HYDROLOGY STUDY

For:

TORRANCE WAREHOUSE

Project Site Location/Address: 2555 W. 190th Street Torrance, CA 90504

Prepared For:

St. Paul Fire and Marine Insurance Co.,
A Connecticut Corporation

385 Washington Street St. Paul, MN 55102

Lead Agency:

City of Torrance

3031 Torrance Blvd. Torrance, CA 90503

Prepared by:

DRC Engineering, Inc.

160 S. Old Springs Road, Suite 210 Anaheim, CA 92808 (714) 685-6860 Matthew Hellesen, P.E.

July 29, 2020

Project No. 19-040

TABLE OF CONTENTS

SECTION 1.0 NARRATIVE

Introduction

Project Description Hydrology Analysis Summary & Conclusions

SECTION 2.0 SITE INFORMATION

Location Map

LA County Hydrology Map

SECTION 3.0 EXISTING SITE HYDROLOGIC ANALYSIS

Existing Hydrology Map 10-year Storm Calculations 50-year Storm Calculations

SECTION 4.0 PROPOSED SITE HYDROLOGIC ANALYSIS

Proposed Hydrology Map 10-year Storm Calculations 50-year Storm Calculations

Detention System and Outlet Control Calculations

SECTION 5.0 CITY OF TORRANCE

Storm Drain Master Plan

Tributary Areas Q and Pipe Capacity Data

Introduction

The proposed project is located at 2555 W. 190th Street in the City of Torrance, California.

The project is 14.2 acres, with approximately 291,000 square feet in building area. The proposed building will be a manufacturing warehouse and surrounded by paved parking and landscape. An existing building and parking will be demolished and regrading of the site will occur.

This report has been prepared to calculate the post-construction hydrologic conditions for peak storm runoff rates and demonstrate the overall impact to the existing drainage infrastructure as well as demonstrate that the proposed project will not exceed the existing condition.

Project Description

Existing Site Conditions:

The project site is 14.23 acres. At its existing state, approximately 1.26 acres is building, 11.38 acres is impervious pavement, and 1.59 acres is landscaping (11.1% pervious).

Per the existing hydrology map, 8.65 acres (DA-1) is sheet flowing southwesterly towards and out of the corner of the property, eventually draining to the street catch basin at the northeast corner of 190th Street and Crenshaw Place. 5.58 acres (DA-2) of runoff flows towards individual inlets throughout the area and collects into an underground storm drain network. The laterals eventually collect into a 27-inch storm drain main that runs from north to south, connecting to a county storm drain RCB at 190th Street.

As part of the study, approximately 2.69 acres of off-site area (DA-0) located north of project site is included due to its drainage runoff contributing to the 27-inch storm drain.

Reference Table 1.1 and Section 3.0 for Existing Hydrology Map

Proposed Site Conditions:

The proposed site will see the demolition of the existing building, pavement, and landscaping and the development of 291,000 square feet (6.68 acres) of a manufacturing warehouse, 6.21 acres of parking/sidewalk (impervious), and 1.34 acres of landscaping.

Per the proposed hydrology map, all 14.32 acres of development will flow towards individual inlets and eventually collects into underground piping and outlets into the existing 6'-9" x 8' LACFCD RCB.

As part of the study, approximately 2.69 acres of off-site area (DA-0) located north of project site is included due to its drainage runoff contributing to the existing 27-inch storm drain, which will be upsized and relocated easterly due to the location of the proposed warehouse.

Reference Tables 1.2 and 1.3 and Section 4.0 for Proposed Hydrology Map



Hydrology Methodology

Methodology

For both existing and proposed conditions, the peak storm discharge for the drainage subareas was calculated based on the Los Angeles County Department of Public Works (LADPW) Hydrology Manual. LA County Hydrocalc spreadsheets (see Section 3.0 for existing and Section 4.0 for proposed) were used to calculate the 10-year and 50-year storm events. Per the LA County Hydrology Map, the soil on-site is type 09 and the 50-year isohyet is 5.8 (see Section 2.0). Due to the peak runoff for each of the above storm events for the proposed conditions exceeding the peak runoff of the existing conditions, hydromodification is required.

The Hydrocalc spreadsheets were also used to calculate the unit hydrographs for the 10-year and 50-year storm events both existing and proposed. The peak runoff from each event in the proposed condition was then input into the Hydraflow Hydrograph program (an extension of Civil 3D) to design the outlet control and detention storage.

The hydrology analysis was done for the property as well as the off-site property to the north. Note: Drainage areas DA-0 and DA-3 in the proposed condition will not be included as part of the unit hydrology analysis and detention design/outlet control (i.e., only DA-1 and DA-2 of the proposed condition will be used).

Existing Condition:

The existing project site peak runoff rates are determined using the LA County Hydrocalc spreadsheets. Refer to Section 3.0, the Existing Condition Hydrology Calculations of this report for data used in the calculations. The following table illustrates the runoff rate calculation results for the existing conditions. These values are used as the benchmark for flow reduction in the post-developed (proposed) condition for the on-site areas.

_					
	Drainage	Area	%	FLOWRATE: 10-YR	FLOWRATE: 50-YR
	Area	(ac.)	Impervious	STORM (cfs)	STORM (cfs)
	DA-0	2.69	90	5.94	8.38
	DA-1	8.65	98	12.22	18.57
	DA-2	5.58	74	12.17	17.38
	ΤΟΤΔΙ	16 92		30 33	44 33

Table 1.1: EXISTING DRAINAGE SUMMARY TABLE

Proposed Condition:

Reference Tables 1.2 and 1.3 and Section 4.0 for Proposed Hydrology Map

The proposed site ultimately discharges to an existing 6'-9"x8' LACFCD RCB. With a fully developed site, the total peak flow rates from each storm event exceeds the existing peak flow rate, as shown in Table 1.2. To maintain or be less than the existing total outflow, underground detention systems are placed at drainage areas DA-1 and DA-2. Per Table 1.3, comparing the flows to that of the existing condition, the post-developed total on-site runoff rates are lower



due to the utilization of the underground detention systems. In addition, prior to flowing offsite, site storm water runoff will be treated by a combination of biofiltration basins and other proprietary water quality systems so as to remove, to acceptable levels, the pollutants of concern generated by the project and the pollutants of concerns for the downstream watercourses.

Table 1.2: PROPOSED DRAINAGE SUMMARY TABLE

Drainage	Area	%	FLOWRATE: 10-YR	FLOWRATE: 50-YR
Area	(ac.)	Impervious	STORM (cfs)	STORM (cfs)
DA-0	2.69	90	5.94	8.38
DA-1	6.38	86	14.04	21.96
DA-2	7.05	95	15.62	19.87
DA-3	0.8	89	1.76	2.49
TOTAL	16.92		37.36	52.70

Table 1.3: PROPOSED DRAINAGE SUMMARY TABLE W/ DET. SYSTEM (DA-1 & DA-2 ONLY)

			-	,
Drainage	Area	%	FLOWRATE: 10-YR	FLOWRATE: 50-YR
Area	(ac.)	Impervious	STORM (cfs)	STORM (cfs)
DA-0	2.69	90	5.94	8.38
DA-1	6.38	86	8.54	16.53
DA-2	7.05	95	7.91	15.90
DA-3	0.8	89	1.76	2.49
TOTAL	16.92		24.15	43.30

DETENTION A DETENTION B

Summary and Conclusions

Refer to sections 3 & 4 for the proposed and existing hydrology maps depicting the drainage areas of concern. Analyzing the effective area, its computed that the total existing site flowrate in the 10-year and 50-year is **30.33 cfs** and **44.33 cfs**, respectively, and the proposed site flowrate is **24.15 cfs** and **43.30 cfs** when detention systems are in place.

Through the implementation of the detention systems and outlet controls, the peak discharges for the 10-year and 50-year storm events were dropped to below existing condition levels.

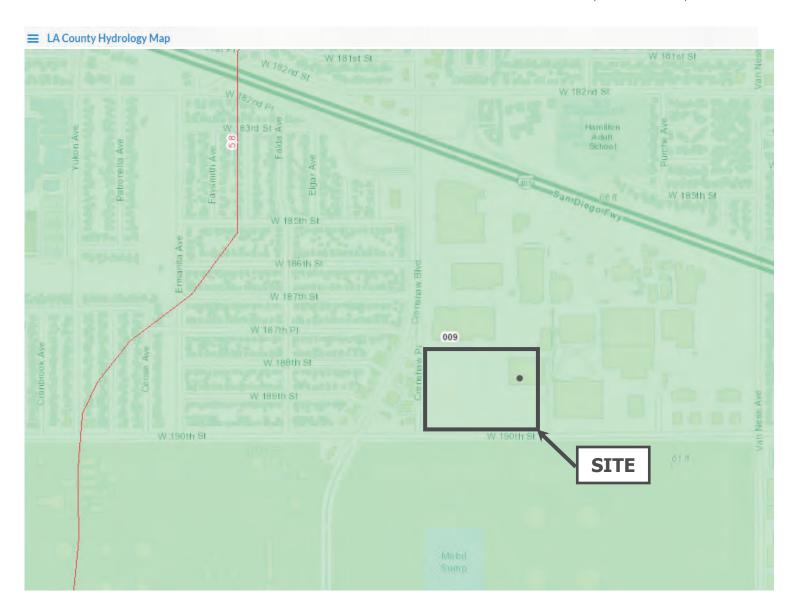


SECTION 2.0

VICINITY MAP (NOT TO SCALE)

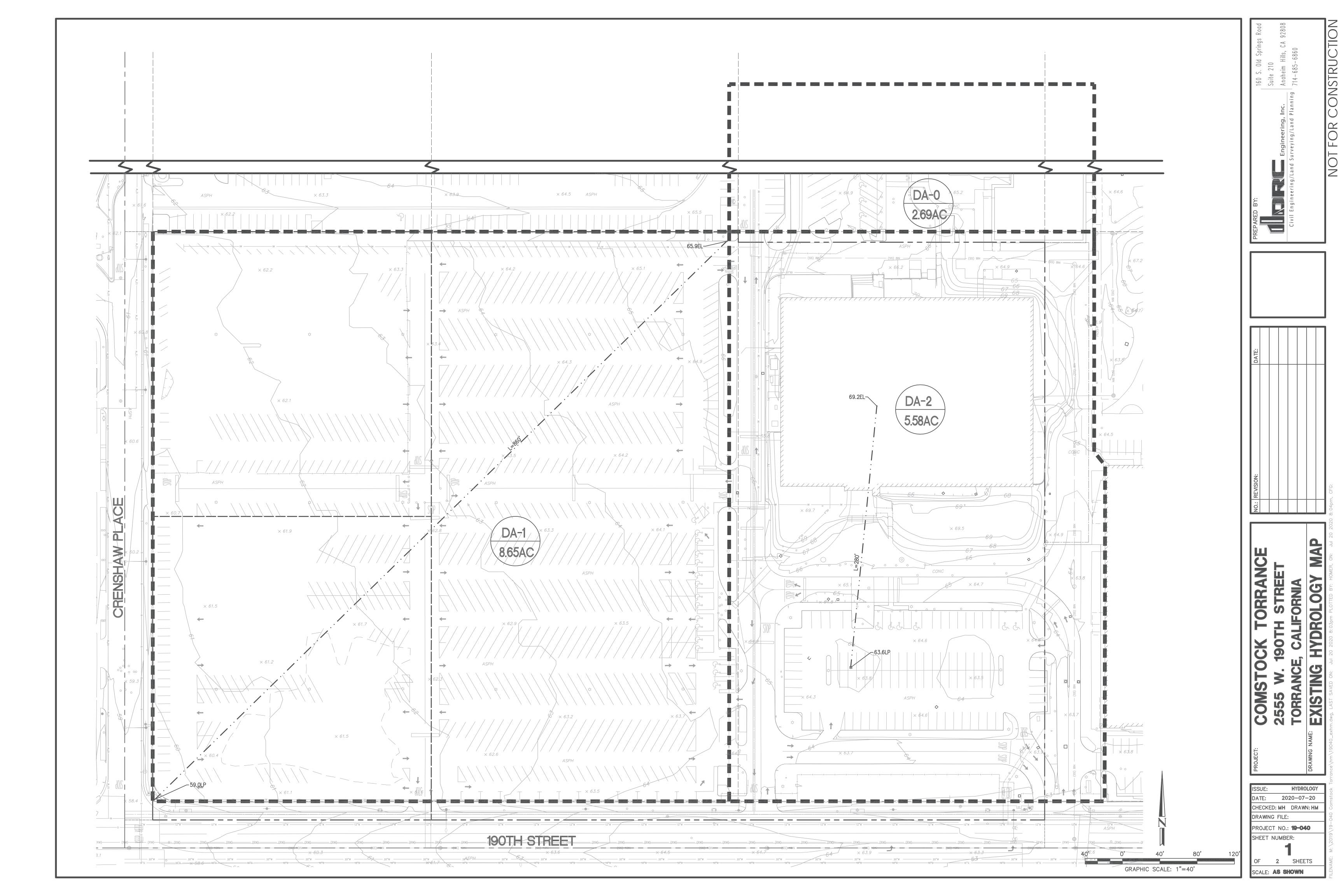


LA COUNTY HYDROLOGY MAP (NOT TO SCALE)



SECTION 3.0

Existing Hydrology Map Existing 10-year Storm Calculations Existing 50-year Storm Calculations

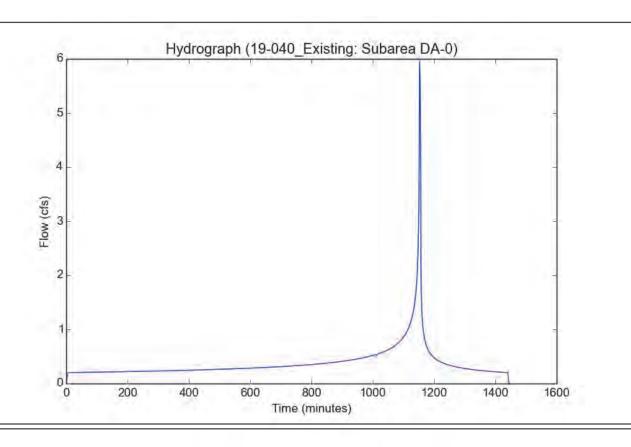


File location: M:/2019/19-040 Comstock Torrance/hm/hydrocalc/19-040_Existing - Subarea DA-0_10yr.pdf Version: HydroCalc 1.0.2

Input	Paran	neters
-------	-------	--------

Project Name	19-040_Existing
Subarea ID	Subarea DA-0
Area (ac)	2.69
Flow Path Length (ft)	150.0
Flow Path Slope (vft/hft)	0.015
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.9
Soil Type	9
Design Storm Frequency	10-yr
Fire Factor	0
LID	False

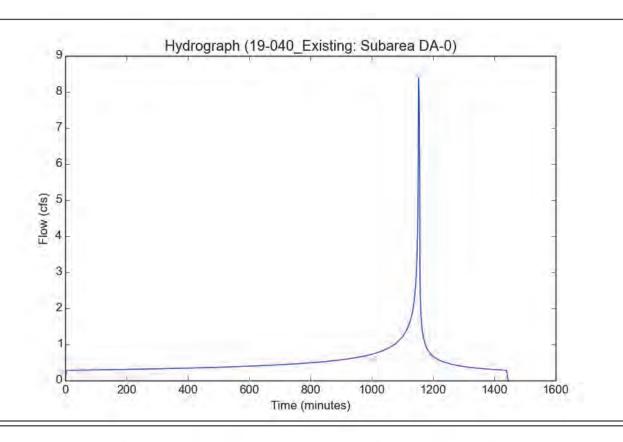
Modeled (10-yr) Rainfall Depth (in)	4.1412
Peak Intensity (in/hr)	2.4708
Undeveloped Runoff Coefficient (Cu)	0.8343
Developed Runoff Coefficient (Cd)	0.8934
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	5.938
Burned Peak Flow Rate (cfs)	5.938
24-Hr Clear Runoff Volume (ac-ft)	0.7617
24-Hr Clear Runoff Volume (cu-ft)	33177.4748



File location: M:/2019/19-040 Comstock Torrance/hm/hydrocalc/19040_Existing - Subarea DA-0_50yr.pdf Version: HydroCalc 1.0.2

Project Name	19-040_Existing
Subarea ID	Subarea DA-0
Area (ac)	2.69
Flow Path Length (ft)	150.0
Flow Path Slope (vft/hft)	0.015
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.9
Soil Type	9
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

o at pat 1 too alto	
Modeled (50-yr) Rainfall Depth (in)	5.8
Peak Intensity (in/hr)	3.4604
Undeveloped Runoff Coefficient (Cu)	0.9013
Developed Runoff Coefficient (Cd)	0.9
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	8.3777
Burned Peak Flow Rate (cfs)	8.3777
24-Hr Clear Runoff Volume (ac-ft)	1.0712
24-Hr Clear Runoff Volume (cu-ft)	46661.3509
,	

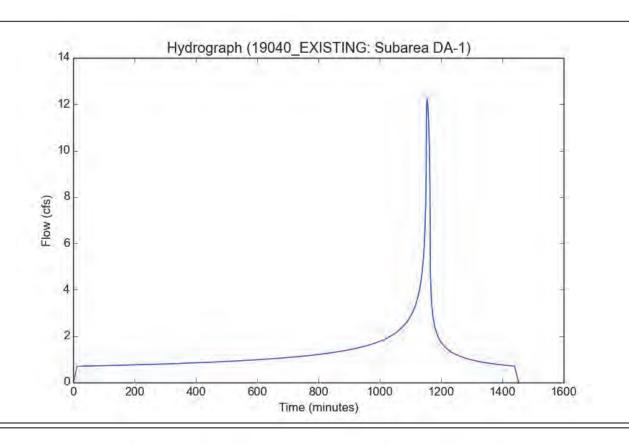


File location: M:/2019/19-040 Comstock Torrance/hm/hydrocalc/19040_Existing - Subarea DA-1_10yr.pdf Version: HydroCalc 1.0.2

Input	Param	eters
-------	--------------	-------

Project Name	19040_EXISTING
Subarea ID	Subarea DA-1
Area (ac)	8.65
Flow Path Length (ft)	860.0
Flow Path Slope (vft/hft)	0.007
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.98
Soil Type	9
Design Storm Frequency	10-yr
Fire Factor	0
LID	False

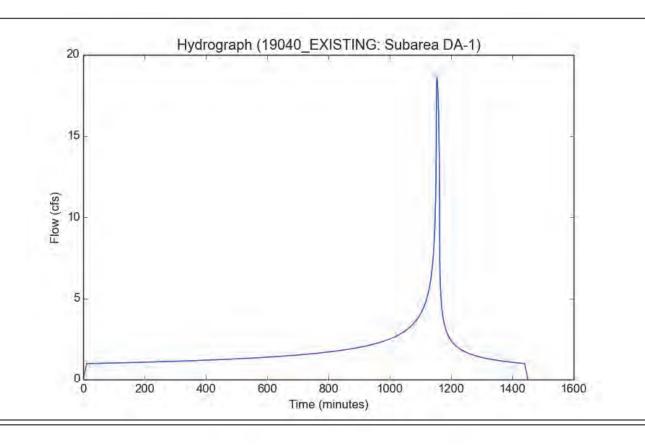
Modeled (10-yr) Rainfall Depth (in)	4.1412
Peak Intensity (in/hr)	1.5769
Undeveloped Runoff Coefficient (Cu)	0.6878
Developed Runoff Coefficient (Cd)	0.8958
Time of Concentration (min)	13.0
Clear Peak Flow Rate (cfs)	12.2179
Burned Peak Flow Rate (cfs)	12.2179
24-Hr Clear Runoff Volume (ac-ft)	2.6212
24-Hr Clear Runoff Volume (cu-ft)	114178.1321



File location: M:/2019/19-040 Comstock Torrance/hm/hydrocalc/19040_Existing - Subarea DA-1_50yr.pdf Version: HydroCalc 1.0.2

Project Name	19040_EXISTING
Subarea ID	Subarea DA-1
Area (ac)	8.65
Flow Path Length (ft)	860.0
Flow Path Slope (vft/hft)	0.007
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.98
Soil Type	9
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

o dipat i toodito		
Modeled (50-yr) Rainfall Depth (in)	5.8	
Peak Intensity (in/hr)	2.3889	
Undeveloped Runoff Coefficient (Cu)	0.8226	
Developed Runoff Coefficient (Cd)	0.8985	
Time of Concentration (min)	11.0	
Clear Peak Flow Rate (cfs)	18.5654	
Burned Peak Flow Rate (cfs)	18.5654	
24-Hr Clear Runoff Volume (ac-ft)	3.6741	
24-Hr Clear Runoff Volume (cu-ft)	160045.7527	
,		

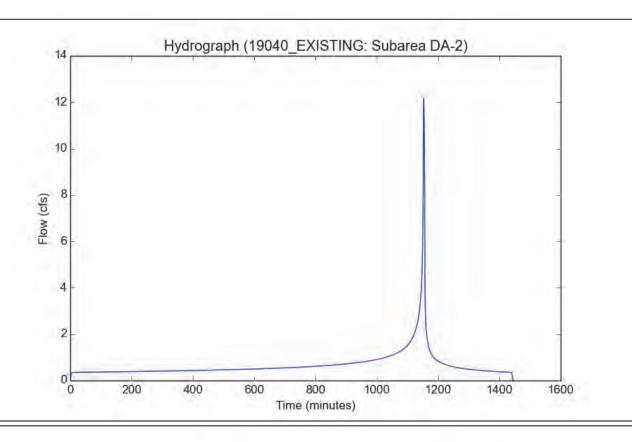


File location: M:/2019/19-040 Comstock Torrance/hm/hydrocalc/19040_Existing - Subarea DA-2_10yr.pdf Version: HydroCalc 1.0.2

Input	Param	eters
-------	--------------	-------

Project Name	19040_EXISTING
Subarea ID	Subarea DA-2
Area (ac)	5.58
Flow Path Length (ft)	280.0
Flow Path Slope (vft/hft)	0.02
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.74
Soil Type	9
Design Storm Frequency	10-yr
Fire Factor	0
LID	False

Output Nesaits	
Modeled (10-yr) Rainfall Depth (in)	4.1412
Peak Intensity (in/hr)	2.4708
Undeveloped Runoff Coefficient (Cu)	0.8343
Developed Runoff Coefficient (Cd)	0.8829
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	12.1725
Burned Peak Flow Rate (cfs)	12.1725
24-Hr Clear Runoff Volume (ac-ft)	1.3578
24-Hr Clear Runoff Volume (cu-ft)	59144.9883

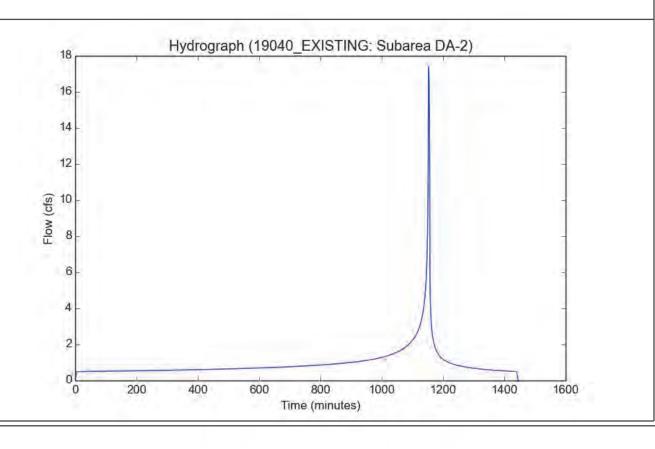


File location: M:/2019/19-040 Comstock Torrance/hm/hydrocalc/19040_Existing - Subarea DA-2_50yr.pdf Version: HydroCalc 1.0.2

Input	Parameters	3
-------	-------------------	---

Project Name	19040_EXISTING
Subarea ID	Subarea DA-2
Area (ac)	5.58
Flow Path Length (ft)	280.0
Flow Path Slope (vft/hft)	0.02
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.74
Soil Type	9
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

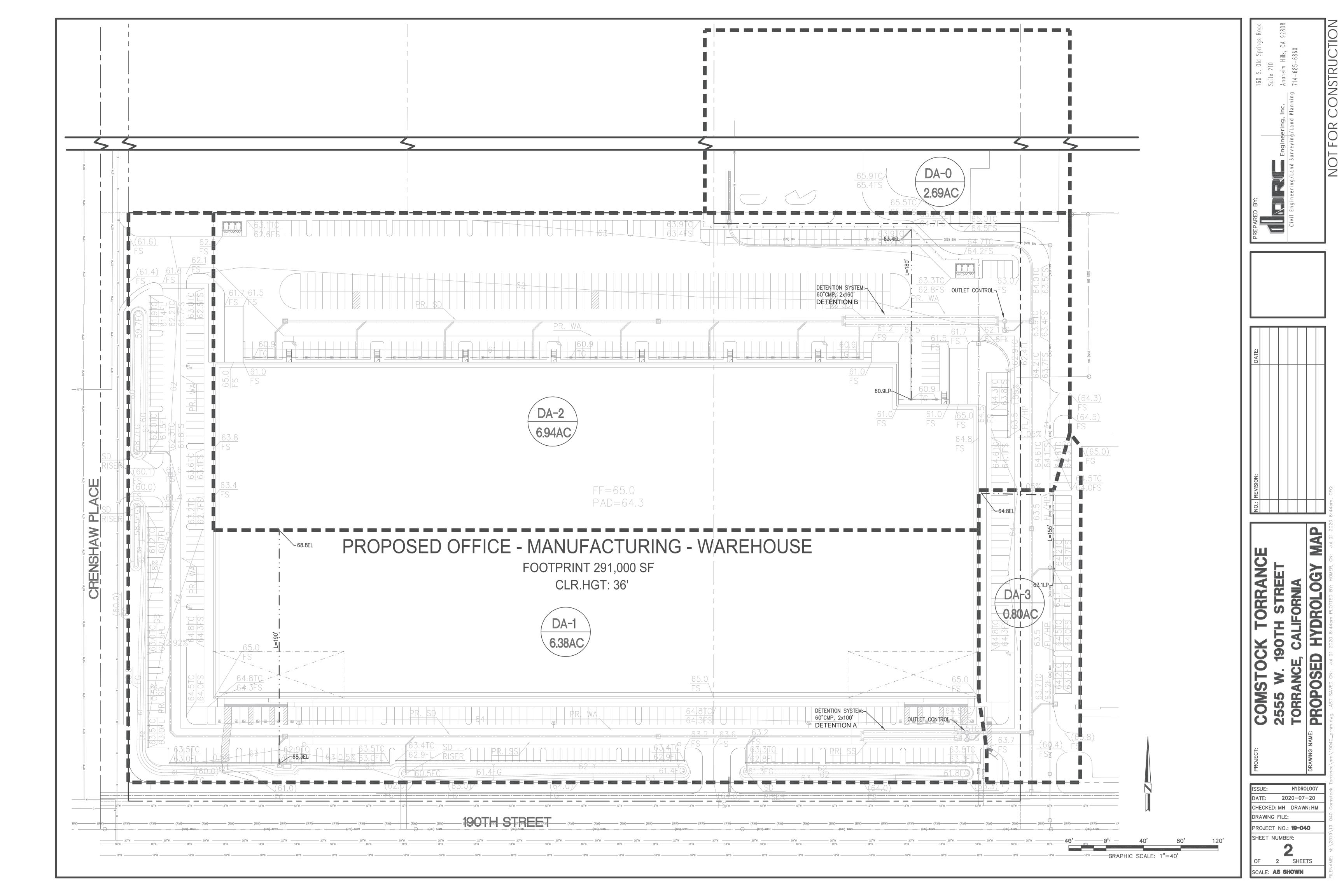
o at part i too aito	
Modeled (50-yr) Rainfall Depth (in)	5.8
Peak Intensity (in/hr)	3.4604
Undeveloped Runoff Coefficient (Cu)	0.9013
Developed Runoff Coefficient (Cd)	0.9
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	17.3783
Burned Peak Flow Rate (cfs)	17.3783
24-Hr Clear Runoff Volume (ac-ft)	1.9257
24-Hr Clear Runoff Volume (cu-ft)	83884.0343
,	



SECTION 4.0

Proposed Hydrology Map Proposed 10-year Storm Calculations Proposed 50-year Storm Calculations Detention System and Outlet Control Calculations



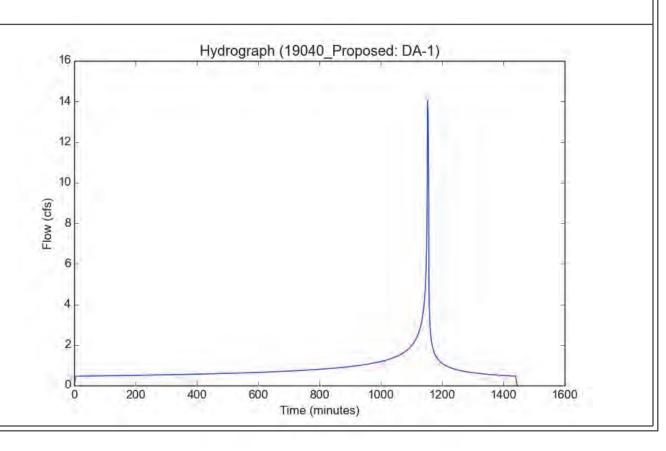


File location: M:/2019/19-040 Comstock Torrance/hm/hydrocalc/19040_Proposed - DA-1_10yr.pdf Version: HydroCalc 1.0.2

Input Parameters

19040_Proposed
DA-1
6.38
190.0
0.02
5.8
0.86
9
10-yr
0
False

output itoodito	
Modeled (10-yr) Rainfall Depth (in)	4.1412
Peak Intensity (in/hr)	2.4708
Undeveloped Runoff Coefficient (Cu)	0.8343
Developed Runoff Coefficient (Cd)	0.8908
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	14.042
Burned Peak Flow Rate (cfs)	14.042
24-Hr Clear Runoff Volume (ac-ft)	1.7429
24-Hr Clear Runoff Volume (cu-ft)	75922.5767

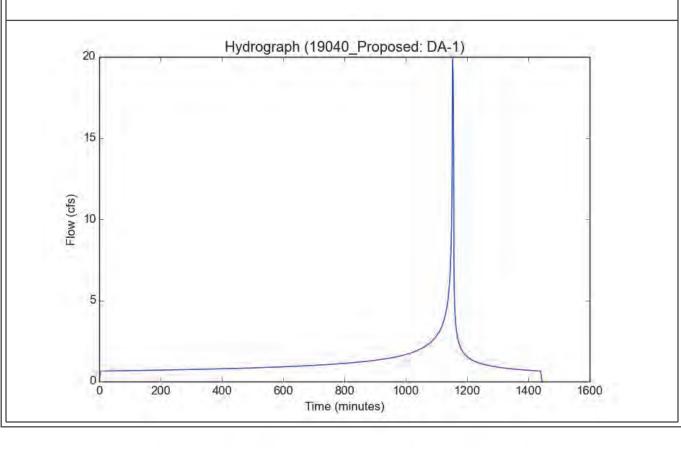


 $\label{location:m:m:2019/19-040} File \ location: \ M:/2019/19-040 \ Comstock \ Torrance/hm/hydrocalc/19040_Proposed - \ DA-1_50yr.pdf \ Version: \ HydroCalc \ 1.0.2$

Input	Param	eters
-------	--------------	-------

Project Name	19040_Proposed
Subarea ID	DA-1
Area (ac)	6.38
Flow Path Length (ft)	190.0
Flow Path Slope (vft/hft)	0.02
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.86
Soil Type	9
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

o at par 1 to care	
Modeled (50-yr) Rainfall Depth (in)	5.8
Peak Intensity (in/hr)	3.4604
Undeveloped Runoff Coefficient (Cu)	0.9013
Developed Runoff Coefficient (Cd)	0.9
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	19.8698
Burned Peak Flow Rate (cfs)	19.8698
24-Hr Clear Runoff Volume (ac-ft)	2.4559
24-Hr Clear Runoff Volume (cu-ft)	106979.3015
,	

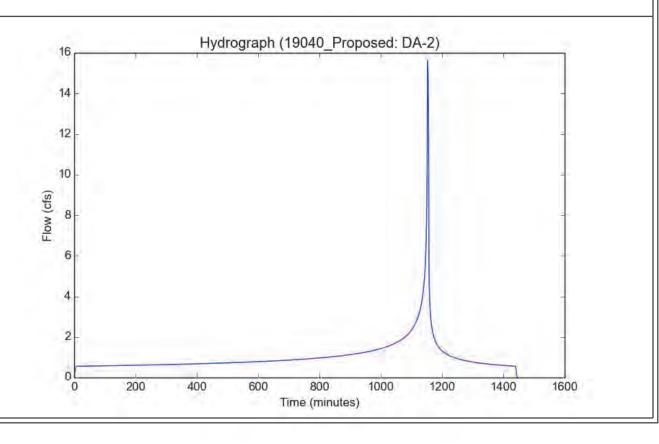


File location: M:/2019/19-040 Comstock Torrance/hm/hydrocalc/19040_Proposed - DA-2_10yr.pdf Version: HydroCalc 1.0.2

Input	Param	eters
-------	--------------	-------

Project Name	19040_Proposed
Subarea ID	DA-2
Area (ac)	7.05
Flow Path Length (ft)	180.0
Flow Path Slope (vft/hft)	0.014
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.95
Soil Type	9
Design Storm Frequency	10-yr
Fire Factor	0
LID	False

Output Nesaits	
Modeled (10-yr) Rainfall Depth (in)	4.1412
Peak Intensity (in/hr)	2.4708
Undeveloped Runoff Coefficient (Cu)	0.8343
Developed Runoff Coefficient (Cd)	0.8967
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	15.6197
Burned Peak Flow Rate (cfs)	15.6197
24-Hr Clear Runoff Volume (ac-ft)	2.0839
24-Hr Clear Runoff Volume (cu-ft)	90772.7191

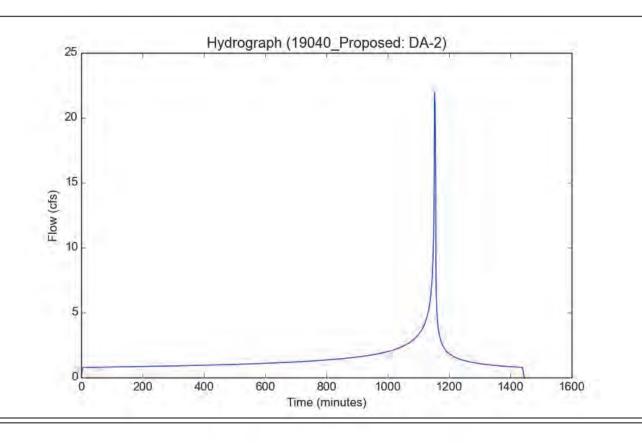


File location: M:/2019/19-040 Comstock Torrance/hm/hydrocalc/19040_Proposed - DA-2_50yr.pdf Version: HydroCalc 1.0.2

Input	Param	eters
-------	--------------	-------

Project Name	19040_Proposed
Subarea ID	DA-2
Area (ac)	7.05
Flow Path Length (ft)	180.0
Flow Path Slope (vft/hft)	0.014
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.95
Soil Type	9
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

Output Results	
Modeled (50-yr) Rainfall Depth (in)	5.8
Peak Intensity (in/hr)	3.4604
Undeveloped Runoff Coefficient (Cu)	0.9013
Developed Runoff Coefficient (Cd)	0.9
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	21.9565
Burned Peak Flow Rate (cfs)	21.9565
24-Hr Clear Runoff Volume (ac-ft)	2.9244
24-Hr Clear Runoff Volume (cu-ft)	127387.2713

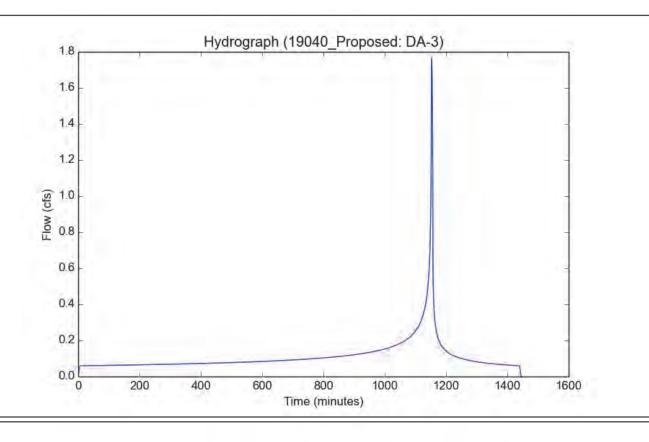


File location: M:/2019/19-040 Comstock Torrance/hm/hydrocalc/19040_Proposed - DA-3_10yr.pdf Version: HydroCalc 1.0.2

Input	Parameters
-------	-------------------

Project Name	19040_Proposed
Subarea ID	DA-3
Area (ac)	0.8
Flow Path Length (ft)	155.0
Flow Path Slope (vft/hft)	0.01
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.89
Soil Type	9
Design Storm Frequency	10-yr
Fire Factor	0
LID	False

Output Nesalts		
Modeled (10-yr) Rainfall Depth (in)	4.1412	
Peak Intensity (in/hr)	2.4708	
Undeveloped Runoff Coefficient (Cu)	0.8343	
Developed Runoff Coefficient (Cd)	0.8928	
Time of Concentration (min)	5.0	
Clear Peak Flow Rate (cfs)	1.7647	
Burned Peak Flow Rate (cfs)	1.7647	
24-Hr Clear Runoff Volume (ac-ft)	0.2245	
24-Hr Clear Runoff Volume (cu-ft)	9780.1983	

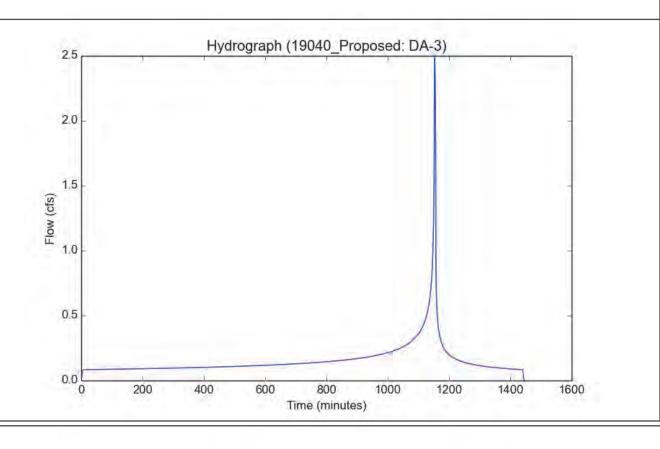


File location: M:/2019/19-040 Comstock Torrance/hm/hydrocalc/19040_Proposed - DA-3_50yr.pdf Version: HydroCalc 1.0.2

Input	Param	eters
-------	--------------	-------

Project Name	19040_Proposed
Subarea ID	DA-3
Area (ac)	0.8
Flow Path Length (ft)	155.0
Flow Path Slope (vft/hft)	0.01
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.89
Soil Type	9
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

output i toodito	
Modeled (50-yr) Rainfall Depth (in)	5.8
Peak Intensity (in/hr)	3.4604
Undeveloped Runoff Coefficient (Cu)	0.9013
Developed Runoff Coefficient (Cd)	0.9
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	2.4915
Burned Peak Flow Rate (cfs)	2.4915
24-Hr Clear Runoff Volume (ac-ft)	0.3159
24-Hr Clear Runoff Volume (cu-ft)	13761.3194
,	



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

łyd. ło.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	Manual	14.04	5	1160	76,302				Prop DA-1_10yr	
2	Reservoir	8.537	5	1165	76,300	1	52.00	3,269	Detention A	
_	TVESELVOII	0.557		1105	70,500	'	32.00	3,209		10-year
4	Manual	15.62	5	1160	91,200				Prop DA-2_10yr	. o you
5	Reservoir	7.909	5	1165	91,197	4	54.20	5,103	Detention B	
7	Manual	1.760	5	1160	9,816				Prop DA-3_10yr	
10	Manual	0.000	5	n/a	0				Prop DA-1_50yr	
11	Reservoir	0.000	5	n/a	0	10	46.90	0.000	Detention A	
13	Manual	0.000	5	n/a	0				Prop DA-2_50yr	
14	Reservoir	0.000	5	n/a	0	13	49.81	0.000	Detention B	
16	Manual	0.000	5	n/a	0				Prop DA-3_50yr	
190	40_Detention	n_2.gpw			Return F	Period: 10 Y	ear/	Tuesday, 0	7 / 21 / 2020	

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	Manual	0.000	5	n/a	0				Prop DA-1_10yr	
2	Reservoir	0.000	5	n/a	0	1	46.90	0.000	Detention A	
4	Manual	0.000	5	n/a	0				Prop DA-2_10yr	
5	Reservoir	0.000	5	n/a	0	4	49.81	0.000	Detention B	
7	Manual	0.000	5	n/a	0				Prop DA-3_10yr	
10	Manual	19.87	5	1160	107,514				Prop DA-1_50yr	
11	Reservoir	16.53	5	1160	107,512	10	52.00	3,663	Detention A	
13	Manual	21.96	5	1160	127,965				Prop DA-2_50yr	50-year
14	Reservoir	15.90	5	1160	127,962	13	54.97	5,788	Detention B	
16	Manual	2.490	5	1160	13,830				Prop DA-3_50yr	
190	40_Detention	 n_2.gpw			Return P	eriod: 50 Y	/ear	Tuesday, 0)7 / 21 / 2020	

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Tuesday, 07 / 21 / 2020

Pond No. 1 - Detention A

Pond Data

UG Chambers -Invert elev. = 46.90 ft, Rise x Span = 5.00 x 5.00 ft, Barrel Len = 100.00 ft, No. Barrels = 2, Slope = 0.10%, Headers = No

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	46.90	n/a	0	0
0.51	47.41	n/a	181	181
1.02	47.92	n/a	355	536
1.53	48.43	n/a	438	974
2.04	48.94	n/a	484	1,458
2.55	49.45	n/a	507	1,964
3.06	49.96	n/a	507	2,471
3.57	50.47	n/a	485	2,956
4.08	50.98	n/a	437	3,392
4.59	51.49	n/a	354	3,747
5.10	52.00	n/a	181	3,928

Culvert / Orifice Structures Weir Structures [B] [C] [PrfRsr] [A] [B] [C] [D] [A] = 12.00 0.00 0.00 0.00 = 4.00 0.00 0.00 0.00 Rise (in) Crest Len (ft) Span (in) = 12.000.00 0.00 0.00 Crest El. (ft) = 50.60 0.00 0.00 0.00 No. Barrels = 1 0 0 0 Weir Coeff. = 3.333.33 3.33 3.33 = 46.90 0.00 0.00 0.00 Weir Type = Rect Invert El. (ft) = 1.00 0.00 0.00 0.00 Multi-Stage = No No No No Length (ft) 0.00 0.00 Slope (%) = 1.00 n/a = .013 .013 N-Value .013 n/a Orifice Coeff. = 0.600.60 0.60 0.60 Exfil.(in/hr) = 0.000 (by Contour) Multi-Stage No No No TW Elev. (ft) = 0.00= n/a

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	CIv A cfs	CIv B cfs	CIv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	46.90	0.00				0.00						0.000
0.51	181	47.41	0.26 oc				0.00						0.261
1.02	536	47.92	0.88 oc				0.00						0.882
1.53	974	48.43	3.74 oc				0.00						3.743
2.04	1,458	48.94	4.69 ic				0.00						4.692
2.55	1,964	49.45	5.41 ic				0.00						5.414
3.06	2,471	49.96	6.05 ic				0.00						6.050
3.57	2,956	50.47	6.63 ic				0.00						6.625
4.08	3,392	50.98	7.15 ic				3.12						10.27
4.59	3,747	51.49	7.65 ic				11.18						18.83
5.10	3,928	52.00	8.11 ic				22.06						30.17

Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

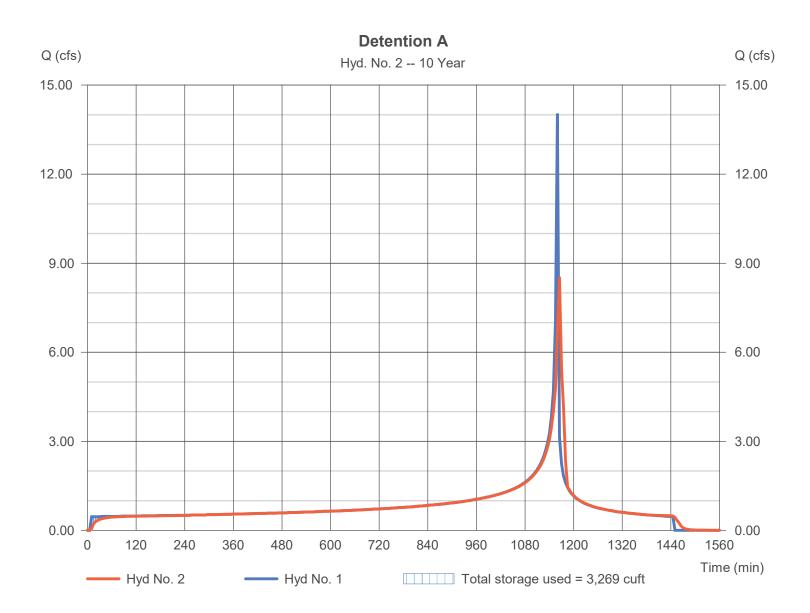
Tuesday, 07 / 21 / 2020

Hyd. No. 2

Detention A

Hydrograph type Peak discharge = 8.537 cfs= Reservoir Storm frequency = 10 yrsTime to peak = 1165 min Time interval = 5 min Hyd. volume = 76,300 cuftInflow hyd. No. = 1 - Prop DA-1_10yr Max. Elevation $= 52.00 \, \text{ft}$ = Detention A Max. Storage Reservoir name = 3,269 cuft

Storage Indication method used.



Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

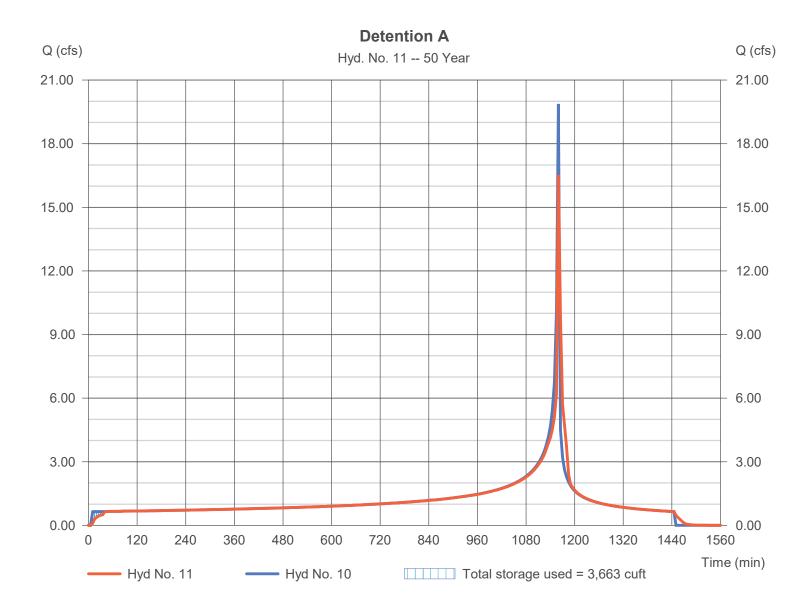
Tuesday, 07 / 21 / 2020

Hyd. No. 11

Detention A

Hydrograph type Peak discharge = 16.53 cfs= Reservoir Storm frequency = 50 yrsTime to peak = 1160 min Time interval = 5 min Hyd. volume = 107,512 cuft Inflow hyd. No. = 10 - Prop DA-1_50yr Max. Elevation $= 52.00 \, \text{ft}$ = Detention A Max. Storage Reservoir name = 3,663 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Tuesday, 07 / 21 / 2020

Pond No. 3 - Detention B

Pond Data

UG Chambers -Invert elev. = 49.81 ft, Rise x Span = 5.00 x 5.00 ft, Barrel Len = 160.00 ft, No. Barrels = 2, Slope = 0.10%, Headers = No

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	49.81	n/a	0	0
0.52	50.33	n/a	269	269
1.03	50.84	n/a	566	835
1.55	51.36	n/a	705	1,540
2.06	51.87	n/a	783	2,323
2.58	52.39	n/a	820	3,143
3.10	52.91	n/a	820	3,963
3.61	53.42	n/a	783	4,746
4.13	53.94	n/a	705	5,451
4.64	54.45	n/a	566	6,016
5.16	54.97	n/a	268	6,284

Culvert / Orifice Structures Weir Structures [B] [A] [B] [C] [D] [A] [C] [PrfRsr] = 12.00 0.00 0.00 0.00 = 4.00 0.00 0.00 0.00 Rise (in) Crest Len (ft) Span (in) = 12.000.00 0.00 0.00 Crest El. (ft) = 53.51 0.00 0.00 0.00 No. Barrels = 1 0 0 0 Weir Coeff. = 3.33 3.33 3.33 3.33 = 49.81 Weir Type = Rect Invert El. (ft) 0.00 0.00 0.00 = 1.00 0.00 0.00 0.00 Multi-Stage = No No No Length (ft) No 0.00 n/a = 1.00 0.00 Slope (%) = .013 N-Value .013 .013 n/a Orifice Coeff. = 0.600.60 0.60 0.60 Exfil.(in/hr) = 0.000 (by Contour) Multi-Stage No No No TW Elev. (ft) = 0.00= n/a

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	CIv A cfs	Clv B cfs	CIv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	49.81	0.00				0.00						0.000
0.52	269	50.33	0.27 oc				0.00						0.265
1.03	835	50.84	1.04 oc				0.00						1.044
1.55	1,540	51.36	3.80 oc				0.00						3.805
2.06	2,323	51.87	4.73 ic				0.00						4.729
2.58	3,143	52.39	5.45 ic				0.00						5.453
3.10	3,963	52.91	6.09 ic				0.00						6.092
3.61	4,746	53.42	6.67 ic				0.00						6.670
4.13	5,451	53.94	7.20 ic				3.73						10.93
4.64	6,016	54.45	7.70 ic				12.22						19.91
5.16	6,284	54.97	8.16 ic				23.50						31.66

Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

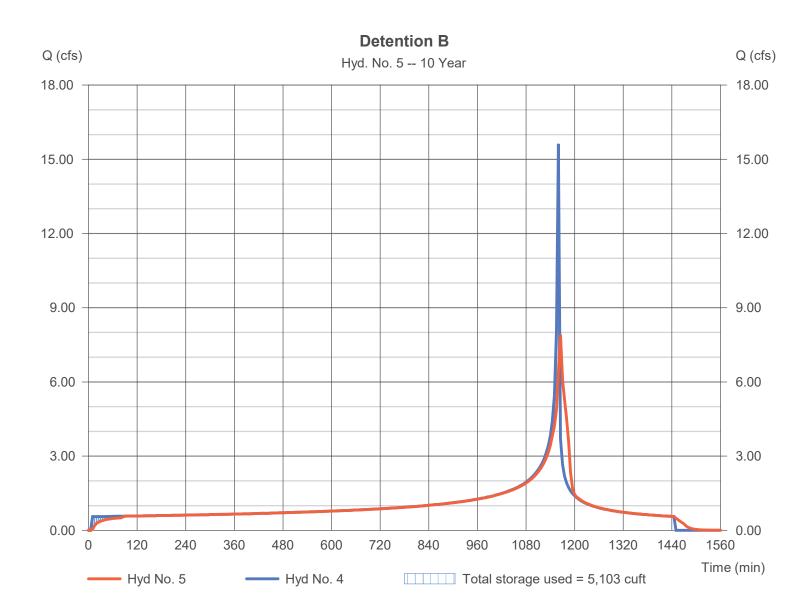
Tuesday, 07 / 21 / 2020

Hyd. No. 5

Detention B

Hydrograph type Peak discharge = 7.909 cfs= Reservoir Storm frequency = 10 yrsTime to peak = 1165 min Time interval = 5 min Hyd. volume = 91,197 cuft Inflow hyd. No. = 4 - Prop DA-2_10yr Max. Elevation = 54.20 ft= Detention B Max. Storage = 5,103 cuftReservoir name

Storage Indication method used.



Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

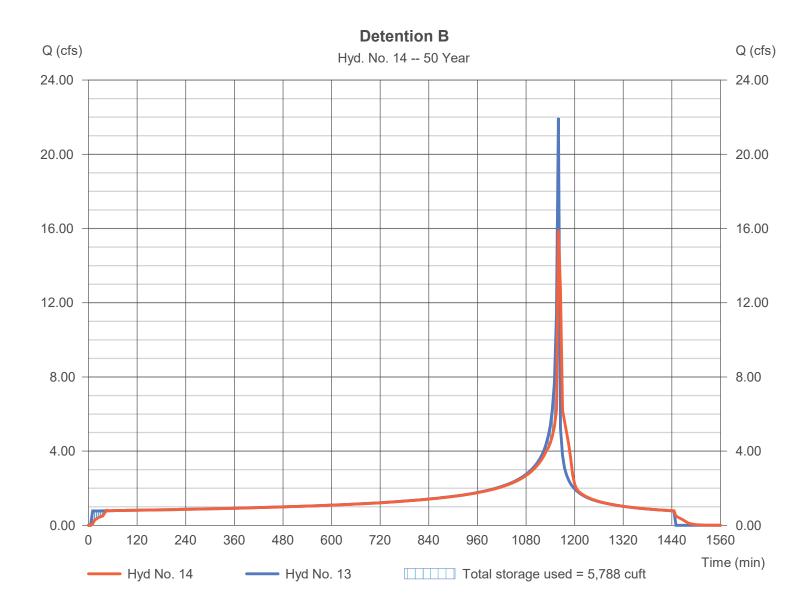
Tuesday, 07 / 21 / 2020

Hyd. No. 14

Detention B

Hydrograph type Peak discharge = 15.90 cfs= Reservoir Storm frequency = 50 yrsTime to peak = 1160 min Time interval = 5 min Hyd. volume = 127,962 cuft Inflow hyd. No. = 13 - Prop DA-2_50yr Max. Elevation = 54.97 ft= Detention B Reservoir name Max. Storage = 5,788 cuft

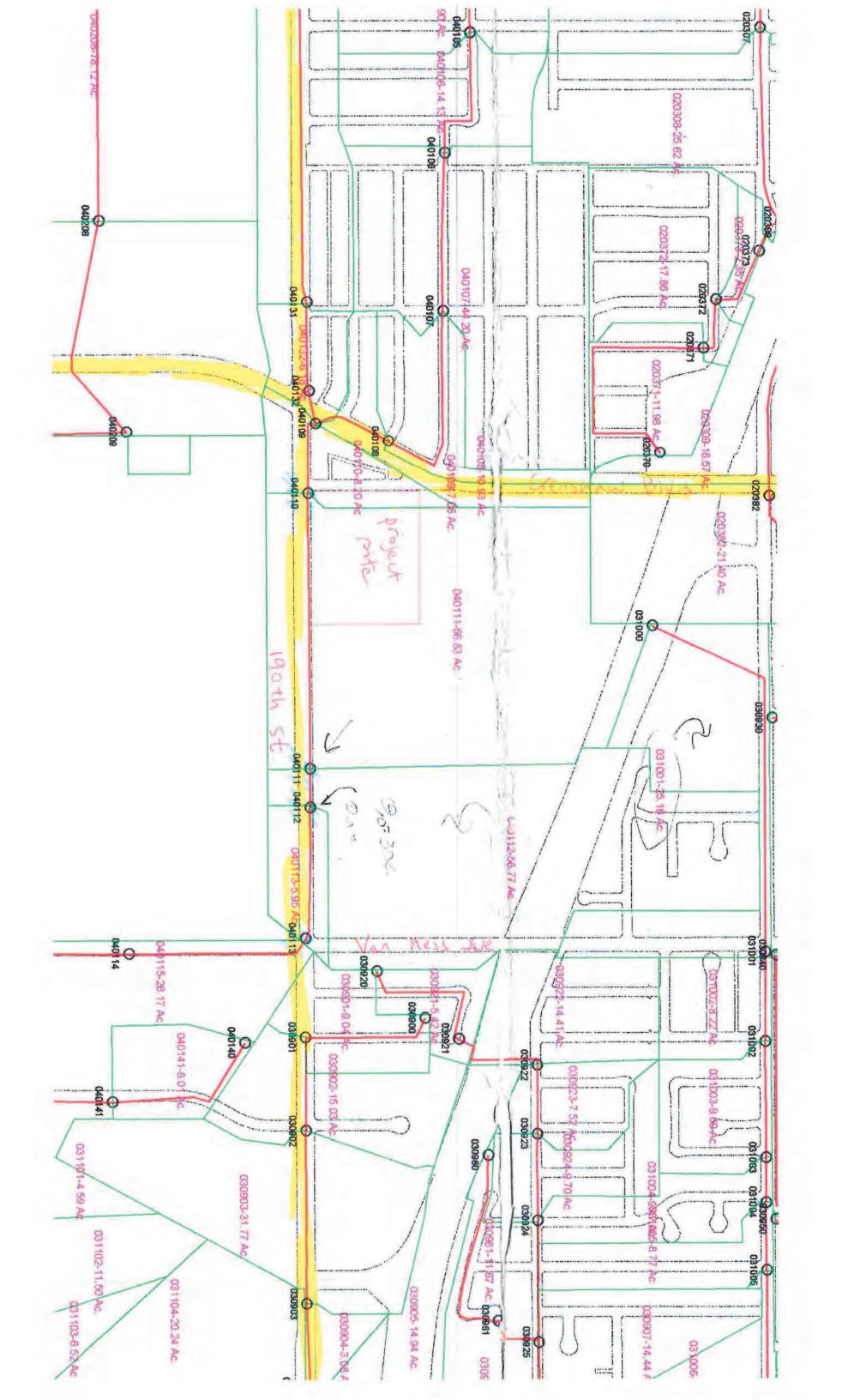
Storage Indication method used.



SECTION 5.0

Storm Drain Master Plan Tributary Areas Q and Pipe Capacity Data





-			-		-	040341-040342 40341	10		-	040323-040309 40323	040322-040323 40322	. 1	1.8		-	-500	37.0	-	-	040304-040305 40303	7	1	1	040126-040127 40126	040125-040126 40125	1	1		10	Æ.	Jo.	040121-040122 40121	100							040166-040167 40166	040165-040166 40165	1	040177-040177 40175	1	040162-040163 40162			51			040155-040156 40155		040153-040153 40152						6.1		040114-040115 40114	1			040110-040111 40110
		-			40344		10	1		40309		-			1			-		40004	1	1		40127	-	1	1				-	1	1	40172	1	100			. 1		40166	1	1	10	1	1		3	1			10	2. 40153		8 40119	100									40110 40111
669644.9 04	1850109 04	213994.8 04	762707.9 04	04	140001004	422581,1104	483305.7 04	815430.3 04	764548.9 04	1256520 04	1122603 04	317482.1 04	270237.5 04	390515.5 04	168529.1 04	04	04	04	1624597 04	20/21/204	120223,5 04	04	52296202.52	195965.5 04	1515313 04	324446.7 04	840910.9 04	2752924104	1453433/04	261926,2104	760760,2104	104	631317.2 04	389352.1104	189553 04	5544397 04	2	1558008 04	176192.5104	713873.9 04	158018 5 04	297027.1.04	123251.7-04	328036,9,04	620491.3104	219135.04	24	467480.4.04	.04	04	404155 2 04	60.8.120689	1700882:04	2497093.04	04	11149700.04	84118.82 04	1140292.04	04	349321.8:04	04	259013.1 04	2473730 04	2910148 04	357373 04
03	03	23	03 03	03 5	2 6	03	03	03	103	03	103	03	03	03	03	103	103	03	2 5	03	103	103	01 Total	101	01	01	101	101	01	107	01	9	101	101	01	101	9	01	01	0.00	2.9	2	0,	01	101	:01	01	101	0	01	2	01	:01	01	0,1	01	01	01	01	01		1	1	01	1
84.38	87.77	97.49	94.86	98 41	90.84	106.76	79.87	99.39	74.47	79,07	92.3	102.11	78.75	75.53	77.8	86.26	92.58	38 95	76.00	91.72	92	94		36.67	34.77	45	47.65	48.06	48.67	48.54	47.21	48.54	46.94	51.18	52.02	45.43	51.93	53.03	46 03	62 92	18.70	67.92	69	67.75	68.11	69.48	51.16	50.34	53	56	600	68	69	70	57	58.3	58.6	59	58.3	58.41	59.2	59.5. 59.2 1048	60	60.5	60
82,29	84.38	83.93	97.49	94 86	80.08	98,94	73.5	79.87	74.61	74.47	79.07	92.3	74.47	78.75	75.53	77.8	BB 25	82.00	02.00	86.92	91.72	92		50	36.67	34.77	34.77	47,65	48,06	48.67	48.54	48.67	46.54	46.94	51.18	46.94	46.54	51 93	53.03	48.03	60 00	18.70	67.92	67.81			51.93	1	50	-	SC SC				51.16			58.6			S	59.2	59	500	60
1026	606	692	1100	480	8/2	368	241	749	40	477	546		I					207						405		1063	923	598	1106	209	406	843							354			242							507								3 1101				9	2 10	7	0 0	T CHANGE
0.002		7	0.0073		T		0.0264				0.0242	1			0.004	1	T	0.0000											0.0006	9000.0-	6 -0.0033							0 0007	0			1							0.0002	1	ľ	n			315 0.0185					460 0,0002	1			0.0000	ייי וכיי כיייי
40				0.2				N		E		T	1	1				T		2.25				9 11.5					5. 35		3: 2.5			I				1	n u	1	-	N		1	1		cu	1	305	1		3.7		33 3.75	-	12 25		1				7	07 709	200	O DOOS OF CHARLE PIPENC
	4	-	-	1	-	-	-3	-	00		4 -	-	-	-			-			=	-1	0		7	7	1	9	9	9	1	1	9	12	3	1	1	0 -					1	, ma	4	1	11	9	-			-	1	1	_	9	9	7	4	1	1	4 6	0	0 0		PIPERU WI
0 0	0 0	2 9	0	2	0	0	0	0	0	0 0	2 0	2 0	0 0	0	0 0	0	2 5	0	0	0	0	0		0	0	0	0	0	0	0	0	0,	0	0	0 0	0	0 0	0		0	0		0	0	0	0	0	0,0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9 5		SIDESTORE
0 00									T	10		1		0 98	-	2 6		1		0 38			1	0 1339		1		0 1282	0. 120	0	0	0 112	0 25	0	0	1	1	0	1		0			i	1	4	0 7		0 1	0	0 1	0 1	0	0	0 6		1	1	0	0	0 0	0 0	0 0	0	NINNAM
		132			1				2 287		67					I				8 44				1536	8 1533	4 30	1	2 1468	1383	7 50	87 41	8 1293	7 255	41	15 17	1	100	1	1	1	72 95	28 32		1	40 47	1	909 908	2000	/1 196	73 197	66 189		3		638 738			384 445	21 24		368 422	371 426	346	223 254	אר עצטטאר
	1						1	52 80		120 7		7 891								50 8		27 9	-	-1	1	32	1	7	1	63	1	-	1	45	522	-		1	1		111	35	13	1	52	-	1	622	-	1	216			-	827	i		500	27	33	485	48/	398	294	COULAR BALL
	6/ 60.00			D	93.45 91				1	83.14										83.52 7		90.61 8	-		1			-	1	1	-	1	37.63	1	47 77	-	-	1		17	55.58	57.82	53.98	1	61.73	-	1	48.06		1	1		-		38.01	40.25	42 45	10.64	40 67	53 22	40,09	48.19	48,63	51.78	IGL U BALL
83.38 32					91.61 0			0.29	12.00		83 /4, 72		73.81 0						78.22 0			86.41 36		27 69 0	0	1	I	T	-	1	35.85	33 98	587	T	37.02 0		1	39.68 0	41.82 0	48.13 0	28			55.58	58 54 46	1	37 47	1		1	12		65.28 60	71	37.38	1	1	43.55	49.67 32	1	44.08 5	46.69 80	48.19 80	48.63	STW
75.7 132	137.1 238		0	0	0	0	0	0 0	0	0	45.6 101		0	0	0	0	0	0	0	52,1	15.8	693	-	0 0	711 :2.40	3 0	2 0	2 6	2	0	3	1	29.1 02	24	0			0	0	0	0	24.2	45.7	0	50 B	n c	-	34.9	41.6	42.5	44	41	38,5	30.5		10,0	100.1	0	2 15.7		2 19.3		0 24.4	0 0	D QST25 Q
9 46.4	.7 108,4		1	0 62.3		4		0 802.8		T	1	0 50.2		0 105.9			0 15.4		0 35.9	12 27.1	34 108	04	0.080.0	1	1	20	0 000	T	0.021	100.	1 35	7.140		1	0 154.9	1		0 142.6	0; 73				1	0 38 4	KE 2 20 9	1	T	76.4 66.4		N	4	89.7 97.3	l V		0.4222 O		200 445.2		1	A	1 00	28.6 413	52.6 496	0 35	ST50 PIPEC
	108.4 2			62.3	30.9	17.1	50.0	17		39.8				1			15.4	15.4	35.9	27.1	100	2	0.080	298.8	1.	1	1	4070	1	400.	7	7.166	7.80	1	15			13		*		1	4	38.4		1		4 66.4			5 115.5	3 97.3		*	0.4272					2 44	o ch	5 413.5	8 496.8	358	1010101 d
1		4					21.0 21	00		39.8 39										79.2: 13	T			85 8.865	1		1-	1	1		10	341.7	4	1	154.9 15		188.9 18	142.6 14	73	73	73	71.5	82.5	1	1	1	1	1	177.4 2	1					27 0.4777	3				1		7	521,2	-	Q25TOT Q5
179.3 12 0						17 1 18			40			73			1	1		1	1	39 2 11			1	598.8 739		04	04	04	0.5	0.11	204	1.7	1.4	8.7	4.9.25	3204	188,9 0	142.6 0	73 35	73 6	73 0	93	23.1	38 A 10	07.5 6	3204	209.4 0	142.8 105	26.9 35	208.7 57	11.9 50	187.70	180 8 37	1623	8.6777	376.9 45	505.4 0	18.4.3	45.9 9	488.9	451.3 0	142.1 0	549.5	-	GSOTOT Q10D
0		4			63 79				51 74	27 39		93 118							1	0 0			937 1146	934 1144						5 13					44 74	1				29 47	22 38	* *	10 30		0 0	1		94 80	1	39 16	-	51 31	-	-	-	84 1		6 9	0 0		0 38				Q10DEF Q25DEF Q50
12	89	46	0 :	47	700	39	24	0	74	38		118	108	2	32	2	33	3	4		2	13			9-						-					The same of the same of		The second second						-		-			(Comments and		-					129					co	Ch		The same of the same of	SODEF MAXO
12 10-Yr			00-11	50-Yr	20-Y	50-Yr	50-Yr		50-Yr		0	30-Yr	3 50-Yr	7 50-Yr	8 50-Yr	23 50-Yr	36 50-Yr	50-Yr	1 50 V	4/10-Yr	2110-Yr		6:50-Yr	4:50-Yr	5:10-Yr	0	0	0	0	3.50-Yr	0	0	0	0.	74 50-Yr	0	34.50-Yr	13 50-Yr	83 50-Yr	47 50-Yr	10 V	3	30.50-Yr	10 10 Yr	6 10-Yr	0	₹0.50-Yr	₽25 10-Yr	35 10 Yr	57 10-Y	50 10 V	3/ 10-Yr	0	0	0	129 50-Yr	1 50-Yr	9'50-Yr	9 10-Yr	0	38:50-Yr	45 50-Yr	0 1	O WOOD	SUNAVIT SE
12	I		-		70				74		1					1		1		4			1146	11144	5			7		13			-	A Section of the last of	74	******	34	13	8:4	47	20		30	10	6		20	105	35	57	5 6	37		-		129	-	9	9		38	45		T TONNE	DABAIL I
2.00					27.25				3.25			5.25	5.00	225	275	250	275	275	7.70	1.25	2.25		12.50	12.50	1.25					1.75				and the second	3.50	-	275	200	375	200	375	T	2.50	1.75	1.50		2.25	4.00	275	3 25	3000	2.75				5.50	0.75	1.75	1.75		3.50	3.7%	-	באות האחתונים	d I Word CE
58 3.50	1	- 1	1	917	3.00				139 4.00				11			- 1		л.		24 2.25			1745 14.50	100						49 3.00		1-1			228 5.2		- 1	. 1	1	120 4.25	4		1	40 3.00			229 5.25	1	- 1		1	4	1			469 9.		j			451 9		-	בנה מפנינת מ	ם הומים היי
0	1	1			6				0		-1	1		1		30	1	1	1	5	11		50 .0		10.1					.0			-		25 .0	1	1	1	1	200	1	-		00 0		1	25 0	1			1	1	1			9.00 0		50 0		1	O ONE		1	CHUDIA ADVI	Charles of the form
3.50	1	21			3.00 0	12			14.00 0	1	- 1-	-	1		41.	- 1	16.	4	100	:2.25 :0		- 4	14.50 0	- 4	- V	-	***	-	1	3.00 0			t is senior		5.25 0	40	1	1	0	4.25	E	× × ×	*	3.00	7	- 0	5.25	4	0	1	1	1	1 0		1	9.00	-	1		1		8		היייטים והיייטים והיייטים במשל היייטים בפרנע מפרנטא Auvocal אבייטוא אייטוא אייטוא אייטוא אייטוא אייטוא אייטוא אייטוא אייטוא א	The state of the s
1	1																1									-							1	-	1		-	10	-	0	-	1	0	0	0	or other section of	0	2 0	3 0	1	1	0				0	0	0			0 0	n	-	KECWID	

CMB_SD_D.XLS