh)								
Storage Bay Dist (ft)	150				200	300		
Storage Blk Time (%)		2		2				
Queuing Penalty (veh)		0		1				

Network Summary

Network wide Queuing Penalty: 347

1: La Sierra Av. & SR-91 WB Ramps



Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	388	377	355	435	1004	1519	292
v/c Ratio	0.80	0.76	0.69	0.82	0.32	0.72	0.38
Control Delay (s/veh)	45.5	35.9	29.4	40.4	24.0	28.2	9.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	45.5	35.9	29.4	40.4	24.0	28.2	9.7
Queue Length 50th (ft)	235	191	150	151	231	307	41
Queue Length 95th (ft)	333	295	244	#196	270	383	111
Internal Link Dist (ft)		1180			412	935	
Turn Bay Length (ft)	500		500	155			140
Base Capacity (vph)	571	570	586	553	3116	2113	765
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.68	0.66	0.61	0.79	0.32	0.72	0.38

Intersection Summary

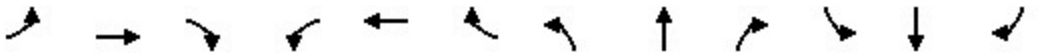
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	194	468	472	1193	341	439	1609
v/c Ratio	0.31	0.86	0.79	0.72	0.46	0.82	0.60
Control Delay (s/veh)	22.4	44.1	32.7	34.2	5.5	33.0	11.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.4
Total Delay (s/veh)	22.4	44.1	32.7	34.2	5.5	33.0	12.1
Queue Length 50th (ft)	87	282	229	261	0	55	238
Queue Length 95th (ft)	134	409	342	320	66	#208	333
Internal Link Dist (ft)		1416		886			412
Turn Bay Length (ft)			450		415	155	
Base Capacity (vph)	722	624	681	1652	739	544	2676
Starvation Cap Reductn	0	0	0	0	0	0	471
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.27	0.75	0.69	0.72	0.46	0.81	0.73

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

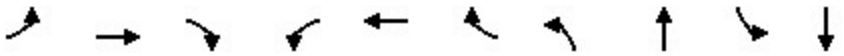


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	426	396	445	195	218	280	260	999	137	325	1801	430
v/c Ratio	1.06	0.44	0.84	0.67	0.27	0.57	0.96	0.57	0.22	0.80	0.92	0.50
Control Delay (s/veh)	107.8	34.0	36.9	60.8	33.8	17.5	95.0	31.4	5.1	62.8	41.1	4.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	107.8	34.0	36.9	60.8	33.8	17.5	95.0	31.4	5.1	62.8	41.1	4.9
Queue Length 50th (ft)	~166	120	181	67	64	58	92	204	0	112	424	0
Queue Length 95th (ft)	#310	163	307	#123	96	138	#203	296	40	#208	#649	72
Internal Link Dist (ft)		2549			2428			2703			886	
Turn Bay Length (ft)	205		205	205		130	200		305	355		535
Base Capacity (vph)	402	1359	704	318	1272	674	272	1765	635	425	1963	856
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.06	0.29	0.63	0.61	0.17	0.42	0.96	0.57	0.22	0.76	0.92	0.50

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	66	117	166	313	85	195	31	1048	214	1803
v/c Ratio	0.35	0.45	0.43	1.18	0.18	0.36	0.36	0.83	0.79	1.01
Control Delay (s/veh)	55.4	46.4	7.0	151.9	33.2	6.5	63.1	37.9	64.0	51.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	55.4	46.4	7.0	151.9	33.2	6.5	63.1	37.9	64.0	51.9
Queue Length 50th (ft)	22	74	0	~247	47	0	20	315	134	~676
Queue Length 95th (ft)	50	127	37	#502	87	53	58	#561	#283	#1045
Internal Link Dist (ft)		810			1637			2644		1335
Turn Bay Length (ft)	130			205		200	215		210	
Base Capacity (vph)	194	528	580	266	690	701	86	1273	318	1779
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.34	0.22	0.29	1.18	0.12	0.28	0.36	0.82	0.67	1.01

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	9	169	910	14	306	1869
v/c Ratio	0.03	0.43	0.68	0.02	0.69	0.77
Control Delay (s/veh)	34.0	9.5	25.8	12.0	38.3	11.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	34.0	9.5	25.8	12.0	38.3	11.5
Queue Length 50th (ft)	4	0	185	1	128	230
Queue Length 95th (ft)	19	55	396	16	305	593
Internal Link Dist (ft)	3339		1159			2644
Turn Bay Length (ft)	150			200	300	
Base Capacity (vph)	671	705	1841	812	757	3240
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.01	0.24	0.49	0.02	0.40	0.58

Intersection Summary

Intersection: 1: La Sierra Av. & SR-91 WB Ramps

Movement	WB	WB	WB	NB	NB	NB	NB	NB	SB	SB	SB	SB
Directions Served	L	LTR	R	L	L	T	T	T	T	T	T	R
Maximum Queue (ft)	290	363	303	221	258	242	234	218	805	766	355	190
Average Queue (ft)	177	255	193	128	156	165	163	164	510	432	197	90
95th Queue (ft)	283	337	283	188	217	229	226	228	733	664	317	196
Link Distance (ft)		1183			444	444	444	444	949	949	949	
Upstream Blk Time (%)									0			
Queuing Penalty (veh)									0			
Storage Bay Dist (ft)	500		500	155								140
Storage Blk Time (%)				1	7						11	0
Queuing Penalty (veh)				3	15						30	0

Intersection: 2: La Sierra Av. & SR-91 EB Ramps

Movement	EB	EB	EB	NB	NB	NB	NB	SB	SB	SB	SB	SB
Directions Served	L	LTR	R	T	T	T	R	L	L	T	T	T
Maximum Queue (ft)	115	386	370	428	348	338	150	191	224	276	214	222
Average Queue (ft)	9	263	218	230	203	204	72	106	135	101	98	126
95th Queue (ft)	68	356	320	395	347	325	122	170	198	207	182	210
Link Distance (ft)	1430	1430		848	848	848				444	444	444
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)			450				415	155	155			
Storage Blk Time (%)						0		0	8	2		
Queuing Penalty (veh)						1		1	39	9		

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	L	T	T	R	L	L	T	T	R	L	L
Maximum Queue (ft)	235	265	871	805	288	117	140	116	158	169	170	221
Average Queue (ft)	207	237	451	366	154	53	77	55	50	79	101	118
95th Queue (ft)	285	318	980	887	255	100	125	98	109	143	171	193
Link Distance (ft)			2554	2554				2432	2432			
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	205	205			205	205	205			130	200	200
Storage Blk Time (%)	17	54	3	2	3				0	2	0	1
Queuing Penalty (veh)	31	99	13	7	5				0	2	1	2

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	NB	NB	NB	NB	SB	SB	SB	SB	SB	SB
Directions Served	T	T	T	R	L	L	T	T	T	R
Maximum Queue (ft)	245	228	278	109	196	402	568	563	574	374
Average Queue (ft)	121	133	159	36	101	178	344	357	361	97
95th Queue (ft)	213	219	268	82	163	401	561	568	573	265
Link Distance (ft)	2691	2691	2691				848	848	848	
Upstream Blk Time (%)										
Queuing Penalty (veh)										
Storage Bay Dist (ft)				305	355	355				535
Storage Blk Time (%)	2		0			0	11		3	
Queuing Penalty (veh)	4		0			0	34		12	

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	L	T	R	L	T	R	L	T	TR	L	T
Maximum Queue (ft)	52	108	171	168	305	1279	330	236	428	469	275	1292
Average Queue (ft)	12	40	76	75	293	731	225	36	226	250	229	764
95th Queue (ft)	40	86	140	138	338	1478	443	131	385	404	347	1391
Link Distance (ft)			843	843		1670			2640	2640		1333
Upstream Blk Time (%)						5						5
Queuing Penalty (veh)						0						53
Storage Bay Dist (ft)	130	130			205		200	215			210	
Storage Blk Time (%)			1		78	0	0		13		3	41
Queuing Penalty (veh)			1		206	2	1		4		26	82

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	SB	B20	B20	B20
Directions Served	TR	T	T	
Maximum Queue (ft)	1297	735	1654	50
Average Queue (ft)	770	56	123	2
95th Queue (ft)	1400	456	823	26
Link Distance (ft)	1333	2691	2691	2691
Upstream Blk Time (%)	5		0	
Queuing Penalty (veh)	57		0	
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 5: La Sierra Av. & McAllister Pkwy.

Movement	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	R	T	T	R	L	T	T
Maximum Queue (ft)	33	86	243	219	31	271	248	273
Average Queue (ft)	6	41	122	101	5	147	73	95
95th Queue (ft)	23	78	209	196	23	241	187	223
Link Distance (ft)		3360	1207	1207			2640	2640
Upstream Blk Time (%)								
Queuing Penalty (veh)								
Storage Bay Dist (ft)	150				200	300		
Storage Blk Time (%)				1		0	0	
Queuing Penalty (veh)				0		1	0	

Network Summary

Network wide Queuing Penalty: 741

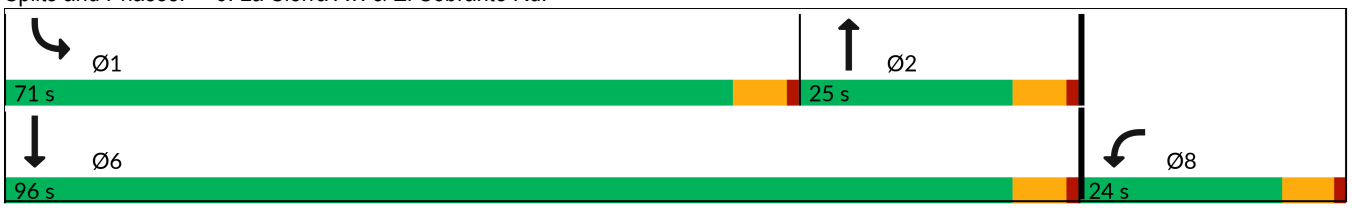
APPENDIX 5.7: EAP (2027) (PROJECT ONLY) CONDITIONS INTERSECTION OPERATIONS ANALYSIS WORKSHEETS WITH IMPROVEMENTS

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Lane Group	WBL	NBT	SBL	SBT
Lane Configurations				
Traffic Volume (vph)	72	115	264	135
Future Volume (vph)	72	115	264	135
Turn Type	Prot	NA	Prot	NA
Protected Phases	8	2	1	6
Permitted Phases				
Detector Phase	8	2	1	6
Switch Phase				
Minimum Initial (s)	5.0	10.0	5.0	10.0
Minimum Split (s)	23.8	23.8	10.8	23.8
Total Split (s)	24.0	25.0	71.0	96.0
Total Split (%)	20.0%	20.8%	59.2%	80.0%
Yellow Time (s)	4.8	4.8	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.8	5.8	5.8	5.8
Lead/Lag		Lag	Lead	
Lead-Lag Optimize?		Yes	Yes	
Recall Mode	None	None	None	None
Act Effct Green (s)	18.4	11.4	13.8	31.2
Actuated g/C Ratio	0.30	0.19	0.23	0.51
v/c Ratio	0.96	0.22	0.70	0.15
Control Delay (s/veh)	43.6	20.1	32.0	8.0
Queue Delay	0.0	0.0	0.0	0.0
Total Delay (s/veh)	43.6	20.1	32.0	8.0
LOS	D	C	C	A
Approach Delay (s/veh)	43.6	20.1		23.9
Approach LOS	D	C		C

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 61.3	
Natural Cycle: 65	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.96	
Intersection Signal Delay (s/veh): 34.0	Intersection LOS: C
Intersection Capacity Utilization 75.3%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 6: La Sierra Av. & El Sobrante Rd.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	72	548	115	24	264	135
Future Volume (veh/h)	72	548	115	24	264	135
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	76	314	121	20	278	142
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	87	360	534	87	343	909
Arrive On Green	0.28	0.28	0.17	0.17	0.19	0.49
Sat Flow, veh/h	315	1301	3156	496	1781	1870
Grp Volume(v), veh/h	391	0	69	72	278	142
Grp Sat Flow(s),veh/h/ln	1620	0	1777	1781	1781	1870
Q Serve(g_s), s	11.2	0.0	1.6	1.7	7.3	2.1
Cycle Q Clear(g_c), s	11.2	0.0	1.6	1.7	7.3	2.1
Prop In Lane	0.19	0.80		0.28	1.00	
Lane Grp Cap(c), veh/h	448	0	310	311	343	909
V/C Ratio(X)	0.87	0.00	0.22	0.23	0.81	0.16
Avail Cap(c_a), veh/h	604	0	698	700	2377	3453
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	16.8	0.0	17.3	17.4	18.9	7.0
Incr Delay (d2), s/veh	8.4	0.0	0.4	0.4	1.8	0.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.3	0.0	0.5	0.6	2.5	0.5
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	25.3	0.0	17.7	17.7	20.6	7.1
LnGrp LOS	C		B	B	C	A
Approach Vol, veh/h	391		141			420
Approach Delay, s/veh	25.3		17.7			16.0
Approach LOS	C		B			B

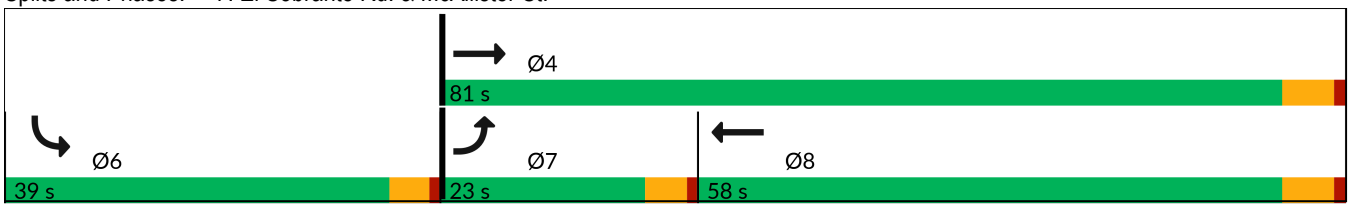
Timer - Assigned Phs	1	2				6	8
Phs Duration (G+Y+Rc), s	15.2	14.3				29.5	19.3
Change Period (Y+Rc), s	5.8	5.8				5.8	5.8
Max Green Setting (Gmax), s	65.2	19.2				90.2	18.2
Max Q Clear Time (g_c+I1), s	9.3	3.7				4.1	13.2
Green Ext Time (p_c), s	0.3	0.5				0.7	0.3

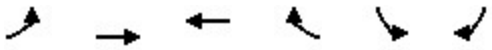
Intersection Summary	
HCM 7th Control Delay, s/veh	20.1
HCM 7th LOS	C

Lane Group	EBL	EBT	WBT	SBL
Lane Configurations				
Traffic Volume (vph)	52	256	541	51
Future Volume (vph)	52	256	541	51
Turn Type	Prot	NA	NA	Prot
Protected Phases	7	4	8	6
Permitted Phases				
Detector Phase	7	4	8	6
Switch Phase				
Minimum Initial (s)	5.0	10.0	10.0	5.0
Minimum Split (s)	10.8	23.8	23.8	23.8
Total Split (s)	23.0	81.0	58.0	39.0
Total Split (%)	19.2%	67.5%	48.3%	32.5%
Yellow Time (s)	3.6	4.8	4.8	3.6
All-Red Time (s)	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.8	5.8	4.6
Lead/Lag	Lead		Lag	
Lead-Lag Optimize?	Yes		Yes	
Recall Mode	None	None	None	None
Act Effct Green (s)	7.3	24.9	19.4	8.8
Actuated g/C Ratio	0.19	0.64	0.50	0.23
v/c Ratio	0.16	0.22	0.38	0.31
Control Delay (s/veh)	21.3	5.9	11.6	12.3
Queue Delay	0.0	0.0	0.0	0.0
Total Delay (s/veh)	21.3	5.9	11.6	12.3
LOS	C	A	B	B
Approach Delay (s/veh)		8.5	11.6	12.3
Approach LOS		A	B	B

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 39.1	
Natural Cycle: 60	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.38	
Intersection Signal Delay (s/veh): 10.8	Intersection LOS: B
Intersection Capacity Utilization 42.1%	ICU Level of Service A
Analysis Period (min) 15	

Splits and Phases: 7: El Sobrante Rd. & McAllister St.





Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	52	256	541	96	51	74
Future Volume (veh/h)	52	256	541	96	51	74
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	54	267	564	100	53	77
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	107	1038	1045	185	73	107
Arrive On Green	0.06	0.55	0.35	0.35	0.11	0.11
Sat Flow, veh/h	1781	1870	3111	533	672	976
Grp Volume(v), veh/h	54	267	331	333	131	0
Grp Sat Flow(s),veh/h/ln	1781	1870	1777	1774	1661	0
Q Serve(g_s), s	0.9	2.3	4.6	4.7	2.4	0.0
Cycle Q Clear(g_c), s	0.9	2.3	4.6	4.7	2.4	0.0
Prop In Lane	1.00			0.30	0.40	0.59
Lane Grp Cap(c), veh/h	107	1038	615	614	181	0
V/C Ratio(X)	0.51	0.26	0.54	0.54	0.72	0.00
Avail Cap(c_a), veh/h	1059	4544	2996	2992	1846	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	14.1	3.6	8.1	8.1	13.3	0.0
Incr Delay (d2), s/veh	1.4	0.1	0.7	0.7	2.0	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	0.1	1.0	1.0	0.7	0.0
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	15.5	3.7	8.9	8.9	15.4	0.0
LnGrp LOS	B	A	A	A	B	
Approach Vol, veh/h		321	664		131	
Approach Delay, s/veh		5.7	8.9		15.4	
Approach LOS		A	A		B	

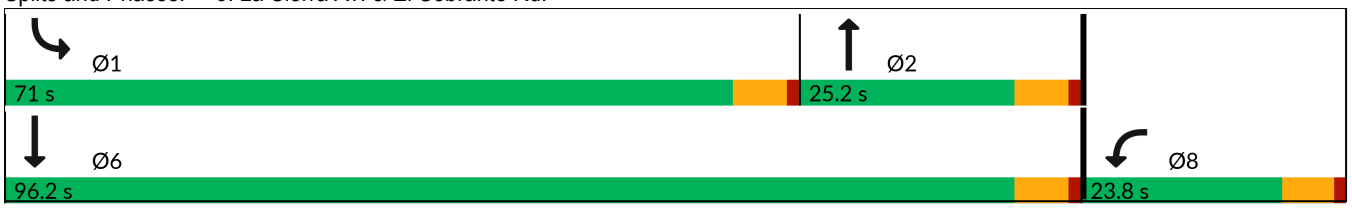
Timer - Assigned Phs	4	6	7	8
Phs Duration (G+Y+Rc), s	23.0	8.0	6.5	16.5
Change Period (Y+Rc), s	5.8	4.6	4.6	5.8
Max Green Setting (Gmax), s	75.2	34.4	18.4	52.2
Max Q Clear Time (g_c+I1), s	4.3	4.4	2.9	6.7
Green Ext Time (p_c), s	1.5	0.2	0.0	4.1

Intersection Summary	
HCM 7th Control Delay, s/veh	8.7
HCM 7th LOS	A

Lane Group	WBL	NBT	SBL	SBT
Lane Configurations				
Traffic Volume (vph)	82	235	728	503
Future Volume (vph)	82	235	728	503
Turn Type	Prot	NA	Prot	NA
Protected Phases	8	2	1	6
Permitted Phases				
Detector Phase	8	2	1	6
Switch Phase				
Minimum Initial (s)	5.0	10.0	5.0	10.0
Minimum Split (s)	23.8	23.8	10.8	23.8
Total Split (s)	23.8	25.2	71.0	96.2
Total Split (%)	19.8%	21.0%	59.2%	80.2%
Yellow Time (s)	4.8	4.8	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.8	5.8	5.8	5.8
Lead/Lag		Lag	Lead	
Lead-Lag Optimize?		Yes	Yes	
Recall Mode	None	None	None	None
Act Effct Green (s)	18.5	14.6	47.1	67.6
Actuated g/C Ratio	0.19	0.15	0.48	0.69
v/c Ratio	1.10	0.66	0.92	0.42
Control Delay (s/veh)	101.0	42.4	40.7	7.3
Queue Delay	0.0	0.0	0.0	0.0
Total Delay (s/veh)	101.0	42.4	40.7	7.3
LOS	F	D	D	A
Approach Delay (s/veh)	101.0	42.4		27.1
Approach LOS	F	D		C

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 98.1	
Natural Cycle: 100	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 1.10	
Intersection Signal Delay (s/veh): 46.1	Intersection LOS: D
Intersection Capacity Utilization 91.6%	ICU Level of Service F
Analysis Period (min) 15	

Splits and Phases: 6: La Sierra Av. & El Sobrante Rd.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	82	366	235	99	728	503
Future Volume (veh/h)	82	366	235	99	728	503
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	88	233	253	79	783	541
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	93	247	345	105	820	1227
Arrive On Green	0.21	0.21	0.13	0.13	0.46	0.66
Sat Flow, veh/h	447	1183	2775	818	1781	1870
Grp Volume(v), veh/h	322	0	166	166	783	541
Grp Sat Flow(s),veh/h/ln	1635	0	1777	1723	1781	1870
Q Serve(g_s), s	16.7	0.0	7.7	8.0	36.5	12.0
Cycle Q Clear(g_c), s	16.7	0.0	7.7	8.0	36.5	12.0
Prop In Lane	0.27	0.72		0.47	1.00	
Lane Grp Cap(c), veh/h	342	0	229	222	820	1227
V/C Ratio(X)	0.94	0.00	0.72	0.75	0.96	0.44
Avail Cap(c_a), veh/h	342	0	400	388	1349	1963
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	33.5	0.0	36.0	36.2	22.4	7.2
Incr Delay (d2), s/veh	33.5	0.0	4.3	5.0	7.7	0.2
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	9.3	0.0	3.3	3.4	14.2	3.3
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	67.1	0.0	40.4	41.2	30.1	7.4
LnGrp LOS	E		D	D	C	A
Approach Vol, veh/h	322		332			1324
Approach Delay, s/veh	67.1		40.8			20.8
Approach LOS	E		D			C

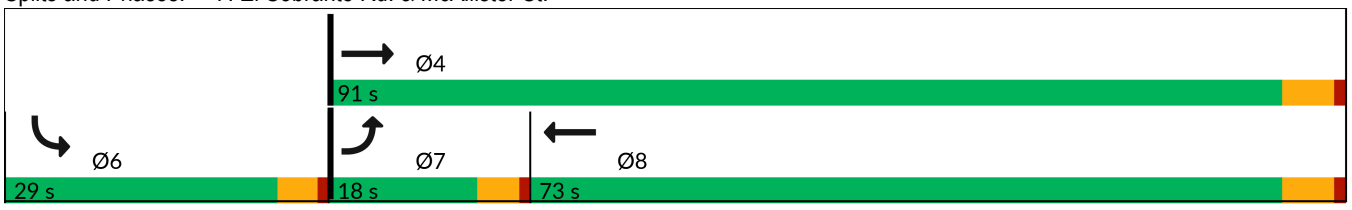
Timer - Assigned Phs	1	2				6	8
Phs Duration (G+Y+Rc), s	45.4	16.9				62.3	23.8
Change Period (Y+Rc), s	5.8	5.8				5.8	5.8
Max Green Setting (Gmax), s	65.2	19.4				90.4	18.0
Max Q Clear Time (g_c+I1), s	38.5	10.0				14.0	18.7
Green Ext Time (p_c), s	1.2	1.1				3.2	0.0

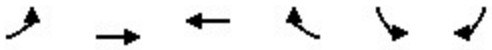
Intersection Summary	
HCM 7th Control Delay, s/veh	31.7
HCM 7th LOS	C

Lane Group	EBL	EBT	WBT	SBL
Lane Configurations				
Traffic Volume (vph)	64	764	350	38
Future Volume (vph)	64	764	350	38
Turn Type	Prot	NA	NA	Prot
Protected Phases	7	4	8	6
Permitted Phases				
Detector Phase	7	4	8	6
Switch Phase				
Minimum Initial (s)	5.0	10.0	10.0	5.0
Minimum Split (s)	10.8	23.8	23.8	23.8
Total Split (s)	18.0	91.0	73.0	29.0
Total Split (%)	15.0%	75.8%	60.8%	24.2%
Yellow Time (s)	3.6	4.8	4.8	3.6
All-Red Time (s)	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.8	5.8	4.6
Lead/Lag	Lead		Lag	
Lead-Lag Optimize?	Yes		Yes	
Recall Mode	None	None	None	None
Act Effct Green (s)	7.4	28.8	22.5	8.1
Actuated g/C Ratio	0.17	0.67	0.52	0.19
v/c Ratio	0.22	0.65	0.22	0.39
Control Delay (s/veh)	23.3	9.9	10.1	11.3
Queue Delay	0.0	0.0	0.0	0.0
Total Delay (s/veh)	23.3	9.9	10.1	11.3
LOS	C	A	B	B
Approach Delay (s/veh)		11.0	10.1	11.3
Approach LOS		B	B	B

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 42.9	
Natural Cycle: 60	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.65	
Intersection Signal Delay (s/veh): 10.8	Intersection LOS: B
Intersection Capacity Utilization 57.8%	ICU Level of Service B
Analysis Period (min) 15	

Splits and Phases: 7: El Sobrante Rd. & McAllister St.





Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	64	764	350	32	38	111
Future Volume (veh/h)	64	764	350	32	38	111
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	68	813	372	34	40	118
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	124	1074	1224	111	51	150
Arrive On Green	0.07	0.57	0.37	0.37	0.12	0.12
Sat Flow, veh/h	1781	1870	3387	299	411	1211
Grp Volume(v), veh/h	68	813	200	206	159	0
Grp Sat Flow(s),veh/h/ln	1781	1870	1777	1816	1632	0
Q Serve(g_s), s	1.3	11.3	2.7	2.8	3.3	0.0
Cycle Q Clear(g_c), s	1.3	11.3	2.7	2.8	3.3	0.0
Prop In Lane	1.00			0.16	0.25	0.74
Lane Grp Cap(c), veh/h	124	1074	660	675	202	0
V/C Ratio(X)	0.55	0.76	0.30	0.31	0.79	0.00
Avail Cap(c_a), veh/h	692	4621	3462	3540	1155	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	15.5	5.5	7.7	7.7	14.7	0.0
Incr Delay (d2), s/veh	1.4	1.1	0.3	0.3	2.6	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	1.1	0.6	0.6	1.0	0.0
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	16.9	6.6	7.9	7.9	17.2	0.0
LnGrp LOS	B	A	A	A	B	
Approach Vol, veh/h		881	406		159	
Approach Delay, s/veh		7.4	7.9		17.2	
Approach LOS		A	A		B	

Timer - Assigned Phs	4	6	7	8
Phs Duration (G+Y+Rc), s	25.6	8.9	7.0	18.6
Change Period (Y+Rc), s	5.8	4.6	4.6	5.8
Max Green Setting (Gmax), s	85.2	24.4	13.4	67.2
Max Q Clear Time (g_c+I1), s	13.3	5.3	3.3	4.8
Green Ext Time (p_c), s	6.5	0.2	0.0	2.3

Intersection Summary	
HCM 7th Control Delay, s/veh	8.6
HCM 7th LOS	A

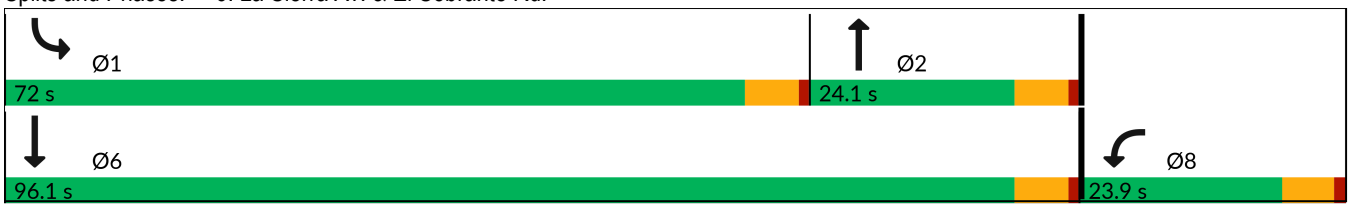
APPENDIX 5.8: EAP (2027) (PROJECT PLUS HIGHLAND GROVE II PLUS GREENTREE) CONDITIONS INTERSECTION OPERATIONS ANALYSIS WORKSHEETS WITH IMPROVEMENTS

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Lane Group	WBL	NBT	SBL	SBT
Lane Configurations				
Traffic Volume (vph)	86	115	296	135
Future Volume (vph)	86	115	296	135
Turn Type	Prot	NA	Prot	NA
Protected Phases	8	2	1	6
Permitted Phases				
Detector Phase	8	2	1	6
Switch Phase				
Minimum Initial (s)	5.0	10.0	5.0	10.0
Minimum Split (s)	23.8	23.8	10.8	23.8
Total Split (s)	23.9	24.1	72.0	96.1
Total Split (%)	19.9%	20.1%	60.0%	80.1%
Yellow Time (s)	4.8	4.8	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.8	5.8	5.8	5.8
Lead/Lag		Lag	Lead	
Lead-Lag Optimize?		Yes	Yes	
Recall Mode	None	None	None	None
Act Effct Green (s)	18.3	11.4	16.3	33.6
Actuated g/C Ratio	0.29	0.18	0.26	0.53
v/c Ratio	1.17	0.24	0.69	0.14
Control Delay (s/veh)	109.6	20.8	30.3	7.7
Queue Delay	0.0	0.0	0.0	0.0
Total Delay (s/veh)	109.6	20.8	30.3	7.7
LOS	F	C	C	A
Approach Delay (s/veh)	109.6	20.8		23.2
Approach LOS	F	C		C

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 63.6	
Natural Cycle: 80	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 1.17	
Intersection Signal Delay (s/veh): 71.1	Intersection LOS: E
Intersection Capacity Utilization 83.5%	ICU Level of Service E
Analysis Period (min) 15	

Splits and Phases: 6: La Sierra Av. & El Sobrante Rd.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	86	639	115	29	296	135
Future Volume (veh/h)	86	639	115	29	296	135
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	91	373	121	26	312	142
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	100	408	468	98	385	897
Arrive On Green	0.31	0.31	0.16	0.16	0.22	0.48
Sat Flow, veh/h	317	1300	3018	612	1781	1870
Grp Volume(v), veh/h	465	0	72	75	312	142
Grp Sat Flow(s),veh/h/ln	1621	0	1777	1760	1781	1870
Q Serve(g_s), s	15.5	0.0	2.0	2.1	9.3	2.4
Cycle Q Clear(g_c), s	15.5	0.0	2.0	2.1	9.3	2.4
Prop In Lane	0.20	0.80		0.35	1.00	
Lane Grp Cap(c), veh/h	508	0	285	282	385	897
V/C Ratio(X)	0.91	0.00	0.25	0.27	0.81	0.16
Avail Cap(c_a), veh/h	523	0	579	574	2101	3009
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	18.5	0.0	20.6	20.7	20.9	8.2
Incr Delay (d2), s/veh	20.5	0.0	0.5	0.5	4.1	0.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	7.5	0.0	0.7	0.7	3.6	0.7
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	39.0	0.0	21.1	21.2	25.0	8.3
LnGrp LOS	D		C	C	C	A
Approach Vol, veh/h	465		147			454
Approach Delay, s/veh	39.0		21.1			19.8
Approach LOS	D		C			B

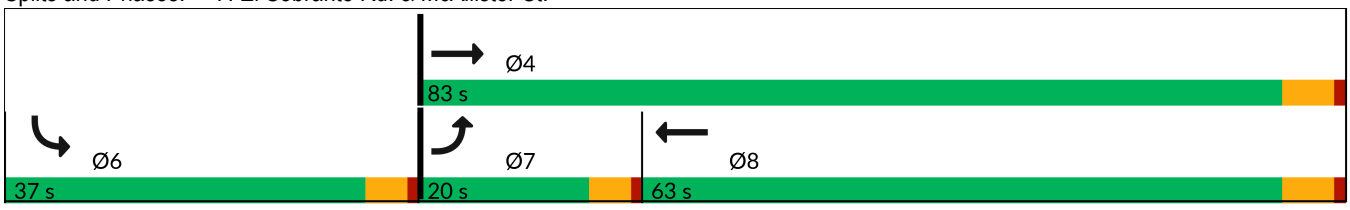
Timer - Assigned Phs	1	2				6	8
Phs Duration (G+Y+Rc), s	17.9	14.8				32.7	23.4
Change Period (Y+Rc), s	5.8	5.8				5.8	5.8
Max Green Setting (Gmax), s	66.2	18.3				90.3	18.1
Max Q Clear Time (g_c+I1), s	11.3	4.1				4.4	17.5
Green Ext Time (p_c), s	0.9	0.5				0.7	0.1

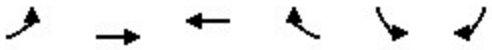
Intersection Summary	
HCM 7th Control Delay, s/veh	28.4
HCM 7th LOS	C

Lane Group	EBL	EBT	WBT	SBL
Lane Configurations				
Traffic Volume (vph)	52	293	646	60
Future Volume (vph)	52	293	646	60
Turn Type	Prot	NA	NA	Prot
Protected Phases	7	4	8	6
Permitted Phases				
Detector Phase	7	4	8	6
Switch Phase				
Minimum Initial (s)	5.0	10.0	10.0	5.0
Minimum Split (s)	10.8	23.8	23.8	23.8
Total Split (s)	20.0	83.0	63.0	37.0
Total Split (%)	16.7%	69.2%	52.5%	30.8%
Yellow Time (s)	3.6	4.8	4.8	3.6
All-Red Time (s)	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.8	5.8	4.6
Lead/Lag	Lead		Lag	
Lead-Lag Optimize?	Yes		Yes	
Recall Mode	None	None	None	None
Act Effct Green (s)	8.7	29.2	23.2	10.5
Actuated g/C Ratio	0.20	0.66	0.52	0.24
v/c Ratio	0.16	0.25	0.44	0.32
Control Delay (s/veh)	23.7	5.9	12.3	15.6
Queue Delay	0.0	0.0	0.0	0.0
Total Delay (s/veh)	23.7	5.9	12.3	15.6
LOS	C	A	B	B
Approach Delay (s/veh)		8.6	12.3	15.6
Approach LOS		A	B	B

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 44.4	
Natural Cycle: 60	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.44	
Intersection Signal Delay (s/veh): 11.7	Intersection LOS: B
Intersection Capacity Utilization 46.3%	ICU Level of Service A
Analysis Period (min) 15	

Splits and Phases: 7: El Sobrante Rd. & McAllister St.





Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	52	293	646	123	60	74
Future Volume (veh/h)	52	293	646	123	60	74
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	54	305	673	128	62	77
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	105	1092	1162	221	82	102
Arrive On Green	0.06	0.58	0.39	0.39	0.11	0.11
Sat Flow, veh/h	1781	1870	3073	566	739	918
Grp Volume(v), veh/h	54	305	401	400	140	0
Grp Sat Flow(s),veh/h/ln	1781	1870	1777	1768	1668	0
Q Serve(g_s), s	1.0	2.8	6.1	6.1	2.8	0.0
Cycle Q Clear(g_c), s	1.0	2.8	6.1	6.1	2.8	0.0
Prop In Lane	1.00			0.32	0.44	0.55
Lane Grp Cap(c), veh/h	105	1092	693	690	185	0
V/C Ratio(X)	0.52	0.28	0.58	0.58	0.76	0.00
Avail Cap(c_a), veh/h	805	4238	2983	2969	1586	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	15.6	3.5	8.2	8.2	14.7	0.0
Incr Delay (d2), s/veh	3.9	0.1	0.8	0.8	6.1	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	0.2	1.3	1.3	1.1	0.0
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	19.5	3.7	9.0	9.0	20.8	0.0
LnGrp LOS	B	A	A	A	C	
Approach Vol, veh/h		359	801		140	
Approach Delay, s/veh		6.0	9.0		20.8	
Approach LOS		A	A		C	

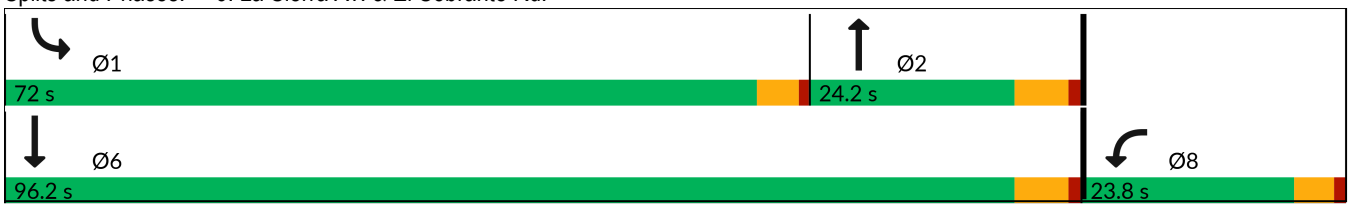
Timer - Assigned Phs	4	6	7	8
Phs Duration (G+Y+Rc), s	25.7	8.4	6.6	19.1
Change Period (Y+Rc), s	5.8	4.6	4.6	5.8
Max Green Setting (Gmax), s	77.2	32.4	15.4	57.2
Max Q Clear Time (g_c+I1), s	4.8	4.8	3.0	8.1
Green Ext Time (p_c), s	1.7	0.4	0.1	5.2

Intersection Summary	
HCM 7th Control Delay, s/veh	9.4
HCM 7th LOS	A

Lane Group	WBL	NBT	SBL	SBT
Lane Configurations				
Traffic Volume (vph)	91	235	833	503
Future Volume (vph)	91	235	833	503
Turn Type	Prot	NA	Prot	NA
Protected Phases	8	2	1	6
Permitted Phases				
Detector Phase	8	2	1	6
Switch Phase				
Minimum Initial (s)	5.0	10.0	5.0	10.0
Minimum Split (s)	23.8	23.8	10.8	23.8
Total Split (s)	23.8	24.2	72.0	96.2
Total Split (%)	19.8%	20.2%	60.0%	80.2%
Yellow Time (s)	3.6	4.8	3.6	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.8	4.6	5.8
Lead/Lag		Lag	Lead	
Lead-Lag Optimize?		Yes	Yes	
Recall Mode	None	None	None	None
Act Effct Green (s)	19.6	15.1	57.2	77.0
Actuated g/C Ratio	0.18	0.14	0.53	0.72
v/c Ratio	1.28	0.72	0.95	0.40
Control Delay (s/veh)	168.9	46.5	43.5	6.7
Queue Delay	0.0	0.0	0.0	0.0
Total Delay (s/veh)	168.9	46.5	43.5	6.7
LOS	F	D	D	A
Approach Delay (s/veh)	168.9	46.5		29.6
Approach LOS	F	D		C

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 107.2	
Natural Cycle: 120	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 1.28	
Intersection Signal Delay (s/veh): 65.1	Intersection LOS: E
Intersection Capacity Utilization 100.3%	ICU Level of Service G
Analysis Period (min) 15	

Splits and Phases: 6: La Sierra Av. & El Sobrante Rd.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	91	428	235	115	833	503
Future Volume (veh/h)	91	428	235	115	833	503
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	98	261	253	86	896	541
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	88	235	331	110	927	1298
Arrive On Green	0.20	0.20	0.13	0.13	0.52	0.69
Sat Flow, veh/h	445	1185	2715	870	1781	1870
Grp Volume(v), veh/h	360	0	169	170	896	541
Grp Sat Flow(s),veh/h/ln	1635	0	1777	1714	1781	1870
Q Serve(g_s), s	19.2	0.0	8.9	9.3	47.0	12.0
Cycle Q Clear(g_c), s	19.2	0.0	8.9	9.3	47.0	12.0
Prop In Lane	0.27	0.72		0.51	1.00	
Lane Grp Cap(c), veh/h	324	0	224	216	927	1298
V/C Ratio(X)	1.11	0.00	0.76	0.78	0.97	0.42
Avail Cap(c_a), veh/h	324	0	338	326	1241	1747
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	38.8	0.0	40.8	41.0	22.4	6.4
Incr Delay (d2), s/veh	82.9	0.0	5.2	7.0	14.1	0.2
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	14.7	0.0	4.0	4.1	19.7	3.3
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	121.7	0.0	46.0	48.0	36.5	6.6
LnGrp LOS	F		D	D	D	A
Approach Vol, veh/h	360		339			1437
Approach Delay, s/veh	121.7		47.0			25.3
Approach LOS	F		D			C

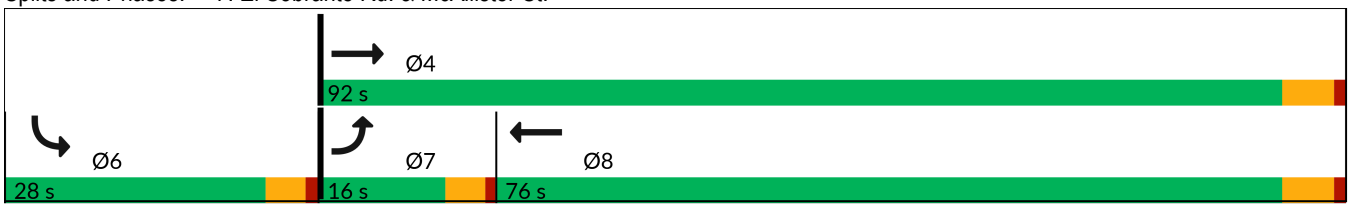
Timer - Assigned Phs	1	2				6	8
Phs Duration (G+Y+Rc), s	55.0	18.0				73.0	23.8
Change Period (Y+Rc), s	4.6	5.8				5.8	4.6
Max Green Setting (Gmax), s	67.4	18.4				90.4	19.2
Max Q Clear Time (g_c+I1), s	49.0	11.3				14.0	21.2
Green Ext Time (p_c), s	1.4	0.9				3.2	0.0

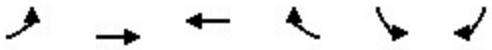
Intersection Summary	
HCM 7th Control Delay, s/veh	45.0
HCM 7th LOS	D

Lane Group	EBL	EBT	WBT	SBL
Lane Configurations				
Traffic Volume (vph)	64	885	422	69
Future Volume (vph)	64	885	422	69
Turn Type	Prot	NA	NA	Prot
Protected Phases	7	4	8	6
Permitted Phases				
Detector Phase	7	4	8	6
Switch Phase				
Minimum Initial (s)	5.0	10.0	10.0	5.0
Minimum Split (s)	10.8	23.8	23.8	23.8
Total Split (s)	16.0	92.0	76.0	28.0
Total Split (%)	13.3%	76.7%	63.3%	23.3%
Yellow Time (s)	3.6	4.8	4.8	3.6
All-Red Time (s)	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.8	5.8	4.6
Lead/Lag	Lead		Lag	
Lead-Lag Optimize?	Yes		Yes	
Recall Mode	None	None	None	None
Act Effct Green (s)	7.6	37.0	30.2	10.3
Actuated g/C Ratio	0.13	0.63	0.51	0.17
v/c Ratio	0.30	0.80	0.28	0.56
Control Delay (s/veh)	33.1	14.4	9.8	25.6
Queue Delay	0.0	0.0	0.0	0.0
Total Delay (s/veh)	33.1	14.4	9.8	25.6
LOS	C	B	A	C
Approach Delay (s/veh)		15.6	9.8	25.6
Approach LOS		B	A	C

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 58.9	
Natural Cycle: 70	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.80	
Intersection Signal Delay (s/veh): 15.0	Intersection LOS: B
Intersection Capacity Utilization 65.9%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 7: El Sobrante Rd. & McAllister St.





Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	64	885	422	50	69	111
Future Volume (veh/h)	64	885	422	50	69	111
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	68	941	449	53	73	118
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	113	1161	1456	171	93	150
Arrive On Green	0.06	0.62	0.45	0.45	0.15	0.15
Sat Flow, veh/h	1781	1870	3297	376	630	1018
Grp Volume(v), veh/h	68	941	248	254	192	0
Grp Sat Flow(s),veh/h/ln	1781	1870	1777	1803	1656	0
Q Serve(g_s), s	1.7	17.2	4.0	4.0	5.0	0.0
Cycle Q Clear(g_c), s	1.7	17.2	4.0	4.0	5.0	0.0
Prop In Lane	1.00			0.21	0.38	0.61
Lane Grp Cap(c), veh/h	113	1161	808	819	243	0
V/C Ratio(X)	0.60	0.81	0.31	0.31	0.79	0.00
Avail Cap(c_a), veh/h	453	3597	2783	2824	864	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	20.4	6.5	7.7	7.8	18.4	0.0
Incr Delay (d2), s/veh	1.9	1.4	0.2	0.2	2.2	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	2.6	1.0	1.0	1.7	0.0
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	22.3	7.9	8.0	8.0	20.6	0.0
LnGrp LOS	C	A	A	A	C	
Approach Vol, veh/h		1009	502		192	
Approach Delay, s/veh		8.9	8.0		20.6	
Approach LOS		A	A		C	

Timer - Assigned Phs			4		6	7	8
Phs Duration (G+Y+Rc), s			33.6		11.2	7.5	26.2
Change Period (Y+Rc), s			5.8		4.6	4.6	5.8
Max Green Setting (Gmax), s			86.2		23.4	11.4	70.2
Max Q Clear Time (g_c+I1), s			19.2		7.0	3.7	6.0
Green Ext Time (p_c), s			8.6		0.2	0.0	2.9

Intersection Summary	
HCM 7th Control Delay, s/veh	9.9
HCM 7th LOS	A

APPENDIX 6.1: EAPC (2027) CONDITIONS INTERSECTION OPERATIONS ANALYSIS WORKSHEETS

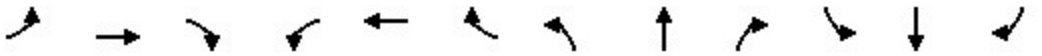
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Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Configurations							
Traffic Volume (vph)	318	0	547	712	1513	1082	193
Future Volume (vph)	318	0	547	712	1513	1082	193
Turn Type	Split	NA	Perm	Prot	NA	NA	Perm
Protected Phases	8	8		5	2	6	
Permitted Phases			8				6
Detector Phase	8	8	8	5	2	6	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	9.0	26.5	29.5	29.5
Total Split (s)	37.0	37.0	37.0	30.0	63.0	33.0	33.0
Total Split (%)	37.0%	37.0%	37.0%	30.0%	63.0%	33.0%	33.0%
Yellow Time (s)	4.0	4.0	4.0	3.0	4.5	4.5	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	4.0	5.5	5.5	5.5
Lead/Lag				Lead		Lag	Lag
Lead-Lag Optimize?				Yes		Yes	Yes
Recall Mode	None	None	None	None	C-Min	C-Min	C-Min
Act Effct Green (s)	23.9	23.9	23.9	24.7	65.6	37.0	37.0
Actuated g/C Ratio	0.24	0.24	0.24	0.25	0.66	0.37	0.37
v/c Ratio	0.74	0.74	0.72	0.87	0.47	0.60	0.30
Control Delay (s/veh)	46.1	36.8	35.6	43.0	17.4	29.0	8.6
Queue Delay	0.0	0.0	0.0	0.0	0.7	0.0	0.0
Total Delay (s/veh)	46.1	36.8	35.6	43.0	18.0	29.0	8.6
LOS	D	D	D	D	B	C	A
Approach Delay (s/veh)		39.5			26.0	25.9	
Approach LOS		D			C	C	

Intersection Summary	
Cycle Length: 100	
Actuated Cycle Length: 100	
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow, Master Intersection	
Natural Cycle: 70	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.87	
Intersection Signal Delay (s/veh): 28.6	Intersection LOS: C
Intersection Capacity Utilization 75.1%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 1: La Sierra Av. & SR-91 WB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	318	0	547	712	1513	0	0	1082	193
Future Volume (veh/h)	0	0	0	318	0	547	712	1513	0	0	1082	193
Initial Q (Qb), veh				0	0	0	0	0	0	0	0	0
Lane Width Adj.				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach					No			No			No	
Adj Sat Flow, veh/h/ln				1856	1900	1885	1870	1870	0	0	1856	1826
Adj Flow Rate, veh/h				459	0	250	734	1560	0	0	1115	156
Peak Hour Factor				0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %				3	0	1	2	2	0	0	3	5
Cap, veh/h				662	0	299	788	3613	0	0	2227	669
Arrive On Green				0.19	0.00	0.19	0.46	1.00	0.00	0.00	0.44	0.44
Sat Flow, veh/h				3534	0	1598	3456	5274	0	0	5233	1522
Grp Volume(v), veh/h				459	0	250	734	1560	0	0	1115	156
Grp Sat Flow(s),veh/h/ln				1767	0	1598	1728	1702	0	0	1689	1522
Q Serve(g_s), s				12.1	0.0	15.1	20.1	0.0	0.0	0.0	15.8	6.4
Cycle Q Clear(g_c), s				12.1	0.0	15.1	20.1	0.0	0.0	0.0	15.8	6.4
Prop In Lane				1.00		1.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h				662	0	299	788	3613	0	0	2227	669
V/C Ratio(X)				0.69	0.00	0.84	0.93	0.43	0.00	0.00	0.50	0.23
Avail Cap(c_a), veh/h				1131	0	511	898	3613	0	0	2227	669
HCM Platoon Ratio				1.00	1.00	1.00	2.00	2.00	1.00	1.00	1.00	1.00
Upstream Filter(I)				1.00	0.00	1.00	0.55	0.55	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh				37.9	0.0	39.1	26.5	0.0	0.0	0.0	20.1	17.5
Incr Delay (d2), s/veh				1.0	0.0	4.6	8.8	0.2	0.0	0.0	0.8	0.8
Initial Q Delay(d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln				5.1	0.0	6.0	6.4	0.1	0.0	0.0	5.9	2.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh				38.9	0.0	43.7	35.3	0.2	0.0	0.0	20.9	18.3
LnGrp LOS				D		D	D	A			C	B
Approach Vol, veh/h					709			2294			1271	
Approach Delay, s/veh					40.6			11.4			20.6	
Approach LOS					D			B			C	

Timer - Assigned Phs	2		5	6		8						
Phs Duration (G+Y+Rc), s	76.3		26.8	49.5		23.7						
Change Period (Y+Rc), s	5.5		4.0	5.5		5.0						
Max Green Setting (Gmax), s	57.5		26.0	27.5		32.0						
Max Q Clear Time (g_c+I1), s	2.0		22.1	17.8		17.1						
Green Ext Time (p_c), s	15.8		0.7	5.2		1.7						

Intersection Summary		
HCM 7th Control Delay, s/veh	19.0	
HCM 7th LOS	B	

Notes

User approved volume balancing among the lanes for turning movement.

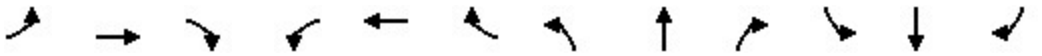
Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	493	1	449	1681	547	372	1023
Future Volume (vph)	493	1	449	1681	547	372	1023
Turn Type	Split	NA	Perm	NA	Perm	Prot	NA
Protected Phases	4	4		2		1	6
Permitted Phases			4		2		
Detector Phase	4	4	4	2	2	1	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	29.5	29.5	9.0	26.5
Total Split (s)	37.0	37.0	37.0	45.0	45.0	18.0	63.0
Total Split (%)	37.0%	37.0%	37.0%	45.0%	45.0%	18.0%	63.0%
Yellow Time (s)	4.0	4.0	4.0	4.5	4.5	3.0	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.5	5.5	4.0	5.5
Lead/Lag				Lag	Lag	Lead	
Lead-Lag Optimize?				Yes	Yes	Yes	
Recall Mode	None	None	None	C-Min	C-Min	None	C-Min
Act Effct Green (s)	25.5	25.5	25.5	45.3	45.3	14.7	64.0
Actuated g/C Ratio	0.26	0.26	0.26	0.45	0.45	0.15	0.64
v/c Ratio	0.78	0.79	0.72	0.77	0.56	0.78	0.33
Control Delay (s/veh)	47.2	42.7	33.0	27.1	4.3	58.1	2.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	47.2	42.7	33.0	27.1	4.3	58.1	2.6
LOS	D	D	C	C	A	E	A
Approach Delay (s/veh)		41.2		21.5			17.4
Approach LOS		D		C			B

Intersection Summary

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 28 (28%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow
Natural Cycle: 70
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.79
Intersection Signal Delay (s/veh): 24.3 — Intersection LOS: C
Intersection Capacity Utilization 75.1% — ICU Level of Service D
Analysis Period (min) 15

Splits and Phases: 2: La Sierra Av. & SR-91 EB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	493	1	449	0	0	0	0	1681	547	372	1023	0
Future Volume (veh/h)	493	1	449	0	0	0	0	1681	547	372	1023	0
Initial Q (Qb), veh	0	0	0				0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No						No			No	
Adj Sat Flow, veh/h/ln	1885	1900	1811				0	1856	1856	1841	1856	0
Adj Flow Rate, veh/h	589	0	161				0	1751	418	388	1066	0
Peak Hour Factor	0.96	0.96	0.96				0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	1	0	6				0	3	3	4	3	0
Cap, veh/h	702	0	300				0	2661	824	456	3543	0
Arrive On Green	0.20	0.00	0.20				0.00	0.53	0.53	0.04	0.23	0.00
Sat Flow, veh/h	3591	0	1535				0	5233	1570	3401	5233	0
Grp Volume(v), veh/h	589	0	161				0	1751	418	388	1066	0
Grp Sat Flow(s),veh/h/ln	1795	0	1535				0	1689	1570	1700	1689	0
Q Serve(g_s), s	15.8	0.0	9.4				0.0	25.1	17.2	11.3	17.4	0.0
Cycle Q Clear(g_c), s	15.8	0.0	9.4				0.0	25.1	17.2	11.3	17.4	0.0
Prop In Lane	1.00		1.00				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	702	0	300				0	2661	824	456	3543	0
V/C Ratio(X)	0.84	0.00	0.54				0.00	0.66	0.51	0.85	0.30	0.00
Avail Cap(c_a), veh/h	1149	0	491				0	2661	824	476	3543	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	0.33	0.33	1.00
Upstream Filter(I)	1.00	0.00	1.00				0.00	0.36	0.36	0.74	0.74	0.00
Uniform Delay (d), s/veh	38.7	0.0	36.1				0.0	17.2	15.4	46.8	18.3	0.0
Incr Delay (d2), s/veh	2.3	0.0	1.1				0.0	0.5	0.8	10.0	0.2	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.8	0.0	3.5				0.0	8.7	5.7	5.7	7.8	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	41.0	0.0	37.3				0.0	17.7	16.2	56.7	18.4	0.0
LnGrp LOS	D		D					B	B	E	B	
Approach Vol, veh/h		750						2169			1454	
Approach Delay, s/veh		40.2						17.4			28.6	
Approach LOS		D						B			C	

Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	17.4	58.0		24.6		75.4
Change Period (Y+Rc), s	4.0	5.5		5.0		5.5
Max Green Setting (Gmax), s	14.0	39.5		32.0		57.5
Max Q Clear Time (g_c+I1), s	13.3	27.1		17.8		19.4
Green Ext Time (p_c), s	0.1	9.4		1.8		8.4

Intersection Summary		
HCM 7th Control Delay, s/veh		25.1
HCM 7th LOS		C

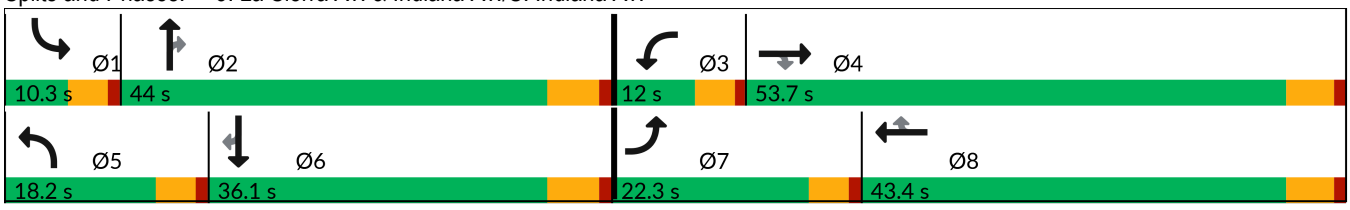
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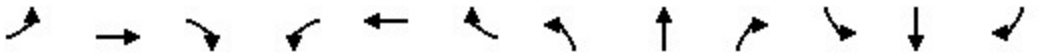
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	468	212	161	112	223	272	380	1643	184	142	1029	475
Future Volume (vph)	468	212	161	112	223	272	380	1643	184	142	1029	475
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4			8			2			6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	9.6	43.4	43.4	9.6	43.4	43.4	9.6	37.8	37.8	9.6	32.8	32.8
Total Split (s)	22.3	53.7	53.7	12.0	43.4	43.4	18.2	44.0	44.0	10.3	36.1	36.1
Total Split (%)	18.6%	44.8%	44.8%	10.0%	36.2%	36.2%	15.2%	36.7%	36.7%	8.6%	30.1%	30.1%
Yellow Time (s)	3.6	4.4	4.4	3.6	4.4	4.4	3.6	4.8	4.8	3.6	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.4	5.4	4.6	5.4	5.4	4.6	5.8	5.8	4.6	5.8	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	17.9	28.0	28.0	7.0	17.1	17.1	13.7	38.6	38.6	5.8	30.6	30.6
Actuated g/C Ratio	0.18	0.28	0.28	0.07	0.17	0.17	0.14	0.39	0.39	0.06	0.31	0.31
v/c Ratio	0.84	0.23	0.31	0.52	0.40	0.70	0.89	0.92	0.28	0.79	0.73	0.64
Control Delay (s/veh)	54.1	27.5	5.2	55.0	37.7	22.6	65.1	39.0	6.6	75.5	35.4	8.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	54.1	27.5	5.2	55.0	37.7	22.6	65.1	39.0	6.6	75.5	35.4	8.4
LOS	D	C	A	D	D	C	E	D	A	E	D	A
Approach Delay (s/veh)		38.0			34.1			40.8			31.1	
Approach LOS		D			C			D			C	

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 99.9	
Natural Cycle: 115	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.92	
Intersection Signal Delay (s/veh): 36.6	Intersection LOS: D
Intersection Capacity Utilization 76.8%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 3: La Sierra Av. & Indiana Av./S. Indiana Av.





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	468	212	161	112	223	272	380	1643	184	142	1029	475
Future Volume (veh/h)	468	212	161	112	223	272	380	1643	184	142	1029	475
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	514	233	124	123	245	219	418	1805	138	156	1131	297
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	579	1062	473	185	657	292	475	1960	607	199	1552	471
Arrive On Green	0.17	0.30	0.30	0.05	0.18	0.18	0.14	0.38	0.38	0.06	0.30	0.30
Sat Flow, veh/h	3456	3554	1583	3456	3554	1577	3456	5106	1583	3456	5106	1550
Grp Volume(v), veh/h	514	233	124	123	245	219	418	1805	138	156	1131	297
Grp Sat Flow(s),veh/h/ln	1728	1777	1583	1728	1777	1577	1728	1702	1583	1728	1702	1550
Q Serve(g_s), s	14.4	4.9	5.9	3.5	6.0	13.0	11.7	33.3	5.8	4.4	19.6	16.3
Cycle Q Clear(g_c), s	14.4	4.9	5.9	3.5	6.0	13.0	11.7	33.3	5.8	4.4	19.6	16.3
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	579	1062	473	185	657	292	475	1960	607	199	1552	471
V/C Ratio(X)	0.89	0.22	0.26	0.66	0.37	0.75	0.88	0.92	0.23	0.78	0.73	0.63
Avail Cap(c_a), veh/h	618	1735	773	258	1365	606	475	1971	611	199	1564	475
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	40.3	26.0	26.4	45.9	35.3	38.2	41.9	29.1	20.6	46.0	30.8	29.7
Incr Delay (d2), s/veh	13.4	0.1	0.3	1.5	0.4	3.9	16.6	7.8	0.3	16.8	1.9	3.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	7.0	2.0	2.2	1.5	2.5	5.2	5.9	13.8	2.1	2.3	7.8	6.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	53.7	26.1	26.7	47.5	35.7	42.1	58.5	36.8	20.9	62.8	32.7	32.7
LnGrp LOS	D	C	C	D	D	D	E	D	C	E	C	C
Approach Vol, veh/h		871			587			2361			1584	
Approach Delay, s/veh		42.5			40.5			39.7			35.7	
Approach LOS		D			D			D			D	

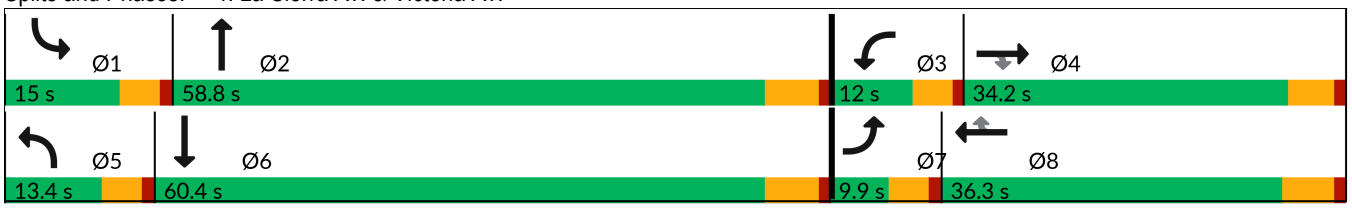
Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	10.3	43.8	9.9	35.0	18.2	35.9	21.2	23.7
Change Period (Y+Rc), s	4.6	5.8	4.6	5.4	4.6	5.8	4.6	5.4
Max Green Setting (Gmax), s	5.7	38.2	7.4	48.3	13.6	30.3	17.7	38.0
Max Q Clear Time (g_c+I1), s	6.4	35.3	5.5	7.9	13.7	21.6	16.4	15.0
Green Ext Time (p_c), s	0.0	2.6	0.0	1.9	0.0	6.2	0.2	2.2

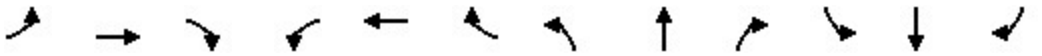
Intersection Summary	
HCM 7th Control Delay, s/veh	39.1
HCM 7th LOS	D

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations										
Traffic Volume (vph)	54	82	59	81	99	225	44	1306	120	639
Future Volume (vph)	54	82	59	81	99	225	44	1306	120	639
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Prot	NA
Protected Phases	7	4		3	8		5	2	1	6
Permitted Phases			4			8				
Detector Phase	7	4	4	3	8	8	5	2	1	6
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	5.0	10.0
Minimum Split (s)	9.6	34.1	34.1	9.6	33.8	33.8	9.6	35.8	9.6	33.8
Total Split (s)	9.9	34.2	34.2	12.0	36.3	36.3	13.4	58.8	15.0	60.4
Total Split (%)	8.3%	28.5%	28.5%	10.0%	30.3%	30.3%	11.2%	49.0%	12.5%	50.3%
Yellow Time (s)	3.6	4.1	4.1	3.6	4.8	4.8	3.6	4.8	3.6	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.1	5.1	4.6	5.8	5.8	4.6	5.8	4.6	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	None	Min
Act Effct Green (s)	5.3	14.2	14.2	7.4	17.6	17.6	7.1	53.3	10.3	58.5
Actuated g/C Ratio	0.05	0.13	0.13	0.07	0.17	0.17	0.07	0.51	0.10	0.56
v/c Ratio	0.36	0.37	0.22	0.73	0.36	0.64	0.42	0.97	0.78	0.39
Control Delay (s/veh)	56.5	44.8	3.1	82.4	42.7	22.5	59.5	42.4	77.9	15.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	56.5	44.8	3.1	82.4	42.7	22.5	59.5	42.4	77.9	15.6
LOS	E	D	A	F	D	C	E	D	E	B
Approach Delay (s/veh)		35.5			39.4			42.8		25.0
Approach LOS		D			D			D		C

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 105.3	
Natural Cycle: 120	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.97	
Intersection Signal Delay (s/veh): 37.1	Intersection LOS: D
Intersection Capacity Utilization 76.3%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 4: La Sierra Av. & Victoria Av.





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	54	82	59	81	99	225	44	1306	220	120	639	33
Future Volume (veh/h)	54	82	59	81	99	225	44	1306	220	120	639	33
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		0.98	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	61	92	21	91	111	135	49	1467	185	135	718	20
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	142	209	175	115	253	210	67	1653	206	165	2030	57
Arrive On Green	0.04	0.11	0.11	0.06	0.14	0.14	0.04	0.52	0.52	0.09	0.57	0.57
Sat Flow, veh/h	3456	1870	1563	1781	1870	1550	1781	3179	397	1781	3531	98
Grp Volume(v), veh/h	61	92	21	91	111	135	49	813	839	135	361	377
Grp Sat Flow(s),veh/h/ln	1728	1870	1563	1781	1870	1550	1781	1777	1799	1781	1777	1853
Q Serve(g_s), s	1.7	4.5	1.2	5.0	5.4	8.1	2.7	40.0	41.4	7.3	10.7	10.7
Cycle Q Clear(g_c), s	1.7	4.5	1.2	5.0	5.4	8.1	2.7	40.0	41.4	7.3	10.7	10.7
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.22	1.00		0.05
Lane Grp Cap(c), veh/h	142	209	175	115	253	210	67	924	935	165	1022	1065
V/C Ratio(X)	0.43	0.44	0.12	0.79	0.44	0.64	0.73	0.88	0.90	0.82	0.35	0.35
Avail Cap(c_a), veh/h	186	552	461	134	579	479	159	955	967	188	1022	1065
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	46.1	40.9	39.4	45.4	39.2	40.4	47.0	21.0	21.3	43.9	11.2	11.2
Incr Delay (d2), s/veh	0.8	1.4	0.3	19.8	1.2	3.3	5.7	9.7	11.1	19.3	0.3	0.3
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.7	2.1	0.5	2.7	2.5	3.2	1.3	16.8	17.9	4.0	3.7	3.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	46.9	42.4	39.7	65.3	40.4	43.6	52.7	30.6	32.4	63.2	11.5	11.5
LnGrp LOS	D	D	D	E	D	D	D	C	C	E	B	B
Approach Vol, veh/h		174			337			1701			873	
Approach Delay, s/veh		43.6			48.4			32.1			19.5	
Approach LOS		D			D			C			B	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	13.7	57.1	11.0	16.8	8.3	62.5	8.7	19.2
Change Period (Y+Rc), s	4.6	5.8	4.6	* 5.8	4.6	5.8	4.6	5.8
Max Green Setting (Gmax), s	10.4	53.0	7.4	* 29	8.8	54.6	5.3	30.5
Max Q Clear Time (g_c+I1), s	9.3	43.4	7.0	6.5	4.7	12.7	3.7	10.1
Green Ext Time (p_c), s	0.0	7.9	0.0	0.5	0.0	7.0	0.0	0.9

Intersection Summary	
HCM 7th Control Delay, s/veh	31.0
HCM 7th LOS	C

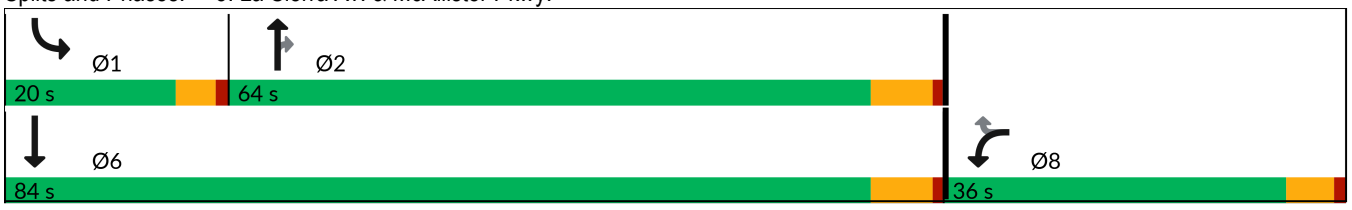
Notes

* HCM 7th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	19	286	1366	23	105	615
Future Volume (vph)	19	286	1366	23	105	615
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	10.0
Minimum Split (s)	33.4	33.4	27.5	27.5	9.6	16.5
Total Split (s)	36.0	36.0	64.0	64.0	20.0	84.0
Total Split (%)	30.0%	30.0%	53.3%	53.3%	16.7%	70.0%
Yellow Time (s)	4.4	4.4	5.5	5.5	3.6	5.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.4	6.5	6.5	4.6	6.5
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	Min	Min	None	Min
Act Effct Green (s)	14.4	14.4	49.7	49.7	10.6	65.1
Actuated g/C Ratio	0.16	0.16	0.54	0.54	0.12	0.71
v/c Ratio	0.08	0.71	0.80	0.03	0.58	0.28
Control Delay (s/veh)	36.4	20.4	22.3	7.5	53.7	5.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	36.4	20.4	22.3	7.5	53.7	5.6
LOS	D	C	C	A	D	A
Approach Delay (s/veh)	21.4		22.0			12.6
Approach LOS	C		C			B

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 91.8	
Natural Cycle: 90	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.80	
Intersection Signal Delay (s/veh): 19.1	Intersection LOS: B
Intersection Capacity Utilization 65.7%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 5: La Sierra Av. & McAllister Pkwy.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	19	286	1366	23	105	615
Future Volume (veh/h)	19	286	1366	23	105	615
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	21	203	1535	23	118	691
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	276	246	1995	890	150	2492
Arrive On Green	0.15	0.15	0.56	0.56	0.08	0.70
Sat Flow, veh/h	1781	1585	3647	1585	1781	3647
Grp Volume(v), veh/h	21	203	1535	23	118	691
Grp Sat Flow(s),veh/h/ln	1781	1585	1777	1585	1781	1777
Q Serve(g_s), s	0.8	10.3	27.6	0.5	5.4	6.0
Cycle Q Clear(g_c), s	0.8	10.3	27.6	0.5	5.4	6.0
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	276	246	1995	890	150	2492
V/C Ratio(X)	0.08	0.83	0.77	0.03	0.79	0.28
Avail Cap(c_a), veh/h	659	586	2471	1102	332	3330
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	29.9	33.9	14.0	8.1	37.2	4.6
Incr Delay (d2), s/veh	0.1	6.9	1.4	0.0	3.5	0.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	4.2	8.6	0.1	2.3	1.3
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	30.0	40.8	15.4	8.1	40.6	4.7
LnGrp LOS	C	D	B	A	D	A
Approach Vol, veh/h	224		1558			809
Approach Delay, s/veh	39.8		15.3			9.9
Approach LOS	D		B			A

Timer - Assigned Phs	1	2			6	8
Phs Duration (G+Y+Rc), s	11.6	52.9			64.5	18.2
Change Period (Y+Rc), s	4.6	6.5			6.5	5.4
Max Green Setting (Gmax), s	15.4	57.5			77.5	30.6
Max Q Clear Time (g_c+I1), s	7.4	29.6			8.0	12.3
Green Ext Time (p_c), s	0.1	16.9			7.0	0.6

Intersection Summary	
HCM 7th Control Delay, s/veh	15.7
HCM 7th LOS	B

Intersection	
Intersection Delay, s/veh	79.9
Intersection LOS	F

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	97	682	115	33	312	136
Future Vol, veh/h	97	682	115	33	312	136
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	102	718	121	35	328	143
Number of Lanes	1	0	2	0	1	1

Approach	WB	NB	SB
Opposing Approach		SB	NB
Opposing Lanes	0	2	2
Conflicting Approach Left	NB		WB
Conflicting Lanes Left	2	0	1
Conflicting Approach Right	SB	WB	
Conflicting Lanes Right	2	1	0
HCM Control Delay, s/veh	126.3	12	21.5
HCM LOS	F	B	C

Lane	NBLn1	NBLn2	WBLn1	SBLn1	SBLn2
Vol Left, %	0%	0%	12%	100%	0%
Vol Thru, %	100%	54%	0%	0%	100%
Vol Right, %	0%	46%	88%	0%	0%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	77	71	779	312	136
LT Vol	0	0	97	312	0
Through Vol	77	38	0	0	136
RT Vol	0	33	682	0	0
Lane Flow Rate	81	75	820	328	143
Geometry Grp	5	5	2	5	5
Degree of Util (X)	0.167	0.149	1.208	0.673	0.273
Departure Headway (Hd)	8.042	7.705	5.302	7.884	7.368
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	449	469	689	463	491
Service Time	5.742	5.405	3.302	5.584	5.068
HCM Lane V/C Ratio	0.18	0.16	1.19	0.708	0.291
HCM Control Delay, s/veh	12.3	11.7	126.3	25.3	12.8
HCM Lane LOS	B	B	F	D	B
HCM 95th-tile Q	0.6	0.5	28.5	4.9	1.1

Intersection	
Intersection Delay, s/veh	15
Intersection LOS	B

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	52	294	647	123	60	74
Future Vol, veh/h	52	294	647	123	60	74
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	54	306	674	128	63	77
Number of Lanes	1	1	2	0	1	0

Approach	EB	WB	SB
Opposing Approach	WB	EB	
Opposing Lanes	2	2	0
Conflicting Approach Left	SB		WB
Conflicting Lanes Left	1	0	2
Conflicting Approach Right		SB	EB
Conflicting Lanes Right	0	1	2
HCM Control Delay, s/veh	13.7	16.3	10.9
HCM LOS	B	C	B

Lane	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1
Vol Left, %	100%	0%	0%	0%	45%
Vol Thru, %	0%	100%	100%	64%	0%
Vol Right, %	0%	0%	0%	36%	55%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	52	294	431	339	134
LT Vol	52	0	0	0	60
Through Vol	0	294	431	216	0
RT Vol	0	0	0	123	74
Lane Flow Rate	54	306	449	353	140
Geometry Grp	5	5	5	5	2
Degree of Util (X)	0.096	0.501	0.678	0.507	0.234
Departure Headway (Hd)	6.395	5.889	5.432	5.175	6.031
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	561	613	669	702	596
Service Time	4.128	3.622	3.132	2.875	4.062
HCM Lane V/C Ratio	0.096	0.499	0.671	0.503	0.235
HCM Control Delay, s/veh	9.8	14.4	18.8	13.1	10.9
HCM Lane LOS	A	B	C	B	B
HCM 95th-tile Q	0.3	2.8	5.3	2.9	0.9

Intersection	
Int Delay, s/veh	5.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	73	296	565	11	31	210
Future Vol, veh/h	73	296	565	11	31	210
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	1	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	79	322	614	12	34	228

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	626	0	-	0	1101	620
Stage 1	-	-	-	-	620	-
Stage 2	-	-	-	-	480	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	956	-	-	-	235	488
Stage 1	-	-	-	-	536	-
Stage 2	-	-	-	-	622	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	956	-	-	-	215	488
Mov Cap-2 Maneuver	-	-	-	-	347	-
Stage 1	-	-	-	-	492	-
Stage 2	-	-	-	-	622	-

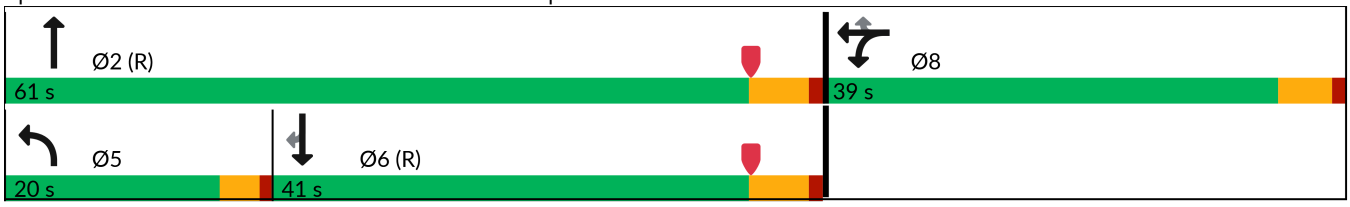
Approach	EB	WB	SB
HCM Control Delay, s/v	1.8	0	22.38
HCM LOS			C

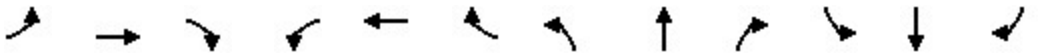
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	956	-	-	-	464
HCM Lane V/C Ratio	0.083	-	-	-	0.565
HCM Control Delay (s/veh)	9.1	-	-	-	22.4
HCM Lane LOS	A	-	-	-	C
HCM 95th %tile Q(veh)	0.3	-	-	-	3.4

Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Configurations							
Traffic Volume (vph)	557	2	613	439	1076	1588	351
Future Volume (vph)	557	2	613	439	1076	1588	351
Turn Type	Split	NA	Perm	Prot	NA	NA	Perm
Protected Phases	8	8		5	2	6	
Permitted Phases			8				6
Detector Phase	8	8	8	5	2	6	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	9.0	26.5	29.5	29.5
Total Split (s)	39.0	39.0	39.0	20.0	61.0	41.0	41.0
Total Split (%)	39.0%	39.0%	39.0%	20.0%	61.0%	41.0%	41.0%
Yellow Time (s)	4.0	4.0	4.0	3.0	4.5	4.5	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	4.0	5.5	5.5	5.5
Lead/Lag				Lead		Lag	Lag
Lead-Lag Optimize?				Yes		Yes	Yes
Recall Mode	None	None	None	None	C-Min	C-Min	C-Min
Act Effct Green (s)	29.8	29.8	29.8	15.9	59.7	39.8	39.8
Actuated g/C Ratio	0.30	0.30	0.30	0.16	0.60	0.40	0.40
v/c Ratio	0.84	0.79	0.76	0.84	0.36	0.80	0.47
Control Delay (s/veh)	47.8	37.7	35.0	38.9	24.5	31.5	11.5
Queue Delay	0.1	0.1	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	47.9	37.8	35.0	38.9	24.5	31.5	11.5
LOS	D	D	D	D	C	C	B
Approach Delay (s/veh)		40.4			28.7	27.9	
Approach LOS		D			C	C	

Intersection Summary	
Cycle Length: 100	
Actuated Cycle Length: 100	
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow, Master Intersection	
Natural Cycle: 75	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.84	
Intersection Signal Delay (s/veh): 31.3	Intersection LOS: C
Intersection Capacity Utilization 78.5%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 1: La Sierra Av. & SR-91 WB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	557	2	613	439	1076	0	0	1588	351
Future Volume (veh/h)	0	0	0	557	2	613	439	1076	0	0	1588	351
Initial Q (Qb), veh				0	0	0	0	0	0	0	0	0
Lane Width Adj.				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach					No			No			No	
Adj Sat Flow, veh/h/ln				1870	1900	1900	1856	1885	0	0	1885	1900
Adj Flow Rate, veh/h				705	0	280	453	1109	0	0	1637	276
Peak Hour Factor				0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %				2	0	0	3	1	0	0	1	0
Cap, veh/h				829	0	375	508	3408	0	0	2440	751
Arrive On Green				0.23	0.00	0.23	0.30	1.00	0.00	0.00	0.47	0.47
Sat Flow, veh/h				3563	0	1610	3428	5316	0	0	5316	1584
Grp Volume(v), veh/h				705	0	280	453	1109	0	0	1637	276
Grp Sat Flow(s),veh/h/ln				1781	0	1610	1714	1716	0	0	1716	1584
Q Serve(g_s), s				18.9	0.0	16.1	12.6	0.0	0.0	0.0	24.5	11.1
Cycle Q Clear(g_c), s				18.9	0.0	16.1	12.6	0.0	0.0	0.0	24.5	11.1
Prop In Lane				1.00		1.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h				829	0	375	508	3408	0	0	2440	751
V/C Ratio(X)				0.85	0.00	0.75	0.89	0.33	0.00	0.00	0.67	0.37
Avail Cap(c_a), veh/h				1211	0	547	549	3408	0	0	2440	751
HCM Platoon Ratio				1.00	1.00	1.00	2.00	2.00	1.00	1.00	1.00	1.00
Upstream Filter(I)				1.00	0.00	1.00	0.57	0.57	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh				36.7	0.0	35.6	34.4	0.0	0.0	0.0	20.3	16.7
Incr Delay (d2), s/veh				3.5	0.0	2.5	9.5	0.1	0.0	0.0	1.5	1.4
Initial Q Delay(d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln				8.2	0.0	6.3	4.9	0.0	0.0	0.0	9.2	4.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh				40.2	0.0	38.1	44.0	0.1	0.0	0.0	21.8	18.1
LnGrp LOS				D		D	D	A			C	B
Approach Vol, veh/h					985			1562			1913	
Approach Delay, s/veh					39.6			12.9			21.2	
Approach LOS					D			B			C	

Timer - Assigned Phs	2		5	6		8						
Phs Duration (G+Y+Rc), s	71.7		18.8	52.9		28.3						
Change Period (Y+Rc), s	5.5		4.0	5.5		5.0						
Max Green Setting (Gmax), s	55.5		16.0	35.5		34.0						
Max Q Clear Time (g_c+I1), s	2.0		14.6	26.5		20.9						
Green Ext Time (p_c), s	9.2		0.2	6.7		2.4						

Intersection Summary	
HCM 7th Control Delay, s/veh	22.4
HCM 7th LOS	C

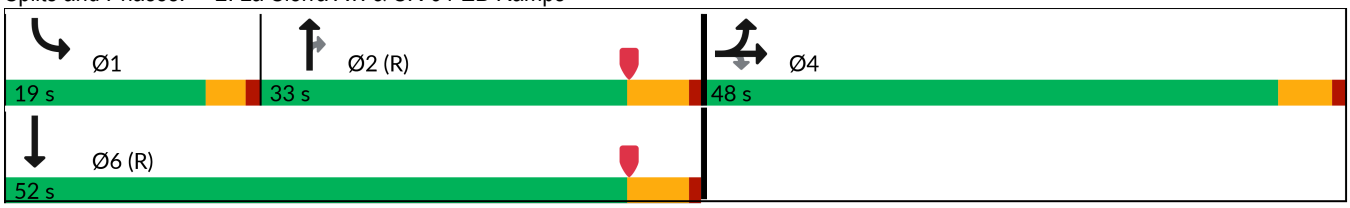
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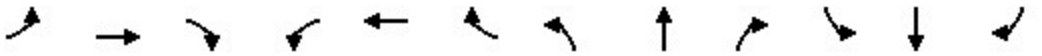
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	270	11	916	1231	351	498	1651
Future Volume (vph)	270	11	916	1231	351	498	1651
Turn Type	Split	NA	Perm	NA	Perm	Prot	NA
Protected Phases	4	4		2		1	6
Permitted Phases			4		2		
Detector Phase	4	4	4	2	2	1	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	10.0	10.0	29.5	29.5	9.0	26.5
Total Split (s)	48.0	48.0	48.0	33.0	33.0	19.0	52.0
Total Split (%)	48.0%	48.0%	48.0%	33.0%	33.0%	19.0%	52.0%
Yellow Time (s)	4.0	4.0	4.0	4.5	4.5	3.0	4.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.5	5.5	4.0	5.5
Lead/Lag				Lag	Lag	Lead	
Lead-Lag Optimize?				Yes	Yes	Yes	
Recall Mode	None	None	None	C-Min	C-Min	None	C-Min
Act Effct Green (s)	38.5	38.5	38.5	29.9	29.9	17.2	51.0
Actuated g/C Ratio	0.39	0.39	0.39	0.30	0.30	0.17	0.51
v/c Ratio	0.38	0.87	0.79	0.83	0.50	0.86	0.64
Control Delay (s/veh)	23.2	45.2	32.5	39.1	5.8	36.2	12.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.4
Total Delay (s/veh)	23.2	45.2	32.5	39.1	5.8	36.2	12.9
LOS	C	D	C	D	A	D	B
Approach Delay (s/veh)		35.6		31.7			18.3
Approach LOS		D		C			B

Intersection Summary	
Cycle Length: 100	
Actuated Cycle Length: 100	
Offset: 28 (28%), Referenced to phase 2:NBT and 6:SBT, Start of Yellow	
Natural Cycle: 80	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.87	
Intersection Signal Delay (s/veh): 26.8	Intersection LOS: C
Intersection Capacity Utilization 78.5%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 2: La Sierra Av. & SR-91 EB Ramps





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	270	11	916	0	0	0	0	1231	351	498	1651	0
Future Volume (veh/h)	270	11	916	0	0	0	0	1231	351	498	1651	0
Initial Q (Qb), veh	0	0	0				0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		0.98	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No						No			No	
Adj Sat Flow, veh/h/ln	1870	1752	1856				0	1870	1885	1870	1885	0
Adj Flow Rate, veh/h	188	0	801				0	1256	220	508	1685	0
Peak Hour Factor	0.98	0.98	0.98				0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	2	10	3				0	2	1	2	1	0
Cap, veh/h	518	0	914				0	2116	651	518	3110	0
Arrive On Green	0.29	0.00	0.29				0.00	0.41	0.41	0.05	0.20	0.00
Sat Flow, veh/h	1781	0	3145				0	5274	1572	3456	5316	0
Grp Volume(v), veh/h	188	0	801				0	1256	220	508	1685	0
Grp Sat Flow(s),veh/h/ln	1781	0	1572				0	1702	1572	1728	1716	0
Q Serve(g_s), s	8.4	0.0	24.2				0.0	19.1	9.5	14.7	29.4	0.0
Cycle Q Clear(g_c), s	8.4	0.0	24.2				0.0	19.1	9.5	14.7	29.4	0.0
Prop In Lane	1.00		1.00				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	518	0	914				0	2116	651	518	3110	0
V/C Ratio(X)	0.36	0.00	0.88				0.00	0.59	0.34	0.98	0.54	0.00
Avail Cap(c_a), veh/h	766	0	1352				0	2116	651	518	3110	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	0.33	0.33	1.00
Upstream Filter(I)	1.00	0.00	1.00				0.00	0.61	0.61	0.49	0.49	0.00
Uniform Delay (d), s/veh	28.1	0.0	33.8				0.0	22.7	19.9	47.4	27.6	0.0
Incr Delay (d2), s/veh	0.3	0.0	4.1				0.0	0.8	0.9	22.8	0.3	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.4	0.0	9.2				0.0	7.2	3.4	8.4	13.4	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	28.4	0.0	37.8				0.0	23.5	20.8	70.2	27.9	0.0
LnGrp LOS	C		D					C	C	E	C	
Approach Vol, veh/h		989						1476			2193	
Approach Delay, s/veh		36.1						23.1			37.7	
Approach LOS		D						C			D	

Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	19.0	46.9		34.1		65.9
Change Period (Y+Rc), s	4.0	5.5		5.0		5.5
Max Green Setting (Gmax), s	15.0	27.5		43.0		46.5
Max Q Clear Time (g_c+I1), s	16.7	21.1		26.2		31.4
Green Ext Time (p_c), s	0.0	4.2		2.8		9.6

Intersection Summary	
HCM 7th Control Delay, s/veh	32.7
HCM 7th LOS	C

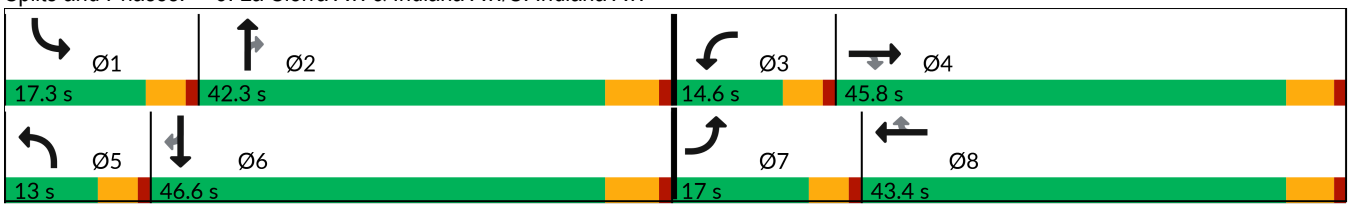
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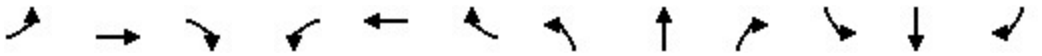
User approved volume balancing among the lanes for turning movement.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	404	364	424	193	201	270	250	976	136	307	1741	404
Future Volume (vph)	404	364	424	193	201	270	250	976	136	307	1741	404
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4			8			2			6
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	9.6	43.4	43.4	9.6	43.4	43.4	9.6	37.8	37.8	9.6	32.8	32.8
Total Split (s)	17.0	45.8	45.8	14.6	43.4	43.4	13.0	42.3	42.3	17.3	46.6	46.6
Total Split (%)	14.2%	38.2%	38.2%	12.2%	36.2%	36.2%	10.8%	35.3%	35.3%	14.4%	38.8%	38.8%
Yellow Time (s)	3.6	4.4	4.4	3.6	4.4	4.4	3.6	4.8	4.8	3.6	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.4	5.4	4.6	5.4	5.4	4.6	5.8	5.8	4.6	5.8	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	Min	None	Min	Min
Act Effct Green (s)	12.5	28.3	28.3	9.4	25.2	25.2	8.5	37.1	37.1	12.5	41.1	41.1
Actuated g/C Ratio	0.12	0.26	0.26	0.09	0.23	0.23	0.08	0.34	0.34	0.12	0.38	0.38
v/c Ratio	1.11	0.43	0.86	0.70	0.26	0.59	1.01	0.61	0.23	0.84	0.98	0.51
Control Delay (s/veh)	121.9	33.8	39.6	62.8	33.3	19.0	108.9	32.6	5.9	67.3	49.9	5.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	121.9	33.8	39.6	62.8	33.3	19.0	108.9	32.6	5.9	67.3	49.9	5.0
LOS	F	C	D	E	C	B	F	C	A	E	D	A
Approach Delay (s/veh)		65.7			36.1			44.0			44.7	
Approach LOS		E			D			D			D	

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 107.9	
Natural Cycle: 115	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 1.11	
Intersection Signal Delay (s/veh): 47.9	Intersection LOS: D
Intersection Capacity Utilization 80.7%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 3: La Sierra Av. & Indiana Av./S. Indiana Av.





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	404	364	424	193	201	270	250	976	136	307	1741	404
Future Volume (veh/h)	404	364	424	193	201	270	250	976	136	307	1741	404
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		0.98	1.00		0.99	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	439	396	298	210	218	228	272	1061	88	334	1892	314
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	412	896	390	274	754	330	279	1826	564	396	1999	606
Arrive On Green	0.12	0.32	0.25	0.08	0.27	0.21	0.08	0.45	0.36	0.11	0.49	0.39
Sat Flow, veh/h	3456	3554	1548	3456	3554	1558	3456	5106	1577	3456	5106	1549
Grp Volume(v), veh/h	439	396	298	210	218	228	272	1061	88	334	1892	314
Grp Sat Flow(s),veh/h/ln	1728	1777	1548	1728	1777	1558	1728	1702	1577	1728	1702	1549
Q Serve(g_s), s	12.4	9.2	18.5	6.2	5.1	14.0	8.2	16.1	3.9	9.8	36.6	16.1
Cycle Q Clear(g_c), s	12.4	9.2	18.5	6.2	5.1	14.0	8.2	16.1	3.9	9.8	36.6	16.1
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	412	896	390	274	754	330	279	1826	564	396	1999	606
V/C Ratio(X)	1.06	0.44	0.76	0.77	0.29	0.69	0.97	0.58	0.16	0.84	0.95	0.52
Avail Cap(c_a), veh/h	412	1381	602	332	1299	570	279	1826	564	422	2004	608
HCM Platoon Ratio	1.00	1.25	1.00	1.00	1.25	1.00	1.00	1.25	1.00	1.00	1.25	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	45.8	29.8	36.0	46.9	32.0	37.8	47.7	22.9	22.7	45.1	25.5	24.1
Incr Delay (d2), s/veh	62.6	0.3	3.1	6.5	0.2	2.6	46.3	0.6	0.2	12.7	10.3	1.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	8.7	3.7	7.1	2.8	2.1	5.4	5.2	5.6	1.4	4.8	14.0	5.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	108.4	30.1	39.1	53.4	32.2	40.4	94.0	23.5	22.9	57.8	35.8	25.2
LnGrp LOS	F	C	D	D	C	D	F	C	C	E	D	C
Approach Vol, veh/h		1133			656			1421			2540	
Approach Delay, s/veh		62.8			41.8			36.9			37.4	
Approach LOS		E			D			D			D	

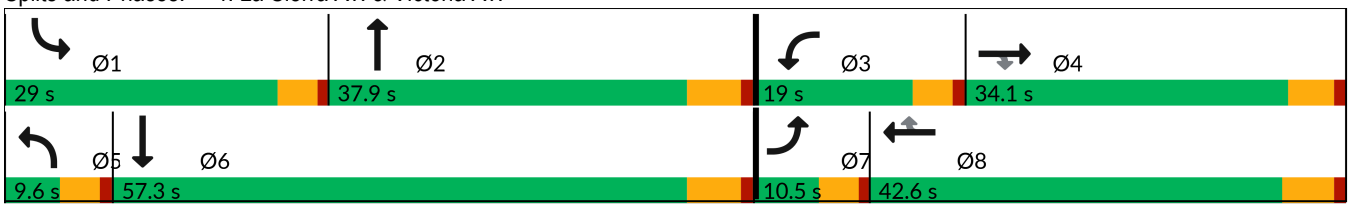
Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	16.5	43.0	12.8	31.6	13.0	46.5	17.0	27.4
Change Period (Y+Rc), s	4.6	5.8	4.6	5.4	4.6	5.8	4.6	5.4
Max Green Setting (Gmax), s	12.7	36.5	10.0	40.4	8.4	40.8	12.4	38.0
Max Q Clear Time (g_c+I1), s	11.8	18.1	8.2	20.5	10.2	38.6	14.4	16.0
Green Ext Time (p_c), s	0.1	9.2	0.1	3.4	0.0	2.1	0.0	2.0

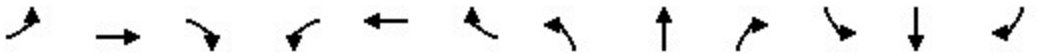
Intersection Summary	
HCM 7th Control Delay, s/veh	42.8
HCM 7th LOS	D

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Configurations										
Traffic Volume (vph)	67	110	156	305	80	191	29	901	212	1722
Future Volume (vph)	67	110	156	305	80	191	29	901	212	1722
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Prot	NA
Protected Phases	7	4		3	8		5	2	1	6
Permitted Phases			4			8				
Detector Phase	7	4	4	3	8	8	5	2	1	6
Switch Phase										
Minimum Initial (s)	5.0	10.0	10.0	5.0	10.0	10.0	5.0	10.0	5.0	10.0
Minimum Split (s)	9.6	34.1	34.1	9.6	33.8	33.8	9.6	35.8	9.6	33.8
Total Split (s)	10.5	34.1	34.1	19.0	42.6	42.6	9.6	37.9	29.0	57.3
Total Split (%)	8.8%	28.4%	28.4%	15.8%	35.5%	35.5%	8.0%	31.6%	24.2%	47.8%
Yellow Time (s)	3.6	4.1	4.1	3.6	4.8	4.8	3.6	4.8	3.6	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.1	5.1	4.6	5.8	5.8	4.6	5.8	4.6	5.8
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	Min	None	Min
Act Effct Green (s)	5.7	14.4	14.4	14.5	24.6	24.6	5.1	36.0	16.7	52.0
Actuated g/C Ratio	0.06	0.14	0.14	0.14	0.24	0.24	0.05	0.35	0.16	0.51
v/c Ratio	0.36	0.45	0.43	1.29	0.19	0.38	0.36	0.87	0.78	1.02
Control Delay (s/veh)	55.0	45.9	6.9	192.9	33.8	6.7	62.6	40.5	60.2	52.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	55.0	45.9	6.9	192.9	33.8	6.7	62.6	40.5	60.2	52.6
LOS	D	D	A	F	C	A	E	D	E	D
Approach Delay (s/veh)		29.4			109.0			41.1		53.4
Approach LOS		C			F			D		D

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 102	
Natural Cycle: 120	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 1.29	
Intersection Signal Delay (s/veh): 56.1	Intersection LOS: E
Intersection Capacity Utilization 91.1%	ICU Level of Service F
Analysis Period (min) 15	

Splits and Phases: 4: La Sierra Av. & Victoria Av.





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	67	110	156	305	80	191	29	901	151	212	1722	72
Future Volume (veh/h)	67	110	156	305	80	191	29	901	151	212	1722	72
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		0.99	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	71	117	60	324	85	59	31	959	55	226	1832	24
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	153	188	157	258	379	317	52	1389	80	260	1890	25
Arrive On Green	0.04	0.10	0.10	0.15	0.20	0.20	0.03	0.60	0.40	0.15	0.77	0.52
Sat Flow, veh/h	3527	1870	1559	1781	1870	1563	1781	3486	200	1781	3665	48
Grp Volume(v), veh/h	71	117	60	324	85	59	31	509	505	226	924	932
Grp Sat Flow(s),veh/h/ln	1763	1870	1559	1781	1870	1563	1781	1852	1834	1781	1852	1862
Q Serve(g_s), s	2.0	6.0	3.6	14.4	3.8	3.1	1.7	18.7	19.2	12.3	44.6	45.4
Cycle Q Clear(g_c), s	2.0	6.0	3.6	14.4	3.8	3.1	1.7	18.7	19.2	12.3	44.6	45.4
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.11	1.00		0.03
Lane Grp Cap(c), veh/h	153	188	157	258	379	317	52	738	731	260	955	960
V/C Ratio(X)	0.47	0.62	0.38	1.25	0.22	0.19	0.60	0.69	0.69	0.87	0.97	0.97
Avail Cap(c_a), veh/h	210	547	456	258	694	579	90	738	731	438	961	966
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.50	1.00	1.00	1.50	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	46.3	42.8	41.7	42.4	33.0	32.8	47.6	15.8	16.7	41.4	10.5	10.9
Incr Delay (d2), s/veh	0.8	3.3	1.5	141.8	0.3	0.3	4.1	3.0	3.1	4.6	21.5	22.2
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9	2.9	1.4	16.2	1.7	1.2	0.8	6.0	6.4	5.5	11.2	11.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	47.2	46.1	43.2	184.2	33.3	33.1	51.7	18.8	19.8	46.1	32.0	33.1
LnGrp LOS	D	D	D	F	C	C	D	B	B	D	C	C
Approach Vol, veh/h		248			468			1045			2082	
Approach Delay, s/veh		45.7			137.7			20.3			34.0	
Approach LOS		D			F			C			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	19.1	45.3	19.0	15.8	7.5	57.0	8.9	25.9
Change Period (Y+Rc), s	4.6	5.8	4.6	* 5.8	4.6	5.8	4.6	5.8
Max Green Setting (Gmax), s	24.4	32.1	14.4	* 29	5.0	51.5	5.9	36.8
Max Q Clear Time (g_c+I1), s	14.3	21.2	16.4	8.0	3.7	47.4	4.0	5.8
Green Ext Time (p_c), s	0.2	5.8	0.0	0.7	0.0	3.8	0.0	0.6

Intersection Summary	
HCM 7th Control Delay, s/veh	43.7
HCM 7th LOS	D

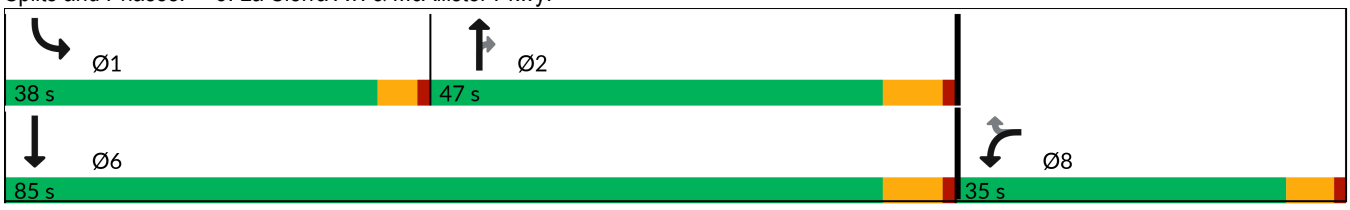
Notes

* HCM 7th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	8	191	891	13	336	1813
Future Volume (vph)	8	191	891	13	336	1813
Turn Type	Prot	Perm	NA	Perm	Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2		
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	10.0	10.0	5.0	10.0
Minimum Split (s)	33.4	33.4	27.5	27.5	9.6	16.5
Total Split (s)	35.0	35.0	47.0	47.0	38.0	85.0
Total Split (%)	29.2%	29.2%	39.2%	39.2%	31.7%	70.8%
Yellow Time (s)	4.4	4.4	5.5	5.5	3.6	5.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.4	5.4	6.5	6.5	4.6	6.5
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	Min	Min	None	Min
Act Effct Green (s)	13.0	13.0	31.8	31.8	22.9	59.5
Actuated g/C Ratio	0.15	0.15	0.37	0.37	0.27	0.70
v/c Ratio	0.03	0.49	0.72	0.02	0.75	0.78
Control Delay (s/veh)	35.0	9.7	27.8	13.0	40.8	11.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	35.0	9.7	27.8	13.0	40.8	11.8
LOS	C	A	C	B	D	B
Approach Delay (s/veh)	10.8		27.6			16.3
Approach LOS	B		C			B

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 85	
Natural Cycle: 90	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.78	
Intersection Signal Delay (s/veh): 19.1	Intersection LOS: B
Intersection Capacity Utilization 68.4%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 5: La Sierra Av. & McAllister Pkwy.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	8	191	891	13	336	1813
Future Volume (veh/h)	8	191	891	13	336	1813
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		0.98	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	9	97	948	14	357	1929
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	200	178	1632	713	399	2632
Arrive On Green	0.11	0.11	0.46	0.46	0.22	0.74
Sat Flow, veh/h	1781	1585	3647	1552	1781	3647
Grp Volume(v), veh/h	9	97	948	14	357	1929
Grp Sat Flow(s),veh/h/ln	1781	1585	1777	1552	1781	1777
Q Serve(g_s), s	0.4	4.7	15.9	0.4	15.7	24.9
Cycle Q Clear(g_c), s	0.4	4.7	15.9	0.4	15.7	24.9
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	200	178	1632	713	399	2632
V/C Ratio(X)	0.05	0.55	0.58	0.02	0.89	0.73
Avail Cap(c_a), veh/h	652	580	1780	778	736	3451
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	32.0	33.9	16.1	11.9	30.4	6.0
Incr Delay (d2), s/veh	0.1	2.6	0.5	0.0	2.9	0.7
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	1.8	5.4	0.1	6.3	4.1
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	32.1	36.5	16.6	11.9	33.3	6.7
LnGrp LOS	C	D	B	B	C	A
Approach Vol, veh/h	106		962			2286
Approach Delay, s/veh	36.1		16.6			10.8
Approach LOS	D		B			B

Timer - Assigned Phs	1	2				6	8
Phs Duration (G+Y+Rc), s	22.7	43.6				66.4	14.5
Change Period (Y+Rc), s	4.6	6.5				6.5	5.4
Max Green Setting (Gmax), s	33.4	40.5				78.5	29.6
Max Q Clear Time (g_c+I1), s	17.7	17.9				26.9	6.7
Green Ext Time (p_c), s	0.4	8.5				33.0	0.3

Intersection Summary	
HCM 7th Control Delay, s/veh	13.3
HCM 7th LOS	B

Intersection	
Intersection Delay, s/veh	222.9
Intersection LOS	F

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	98	458	236	128	882	503
Future Vol, veh/h	98	458	236	128	882	503
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	105	492	254	138	948	541
Number of Lanes	1	0	2	0	1	1

Approach	WB	NB	SB
Opposing Approach		SB	NB
Opposing Lanes	0	2	2
Conflicting Approach Left	NB		WB
Conflicting Lanes Left	2	0	1
Conflicting Approach Right	SB	WB	
Conflicting Lanes Right	2	1	0
HCM Control Delay, s/veh	63.1	16.2	341.4
HCM LOS	F	C	F

Lane	NBLn1	NBLn2	WBLn1	SBLn1	SBLn2
Vol Left, %	0%	0%	18%	100%	0%
Vol Thru, %	100%	38%	0%	0%	100%
Vol Right, %	0%	62%	82%	0%	0%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	157	207	556	882	503
LT Vol	0	0	98	882	0
Through Vol	157	79	0	0	503
RT Vol	0	128	458	0	0
Lane Flow Rate	169	222	598	948	541
Geometry Grp	5	5	2	5	5
Degree of Util (X)	0.368	0.457	1.006	2.021	1.075
Departure Headway (Hd)	8.152	7.702	6.057	7.937	7.421
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	445	471	598	465	492
Service Time	5.852	5.402	4.116	5.637	5.121
HCM Lane V/C Ratio	0.38	0.471	1	2.039	1.1
HCM Control Delay, s/veh	15.5	16.7	63.1	485.3	89.2
HCM Lane LOS	C	C	F	F	F
HCM 95th-tile Q	1.7	2.4	15.1	63.3	16.4

Intersection	
Intersection Delay, s/veh	151.6
Intersection LOS	F

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	64	886	423	50	69	111
Future Vol, veh/h	64	886	423	50	69	111
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	68	943	450	53	73	118
Number of Lanes	1	1	2	0	1	0

Approach	EB	WB	SB
Opposing Approach	WB	EB	
Opposing Lanes	2	2	0
Conflicting Approach Left	SB		WB
Conflicting Lanes Left	1	0	2
Conflicting Approach Right		SB	EB
Conflicting Lanes Right	0	1	2
HCM Control Delay, s/veh	246.2	14	13.6
HCM LOS	F	B	B

Lane	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1
Vol Left, %	100%	0%	0%	0%	38%
Vol Thru, %	0%	100%	100%	74%	0%
Vol Right, %	0%	0%	0%	26%	62%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	64	886	282	191	180
LT Vol	64	0	0	0	69
Through Vol	0	886	282	141	0
RT Vol	0	0	0	50	111
Lane Flow Rate	68	943	300	203	191
Geometry Grp	5	5	5	5	2
Degree of Util (X)	0.12	1.531	0.494	0.324	0.333
Departure Headway (Hd)	6.355	5.848	6.494	6.307	7.108
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	568	632	558	573	509
Service Time	4.055	3.548	4.194	4.007	5.108
HCM Lane V/C Ratio	0.12	1.492	0.538	0.354	0.375
HCM Control Delay, s/veh	9.9	263.3	15.4	12	13.6
HCM Lane LOS	A	F	C	B	B
HCM 95th-tile Q	0.4	48.2	2.7	1.4	1.4

Intersection	
Int Delay, s/veh	3.2

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	241	724	348	36	21	143
Future Vol, veh/h	241	724	348	36	21	143
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	1	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	262	787	378	39	23	155

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	417	0	-	0	1709	398
Stage 1	-	-	-	-	398	-
Stage 2	-	-	-	-	1311	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1142	-	-	-	100	652
Stage 1	-	-	-	-	679	-
Stage 2	-	-	-	-	252	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	1142	-	-	-	77	652
Mov Cap-2 Maneuver	-	-	-	-	185	-
Stage 1	-	-	-	-	523	-
Stage 2	-	-	-	-	252	-

Approach	EB	WB	SB
HCM Control Delay, s/v	2.27	0	16.39
HCM LOS			C

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1142	-	-	-	493
HCM Lane V/C Ratio	0.229	-	-	-	0.362
HCM Control Delay (s/veh)	9.1	-	-	-	16.4
HCM Lane LOS	A	-	-	-	C
HCM 95th %tile Q(veh)	0.9	-	-	-	1.6

APPENDIX 6.2: EAPC (2027) CONDITIONS QUEUING ANALYSIS WORKSHEETS

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Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	295	298	299	734	1560	1115	199
v/c Ratio	0.74	0.74	0.72	0.87	0.47	0.60	0.30
Control Delay (s/veh)	46.1	36.8	35.6	43.0	17.4	29.0	8.6
Queue Delay	0.0	0.0	0.0	0.0	0.7	0.0	0.0
Total Delay (s/veh)	46.1	36.8	35.6	43.0	18.0	29.0	8.6
Queue Length 50th (ft)	183	147	141	257	264	213	16
Queue Length 95th (ft)	253	227	216	311	391	296	75
Internal Link Dist (ft)		1180			412	935	
Turn Bay Length (ft)	500		500	155			140
Base Capacity (vph)	532	519	534	906	3336	1861	657
Starvation Cap Reductn	0	0	0	0	1270	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.55	0.57	0.56	0.81	0.76	0.60	0.30

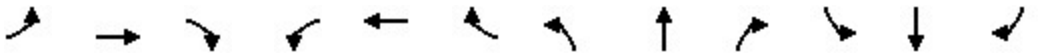
Intersection Summary



Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	339	330	314	1751	570	388	1066
v/c Ratio	0.78	0.79	0.72	0.77	0.56	0.78	0.33
Control Delay (s/veh)	47.2	42.7	33.0	27.1	4.3	58.1	2.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	47.2	42.7	33.0	27.1	4.3	58.1	2.6
Queue Length 50th (ft)	209	184	135	352	0	78	13
Queue Length 95th (ft)	293	275	222	440	66	#171	40
Internal Link Dist (ft)		1416		886			412
Turn Bay Length (ft)			450		415	155	
Base Capacity (vph)	543	516	524	2280	1010	507	3224
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.62	0.64	0.60	0.77	0.56	0.77	0.33

Intersection Summary

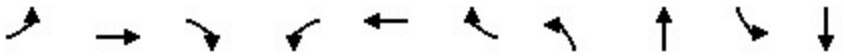
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	514	233	177	123	245	299	418	1805	202	156	1131	522
v/c Ratio	0.84	0.23	0.31	0.52	0.40	0.70	0.89	0.92	0.28	0.79	0.73	0.64
Control Delay (s/veh)	54.1	27.5	5.2	55.0	37.7	22.6	65.1	39.0	6.6	75.5	35.4	8.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	54.1	27.5	5.2	55.0	37.7	22.6	65.1	39.0	6.6	75.5	35.4	8.4
Queue Length 50th (ft)	155	60	0	37	73	60	129	365	9	48	219	15
Queue Length 95th (ft)	#326	88	44	81	106	144	#288	#684	68	#133	#371	134
Internal Link Dist (ft)		2549			2428			2703			886	
Turn Bay Length (ft)	205		205	205		130	200		305	355		535
Base Capacity (vph)	614	1727	853	256	1359	718	472	1963	712	197	1557	810
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.84	0.13	0.21	0.48	0.18	0.42	0.89	0.92	0.28	0.79	0.73	0.64

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	61	92	66	91	111	253	49	1714	135	755
v/c Ratio	0.36	0.37	0.22	0.73	0.36	0.64	0.42	0.97	0.78	0.39
Control Delay (s/veh)	56.5	44.8	3.1	82.4	42.7	22.5	59.5	42.4	77.9	15.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	56.5	44.8	3.1	82.4	42.7	22.5	59.5	42.4	77.9	15.6
Queue Length 50th (ft)	20	57	0	59	68	53	31	517	86	135
Queue Length 95th (ft)	46	102	9	#167	118	131	77	#920	#221	257
Internal Link Dist (ft)		810			1637			2644		1335
Turn Bay Length (ft)	130			205		200	215		210	
Base Capacity (vph)	173	517	512	125	542	570	148	1760	175	1954
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.35	0.18	0.13	0.73	0.20	0.44	0.33	0.97	0.77	0.39

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	21	321	1535	26	118	691
v/c Ratio	0.08	0.71	0.80	0.03	0.58	0.28
Control Delay (s/veh)	36.4	20.4	22.3	7.5	53.7	5.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	36.4	20.4	22.3	7.5	53.7	5.6
Queue Length 50th (ft)	11	44	320	2	66	53
Queue Length 95th (ft)	34	138	633	18	144	133
Internal Link Dist (ft)	3339		1159			2644
Turn Bay Length (ft)	150			200	300	
Base Capacity (vph)	611	704	2298	1033	308	2996
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.46	0.67	0.03	0.38	0.23

Intersection Summary

Intersection: 1: La Sierra Av. & SR-91 WB Ramps

Movement	WB	WB	WB	NB	NB	NB	NB	NB	SB	SB	SB	SB
Directions Served	L	LTR	R	L	L	T	T	T	T	T	T	R
Maximum Queue (ft)	297	349	298	244	329	302	290	291	574	495	260	186
Average Queue (ft)	127	232	185	230	257	199	199	206	360	270	128	60
95th Queue (ft)	245	316	275	275	315	267	263	267	517	439	218	125
Link Distance (ft)		1183			444	444	444	444	949	949	949	
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	500		500	155								140
Storage Blk Time (%)				12	24						5	0
Queuing Penalty (veh)				44	86						9	0

Intersection: 2: La Sierra Av. & SR-91 EB Ramps

Movement	EB	EB	EB	NB	NB	NB	NB	SB	SB	SB	SB	SB
Directions Served	L	LTR	R	T	T	T	R	L	L	T	T	T
Maximum Queue (ft)	620	712	540	760	762	757	458	190	217	164	158	194
Average Queue (ft)	174	432	359	558	505	473	259	125	148	28	36	62
95th Queue (ft)	504	646	576	845	847	817	560	193	212	103	104	141
Link Distance (ft)	1430	1430		848	848	848				444	444	444
Upstream Blk Time (%)				1	0	0				0	0	0
Queuing Penalty (veh)				6	2	3				0	0	0
Storage Bay Dist (ft)			450				415	155	155			
Storage Blk Time (%)		16	1			14	0	4	24	1		
Queuing Penalty (veh)		35	2			76	0	14	83	2		

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	L	T	T	R	L	L	T	T	R	L	L
Maximum Queue (ft)	234	260	479	366	109	75	101	128	229	190	237	275
Average Queue (ft)	169	201	153	94	39	28	52	68	63	98	148	226
95th Queue (ft)	264	285	439	298	83	62	89	110	142	174	226	331
Link Distance (ft)			2554	2554				2432	2432			
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	205	205			205	205	205			130	200	200
Storage Blk Time (%)	7	20	1	0					1	5	2	6
Queuing Penalty (veh)	7	21	5	0					2	6	13	33

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	NB	NB	NB	NB	SB	SB	SB	SB	SB	SB
Directions Served	T	T	T	R	L	L	T	T	T	R
Maximum Queue (ft)	799	761	768	424	92	97	269	275	268	258
Average Queue (ft)	418	410	443	162	42	57	165	172	168	131
95th Queue (ft)	858	834	862	456	79	95	256	259	251	233
Link Distance (ft)	2691	2691	2691				848	848	848	
Upstream Blk Time (%)										
Queuing Penalty (veh)										
Storage Bay Dist (ft)				305	355	355				535
Storage Blk Time (%)	33		28				0			
Queuing Penalty (veh)	127		52				0			

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	L	T	R	L	T	R	L	T	TR	L	T
Maximum Queue (ft)	45	94	152	62	165	186	178	274	632	615	183	220
Average Queue (ft)	11	38	62	29	68	66	85	64	329	357	87	72
95th Queue (ft)	37	81	119	54	142	132	147	207	539	557	156	174
Link Distance (ft)			843	843		1670			2640	2640		1333
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	130	130			205		200	215			210	
Storage Blk Time (%)		0	1		1	0	0		22		0	0
Queuing Penalty (veh)		0	0		3	0	0		10		1	0

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	SB	B20	B20
Directions Served	TR	T	T
Maximum Queue (ft)	228	153	200
Average Queue (ft)	87	7	15
95th Queue (ft)	196	93	149
Link Distance (ft)	1333	2691	2691
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 5: La Sierra Av. & McAllister Pkwy.

Movement	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	R	T	T	R	L	T	T
Maximum Queue (ft)	38	250	370	318	36	158	109	145
Average Queue (ft)	9	116	194	174	7	75	34	53
95th Queue (ft)	31	206	311	287	27	131	85	115
Link Distance (ft)		3360	1207	1207			2640	2640
Upstream Blk Time (%)								
Queuing Penalty (veh)								
Storage Bay Dist (ft)	150				200	300		
Storage Blk Time (%)		5		4				
Queuing Penalty (veh)		1		1				

Network Summary

Network wide Queuing Penalty: 645



Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	419	403	386	453	1109	1637	362
v/c Ratio	0.84	0.79	0.76	0.84	0.36	0.80	0.47
Control Delay (s/veh)	47.8	37.7	35.0	38.9	24.5	31.5	11.5
Queue Delay	0.1	0.1	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	47.9	37.8	35.0	38.9	24.5	31.5	11.5
Queue Length 50th (ft)	253	206	184	157	254	357	62
Queue Length 95th (ft)	367	325	292	m185	294	#427	148
Internal Link Dist (ft)		1180			412	935	
Turn Bay Length (ft)	500		500	155			140
Base Capacity (vph)	571	569	571	558	3065	2044	763
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	6	6	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.74	0.72	0.68	0.81	0.36	0.80	0.47

Intersection Summary

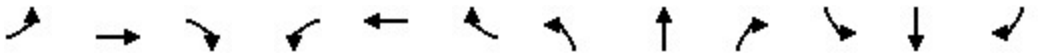
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.



Lane Group	EBL	EBT	EBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	248	488	486	1256	358	508	1685
v/c Ratio	0.38	0.87	0.79	0.83	0.50	0.86	0.64
Control Delay (s/veh)	23.2	45.2	32.5	39.1	5.8	36.2	12.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.4
Total Delay (s/veh)	23.2	45.2	32.5	39.1	5.8	36.2	12.9
Queue Length 50th (ft)	111	291	231	282	0	100	241
Queue Length 95th (ft)	172	#452	360	#364	68	m#244	390
Internal Link Dist (ft)		1416		886			412
Turn Bay Length (ft)			450		415	155	
Base Capacity (vph)	722	623	681	1518	719	588	2620
Starvation Cap Reductn	0	0	0	0	0	0	415
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.34	0.78	0.71	0.83	0.50	0.86	0.76

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.

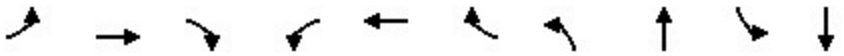


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	439	396	461	210	218	293	272	1061	148	334	1892	439
v/c Ratio	1.11	0.43	0.86	0.68	0.26	0.58	1.01	0.61	0.24	0.82	0.98	0.51
Control Delay (s/veh)	122.8	34.1	39.9	61.3	33.2	18.1	109.4	33.3	6.0	64.5	50.4	5.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	122.8	34.1	39.9	61.3	33.2	18.1	109.4	33.3	6.0	64.5	50.4	5.0
Queue Length 50th (ft)	~184	121	200	74	64	65	~105	230	0	119	480	0
Queue Length 95th (ft)	#322	164	333	#126	96	149	#214	319	48	#213	#703	73
Internal Link Dist (ft)		2549			2428			2703			886	
Turn Bay Length (ft)	205		205	205		130	200		305	355		535
Base Capacity (vph)	396	1313	685	339	1254	668	269	1726	625	429	1935	855
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.11	0.30	0.67	0.62	0.17	0.44	1.01	0.61	0.24	0.78	0.98	0.51

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	71	117	166	324	85	203	31	1120	226	1909
v/c Ratio	0.36	0.45	0.43	1.29	0.19	0.38	0.36	0.87	0.78	1.02
Control Delay (s/veh)	55.0	45.9	6.9	192.9	33.8	6.7	62.6	40.5	60.2	52.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	55.0	45.9	6.9	192.9	33.8	6.7	62.6	40.5	60.2	52.6
Queue Length 50th (ft)	23	74	0	~274	48	0	20	321	143	~684
Queue Length 95th (ft)	52	127	37	#533	88	54	58	#643	248	#1051
Internal Link Dist (ft)		819			1637			2644		1335
Turn Bay Length (ft)	130			205		200	215		210	
Base Capacity (vph)	204	535	586	252	679	698	87	1285	427	1872
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.35	0.22	0.28	1.29	0.13	0.29	0.36	0.87	0.53	1.02

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	9	203	948	14	357	1929
v/c Ratio	0.03	0.49	0.72	0.02	0.75	0.78
Control Delay (s/veh)	35.0	9.7	27.8	13.0	40.8	11.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	35.0	9.7	27.8	13.0	40.8	11.8
Queue Length 50th (ft)	4	0	208	1	160	247
Queue Length 95th (ft)	19	60	422	16	356	636
Internal Link Dist (ft)	3339		1159			2644
Turn Bay Length (ft)	150			200	300	
Base Capacity (vph)	644	705	1763	777	727	3187
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.01	0.29	0.54	0.02	0.49	0.61

Intersection Summary

Intersection: 1: La Sierra Av. & SR-91 WB Ramps

Movement	WB	WB	WB	NB	NB	NB	NB	NB	SB	SB	SB	SB
Directions Served	L	LTR	R	L	L	T	T	T	T	T	T	R
Maximum Queue (ft)	322	407	342	215	222	237	234	227	990	965	930	190
Average Queue (ft)	199	284	226	134	161	168	172	173	891	829	582	163
95th Queue (ft)	295	367	320	195	214	225	226	224	1114	1085	1071	249
Link Distance (ft)		1183			444	444	444	444	949	949	949	
Upstream Blk Time (%)									46	6	8	
Queuing Penalty (veh)									0	0	0	
Storage Bay Dist (ft)	500		500	155								140
Storage Blk Time (%)				1	7						41	0
Queuing Penalty (veh)				2	16						146	2

Intersection: 2: La Sierra Av. & SR-91 EB Ramps

Movement	EB	EB	EB	NB	NB	NB	NB	SB	SB	SB	SB	SB
Directions Served	L	LTR	R	T	T	T	R	L	L	T	T	T
Maximum Queue (ft)	527	723	524	494	451	423	162	191	227	398	400	413
Average Queue (ft)	107	405	342	263	221	222	75	121	152	153	158	175
95th Queue (ft)	443	679	558	450	373	350	125	184	218	348	343	346
Link Distance (ft)	1430	1430		848	848	848				444	444	444
Upstream Blk Time (%)										1	1	1
Queuing Penalty (veh)										9	5	6
Storage Bay Dist (ft)			450				415	155	155			
Storage Blk Time (%)		11	4			0		2	11	8		
Queuing Penalty (veh)		50	22			2		9	63	38		

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	L	T	T	R	L	L	T	T	R	L	L
Maximum Queue (ft)	235	265	1012	953	355	130	136	123	180	178	187	232
Average Queue (ft)	218	250	517	419	171	59	81	57	55	76	100	114
95th Queue (ft)	270	299	1174	1094	288	105	123	101	116	139	162	185
Link Distance (ft)			2554	2554				2432	2432			
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	205	205			205	205	205			130	200	200
Storage Blk Time (%)	16	59	3	0	4				0	2	0	1
Queuing Penalty (veh)	30	107	11	2	8				1	2	1	3

Intersection: 3: La Sierra Av. & Indiana Av./S. Indiana Av.

Movement	NB	NB	NB	NB	SB	SB	SB	SB	SB	SB
Directions Served	T	T	T	R	L	L	T	T	T	R
Maximum Queue (ft)	263	250	299	99	242	445	868	887	880	660
Average Queue (ft)	136	143	172	36	118	317	591	607	617	353
95th Queue (ft)	229	235	274	76	217	581	968	978	994	823
Link Distance (ft)	2691	2691	2691				848	848	848	
Upstream Blk Time (%)							2	3	5	
Queuing Penalty (veh)							19	23	45	
Storage Bay Dist (ft)				305	355	355				535
Storage Blk Time (%)	2		0		0	0	39		29	1
Queuing Penalty (veh)	6		0		0	2	120		118	9

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	L	T	R	L	T	R	L	T	TR	L	T
Maximum Queue (ft)	52	121	211	151	305	1542	330	233	507	562	275	1343
Average Queue (ft)	13	42	79	70	293	962	217	33	244	269	228	922
95th Queue (ft)	41	88	155	127	341	1848	441	128	434	468	342	1549
Link Distance (ft)			852	852		1670			2639	2639		1333
Upstream Blk Time (%)						13						11
Queuing Penalty (veh)						0						126
Storage Bay Dist (ft)	130	130			205		200	215			210	
Storage Blk Time (%)		0	3		79	2	0		15		3	44
Queuing Penalty (veh)		0	2		214	9	2		4		22	93

Intersection: 4: La Sierra Av. & Victoria Av.

Movement	SB	B20	B20	B20
Directions Served	TR	T	T	
Maximum Queue (ft)	1336	2354	2651	439
Average Queue (ft)	936	319	323	86
95th Queue (ft)	1560	1354	1348	468
Link Distance (ft)	1333	2691	2691	2691
Upstream Blk Time (%)	11	0	0	
Queuing Penalty (veh)	126	0	0	
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 5: La Sierra Av. & McAllister Pkwy.

Movement	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	R	T	T	R	L	T	T
Maximum Queue (ft)	28	123	273	257	31	311	216	222
Average Queue (ft)	5	50	129	106	4	172	65	82
95th Queue (ft)	21	95	225	205	21	274	160	184
Link Distance (ft)		3360	1207	1207			2639	2639
Upstream Blk Time (%)								
Queuing Penalty (veh)								
Storage Bay Dist (ft)	150				200	300		
Storage Blk Time (%)		0		1		1		
Queuing Penalty (veh)		0		0		6		

Network Summary

Network wide Queuing Penalty: 1480

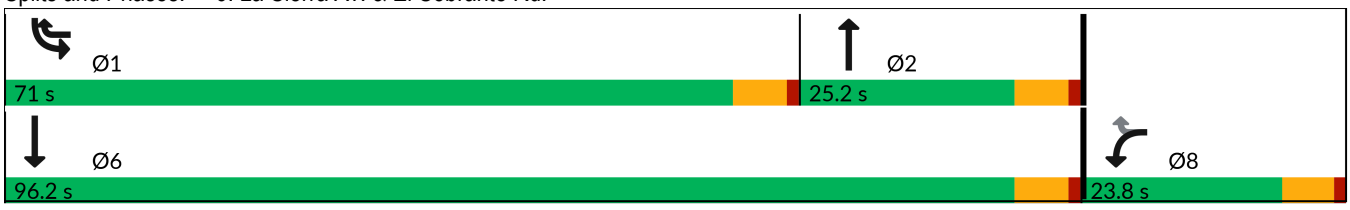
APPENDIX 6.3: EAPC (2027) CONDITIONS INTERSECTION OPERATIONS ANALYSIS WORKSHEETS WITH IMPROVEMENTS

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Lane Group	WBL	WBR	NBT	SBL	SBT
Lane Configurations					
Traffic Volume (vph)	97	682	115	312	136
Future Volume (vph)	97	682	115	312	136
Turn Type	Prot	pm+ov	NA	Prot	NA
Protected Phases	8	1	2	1	6
Permitted Phases		8			
Detector Phase	8	1	2	1	6
Switch Phase					
Minimum Initial (s)	5.0	5.0	10.0	5.0	10.0
Minimum Split (s)	23.8	10.8	23.8	10.8	23.8
Total Split (s)	23.8	71.0	25.2	71.0	96.2
Total Split (%)	19.8%	59.2%	21.0%	59.2%	80.2%
Yellow Time (s)	4.8	4.8	4.8	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.8	5.8	5.8	5.8	5.8
Lead/Lag		Lead	Lag	Lead	
Lead-Lag Optimize?		Yes	Yes	Yes	
Recall Mode	None	None	None	None	None
Act Effct Green (s)	8.7	26.5	12.0	15.1	24.6
Actuated g/C Ratio	0.17	0.52	0.23	0.29	0.48
v/c Ratio	0.34	0.70	0.19	0.63	0.16
Control Delay (s/veh)	24.8	7.4	17.4	23.5	6.7
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	24.8	7.4	17.4	23.5	6.7
LOS	C	A	B	C	A
Approach Delay (s/veh)	9.5		17.4		18.4
Approach LOS	A		B		B

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 51.2	
Natural Cycle: 70	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.70	
Intersection Signal Delay (s/veh): 13.3	Intersection LOS: B
Intersection Capacity Utilization 60.2%	ICU Level of Service B
Analysis Period (min) 15	

Splits and Phases: 6: La Sierra Av. & El Sobrante Rd.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	97	682	115	33	312	136
Future Volume (veh/h)	97	682	115	33	312	136
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	102	418	121	26	328	143
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	435	739	518	108	396	969
Arrive On Green	0.24	0.24	0.18	0.18	0.22	0.52
Sat Flow, veh/h	1781	1585	3018	612	1781	1870
Grp Volume(v), veh/h	102	418	72	75	328	143
Grp Sat Flow(s),veh/h/ln	1781	1585	1777	1760	1781	1870
Q Serve(g_s), s	2.2	9.3	1.7	1.8	8.6	1.9
Cycle Q Clear(g_c), s	2.2	9.3	1.7	1.8	8.6	1.9
Prop In Lane	1.00	1.00		0.35	1.00	
Lane Grp Cap(c), veh/h	435	739	315	312	396	969
V/C Ratio(X)	0.23	0.57	0.23	0.24	0.83	0.15
Avail Cap(c_a), veh/h	657	937	707	700	2381	3466
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	14.8	9.4	17.2	17.3	18.1	6.1
Incr Delay (d2), s/veh	0.1	0.3	0.4	0.4	1.7	0.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.7	2.1	0.6	0.6	2.8	0.4
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	14.9	9.7	17.6	17.6	19.8	6.2
LnGrp LOS	B	A	B	B	B	A
Approach Vol, veh/h	520		147			471
Approach Delay, s/veh	10.7		17.6			15.7
Approach LOS	B		B			B

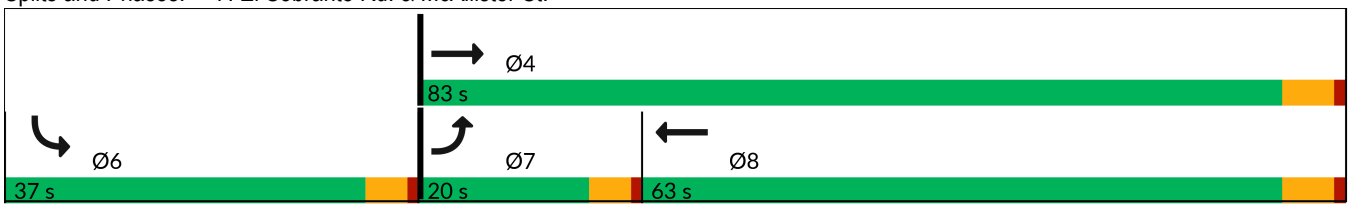
Timer - Assigned Phs	1	2				6	8
Phs Duration (G+Y+Rc), s	16.6	14.4				31.1	17.7
Change Period (Y+Rc), s	5.8	5.8				5.8	5.8
Max Green Setting (Gmax), s	65.2	19.4				90.4	18.0
Max Q Clear Time (g_c+I1), s	10.6	3.8				3.9	11.3
Green Ext Time (p_c), s	0.4	0.5				0.7	0.6

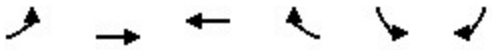
Intersection Summary	
HCM 7th Control Delay, s/veh	13.7
HCM 7th LOS	B

Lane Group	EBL	EBT	WBT	SBL
Lane Configurations				
Traffic Volume (vph)	52	294	647	60
Future Volume (vph)	52	294	647	60
Turn Type	Prot	NA	NA	Prot
Protected Phases	7	4	8	6
Permitted Phases				
Detector Phase	7	4	8	6
Switch Phase				
Minimum Initial (s)	5.0	10.0	10.0	5.0
Minimum Split (s)	10.8	23.8	23.8	23.8
Total Split (s)	20.0	83.0	63.0	37.0
Total Split (%)	16.7%	69.2%	52.5%	30.8%
Yellow Time (s)	3.6	4.8	4.8	3.6
All-Red Time (s)	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.8	5.8	4.6
Lead/Lag	Lead		Lag	
Lead-Lag Optimize?	Yes		Yes	
Recall Mode	None	None	None	None
Act Effct Green (s)	7.6	28.1	22.5	9.5
Actuated g/C Ratio	0.18	0.66	0.53	0.22
v/c Ratio	0.17	0.25	0.44	0.34
Control Delay (s/veh)	23.8	5.8	11.7	15.6
Queue Delay	0.0	0.0	0.0	0.0
Total Delay (s/veh)	23.8	5.8	11.7	15.6
LOS	C	A	B	B
Approach Delay (s/veh)		8.5	11.7	15.6
Approach LOS		A	B	B

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 42.7	
Natural Cycle: 60	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.44	
Intersection Signal Delay (s/veh): 11.2	Intersection LOS: B
Intersection Capacity Utilization 46.3%	ICU Level of Service A
Analysis Period (min) 15	

Splits and Phases: 7: El Sobrante Rd. & McAllister St.





Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	52	294	647	123	60	74
Future Volume (veh/h)	52	294	647	123	60	74
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	54	306	674	128	62	77
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	105	1095	1166	221	80	99
Arrive On Green	0.06	0.59	0.39	0.39	0.11	0.11
Sat Flow, veh/h	1781	1870	3074	565	739	918
Grp Volume(v), veh/h	54	306	402	400	140	0
Grp Sat Flow(s),veh/h/ln	1781	1870	1777	1769	1668	0
Q Serve(g_s), s	1.0	2.8	6.0	6.0	2.8	0.0
Cycle Q Clear(g_c), s	1.0	2.8	6.0	6.0	2.8	0.0
Prop In Lane	1.00			0.32	0.44	0.55
Lane Grp Cap(c), veh/h	105	1095	695	692	180	0
V/C Ratio(X)	0.52	0.28	0.58	0.58	0.78	0.00
Avail Cap(c_a), veh/h	809	4256	2996	2982	1593	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	15.5	3.5	8.1	8.1	14.7	0.0
Incr Delay (d2), s/veh	1.5	0.1	0.8	0.8	2.7	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	0.2	1.3	1.3	0.9	0.0
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	17.0	3.6	8.9	8.9	17.5	0.0
LnGrp LOS	B	A	A	A	B	
Approach Vol, veh/h		360	802		140	
Approach Delay, s/veh		5.6	8.9		17.5	
Approach LOS		A	A		B	

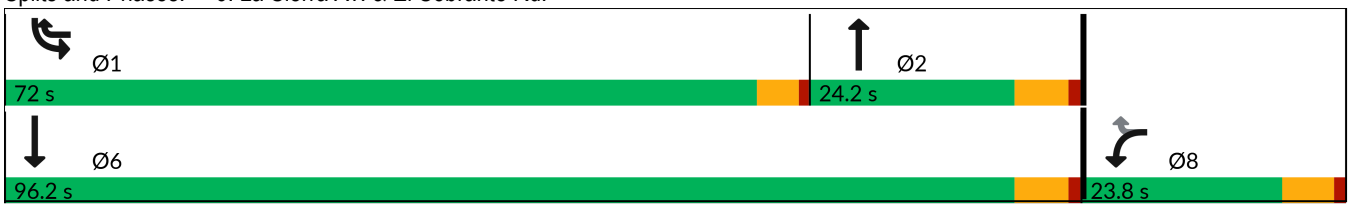
Timer - Assigned Phs			4		6	7	8
Phs Duration (G+Y+Rc), s			25.7		8.3	6.6	19.1
Change Period (Y+Rc), s			5.8		4.6	4.6	5.8
Max Green Setting (Gmax), s			77.2		32.4	15.4	57.2
Max Q Clear Time (g_c+I1), s			4.8		4.8	3.0	8.0
Green Ext Time (p_c), s			1.8		0.2	0.0	5.2

Intersection Summary		
HCM 7th Control Delay, s/veh		8.9
HCM 7th LOS		A

Lane Group	WBL	WBR	NBT	SBL	SBT
Lane Configurations					
Traffic Volume (vph)	98	458	236	882	503
Future Volume (vph)	98	458	236	882	503
Turn Type	Prot	pm+ov	NA	Prot	NA
Protected Phases	8	1	2	1	6
Permitted Phases		8			
Detector Phase	8	1	2	1	6
Switch Phase					
Minimum Initial (s)	10.0	5.0	10.0	5.0	10.0
Minimum Split (s)	23.8	10.8	23.8	10.8	23.8
Total Split (s)	23.8	72.0	24.2	72.0	96.2
Total Split (%)	19.8%	60.0%	20.2%	60.0%	80.2%
Yellow Time (s)	4.8	3.6	4.8	3.6	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.8	4.6	5.8	4.6	5.8
Lead/Lag		Lead	Lag	Lead	
Lead-Lag Optimize?		Yes	Yes	Yes	
Recall Mode	None	None	None	None	None
Act Effct Green (s)	12.7	78.9	15.2	60.2	80.2
Actuated g/C Ratio	0.12	0.75	0.15	0.57	0.77
v/c Ratio	0.49	0.40	0.71	0.93	0.38
Control Delay (s/veh)	54.3	4.4	43.9	37.9	5.0
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	54.3	4.4	43.9	37.9	5.0
LOS	D	A	D	D	A
Approach Delay (s/veh)	13.2		43.9		25.9
Approach LOS	B		D		C

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 104.8	
Natural Cycle: 110	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.93	
Intersection Signal Delay (s/veh): 25.7	Intersection LOS: C
Intersection Capacity Utilization 81.3%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 6: La Sierra Av. & El Sobrante Rd.





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	98	458	236	128	882	503
Future Volume (veh/h)	98	458	236	128	882	503
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	105	293	254	100	948	541
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	212	1063	343	131	983	1390
Arrive On Green	0.12	0.12	0.14	0.14	0.55	0.74
Sat Flow, veh/h	1781	1585	2605	963	1781	1870
Grp Volume(v), veh/h	105	293	178	176	948	541
Grp Sat Flow(s),veh/h/ln	1781	1585	1777	1697	1781	1870
Q Serve(g_s), s	4.6	6.3	8.1	8.4	42.9	8.8
Cycle Q Clear(g_c), s	4.6	6.3	8.1	8.4	42.9	8.8
Prop In Lane	1.00	1.00		0.57	1.00	
Lane Grp Cap(c), veh/h	212	1063	242	232	983	1390
V/C Ratio(X)	0.50	0.28	0.73	0.76	0.96	0.39
Avail Cap(c_a), veh/h	381	1214	389	371	1428	2011
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	34.7	5.6	34.8	35.0	18.0	3.9
Incr Delay (d2), s/veh	1.8	0.1	4.2	5.1	11.1	0.2
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.0	1.5	3.5	3.5	16.1	1.6
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	36.5	5.7	39.1	40.1	29.2	4.1
LnGrp LOS	D	A	D	D	C	A
Approach Vol, veh/h	398		354			1489
Approach Delay, s/veh	13.8		39.6			20.1
Approach LOS	B		D			C

Timer - Assigned Phs	1	2				6	8
Phs Duration (G+Y+Rc), s	51.0	17.3				68.3	15.8
Change Period (Y+Rc), s	4.6	5.8				5.8	5.8
Max Green Setting (Gmax), s	67.4	18.4				90.4	18.0
Max Q Clear Time (g_c+I1), s	44.9	10.4				10.8	8.3
Green Ext Time (p_c), s	1.5	1.1				3.2	0.9

Intersection Summary	
HCM 7th Control Delay, s/veh	22.0
HCM 7th LOS	C

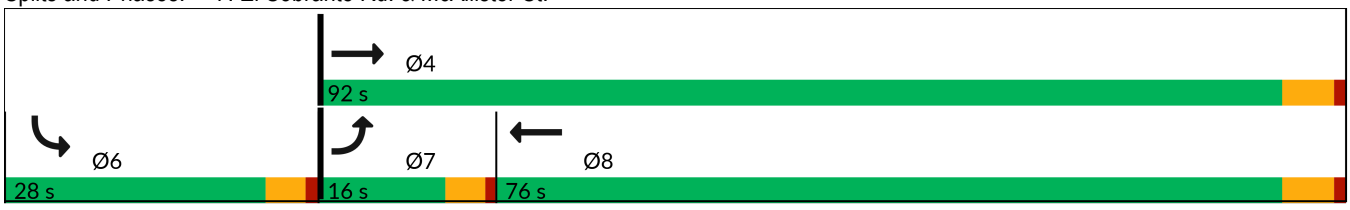
Notes

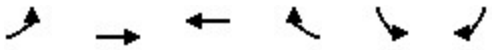
User approved pedestrian interval to be less than phase max green.

Lane Group	EBL	EBT	WBT	SBL
Lane Configurations				
Traffic Volume (vph)	64	886	423	69
Future Volume (vph)	64	886	423	69
Turn Type	Prot	NA	NA	Prot
Protected Phases	7	4	8	6
Permitted Phases				
Detector Phase	7	4	8	6
Switch Phase				
Minimum Initial (s)	5.0	10.0	10.0	5.0
Minimum Split (s)	10.8	23.8	23.8	23.8
Total Split (s)	16.0	92.0	76.0	28.0
Total Split (%)	13.3%	76.7%	63.3%	23.3%
Yellow Time (s)	3.6	4.8	4.8	3.6
All-Red Time (s)	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.8	5.8	4.6
Lead/Lag	Lead		Lag	
Lead-Lag Optimize?	Yes		Yes	
Recall Mode	None	None	None	None
Act Effct Green (s)	7.6	37.1	30.3	10.3
Actuated g/C Ratio	0.13	0.63	0.51	0.17
v/c Ratio	0.30	0.81	0.28	0.56
Control Delay (s/veh)	33.3	14.4	9.8	25.7
Queue Delay	0.0	0.0	0.0	0.0
Total Delay (s/veh)	33.3	14.4	9.8	25.7
LOS	C	B	A	C
Approach Delay (s/veh)		15.7	9.8	25.7
Approach LOS		B	A	C

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 59.1	
Natural Cycle: 70	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.81	
Intersection Signal Delay (s/veh): 15.1	Intersection LOS: B
Intersection Capacity Utilization 65.9%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 7: El Sobrante Rd. & McAllister St.





Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	64	886	423	50	69	111
Future Volume (veh/h)	64	886	423	50	69	111
Initial Q (Qb), veh	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	68	943	450	53	73	118
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	113	1163	1460	171	93	150
Arrive On Green	0.06	0.62	0.46	0.46	0.15	0.15
Sat Flow, veh/h	1781	1870	3298	376	630	1018
Grp Volume(v), veh/h	68	943	249	254	192	0
Grp Sat Flow(s),veh/h/ln	1781	1870	1777	1803	1656	0
Q Serve(g_s), s	1.7	17.3	4.0	4.0	5.0	0.0
Cycle Q Clear(g_c), s	1.7	17.3	4.0	4.0	5.0	0.0
Prop In Lane	1.00			0.21	0.38	0.61
Lane Grp Cap(c), veh/h	113	1163	810	822	243	0
V/C Ratio(X)	0.60	0.81	0.31	0.31	0.79	0.00
Avail Cap(c_a), veh/h	452	3585	2774	2814	862	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	20.5	6.5	7.7	7.8	18.5	0.0
Incr Delay (d2), s/veh	1.9	1.4	0.2	0.2	2.2	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	2.7	1.0	1.0	1.7	0.0
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	22.4	7.9	8.0	8.0	20.7	0.0
LnGrp LOS	C	A	A	A	C	
Approach Vol, veh/h		1011	503		192	
Approach Delay, s/veh		8.9	8.0		20.7	
Approach LOS		A	A		C	

Timer - Assigned Phs	4	6	7	8
Phs Duration (G+Y+Rc), s	33.8	11.2	7.5	26.3
Change Period (Y+Rc), s	5.8	4.6	4.6	5.8
Max Green Setting (Gmax), s	86.2	23.4	11.4	70.2
Max Q Clear Time (g_c+I1), s	19.3	7.0	3.7	6.0
Green Ext Time (p_c), s	8.7	0.2	0.0	2.9

Intersection Summary	
HCM 7th Control Delay, s/veh	9.9
HCM 7th LOS	A