

Appendices

Appendix O General Plan Update Sewer Study

Appendices

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TECHNICAL MEMORANDUM

To: Heather Allen
From: Kim Alexander
Date: June 17, 2024
Subject: General Plan Update Draft Sewer Study

PROJECT SUMMARY

The proposed project involves a focused update to specific elements of the City of Anaheim General Plan, as well as targeted updates to the City's Zoning Code and Zoning Map. The project area encompasses the City of Anaheim, which is approximately 35 miles southeast of downtown Los Angeles and 7 miles north of Santa Ana. Anaheim is surrounded by the cities of Fullerton, Placentia, and Yorba Linda to the north; Riverside County to the east; the cities of Orange, Garden Grove, and Stanton and unincorporated Orange County to the south; and the cities of Cypress and Buena Park to the west. The city encompasses over 32,000 acres of land, stretching nearly 20 miles along State Route 91 (SR-91).

The proposed project involves a focused update of the City of Anaheim General Plan that reflects zoning and land use updates resulting from the 2021-2029 Housing Element to address the City's Regional Housing Needs Assessment (RHNA) growth allocation of 17,453 housing units. The proposed project includes land use, zoning, and policy changes in the Center City Corridors (C3) area, which the City originally intended on addressing in its own specific plan area (and EIR) but has since determined that they can be integrated into the General Plan Update. In addition to updates to the City's Zoning Code and Zoning Map to implement the 2021-2029 Housing Element, the City is updating the Circulation Element to reflect changes in transportation needs, new technologies, and future projects; and has prepared a new Environmental Justice Element, as required by state law (SB 1000).

The purpose of this memorandum is to document a sewer study prepared for the proposed project. The primary objective is to evaluate the capacity of the City's existing sewer collection system and any impacts due to project sewer flows. The analysis will identify potential capacity issues and develop recommended capital improvement projects that would mitigate those issues and provide additional downstream system capacity for the buildout of the General Plan and other approved tributary development. The study area includes the parcels that are changing General Plan Land Use, identified on Figure 1 (attached at the end of the Technical Memorandum). For this sewer analysis, the sewer collection system serving the study area parcels was separated by sewer basin, each tributary to a sewer outfall location discharging to the Orange County Sanitation District (OC San) trunk sewer system.

METHODOLOGY

The study area pipelines in this analysis are evaluated utilizing the East Anaheim and Central Anaheim sewer models developed as part of City sewer master planning efforts. The Central Anaheim model was developed for the Central Anaheim Master Plan of Sanitary Sewers (CAMPSS), dated December 2017, and subsequently updated with new development and as additional system data became available. Updates have included the South Central Anaheim Sewer Study (SCASS) dated May 2020 and the DisneylandForward Sewer Study dated August 2023. The East Anaheim model was developed for the East Anaheim Master Plan of Sanitary Sewers (EAMPSS), dated March 2023. Both models are loaded for residential and non-residential uses, based on dwelling units (DUs) and parcel acreage, respectively. For site specific sewer study areas, non-residential parcels may alternately be loaded based on thousand square feet (ksf) of building area. The existing model scenarios represent sewer flow from existing land uses plus approved and soon to be constructed development. The buildout scenarios include existing sewer flow and flow from buildout land uses including approved specific plan areas and general growth and infill.

For this analysis, study area pipelines are those pipelines that serve parcels with proposed General Plan Land Use changes. The study area sewer collection systems in the Central Anaheim Area generally convey wastewater from the northeast to the southwest by gravity flow to OC San trunk sewers on Euclid Street, State College Boulevard, and Orangewood Avenue. The study area sewer collection systems in the East Anaheim Area generally convey wastewater from east to west to OC San trunk sewers on Miraloma Avenue, Orangethorpe Avenue, La Palma Avenue, and Riverdale Avenue. For this study, area-specific model output and graphics are provided by tributary basin that identify recommended system improvements to meet buildout peak flow capacity requirements. Primary trunk sewer facilities within each basin serve large tributary areas and are sized to serve basin buildout. There are also numerous 6-inch through 10-inch existing sewers within the study area that will require upsizing but specifically which sewers need upsizing will depend on where each future development project is located and to what existing sewer it is loaded. That information is not available at the General Plan level. A preliminary estimate of these smaller diameter sewer improvements has been provided in this analysis based on assumed project loading as described below. When specific development projects are proposed, a detailed, site-specific sewer study will be required, which will build on this more programmatic study.

The initial project buildout analysis consisted of development of average flow estimates for the proposed land uses in the project area and where those flows would logically be loaded to the existing sewer collection facilities. Proposed General Plan Land Use data was provided by the City in GIS and excel formats dated May 2023 and is summarized in Appendix A. Figure 1 illustrates the proposed land use designations.

The General Plan statistical data provided dwelling units and non-residential square footage by project parcel. The mix of non-residential uses (retail, restaurant, office, and hotel) for mixed-use designation flow projections was estimated based on the Center City Corridors (C3) plan area Market Study (2021) which provided a breakdown by square footage for the mixed-use development areas.

Average flow factors were generated in close consultation with City staff and used to calculate project related buildout sewer flows. Table 1 summarizes the flow factors in gallons per day (gpd) per ksf for non-residential; gpd per room for hotel; and gpd per dwelling unit (DU) for residential

land uses. The project parcels have a mix of existing and proposed dwelling units and non-residential square footage. As such, flow factors were applied on a per dwelling unit and per thousand square foot basis to calculate the increase in sewer flows due to project buildout.

Table 1 - Sewer Flow Factors

Land Use Designation	Flow Factor	Units
Non-Residential		
Office	100	gpd/ksf
Restaurant	800	gpd/ksf
Retail	195	gpd/ksf
Hotel	150	gpd/room
General Commercial	195	gpd/ksf
Industrial	195	gpd/ksf
Public/Institutional	100	gpd/ksf
Residential		
Mixed-Use Residential	200	gpd/du
Residential - Mid/Medium	200	gpd/du
Residential - Low Medium	225	gpd/du
Residential - Low	250	gpd/du

gpd = gallons per day; ksf = thousand square feet,

DU = dwelling unit

The assumed percentage breakdown for non-residential land uses from the project area Market Study (2021) was multiplied by the corresponding flow factors in Table 1 to calculate the weighted average non-residential flow for each of the mixed-use designations shown in Table 2. The prorated non-residential factor, in gpd/ksf, was then applied to the square footage provided in the project statistics for each mixed-use parcel. Similarly, the residential factor of 200 gpd/DU was applied to each parcel based on the number of DUs provided in the project statistics. The total number of potential units and potential non-residential square feet were loaded for each of the designated Mixed-Use parcels to simulate the worst-case flow (highest) for evaluating sewer system capacity.

Table 2 - Weighted Factors for Mixed-Use Designation

Proposed Development Areas	Non-Residential (gpd/ksf)
Mixed-Use Low-Medium	329
Mixed-Use Mid	338
Mixed-Use Medium	317
Mixed-Use High	357
Mixed-Use Urban Core	339
Mixed-Use Industrial	237

Sewer flows were calculated for each parcel using the project statistics and flow factors provided in Tables 1 and 2 and were then allocated to the appropriate manhole in the model for the buildout scenario. Non-project parcels maintained the buildout flows utilized in the sewer master plan models.

If flow from a particular parcel enters the system via a lateral between manholes its flow is loaded to the next upstream manhole. For project parcels, the model buildout flows were replaced with project buildout flows, as described above. This was done to avoid double counting development intensification.

The proposed study area flows were analyzed by tributary pipeline system. The project tributary basins within the Central Anaheim model are illustrated on Figure 2. The project tributary basins within the East Anaheim model are illustrated on Figure 3. Table 3 summarizes the existing and proposed average flow in million gallons per day (mgd) to the OC San outfall from each tributary system serving parcels that are changing General Plan Land Use.

Table 3 - Average Flow for Project Tributary Basins

Tributary System	Existing Flow (mgd)	Project Flow (mgd)	Difference (mgd)
Central Anaheim			
Romneya	1.101	1.620	0.519
La Palma	1.549	1.687	0.138
Ball	4.819	6.171	1.352
Katella	5.583	6.298	0.715
Orangewood	5.058	5.352	0.294
Howell	0.197	0.574	0.377
Santa Cruz	0.364	0.404	0.040
Durst	0.052	0.103	0.051
East Anaheim			
Miraloma	0.402	0.530	0.128
La Palma	0.505	0.585	0.080
Etchandy	0.009	0.053	0.044
Orangethorpe	0.108	0.153	0.045
Kraemer	0.821	1.290	0.469
Riverdale	1.284	1.333	0.049
Total	21.852	26.153	4.301

MODELED SEWER CAPACITY

This sewer study provides model output data and sewer capacity analysis for project buildout development flows and also includes sewer flows for existing and reasonably anticipated proposed development from the upstream tributary areas. The existing condition scenario for the Central Anaheim model has been updated in the model to reflect all approved sewer study flows. Included in the anticipated development is DisneylandForward with projected sewer flows included in the existing condition model based on the DisneylandForward Sewer Study (August 2023). The East Anaheim model, developed as part of the March 2023 Master Plan, includes recent and planned development including specific plan areas. Consistent with previous sewer modeling practices, the models generate peak flows using two diurnal curves, residential and non-residential. Likewise, projected sewer flow from the project mixed-use and other parcels were modeled using separate residential and non-residential loading along with corresponding diurnal curves.

The maximum depth-to-diameter (d/D) ratio for the sewer collection system was used to identify pipe segment capacity deficiencies. Current City criteria was used when determining the pipe diameter sizes and flagging pipe segments as deficient. For peak dry weather flow conditions, the maximum d/D ratio for identifying an existing pipeline as deficient is 0.67 for pipelines smaller than 12-inch and 0.75 for pipelines 12-inch and larger. This is referred to as the analysis criteria in this study. For recommended improvement pipelines, a maximum d/D of 0.50 is used for pipelines smaller than 12-inch and 0.60 for pipelines 12-inch and larger. This is referred to as the design criteria in this study.

To establish an updated baseline condition and determine additional available capacity, an existing scenario was modeled excluding additional development intensity proposed in the project. Figure 4 illustrates the existing scenario limiting capacity pipelines with d/D ratios above the analysis criteria shown in red. Corresponding model output for the existing scenario is included in Table 4 (attached at the end of the Technical Memorandum). For comparison, CAMPSS existing condition deficiencies are included in Appendix B and these segments are highlighted in green in Figure 4. None of the project tributary pipelines within the East Anaheim area were identified as deficient under existing conditions.

Limiting capacity pipelines identified for the buildout scenario with additional project flows are illustrated on Figures 5 and 6 with limiting capacity pipelines identified in red, having maximum d/D ratios above the pipeline analysis criteria. A complete list of pipelines exhibiting capacity deficiencies for the buildout scenario is shown in Table 4. For comparison, the existing condition model output for these same pipelines is also included in Table 4. Buildout deficiencies previously identified in the Central and East Anaheim Master Plans are included in Appendix B.

The modeled peak flow at each tributary basin outfall is summarized in Table 5 for both the existing and buildout scenarios.

Table 5 - Modeled Peak Flow at Tributary Outfall

Tributary System	Existing Flow (mgd)	Project Flow (mgd)	Difference (mgd)
Central Anaheim			
Romneya	2.083	3.222	1.139
La Palma	3.454	3.743	0.289
Ball	10.559	13.333	2.774
Katella	9.334	10.234	0.900
Orangewood	8.403	9.005	0.602
Howell	0.387	1.228	0.841
Santa Cruz	0.852	0.944	0.092
Durst	0.083	0.222	0.139
East Anaheim			
Miraloma	0.646	0.855	0.209
La Palma	0.820	1.009	0.189
Etchandy	0.016	0.100	0.084
Orangethorpe	0.200	0.284	0.084
Kraemer	1.395	2.200	0.805
Riverdale	2.137	2.214	0.077
Total (gpm)	40.368	48.593	8.224

Model output for major collection pipelines, 12-inch and larger, within each tributary system illustrated on Figures 2 and 3 (for the Central and Eastern areas, respectively) is provided in Appendix C for both existing and proposed project buildout conditions. The tables include peak flow in mgd, peak d/D ratio, analysis flow equal to the capacity of the pipe with a maximum analysis d/D ratio, analysis excess equal to the difference between the pipeline capacity and peak dry weather flow, additional DUs that would fit in the pipe without exceeding the allowable analysis d/D ratio, and the peak additional project flow into the system. The excess pipeline capacity was converted into gpd and divided by 200 gpd times a peak factor of 2.4 for residential use to calculate the additional amount of DUs that could possibly develop in each tributary pipeline before requiring a capital improvement.

The Capital Improvement Projects (CIP) for deficient pipelines are discussed below, with area-specific graphics and output. Numerous wastewater collection system improvement projects are recommended for implementation to provide sufficient capacity for future development within the study area. These improvement projects address the major trunk mains with smaller diameter improvements included in the CIP tables and figures for completeness. Improvement areas are labeled using letters A through J on Figures 5 and 6 with detailed locations for each presented in Figures A through J and discussed in the following sections. Model output for the improved condition is included in Table 6 for each area.

ROMNEYA SYSTEM ANALYSIS

The Romneya tributary system is located in the northern area of Central Anaheim (Figure 2) and consists of a recently constructed 24-inch trunk pipeline. Model results for the Romneya System major tributary pipelines (12-inch and larger) are included in Appendix C and show sufficient capacity within the existing sewer collection pipelines for the increased sewer flow generated by project development intensities. There is one reach of 8-inch diameter pipeline within the tributary basin along Harbor Boulevard and just north of Romneya (shown on Figure 5) that is modeled as deficient in the buildup condition. The modeled improvement diameter is 10-inch with model output data included in Table 6 (SW070102-SW070301-A), though located outside of the Figure A viewport.

LA PALMA SYSTEM ANALYSIS

The La Palma tributary system is also located in the northern area of Central Anaheim and includes parallel pipelines along La Palma Avenue. The La Palma South System major trunk lines consist of 15-, 18-, 21-, and 24-inch segments with model output provided in Appendix C. The hydraulic analysis identifies six deficient segments along La Palma Avenue under project buildup conditions. Two additional segments that are not identified as deficient along the same reach are included to be upsized to meet City design criteria in the improvement area and to avoid a smaller diameter pipeline downstream of an improved pipeline. This improvement area includes 2,926 LF of proposed 15-inch, 18-inch, and 21-inch pipeline. The improvement pipelines and model output are included in Figure A and Table 6.

There are two additional smaller diameter pipelines that are modeled as deficient under the project buildup condition and are included on Figure A and in Table 6. These include a 6-inch reach in an alleyway parallel to Topeka Street to be upsized to 10-inch pipeline and an 8-inch reach along North Street to be upsized to a 12-inch pipeline.

The La Palma North System consists of 10- and 12-inch major tributary pipelines. Model results included in Appendix C show sufficient capacity within the existing sewer collection pipelines for the increased sewer flow generated by the proposed development intensities under buildup demand conditions.

BALL SYSTEM ANALYSIS

The Ball System pipe network is spread across all areas within Central Anaheim as shown on Figure 2. Model output for major collection pipelines is provided in Appendix C. Significant improvements to the sewer system are necessary to address capacity deficiencies. The deficient pipe segments identified in the hydraulic model within the Ball System are discussed in more detail below.

On Lincoln Avenue there are six deficient segments along with three additional segments added to the improvement area to avoid a smaller diameter downstream from improved pipelines and to meet City design criteria along the entire segment. To be conservative, these pipelines, and associated costs, have been included with the recommended improvements in this analysis. The improved diameter segments are illustrated in Figure B. Project buildup and improved condition model output is included in Table 6 along with proposed pipeline diameters. The improvements

along Lincoln Avenue include 8 reaches of existing 10-inch pipelines to upsize to a 12-inch and one reach of 15-inch pipeline at Lemon Street to upsize to 18-inch to eliminate any deficiencies.

Figure B and Table 6 include two additional large pipeline reaches that are deficient for project buildout and identified for improvement. The first is an 18-inch pipe on Lemon Street to be upsized to 24-inch and the second is a 12-inch pipe on Broadway to be upsized to 15-inch. There are additional smaller diameter pipelines tributary to Lincoln Avenue and near the intersection of Anaheim and Broadway identified as deficient for project buildout conditions as shown on Figure B and included in Table 6.

On Water Street there are 11 continuous deficient segments of 15-inch pipe as illustrated on Figure C. Project buildout and improved condition model output is included in Table 6 along with proposed pipeline diameters. This is considered the worst-case scenario to upsize the 15-inch pipe to an 18-inch and is the assumed required buildout improvement in this analysis. An alternative solution is to adjust the estimated 50/50 pipe flow split in manhole SW084328 at Water Street and Lemon Street to allow more flow down Lemon Street to the south. The feasibility of this alternative may be evaluated as a potential cost saving option in the future prior to pipeline upsizing. All the flow in this system eventually combines before crossing the I-5 Freeway.

Along Ball Road, from the I-5 Freeway and Walnut, there is a modeled deficiency along the 33-inch pipeline segment. Figure D illustrates the deficient segments on Ball Road with model output provided in Table 6. The recommended upsizing consists of 39-inch pipelines. Downstream of this section, the parallel pipelines on Ball Road join on Walnut Street and discharge to the OC San trunk sewer system in Euclid Street.

There are two additional segments of 18-inch pipeline that are modeled as deficient along the northern parallel pipeline in Ball Road at Harbor Boulevard. The improved size of these pipelines is 21-inch. There are alternative improvements identified in the South Central Anaheim Sewer Study that involve diverting flow from the northern to the southern parallel pipeline along Ball Road to avoid upsizing these segments within this major intersection. Upsizing these segments is considered the worst-case scenario and has been included as an improvement in this analysis.

Two 8-inch pipeline reaches along Palm Street were also modeled as deficient as shown in Figure D and Table 6. These segments feed to a siphon prior to connecting to the 24-inch southern pipeline in Ball Road.

KATELLA SYSTEM ANALYSIS

The Katella System is located in the south area of Central Anaheim as shown on Figure 2. Appendix B lists the model output for both existing and buildout conditions along the major collection pipelines. The hydraulic model identified 60 deficient pipe segments within the Katella System.

Flow from a portion of the project development discharges westerly in Midway Drive and across the I-5 Freeway to Manchester Avenue then southerly in the two parallel pipelines in Harbor Boulevard. There are deficient segments within the system tributary to Harbor as shown on Figure E with existing and recommended pipe sizes indicated. Model output, including project buildout and improved scenarios, is provided in Table 6. Flow from Manchester is split into two parallel pipelines along Harbor Boulevard. The two pipelines along Harbor are also interconnected at

Disney Way. The buildout improvement recommends upsizing the eastern parallel pipeline in Harbor Boulevard to provide sufficient combined capacity for buildout flows. There are also two reaches of 10-inch pipeline along Anaheim Boulevard, just north of the I-5 Freeway, that model as deficient as shown on Figure E and in Table 6.

A recently constructed flow diversion is in place at Harbor Boulevard and Katella Avenue that sends all of the flow from the eastern Harbor pipeline to the southern Katella pipeline. Conversely, all of the flow in the western Harbor pipeline discharges to the northern Katella pipeline. With this Harbor flow diversion and the additional project buildout flows, it is currently recommended to upsize the southern Katella pipeline between Clementine Street and 9th Street as shown on Figure F with model output for project buildout and the improved conditions provided in Table 6. Additionally, there are two deficient reaches in the northern Katella pipeline between Walnut Street and 9th Street. As discussed previously, flows from DisneylandForward are included in the existing and proposed project buildout scenarios. There are two flow splits that divert a portion of the flow to the south from the southern Katella pipeline, one at West Street and another at 9th Street. In addition, the two parallel pipelines along Katella have several interconnections. A detailed study should be conducted for the parallel pipeline systems along Harbor Boulevard and Katella Avenue to evaluate the combined capacity and potentially revise the design standards to consider the added flexibility and redundancy provided by these parallel systems.

There are additional, smaller diameter pipeline improvements shown on Figures G and H within the Katella tributary system and included in Table 6 along with improved diameters.

EAST ANAHEIM SYSTEM ANALYSIS

The East Anaheim area has been separated into six project tributary basins as illustrated on Figure 3. There are two areas with project related capacity deficiencies in the Eastern Anaheim area, both along East La Palma Avenue, shown on Figure 6. The first area consists of 6 reaches of 8- and 10-inch pipelines within the Kraemer System as illustrated on Figure I. Model output for the project buildout and improved condition is provided in Table 6. The second deficient area consists of 6 reaches of 10-inch pipeline within the La Palma System as illustrated on Figure J. Model output for project buildout and the improved condition is provided in Table 6.

COST ESTIMATE METHODOLOGY

The Engineer's opinion of the probable construction cost is shown in Table 6 for each of the capital improvement projects, based on the April 2024 Los Angeles Engineering News-Record (ENR) Construction Cost Index of 15245. The pipe unit costs from the CAMPSS were used and escalated to April 2024 dollars based on the ratio of the two indices (CAMPSS March 2016 ENR of 11158). The estimated total cost includes the construction costs plus 45% for Professional Services and Contingencies (15% for design and technical services; 10% for administration, construction management, inspection, and project administration; and 20% for contingencies).

Results of the collection system needs for buildout flow conditions are summarized in Table 6, which includes a summary of deficient pipe lengths and cost of improvements. As shown in Table 6, the cost of improvements totals \$85,653,000 for buildout sewer needs. The cost for required improvements without the project is \$21,671,000, resulting in a project related cost of approximately \$63,982,000.

CONCLUSIONS

The total increase (proposed minus existing) in average daily sewer flow from the project is projected to equal 4.4 million gallons per day (mgd). The anticipated CIP costs to accommodate these flows along the major tributary sewer pipeline reaches (existing 12-inch diameter and larger) and the smaller diameter pipelines (existing 10-inch and smaller) totals \$85,653,000. As new development occurs within the project area, individual sewer studies will be conducted to evaluate the specific impact on downstream tributary pipelines. When a larger CIP pipeline is identified as capacity deficient, it is recommended to upsize the pipeline to accommodate buildout flows as evaluated in this study. Pipeline improvements along smaller diameter reaches will also be determined by individual sewer studies and improved as needed and as a condition of the specific development project approval by the City. These improvements may require 6-, 8-, and 10-inch diameter pipelines to be increased by one or two diameters and will depend on the specific project land use and location. Modeled deficient pipelines are shown in red on Figures 5 and 6. Modeled pipeline improvements are tabulated in Table 6 along with recommended pipeline diameters. There are pipelines included in the recommended improvements that are not modeled as deficient in the buildout condition scenario using the analysis criteria but are upsized due to their location downstream of an improved segment to provide a consistent diameter and to meet City design criteria.

There are currently approved impact fee schedules for new development within the Central and East Anaheim service areas that fund sewer improvement projects. In addition, the City may require sewer improvements as a condition of a specific development project that has an impact on the downstream sewer system. The large diameter pipeline improvements (existing 12-inch and larger pipelines) are limited to the Central Anaheim area. The City is currently in the process of updating its Central Area Sewer Master Plan and impact fee schedules. These fees will consider buildout capacity needs and associated costs, including this project. Approximately 2,500 LF and \$3.3 million of project related improvements are located in the East Anaheim area. As this is a long-term project, with the buildout condition extending out 20 years or more, the East Anaheim area impact fees will also be updated within the planning horizon to ensure sufficient funds are available.

Table 7 provides a summary of the CIP pipeline projects identified in this analysis and the combined length of pipeline improvements based on assumed project loading. Major CIP pipelines are identified by location in Table 7 with smaller diameter improvements included as "Various Other." Cost estimates for each CIP include construction cost plus an additional 45% to account for engineering and other technical services, contingencies, contract administration, permitting, and construction management. When specific development projects are proposed, a detailed, site-specific sewer study will be required, which will build on this more programmatic study. The sewer impact fees, sewer study, and any additional project related sewer improvements not previously identified will be paid for by the developer prior to the approval of sewer service.

Table 7 - Pipeline CIP Summary

Map ID	Location	Pipeline Replacement	Pipe Length (LF)
A	LA PALMA AVE	15, 18, and 21-inch	2,926
	VARIOUS OTHER	10-inch	876
B	E LINCOLN AVE	12-inch	1,743
	W LINCOLN AVE	18-inch	334
	E BROADWAY	15-inch	316
	LEMON ST	24-inch	207
	VARIOUS OTHER	8, 10, and 12-inch	3,708
C	WATER ST	21-inch	2,520
	VERMONT AVE	21-inch	831
	LEMON ST	27-inch	365
	VARIOUS OTHER	8, 10, and 12-inch	3,692
D	W BALL RD	21 and 39-inch	3,788
	VARIOUS OTHER	10-inch	379
E	MIDWAY DR	15, 18, and 21-inch	1,200
	HARBOR BLVD	18-inch	1,662
	VARIOUS OTHER	12-inch	3,757
F	KATELLA SOUTH	27, 30, and 33-inch	6,903
	KATELLA NORTH	36-inch	1,318
G	VARIOUS OTHER	10, 12-inch	4,205
H	VARIOUS OTHER	12, and 15-inch	1,633
I	VARIOUS OTHER	10, 12, and 15-inch	1,599
J	VARIOUS OTHER	10, 12, and 15-inch	939
Total			44,901

Attachments:

Figures 1 through 6, Figures A through J, and Tables 4 and 6 (large format)

Appendix A: Proposed General Plan Land Use Statistics

Appendix B: Central and East Area Master Plans Identified Deficiencies

Appendix C: Project Existing and Buildout Condition Model Output

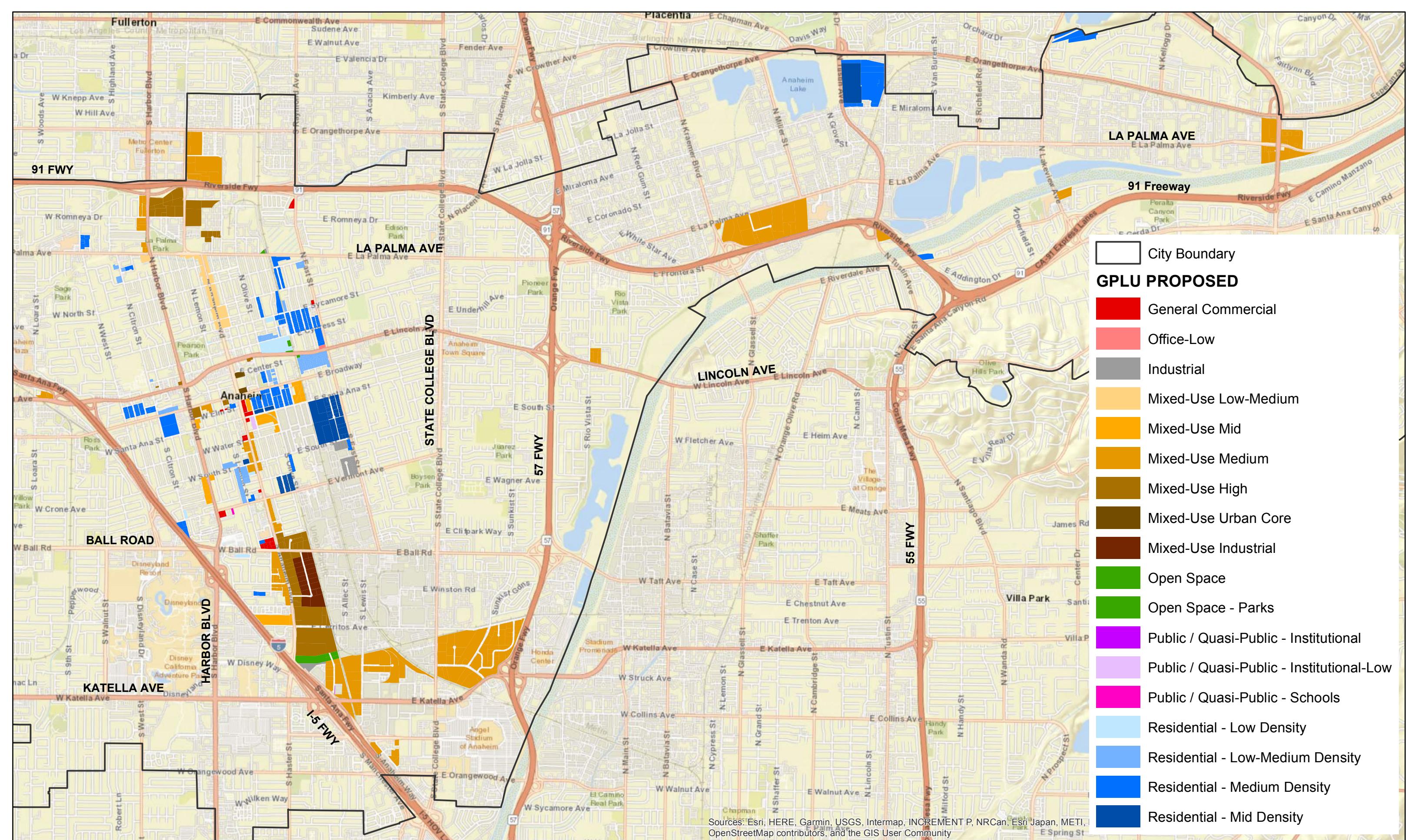
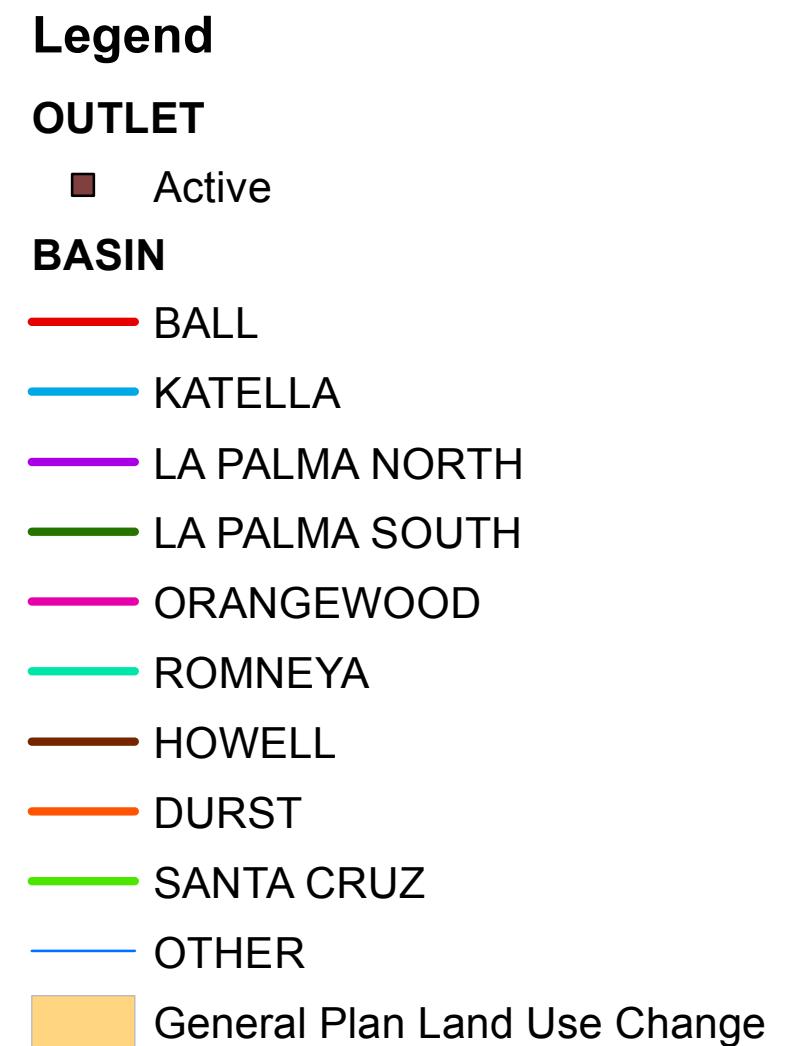
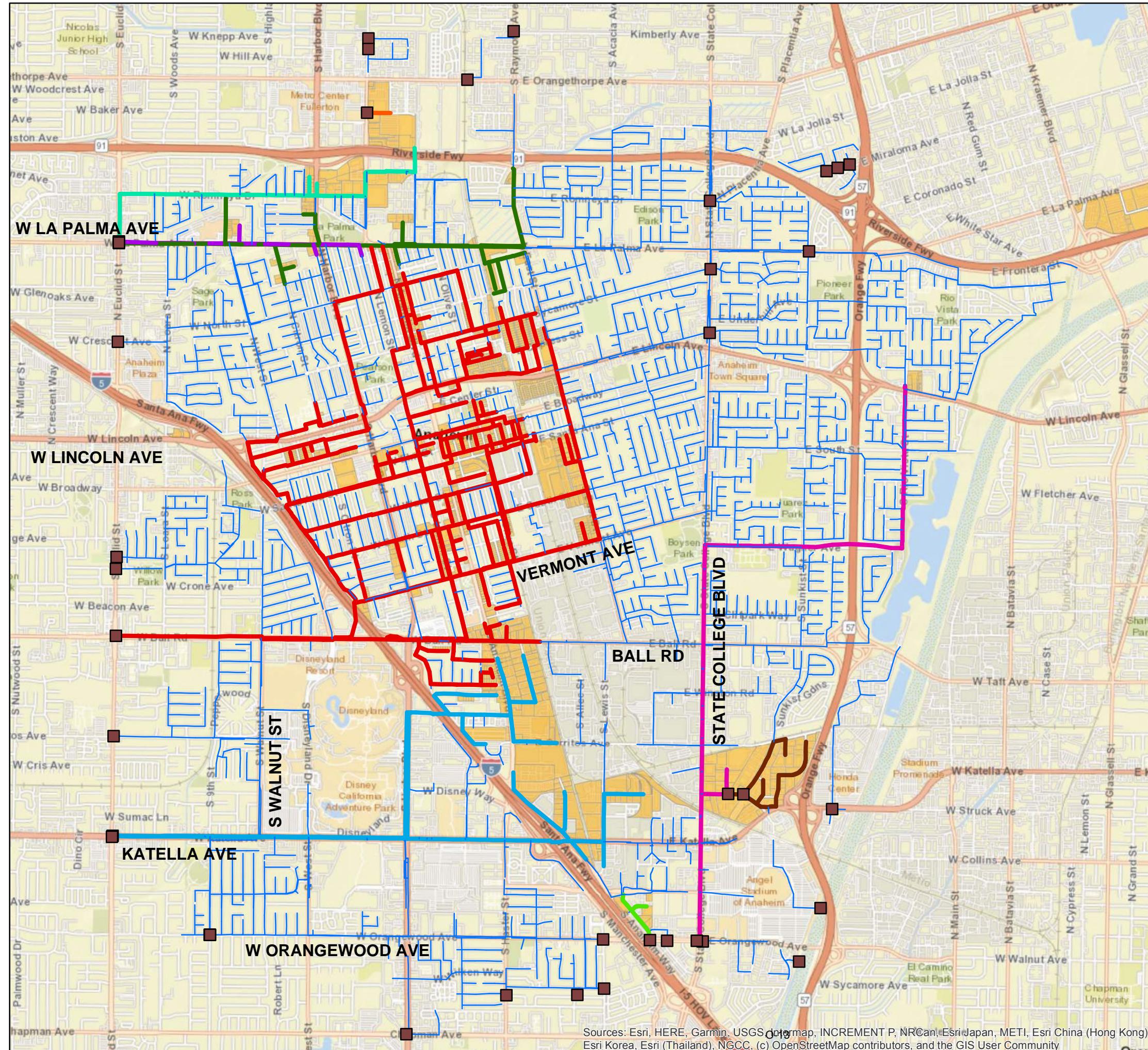


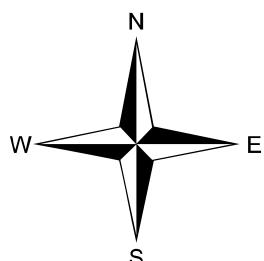
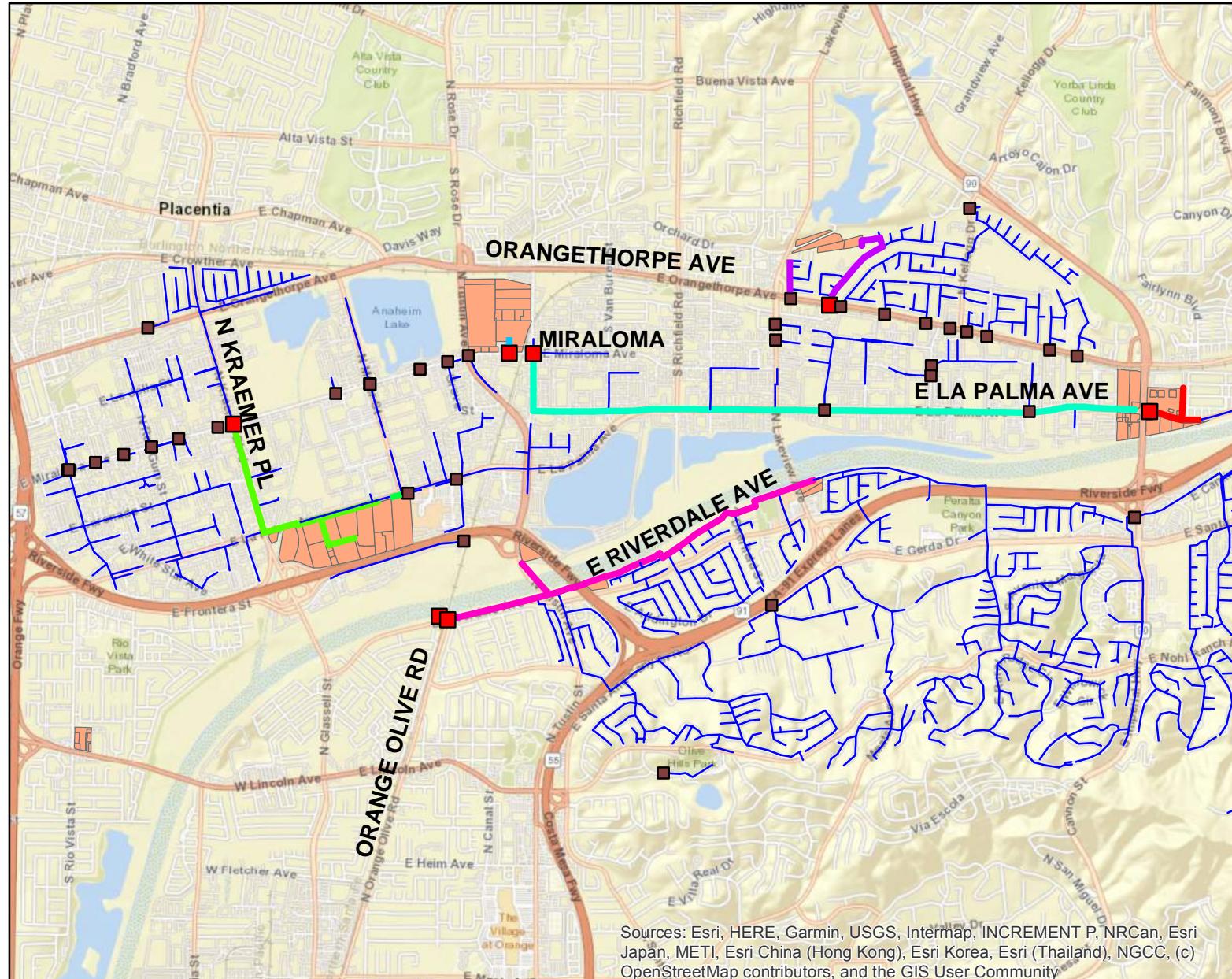
FIGURE 2
CENTRAL ANAHEIM PROJECT
TRIBUTARY PIPELINES



P S O M A S

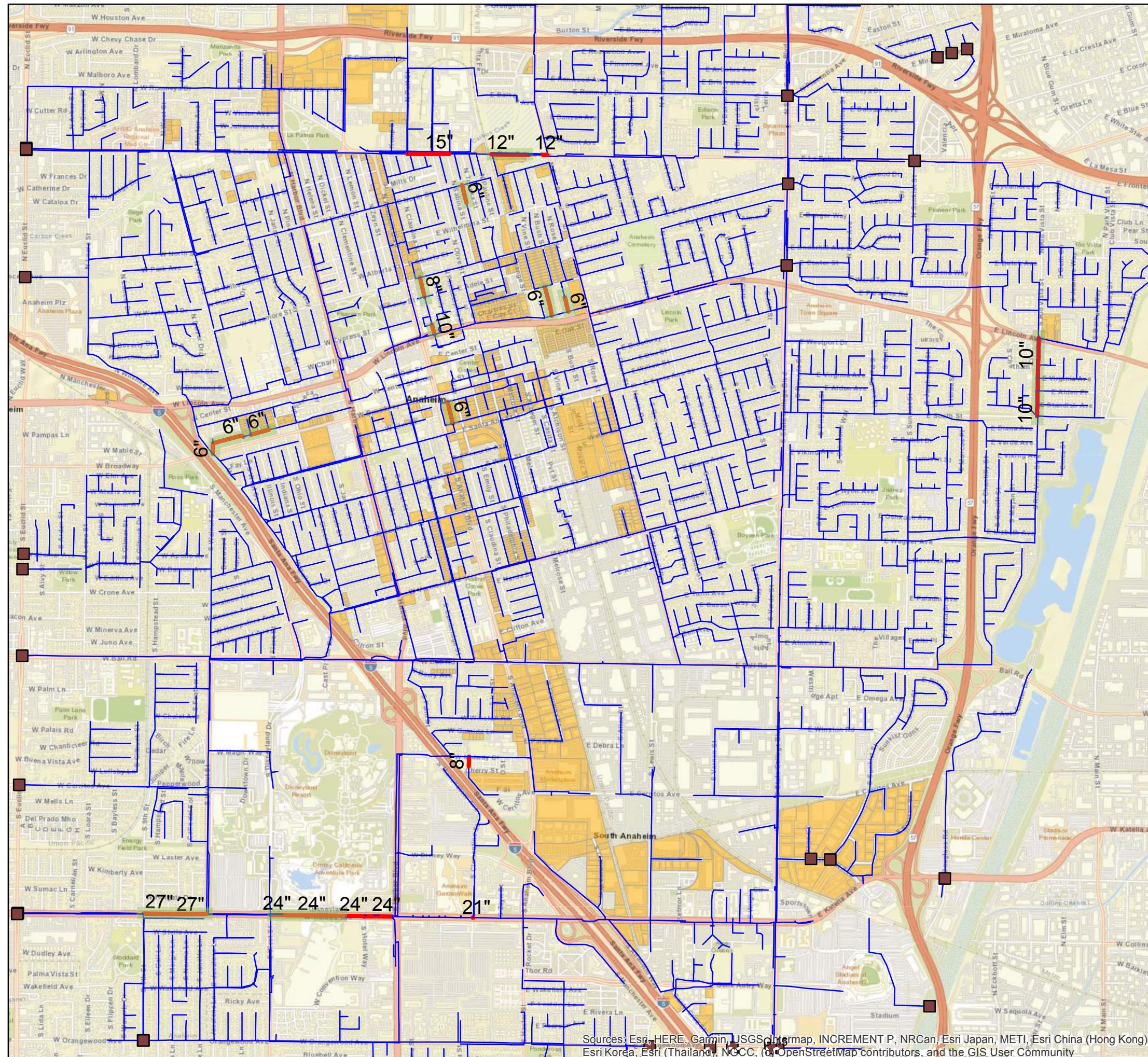
0 2,650 5,300 10,600
Feet

FIGURE 3
EAST ANAHEIM PROJECT
TRIBUTARY PIPELINES



P S O M A S

FIGURE 4
CENTRAL ANAHEIM
EXISTING DEFICIENT PIPELINES



Legend

OUTLET

■ Active

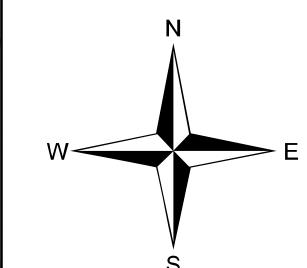
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— DEFICIENT

— OTHER

■ General Plan Land Use Change

*GREEN HIGHLIGHTED PIPELINES ARE ALSO DEFICIENT IN CAMPSS

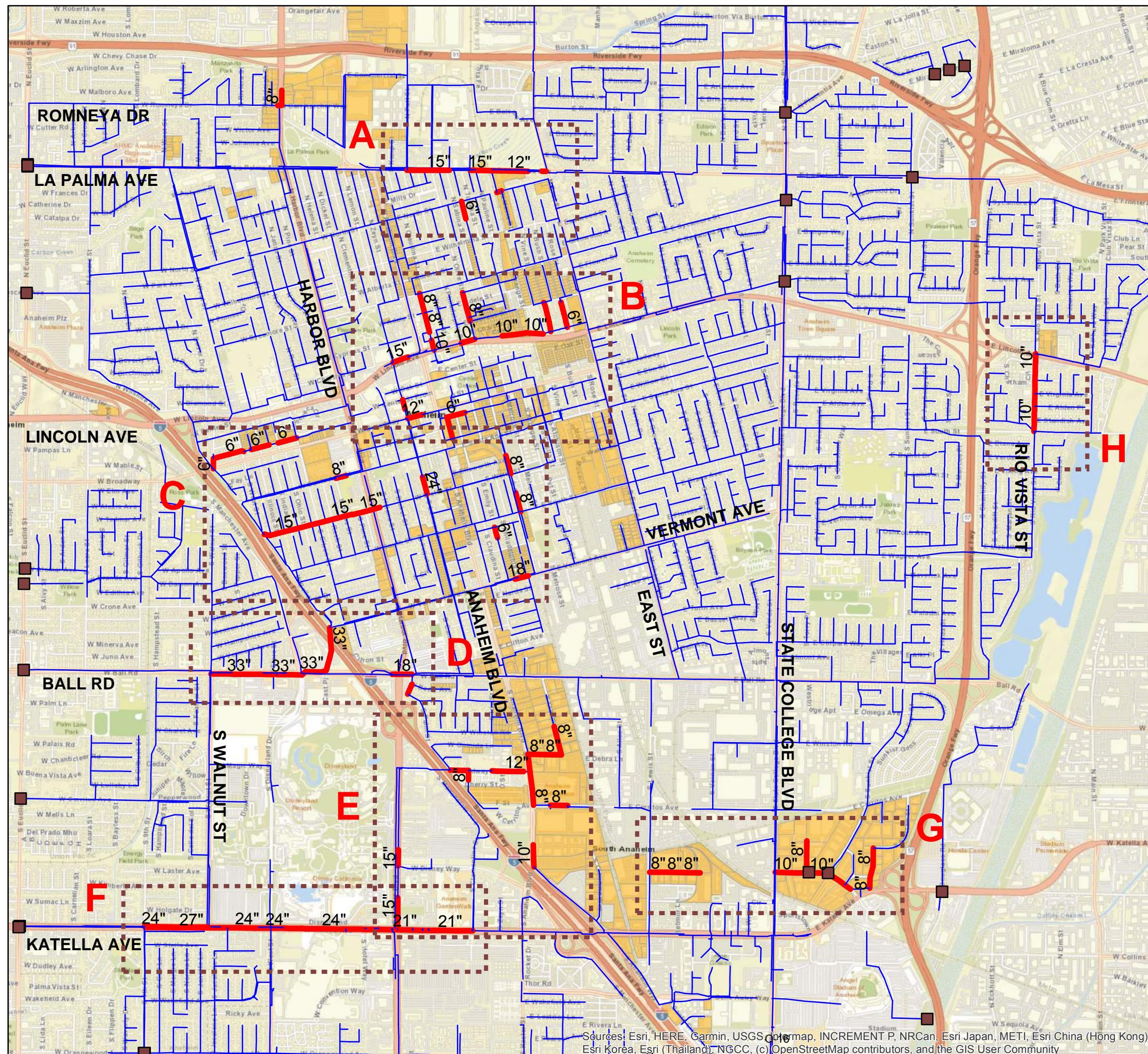


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Feet

FIGURE 5
CENTRAL ANAHEIM
PROJECT DEFICIENT PIPELINES



Legend

OUTLET

■ Active

TYPE

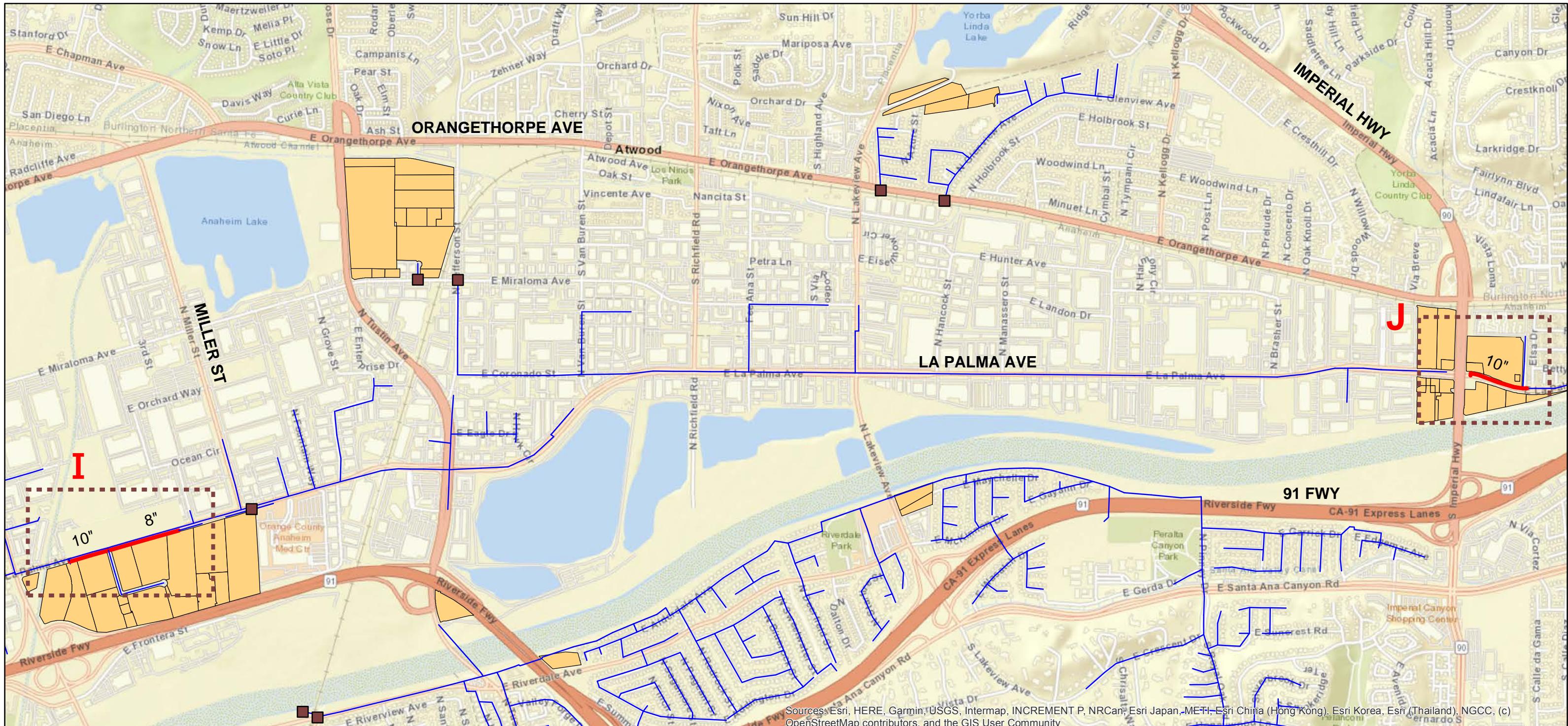
— DEFICIENT

— OTHER

General Plan Land Use Change

A Figure Designation for Details

P S O M A S



Legend

Outlet

■ Active

TYPE

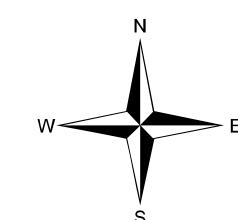
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— DEFICIENT

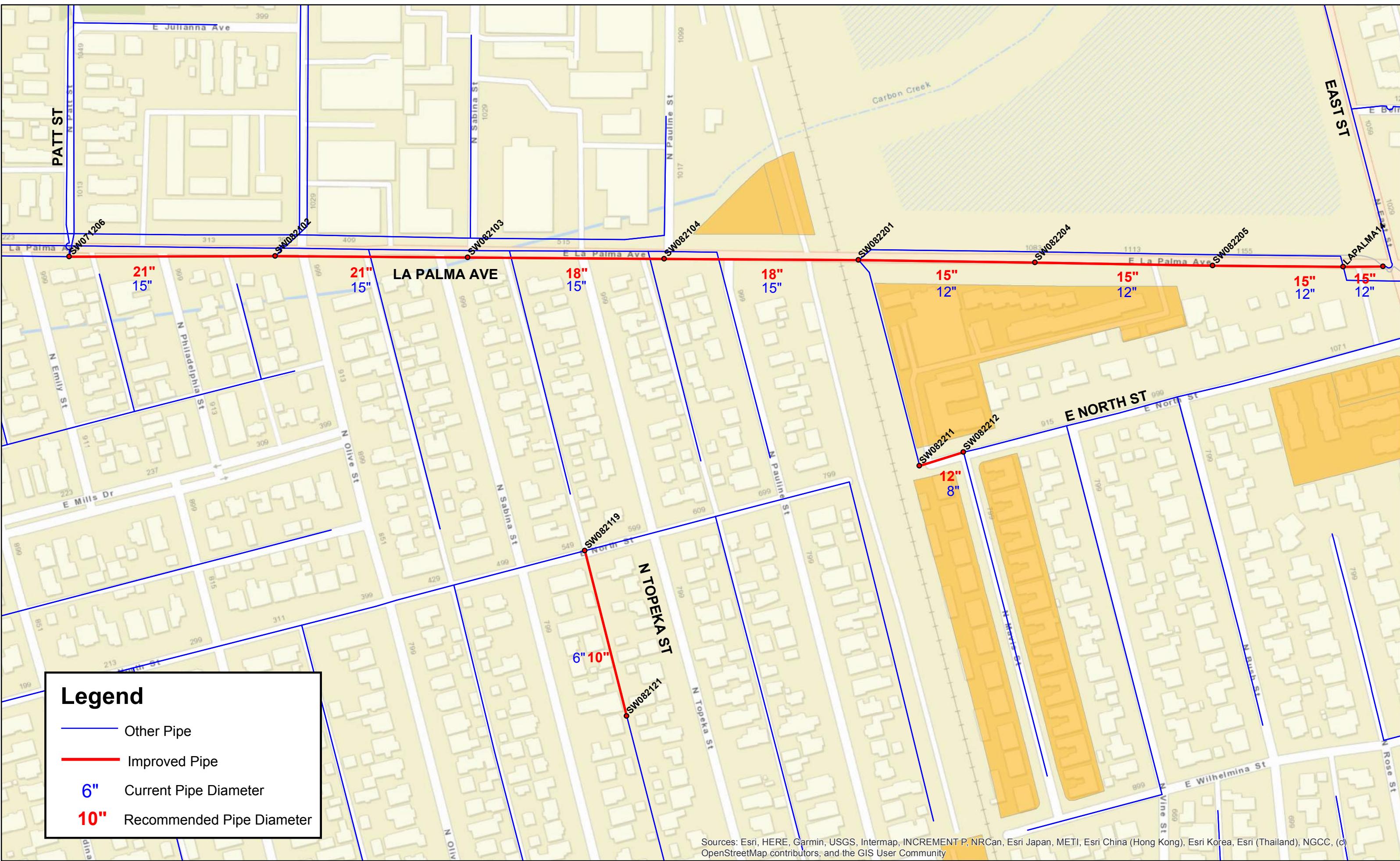
■ General Plan Land Use Change

J Figure Designation for Details

FIGURE 6
EAST ANAHEIM
PROJECT DEFICIENT PIPELINES

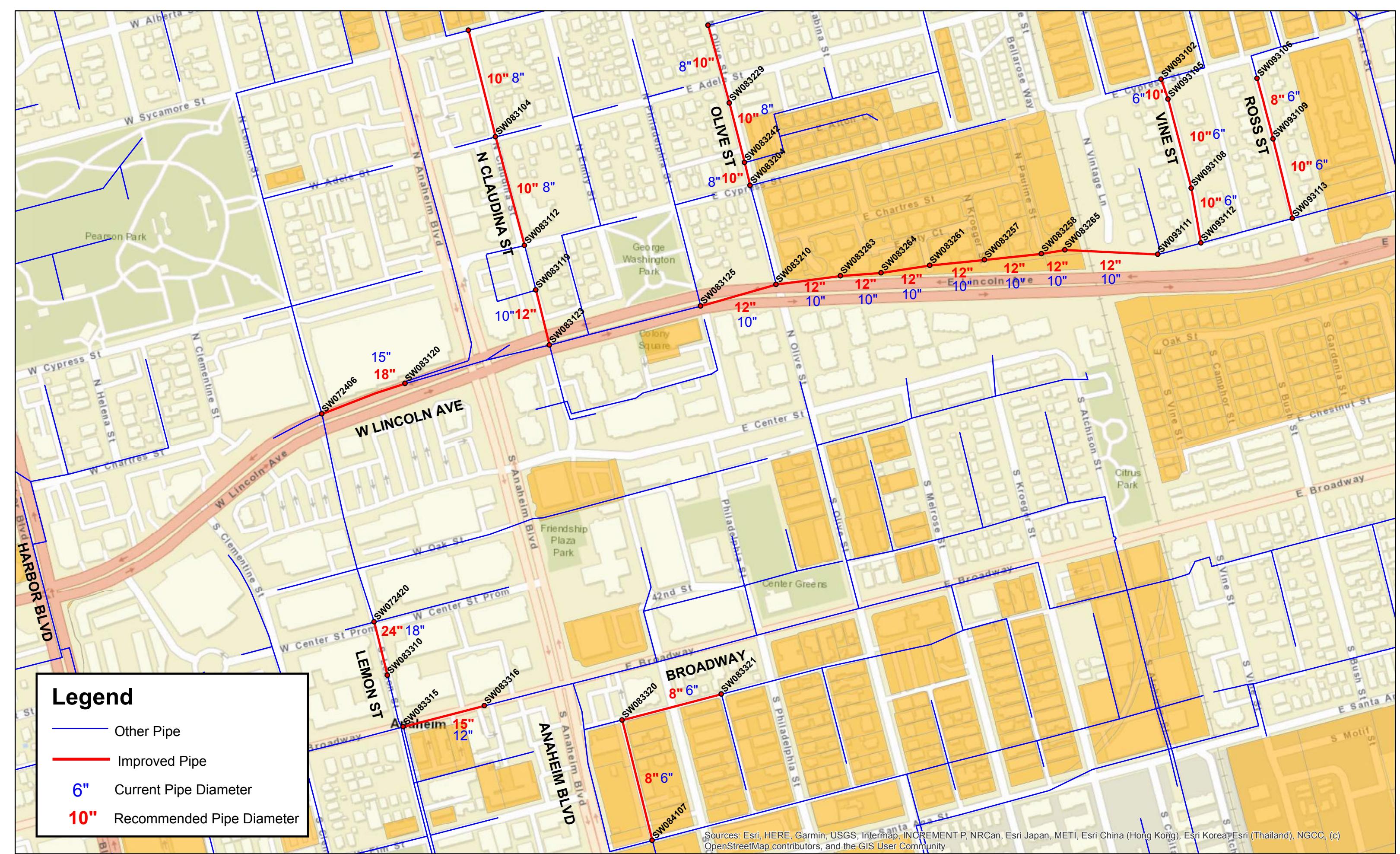


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Feet



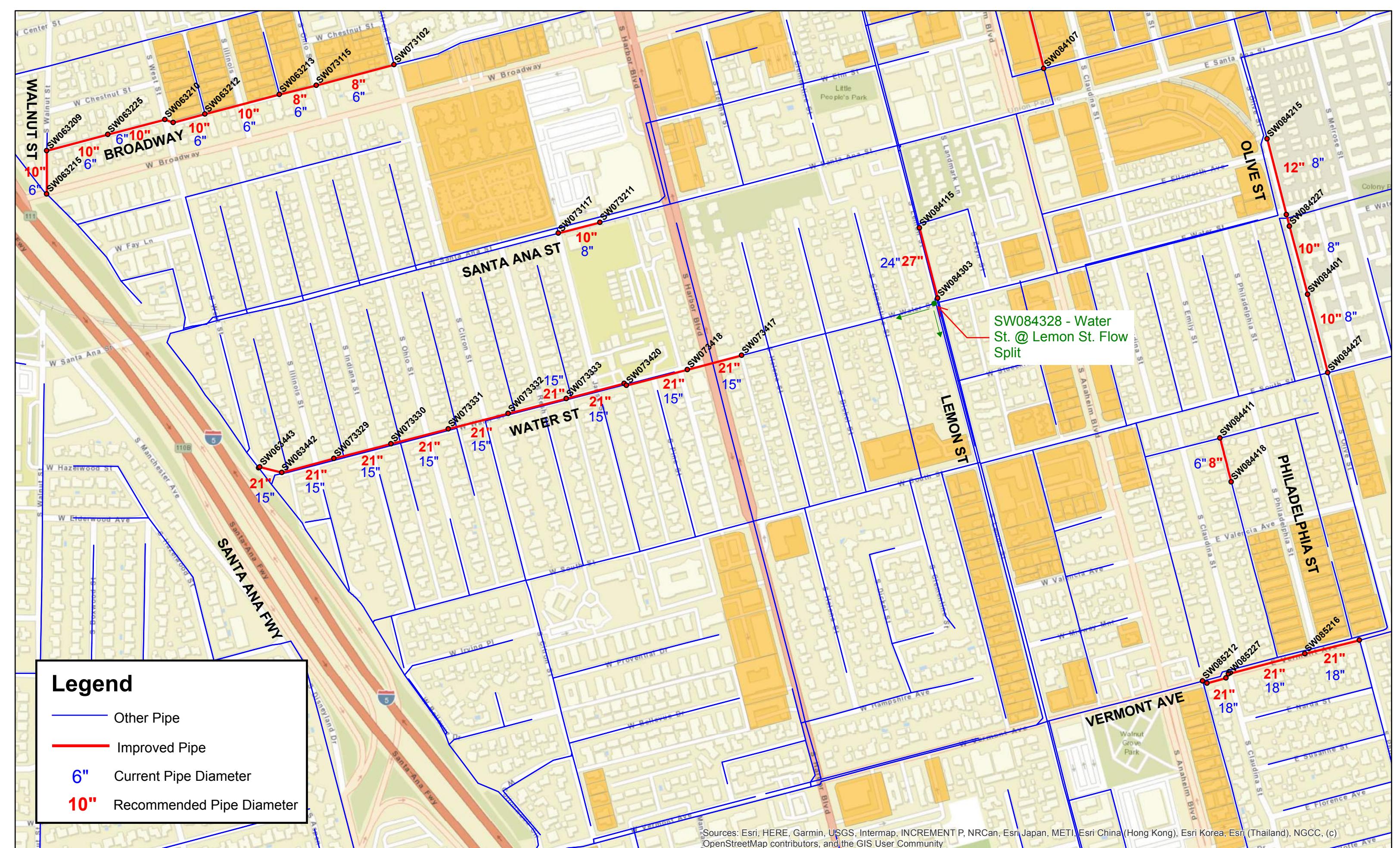
PSOMAS

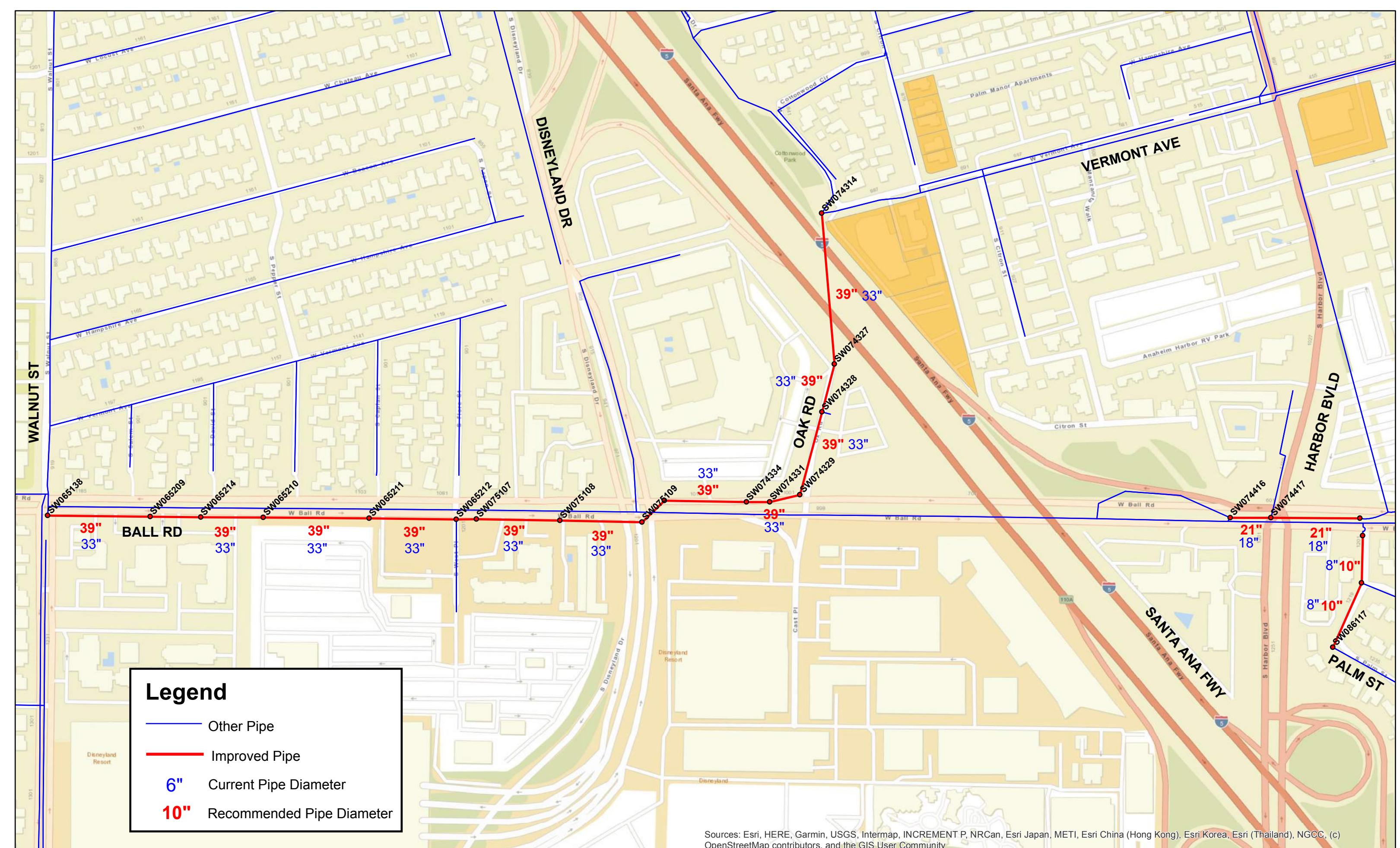
FIGURE A
LA PALMA BASIN CIP AREA A

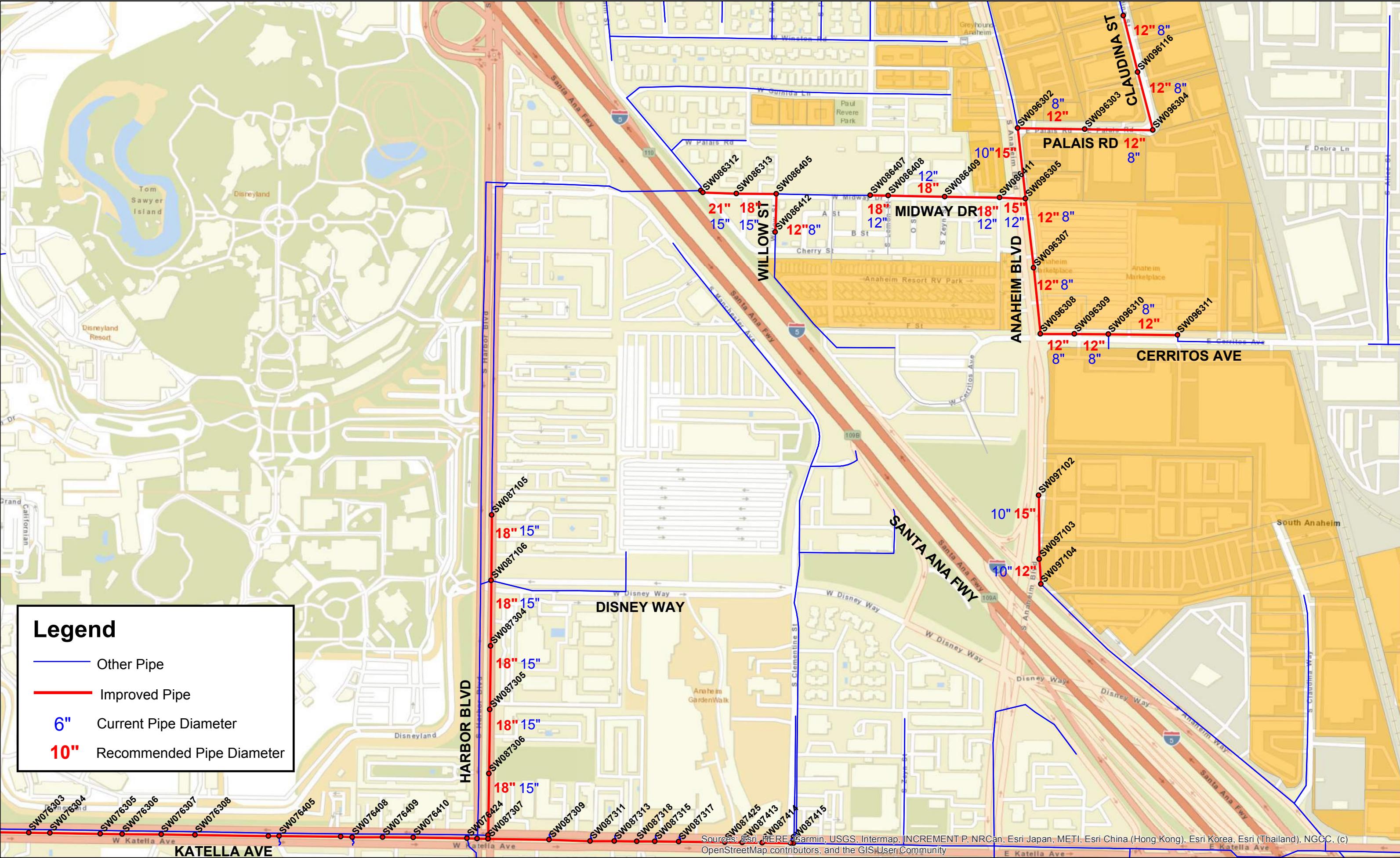


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FIGURE B
BALL ROAD BASIN CIP AREA B

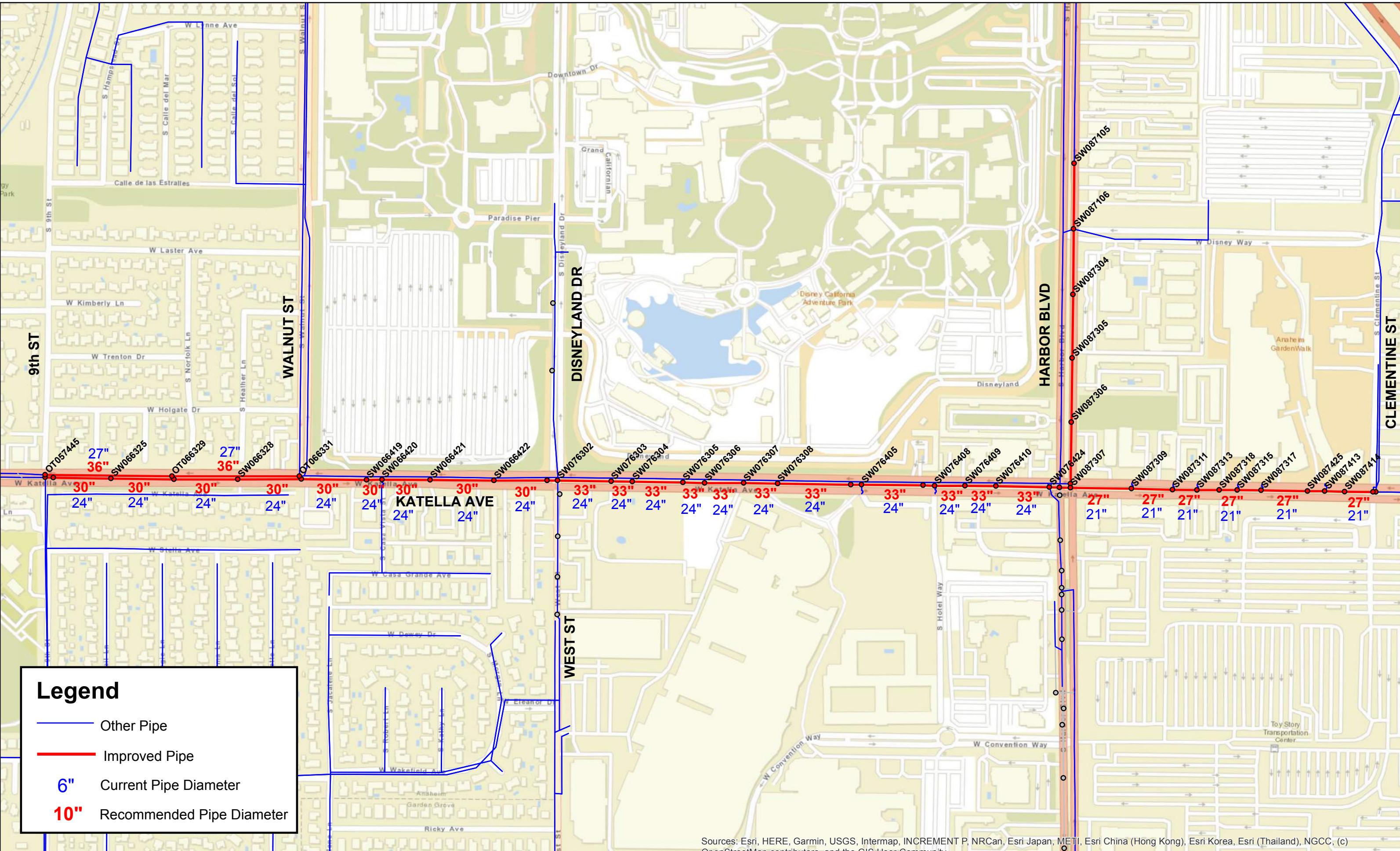


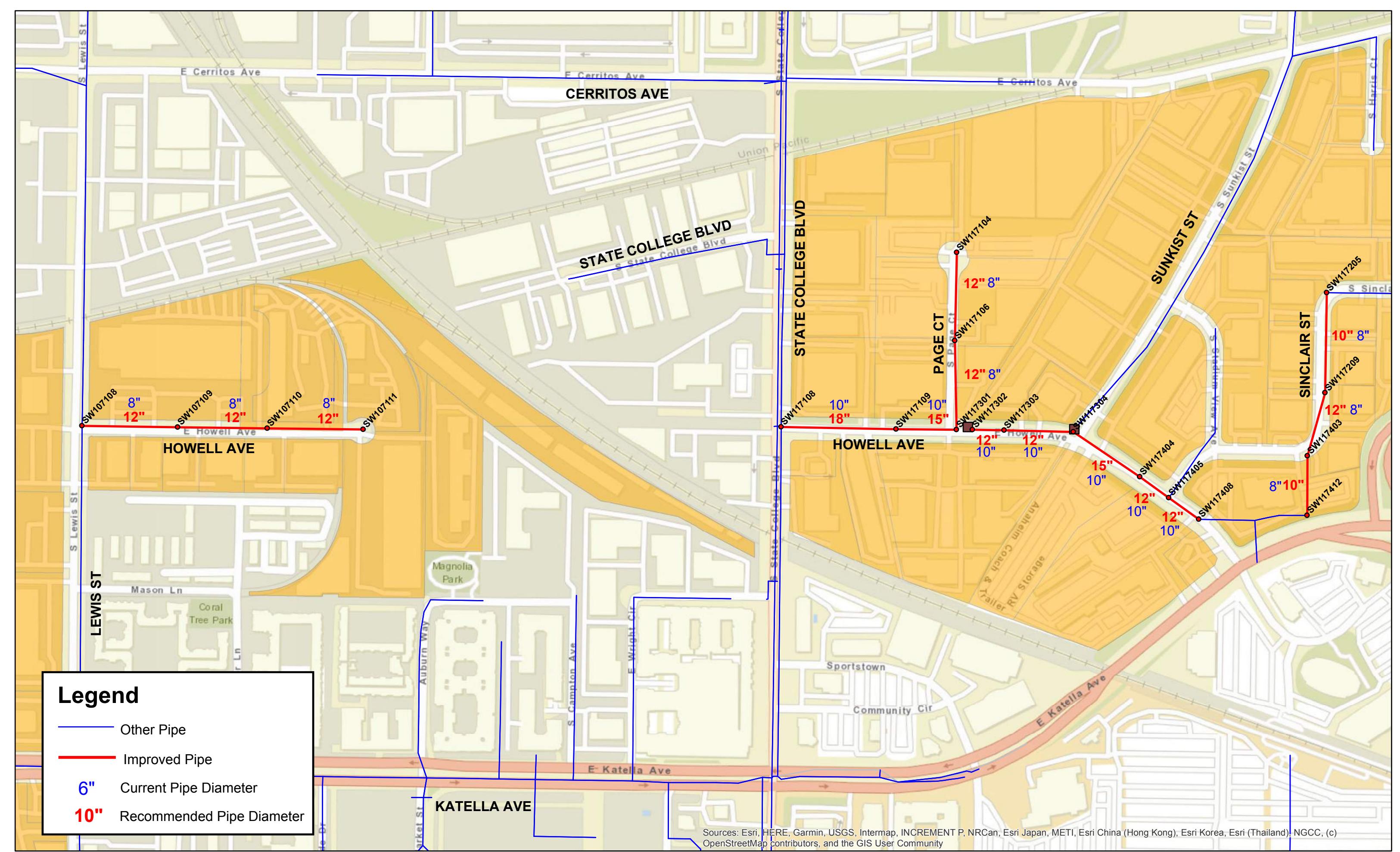


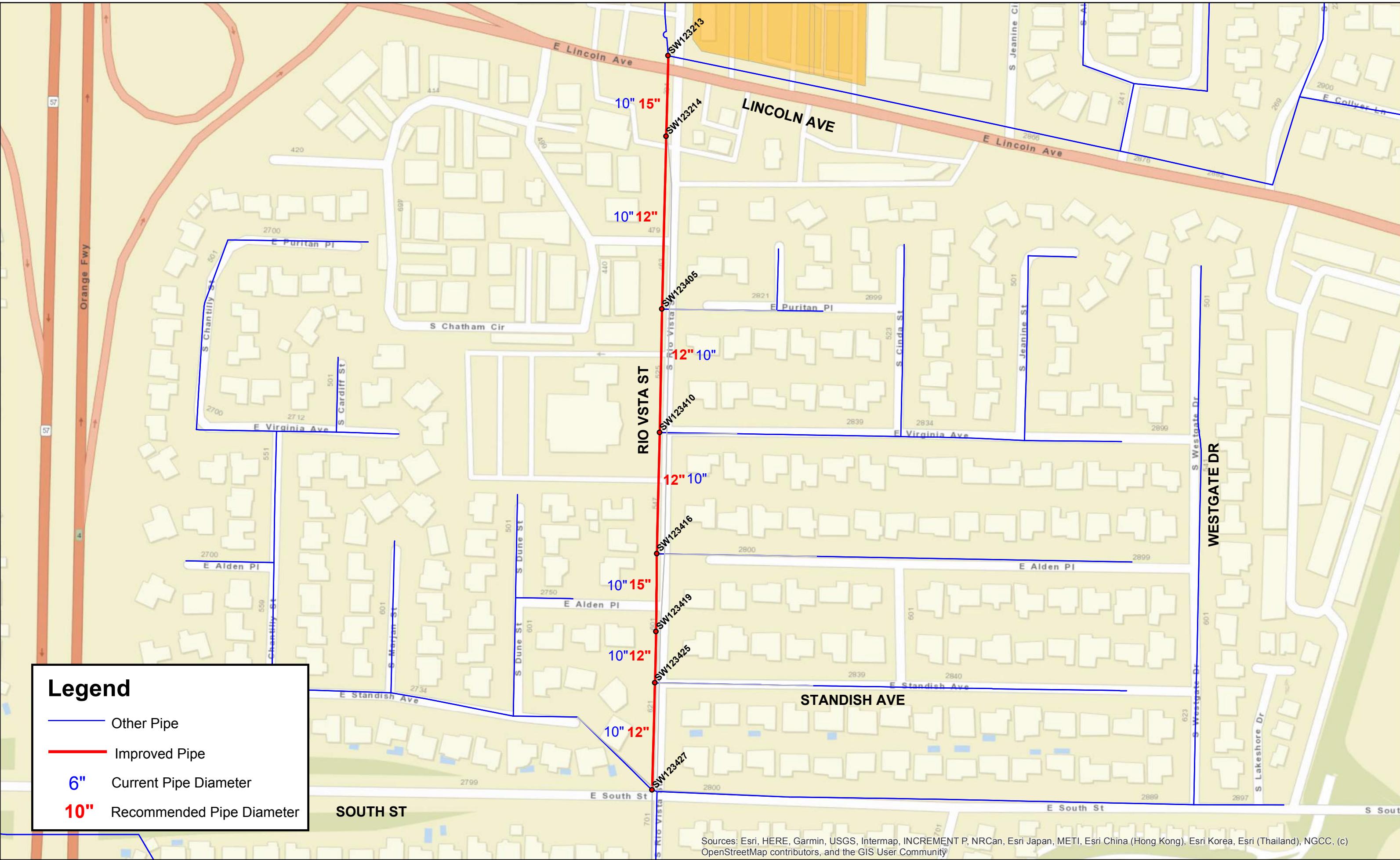


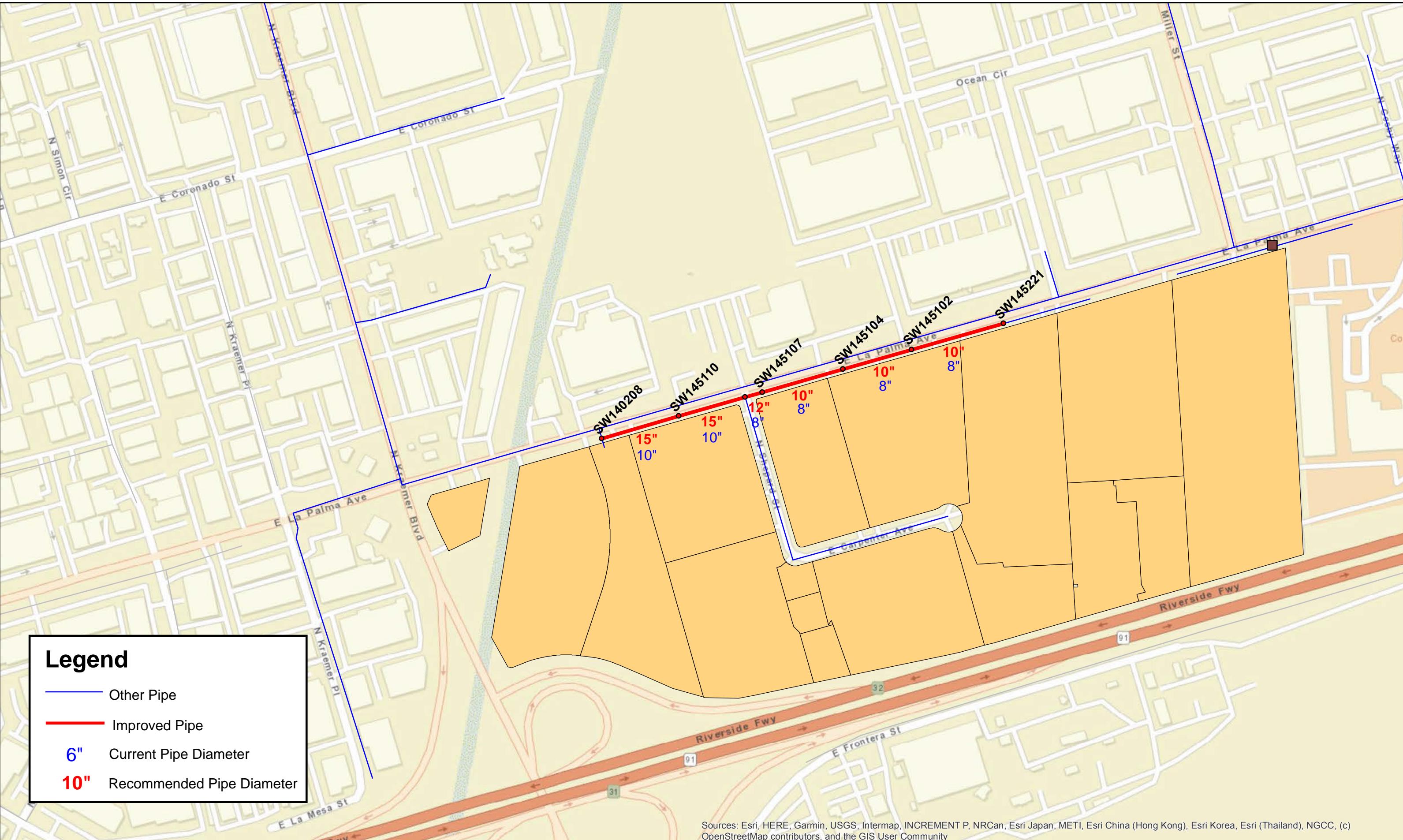
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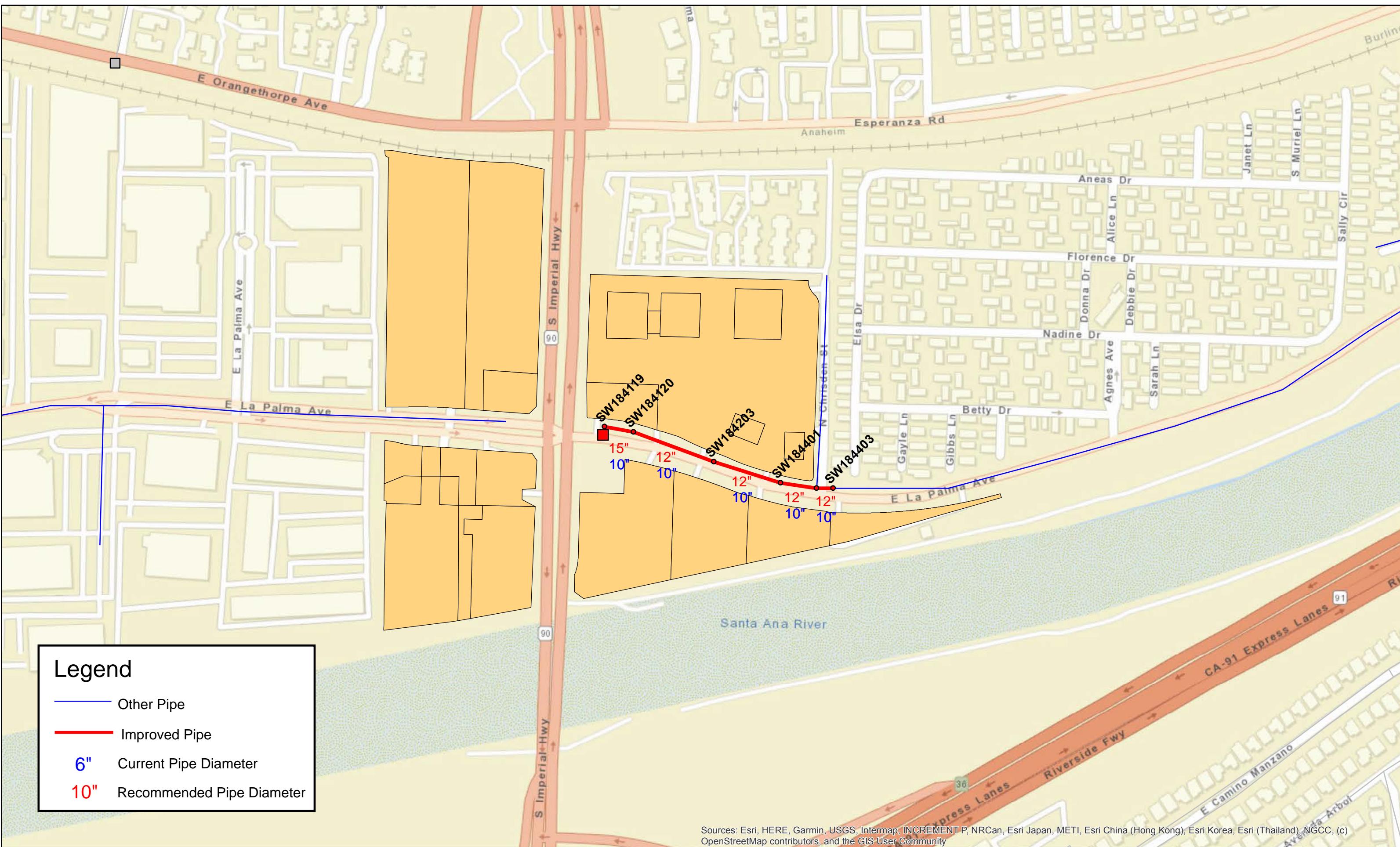
FIGURE E
KATELLA BASIN CIP AREA E











P S O M A S

FIGURE J
LA PALMA BASIN CIP AREA J

**APPENDIX A
PROPOSED GENERAL PLAN
STATISTICAL SUMMARY**

Summary of Proposed and Existing General Plan Land Use

Land Use	General Plan Dwelling Units (DUs)		General Plan Non-Residential Square Feet (sqft)	
	Proposed DUs	Existing DUs	Proposed sqft	Existing sqft
General Commercial	0	138	104,341	27,136
Industrial	0	0	157,565	29,875
Mixed-Use High	3,864	22	1,306,017	1,143,951
Mixed-Use Industrial	305	0	989,531	315,316
Mixed-Use Low-Medium	243	223	215,490	0
Mixed-Use Medium	9,007	496	3,782,489	4,249,979
Mixed-Use Mid	362	523	145,033	28,413
Mixed-Use Urban Core	103	0	126,879	18,404
No Change	10,857	6,141	4,258,917	4,262,511
Office-Low	0	0	8,504	6,378
Open Space	0	5	0	94,276
Open Space - Parks	0	8	0	0
Public / Quasi-Public - Institutional	0	0	20,124	17,250
Public / Quasi-Public - Institutional-Low	0	0	0	0
Public / Quasi-Public - Schools	0	9	0	0
Residential - Low Density	65	200	0	21,283
Residential - Low-Medium Density	525	133	555,587	558,167
Residential - Medium Density	3,151	766	467,925	878,000
Residential - Mid Density	1,620	776	222,875	476,574
Grand Total	30,102	9,440	12,361,277	12,127,513

**APPENDIX B
SEWER MASTER PLAN
IDENTIFIED DEFICIENCIES**

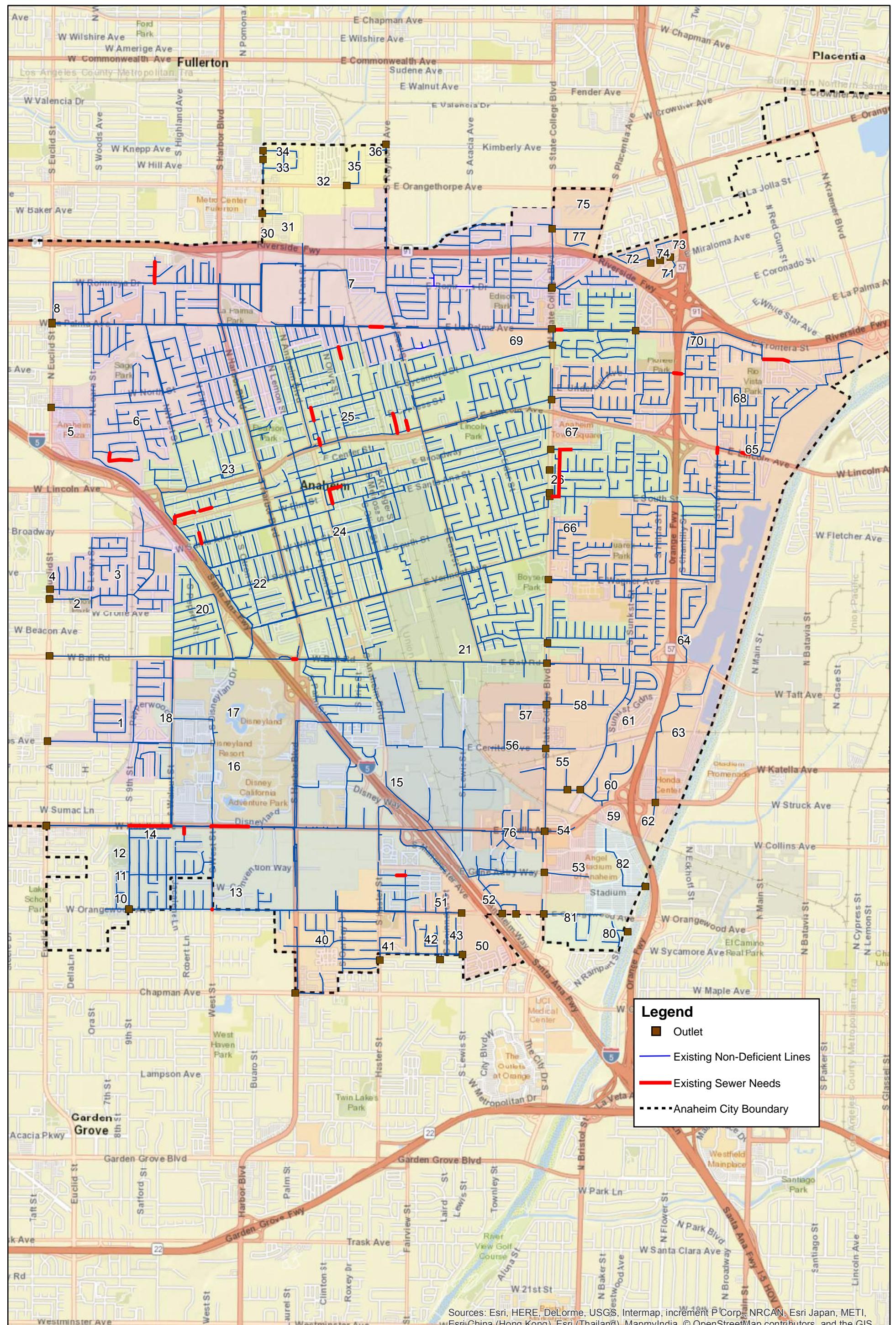
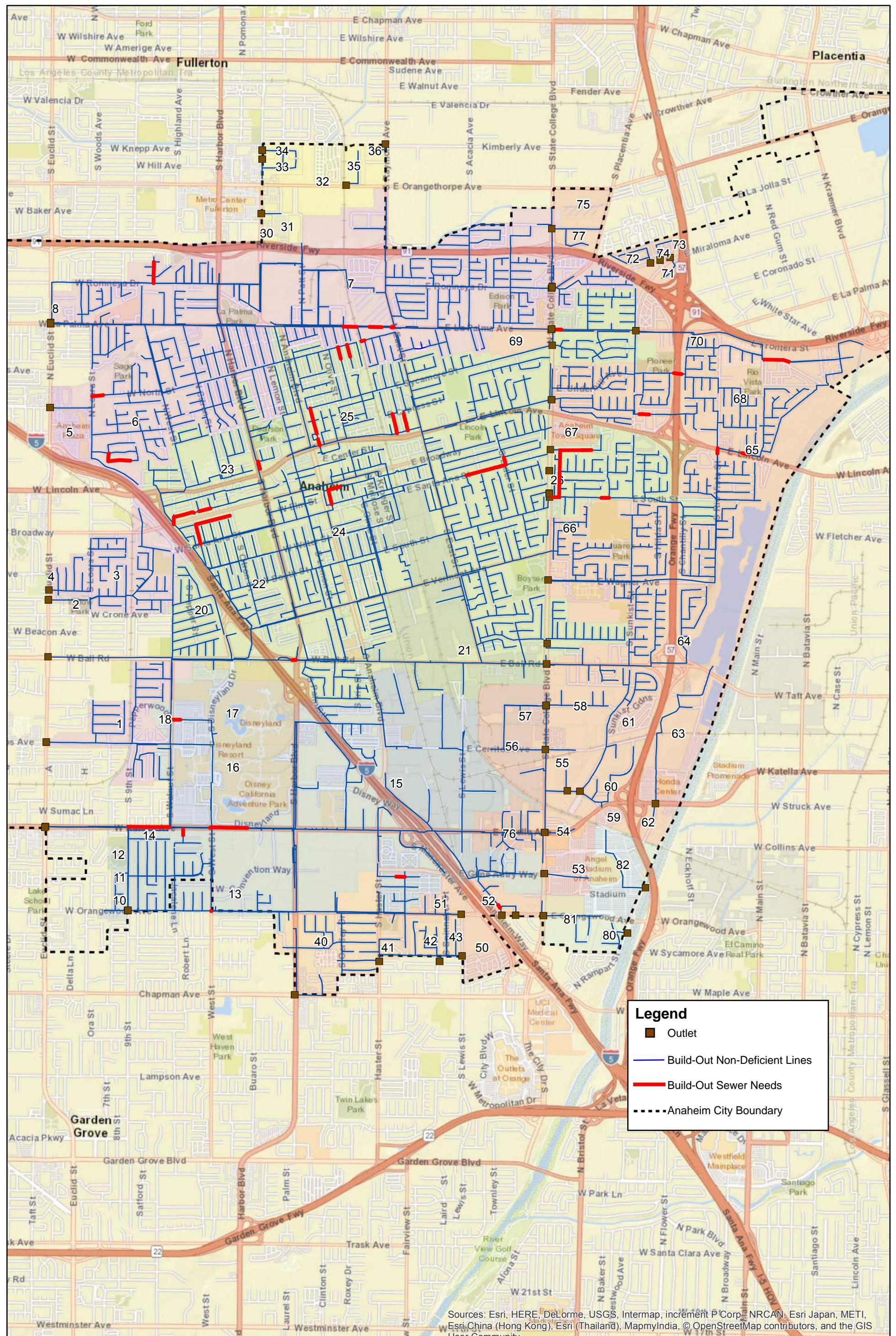


Figure 3.1
Existing Conditions

Central Anaheim Master Plan of Sanitary Sewers

PSOMAS
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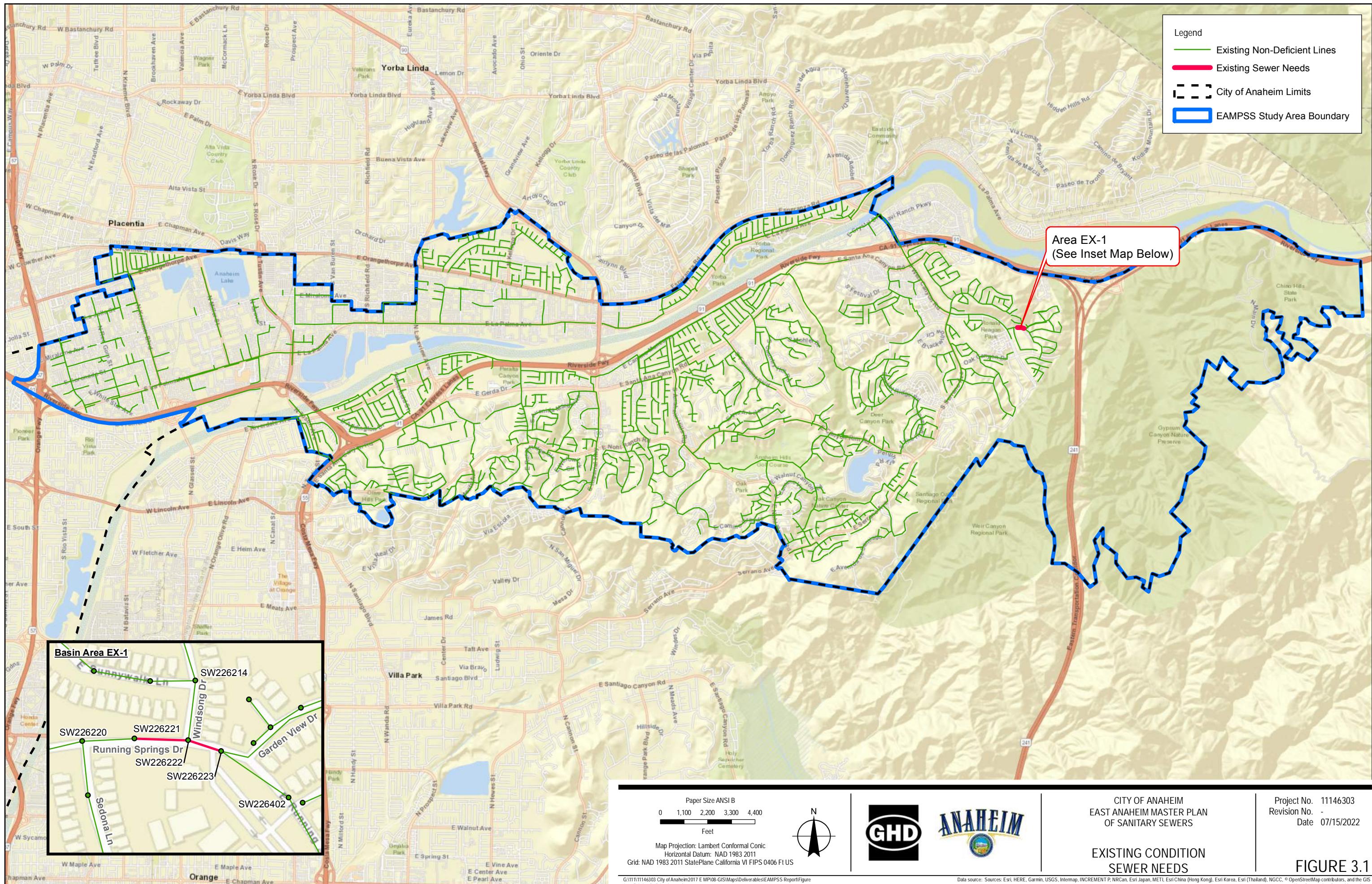


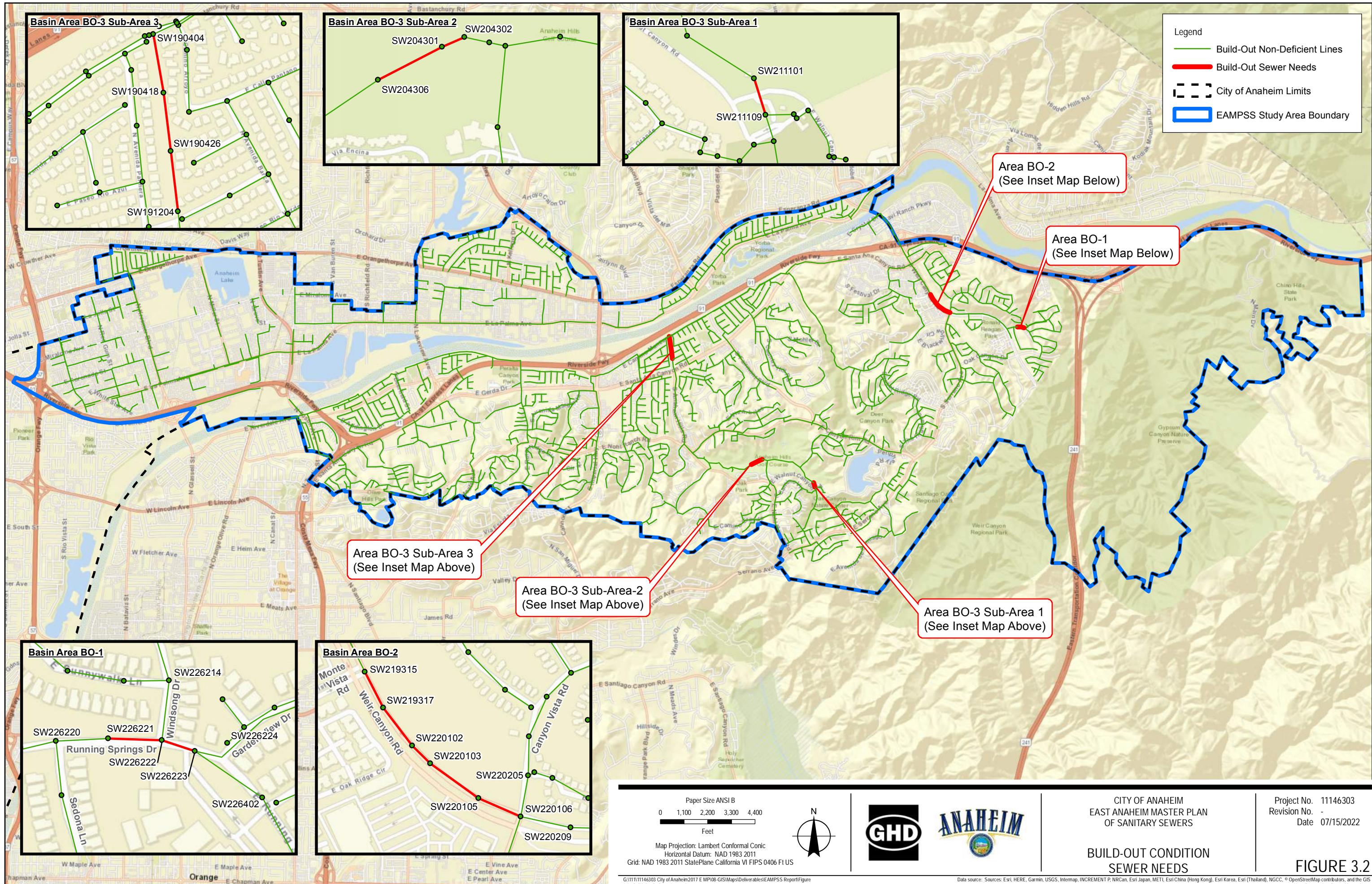
Central Anaheim Master Plan of Sanitary Sewers

Figure 3.2
Build-Out Conditions

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APPENDIX C
MODEL OUTPUT FOR PROJECT
TRIBUTARY BASINS
(PIPELINES 12-INCH AND LARGER)

Katella System Tributary Pipelines

Street	Cross Street	Upstrm MH- Dwnstrm MH	Pipe Data			Existing Scenario					Buildout Scenario with GPU					
			Size (in)	Length (ft)	Slope (ft/ft)	Peak Flow (mgd)	d/D Ratio	Analysis Flow (mgd)	Analysis Excess (mgd)	Additional DUs that would fit	Peak Flow (mgd)	d/D Ratio	Analysis Flow (mgd)	Analysis Excess (mgd)	Additional DUs that would fit	Additional GPU Flow into System (mgd)
K	9th St	SW066327-SW066325	24	326.9	0.002	4.43	0.66	5.18	0.76	1,573	5.93	1.00	5.18	-0.75	0	1.51
A		SW066325-SW066324	24	299.2	0.002	4.43	0.66	5.18	0.75	1,569	5.93	1.00	5.18	-0.76	0	1.51
T		SW066324-OT057444	24	42.9	0.002	4.43	0.66	5.19	0.76	1,583	5.93	1.00	5.19	-0.75	0	1.51
E		OT057444-SW057450	24	120.1	0.002	2.28	0.44	5.19	2.91	6,065	2.94	0.51	5.19	2.25	4,685	0.66
L		SW057450-SW057442	24	224.7	0.002	2.28	0.44	5.18	2.90	6,044	2.94	0.51	5.18	2.24	4,667	0.66
L		SW057442-SW057441	24	350.0	0.002	2.32	0.45	5.18	2.86	5,960	2.98	0.52	5.18	2.20	4,581	0.66
A		SW057441-SW057440	24	338.4	0.002	2.32	0.45	5.18	2.86	5,963	2.98	0.52	5.18	2.20	4,581	0.66
A		SW057440-SW057341	24	361.7	0.002	2.34	0.45	5.18	2.84	5,923	3.00	0.52	5.18	2.18	4,542	0.66
V		SW057341-SW057340	24	78.9	0.002	2.34	0.45	5.19	2.85	5,935	3.00	0.52	5.19	2.19	4,552	0.66
E		SW057340-SW057339	24	267.3	0.002	2.37	0.45	5.18	2.81	5,854	3.03	0.52	5.18	2.15	4,471	0.66
S		SW057339-SW057338	24	356.0	0.002	2.37	0.45	5.18	2.81	5,858	3.03	0.52	5.18	2.15	4,475	0.66
O		SW057338-SW057337	24	349.8	0.002	2.37	0.45	5.18	2.81	5,863	3.03	0.52	5.18	2.15	4,479	0.66
U		SW057337-SW057336	24	94.0	0.002	2.37	0.45	5.18	2.81	5,863	3.03	0.52	5.18	2.15	4,479	0.66
T		SW057336-SW057335	24	19.1	0.033	2.37	0.22	21.14	18.77	39,108	3.03	0.24	21.14	18.11	37,725	0.66
H	Euclid St	SW057335-SW057334	24	44.0	0.007	2.37	0.32	9.90	7.53	15,688	3.03	0.36	9.90	6.87	14,304	0.66
		SW057334-SW057333	24	36.1	0.007	2.37	0.32	9.89	7.53	15,677	3.03	0.36	9.89	6.86	14,294	0.66

Orangewood System Tributary Pipelines

Pipe Data			Existing Scenario						Buildout Scenario with GPU						
Street	Cross Street	Upstrm MH-Dwnstrm MH	Size (in)	Length (ft)	Peak Flow (mgd)	d/D Ratio	Analysis Flow (mgd)	Analysis Excess (mgd)	Additional DUs that would fit	Peak Flow (mgd)	d/D Ratio	Analysis Flow (mgd)	Analysis Excess (mgd)	Additional DUs that would fit	Additional GPU Flow into System (mgd)
STATE COLLEGE BLVD	Cerritos Ave	OT116311-OT116313	54	498.8	4.49	0.20	46.21	41.72	86,923	4.55	0.20	46.21	41.66	86,792	0.06
		OT116313-OT117116	54	187.2	4.49	0.20	45.97	41.49	86,429	4.55	0.20	45.97	41.42	86,298	0.06
		OT117116-OT117115	54	703.2	4.56	0.20	46.05	41.49	86,440	4.62	0.20	46.05	41.43	86,308	0.06
		OT117115-OT117114	54	587.7	4.56	0.20	46.08	41.52	86,492	4.62	0.20	46.08	41.45	86,360	0.06
		OT117114-OT117316	54	578.3	4.56	0.17	66.49	61.93	129,017	4.62	0.17	66.49	61.87	128,885	0.06
		OT117316-OT117315	54	558	4.56	0.21	43.48	38.92	81,088	4.62	0.21	43.48	38.86	80,956	0.06
	Katella Ave	OT117315-OT118106	54	171.16	4.56	0.22	40.00	35.44	73,840	4.62	0.22	40.00	35.38	73,708	0.06
		OT118106-OT118125	54	710.39	6.20	0.24	43.25	37.05	77,192	6.80	0.26	43.25	36.45	75,942	0.60
		OT118125-OT118124	54	597.79	6.22	0.27	34.71	28.49	59,360	6.82	0.29	34.71	27.89	58,113	0.60
		OT118124-OT118322	54	600.66	6.38	0.28	34.87	28.50	59,365	6.98	0.29	34.87	27.89	58,113	0.60
		OT118322-OT118323	54	592.72	8.40	0.32	34.61	26.21	54,602	9.01	0.33	34.61	25.61	53,348	0.60
		OT118323-OT109203	39	117.12	4.48	0.31	19.54	15.05	31,363	4.80	0.32	19.54	14.74	30,710	0.31
	Orangewood	OT118323-OT119103	48	132.28	4.03	0.25	26.37	22.34	46,542	4.32	0.26	26.37	22.05	45,938	0.29

