

Appendix M

Residential Site Preliminary Hydrology Study

PRELIMINARY HYDROLOGY STUDY
Azusa Greens Redevelopment Senior Housing
City of Azusa, County of Los Angeles

Project Address:
919 Sierra Madre Avenue
Azusa, California 91702

Prepared For:

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Prepared: June 2023
Revised: March 2025

**Preliminary Hydrology Study
For
Azusa greens Redevelopment Senior Housing**

Acknowledgement and Signature Page

This Preliminary Hydrology Study was prepared by C&V Consulting, Inc. under the supervision of Ryan Bittner, P.E.

Ryan Bittner, P.E. 68167
C&V Consulting, Inc.

Date

TABLE OF CONTENTS

I.	Purpose	1
II.	Site Description.....	1
III.	Existing Conditions.....	1
IV.	Proposed Conditions	2
V.	Methodology	2
VI.	Design Assumption.....	3
VII.	Hydrology Analysis	4
	Hydrology Summary.....	4
	Detention Sizing/ Routing Analysis.....	5
VIII.	Hydraulics Analysis	6
	Pipe Sizing	6
	Catch Basin Inlet Sizing.....	6
	100-Year Water Surface Elevation (WSE)	6
IX.	Conclusion	7
X.	References.....	7

APPENDICES

APPENDIX A Maps and Exhibits

Preliminary Existing Conditions Hydrology Map

Preliminary Post-Development Hydrology Map

Isohyet Map

FEMA Flood Map

APPENDIX B HydroCalc Outputs

Existing Conditions – HydroCalc

Proposed Conditions – HydroCalc

APPENDIX C Detention Routing Analysis

APPENDIX D Hydraulics Analysis

APPENDIX E References

Los Angeles County Public Works – Allowable Q

I. Purpose

The purpose of this preliminary hydrology report is to provide quantitative information to verify the design of the storm drain infrastructure and hydrologic mitigation method of the project site. The values and statements within confirm the subject site is designed and planned in accordance with the Los Angeles County Hydrology Manual and the City of Azusa drainage requirements.

II. Site Description

The proposed project site comprises areas of adjusted lots with a total of 21.72 acres. It is located at 919 Sierra Madre Avenue, City of Azusa, County of Los Angeles. (APN:8617-013-001; 8684-013-014; 8684-043-002; 8617-011-001; 8684-013-030; 8617-001-013; 8617-001-005). The site is bounded by Sierra Madre Avenue to the south, a water treatment facility to the north, residential Tract No. 25309 to the east, and residential areas to the west.

According to the federal Emergency Management Agency (FEMA) FIRM rate map number 06037C1420F, effective date September 26, 2008, the site is located within flood Zone X and D, area of minimal flood hazard.

Refer to Appendix A for additional information.

III. Existing Conditions

The site is currently a golf course and appears to have been closed for a long period of time. The perimeter of the site is bounded by chain link fence and some block walls are present along the boundary adjacent to the residential area. Vegetation within the site consists of large trees dividing the different golf holes. The vast majority of the site is covered by grass with limited types of vegetation. A few structures are present onsite with a club house located adjacent to the entrance parking lot.

The existing drainage condition of the site generally flows south-westerly towards Sierra Madre Avenue. An existing inlet is located in Sierra Madre Avenue adjacent to the parking lot near the entrance to intercept a portion of the flows generated onsite. The northerly portion of the site flows southerly along Ave Conejo and golf range, which ultimately routes to downstream inlets located along Sierra Madre Avenue. These inlets connect to a Los Angeles County Flood Control District's 11'-3" x 12'-0" box which runs along the Sierra Madre Avenue that intercepts all the site generated runoff.

A water basin is located directly north of the property and the slope of the basin descends northerly approximately 40 feet in height based on topographic data. An offsite golf range east of the proposed development is vegetated with localized sumps and slopes southerly to Sierra Madre Avenue, which is not included as part of the proposed development.

Refer to the Preliminary Existing Conditions Hydrology Map within Appendix A.

IV. Proposed Conditions

The project proposes the construction of 38 one-story residential buildings and 2 three-story multi-family residential buildings with private garages, private drive aisles, sidewalks, and common landscaped areas. The project site will be accessible with an entrance/ exit along Sierra Madre Avenue. Drive aisles and parking areas will be composed of asphalt concrete pavement. Proposed development imperviousness is assumed to be 86% per Los Angeles County Hydrology Manual recommended value based on development type.

Onsite residential development and the parking lot areas are analyzed as a separate drainage management area as it drains towards Sierra Madre Avenue. The golf course east of the site, which will not be a part of this hydrological analysis, is to remain in-kind to preserve the perviousness of land usage and drainage pattern per existing conditions. The onsite residential development drainage management area is further delineated into 6 subareas per proposed site grading low points with inlets. The inlets collect and direct runoffs from the drainage management area into the proposed interconnected underground infiltration / detention system. A separate infiltration / detention system is proposed for the clubhouse parking area, the outlet confluences with the overflow outlet of the proposed residential development. The proposed systems will also conform with water quality treatment standards.

During the design storm event when the infiltration/ retention system reaches full capacity, excess runoff is routed to the connection to LACFCD public storm drain via a weir system. In cases of a larger storm event beyond the design storm, runoff will pond at the localized sump inlets and the site is graded to overflow towards Sierra Madre Avenue following historical drainage pattern.

Refer to the Preliminary Post-Development Hydrology Map within Appendix A.

A separate Preliminary LID plan is prepared to address the Los Angeles County storm water quality requirements.

V. Methodology

The project site's drainage was analyzed per the Los Angeles County Hydrology Manual. The existing and proposed conditions of the site were analyzed for peak flow rate and time of concentration for the 25- and 100- storm events based on acreage and land cover per LACDPW HydroCalc program.

VI. Design Assumption

1. The LACDPW HydroCalc Calculator Program was used to determine Tc, Peak run-off flow rate, and run-off volume for subarea based on the longest flow path and elevation difference.
2. The Property is located in the “Azusa” rainfall region per Los Angeles County Hydrology Map. A depth of 7.8 inches of rainfall was determined from the 50-year storm 24-hour isohyet map.
3. The site is in the soil classification of “008” per Los Angeles County Hydrology Map.
4. Proposed conditions are assumed to be approximately 86% impervious base on the recommended values for proposed development type per Hydrology Manual.
5. Existing conditions are analyzed conservatively for reference only as allowable Q is provided by Los Angeles County Public Works.
6. Direct connection to Los Angeles County public storm drain is proposed with given allowable Q of 3.6 cfs per acre. Therefore, proposed peak flow mitigation will be governed by the allowable flow.
7. Proposed storm drains will be sized to convey the 100-year storm per urban flood requirement.

VII. Hydrology Analysis

Hydrology Summary

Proposed storm drainage is designed to maintain the two separate drainage areas to confluence prior to the outlet connection to the public storm drain. A summary of the project site proposed conditions peak runoff values for two DMAs of the project site is provided below.

Drainage summary for DMA-A is further delineated into subareas per inlets as it drains toward the interconnected detention system prior to be routed downstream towards the outlet as shown:

Proposed Conditions Drainage Area	Area (ac)	Q ₁₀₀ (cfs)
DMA-A-1	1.252	5.88
DMA-A-2	2.964	11.89
DMA-A-3	4.635	17.46
DMA-A-4	1.716	8.06
DMA-A-5	5.747	24.79
DMA-A-6	3.502	15.11
Total (Outlet A)	19.815	83.20

Drainage summary at the parking lot adjacent to the entrance as DMA-B:

Proposed Condition Drainage Area	Area (ac)	Q ₁₀₀ (cfs)
DMA-B-7 (Outlet B)	1.908	8.97

Drainage summary of the entire project site:

	Area (ac)	Q ₁₀₀ (cfs)
Allowable Q	21.723	78.20
Proposed Conditions Generated Runoff	21.723	92.16
Proposed Mitigated Drainage	21.723	73.94
Percent Difference	-	-5.5%

Refer to Appendix B & C of this report for additional information and LACDPW HydroCalc output data, as well as the existing and post-development conditions hydrology maps.

Detention Sizing/ Routing Analysis

As the proposed conditions of the site generate a higher runoff peak flowrate compared to the allowable flow, detention is required for hydrologic mitigation. An Infiltration/ Retention system is provided for hydrologic mitigation while satisfying the water quality treatment requirement. The infiltration rate will further benefit in reducing peak runoff as the site generated runoff peak flow is detained and peak outflow is controlled with a weir outlet towards the public storm drain system. Refer to summary of detention sizing below:

Proposed Condition Drainage Area	Area (ac)	Water Quality Treatment Volume (cu-ft)	Proposed Detention Volume (cu-ft)
DMA-A	19.815	45,711.6	70977.1
DMA-B	1.908	6,491.0	7,202.5
Total	21.723	52,202.7	78,179.6

Refer to separate prepared LID for additional information on water quality treatment volume.

Refer to Appendix C for additional Detention Routing Analysis.

VIII. Hydraulics Analysis

Pipe Sizing

Onsite storm drain piping will be sized for the 100-year storm event and will be analyzed based on open channel flow. Therefore, WSPG hydraulic pressure analysis is not warranted. Preliminary outflow pipe capacity sizing is provided within Appendix D.

$$Q = \frac{k'}{n} d^{8/3} S^{1/2} \text{ per King's Handbook}$$

$$k' = 0.463$$

d = pipe diameter

$$n=0.013^*$$

$$S=0.005$$

Pipe Diameter	Max. Q (cfs)
8"	0.854
12"	2.518
15"	4.566
18"	7.425
24"	15.991
36"	47.146

$$n=0.013^*$$

$$S=0.010$$

Pipe Diameter	Max. Q (cfs)
8"	1.208
12"	3.562
15"	6.457
18"	10.501
24"	22.614
36"	66.675

*A Manning's Roughness Coefficient of 0.013 has been utilized to represent the roughness coefficient of PVC and/or HDPE piping.

Catch Basin Inlet Sizing

Catch Basin inlet will be sized with a capacity based on the 100-year storm event to conform with LACDPW Hydrology urban flood street capacity.

Analysis is to be provided during final engineering per final grading plan.

100-Year Water Surface Elevation (WSE)

The elevations of the 100-year storm event water surface are analyzed at local collection points to determine the proper flood protection of the buildings. Building finished floors are to be set above the 100-year storm event water surface elevation or proper overflow mitigation is provided where storm runoff continues downstream contained within the street capacity to ensure buildings are not susceptible to flooding.

Analysis is to be provided during final engineering per final grading plan.

IX. Conclusion

The results from the hydrologic analysis demonstrate that the overall proposed development condition will generate a higher peak runoff flowrate than the allowable outflow for the project site; therefore, a detention system is proposed to detain the generated runoff and a proposed outlet weir is utilized to control the peak runoff outflow towards the connection to the public LACFCD's storm drain system. The detention system also infiltrates to conform with the water quality treatment requirement. The impervious area will be verified with the final site plan during final engineering to validate the water quality treatment detention volume.

During the design storm event when the infiltration/ retention system reaches full capacity, excess runoff is routed to the connection to LACFCD public storm drain via a weir system. In cases of a larger storm event beyond the design storm, runoff will pond at the localized sump inlets and the site is graded to overflow towards Sierra Madre Avenue following historical drainage pattern.

X. References

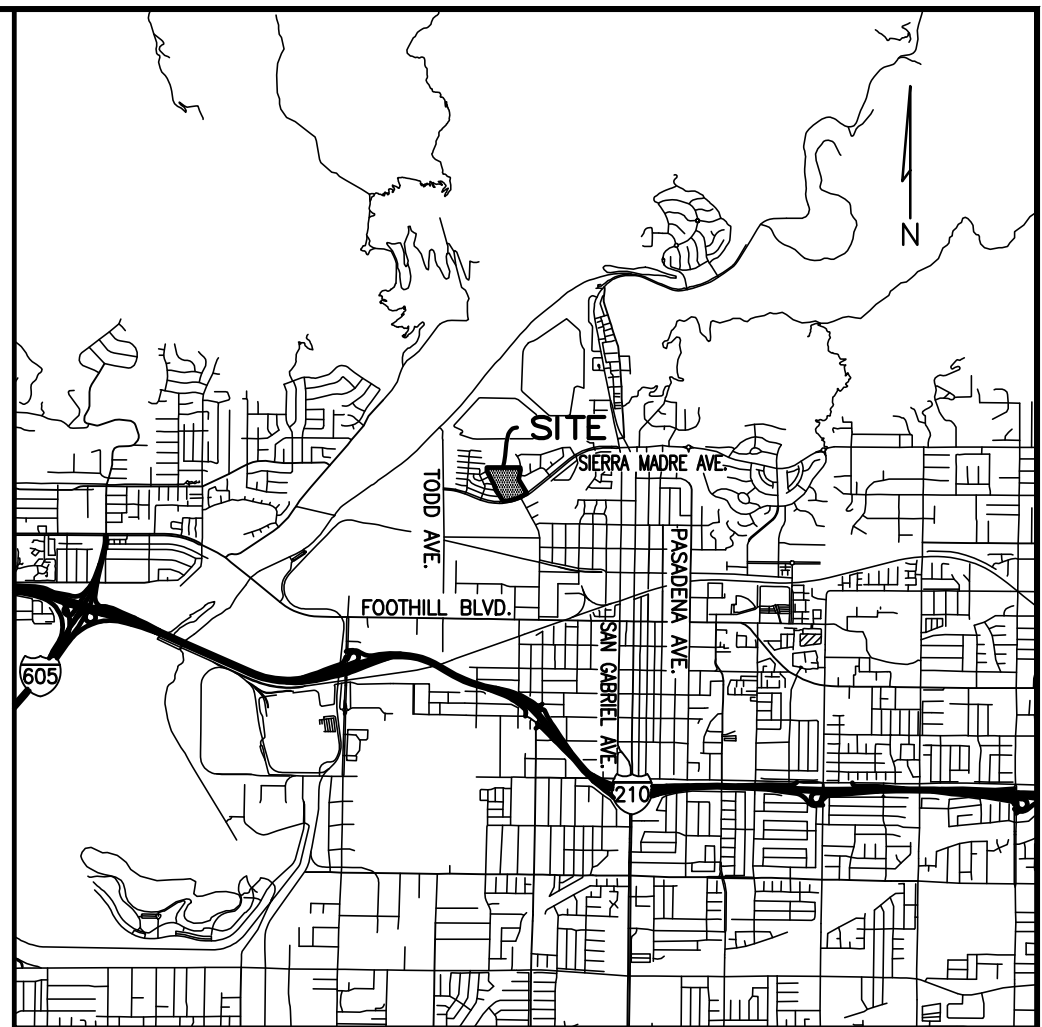
1. Los Angeles County Department of Public Works (LACDPW) Hydrology Manual, January 2006.
2. LACDPW HydroCalc Program was utilized to determine flow rates and time of concentrations.
3. Preliminary Low Impact Development Plan. 919 Sierra Madre Avenue, City of Azusa. Prepared by C&V Consulting, Inc. Dated June 2023.
4. National Flood Hazard Layer FIRMette. Federal Emergency Management Agency. Data refreshed October 2020.
5. KING, HORACE WILLIAMS HANDBOOK OF HYDRAULICS: for the Solution of Hydraulic Problems (Classic Reprint). FORGOTTEN Books, 2015
6. Hydraulic Toolbox 5.0. Federal Highways Administration. Build: 21 Aug 2021

APPENDIX A

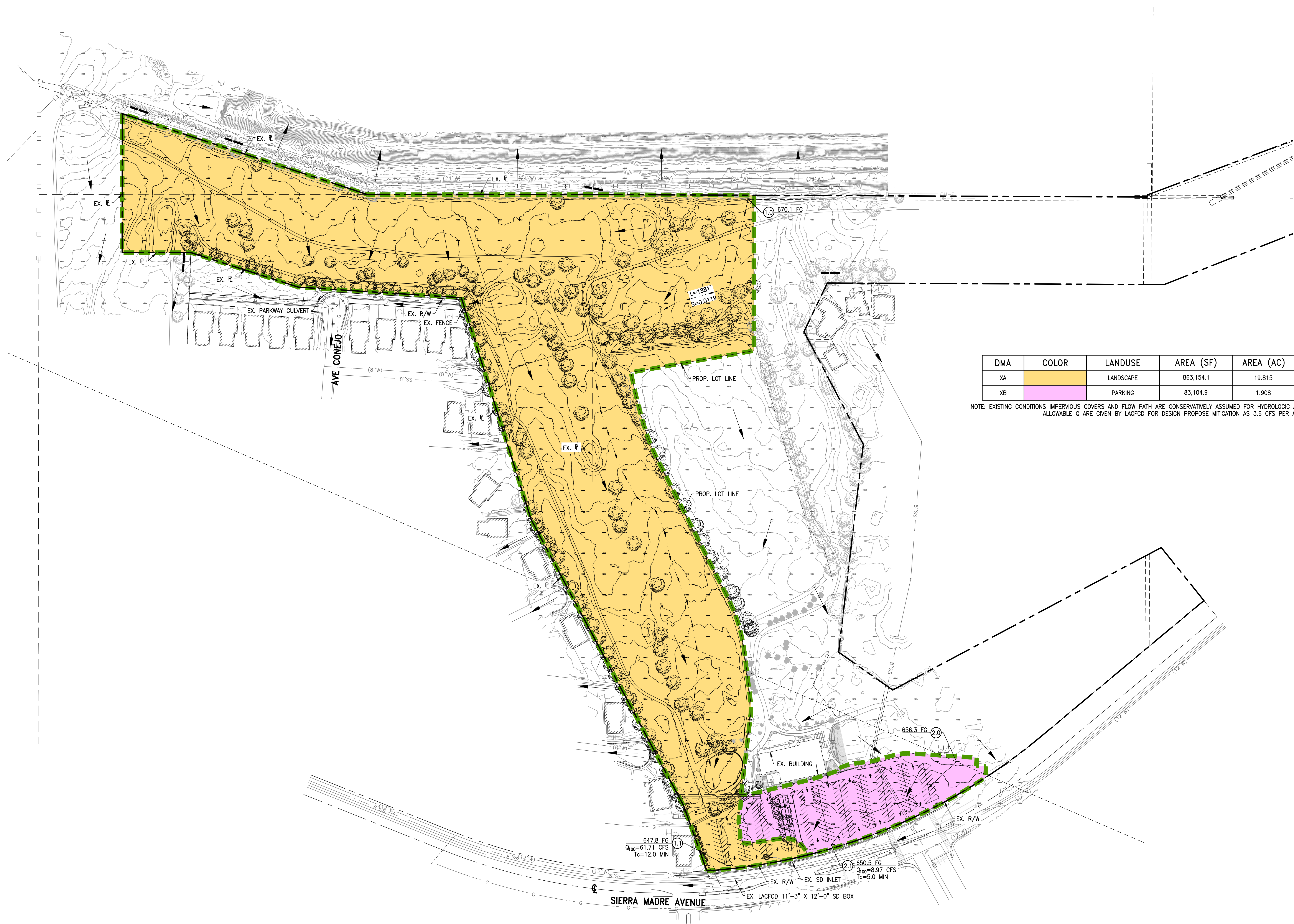
Maps and Exhibits

Preliminary Existing Conditions Hydrology Map

PRELIMINARY EXISTING CONDITIONS HYDROLOGY MAP



VICINITY MAP
N.T.S.

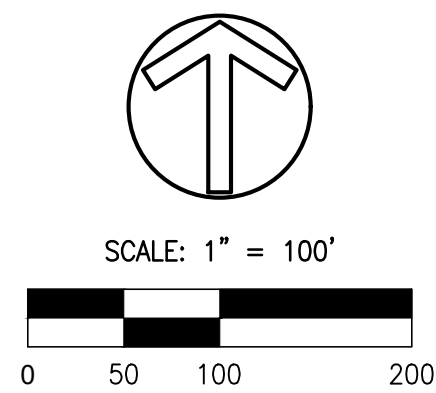


DMA	COLOR	LANDUSE	AREA (SF)	AREA (AC)	IMPERVIOUS %
XA		LANDSCAPE	863,154.1	19.815	1%
XB		PARKING	83,104.9	1.908	100%

NOTE: EXISTING CONDITIONS IMPERVIOUS COVERS AND FLOW PATH ARE CONSERVATIVELY ASSUMED FOR HYDROLOGIC ANALYSIS AS REFERENCE ONLY.
ALLOWABLE Q ARE GIVEN BY LACFCD FOR DESIGN PROPOSED MITIGATION AS 3.6 CFS PER ACRE.

LEGEND:

- DRAINAGE MANAGEMENT AREA (DMA) BOUNDARY
- SITE BOUNDARY
- EXISTING RIGHT OF WAY
- EXISTING LOT LINE
- EXISTING EASEMENT LINE
- FLOW DIRECTION
- FLOW LINE
- PERVIOUS AREA
- INITIAL SUBAREA NODE SPOT ELEVATION
- PEAK RUNOFF IN CUBIC FEET PER SECOND (CFS)
- TIME OF CONCENTRATION IN MINUTES (MIN) PROVIDED FOR 100-YR STORM EVENT



SCALE: 1" = 100'



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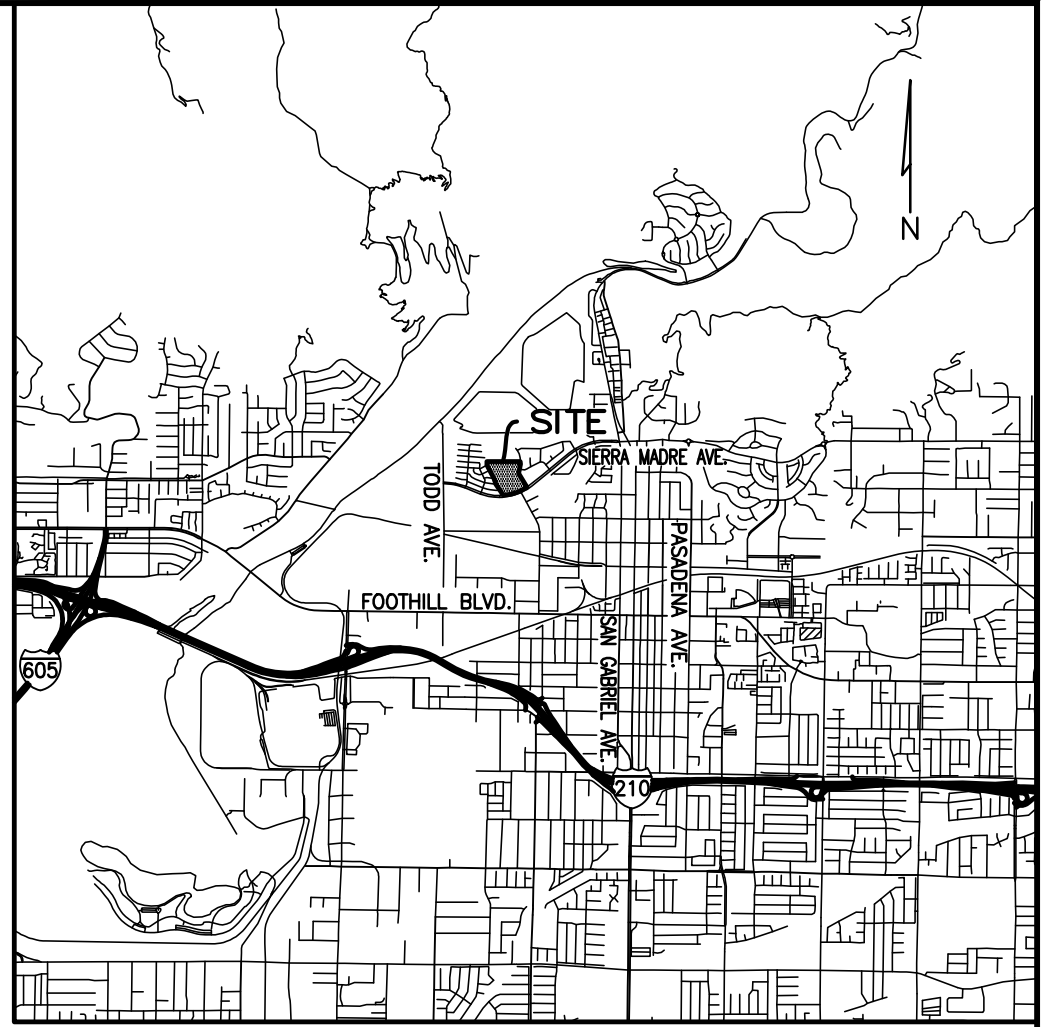
PRELIMINARY EXISTING CONDITIONS HYDROLOGY MAP
AZUZA GREENS REDEVELOPMENT
919 SIERRA MADRE AVENUE
CITY OF AZUSA, CA 91702

PROJECT REF. NO.

SHEET 1 OF 1
CONTRACT
ACCOUNT
DWG. NO.

Preliminary Post-Development Hydrology Map

PRELIMINARY POST-DEVELOPMENT HYDROLOGY MAP



VICINITY MAP
N.T.S.

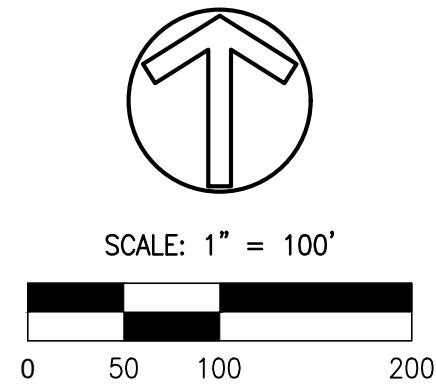


DMA	COLOR	LANDUSE	AREA (SF)	AREA (AC)	IMPERVIOUS %
A-1	Green	RESIDENTIAL	54,536.9	1.252	63%
A-2	Light Green	RESIDENTIAL	129,117.0	2.964	63%
A-3	Orange	RESIDENTIAL	201,889.3	4.635	63%
A-4	Light Orange	RESIDENTIAL	74,733.8	1.716	63%
A-5	Light Blue	RESIDENTIAL	250,335.9	5.747	63%
A-6	Light Cyan	RESIDENTIAL	152,542.9	3.502	63%
B-7	Pink	PARKING	83,104.9	1.908	100%

NOTE: PROPOSED DEVELOPMENT IMPERVIOUS COVERS ARE DELINEATED PER LANDSCAPE PLAN AS SHOWN WITHIN THIS EXHIBIT.
ALLOWABLE Q GIVEN BY LACFCD AS 3.6 CFS PER ACRE UTILIZED FOR DESIGN MITIGATION.

LEGEND:

- DRAINAGE MANAGEMENT AREA (DMA) BOUNDARY
- SITE BOUNDARY
- EXISTING RIGHT OF WAY
- EXISTING LOT LINE
- EXISTING EASEMENT LINE
- SURFACE FLOW DIRECTION
- PIPE FLOW DIRECTION
- FLOW LINE
- PERVIOUS AREA
- INITIAL SUBAREA NODE
SPOT ELEVATION
- PEAK RUNOFF IN CUBIC FEET
PER SECOND (CFS)
 $Q_{100}=X$ CFS
 $T_c=X$ MIN
- TIME OF CONCENTRATION IN MINUTES (MIN)
PROVIDED FOR 100-YR STORM EVENT



OMP
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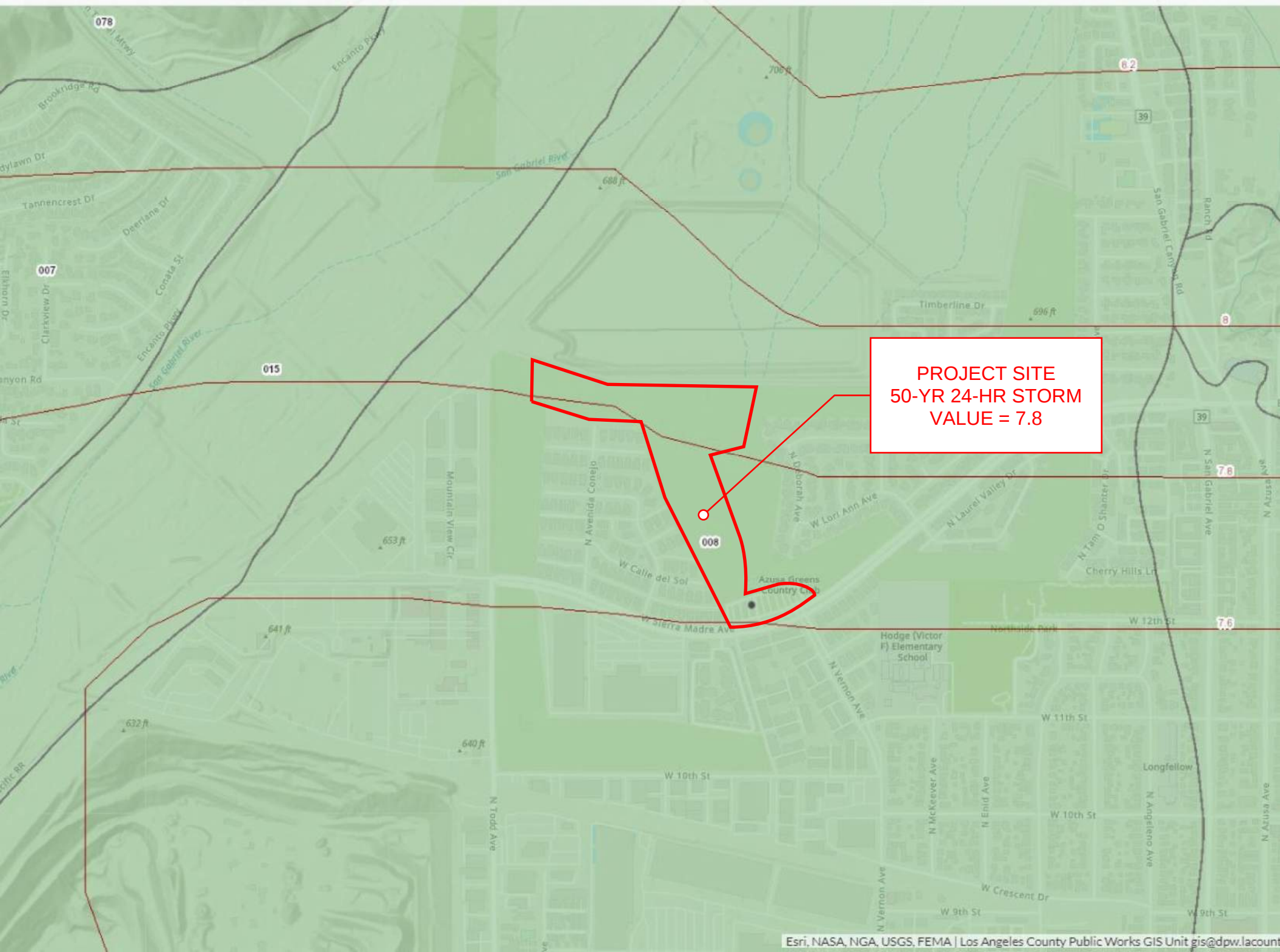


PRELIMINARY POST-DEVELOPMENT HYDROLOGY MAP
AZUSA GREENS REDEVELOPMENT
919 SIERRA MADRE AVENUE
CITY OF AZUSA, CA 91702

PROJECT REF. NO.

SHEET 1 OF 1
CONTRACT
ACCOUNT
DWG. NO.

Isohyet Map



FEMA Flood Map

^v."1



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VHOG

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		\$JHDZWK\$GFGJRG\$VNGHWR HMH GHJWHV =RQ;
		\$JHDZWKPRG\$VNGHWRHMH =RQ*

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 — &URVIFWLRQZWKSDOD&QFH
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|      |  | QPS-G            |

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SRLQV VHDHFWHGEWKHXHU DQGRHVQRW UHSUH  
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7LVSFBSH/ZWK\$VWDDGUG/IRU WKHXHR  
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## **APPENDIX B**

### HydroCalc Outputs

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## Existing Conditions – HydroCalc

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## Peak Flow Hydrologic Analysis

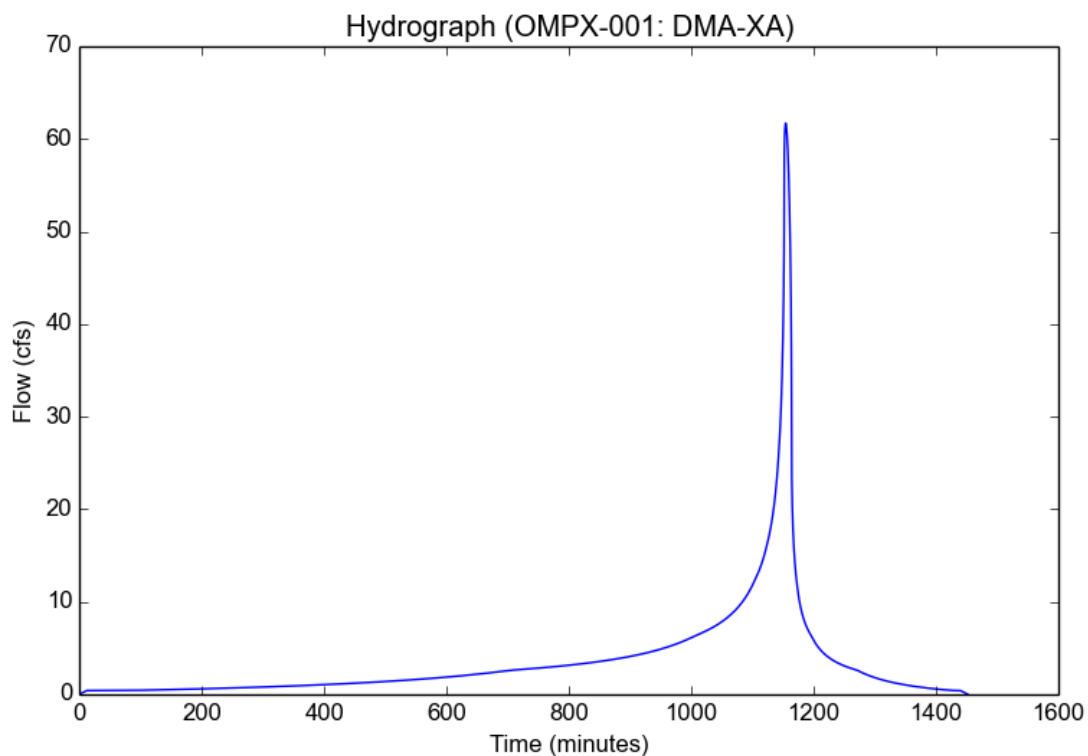
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Version: HydroCalc 1.0.3

### Input Parameters

|                           |          |
|---------------------------|----------|
| Project Name              | OMPX-001 |
| Subarea ID                | DMA-XA   |
| Area (ac)                 | 19.815   |
| Flow Path Length (ft)     | 1881.0   |
| Flow Path Slope (vft/hft) | 0.0119   |
| 50-yr Rainfall Depth (in) | 7.8      |
| Percent Impervious        | 0.01     |
| Soil Type                 | 8        |
| Design Storm Frequency    | 100-yr   |
| Fire Factor               | 0        |
| LID                       | False    |

### Output Results

|                                      |             |
|--------------------------------------|-------------|
| Modeled (100-yr) Rainfall Depth (in) | 8.7516      |
| Peak Intensity (in/hr)               | 3.4601      |
| Undeveloped Runoff Coefficient (Cu)  | 0.9         |
| Developed Runoff Coefficient (Cd)    | 0.9         |
| Time of Concentration (min)          | 12.0        |
| Clear Peak Flow Rate (cfs)           | 61.7061     |
| Burned Peak Flow Rate (cfs)          | 61.7061     |
| 24-Hr Clear Runoff Volume (ac-ft)    | 7.1736      |
| 24-Hr Clear Runoff Volume (cu-ft)    | 312479.9972 |



## Peak Flow Hydrologic Analysis

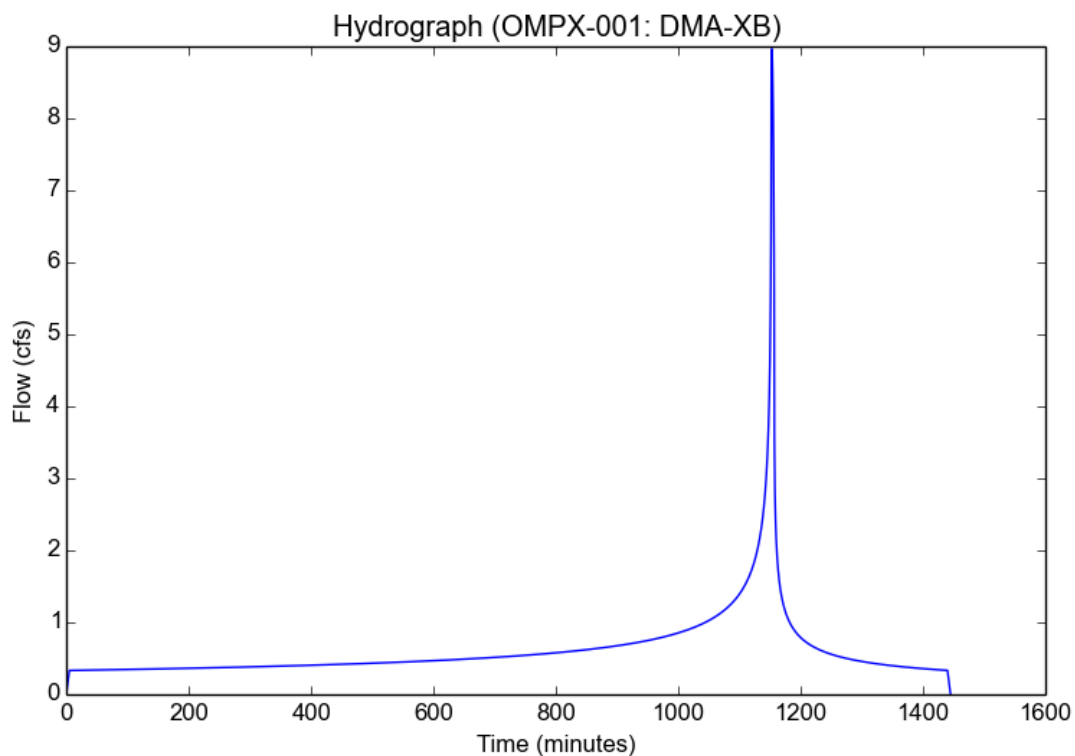
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Version: HydroCalc 1.0.3

### Input Parameters

|                           |          |
|---------------------------|----------|
| Project Name              | OMPX-001 |
| Subarea ID                | DMA-XB   |
| Area (ac)                 | 1.908    |
| Flow Path Length (ft)     | 368.0    |
| Flow Path Slope (vft/hft) | 0.0158   |
| 50-yr Rainfall Depth (in) | 7.8      |
| Percent Impervious        | 1.0      |
| Soil Type                 | 8        |
| Design Storm Frequency    | 100-yr   |
| Fire Factor               | 0        |
| LID                       | False    |

### Output Results

|                                      |           |
|--------------------------------------|-----------|
| Modeled (100-yr) Rainfall Depth (in) | 8.7516    |
| Peak Intensity (in/hr)               | 5.2214    |
| Undeveloped Runoff Coefficient (Cu)  | 0.9       |
| Developed Runoff Coefficient (Cd)    | 0.9       |
| Time of Concentration (min)          | 5.0       |
| Clear Peak Flow Rate (cfs)           | 8.9663    |
| Burned Peak Flow Rate (cfs)          | 8.9663    |
| 24-Hr Clear Runoff Volume (ac-ft)    | 1.242     |
| 24-Hr Clear Runoff Volume (cu-ft)    | 54101.708 |



## Proposed Conditions – HydroCalc

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## Peak Flow Hydrologic Analysis

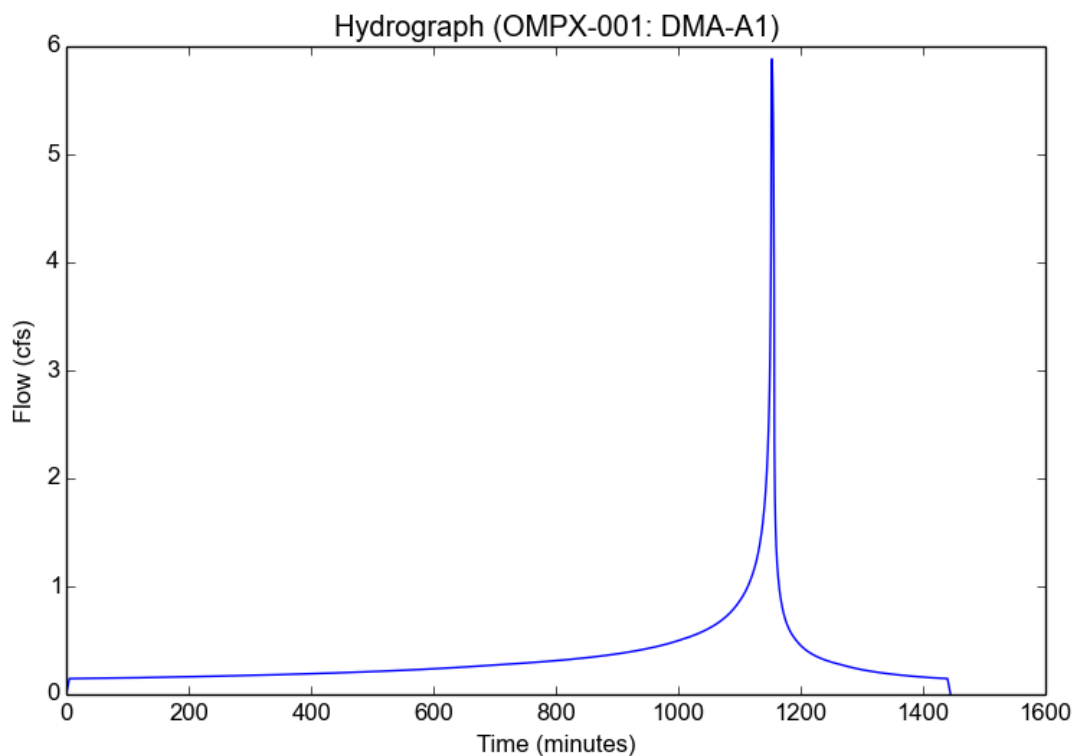
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### Input Parameters

|                           |          |
|---------------------------|----------|
| Project Name              | OMPX-001 |
| Subarea ID                | DMA-A1   |
| Area (ac)                 | 1.252    |
| Flow Path Length (ft)     | 274.0    |
| Flow Path Slope (vft/hft) | 0.0102   |
| 50-yr Rainfall Depth (in) | 7.8      |
| Percent Impervious        | 0.63     |
| Soil Type                 | 8        |
| Design Storm Frequency    | 100-yr   |
| Fire Factor               | 0        |
| LID                       | False    |

### Output Results

|                                      |            |
|--------------------------------------|------------|
| Modeled (100-yr) Rainfall Depth (in) | 8.7516     |
| Peak Intensity (in/hr)               | 5.2214     |
| Undeveloped Runoff Coefficient (Cu)  | 0.9        |
| Developed Runoff Coefficient (Cd)    | 0.9        |
| Time of Concentration (min)          | 5.0        |
| Clear Peak Flow Rate (cfs)           | 5.8835     |
| Burned Peak Flow Rate (cfs)          | 5.8835     |
| 24-Hr Clear Runoff Volume (ac-ft)    | 0.6796     |
| 24-Hr Clear Runoff Volume (cu-ft)    | 29603.7079 |



## Peak Flow Hydrologic Analysis

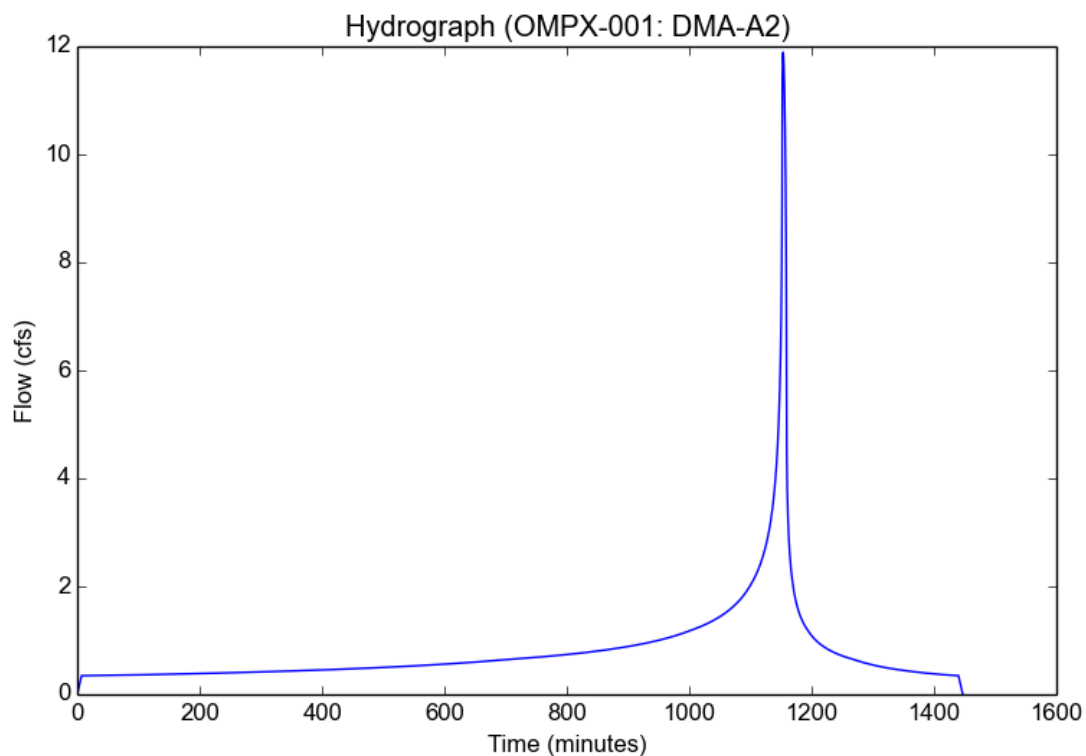
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Version: HydroCalc 1.0.3

### Input Parameters

|                           |          |
|---------------------------|----------|
| Project Name              | OMPX-001 |
| Subarea ID                | DMA-A2   |
| Area (ac)                 | 2.964    |
| Flow Path Length (ft)     | 650.0    |
| Flow Path Slope (vft/hft) | 0.006    |
| 50-yr Rainfall Depth (in) | 7.8      |
| Percent Impervious        | 0.63     |
| Soil Type                 | 8        |
| Design Storm Frequency    | 100-yr   |
| Fire Factor               | 0        |
| LID                       | False    |

### Output Results

|                                      |            |
|--------------------------------------|------------|
| Modeled (100-yr) Rainfall Depth (in) | 8.7516     |
| Peak Intensity (in/hr)               | 4.4577     |
| Undeveloped Runoff Coefficient (Cu)  | 0.9        |
| Developed Runoff Coefficient (Cd)    | 0.9        |
| Time of Concentration (min)          | 7.0        |
| Clear Peak Flow Rate (cfs)           | 11.8914    |
| Burned Peak Flow Rate (cfs)          | 11.8914    |
| 24-Hr Clear Runoff Volume (ac-ft)    | 1.609      |
| 24-Hr Clear Runoff Volume (cu-ft)    | 70088.2547 |



## Peak Flow Hydrologic Analysis

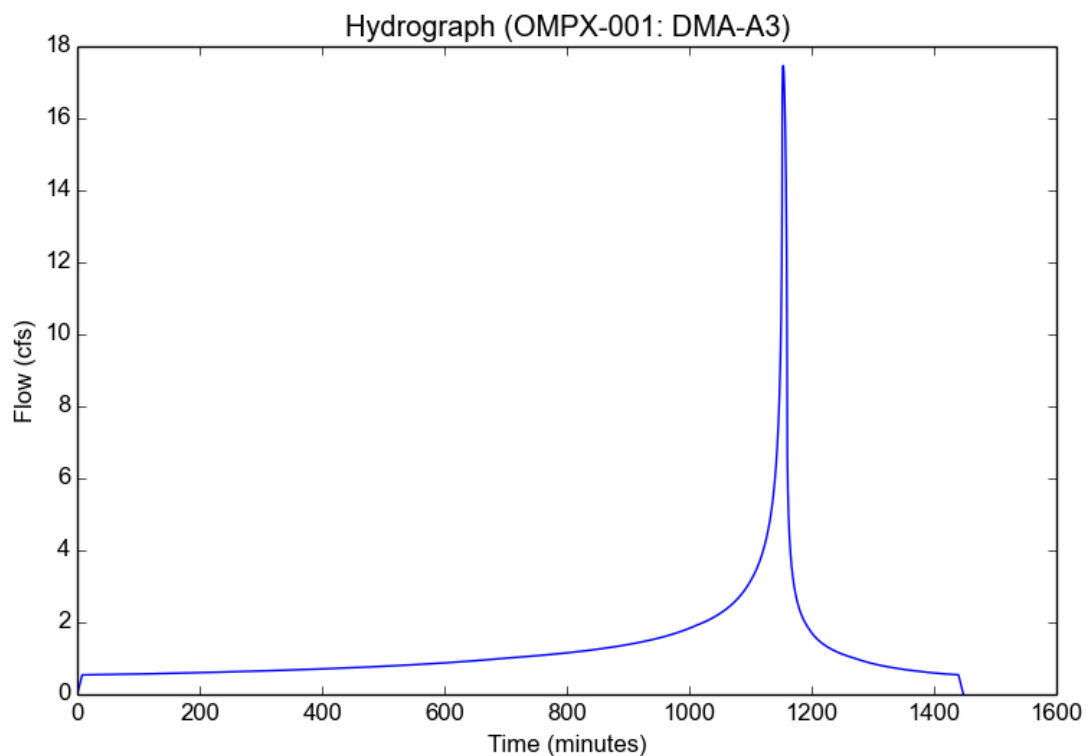
File location: P:/O/OMPX-001/Admin/Reports/Hydrology/Appendix B - Hydrology/HydroCalc/OMPX-001 HydroCalc Report.pdf  
Version: HydroCalc 1.0.3

### Input Parameters

|                           |          |
|---------------------------|----------|
| Project Name              | OMPX-001 |
| Subarea ID                | DMA-A3   |
| Area (ac)                 | 4.635    |
| Flow Path Length (ft)     | 907.0    |
| Flow Path Slope (vft/hft) | 0.0073   |
| 50-yr Rainfall Depth (in) | 7.8      |
| Percent Impervious        | 0.63     |
| Soil Type                 | 8        |
| Design Storm Frequency    | 100-yr   |
| Fire Factor               | 0        |
| LID                       | False    |

### Output Results

|                                      |             |
|--------------------------------------|-------------|
| Modeled (100-yr) Rainfall Depth (in) | 8.7516      |
| Peak Intensity (in/hr)               | 4.1865      |
| Undeveloped Runoff Coefficient (Cu)  | 0.9         |
| Developed Runoff Coefficient (Cd)    | 0.9         |
| Time of Concentration (min)          | 8.0         |
| Clear Peak Flow Rate (cfs)           | 17.4641     |
| Burned Peak Flow Rate (cfs)          | 17.4641     |
| 24-Hr Clear Runoff Volume (ac-ft)    | 2.5162      |
| 24-Hr Clear Runoff Volume (cu-ft)    | 109605.5258 |



## Peak Flow Hydrologic Analysis

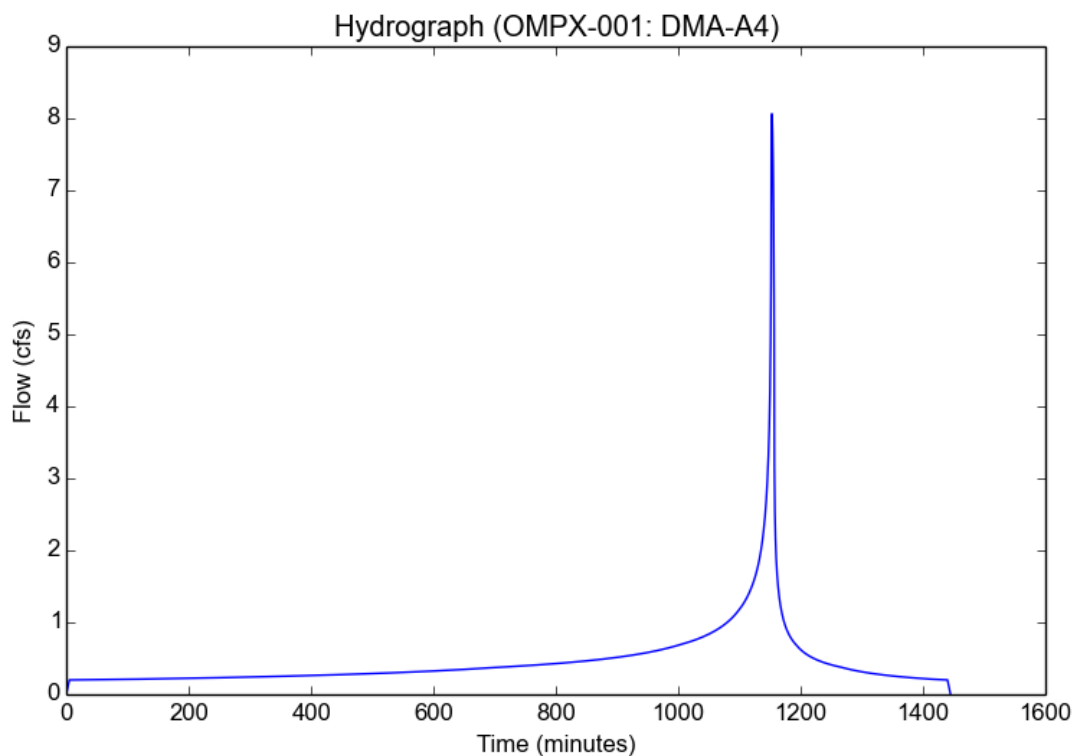
File location: P:/O/OMPX-001/Admin/Reports/Hydrology/Appendix B - Hydrology/HydroCalc/OMPX-001 HydroCalc Report.pdf  
Version: HydroCalc 1.0.3

### Input Parameters

|                           |          |
|---------------------------|----------|
| Project Name              | OMPX-001 |
| Subarea ID                | DMA-A4   |
| Area (ac)                 | 1.716    |
| Flow Path Length (ft)     | 259.0    |
| Flow Path Slope (vft/hft) | 0.0181   |
| 50-yr Rainfall Depth (in) | 7.8      |
| Percent Impervious        | 0.63     |
| Soil Type                 | 8        |
| Design Storm Frequency    | 100-yr   |
| Fire Factor               | 0        |
| LID                       | False    |

### Output Results

|                                      |            |
|--------------------------------------|------------|
| Modeled (100-yr) Rainfall Depth (in) | 8.7516     |
| Peak Intensity (in/hr)               | 5.2214     |
| Undeveloped Runoff Coefficient (Cu)  | 0.9        |
| Developed Runoff Coefficient (Cd)    | 0.9        |
| Time of Concentration (min)          | 5.0        |
| Clear Peak Flow Rate (cfs)           | 8.064      |
| Burned Peak Flow Rate (cfs)          | 8.064      |
| 24-Hr Clear Runoff Volume (ac-ft)    | 0.9315     |
| 24-Hr Clear Runoff Volume (cu-ft)    | 40575.0501 |



## Peak Flow Hydrologic Analysis

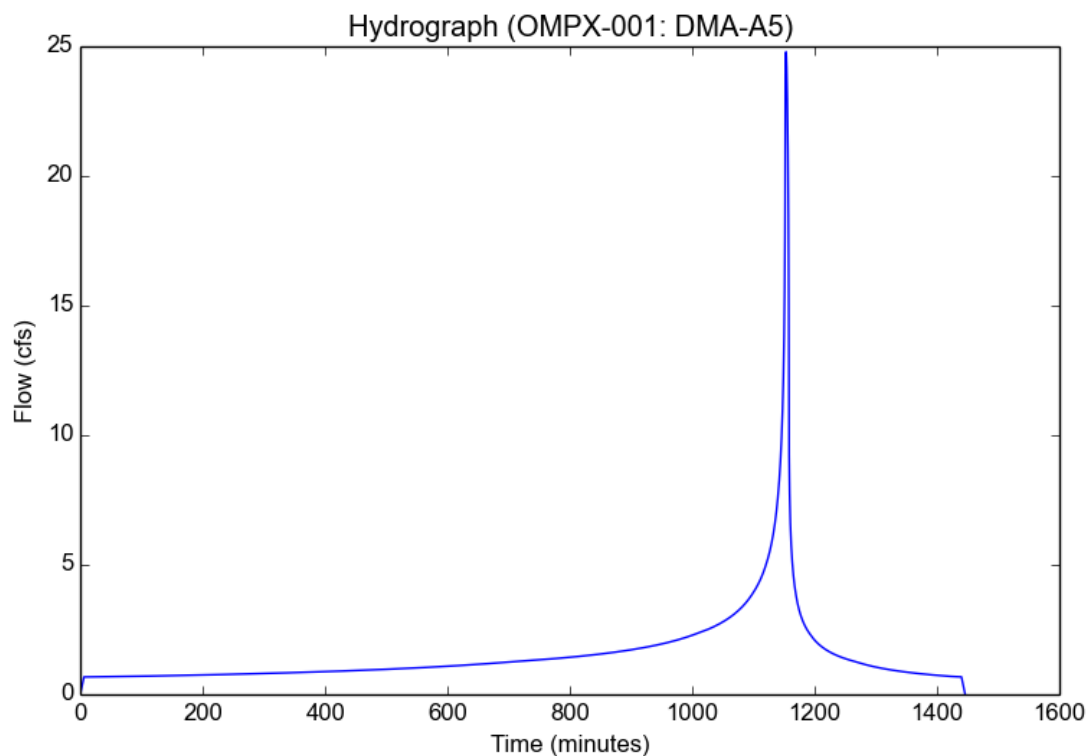
File location: P:/O/OMPX-001/Admin/Reports/Hydrology/Appendix B - Hydrology/HydroCalc/OMPX-001 HydroCalc Report.pdf  
Version: HydroCalc 1.0.3

### Input Parameters

|                           |          |
|---------------------------|----------|
| Project Name              | OMPX-001 |
| Subarea ID                | DMA-A5   |
| Area (ac)                 | 5.747    |
| Flow Path Length (ft)     | 638.0    |
| Flow Path Slope (vft/hft) | 0.015    |
| 50-yr Rainfall Depth (in) | 7.8      |
| Percent Impervious        | 0.63     |
| Soil Type                 | 8        |
| Design Storm Frequency    | 100-yr   |
| Fire Factor               | 0        |
| LID                       | False    |

### Output Results

|                                      |             |
|--------------------------------------|-------------|
| Modeled (100-yr) Rainfall Depth (in) | 8.7516      |
| Peak Intensity (in/hr)               | 4.7926      |
| Undeveloped Runoff Coefficient (Cu)  | 0.9         |
| Developed Runoff Coefficient (Cd)    | 0.9         |
| Time of Concentration (min)          | 6.0         |
| Clear Peak Flow Rate (cfs)           | 24.789      |
| Burned Peak Flow Rate (cfs)          | 24.789      |
| 24-Hr Clear Runoff Volume (ac-ft)    | 3.1197      |
| 24-Hr Clear Runoff Volume (cu-ft)    | 135892.1242 |



## Peak Flow Hydrologic Analysis

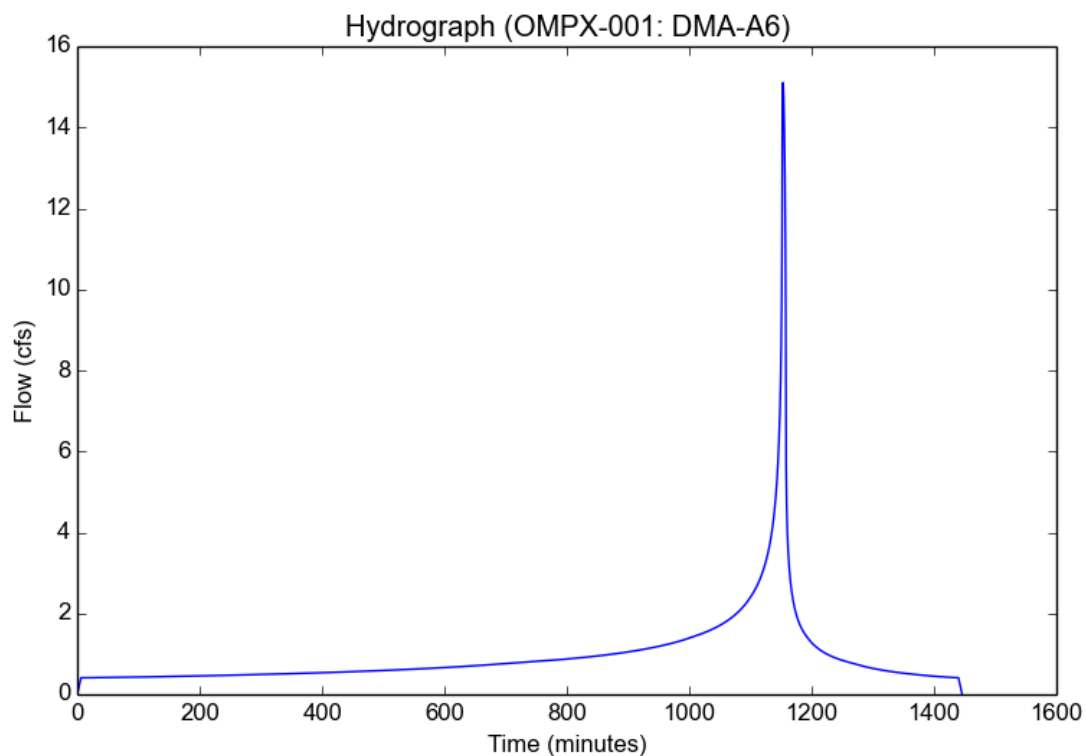
File location: P:/O/OMPX-001/Admin/Reports/Hydrology/Appendix B - Hydrology/HydroCalc/OMPX-001 HydroCalc Report.pdf  
Version: HydroCalc 1.0.3

### Input Parameters

|                           |          |
|---------------------------|----------|
| Project Name              | OMPX-001 |
| Subarea ID                | DMA-A6   |
| Area (ac)                 | 3.502    |
| Flow Path Length (ft)     | 621.0    |
| Flow Path Slope (vft/hft) | 0.01     |
| 50-yr Rainfall Depth (in) | 7.8      |
| Percent Impervious        | 0.63     |
| Soil Type                 | 8        |
| Design Storm Frequency    | 100-yr   |
| Fire Factor               | 0        |
| LID                       | False    |

### Output Results

|                                      |            |
|--------------------------------------|------------|
| Modeled (100-yr) Rainfall Depth (in) | 8.7516     |
| Peak Intensity (in/hr)               | 4.7926     |
| Undeveloped Runoff Coefficient (Cu)  | 0.9        |
| Developed Runoff Coefficient (Cd)    | 0.9        |
| Time of Concentration (min)          | 6.0        |
| Clear Peak Flow Rate (cfs)           | 15.1055    |
| Burned Peak Flow Rate (cfs)          | 15.1055    |
| 24-Hr Clear Runoff Volume (ac-ft)    | 1.901      |
| 24-Hr Clear Runoff Volume (cu-ft)    | 82807.4158 |



## Peak Flow Hydrologic Analysis

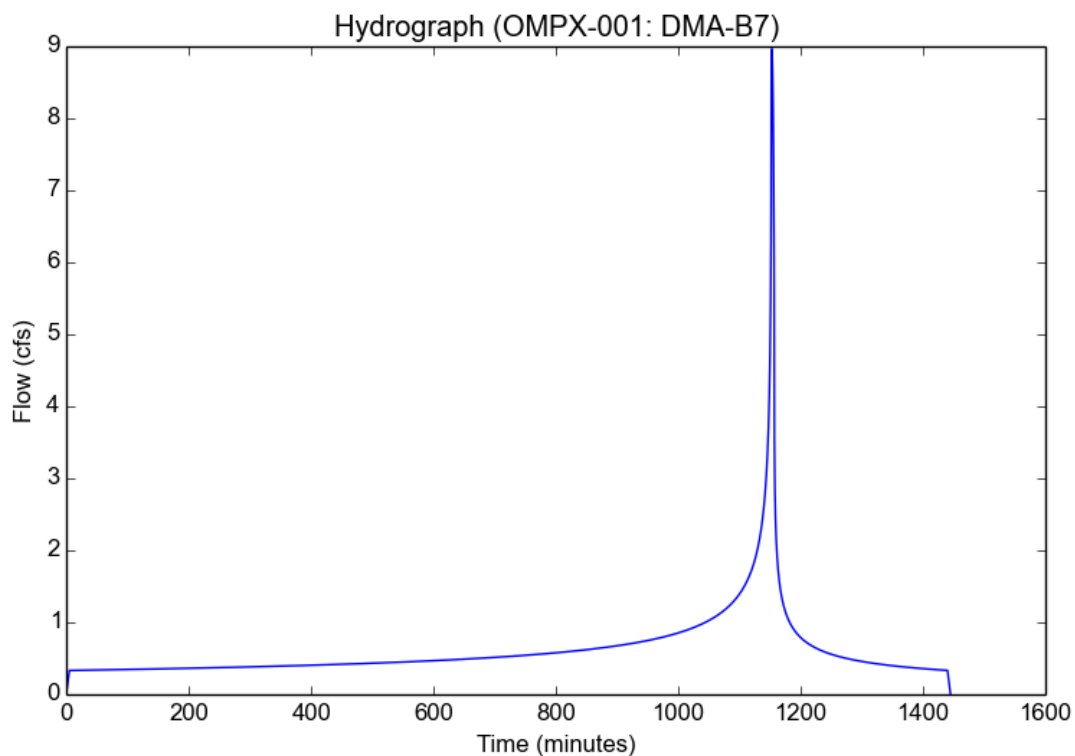
File location: P:/O/OMPX-001/Admin/Reports/Hydrology/Appendix B - Hydrology/HydroCalc/OMPX-001 HydroCalc Report.pdf  
Version: HydroCalc 1.0.3

### Input Parameters

|                           |          |
|---------------------------|----------|
| Project Name              | OMPX-001 |
| Subarea ID                | DMA-B7   |
| Area (ac)                 | 1.908    |
| Flow Path Length (ft)     | 460.0    |
| Flow Path Slope (vft/hft) | 0.0137   |
| 50-yr Rainfall Depth (in) | 7.8      |
| Percent Impervious        | 1.0      |
| Soil Type                 | 8        |
| Design Storm Frequency    | 100-yr   |
| Fire Factor               | 0        |
| LID                       | False    |

### Output Results

|                                      |           |
|--------------------------------------|-----------|
| Modeled (100-yr) Rainfall Depth (in) | 8.7516    |
| Peak Intensity (in/hr)               | 5.2214    |
| Undeveloped Runoff Coefficient (Cu)  | 0.9       |
| Developed Runoff Coefficient (Cd)    | 0.9       |
| Time of Concentration (min)          | 5.0       |
| Clear Peak Flow Rate (cfs)           | 8.9663    |
| Burned Peak Flow Rate (cfs)          | 8.9663    |
| 24-Hr Clear Runoff Volume (ac-ft)    | 1.242     |
| 24-Hr Clear Runoff Volume (cu-ft)    | 54101.708 |



## **APPENDIX C**

### Detention Routing Analysis

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# Stage-Storage Table

Project: OMPX-001  
Address: 919 Sierra Madre Avenue, City of Azusa

## ADS StormTech MC-7200 Detention

ADS chamber/ end cap      # of chamber      # of end cap      34  
260

| Depth (ft) | Cumulative Volume (cf) | Cumulative Volume (cf) |
|------------|------------------------|------------------------|
| 0          | 0                      | 0                      |
| 1          | 8174.40                | 528.02                 |
| 2          | 21928.40               | 1241.00                |
| 3          | 35066.20               | 1919.64                |
| 4          | 47195.20               | 2543.88                |
| 5          | 57657.60               | 3095.36                |
| 6          | 64825.80               | 3573.74                |
| 6.67       | 69498.00               | 3919.52                |

## Syste Storage Table

Conversion      43560

| Depth (ft) | Volume (cf) | Volume (af) |
|------------|-------------|-------------|
| 0          | 0           | 0           |
| 1          | 8702.42     | 0.1998      |
| 2          | 23169.40    | 0.5319      |
| 3          | 36985.84    | 0.8491      |
| 4          | 49739.08    | 1.1419      |
| 5          | 60752.96    | 1.3947      |
| 6          | 68399.54    | 1.5702      |
| 6.67       | 73417.52    | 1.6854      |

## Infiltrated Outflow

| Design Infiltration rate | Infiltrated Area | Infiltrated Q |
|--------------------------|------------------|---------------|
| 2.33 in/hr               | 18493.4          | 0.9974        |

Design Infiltration rate per Geotechnical Report Dated April 26, 2023.

Infiltrated Surface Area per Manufacturer's Specification.

## Site Mitigated Outflow

| Allowable Q | Mitigated Peak Outflow |
|-------------|------------------------|
| 78.2        | 73.94                  |

## User Inputs

|                               |                     |
|-------------------------------|---------------------|
| Chamber Model:                | MC-7200             |
| Outlet Control Structure:     | No                  |
| Project Name:                 | OMPX-001 DMA-A      |
| Bed Name                      | Bed 1               |
| Engineer:                     | undefined undefined |
| Project Location:             | California          |
| Measurement Type:             | Imperial            |
| Required Storage Volume:      | 500 cubic ft.       |
| Stone Porosity:               | 40%                 |
| Stone Foundation Depth:       | 9 in.               |
| Stone Above Chambers:         | 12 in.              |
| Average Cover Over Chambers:  | 24 in.              |
| Design Constraint Dimensions: | (20 ft. x 20 ft.)   |

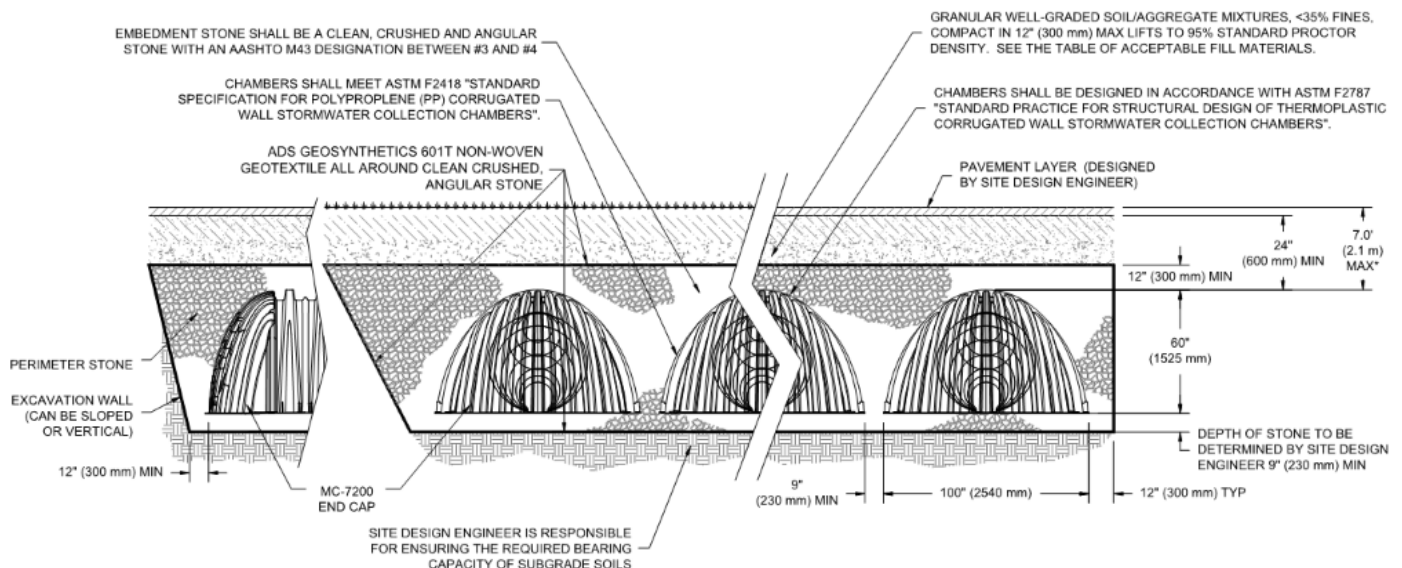
## Results

### System Volume and Bed Size

|                              |                    |
|------------------------------|--------------------|
| Installed Storage Volume:    | 32617.90 cubic ft. |
| Storage Volume Per Chamber:  | 175.90 cubic ft.   |
| Number Of Chambers Required: | 108                |
| Number Of End Caps Required: | 18                 |
| Chamber Rows:                | 9                  |
| Maximum Length:              | 92.78 ft.          |
| Maximum Width:               | 83.00 ft.          |
| Approx. Bed Size Required:   | 7700.88 square ft. |

### System Components

|                                                     |                   |
|-----------------------------------------------------|-------------------|
| Amount Of Stone Required:                           | 1196 cubic yards  |
| Volume Of Excavation (Not Including Fill):          | 1926 cubic yards  |
| Total Non-woven Geotextile Required:                | 2370 square yards |
| Woven Geotextile Required (excluding Isolator Row): | 234 square yards  |
| Woven Geotextile Required (Isolator Row):           | 198 square yards  |
| Total Woven Geotextile Required:                    | 431 square yards  |
| Impervious Liner Required:                          | 0 square yards    |



\*MINIMUM COVER TO BOTTOM OF FLEXIBLE PAVEMENT. FOR UNPAVED INSTALLATIONS WHERE RUTTING FROM VEHICLES MAY OCCUR, INCREASE COVER TO 30" (750 mm).



## User Inputs

|                               |                     |
|-------------------------------|---------------------|
| Chamber Model:                | MC-7200             |
| Outlet Control Structure:     | No                  |
| Project Name:                 | OMPX-001 DMA-A      |
| Bed Name                      | Bed 2               |
| Engineer:                     | undefined undefined |
| Project Location:             | California          |
| Measurement Type:             | Imperial            |
| Required Storage Volume:      | 500 cubic ft.       |
| Stone Porosity:               | 40%                 |
| Stone Foundation Depth:       | 9 in.               |
| Stone Above Chambers:         | 12 in.              |
| Average Cover Over Chambers:  | 24 in.              |
| Design Constraint Dimensions: | (20 ft. x 20 ft.)   |

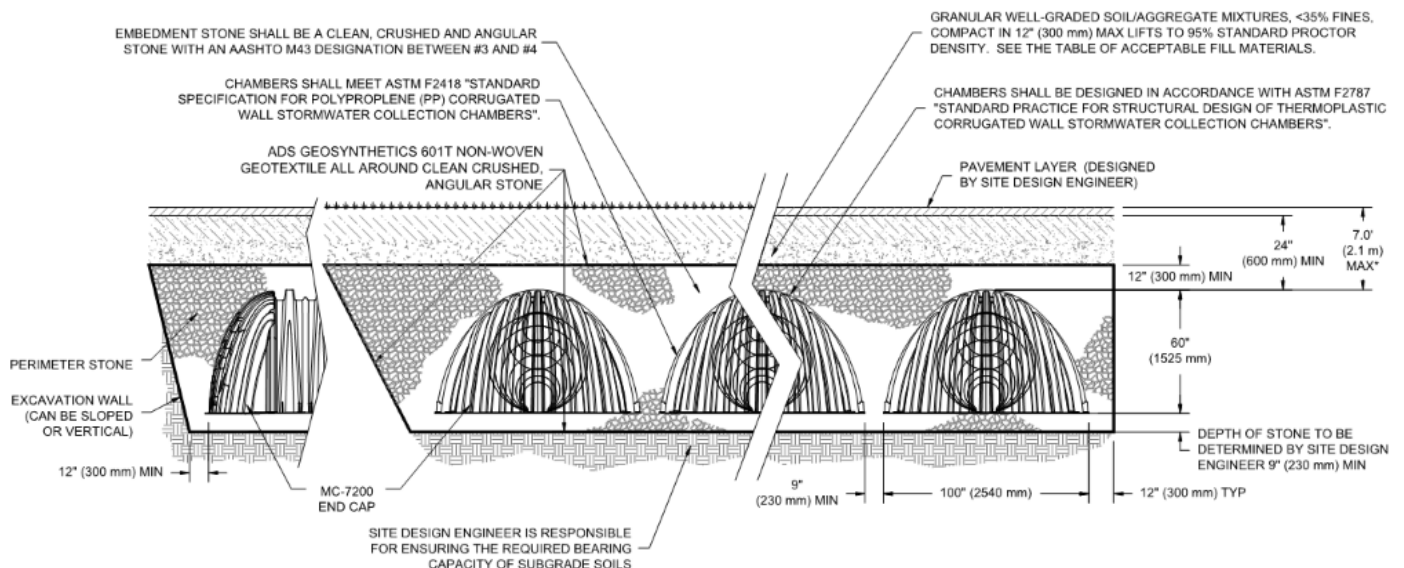
## Results

### System Volume and Bed Size

|                              |                    |
|------------------------------|--------------------|
| Installed Storage Volume:    | 12674.51 cubic ft. |
| Storage Volume Per Chamber:  | 175.90 cubic ft.   |
| Number Of Chambers Required: | 40                 |
| Number Of End Caps Required: | 6                  |
| Chamber Rows:                | 3                  |
| Maximum Length:              | 120.81 ft.         |
| Maximum Width:               | 28.50 ft.          |
| Approx. Bed Size Required:   | 3077.96 square ft. |

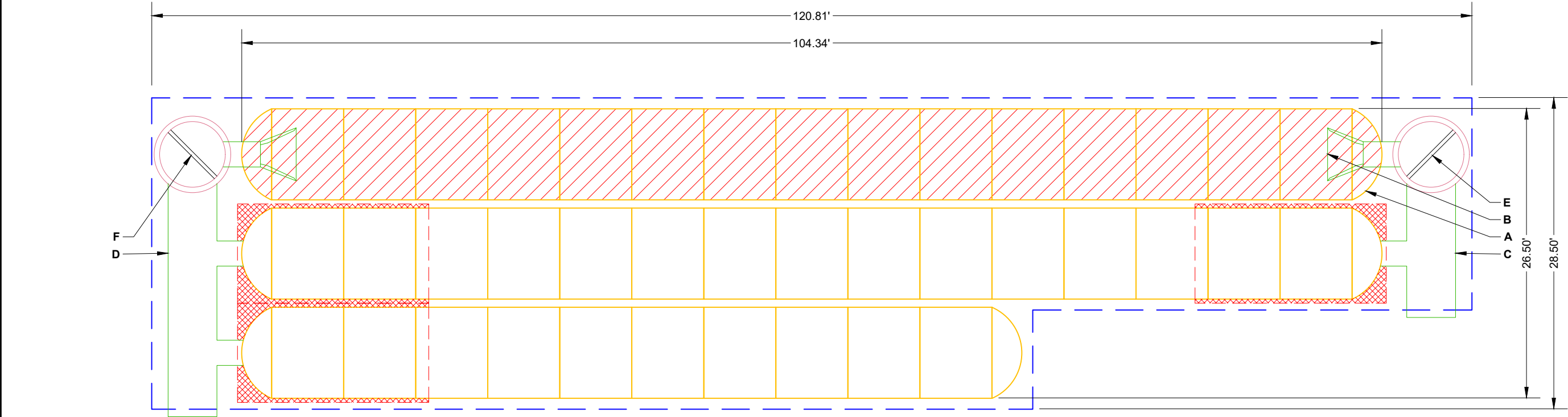
### System Components

|                                                     |                   |
|-----------------------------------------------------|-------------------|
| Amount Of Stone Required:                           | 501 cubic yards   |
| Volume Of Excavation (Not Including Fill):          | 770 cubic yards   |
| Total Non-woven Geotextile Required:                | 1090 square yards |
| Woven Geotextile Required (excluding Isolator Row): | 88 square yards   |
| Woven Geotextile Required (Isolator Row):           | 244 square yards  |
| Total Woven Geotextile Required:                    | 331 square yards  |
| Impervious Liner Required:                          | 0 square yards    |

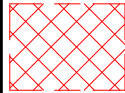


\*MINIMUM COVER TO BOTTOM OF FLEXIBLE PAVEMENT. FOR UNPAVED INSTALLATIONS WHERE RUTTING FROM VEHICLES MAY OCCUR, INCREASE COVER TO 30" (750 mm).

| PROPOSED LAYOUT: BED 2 |                                                                                     |                                                           |       | CONCEPTUAL ELEVATIONS |                |                                                                                                                 |  | *INVERT ABOVE BASE OF CHAMBER |             |  |  |
|------------------------|-------------------------------------------------------------------------------------|-----------------------------------------------------------|-------|-----------------------|----------------|-----------------------------------------------------------------------------------------------------------------|--|-------------------------------|-------------|--|--|
|                        |                                                                                     |                                                           |       |                       |                |                                                                                                                 |  |                               |             |  |  |
| 40                     | STORMTECH MC-7200 CHAMBERS                                                          | MAXIMUM ALLOWABLE GRADE (TOP OF PAVEMENT/UNPAVED):        | 12.75 | PART TYPE             | ITEM ON LAYOUT | DESCRIPTION                                                                                                     |  | INVERT*                       | MAX FLOW    |  |  |
| 6                      | STORMTECH MC-7200 END CAPS                                                          | MINIMUM ALLOWABLE GRADE (UNPAVED WITH TRAFFIC):           | 8.25  | PREFABRICATED END CAP | A              | 24" BOTTOM PARTIAL CUT END CAP, PART#: MC7200IEPP24B / TYP OF ALL 24" BOTTOM CONNECTIONS AND ISOLATOR PLUS ROWS |  | 2.26"                         |             |  |  |
| 12                     | STONE ABOVE (in)                                                                    | MINIMUM ALLOWABLE GRADE (UNPAVED NO TRAFFIC):             | 7.75  | FLAMP                 | B              | INSTALL FLAMP ON 24" ACCESS PIPE / PART#: MCFLAMP (TYP 2 PLACES)                                                |  |                               |             |  |  |
| 9                      | STONE BELOW (in)                                                                    | MINIMUM ALLOWABLE GRADE (TOP OF RIGID CONCRETE PAVEMENT): | 7.75  | MANIFOLD              | C              | 48" x 24" ADS N-12 (48" PIPE)                                                                                   |  | -21.26"                       |             |  |  |
| 40                     | STONE VOID                                                                          | MINIMUM ALLOWABLE GRADE (BASE OF FLEXIBLE PAVEMENT):      | 7.75  | MANIFOLD              | D              | 48" x 24" ADS N-12 (24" PIPE)                                                                                   |  | -21.26"                       |             |  |  |
| 103                    | INSTALLED SYSTEM VOLUME (CF)<br>(PERIMETER STONE INCLUDED)<br>(BASE STONE INCLUDED) | TOP OF STONE:                                             | 6.75  | MANIFOLD              |                | 48" x 24" ADS N-12 (48" PIPE)                                                                                   |  | -21.26"                       |             |  |  |
|                        |                                                                                     | TOP OF MC-7200 CHAMBER:                                   | 5.75  | MANIFOLD              |                | 48" x 24" ADS N-12 (24" PIPE)                                                                                   |  | -21.26"                       |             |  |  |
| 3078                   | SYSTEM AREA (SF)                                                                    | 48" x 24" BOTTOM MANIFOLD INVERT (24" PIPE):              | 0.94  | CONCRETE STRUCTURE    | E              | (DESIGN BY ENGINEER / PROVIDED BY OTHERS)                                                                       |  |                               | 9.5 CFS IN  |  |  |
| 298.6                  | SYSTEM PERIMETER (ft)                                                               | 24" ISOLATOR ROW PLUS INVERT:                             | 0.94  | W/WEIR                |                |                                                                                                                 |  |                               |             |  |  |
|                        |                                                                                     | 24" ISOLATOR ROW PLUS INVERT:                             | 0.94  | CONCRETE STRUCTURE    | F              | (DESIGN BY ENGINEER / PROVIDED BY OTHERS)                                                                       |  |                               | 19.0 CFS IN |  |  |
|                        |                                                                                     | BOTTOM OF MC-7200 CHAMBER:                                | 0.75  | W/WEIR                |                |                                                                                                                 |  |                               |             |  |  |
|                        |                                                                                     | BOTTOM OF STONE:                                          | 0.00  |                       |                |                                                                                                                 |  |                               |             |  |  |
|                        |                                                                                     | 48" x 24" BOTTOM MANIFOLD INVERT (48" PIPE):              | -1.02 |                       |                |                                                                                                                 |  |                               |             |  |  |
|                        |                                                                                     | 48" x 24" BOTTOM MANIFOLD INVERT (48" PIPE):              | -1.02 |                       |                |                                                                                                                 |  |                               |             |  |  |



- 

ISOLATOR ROW PLUS  
(SEE DETAIL)
- 

PLACE MINIMUM 17.50' OF ADSPLUS125 WOVEN GEOTEXTILE OVER BEDDING STONE AND UNDERNEATH CHAMBER FEET FOR SCOUR PROTECTION AT ALL CHAMBER INLET ROWS

— BED LIMITS

- NOTES**
- THE SITE DESIGN ENGINEER MUST REVIEW ELEVATIONS AND IF NECESSARY ADJUST GRADING TO ENSURE THE CHAMBER COVER REQUIREMENTS ARE MET.
  - **NOT FOR CONSTRUCTION:** THIS LAYOUT IS FOR DIMENSIONAL PURPOSES ONLY TO PROVE CONCEPT & THE REQUIRED STORAGE VOLUME CAN BE ACHIEVED ON SITE.

OMPX-001 DMA-A

AZUSA, CA, USA

DATE: 03/17/2025

PROJECT #:

CHECKED: N/A

DESCRIPTION

CHK

DRW

DATE

**StormTech®**  
Chamber System

1-800-821-6710 | WWW.STORMTECH.COM

4640 TRUEMAN BLVD  
HILLIARD, OH 43026  
1-800-733-7473



SHEET

2 OF 5

0

10

20

THIS DRAWING HAS BEEN PREPARED BASED ON INFORMATION PROVIDED TO ADS BY STORMTECH UNDER THE DIRECTION OF THE PROJECT'S ENGINEER OF RECORD (EOR) OR OTHER PROJECT REPRESENTATIVE. THIS DRAWING IS NOT INTENDED FOR USE IN BIDDING OR CONSTRUCTION WITHOUT THE EOR'S PRIOR APPROVAL. EOR SHALL REVIEW THIS DRAWING PRIOR TO BIDDING AND/OR CONSTRUCTION. IT IS THE ULTIMATE RESPONSIBILITY OF THE EOR TO ENSURE THAT THE PRODUCT(S) DEPICTED AND ALL ASSOCIATED DETAILS MEET ALL APPLICABLE LAWS, REGULATIONS, AND PROJECT REQUIREMENTS.

## User Inputs

|                               |                     |
|-------------------------------|---------------------|
| Chamber Model:                | MC-7200             |
| Outlet Control Structure:     | No                  |
| Project Name:                 | OMPX-001 DMA-A      |
| Bed Name                      | Bed 3               |
| Engineer:                     | undefined undefined |
| Project Location:             | California          |
| Measurement Type:             | Imperial            |
| Required Storage Volume:      | 500 cubic ft.       |
| Stone Porosity:               | 40%                 |
| Stone Foundation Depth:       | 9 in.               |
| Stone Above Chambers:         | 12 in.              |
| Average Cover Over Chambers:  | 24 in.              |
| Design Constraint Dimensions: | (20 ft. x 20 ft.)   |

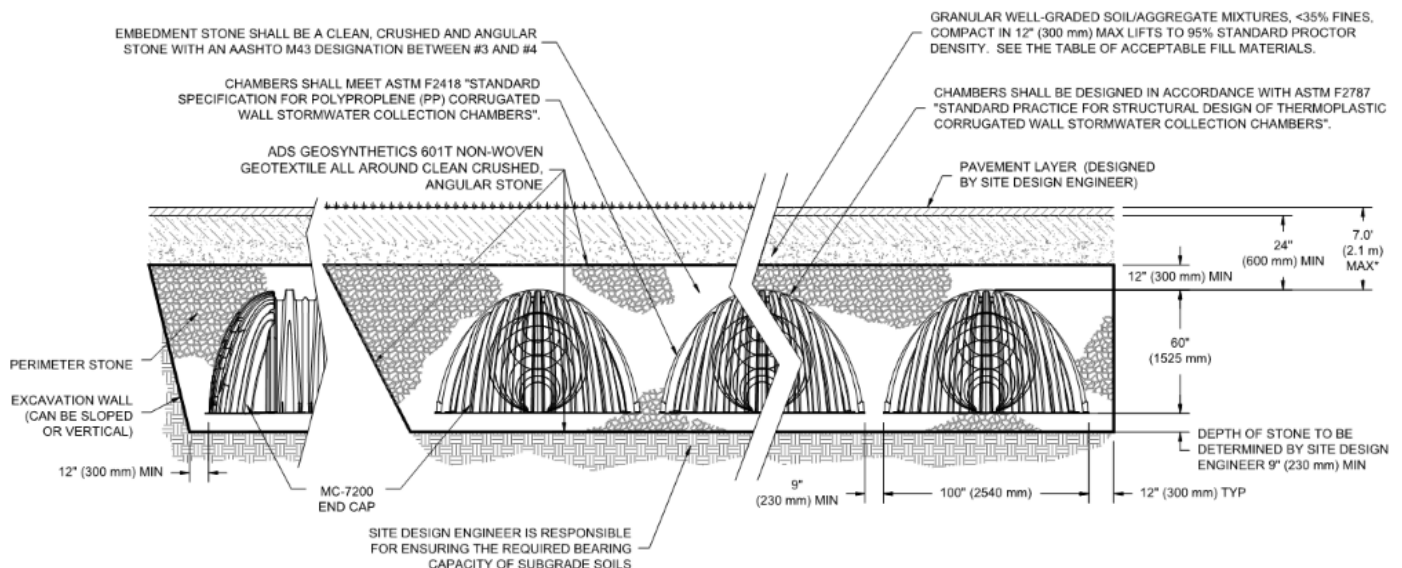
## Results

### System Volume and Bed Size

|                              |                    |
|------------------------------|--------------------|
| Installed Storage Volume:    | 25684.69 cubic ft. |
| Storage Volume Per Chamber:  | 175.90 cubic ft.   |
| Number Of Chambers Required: | 88                 |
| Number Of End Caps Required: | 8                  |
| Chamber Rows:                | 4                  |
| Maximum Length:              | 159.72 ft.         |
| Maximum Width:               | 37.58 ft.          |
| Approx. Bed Size Required:   | 6002.68 square ft. |

### System Components

|                                                     |                   |
|-----------------------------------------------------|-------------------|
| Amount Of Stone Required:                           | 916 cubic yards   |
| Volume Of Excavation (Not Including Fill):          | 1501 cubic yards  |
| Total Non-woven Geotextile Required:                | 1956 square yards |
| Woven Geotextile Required (excluding Isolator Row): | 88 square yards   |
| Woven Geotextile Required (Isolator Row):           | 352 square yards  |
| Total Woven Geotextile Required:                    | 439 square yards  |
| Impervious Liner Required:                          | 0 square yards    |



\*MINIMUM COVER TO BOTTOM OF FLEXIBLE PAVEMENT. FOR UNPAVED INSTALLATIONS WHERE RUTTING FROM VEHICLES MAY OCCUR, INCREASE COVER TO 30" (750 mm).



## User Inputs

|                                      |                   |
|--------------------------------------|-------------------|
| <b>Chamber Model:</b>                | MC-7200           |
| <b>Outlet Control Structure:</b>     | No                |
| <b>Project Name:</b>                 | OMPX-001 DMA-B    |
| <b>Engineer:</b>                     | Ka Hei Lam        |
| <b>Project Location:</b>             | California        |
| <b>Measurement Type:</b>             | Imperial          |
| <b>Required Storage Volume:</b>      | 500 cubic ft.     |
| <b>Stone Porosity:</b>               | 40%               |
| <b>Stone Foundation Depth:</b>       | 9 in.             |
| <b>Stone Above Chambers:</b>         | 12 in.            |
| <b>Average Cover Over Chambers:</b>  | 24 in.            |
| <b>Design Constraint Dimensions:</b> | (20 ft. x 20 ft.) |

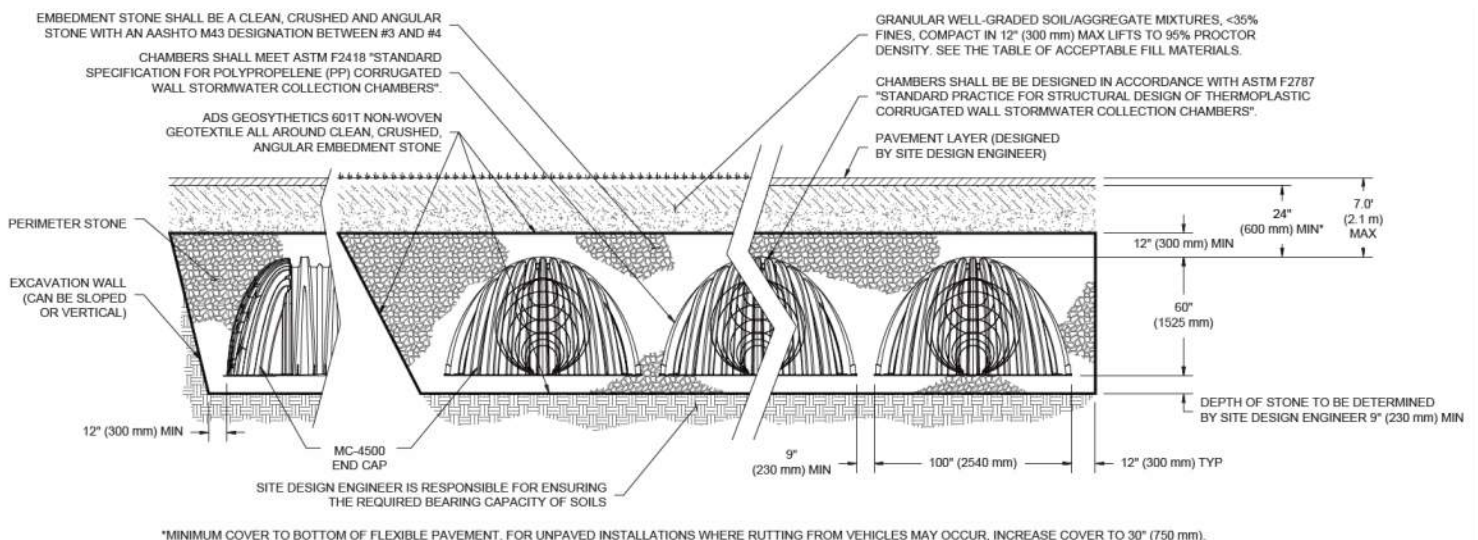
## Results

### System Volume and Bed Size

|                                     |                    |
|-------------------------------------|--------------------|
| <b>Installed Storage Volume:</b>    | 7202.46 cubic ft.  |
| <b>Storage Volume Per Chamber:</b>  | 175.90 cubic ft.   |
| <b>Number Of Chambers Required:</b> | 24                 |
| <b>Number Of End Caps Required:</b> | 2                  |
| <b>Chamber Rows:</b>                | 1                  |
| <b>Maximum Length:</b>              | 165.67 ft.         |
| <b>Maximum Width:</b>               | 10.33 ft.          |
| <b>Approx. Bed Size Required:</b>   | 1711.89 square ft. |

### System Components

|                                                            |                  |
|------------------------------------------------------------|------------------|
| <b>Amount Of Stone Required:</b>                           | 269 cubic yards  |
| <b>Volume Of Excavation (Not Including Fill):</b>          | 428 cubic yards  |
| <b>Total Non-woven Geotextile Required:</b>                | 774 square yards |
| <b>Woven Geotextile Required (excluding Isolator Row):</b> | 0 square yards   |
| <b>Woven Geotextile Required (Isolator Row):</b>           | 382 square yards |
| <b>Total Woven Geotextile Required:</b>                    | 382 square yards |
| <b>Impervious Liner Required:</b>                          | 0 square yards   |



# Hydraulic Analysis Report

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## Project Data

Project Title: OMPX-001 Detention Routing

Designer: C&V Consulting, Inc.

Project Date: Monday, March 17, 2025

Project Units: U.S. Customary Units

Notes: Federal Highway Administration. Hydraulic Toolbox 5.3.0.0

## Detention Basin Analysis: Detention Analysis - Q100

Notes:

### Storage Input Parameters

Storage Capacity: ADS Stormtech

| Elevation ft | Volume acre-ft |
|--------------|----------------|
| 0.00         | 0.00           |
| 1.00         | 0.20           |
| 2.00         | 0.53           |
| 3.00         | 0.85           |
| 4.00         | 1.14           |
| 5.00         | 1.39           |
| 6.00         | 1.57           |
| 6.67         | 1.69           |

### Discharge Input Parameters

Known Discharge: Infiltrated Q

Discharge cfs

|      |      |
|------|------|
| 0.00 | 1.00 |
| 1.00 | 1.00 |
| 2.00 | 1.00 |
| 3.00 | 1.00 |
| 4.00 | 1.00 |
| 5.00 | 1.00 |
| 6.00 | 1.00 |
| 6.67 | 1.00 |

Riser: Riser

Rectangular Opening

Opening Width: 6.0000 ft

Opening Height: 2.0000 ft

Weir Coefficient: 3.3300

Orifice Coefficient: 0.6000

Height above Base Elev to Bottom of Opening: 4.0000 ft

Base Elevation: 0.0000 ft

### Storage & Discharge Input Parameters

#### Elevation - Storage - Discharge

| Elevation ft | Storage acre-ft | Discharge cfs |
|--------------|-----------------|---------------|
| 0.00         | 0.00            | 1.00          |
| 1.00         | 0.20            | 1.00          |
| 2.00         | 0.53            | 1.00          |
| 3.00         | 0.85            | 1.00          |
| 4.00         | 1.14            | 1.00          |
| 5.00         | 1.39            | 20.98         |
| 6.00         | 1.57            | 57.51         |
| 6.67         | 1.69            | 75.67         |

### Detention Basin Input Parameters

#### Inflow Hydrograph

Peak Inflow Discharge: 91.874 cfs, Time to Peak: 1153.00 (min), Total Inflow Volume: 599764.67 ft<sup>3</sup>

| Time (min) | Discharge cfs |
|------------|---------------|
| 0.00       | 0.00          |
| 1.00       | 0.53          |
| 2.00       | 1.06          |
| 3.00       | 1.58          |
| 4.00       | 2.11          |
| 5.00       | 2.64          |
| 6.00       | 3.00          |
| 7.00       | 3.28          |
| 8.00       | 3.37          |
| 9.00       | 3.47          |
| 10.00      | 3.47          |
| 11.00      | 3.47          |
| 12.00      | 3.47          |

|       |      |
|-------|------|
| 13.00 | 3.47 |
| 14.00 | 3.47 |
| 15.00 | 3.48 |
| 16.00 | 3.48 |
| 17.00 | 3.48 |
| 18.00 | 3.48 |
| 19.00 | 3.48 |
| 20.00 | 3.48 |
| 21.00 | 3.48 |
| 22.00 | 3.49 |
| 23.00 | 3.49 |
| 24.00 | 3.49 |
| 25.00 | 3.49 |
| 26.00 | 3.49 |
| 27.00 | 3.49 |
| 28.00 | 3.49 |
| 29.00 | 3.50 |
| 30.00 | 3.50 |
| 31.00 | 3.50 |
| 32.00 | 3.50 |
| 33.00 | 3.50 |
| 34.00 | 3.50 |
| 35.00 | 3.50 |
| 36.00 | 3.51 |
| 37.00 | 3.51 |
| 38.00 | 3.51 |
| 39.00 | 3.51 |
| 40.00 | 3.51 |
| 41.00 | 3.51 |
| 42.00 | 3.51 |
| 43.00 | 3.52 |
| 44.00 | 3.52 |
| 45.00 | 3.52 |
| 46.00 | 3.52 |
| 47.00 | 3.52 |
| 48.00 | 3.52 |
| 49.00 | 3.53 |
| 50.00 | 3.53 |
| 51.00 | 3.53 |
| 52.00 | 3.53 |
| 53.00 | 3.53 |
| 54.00 | 3.53 |
| 55.00 | 3.53 |
| 56.00 | 3.54 |
| 57.00 | 3.54 |
| 58.00 | 3.54 |
| 59.00 | 3.54 |
| 60.00 | 3.54 |
| 61.00 | 3.54 |
| 62.00 | 3.54 |

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|--------|------|
| 63.00  | 3.55 |
| 64.00  | 3.55 |
| 65.00  | 3.55 |
| 66.00  | 3.55 |
| 67.00  | 3.55 |
| 68.00  | 3.55 |
| 69.00  | 3.56 |
| 70.00  | 3.56 |
| 71.00  | 3.56 |
| 72.00  | 3.56 |
| 73.00  | 3.56 |
| 74.00  | 3.56 |
| 75.00  | 3.56 |
| 76.00  | 3.57 |
| 77.00  | 3.57 |
| 78.00  | 3.57 |
| 79.00  | 3.57 |
| 80.00  | 3.57 |
| 81.00  | 3.57 |
| 82.00  | 3.58 |
| 83.00  | 3.58 |
| 84.00  | 3.58 |
| 85.00  | 3.58 |
| 86.00  | 3.58 |
| 87.00  | 3.58 |
| 88.00  | 3.59 |
| 89.00  | 3.59 |
| 90.00  | 3.59 |
| 91.00  | 3.59 |
| 92.00  | 3.59 |
| 93.00  | 3.59 |
| 94.00  | 3.60 |
| 95.00  | 3.60 |
| 96.00  | 3.60 |
| 97.00  | 3.60 |
| 98.00  | 3.60 |
| 99.00  | 3.61 |
| 100.00 | 3.61 |
| 101.00 | 3.61 |
| 102.00 | 3.61 |
| 103.00 | 3.61 |
| 104.00 | 3.61 |
| 105.00 | 3.62 |
| 106.00 | 3.62 |
| 107.00 | 3.62 |
| 108.00 | 3.62 |
| 109.00 | 3.62 |
| 110.00 | 3.62 |
| 111.00 | 3.63 |
| 112.00 | 3.63 |

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|--------|------|
| 113.00 | 3.63 |
| 114.00 | 3.63 |
| 115.00 | 3.63 |
| 116.00 | 3.64 |
| 117.00 | 3.64 |
| 118.00 | 3.64 |
| 119.00 | 3.64 |
| 120.00 | 3.64 |
| 121.00 | 3.65 |
| 122.00 | 3.65 |
| 123.00 | 3.65 |
| 124.00 | 3.65 |
| 125.00 | 3.65 |
| 126.00 | 3.65 |
| 127.00 | 3.66 |
| 128.00 | 3.66 |
| 129.00 | 3.66 |
| 130.00 | 3.66 |
| 131.00 | 3.66 |
| 132.00 | 3.67 |
| 133.00 | 3.67 |
| 134.00 | 3.67 |
| 135.00 | 3.67 |
| 136.00 | 3.67 |
| 137.00 | 3.67 |
| 138.00 | 3.68 |
| 139.00 | 3.68 |
| 140.00 | 3.68 |
| 141.00 | 3.68 |
| 142.00 | 3.68 |
| 143.00 | 3.69 |
| 144.00 | 3.69 |
| 145.00 | 3.69 |
| 146.00 | 3.69 |
| 147.00 | 3.69 |
| 148.00 | 3.70 |
| 149.00 | 3.70 |
| 150.00 | 3.70 |
| 151.00 | 3.70 |
| 152.00 | 3.70 |
| 153.00 | 3.71 |
| 154.00 | 3.71 |
| 155.00 | 3.71 |
| 156.00 | 3.71 |
| 157.00 | 3.71 |
| 158.00 | 3.71 |
| 159.00 | 3.72 |
| 160.00 | 3.72 |
| 161.00 | 3.72 |
| 162.00 | 3.72 |

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|--------|------|
| 163.00 | 3.72 |
| 164.00 | 3.73 |
| 165.00 | 3.73 |
| 166.00 | 3.73 |
| 167.00 | 3.73 |
| 168.00 | 3.73 |
| 169.00 | 3.74 |
| 170.00 | 3.74 |
| 171.00 | 3.74 |
| 172.00 | 3.74 |
| 173.00 | 3.74 |
| 174.00 | 3.75 |
| 175.00 | 3.75 |
| 176.00 | 3.75 |
| 177.00 | 3.75 |
| 178.00 | 3.75 |
| 179.00 | 3.76 |
| 180.00 | 3.76 |
| 181.00 | 3.76 |
| 182.00 | 3.76 |
| 183.00 | 3.76 |
| 184.00 | 3.77 |
| 185.00 | 3.77 |
| 186.00 | 3.77 |
| 187.00 | 3.77 |
| 188.00 | 3.77 |
| 189.00 | 3.78 |
| 190.00 | 3.78 |
| 191.00 | 3.78 |
| 192.00 | 3.78 |
| 193.00 | 3.79 |
| 194.00 | 3.79 |
| 195.00 | 3.79 |
| 196.00 | 3.79 |
| 197.00 | 3.79 |
| 198.00 | 3.80 |
| 199.00 | 3.80 |
| 200.00 | 3.80 |
| 201.00 | 3.80 |
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| 203.00 | 3.81 |
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| 205.00 | 3.81 |
| 206.00 | 3.81 |
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| 208.00 | 3.82 |
| 209.00 | 3.82 |
| 210.00 | 3.82 |
| 211.00 | 3.82 |
| 212.00 | 3.82 |

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| 213.00 | 3.83 |
| 214.00 | 3.83 |
| 215.00 | 3.83 |
| 216.00 | 3.83 |
| 217.00 | 3.84 |
| 218.00 | 3.84 |
| 219.00 | 3.84 |
| 220.00 | 3.84 |
| 221.00 | 3.84 |
| 222.00 | 3.85 |
| 223.00 | 3.85 |
| 224.00 | 3.85 |
| 225.00 | 3.85 |
| 226.00 | 3.85 |
| 227.00 | 3.86 |
| 228.00 | 3.86 |
| 229.00 | 3.86 |
| 230.00 | 3.86 |
| 231.00 | 3.87 |
| 232.00 | 3.87 |
| 233.00 | 3.87 |
| 234.00 | 3.87 |
| 235.00 | 3.87 |
| 236.00 | 3.88 |
| 237.00 | 3.88 |
| 238.00 | 3.88 |
| 239.00 | 3.88 |
| 240.00 | 3.89 |
| 241.00 | 3.89 |
| 242.00 | 3.89 |
| 243.00 | 3.89 |
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| 246.00 | 3.90 |
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| 248.00 | 3.90 |
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| 256.00 | 3.92 |
| 257.00 | 3.92 |
| 258.00 | 3.93 |
| 259.00 | 3.93 |
| 260.00 | 3.93 |
| 261.00 | 3.93 |
| 262.00 | 3.94 |

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| 264.00 | 3.94 |
| 265.00 | 3.94 |
| 266.00 | 3.94 |
| 267.00 | 3.95 |
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| 272.00 | 3.96 |
| 273.00 | 3.96 |
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| 277.00 | 3.97 |
| 278.00 | 3.97 |
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| 282.00 | 3.98 |
| 283.00 | 3.98 |
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| 290.00 | 4.00 |
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| 293.00 | 4.01 |
| 294.00 | 4.01 |
| 295.00 | 4.01 |
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| 297.00 | 4.02 |
| 298.00 | 4.02 |
| 299.00 | 4.02 |
| 300.00 | 4.03 |
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| 303.00 | 4.03 |
| 304.00 | 4.04 |
| 305.00 | 4.04 |
| 306.00 | 4.04 |
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| 308.00 | 4.05 |
| 309.00 | 4.05 |
| 310.00 | 4.05 |
| 311.00 | 4.05 |
| 312.00 | 4.06 |

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| 314.00 | 4.06 |
| 315.00 | 4.06 |
| 316.00 | 4.07 |
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| 319.00 | 4.07 |
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| 322.00 | 4.08 |
| 323.00 | 4.08 |
| 324.00 | 4.09 |
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| 329.00 | 4.10 |
| 330.00 | 4.10 |
| 331.00 | 4.11 |
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| 334.00 | 4.11 |
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| 340.00 | 4.13 |
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| 344.00 | 4.14 |
| 345.00 | 4.14 |
| 346.00 | 4.15 |
| 347.00 | 4.15 |
| 348.00 | 4.15 |
| 349.00 | 4.15 |
| 350.00 | 4.16 |
| 351.00 | 4.16 |
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| 353.00 | 4.16 |
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| 355.00 | 4.17 |
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| 360.00 | 4.18 |
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| 362.00 | 4.19 |

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| 364.00 | 4.19 |
| 365.00 | 4.20 |
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| 375.00 | 4.23 |
| 376.00 | 4.23 |
| 377.00 | 4.23 |
| 378.00 | 4.23 |
| 379.00 | 4.24 |
| 380.00 | 4.24 |
| 381.00 | 4.24 |
| 382.00 | 4.25 |
| 383.00 | 4.25 |
| 384.00 | 4.25 |
| 385.00 | 4.25 |
| 386.00 | 4.26 |
| 387.00 | 4.26 |
| 388.00 | 4.26 |
| 389.00 | 4.27 |
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| 391.00 | 4.27 |
| 392.00 | 4.28 |
| 393.00 | 4.28 |
| 394.00 | 4.28 |
| 395.00 | 4.28 |
| 396.00 | 4.29 |
| 397.00 | 4.29 |
| 398.00 | 4.29 |
| 399.00 | 4.30 |
| 400.00 | 4.30 |
| 401.00 | 4.30 |
| 402.00 | 4.30 |
| 403.00 | 4.31 |
| 404.00 | 4.31 |
| 405.00 | 4.31 |
| 406.00 | 4.32 |
| 407.00 | 4.32 |
| 408.00 | 4.32 |
| 409.00 | 4.33 |
| 410.00 | 4.33 |
| 411.00 | 4.33 |
| 412.00 | 4.34 |

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| 413.00 | 4.34 |
| 414.00 | 4.34 |
| 415.00 | 4.34 |
| 416.00 | 4.35 |
| 417.00 | 4.35 |
| 418.00 | 4.35 |
| 419.00 | 4.36 |
| 420.00 | 4.36 |
| 421.00 | 4.36 |
| 422.00 | 4.37 |
| 423.00 | 4.37 |
| 424.00 | 4.37 |
| 425.00 | 4.38 |
| 426.00 | 4.38 |
| 427.00 | 4.38 |
| 428.00 | 4.39 |
| 429.00 | 4.39 |
| 430.00 | 4.39 |
| 431.00 | 4.40 |
| 432.00 | 4.40 |
| 433.00 | 4.40 |
| 434.00 | 4.41 |
| 435.00 | 4.41 |
| 436.00 | 4.41 |
| 437.00 | 4.41 |
| 438.00 | 4.42 |
| 439.00 | 4.42 |
| 440.00 | 4.42 |
| 441.00 | 4.43 |
| 442.00 | 4.43 |
| 443.00 | 4.43 |
| 444.00 | 4.44 |
| 445.00 | 4.44 |
| 446.00 | 4.44 |
| 447.00 | 4.45 |
| 448.00 | 4.45 |
| 449.00 | 4.45 |
| 450.00 | 4.46 |
| 451.00 | 4.46 |
| 452.00 | 4.46 |
| 453.00 | 4.47 |
| 454.00 | 4.47 |
| 455.00 | 4.47 |
| 456.00 | 4.48 |
| 457.00 | 4.48 |
| 458.00 | 4.48 |
| 459.00 | 4.49 |
| 460.00 | 4.49 |
| 461.00 | 4.50 |
| 462.00 | 4.50 |

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|--------|------|
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| 464.00 | 4.51 |
| 465.00 | 4.51 |
| 466.00 | 4.51 |
| 467.00 | 4.52 |
| 468.00 | 4.52 |
| 469.00 | 4.52 |
| 470.00 | 4.53 |
| 471.00 | 4.53 |
| 472.00 | 4.53 |
| 473.00 | 4.54 |
| 474.00 | 4.54 |
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| 477.00 | 4.55 |
| 478.00 | 4.55 |
| 479.00 | 4.56 |
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| 483.00 | 4.57 |
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| 487.00 | 4.59 |
| 488.00 | 4.59 |
| 489.00 | 4.59 |
| 490.00 | 4.60 |
| 491.00 | 4.60 |
| 492.00 | 4.61 |
| 493.00 | 4.61 |
| 494.00 | 4.61 |
| 495.00 | 4.62 |
| 496.00 | 4.62 |
| 497.00 | 4.62 |
| 498.00 | 4.63 |
| 499.00 | 4.63 |
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| 501.00 | 4.64 |
| 502.00 | 4.64 |
| 503.00 | 4.65 |
| 504.00 | 4.65 |
| 505.00 | 4.65 |
| 506.00 | 4.66 |
| 507.00 | 4.66 |
| 508.00 | 4.67 |
| 509.00 | 4.67 |
| 510.00 | 4.67 |
| 511.00 | 4.68 |
| 512.00 | 4.68 |

|        |      |
|--------|------|
| 513.00 | 4.68 |
| 514.00 | 4.69 |
| 515.00 | 4.69 |
| 516.00 | 4.70 |
| 517.00 | 4.70 |
| 518.00 | 4.70 |
| 519.00 | 4.71 |
| 520.00 | 4.71 |
| 521.00 | 4.72 |
| 522.00 | 4.72 |
| 523.00 | 4.72 |
| 524.00 | 4.73 |
| 525.00 | 4.73 |
| 526.00 | 4.74 |
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| 530.00 | 4.75 |
| 531.00 | 4.76 |
| 532.00 | 4.76 |
| 533.00 | 4.76 |
| 534.00 | 4.77 |
| 535.00 | 4.77 |
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| 540.00 | 4.79 |
| 541.00 | 4.80 |
| 542.00 | 4.80 |
| 543.00 | 4.81 |
| 544.00 | 4.81 |
| 545.00 | 4.81 |
| 546.00 | 4.82 |
| 547.00 | 4.82 |
| 548.00 | 4.83 |
| 549.00 | 4.83 |
| 550.00 | 4.84 |
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| 558.00 | 4.87 |
| 559.00 | 4.87 |
| 560.00 | 4.88 |
| 561.00 | 4.88 |
| 562.00 | 4.89 |

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| 565.00 | 4.90 |
| 566.00 | 4.91 |
| 567.00 | 4.91 |
| 568.00 | 4.91 |
| 569.00 | 4.92 |
| 570.00 | 4.92 |
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| 572.00 | 4.93 |
| 573.00 | 4.94 |
| 574.00 | 4.94 |
| 575.00 | 4.95 |
| 576.00 | 4.95 |
| 577.00 | 4.95 |
| 578.00 | 4.96 |
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| 580.00 | 4.97 |
| 581.00 | 4.97 |
| 582.00 | 4.98 |
| 583.00 | 4.98 |
| 584.00 | 4.99 |
| 585.00 | 4.99 |
| 586.00 | 5.00 |
| 587.00 | 5.00 |
| 588.00 | 5.01 |
| 589.00 | 5.01 |
| 590.00 | 5.02 |
| 591.00 | 5.02 |
| 592.00 | 5.03 |
| 593.00 | 5.03 |
| 594.00 | 5.03 |
| 595.00 | 5.04 |
| 596.00 | 5.04 |
| 597.00 | 5.05 |
| 598.00 | 5.05 |
| 599.00 | 5.06 |
| 600.00 | 5.06 |
| 601.00 | 5.07 |
| 602.00 | 5.07 |
| 603.00 | 5.08 |
| 604.00 | 5.08 |
| 605.00 | 5.09 |
| 606.00 | 5.09 |
| 607.00 | 5.10 |
| 608.00 | 5.10 |
| 609.00 | 5.11 |
| 610.00 | 5.11 |
| 611.00 | 5.12 |
| 612.00 | 5.12 |

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|--------|------|
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| 614.00 | 5.13 |
| 615.00 | 5.14 |
| 616.00 | 5.14 |
| 617.00 | 5.15 |
| 618.00 | 5.15 |
| 619.00 | 5.16 |
| 620.00 | 5.16 |
| 621.00 | 5.17 |
| 622.00 | 5.17 |
| 623.00 | 5.18 |
| 624.00 | 5.19 |
| 625.00 | 5.19 |
| 626.00 | 5.20 |
| 627.00 | 5.20 |
| 628.00 | 5.21 |
| 629.00 | 5.21 |
| 630.00 | 5.22 |
| 631.00 | 5.22 |
| 632.00 | 5.23 |
| 633.00 | 5.23 |
| 634.00 | 5.24 |
| 635.00 | 5.24 |
| 636.00 | 5.25 |
| 637.00 | 5.25 |
| 638.00 | 5.26 |
| 639.00 | 5.27 |
| 640.00 | 5.27 |
| 641.00 | 5.28 |
| 642.00 | 5.28 |
| 643.00 | 5.29 |
| 644.00 | 5.29 |
| 645.00 | 5.30 |
| 646.00 | 5.30 |
| 647.00 | 5.31 |
| 648.00 | 5.32 |
| 649.00 | 5.32 |
| 650.00 | 5.33 |
| 651.00 | 5.33 |
| 652.00 | 5.34 |
| 653.00 | 5.34 |
| 654.00 | 5.35 |
| 655.00 | 5.36 |
| 656.00 | 5.36 |
| 657.00 | 5.37 |
| 658.00 | 5.37 |
| 659.00 | 5.38 |
| 660.00 | 5.38 |
| 661.00 | 5.39 |
| 662.00 | 5.40 |

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| 664.00 | 5.41 |
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| 712.00 | 5.71 |

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| 1012.00 | 10.01 |

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| 1262.00 | 5.83 |

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| 1322.00 | 4.60 |
| 1323.00 | 4.58 |
| 1324.00 | 4.57 |
| 1325.00 | 4.55 |
| 1326.00 | 4.54 |
| 1327.00 | 4.52 |
| 1328.00 | 4.51 |
| 1329.00 | 4.50 |
| 1330.00 | 4.48 |
| 1331.00 | 4.47 |
| 1332.00 | 4.46 |
| 1333.00 | 4.44 |
| 1334.00 | 4.43 |
| 1335.00 | 4.42 |
| 1336.00 | 4.40 |
| 1337.00 | 4.39 |
| 1338.00 | 4.38 |
| 1339.00 | 4.36 |
| 1340.00 | 4.35 |
| 1341.00 | 4.34 |
| 1342.00 | 4.33 |
| 1343.00 | 4.31 |
| 1344.00 | 4.30 |
| 1345.00 | 4.29 |
| 1346.00 | 4.28 |
| 1347.00 | 4.27 |
| 1348.00 | 4.26 |
| 1349.00 | 4.24 |
| 1350.00 | 4.23 |
| 1351.00 | 4.22 |
| 1352.00 | 4.21 |
| 1353.00 | 4.20 |
| 1354.00 | 4.19 |
| 1355.00 | 4.18 |
| 1356.00 | 4.16 |
| 1357.00 | 4.15 |
| 1358.00 | 4.14 |
| 1359.00 | 4.13 |
| 1360.00 | 4.12 |
| 1361.00 | 4.11 |
| 1362.00 | 4.10 |

|         |      |
|---------|------|
| 1363.00 | 4.09 |
| 1364.00 | 4.08 |
| 1365.00 | 4.07 |
| 1366.00 | 4.06 |
| 1367.00 | 4.05 |
| 1368.00 | 4.04 |
| 1369.00 | 4.03 |
| 1370.00 | 4.02 |
| 1371.00 | 4.01 |
| 1372.00 | 4.00 |
| 1373.00 | 3.99 |
| 1374.00 | 3.98 |
| 1375.00 | 3.97 |
| 1376.00 | 3.96 |
| 1377.00 | 3.95 |
| 1378.00 | 3.94 |
| 1379.00 | 3.93 |
| 1380.00 | 3.92 |
| 1381.00 | 3.92 |
| 1382.00 | 3.91 |
| 1383.00 | 3.90 |
| 1384.00 | 3.89 |
| 1385.00 | 3.88 |
| 1386.00 | 3.87 |
| 1387.00 | 3.86 |
| 1388.00 | 3.85 |
| 1389.00 | 3.84 |
| 1390.00 | 3.84 |
| 1391.00 | 3.83 |
| 1392.00 | 3.82 |
| 1393.00 | 3.81 |
| 1394.00 | 3.80 |
| 1395.00 | 3.79 |
| 1396.00 | 3.79 |
| 1397.00 | 3.78 |
| 1398.00 | 3.77 |
| 1399.00 | 3.76 |
| 1400.00 | 3.75 |
| 1401.00 | 3.75 |
| 1402.00 | 3.74 |
| 1403.00 | 3.73 |
| 1404.00 | 3.72 |
| 1405.00 | 3.71 |
| 1406.00 | 3.71 |
| 1407.00 | 3.70 |
| 1408.00 | 3.69 |
| 1409.00 | 3.68 |
| 1410.00 | 3.68 |
| 1411.00 | 3.67 |
| 1412.00 | 3.66 |

|         |      |
|---------|------|
| 1413.00 | 3.65 |
| 1414.00 | 3.65 |
| 1415.00 | 3.64 |
| 1416.00 | 3.63 |
| 1417.00 | 3.62 |
| 1418.00 | 3.62 |
| 1419.00 | 3.61 |
| 1420.00 | 3.60 |
| 1421.00 | 3.60 |
| 1422.00 | 3.59 |
| 1423.00 | 3.58 |
| 1424.00 | 3.57 |
| 1425.00 | 3.57 |
| 1426.00 | 3.56 |
| 1427.00 | 3.56 |
| 1428.00 | 3.55 |
| 1429.00 | 3.54 |
| 1430.00 | 3.54 |
| 1431.00 | 3.53 |
| 1432.00 | 3.53 |
| 1433.00 | 3.52 |
| 1434.00 | 3.51 |
| 1435.00 | 3.51 |
| 1436.00 | 3.50 |
| 1437.00 | 3.50 |
| 1438.00 | 3.49 |
| 1439.00 | 3.48 |
| 1440.00 | 3.48 |
| 1441.00 | 2.95 |
| 1442.00 | 2.42 |
| 1443.00 | 1.88 |
| 1444.00 | 1.36 |
| 1445.00 | 0.83 |
| 1446.00 | 0.46 |
| 1447.00 | 0.18 |
| 1448.00 | 0.09 |
| 1449.00 | 0.00 |

Initial Storage = 0.0000 acre-ft

## Detention Basin Result Parameters

### Routed Hydrograph

Peak Outflow Discharge: 73.945 cfs, Time to Peak: 1157.00 (min), Total Outflow Volume: 599769.18 ft<sup>3</sup>

| Time min | Discharge cfs | Storage acre-ft |
|----------|---------------|-----------------|
| 0.00     | 0.00          | 0.000000        |
| 1.00     | 0.00          | 0.000727        |
| 2.00     | 1.00          | 0.000806        |
| 3.00     | 1.00          | 0.001612        |
| 4.00     | 1.00          | 0.003145        |
| 5.00     | 1.00          | 0.005405        |
| 6.00     | 1.00          | 0.008168        |
| 7.00     | 1.00          | 0.011314        |
| 8.00     | 1.00          | 0.014587        |
| 9.00     | 1.00          | 0.017989        |
| 10.00    | 1.00          | 0.021392        |
| 11.00    | 1.00          | 0.024798        |
| 12.00    | 1.00          | 0.028205        |
| 13.00    | 1.00          | 0.031615        |
| 14.00    | 1.00          | 0.035026        |
| 15.00    | 1.00          | 0.038440        |
| 16.00    | 1.00          | 0.041855        |
| 17.00    | 1.00          | 0.045272        |
| 18.00    | 1.00          | 0.048691        |
| 19.00    | 1.00          | 0.052113        |
| 20.00    | 1.00          | 0.055536        |
| 21.00    | 1.00          | 0.058961        |
| 22.00    | 1.00          | 0.062388        |
| 23.00    | 1.00          | 0.065818        |
| 24.00    | 1.00          | 0.069249        |
| 25.00    | 1.00          | 0.072682        |
| 26.00    | 1.00          | 0.076117        |
| 27.00    | 1.00          | 0.079554        |
| 28.00    | 1.00          | 0.082994        |
| 29.00    | 1.00          | 0.086435        |
| 30.00    | 1.00          | 0.089878        |
| 31.00    | 1.00          | 0.093323        |
| 32.00    | 1.00          | 0.096771        |
| 33.00    | 1.00          | 0.100220        |
| 34.00    | 1.00          | 0.103671        |
| 35.00    | 1.00          | 0.107125        |
| 36.00    | 1.00          | 0.110580        |
| 37.00    | 1.00          | 0.114037        |
| 38.00    | 1.00          | 0.117497        |
| 39.00    | 1.00          | 0.120958        |
| 40.00    | 1.00          | 0.124422        |
| 41.00    | 1.00          | 0.127887        |

|       |      |          |
|-------|------|----------|
| 42.00 | 1.00 | 0.131355 |
| 43.00 | 1.00 | 0.134824 |
| 44.00 | 1.00 | 0.138296 |
| 45.00 | 1.00 | 0.141770 |
| 46.00 | 1.00 | 0.145245 |
| 47.00 | 1.00 | 0.148723 |
| 48.00 | 1.00 | 0.152203 |
| 49.00 | 1.00 | 0.155685 |
| 50.00 | 1.00 | 0.159169 |
| 51.00 | 1.00 | 0.162655 |
| 52.00 | 1.00 | 0.166143 |
| 53.00 | 1.00 | 0.169633 |
| 54.00 | 1.00 | 0.173126 |
| 55.00 | 1.00 | 0.176620 |
| 56.00 | 1.00 | 0.180116 |
| 57.00 | 1.00 | 0.183615 |
| 58.00 | 1.00 | 0.187116 |
| 59.00 | 1.00 | 0.190618 |
| 60.00 | 1.00 | 0.194123 |
| 61.00 | 1.00 | 0.197630 |
| 62.00 | 1.00 | 0.201139 |
| 63.00 | 1.00 | 0.204650 |
| 64.00 | 1.00 | 0.208163 |
| 65.00 | 1.00 | 0.211678 |
| 66.00 | 1.00 | 0.215196 |
| 67.00 | 1.00 | 0.218715 |
| 68.00 | 1.00 | 0.222237 |
| 69.00 | 1.00 | 0.225761 |
| 70.00 | 1.00 | 0.229287 |
| 71.00 | 1.00 | 0.232815 |
| 72.00 | 1.00 | 0.236345 |
| 73.00 | 1.00 | 0.239877 |
| 74.00 | 1.00 | 0.243411 |
| 75.00 | 1.00 | 0.246948 |
| 76.00 | 1.00 | 0.250487 |
| 77.00 | 1.00 | 0.254027 |
| 78.00 | 1.00 | 0.257570 |
| 79.00 | 1.00 | 0.261115 |
| 80.00 | 1.00 | 0.264663 |
| 81.00 | 1.00 | 0.268212 |
| 82.00 | 1.00 | 0.271764 |
| 83.00 | 1.00 | 0.275317 |
| 84.00 | 1.00 | 0.278873 |
| 85.00 | 1.00 | 0.282431 |
| 86.00 | 1.00 | 0.285992 |
| 87.00 | 1.00 | 0.289555 |
| 88.00 | 1.00 | 0.293120 |
| 89.00 | 1.00 | 0.296688 |
| 90.00 | 1.00 | 0.300258 |
| 91.00 | 1.00 | 0.303831 |

|        |      |          |
|--------|------|----------|
| 92.00  | 1.00 | 0.307406 |
| 93.00  | 1.00 | 0.310983 |
| 94.00  | 1.00 | 0.314563 |
| 95.00  | 1.00 | 0.318145 |
| 96.00  | 1.00 | 0.321730 |
| 97.00  | 1.00 | 0.325317 |
| 98.00  | 1.00 | 0.328907 |
| 99.00  | 1.00 | 0.332499 |
| 100.00 | 1.00 | 0.336094 |
| 101.00 | 1.00 | 0.339691 |
| 102.00 | 1.00 | 0.343290 |
| 103.00 | 1.00 | 0.346892 |
| 104.00 | 1.00 | 0.350496 |
| 105.00 | 1.00 | 0.354103 |
| 106.00 | 1.00 | 0.357712 |
| 107.00 | 1.00 | 0.361324 |
| 108.00 | 1.00 | 0.364939 |
| 109.00 | 1.00 | 0.368555 |
| 110.00 | 1.00 | 0.372175 |
| 111.00 | 1.00 | 0.375796 |
| 112.00 | 1.00 | 0.379421 |
| 113.00 | 1.00 | 0.383047 |
| 114.00 | 1.00 | 0.386677 |
| 115.00 | 1.00 | 0.390308 |
| 116.00 | 1.00 | 0.393943 |
| 117.00 | 1.00 | 0.397579 |
| 118.00 | 1.00 | 0.401219 |
| 119.00 | 1.00 | 0.404860 |
| 120.00 | 1.00 | 0.408505 |
| 121.00 | 1.00 | 0.412151 |
| 122.00 | 1.00 | 0.415801 |
| 123.00 | 1.00 | 0.419453 |
| 124.00 | 1.00 | 0.423107 |
| 125.00 | 1.00 | 0.426764 |
| 126.00 | 1.00 | 0.430424 |
| 127.00 | 1.00 | 0.434086 |
| 128.00 | 1.00 | 0.437750 |
| 129.00 | 1.00 | 0.441418 |
| 130.00 | 1.00 | 0.445087 |
| 131.00 | 1.00 | 0.448760 |
| 132.00 | 1.00 | 0.452435 |
| 133.00 | 1.00 | 0.456112 |
| 134.00 | 1.00 | 0.459792 |
| 135.00 | 1.00 | 0.463475 |
| 136.00 | 1.00 | 0.467160 |
| 137.00 | 1.00 | 0.470848 |
| 138.00 | 1.00 | 0.474538 |
| 139.00 | 1.00 | 0.478231 |
| 140.00 | 1.00 | 0.481927 |
| 141.00 | 1.00 | 0.485625 |

|        |      |          |
|--------|------|----------|
| 142.00 | 1.00 | 0.489326 |
| 143.00 | 1.00 | 0.493029 |
| 144.00 | 1.00 | 0.496735 |
| 145.00 | 1.00 | 0.500444 |
| 146.00 | 1.00 | 0.504155 |
| 147.00 | 1.00 | 0.507869 |
| 148.00 | 1.00 | 0.511586 |
| 149.00 | 1.00 | 0.515305 |
| 150.00 | 1.00 | 0.519027 |
| 151.00 | 1.00 | 0.522752 |
| 152.00 | 1.00 | 0.526479 |
| 153.00 | 1.00 | 0.530209 |
| 154.00 | 1.00 | 0.533941 |
| 155.00 | 1.00 | 0.537676 |
| 156.00 | 1.00 | 0.541414 |
| 157.00 | 1.00 | 0.545155 |
| 158.00 | 1.00 | 0.548898 |
| 159.00 | 1.00 | 0.552644 |
| 160.00 | 1.00 | 0.556392 |
| 161.00 | 1.00 | 0.560143 |
| 162.00 | 1.00 | 0.563897 |
| 163.00 | 1.00 | 0.567654 |
| 164.00 | 1.00 | 0.571413 |
| 165.00 | 1.00 | 0.575176 |
| 166.00 | 1.00 | 0.578940 |
| 167.00 | 1.00 | 0.582708 |
| 168.00 | 1.00 | 0.586478 |
| 169.00 | 1.00 | 0.590251 |
| 170.00 | 1.00 | 0.594026 |
| 171.00 | 1.00 | 0.597805 |
| 172.00 | 1.00 | 0.601586 |
| 173.00 | 1.00 | 0.605370 |
| 174.00 | 1.00 | 0.609156 |
| 175.00 | 1.00 | 0.612946 |
| 176.00 | 1.00 | 0.616738 |
| 177.00 | 1.00 | 0.620533 |
| 178.00 | 1.00 | 0.624330 |
| 179.00 | 1.00 | 0.628131 |
| 180.00 | 1.00 | 0.631934 |
| 181.00 | 1.00 | 0.635740 |
| 182.00 | 1.00 | 0.639549 |
| 183.00 | 1.00 | 0.643360 |
| 184.00 | 1.00 | 0.647174 |
| 185.00 | 1.00 | 0.650991 |
| 186.00 | 1.00 | 0.654811 |
| 187.00 | 1.00 | 0.658634 |
| 188.00 | 1.00 | 0.662460 |
| 189.00 | 1.00 | 0.666288 |
| 190.00 | 1.00 | 0.670119 |
| 191.00 | 1.00 | 0.673953 |

|        |      |          |
|--------|------|----------|
| 192.00 | 1.00 | 0.677790 |
| 193.00 | 1.00 | 0.681630 |
| 194.00 | 1.00 | 0.685472 |
| 195.00 | 1.00 | 0.689317 |
| 196.00 | 1.00 | 0.693165 |
| 197.00 | 1.00 | 0.697016 |
| 198.00 | 1.00 | 0.700870 |
| 199.00 | 1.00 | 0.704727 |
| 200.00 | 1.00 | 0.708586 |
| 201.00 | 1.00 | 0.712449 |
| 202.00 | 1.00 | 0.716314 |
| 203.00 | 1.00 | 0.720182 |
| 204.00 | 1.00 | 0.724053 |
| 205.00 | 1.00 | 0.727927 |
| 206.00 | 1.00 | 0.731804 |
| 207.00 | 1.00 | 0.735684 |
| 208.00 | 1.00 | 0.739566 |
| 209.00 | 1.00 | 0.743452 |
| 210.00 | 1.00 | 0.747340 |
| 211.00 | 1.00 | 0.751232 |
| 212.00 | 1.00 | 0.755126 |
| 213.00 | 1.00 | 0.759023 |
| 214.00 | 1.00 | 0.762923 |
| 215.00 | 1.00 | 0.766826 |
| 216.00 | 1.00 | 0.770732 |
| 217.00 | 1.00 | 0.774641 |
| 218.00 | 1.00 | 0.778553 |
| 219.00 | 1.00 | 0.782468 |
| 220.00 | 1.00 | 0.786386 |
| 221.00 | 1.00 | 0.790307 |
| 222.00 | 1.00 | 0.794231 |
| 223.00 | 1.00 | 0.798157 |
| 224.00 | 1.00 | 0.802087 |
| 225.00 | 1.00 | 0.806020 |
| 226.00 | 1.00 | 0.809956 |
| 227.00 | 1.00 | 0.813894 |
| 228.00 | 1.00 | 0.817836 |
| 229.00 | 1.00 | 0.821781 |
| 230.00 | 1.00 | 0.825728 |
| 231.00 | 1.00 | 0.829679 |
| 232.00 | 1.00 | 0.833633 |
| 233.00 | 1.00 | 0.837589 |
| 234.00 | 1.00 | 0.841549 |
| 235.00 | 1.00 | 0.845512 |
| 236.00 | 1.00 | 0.849478 |
| 237.00 | 1.00 | 0.853447 |
| 238.00 | 1.00 | 0.857419 |
| 239.00 | 1.00 | 0.861394 |
| 240.00 | 1.00 | 0.865372 |
| 241.00 | 1.00 | 0.869353 |

|        |      |          |
|--------|------|----------|
| 242.00 | 1.00 | 0.873337 |
| 243.00 | 1.00 | 0.877325 |
| 244.00 | 1.00 | 0.881315 |
| 245.00 | 1.00 | 0.885308 |
| 246.00 | 1.00 | 0.889305 |
| 247.00 | 1.00 | 0.893305 |
| 248.00 | 1.00 | 0.897307 |
| 249.00 | 1.00 | 0.901313 |
| 250.00 | 1.00 | 0.905322 |
| 251.00 | 1.00 | 0.909334 |
| 252.00 | 1.00 | 0.913350 |
| 253.00 | 1.00 | 0.917368 |
| 254.00 | 1.00 | 0.921389 |
| 255.00 | 1.00 | 0.925414 |
| 256.00 | 1.00 | 0.929442 |
| 257.00 | 1.00 | 0.933473 |
| 258.00 | 1.00 | 0.937507 |
| 259.00 | 1.00 | 0.941544 |
| 260.00 | 1.00 | 0.945585 |
| 261.00 | 1.00 | 0.949628 |
| 262.00 | 1.00 | 0.953675 |
| 263.00 | 1.00 | 0.957725 |
| 264.00 | 1.00 | 0.961778 |
| 265.00 | 1.00 | 0.965835 |
| 266.00 | 1.00 | 0.969894 |
| 267.00 | 1.00 | 0.973957 |
| 268.00 | 1.00 | 0.978023 |
| 269.00 | 1.00 | 0.982092 |
| 270.00 | 1.00 | 0.986165 |
| 271.00 | 1.00 | 0.990241 |
| 272.00 | 1.00 | 0.994319 |
| 273.00 | 1.00 | 0.998402 |
| 274.00 | 1.00 | 1.002487 |
| 275.00 | 1.00 | 1.006576 |
| 276.00 | 1.00 | 1.010668 |
| 277.00 | 1.00 | 1.014763 |
| 278.00 | 1.00 | 1.018861 |
| 279.00 | 1.00 | 1.022963 |
| 280.00 | 1.00 | 1.027068 |
| 281.00 | 1.00 | 1.031176 |
| 282.00 | 1.00 | 1.035288 |
| 283.00 | 1.00 | 1.039403 |
| 284.00 | 1.00 | 1.043521 |
| 285.00 | 1.00 | 1.047642 |
| 286.00 | 1.00 | 1.051767 |
| 287.00 | 1.00 | 1.055895 |
| 288.00 | 1.00 | 1.060027 |
| 289.00 | 1.00 | 1.064162 |
| 290.00 | 1.00 | 1.068300 |
| 291.00 | 1.00 | 1.072441 |

|        |      |          |
|--------|------|----------|
| 292.00 | 1.00 | 1.076586 |
| 293.00 | 1.00 | 1.080735 |
| 294.00 | 1.00 | 1.084886 |
| 295.00 | 1.00 | 1.089041 |
| 296.00 | 1.00 | 1.093199 |
| 297.00 | 1.00 | 1.097361 |
| 298.00 | 1.00 | 1.101526 |
| 299.00 | 1.00 | 1.105695 |
| 300.00 | 1.00 | 1.109867 |
| 301.00 | 1.00 | 1.114042 |
| 302.00 | 1.00 | 1.118221 |
| 303.00 | 1.00 | 1.122403 |
| 304.00 | 1.00 | 1.126589 |
| 305.00 | 1.00 | 1.130778 |
| 306.00 | 1.00 | 1.134970 |
| 307.00 | 1.00 | 1.139166 |
| 308.00 | 1.00 | 1.143366 |
| 309.00 | 1.26 | 1.147200 |
| 310.00 | 1.55 | 1.150642 |
| 311.00 | 1.81 | 1.153732 |
| 312.00 | 2.04 | 1.156506 |
| 313.00 | 2.25 | 1.158998 |
| 314.00 | 2.44 | 1.161235 |
| 315.00 | 2.60 | 1.163244 |
| 316.00 | 2.76 | 1.165050 |
| 317.00 | 2.89 | 1.166672 |
| 318.00 | 3.01 | 1.168130 |
| 319.00 | 3.12 | 1.169441 |
| 320.00 | 3.22 | 1.170620 |
| 321.00 | 3.31 | 1.171681 |
| 322.00 | 3.39 | 1.172635 |
| 323.00 | 3.46 | 1.173495 |
| 324.00 | 3.53 | 1.174269 |
| 325.00 | 3.58 | 1.174966 |
| 326.00 | 3.64 | 1.175595 |
| 327.00 | 3.68 | 1.176162 |
| 328.00 | 3.73 | 1.176675 |
| 329.00 | 3.76 | 1.177137 |
| 330.00 | 3.80 | 1.177556 |
| 331.00 | 3.83 | 1.177934 |
| 332.00 | 3.86 | 1.178277 |
| 333.00 | 3.88 | 1.178588 |
| 334.00 | 3.91 | 1.178870 |
| 335.00 | 3.93 | 1.179127 |
| 336.00 | 3.95 | 1.179360 |
| 337.00 | 3.97 | 1.179573 |
| 338.00 | 3.98 | 1.179767 |
| 339.00 | 4.00 | 1.179945 |
| 340.00 | 4.01 | 1.180108 |
| 341.00 | 4.02 | 1.180258 |

|        |      |          |
|--------|------|----------|
| 342.00 | 4.03 | 1.180395 |
| 343.00 | 4.04 | 1.180522 |
| 344.00 | 4.05 | 1.180639 |
| 345.00 | 4.06 | 1.180748 |
| 346.00 | 4.07 | 1.180849 |
| 347.00 | 4.08 | 1.180943 |
| 348.00 | 4.09 | 1.181031 |
| 349.00 | 4.09 | 1.181114 |
| 350.00 | 4.10 | 1.181191 |
| 351.00 | 4.11 | 1.181264 |
| 352.00 | 4.11 | 1.181333 |
| 353.00 | 4.12 | 1.181399 |
| 354.00 | 4.12 | 1.181461 |
| 355.00 | 4.13 | 1.181520 |
| 356.00 | 4.13 | 1.181577 |
| 357.00 | 4.14 | 1.181632 |
| 358.00 | 4.14 | 1.181685 |
| 359.00 | 4.14 | 1.181735 |
| 360.00 | 4.15 | 1.181785 |
| 361.00 | 4.15 | 1.181832 |
| 362.00 | 4.16 | 1.181879 |
| 363.00 | 4.16 | 1.181924 |
| 364.00 | 4.16 | 1.181968 |
| 365.00 | 4.17 | 1.182011 |
| 366.00 | 4.17 | 1.182054 |
| 367.00 | 4.17 | 1.182096 |
| 368.00 | 4.18 | 1.182137 |
| 369.00 | 4.18 | 1.182177 |
| 370.00 | 4.18 | 1.182218 |
| 371.00 | 4.19 | 1.182257 |
| 372.00 | 4.19 | 1.182296 |
| 373.00 | 4.19 | 1.182335 |
| 374.00 | 4.19 | 1.182374 |
| 375.00 | 4.20 | 1.182412 |
| 376.00 | 4.20 | 1.182451 |
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| 391.00 | 4.25 | 1.183011 |

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| 722.00 | 5.72 | 1.201667 |
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| 735.00 | 5.80 | 1.202744 |
| 736.00 | 5.81 | 1.202829 |
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| 744.00 | 5.86 | 1.203516 |
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| 746.00 | 5.88 | 1.203691 |
| 747.00 | 5.88 | 1.203779 |
| 748.00 | 5.89 | 1.203867 |
| 749.00 | 5.90 | 1.203956 |
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| 751.00 | 5.91 | 1.204134 |
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| 754.00 | 5.93 | 1.204403 |
| 755.00 | 5.94 | 1.204494 |
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| 764.00 | 6.01 | 1.205323 |
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| 793.00 | 6.23 | 1.208198 |
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| 797.00 | 6.27 | 1.208622 |
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| 799.00 | 6.28 | 1.208836 |
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| 811.00 | 6.39 | 1.210161 |
| 812.00 | 6.40 | 1.210275 |
| 813.00 | 6.41 | 1.210389 |
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| 824.00 | 6.51 | 1.211676 |
| 825.00 | 6.52 | 1.211796 |
| 826.00 | 6.53 | 1.211917 |
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| 828.00 | 6.55 | 1.212159 |
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| 1312.00 | 4.91 | 1.210403 |
| 1313.00 | 4.90 | 1.210178 |
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| 1316.00 | 4.84 | 1.209518 |
| 1317.00 | 4.83 | 1.209302 |
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| 1325.00 | 4.70 | 1.207656 |
| 1326.00 | 4.68 | 1.207459 |
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| 1743.00 | 1.00 | 0.764938 |
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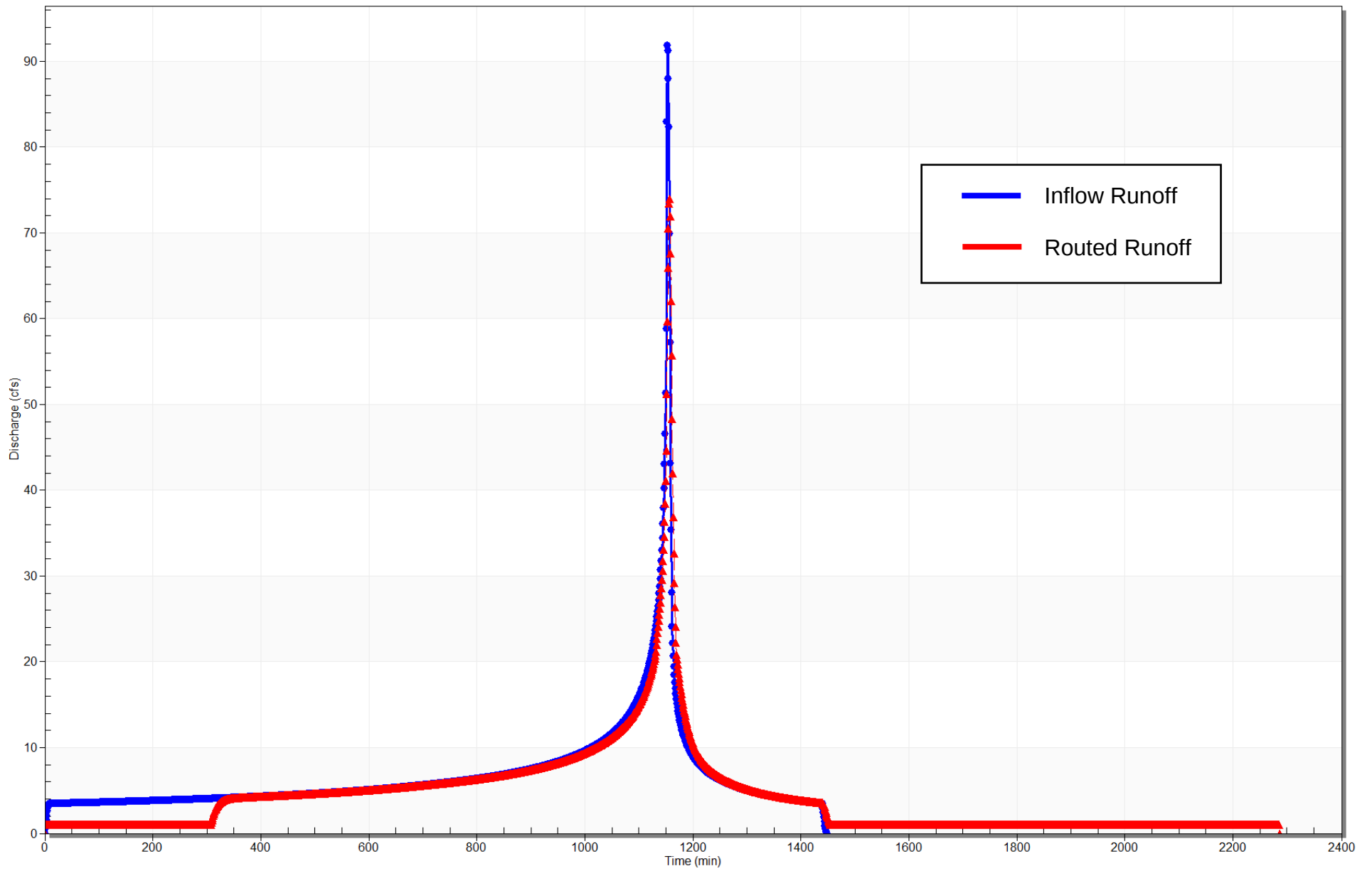
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| 2209.00 | 1.00 | 0.124736 |
| 2210.00 | 1.00 | 0.123362 |
| 2211.00 | 1.00 | 0.121989 |
| 2212.00 | 1.00 | 0.120615 |
| 2213.00 | 1.00 | 0.119241 |
| 2214.00 | 1.00 | 0.117867 |
| 2215.00 | 1.00 | 0.116493 |
| 2216.00 | 1.00 | 0.115119 |
| 2217.00 | 1.00 | 0.113746 |
| 2218.00 | 1.00 | 0.112372 |
| 2219.00 | 1.00 | 0.110998 |
| 2220.00 | 1.00 | 0.109624 |
| 2221.00 | 1.00 | 0.108250 |
| 2222.00 | 1.00 | 0.106877 |
| 2223.00 | 1.00 | 0.105503 |
| 2224.00 | 1.00 | 0.104129 |
| 2225.00 | 1.00 | 0.102755 |
| 2226.00 | 1.00 | 0.101381 |
| 2227.00 | 1.00 | 0.100007 |
| 2228.00 | 1.00 | 0.098634 |
| 2229.00 | 1.00 | 0.097260 |
| 2230.00 | 1.00 | 0.095886 |
| 2231.00 | 1.00 | 0.094512 |
| 2232.00 | 1.00 | 0.093138 |
| 2233.00 | 1.00 | 0.091764 |
| 2234.00 | 1.00 | 0.090391 |
| 2235.00 | 1.00 | 0.089017 |
| 2236.00 | 1.00 | 0.087643 |
| 2237.00 | 1.00 | 0.086269 |
| 2238.00 | 1.00 | 0.084895 |
| 2239.00 | 1.00 | 0.083522 |
| 2240.00 | 1.00 | 0.082148 |
| 2241.00 | 1.00 | 0.080774 |

|         |      |          |
|---------|------|----------|
| 2242.00 | 1.00 | 0.079400 |
| 2243.00 | 1.00 | 0.078026 |
| 2244.00 | 1.00 | 0.076652 |
| 2245.00 | 1.00 | 0.075279 |
| 2246.00 | 1.00 | 0.073905 |
| 2247.00 | 1.00 | 0.072531 |
| 2248.00 | 1.00 | 0.071157 |
| 2249.00 | 1.00 | 0.069783 |
| 2250.00 | 1.00 | 0.068409 |
| 2251.00 | 1.00 | 0.067036 |
| 2252.00 | 1.00 | 0.065662 |
| 2253.00 | 1.00 | 0.064288 |
| 2254.00 | 1.00 | 0.062914 |
| 2255.00 | 1.00 | 0.061540 |
| 2256.00 | 1.00 | 0.060167 |
| 2257.00 | 1.00 | 0.058793 |
| 2258.00 | 1.00 | 0.057419 |
| 2259.00 | 1.00 | 0.056045 |
| 2260.00 | 1.00 | 0.054671 |
| 2261.00 | 1.00 | 0.053297 |
| 2262.00 | 1.00 | 0.051924 |
| 2263.00 | 1.00 | 0.050550 |
| 2264.00 | 1.00 | 0.049176 |
| 2265.00 | 1.00 | 0.047802 |
| 2266.00 | 1.00 | 0.046428 |
| 2267.00 | 1.00 | 0.045054 |
| 2268.00 | 1.00 | 0.043681 |
| 2269.00 | 1.00 | 0.042307 |
| 2270.00 | 1.00 | 0.040933 |
| 2271.00 | 1.00 | 0.039559 |
| 2272.00 | 1.00 | 0.038185 |
| 2273.00 | 1.00 | 0.036812 |
| 2274.00 | 1.00 | 0.035438 |
| 2275.00 | 1.00 | 0.034064 |
| 2276.00 | 1.00 | 0.032690 |
| 2277.00 | 1.00 | 0.031316 |
| 2278.00 | 1.00 | 0.029942 |
| 2279.00 | 1.00 | 0.028569 |
| 2280.00 | 1.00 | 0.027195 |
| 2281.00 | 1.00 | 0.025821 |
| 2282.00 | 1.00 | 0.024447 |
| 2283.00 | 1.00 | 0.023073 |
| 2284.00 | 1.00 | 0.021699 |
| 2285.00 | 1.00 | 0.020326 |
| 2286.00 | 1.00 | 0.018952 |
| 2287.00 | 0.00 | 0.018952 |

Routed Hydrographs  
Q100 Storm Event



## **APPENDIX D**

### Hydraulics Analysis

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# Channel Report

## Outlet Pipe - Q100

### Circular

Diameter (ft) = 3.00

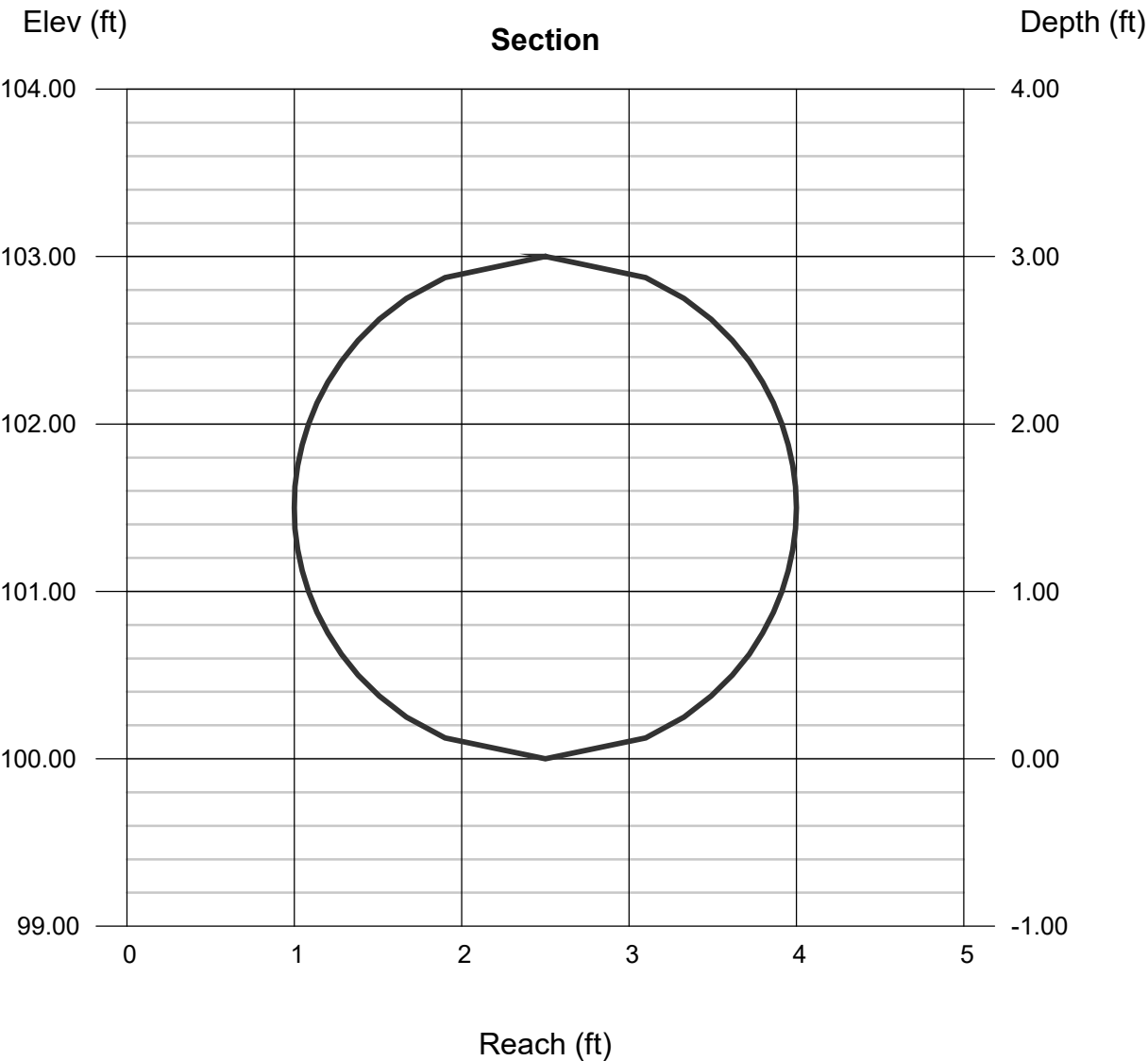
Invert Elev (ft) = 100.00  
Slope (%) = 1.25  
N-Value = 0.013

### Calculations

Compute by: Q vs Depth  
No. Increments = 1

### Highlighted

Depth (ft) = 3.00  
Q (cfs) = 74.56  
Area (sqft) = 7.07  
Velocity (ft/s) = 10.55  
Wetted Perim (ft) = 9.42  
Crit Depth, Yc (ft) = 2.72  
Top Width (ft) = 0.00  
EGL (ft) = 4.73



## **APPENDIX E**

### References

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## Los Angeles County Public Works – Allowable Q

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**LOS ANGELES COUNTY  
PUBLIC WORKS  
DESIGN DIVISION – HYDRAULIC ANALYSIS UNIT**

**INFORMATION REQUEST INSTRUCTIONS**

1. Please fill out the Information Request Summary on page 2 of this PDF form. **The fields marked with an asterisk are required.** At the bottom are buttons that you may use to Save and/or Print the form. Please save the form and email it as an attachment with "Information Request" in the subject line along with any other attachments for requests as described in items 2 - 4 below. Address the email to [hydraulicinfo@dpw.lacounty.gov](mailto:hydraulicinfo@dpw.lacounty.gov). Please note that an incomplete form will cause a delay in processing.
2. Include the as-built drawing showing the proposed connection location and identify the station or catch basin at the proposed connection. Most as-built drawings can be obtained from the following website: <http://dpw.lacounty.gov/des/plans/>. Please make sure that you select the Drainage tab prior to searching. The Los Angeles County Storm Drain System website <http://dpw.lacounty.gov/fcd/stormdrain/> is also a useful reference.
3. A project Location Map is required with your submission. The Location Map can be made using Google Maps. Please shade the area with your proposed improvements on the location map, including any offsite areas draining into this area, and attach it with your email request. Alternatively you can use an assessor map as your location map while still adhering to the above instructions. You can obtain the assessor map from <http://assessor.lacounty.gov/homeowners/property-search>.
4. In addition, submit a preliminary sketch or plan showing any proposed improvements in the areas you indicated on the assessor map.

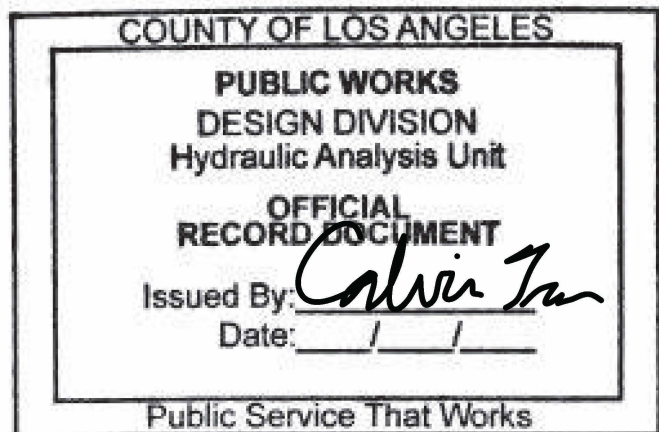
**If you have any questions, contact Design Division staff via email  
at [hydraulicinfo@dpw.lacounty.gov](mailto:hydraulicinfo@dpw.lacounty.gov) or at (626) 458-7806**

***BELOW SECTION TO BE COMPLETED BY THE HYDRAULIC ANALYSIS UNIT***

**REFERENCES SEARCHED:** LA County Storm Drain Network

**RESPONSE:**

The Allowable Q = 3.6 cfs



**INFORMATION PROVIDED BY:** Sama Ali

**Date:** 5/3/2023

**INFORMATION REVIEWED BY:** C. TRAN

**Date:** 5/12/23



LOS ANGELES COUNTY  
PUBLIC WORKS  
DESIGN DIVISION – HYDRAULIC ANALYSIS UNIT

INFORMATION REQUEST SUMMARY

INFORMATION REQUESTED BY

\*Requester's Name Evan Salcido  
\*Company: C&V Consulting, Inc.  
\*Phone Number: (949) 445-1812 Date: 3/10/23  
\*Email: esalcido@cvc-inc.net

Intended Use: Allowable Q Request Storm Water Drainage

Proposed Project Type: Residential Acreage Involved: 20.53

\*Will information be used in any litigation? ☐ YES ☒ NO  
Case Info. Name: \_\_\_\_\_ No: \_\_\_\_\_ Location: \_\_\_\_\_

INFORMATION NEEDED:

☒ Location Map Attached

☒ Plans Attached

LACFCD Facility: Name: Beatty Canyon Channel

Unit: \_\_\_\_\_ Line: DWG 318 - D 1.5

City: Azusa Station: 38+00 to 49+56.6

Address: 919 West Sierra Madre Avenue

\*Street/Cross-street: W. Sierra Madre Ave/N. Ve

REQUEST INFORMATION:  
(please select all that apply)

☐

HYDROLOGY

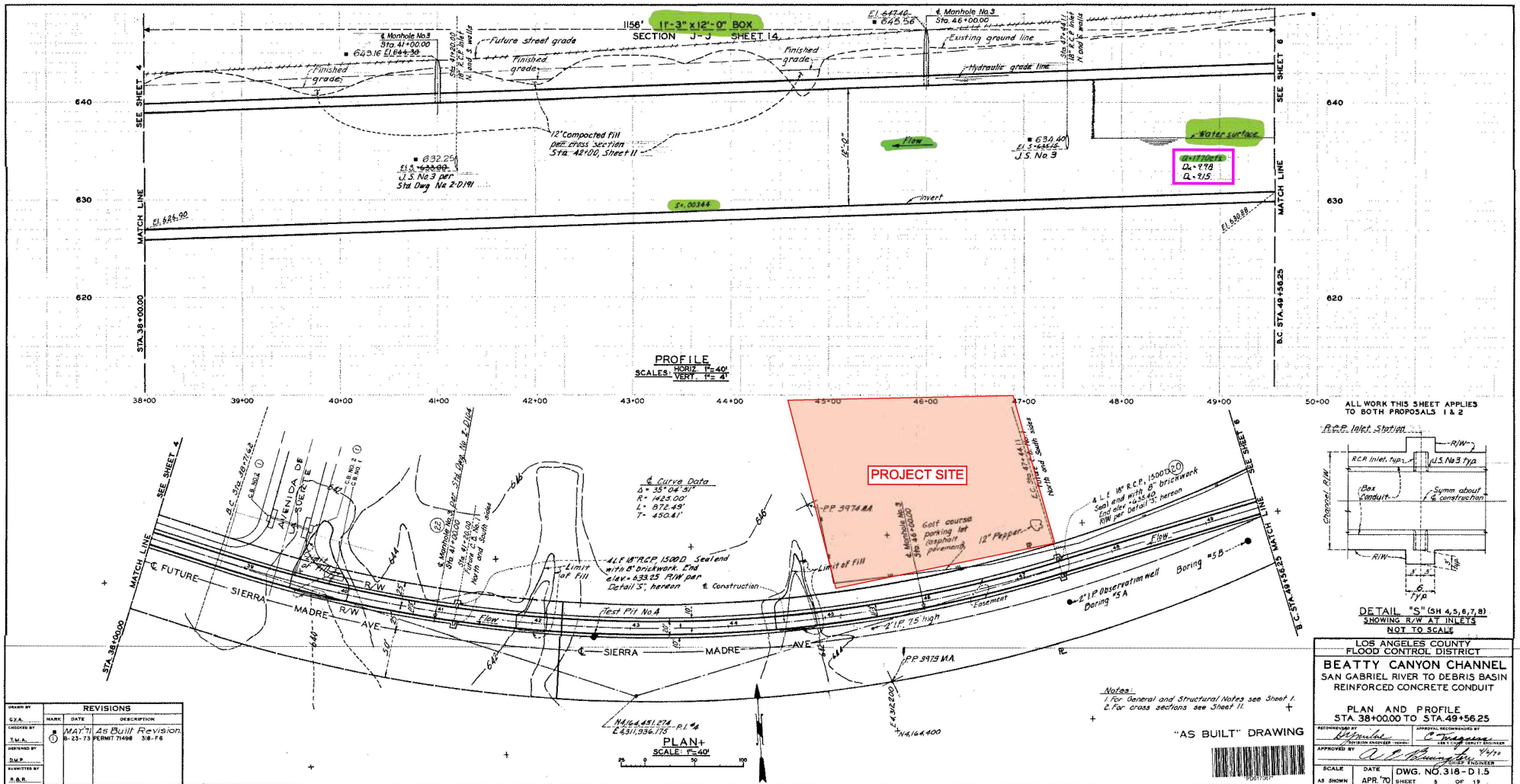
☐

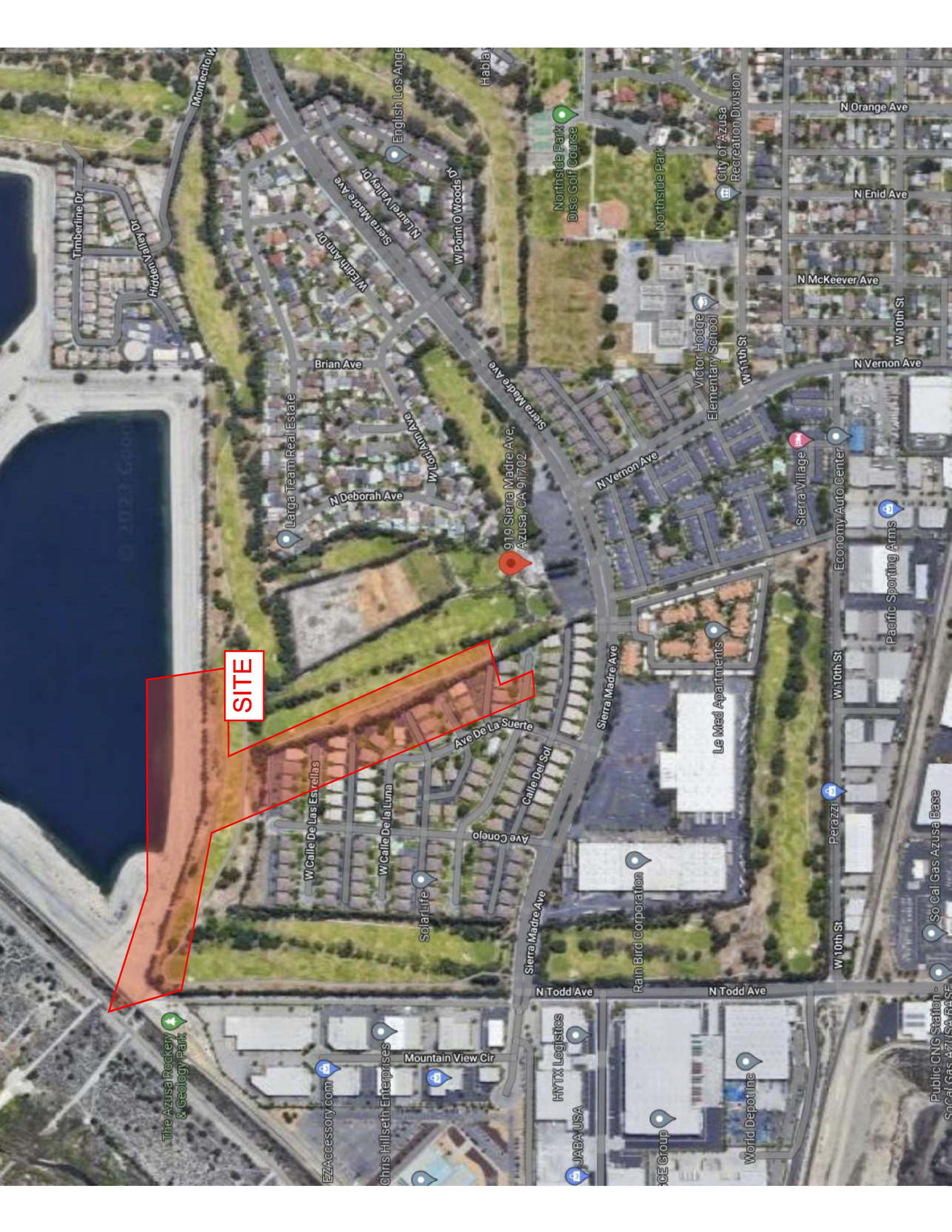
HYDRAULICS

☒

ALLOWABLE Q  
(DISCHARGE)

Additional Info Requested:





SITE



#### VILLAS

3 story buildings  
150 homes  
6.19 acres (approximate)  
24.2 du/ac  
140,025 to 149,250 total NRSF  
933 to 995 average NRSF  
10,000 sf leasing/club/fitness/yoga/kitchen/mailroom  
1,500 sf cabana

#### MIX

|       |                  |     |                 |                                 |           |
|-------|------------------|-----|-----------------|---------------------------------|-----------|
| A1    | 1 bedroom/1 bath | 18  | 650 to 700 sf   | stacked flat                    | 24' x 32' |
| A2    | 1 bedroom/1 bath | 42  | 750 to 800 sf   | stacked flat                    | 26' x 32' |
| A3    | 1 bedroom/1 bath | 21  | 900 to 950 sf   | stacked flat                    | 28' x 35' |
| B1    | 2 bed/2 bath     | 21  | 1025 to 1100 sf | stacked flat (could be 1 + den) | 36' x 32' |
| B2    | 2 bed/2 bath     | 24  | 1125 to 1200 sf | stacked flat                    | 37' x 34' |
| B3    | 2 bed/2 bath     | 24  | 1225 to 1300 sf | stacked flat (corner location)  | 37' x 32' |
| TOTAL |                  | 69  | 46%             | 2 bedroom                       |           |
|       |                  | 150 |                 |                                 |           |

#### PARKING (per city code)

|                            |                            |                      |
|----------------------------|----------------------------|----------------------|
| Required                   | 150 units x 1.00 spaces/du | 150 (in garage)      |
| 150 guest x 0.25 spaces/du | 38                         |                      |
| TOTAL                      | 188 (1.25 spaces/du)       |                      |
| Provided                   | Open/Carport               | 188 (1.25 spaces/du) |

#### Notes:

1. Open parking stalls 9' by 20'
2. Drive aisles 24' (fire may require wider)
3. Single car garages 10' x 20' minimum

#### COTTAGES

1 story buildings  
3 building types - Type 100 is B1/B2, Type 200 is B2/B3 and Type 300 is B1/B3  
There are 14 Type 100, 16 Type 200 and 10 Type 300  
80 homes  
14.91 acres (approximate)  
5.4 du/ac  
114,200 to 120,200 total NRSF  
1427 to 1502 average NRSF  
4 outside BBQ/tables/gathering areas

#### MIX

|       |                  |    |     |                 |                                         |           |
|-------|------------------|----|-----|-----------------|-----------------------------------------|-----------|
| B1    | 2 bed/2 bath     | 24 | 30% | 1325 to 1400 sf | stacked flat with 1 car attached garage | 26' x 58' |
| B2    | 2 bed/2 bath     | 30 | 38% | 1425 to 1500 sf | stacked flat with 1 car attached garage | 32' x 55' |
| B3    | 2 bed+den/2 bath | 26 | 33% | 1525 to 1600 sf | stacked flat with 2 car attached garage | 36' x 55' |
| TOTAL |                  | 80 |     |                 |                                         |           |

#### PARKING (per city code)

|                           |                           |                |
|---------------------------|---------------------------|----------------|
| Required                  | 80 units x 1.00 spaces/du | 80 (in garage) |
| 80 guest x 0.25 spaces/du | 20                        |                |
| TOTAL                     | 100 (1.25 spaces/du)      |                |

#### Provided

|                           |                                           |  |
|---------------------------|-------------------------------------------|--|
| Garages                   |                                           |  |
| 1 car                     | 54 (24 B1 and 30 B2 have 1 car garage)    |  |
| 2 car                     | 52 (26 B3 have 2 car garage)              |  |
| Garage Apron (2nd tandem) | TBD                                       |  |
| Open for guest            | 20 (90 degree)                            |  |
| TOTAL                     | 126 (1.58 spaces/du) (plus garage aprons) |  |

#### Notes:

1. Open parking stalls 9' by 20'
2. Drive aisles 24' (fire may require wider)
3. Single car garages 10' x 20' minimum
4. The number and location of the buildings with long aprons may vary as the site plan and plotting of the building adjust

## GREENS GOLF COURSE

Azusa, California

OVERTON MOORE PROPERTIES

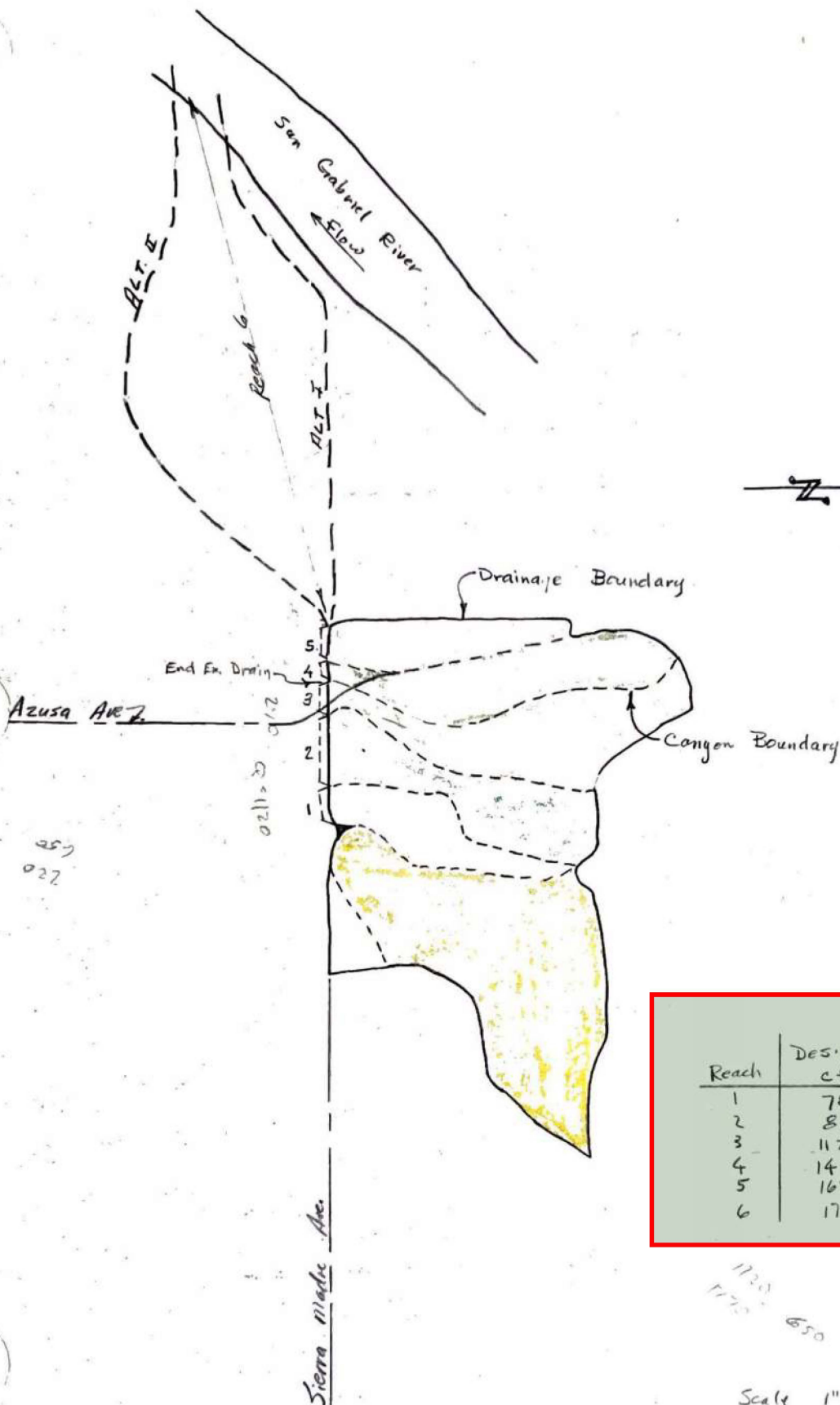


Site Plan



01

0022-041  
0022-01-16



Scale 1"=2000'

Copy of map, dated Oct. 1968  
from Hydrologist

## LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

STORM DRAIN ANALYSIS  
(INPUT)

EDP JOB R4412

DATE 1-12-70

PAGE 1

PROJECT BEATTY CANYON CHANNEL BOX ALTERNATE  
DESIGNER ED MARCISZ

| STATION         | CD | L2 | MAX Q  | ADJ Q  | LENGTH  | FL 1   | FL 2    | CTL/TW | D    | W    | S | KJ  | KE  | KM   | LC | L1 | L3 | L4 | A1 | A3  | A4 | J     | N     |
|-----------------|----|----|--------|--------|---------|--------|---------|--------|------|------|---|-----|-----|------|----|----|----|----|----|-----|----|-------|-------|
| 8               | 1  |    |        |        |         |        |         | 629.50 |      |      |   |     |     |      |    |    |    |    |    |     |    |       |       |
| (8+55)          | 2  | 2  | 1770.0 | 1770.0 | 4135.00 | 616.76 | 631.00  | 0.0    | 144. | 135. | 3 | 0.0 | 0.0 | 0.30 | 1  | 0  | 0  | 0  | 0. | 0.  | 0. | 10.00 | 0.013 |
| (49+79) (50+00) | 2  | 3  | 1770.0 | 1770.0 | 1200.00 | 632.50 | 645.86  | 0.0    | 126. | 106. | 3 | 0.0 | 0.0 | 0.0  | 0  | 0  | 0  | 0  | 0. | 0.  | 0. | 0.0   | 0.013 |
| (62+00)         | 2  | 4  | 1770.0 | 1770.0 | 1240.00 | 645.86 | 661.00  | 0.0    | 126. | 106. | 3 | 0.0 | 0.0 | 0.0  | 0  | 0  | 18 | 0  | 0. | 30. | 0. | 15.00 | 0.013 |
| (74+49) (74+55) | 2  | 5  | 1550.0 | 1550.0 | 495.00  | 661.67 | 670.00  | 0.0    | 108. | 90.  | 3 | 0.0 | 0.0 | 0.0  | 0  | 0  | 0  | 0  | 0. | 0.  | 0. | 10.00 | 0.013 |
| (74+55) (74+60) | 2  | 6  | 1550.0 | 1550.0 | 150.00  | 672.00 | 673.75  | 0.0    | 114. | 96.  | 3 | 0.0 | 0.0 | 0.0  | 0  | 0  | 19 | 0  | 0. | 30. | 0. | 10.00 | 0.013 |
| (81+11) (81+20) | 2  | 7  | 1410.0 | 1410.0 | 250.00  | 674.75 | 678.36  | 0.0    | 108. | 90.  | 3 | 0.0 | 0.0 | 0.0  | 0  | 0  | 20 | 0  | 0. | 30. | 0. | 10.00 | 0.013 |
| (83+79) (83+60) | 2  | 8  | 1120.0 | 1120.0 | 92.76   | 678.50 | 684.30  | 0.0    | 90.  | 76.  | 3 | 0.0 | 0.0 | 0.0  | 0  | 0  | 0  | 0  | 0. | 0.  | 0. | 10.00 | 0.013 |
| (84+73) (84+50) | 2  | 9  | 1120.0 | 1120.0 | 237.00  | 684.93 | 689.16  | 0.0    | 90.  | 0.   | 3 | 0.0 | 0.0 | 0.05 | 0  | 0  | 21 | 0  | 0. | 30. | 0. | 15.00 | 0.013 |
| (124+87)        | 2  | 10 | 860.0  | 860.0  | 197.00  | 689.51 | 694.10  | 0.0    | 78.  | 0.   | 3 | 0.0 | 0.0 | 0.0  | 0  | 0  | 0  | 0  | 0. | 0.  | 0. | 4.00  | 0.013 |
|                 | 2  | 11 | 860.0  | 860.0  | 168.00  | 694.70 | 699.02  | 0.0    | 72.  | 0.   | 3 | 0.0 | 0.0 | 0.0  | 0  | 0  | 0  | 0  | 0. | 0.  | 0. | 4.66  | 0.013 |
|                 | 2  | 12 | 860.0  | 860.0  | 490.34  | 699.16 | 716.90  | 0.0    | 72.  | 0.   | 3 | 0.0 | 0.0 | 0.0  | 0  | 0  | 22 | 0  | 0. | 25. | 0. | 11.00 | 0.013 |
|                 | 2  | 13 | 780.0  | 780.0  | 300.00  | 717.76 | 734.33  | 0.0    | 69.  | 0.   | 3 | 0.0 | 0.0 | 0.0  | 0  | 0  | 0  | 0  | 0. | 0.  | 0. | 13.48 | 0.013 |
|                 | 2  | 14 | 780.0  | 780.0  | 125.54  | 735.07 | 743.23  | 0.0    | 69.  | 0.   | 3 | 0.0 | 0.0 | 0.13 | 0  | 0  | 0  | 0  | 0. | 0.  | 0. | 0.0   | 0.013 |
|                 | 2  | 15 | 780.0  | 780.0  | 188.89  | 743.23 | 755.20  | 0.0    | 69.  | 0.   | 3 | 0.0 | 0.0 | 0.0  | 0  | 0  | 0  | 0  | 0. | 0.  | 0. | 10.00 | 0.013 |
|                 | 2  | 16 | 780.0  | 780.0  | 350.00  | 755.83 | 778.00  | 782.36 | 78.  | 0.   | 3 | 0.0 | 0.0 | 0.09 | 0  | 0  | 0  | 0  | 0. | 0.  | 0. | 0.0   | 0.013 |
|                 | 2  | 17 | 780.0  | 780.0  | 9000.00 | 778.00 | 1097.71 | 0.0    | 78.  | 0.   | 1 | 0.0 | 0.0 | 0.0  | 0  | 0  | 0  | 0  | 0. | 0.  | 0. | 0.0   | 0.013 |
|                 | 2  | 18 | 220.0  | 220.0  | 1.00    | 662.75 | 662.76  | 0.0    | 54.  | 0.   | 1 | 0.0 | 0.0 | 0.0  | 5  | 0  | 0  | 0  | 0. | 0.  | 0. | 0.0   | 0.013 |
|                 | 2  | 19 | 140.0  | 140.0  | 1.00    | 674.75 | 674.77  | 0.0    | 42.  | 0.   | 1 | 0.0 | 0.0 | 0.0  | 7  | 0  | 0  | 0  | 0. | 0.  | 0. | 0.0   | 0.013 |
|                 | 2  | 20 | 290.0  | 290.0  | 1.00    | 678.50 | 678.51  | 0.0    | 60.  | 0.   | 1 | 0.0 | 0.0 | 0.0  | 8  | 0  | 0  | 0  | 0. | 0.  | 0. | 0.0   | 0.013 |
|                 | 2  | 21 | 260.0  | 260.0  | 1.00    | 692.30 | 692.45  | 0.0    | 36.  | 0.   | 1 | 0.0 | 0.0 | 0.0  | 10 | 0  | 0  | 0  | 0. | 0.  | 0. | 0.0   | 0.013 |
|                 | 2  | 22 | 80.0   | 80.0   | 1.00    | 723.67 | 723.79  | 0.0    | 27.  | 0.   | 1 | 0.0 | 0.0 | 0.0  | 13 | 0  | 0  | 0  | 0. | 0.  | 0. | 0.0   | 0.013 |