Appendix I

Hydraulic Model Evaluation



**CITY OF SANTA ANA** 

#### **TECHNICAL MEMORANDUM**

## HYDRAULIC MODEL EVALUATION FOR THE VILLAGE PROJECT

**Final** June 2024

Prepared For:



Prepared By:



Smart Planning Our Water Resources



June 26, 2024

City of Santa Ana 215 S Center Street Santa Ana, CA 92703

Attention: Christine Le, Assistant Engineer

#### Subject: Hydraulic Model Evaluation for the Village Project

The objective of this technical memorandum is to present the findings of the hydraulic modeling analysis conducted for the Village Project development (project). The project is located at the northeast corner of Bear Street and Sunflower Avenue. The boundaries of the project encompass the areas bordered by South Plaza Drive to the east, Bear Street to the west, the multi-family housing communities to the north, and Sunflower Drive to the south. **Figure 1** illustrates the existing water infrastructure and the location of the project. The hydraulic analysis was performed to determine the required size for the water mains to serve the development and assess the impacts of the development on the system operations.

#### 1.0 SUMMARY OF RESULTS

The hydraulic analysis was performed for maximum day demands, peak hour demands, and maximum day demands plus a 3,000 gallons per minute (gpm) fire flow to determine the impacts on the existing system and to recommend system improvements for the project. The hydraulic analysis indicated only minor impacts, summarized as follows:

- During peak hour demands only minor pressure reductions of 2.8 pound per square inch (psi) were observed and no major impacts to the existing system pipelines were evident.
- During maximum day demands plus 3,000 gpm fire flow, the existing system is capable of providing pressures above 20 psi and pipeline velocities under 7 feet per second (fps).

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 An extended-period simulation (EPS) scenario was performed to determine if the project demands impacted the existing system operations. The analysis indicated no major impacts or deficiencies were caused by the project.

#### 2.0 HYDRAULIC MODELING ASSUMPTIONS

The City's existing hydraulic model (WaterCAD) was used to simulate steady-state and extendedperiod simulations (EPS) to evaluate the development impacts. In order to reflect the recent water conservation efforts, the system water demands were updated as follows:

- Average Day Demand (ADD): 30.2 MGD = 20,947 gpm
- Maximum Day Demand (MDD): 60.3 MGD = 41,894 gpm
- Peak Hour Demand (PHD): 105.6 MGD = 73,315 gpm

These values are consistent with the annual demand documented in 2020 Urban Water Management Plan (2020 UWMP) and the peaking factors for MDD and PHD in Santa Ana Design Guidelines.

To account for the recent operational changes and newly constructed facilities, the hydraulic water model was updated as follows:

- Groundwater Wells 16, 22, 32, 42, and 43 were inactive.
- Interconnections SA-3, SA-4, SA-5, and SA-7 were inactive.
- Pump curves were updated based on the pump test reports provided by Santa Ana staff in December 2022.
- The storage reservoir levels were updated to the Tank Operational Ranges provided by Santa Staff in January 2023.
- The operational controls of reservoir pumping stations were updated to the SCADA- Reservoir Related Setpoints provided by Santa Ana staff in January 2023 (Appendix A).

The status of the water system wells, pump stations, tanks, interconnections, and pressure regulating stations in the hydraulic model is documented on Table 1.

#### 3.0 ANALYSIS CRITERIA AND ASSUMPTIONS

The water system performance criteria used for the project were obtained from the City of Santa Ana Design Guidelines, which were published in November 2020. Table 2 documents the development summary, peaking factors, unit factors, required fire flows, pipeline performance criteria, and system pressure criteria.

The project's fire flow requirement of 3,000 gpm at 20 psi for a duration of 3 hours was assumed based on the requirements provided in City's 2017 Water Master Plan. The actual fire flow requirements will be determined by the Orange County Fire Authority (OCFA) and may affect the final design requirements.

The demands of the development were estimated based on the project land use areas and the unit factors found in the City's criteria. The project land use area details are provided in Appendix B for reference purposes. Average occupants per unit value was set 2.41 capita/du based on Santa Ana 2045 General Plan's persons per household assumptions. Table 3 documents the existing and estimated demands for land use types within the project.

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The following project demands were estimated and incorporated into the hydraulic model:

- Average Day Demand (ADD):
- Maximum Day Demand (MDD): 690 gpm
- Peak Hour Demand (PHD): 1,207 gpm

#### 4.0 STEADY STATE HYDRAULIC ANALYSIS SCENARIOS

The hydraulic analysis will be performed for 3 scenarios to determine the recommended pipelines for the development of the Village Project:

345 gpm

- Scenario 1: Maximum Day Demands
- Scenario 2: Peak Hour Demands
- Scenario 3: Maximum Day Demands + 3,000 gpm fire flow requirement.

#### 5.0 STEADY STATE HYDRAULIC ANALYSIS RESULTS

Steady-state evaluations during maximum day demands (MDD), peak hour demands (PHD), and maximum day demands plus fire flow (MDD+FF) were performed for each scenario to identify deficiencies caused by the project.

#### Scenario 1: Maximum Day Demands

According to the hydraulic analysis, the service pressures drop approximately 0.5 psi during the maximum day demand scenario with the addition of the project, decreasing from 92.0 psi (existing) to 91.5 psi (buildout). However, both the pre- and post-development service pressures remain within the City's criteria, maintaining a minimum pressure of 40 psi. Figure 2 documents the existing and buildout system pressures under maximum day demands.

#### Scenario 2: Peak Hour Demands

During this scenario, the service pressures drop approximately 2.8 psi with the addition of the project, dropping from the existing 82.7 psi to 79.9 psi at buildout. Despite this drop, the minimum pressure requirement of 40 psi set by City's criteria is still satisfied both before and after the project development. Additionally, the hydraulic analysis indicates that during the peak hour demand scenario, the pipeline velocities will maintain the City's criteria, which allows for a maximum velocity of 5 fps under peak hour demands. **Figure 2** documents the system pressures and pipeline velocities observed during the peak hour demand (PHD) scenario.

#### Scenario 3: Maximum Day Demands + 3,000 gpm Fire Flow

The hydraulic analysis indicates that during this scenario, the system will maintain fire flow residual pressures above 20 psi and pipeline velocities under 7 fps. The critical fire flow location is observed at the 12-inch pipe segment along Bear Street and the pipeline velocity during a fire flow event will be approximately 6.2 fps with a residual pressure of 86.8 psi. The fire flow results, and the pipeline velocities are documented in Figure 3.

#### 6.0 WATER SYSTEM OPERATIONS EPS HYDRAULIC ANALYSIS SCENARIOS

In order to assess the overall system impact of adding the Village development to the domestic water system, City staff requested the extended-period simulation (EPS) scenarios to be conducted under the maximum day demand (MDD) conditions. The scenarios are as follows:

- Existing: Existing operations
- Buildout: Existing operations plus the Village Project

#### 7.0 WATER SYSTEM OPERATIONS EPS HYDRAULIC ANALYSIS RESULTS

The hydraulic analysis was performed for buildout scenario and compared to the existing operations. The results of the EPS analysis provide valuable insights into the system's performance with the addition of the project.

The EPS analysis results/impacts for buildout condition are summarized in **Table 4.** This table documents the change in pressures and increase or decrease of production at each of the wells, booster stations, pressure reducing valves and turnouts (interconnections). The EPS exhibits for each facility are provided in **Appendix C**.

The hydraulic analysis indicates that the development of the project will not have major impacts on the existing system operations. The EPS modeled results are documented as follows:

#### **Development's Demand and Pressure**

The project adds an additional demand of 690 gpm to the system's maximum day demand. The average pressure in the MDD EPS scenario indicated a drop of approximately 0.6 psi, decreasing from 91.5 psi (existing) to 90.8 psi (buildout).

#### **Groundwater Wells**

Well-37 production increased from 1,345 gpm (existing) to 1,435 gpm (buildout), resulting in additional production of 90 gpm flow. Well-41 production increased from 536 gpm (existing) to 628 gpm (buildout), resulting in additional production of 93 gpm flow.

#### **Booster Stations**

South Booster Station outflow increased from 964 gpm (existing) to 1,356 gpm (buildout), resulting in additional outflow of 392 gpm.

#### **Pressure Sustaining Valves**

There was no significant impact to pressure sustaining valves during the EPS simulation with the addition of the project.

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#### 8.0 ANALYSIS RECOMMENDATION SUMMARY

The hydraulic analysis indicates that with the addition of the Village Project, the existing system does not require pipeline improvements based on the performance criteria documented on **Table 2**. During maximum day demands plus 3,000 gpm fire flow scenario, the residual pressures maintain above 20 psi, and the highest pipeline velocity of 6.2 fps is observed at the 12-inch pipe segment along Bear Street. During peak hour demands, the pipelines velocities remain under City's velocity criteria of 5 fps, and the service pressures indicate 79.5 psi which is only approximately 2.8 psi decrease from the existing conditions.

The extended-period simulation results indicated that with the addition of the Village Project, there were no major impacts to the system operations as documented on Table 4.

Sincerely,

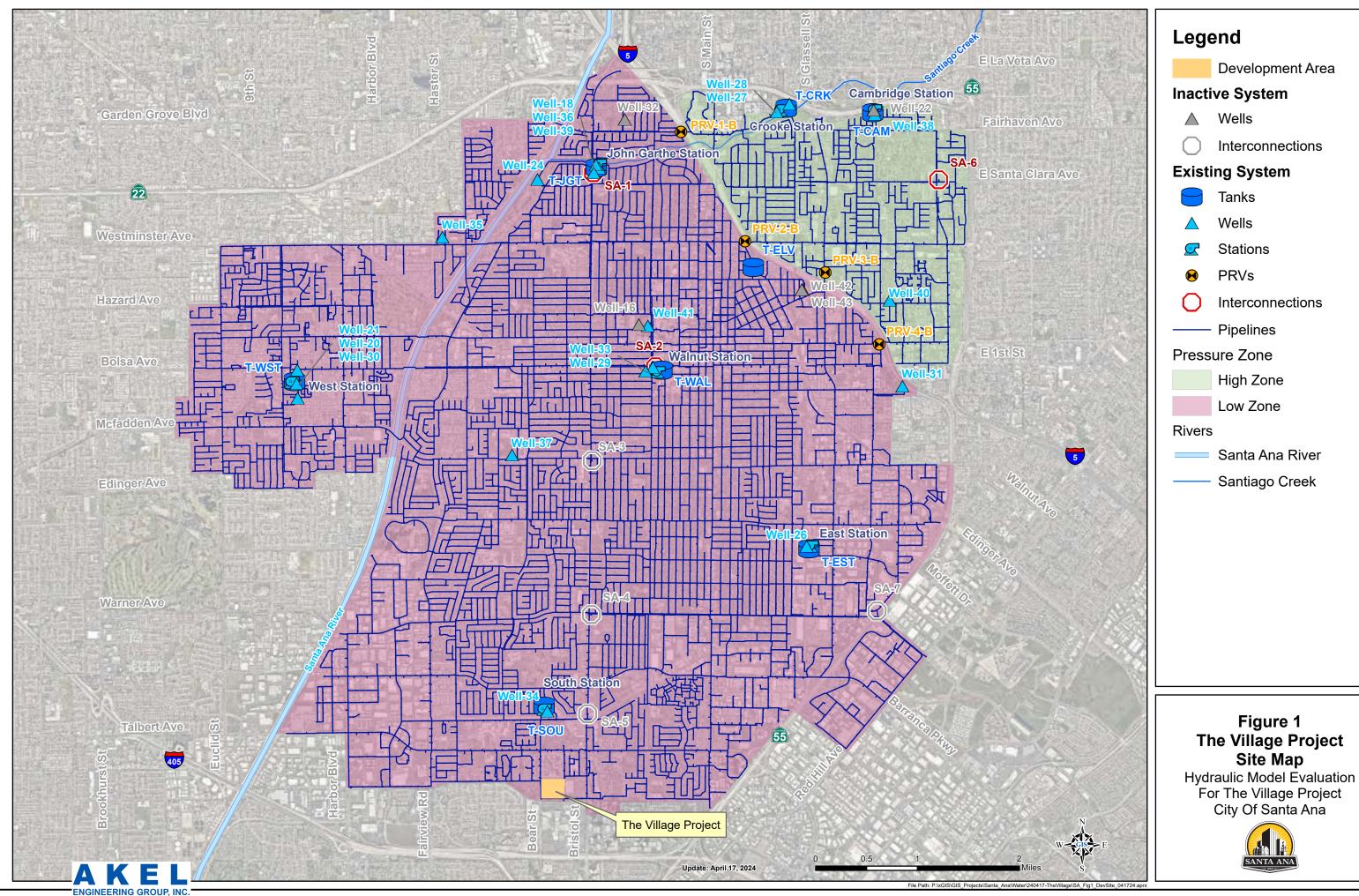
AKEL ENGINEERING GROUP, INC.

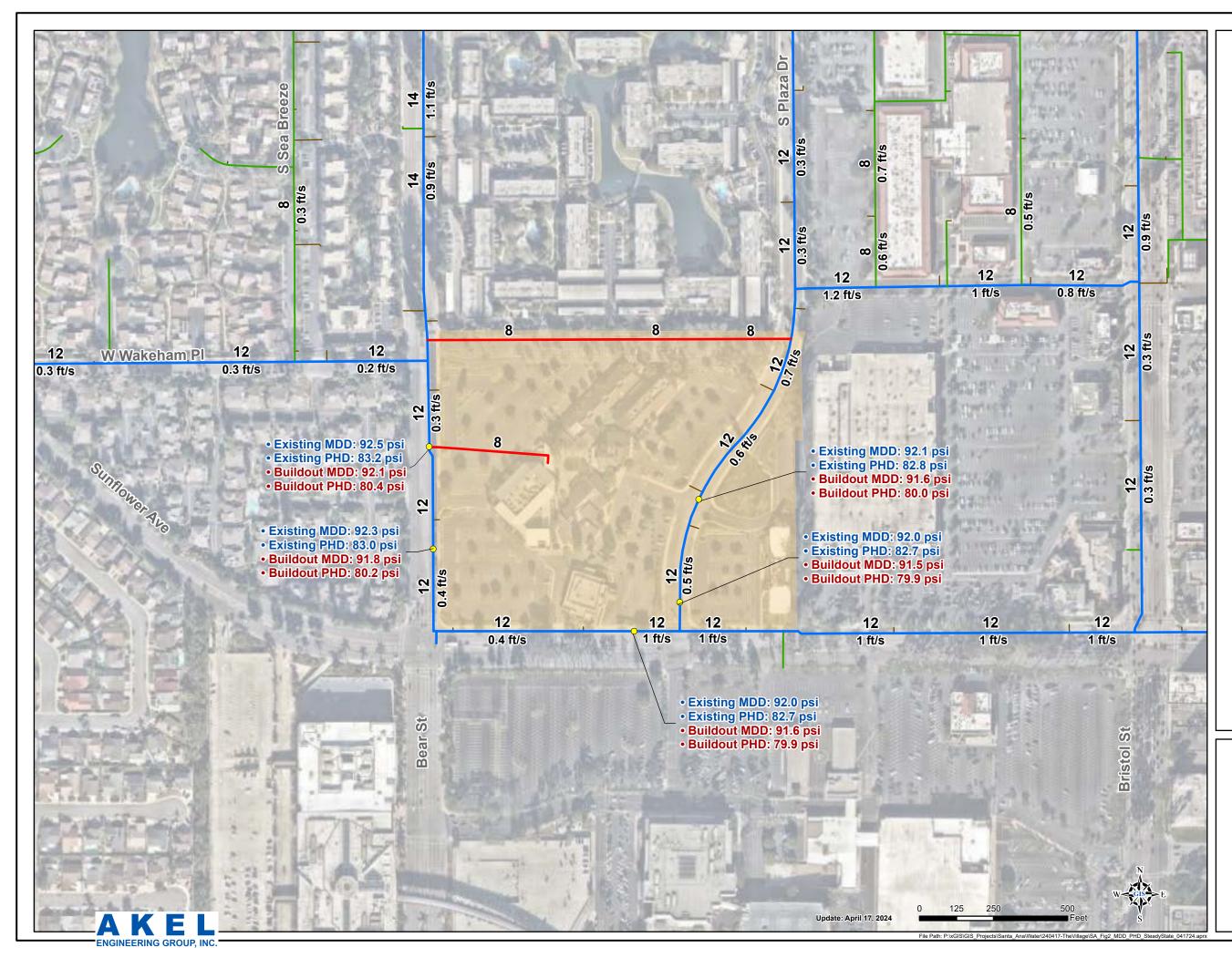
Tony Akel

Tony Akel, P.E. Principal

Hydraulic Model Evaluation for the Village Project

## **FIGURES**





#### Legend

#### Hydraulic Analysis

1.2ft/s PHD Velocity

- Pressure Junction
- ---- Proposed Private Pipelines

#### **Existing System**

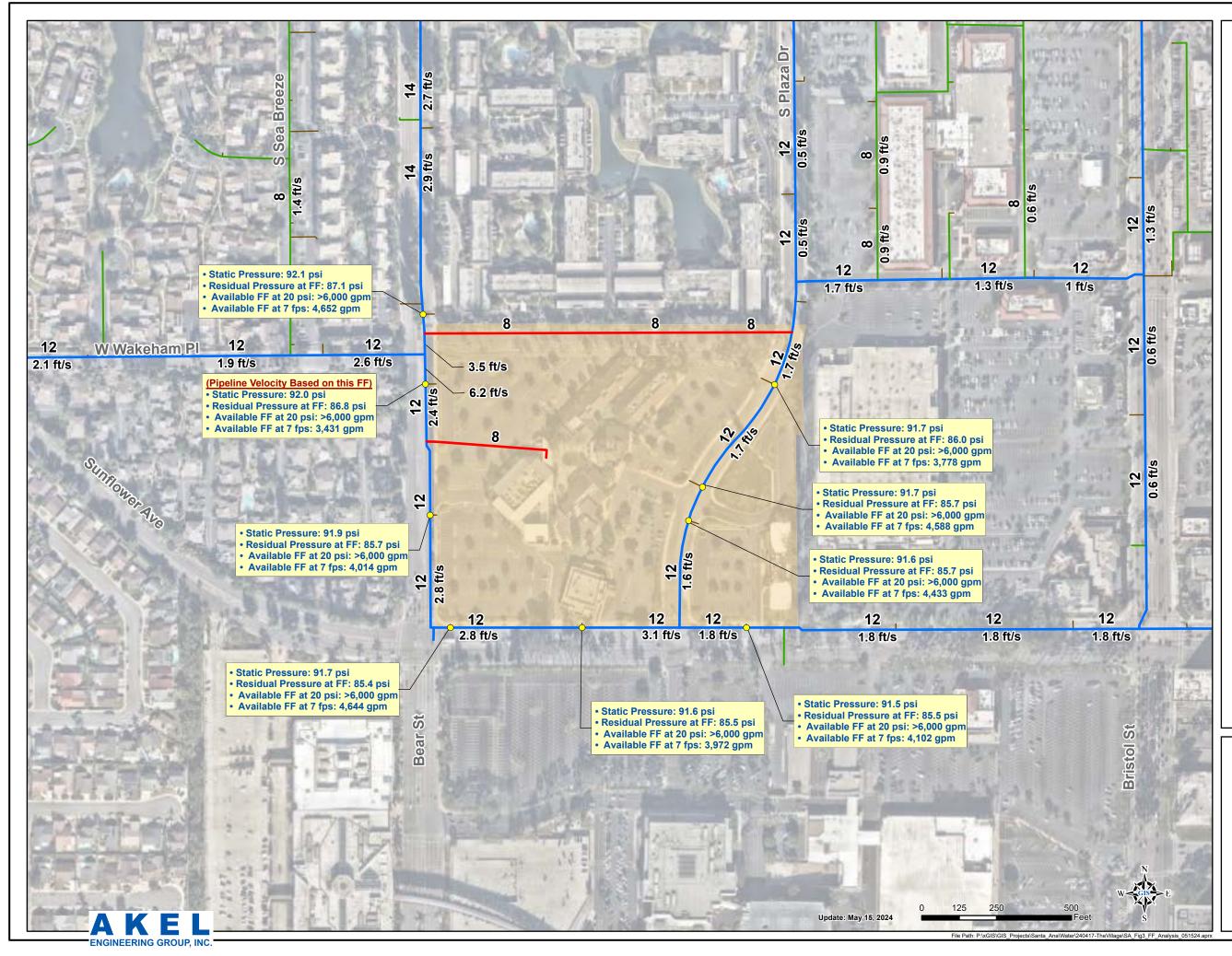
Pipelines by Diameter

- 6" or Smaller
- - 12" or Larger

Development Area

Figure 2 MDD and PHD Steady State Hydraulic Model Evaluation For The Village Project City Of Santa Ana





#### Legend

## Hydraulic Analysis 1.2ft/s Velocity at 3,000 gpm FF • Fire Flow Junction **Proposed Private Pipelines Existing System Pipelines by Diameter** 6" or Smaller 8" - 10" 12" or Larger Development Area Note: • Modeled Scenario: MDD + Fire Velocity Constraint: 7 fps Pressure Constraint: 20 psi Design Fire Flow: 3,000 gpm Figure 3 3,000 gpm Fire Flow **Analysis Results** Hydraulic Model Evaluation For The Village Project

SANTA ANA

City Of Santa Ana

Hydraulic Model Evaluation for the Village Project

## **TABLES**

#### Table 1 Modeled Facility Inventory

Hydraulic Model Evaluation for The Village Project City of Santa Ana

Facility	Pressure Zone	Status	Facility	Pressure Zone	Status
Groundwater V	Vells		Pump Stations		
Well 16	Low	Inactive	Cambridge	High	Active
Well 18	Low	Active	Crook	High	Active
Well 20	Low	Active	East	Low	Active
Well 21	Low	Active	John Garthe	Low	Active
Well 22	High	Inactive	South	Low	Active
Well 24	Low	Active	Walnut	Low	Active
Well 26	Low	Active	West	Low	Active
Well 27	High	Active	Storage Tanks		
Well 28	High	Active	Cambridge	High	Active
Well 29	Low	Active	Crook	High	Active
Well 30	Low	Active	East	Low	Active
Well 31	Low	Active	Elevated	Low	Active
Well 32	Low	Inactive	John Garthe	Low	Active
Well 33	Low	Active	South	Low	Active
Well 34	Low	Active	Walnut	Low	Active
Well 35	Low	Active	West	Low	Active
Well 36	Low	Active	Interconnections	5	
Well 37	Low	Active	SA-1	Low	Active
Well 38	High	Active	SA-2	Low	Active
Well 39	Low	Active	SA-3	Low	Inactive
Well 40	High	Active	SA-4	Low	Inactive
Well 41	Low	Active	SA-5	Low	Inactive
Well 42	Low	Inactive	SA-6	High	Active
Well 43	Low	Inactive	SA-7	Low	Inactive
Pressure Regula	ating Stations				
PRV-1	Low	Active	PRV-3	Low	Active
PRV-2	Low	Active	PRV-4	Low	Active

ENGINEERING GROUP, INC. Notes:

1. The modeled elements are validated by Santa Ana Staff in December 2022.

#### Table 2 The Village Project Development Hydraulic Analysis Criteria

Hydraulic Model Evaluation for the Village Project City of Santa Ana

I.1       Development Summary       Proposed Land Use: Mixed Use (Residential, Commercial, Office)       Residential - 1,583 du Commercial - 80,000 sf Office - 300,000 sf       Development information extracted fr Specific Plan Draft August 2023, Table 3-1.         I.2       Project Demands Estimates       Peaking Factors: Average Day Demand Maximum Day Demand (2.0 x ADD) Peak Hour Demand (3.5 x ADD)       Unit Factors: 1. MFR: 120 gpd/capita 0.000 sf       Water demand factors and peaking factors: 0.000 sf         I.3       Fire Flows       Minimum Required Fire Flow and Duration       3,000 gpm at 20 psi residual for a duration of 3 hours       It should be noted that, at the time of medeling effort, the project-specific fin requirements based on land use proviv Water Master Plan. The actual fire flow and any affect the final design	No. Item	ltem	Item Description	Comments
I.1.       Development Summary       Proposed Land Use: Mixed Use (Residential, Commercial, Office)       Commercial - 80,000 sf Office - 300,000 sf Open Space (Public) - 158,300 sf Landscape - 90,000 sf       Development information extracted fr Specific Plan Draft August 2023, Table Landscape area received from City stal         1.2       Project Demands Estimates       Peaking Factors: Average Day Demand Maximum Day Demand (2.0 x ADD)       1. MFR: 120 gpd/capita (Occupants per Unit: 2.41)       Water demand factors and peaking fact Design Guidelines and Standard Drawi Sever Facilities, City of Santa Ana, No Average Occupants per unit value extr Ana 2045 General Plan, persons per br assumptions.         1.3       Fire Flows       Minimum Required Fire Flow and Duration       3,000 gpm at 20 psi residual for a duration of 3 hours       It should be noted that, at the time of modeling effort, the project-specific fil requirements based on land use provi Water Master Plan. The actual fire flow be determined by the Orange County I (OCFA) and may affect the final design MDD +FFF Max Velocity:       Extracted from City of Santa Ana Desig Guidelines Published Nov. 2020			Gross Land Area: 17 ac	Project Area extracted from The Village Specific Plan - Draft August 2023, Table 3-1.
1.2       Project Demands Estimates       Peaking Factors: Average Day Demand (2.0 x ADD) Peak Hour Demand (2.0 x ADD)       1. MFR: 120 gpd/capita (Occupants per Unit: 2.41)       Design Guidelines and Standard Drawi Sewer Facilities, City of Santa Ana, Nov Average Occupants per unit value extr Ana 2045 General Plan, persons per ho assumptions.         1.3       Fire Flows       Minimum Required Fire Flow and Duration       3,000 gpm at 20 psi residual for a duration of 3 hours       It should be noted that, at the time of modeling effort, the project-specific fil requirements were not yet available. T infrastructure recommendations are b requirements based on land use provit Water Master Plan. The actual fire flow be determined by the Orange County II (OCFA) and may affect the final design MDD +fFF Max Velocity: S fps MDD +fFF Max Velocity:       Extracted from City of Santa Ana Design Guidelines Published Nov. 2020	1.1 Development Summary	Development Summary	Proposed Land Use: Mixed Use (Residential, Commercial, Office) Commercial - 80,000 sf Office - 300,000 sf Open Space (Public) - 158,300 sf	Development information extracted from The Village Specific Plan Draft August 2023, Table 3-1. Landscape area received from City staff (3/28/2024).
1.3Fire FlowsMinimum Required Fire Flow and Duration3,000 gpm at 20 psi residual for a duration of 3 hoursmodeling effort, the project-specific fir requirements were not yet available. T infrastructure recommendations are b requirements based on land use provid Water Master Plan. The actual fire flow be determined by the Orange County H (OCFA) and may affect the final design1.4Pipeline CriteriaPHD Max Velocity: S fps MDD +FF Max Velocity:Extracted from City of Santa Ana Design Guidelines Published Nov. 2020	1.2 Project Demands Estimates	Project Demands Estimates	Peaking Factors:1. MFR: 120 gpd/capitaAverage Day Demand(Occupants per Unit: 2.41)Maximum Day Demand (2.0 x ADD)2. Commercial: 2,500 gpd/ac	Water demand factors and peaking factors extracted from Design Guidelines and Standard Drawings for Water and Sewer Facilities, City of Santa Ana, November 2020. Average Occupants per unit value extracted from Santa Ana 2045 General Plan, persons per household assumptions.
1.4Pipeline Criteria5 fpsExtracted from City of Santa Ana DesigMDD +FF Max Velocity:Guidelines Published Nov. 2020	1.3 Fire Flows	Fire Flows	Minimum Required Fire Flow and Duration	It should be noted that, at the time of this hydraulic modeling effort, the project-specific fire flow requirements were not yet available. Therefore, infrastructure recommendations are based on the fire flow requirements based on land use provided in City's 2017 Water Master Plan. The actual fire flow requirements will be determined by the Orange County Fire Authority (OCFA) and may affect the final design requirements.
	1.4 Pipeline Criteria	Pipeline Criteria	<b>5 fps</b> MDD +FF Max Velocity:	Extracted from City of Santa Ana Design Guidelines Published Nov. 2020
Maximum static pressure:     Maximum static pressure:     Extracted from City of Santa Ana Design       1.5     Pressure Criteria     40 psi at PHD.     Published Nov. 2020       Minimum residual pressure:     20 psi at MDD + FF     Published Nov. 2020	1.5 Pressure Criteria	Pressure Criteria	<b>100 psi</b> . Minimum residual pressure: <b>40 psi at PHD</b> . Minimum residual pressure:	Extracted from City of Santa Ana Design Guidelines Published Nov. 2020

#### Table 3 Domestic Water Demands

Hydraulic Model Evaluation for the Village Project City of Santa Ana

				Development	Information					Model Inputs <sup>3</sup>		
Classification	Existing Units <sup>1</sup>	Proposed Units <sup>1</sup>	Average Occupants per Unit <sup>2</sup>	Water Duty Factor <sup>3</sup>	Existing Daily Water Demand	Proposed Daily Water Demand	Existing Annual Water Demand	Proposed Annual Water Demand	Change in Annual Demand (AFY)	ADD	MDD	PHD
					(gpd)	(gpd)	(AFY)	(AFY)	(AFY)	(gpm)	(gpm)	(gpm)
Residential Deman	ds											
Residential	0	1,583 du	2.41 capita/du	120 gpd/capita	0	457,804	0	513	513	318	636	1,113
Non-Residential De	emands											
Commercial	13.2 ac	80,000 sf		2,500 gpd/ac	33,000	4,591	37	5	-32	3	6	11
Office	0	300,000 sf		2,500 gpd/ac	0	17,218	0	19	19	12	24	42
Open Space (Public)	0.5 ac	3.6 ac		3,000 gpd/ac	1,500	10,902	2	12	11	8	15	26
Landscape Area	0	2.1 ac		3,000 gpd/ac	0	6,198	0	7	7	4	9	15
ΑΚΕΙ	-			Total	34,500	496,713	39	556	518	345	690	1,207
ENGINEERING GROUP, IN	IC.											4/4/2024

1. Data extracted from The Village Santa Ana Specific Draft Plan (August 2023), Table 1-1 and Table 3-1, and the project information received from City staff (3/28/2024).

2. Average occupants per unit value extracted from Santa Ana 2045 General Plan, persons per household assumptions.

3. Water duty factors and peaking factors based on Design Guidelines, City of Santa Ana, November 2020.

#### Table 4Water Facility Impacts

Hydraulic Model Evaluation for the The Village Project City of Santa Ana

Facility ID	Pressure Zone	Data Type	Existing EPS	Existing EPS Buildou	
1	2	3	4	Results 7	Changes 8
The Village Development			(gpm)	(gpm)	(gpm)
The Village The Village	Low	Demand Pressure (psi)	0 91.5	690 90.8	+690 -0.6
Groundwater Wells			(gpm)	(gpm)	(gpm)
Well 31 Well 35 Well 37 Well 41 Well 28 Well 38 Well 40	Low Low Low High High High	Outflow Outflow Outflow Outflow Outflow Outflow Outflow	1,286 1,916 1,345 536 2,497 0 0	1,288 1,929 1,435 628 2,496 0 0	+3 +13 +90 +93 0 0 0
Booster Sta	ations		(gpm)	(gpm)	(gpm)
John Garthe Walnut East West South Cambridge Crook	Low Low Low Low High High	Outflow Outflow Outflow Outflow Outflow Outflow Outflow	7,403 5,133 2,446 5,983 964 1 0	7,449 5,122 2,453 6,036 1,356 1 0	+46 -11 +7 +52 +392 0 0
ENGINEERING GROUP, I	NC.				4/18/2024

Hydraulic Model Evaluation for the Village Project

## **APPENDICES**

Hydraulic Model Evaluation for the Village Project

### **Appendix A**

### City of Santa Ana SCADA – Reservoir Related Set Points

SCADA – Reservoir Related Setpoints

# Cambridge

1	RTU-4 - Cambridge Reservoi	U-4 - Cambridge Reservoir and Pump Station						
RUN STAT. Booster 1 (75 HP) OFF Booster 2 (75 HP) OFF Booster 3 (75 HP) OFF Well 38 (350 HP) OFF	SWITCH POS. FAIL STAG AUTO NORMAL 1 AUTO NORMAL 3 AUTO NORMAL 2 MIT NORMAL	Stage 1 Stage 2 Stage 3 Well 38 VFD	STARTSTOP55Psi85Psi45Psi67.9Psi40Psi65.9Psi45Psi50Psi55Psi50Psi					
ANALOG VALUES M.O.L 16' Res. Level 5 FT Disch. Flow 0 CFS Bypass Flow .8 CFS	STATUS / CONTROLAutomatic ControlAUTOBypass ValveOnBPV Time StatusINACTIVE	Bypa Pressure Setpoint Open < Close > Start Time	POINTS ass Valve       64.9     PSI       10     FT       12     FT       255     (Military Time)       640     (Military Time)					
System Press66.6PSIWell 38 Flow0CFSWell 38 Press67.7PSICL2 Day Tank-12.8GALCL2 Residual.00PPMAUTO START/STOPLOCAL	Mid-Peak Tou Status INACTIVE On Peak TOU Status INACTIVE ENABLE MANUAL SPEED CONTROL SCADA MANUAL SPEED SETTING 7.5 %	Mid-Peak TOU Begin On Peak TOU Begin Low Level Shutdown Low Level Reset High Level Shutdown High Level Reset	Is & TOU Controls           999         End         999         MT           999         End         999         MT           3         4.99         16.4         16           3600         Second         Second         Second					

# Crooke

	RTU-3 - Crooke Res	ervoir and Pu	ump Station	Ja	7:51:59 an/11/2023 8:45:09 AM
RUN STAT.	SWITCH POS. FAIL	STAGE SEL.	STA	RT	STOP
Booster 1 (150HP) OFF	AUTO NORMAL	1	Stage 1 77		115 PSI
Booster 2 (150HP) OFF	AUTO NORMAL	2	Stage 2 45	PSI	98 PSI
Booster 3 (150HP) OFF	AUTO NORMAL	3	Stage 3 40	PSI	95 PSI
Well 27 (300HP) OFF	MANUAL NORMAL		Well 27 0	FT	0 FT
Well 28 (350HP) OFF	MANUAL NORMAL		Maximum C	operating	Level is 18 Feet
Well 27 NaClO Tank Level	126 Gallons		Well 28 0	PSI	0 PSI
Well 28 NaCIO Tank Level	223 Gallons		VF	D 8:	B PSI
			Fixed Spee	d 8(	) %
ANALOG VALUES	STATUS / CONTRO		BYPASS VA		
		Pres	sure Setpoint	85	PSI
Bypass Flow 0 CFS		AUTO Ope		7	FT
Res. Level 12.1 Ft		LOSED Clos	se >	12	FT
Disch. Flow 0 CFS		and a second sec	t Time	999	(Military Time)
System Press. 87 PSI	Mid-Peak TOU Status	ACTIVE Stop	o Time	999	(Military Time)
Well 27 Flow 0 CFS	On-Peak TOU Status	ACTIVE	<u></u> T.O.U.	SETPOIN	ITS
Well 28 Press. 82.8 PSI	Turnover Sequence	OFF Mid-	Peak TOU Begin	999	End 999
Well 28 Flow 0 CFS	Auto Start/Stop		eak TOU Begin	999	End 999
CL2 Residual 1.07 PPM	Excessive Starts SP	5 Boos	sters TOU Override	45	PSI <sup>(Military Time)</sup>
	Starts This Hour	0 Well	27 TOU Override	3	Ft
		Low	Level Shutdown	3	Ft
		Low	Level Reset	4	Ft

## East

		7:52:59		
	RTU-5 - East Reservoir and Pu	Imp Station Jan/11/2023 8:45:09 AM		
RUN STA	TUS SWITCH POSITION SCADA READY	Station Start SP 73.25 PSI		
Booster 1 (125 HP) OFF	AUTO YES	Station Maintain SP 78 PSI		
Booster 2 (125 HP) OFF	AUTO YES	SCADA Station Maintain SP 78 PSI		
Well 26 (200 HP) OFF	OFF NO	M.O.L 27.5' Start Stop		
(Bowl Depth - 250 feet/Static Sounding -	136 feet/Running Sounding - 220 feet)	Well 26 0 FT 0 FT		
	ANALOG VALUES	BYPASS VALVE SETPOINTS		
System Pressure 77.7	PSI Reservoir Level 12.4 FT	Bypass PID Pressure SP 78		
Discharge Flow 0	CFS Well 26 Flow 0 GPM	Open < 18.5 FT AND > 70 PSI		
VFD #1 Speed 0	% NaClO VFD Speed 0 Hz	Close > 24 FT OR < 65 PSI		
VFD #2 Speed 0	% Prelube Flow 0 GPM	Start Time 100 Stop Time 500		
Bypass Flow 0	CFS Well 26 NaClO Level 300.5 Gallons	BPV Fill Control ENABLE Open Command O		
Bypass Valve Position 0	% Open Residual .06 PPM	BPV Fill Status INACTIVE Close Command		
	STATUS / CONTROL	TIME OF USE SETPOINTS		
Pressure Control Status	ACTIVE Well 26 NaCIO VFD Ready	Well 26 TOU Status INACTIVE		
Booster Enable/Disable	ENABLE	Well 26 TOU Start Time 999 Stop Time 999		
Relief Valve Position	CLOSED Well 26 NaClO VFD Running	Well 26 TOU Override Level SP 3 FT		
NaCIO VFD Switch Pos.	Well 26 Prelube Ready     AUTO	Well 26 TOU Override Stop Level SP 21 FT		
Pressure Control Switch Pos	Well 26 Prelube Satisfied   s. PID			
VFD Lead/Lag Switch Pos.	Well 26 Prelube In Test Mode  SCADA	MISCELLANEOUS SETPOINTS Min. Res. Level to Start Pumps 5 FT		
Lead VFD	BPV Fully Open	Reservoir Level Hi Hi Alarm SP 28 FT		
Lead/Lag Runtime Ratio	50 % BPV Fully Closed	Reservoir Level Hi Hi Alarm SP 28 FT Reservoir Level Lo Lo Alarm SP 4.5 FT		
	50 70	Reservoir Level Lo Lo Alarm SP 4.5 FT		

# Garthe

		RTU	RTU-2 - John Garthe Reservoir						52:59 23 8:45:0	9 AM
Booster #1 (100 HP) Booster #2 (150 HP) Booster #3 (150 HP) Booster #4 (200 HP) Booster #5 (250 HP) Well #39 (250 HP) Well #18 (150 HP) Well #24 (150 HP)	RUN STAT. OFF RUN OFF RUN OFF RUN RUN OFF	AVAILABLE IN-SERVICE IN-SERVICE IN-SERVICE IN-SERVICE 2 1 1 2 2 2	FAIL NORMAL NORMAL NORMAL NORMAL NORMAL NORMAL NORMAL	SW. I AU AU AU AU AU AU AU	ТО ТО ТО ТО ТО ТО ТО ТО	Start Setpo Maintain S Well Stage 1 Well Stage 2 TOU Low I Override S	etpoint STAF 22 21.4 PSI	Ft 5 Ft	PSI PSI 24 F 23.5 F .O.L 25' PSI	
ANALOG V/ Bypass Flow Res. Level Disch. Flow	ALUES           0         CFS           21.7         Ft           11.3         CFS	STATUS Automatic Con Bypass Valve Mid-Peak TOU On Peak TOU S	CLOS Status INACT	ED IVE	Bypass	SE Valve Enable Valve Disable Ctrl Time Start	TPOINTS 18 4.7 999	(AND) (OR) Stop	65 60 999	
System Press SA-1 Flow CL2 Residual W36 CL2 Level W39 CL2 Level	64.1         PSI           0         CFS           0.69         PPM           352.3         GAL           398         GAL	MWD Plug Valv Well 24 Auto C BPV TOU Statu Engine Ctrl W-24 MOV Ope W-24 MOV Close	re UNKNC trl AUT is ACTM ACTM	OWN O /E	On Pea TOU Ov Low Lev	ak TOU Begin Ik TOU Begin	U. SETP 999 999 2.5 2 999	OINTS End End Reset Stop	999 999 7 999	MT MT Ft MT

# Walnut

		R	ru-1A - Wa	alnut Rese	ervoir an	d P	ump St	ation		7:52:59 2023 8:45:09	АМ	
	RUN STAT.	SW. POS.	FAIL	AVAILABL	E SPD		ST	ATION & VF	D STATU	S/CONTROL	-	
Booster 1 (200 HP)	OFF	AUTO	NORMAL	IN-SERVIC	E 0	%		Pressure Se		67.5	PSI	
Booster 2 (200 HP)		AUTO	NORMAL	IN-SERVIC	E 97	%		Pressure De	eadband	1.25		
Booster 3 (200 HP)		AUTO	NORMAL	IN-SERVIC	E			Stage Delay		200	_	
Booster 4 (150 HP)	OFF	AUTO	NORMAL	IN-SERVIC	E			Auto Rotat.			CFS	5
Booster 5 (100 HP)	OFF	AUTO	NORMAL	IN-SERVIC	E			Auto Rotat.			-	
		WELL	S STAGE SEL	ECTION			Pump	Auto Rotat.	_			
Well 16 (150 HP)									VALVE SE	T POINTS		
Well 29 (200 HP)	RUN	AUTO	NORMAL	IN-SERVIC	E		PV TOU S		OFF	4		
Well 33 (300 HP)	OFF	AUTO	NORMAL				/pass Val A-2 Press		OPEN			
	VALUES			JS / CONTROL				ss Setpoint	0	PSI		
								ve Enable	0	FT (AND)	0	PSI
Bypass Flow	<u> </u>		J Status		FF			ve Disable	0	FT (OR)	0	PSI
Res. Level	15F		v Press Overr		5 PSI			I Time Start	0		0	Stop
Disch. Flow	5.5 C	FS <sup>Byp</sup>	bass Ctrl Ena	ble DISA	BLED			ve Max Flow		CFS	•	otop
System Press	67.2 P	si	_	_	_			E STATUS		0.0		
CL2 Day Tank	765 0	AL			Open Stat.		W. Pos.	FAIL	AVAIL	ABLE		
Residual	1.01 P	PM	Well 16	Res. Valve	CLOSED			ALARM	OUT-SE	RVICE		
			Well 16	Waste Valve	CLOSED			ALARM	OUT-SE	RVICE		
WELL SE M.O.L. 19.5' Sta		on	Well 29	Res. Valve	MOVING			ALARM	OUT-SE	RVICE		
Well 16 .1			Well 29	Waste Valve	CLOSED			NORMAL	OUT-SE	RVICE		
Well 29 15			Well 33	Res. Valve	OPEN			NORMAL	OUT-SE	RVICE		
Well 33 14.			Well 33	Waste Valve	CLOSED			NORMAL	OUT-SE	RVICE		

## West

	R	TU-6 - Wes	t Reservoir	and Pump	Station	7 Jan/11/20	: 53 : 59 23 8:45:0	09 AM
RUN STAT.	SW. POS.	FAIL	AVAILABLE		STATION & VFD STATUS/CONTROL			
Booster 1 (200 HP) RUN	AUTO	NORMAL	IN-SERVICE	Backup Control	Station Start Se	etpoint	80	PSI
Booster 2 (200 HP) OFF	AUTO	NORMAL	IN-SERVICE	STAGING	Station Maintai	n Setpoint	83	PSI
Booster 3 (200 HP) OFF	AUTO	NORMAL	IN-SERVICE	2	Auto VFD Lead Select		88	
Booster 4 (150 HP) OFF	AUTO	NORMAL	IN-SERVICE	1	Select PID Con	trol	0	
Booster 5 (100 HP) OFF	AUTO	NORMAL	IN-SERVICE	3	Lead VFD Speed		94.98	%
	WELLS STAGE SELECTION							
Well 20 (150 HP) RUN	AUTO	NORMAL IN-SERVICE 1		1	<u></u>	ILORINATIO	<u>N</u>	
Well 21 (150 HP) OFF	AUTO	NORMAL	IN-SERVICE	2	Residual		.88	PPM
Well 30 (200 HP) OFF	AUTO	NORMAL	IN-SERVICE	0	NaCIO Level		775.6	GAL
ANALOG VALUES		STATUS /	CONTROL		SET P	OINTS		
Bypass Flow 0	CFS AL	utomatic Contr	ol AUTO	Bypass V	alve Enable 2	5 FT (ANI	D) 86	PSI
Res. Level 23.1	т Ву	pass Valve	CLOSED	Bypass V	/alve Disable 2	6 FT (OR	) 81	PSI
		d-Peak TOU	INACTIVE	Bypass C	Bypass Ctrl Time Start 2300 Stop 600			
		1-Peak TOU	INACTIVE	Bypass P	PID Pressure Set Po	oint	83	PSI
					TOU SET	POINTS		
CL2 Day Tank 775.6	GAL BP	PV TOU Status	INACTIVE		(TOU Begin (MT)		END 9	99
Backup Control - Boosters 3, 4	4, & 5 M.C	D.L. 27.5'			(TOU Begin (MT)			99
START S	ГОР	STA	RT STOP		U Override	3 FT		
Pump Stage 1 80.0 PSI 8	6.0 We	Il Stage 1 23	3 FT 26.75	-(	ssure Override SP	0 PS		
Pump Stage 2 79.0 PSI 8	4.0 We	Il Stage 2 22	2 FT 26.5		el Shutdown	3 FT		
Pump Stage 3 77.0 PSI 8	2.0 We	ell Stage 3 2'	1 FT 26.25	Low Leve		7 FT		

# South

	RTU-7 - South Reservoir	and Pump Station 7 : 53 : 59 Jan/11/2023 8:45:09 AM
RUN STAT. Booster 1 (125 HP) OFF Booster 2 (125 HP) OFF RUN STAT. Well 34 (125 HP) OFF	SWITCH POS.FAILAUTONORMALAUTONORMALSWITCH POS.FAILAUTONORMAL	STARTSTOPLead VFD88.9PSI100PSILag VFD87PSI96PSISystem Maintain Setpoint94PSIM.O.L. 17.5'
ANALOG VALUES Bypass Flow 0 CFS Res. Level 15 Ft Disch. Flow 0 CFS System Press 93.5 PSI	STATUS / CONTROLAutomatic ControlAUTOBypass ValveCLOSEDBypass ValveINACTIVETOU StatusStatus	BYPASS VALVE SET POINTSOpen<
Well 34 Flow0CFSWell 34 CL2 Tank Level400.8GalFwd Totalizer64,428,748CFRev Totalizer-57,581CF	Mid-Peak Tou Status INACTIVE On Peak TOU Status INACTIVE	T.O.U. SET POINTSMid-Peak TOU Begin999End999MTOn Peak TOU Begin999End999MTStationStation0Low Level Shutdown0Low Level Reset2.52.50

Hydraulic Model Evaluation for the Village Project

## **Appendix B**

The Village Project Proposed Land Use Plan

## 3.2 Land Use Plan

The land use plan is the basis for establishing the range of uses, maximum buildout, and development standards for the Village. The Village is a mixed-use community that allows for vertical and horizontal mixed use across the site. As shown on Figure 3-1, Proposed Land Use Plan, a variety of residential, commercial, and community uses are planned throughout the Village. A central commercial area in a park setting allows for restaurants and retail uses to activate the area during both day and night. A continuation of those commercial uses is encouraged at the ground floor of adjacent residential buildings. Parking would be readily available in atgrade and underground structures. Figure 3-2a, Conceptual Site Plan, depicts how the plan could be implemented. As noted in the figure, various commercial uses, including

restaurants, retail shops, and a grocery store/market, may enhance the plaza and create a sense of place at the heart of the Village. Stand-alone residential and mixeduse buildings provide a variety of housing opportunities for residents who seek a unique community in the city's South Bristol Focus Area.

#### 3.2.1 Maximum Buildout

The maximum buildout for the Village is provided in Table 3-1, *Land Use Statistical Summary*. This Specific Plan allows up to 1,583 dwelling units, 80,000 square feet of commercial space, 300,000 square feet of office, and over 3.6 acres of open space. Chapter 4, *Development Regulations*, provides more detail regarding permitted uses and development standards for each use.

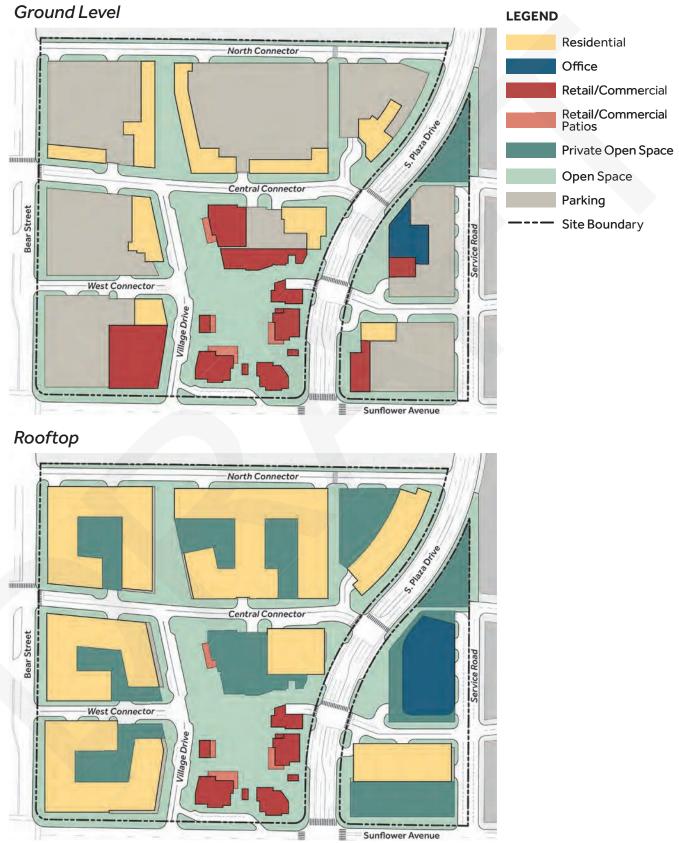
Uses <sup>1</sup>	Development	Site Size		
Residential	1,583 units (maximum)			
Commercial	17 Acres			
Office	fice 300,000 sq ft (maximum)			
Open Space <sup>2</sup>	158,300 sq ft (approximately 3.6 acres, minimum)			

#### Table 3-1: Land Use Statistical Summary

Notes:

1. Uses are permitted as vertical and/or horizontal mixed use.

2. Open Space areas consist of passive and active areas as defined by the City of Santa Ana 2022 Housing Element.



#### Figure 3-1: Proposed Land Use Plan

Source: Gensler, PlaceWorks

Hydraulic Model Evaluation for the Village Project

## **Appendix C**

### **Extended-Period Simulation Results**

