

City of Escondido PRIORITY DEVELOPMENT PROJECT (PDP) SWQMP

PALOMAR HEIGHTS
[INSERT RECORD ID (PERMIT) NUMBERS]

555 E Valley Pkwy, Escondido, CA 92025

ASSESSOR'S PARCEL NUMBER(S): 229-450-05 & 06, 230-163-01, 02, 03 & 05

ENGINEER OF WORK:

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PREPARED FOR:

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DATE OF SWQMP: December 2nd 2019

PLANS PREPARED BY: HUNSAKER & ASSOCIATES 9707 WAPLES STREET SAN DIEGO, CA 92121 (858) 558-4500 SWQMP APPROVED BY:

APPROVAL DATE:



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Template Date: October 2016 Preparation Date: 6/18/2019 PDP SWQMP

TABLE OF CONTENTS

TABLE OF C	ONTENTS	iii				
ATTACHMEN	ΓSi					
ACRONYMS	RONYMS					
PDP SWQMP	PREPARER'S CERTIFICATION PAGE	v				
SUBMITTAL	RECORD	vii				
PROJECT VI	CINITY MAP	viii				
Step 1: Pro	oject type determination (Standard or Priority Development Project) (Form I-2a	a) 1				
Step 1.1:	Storm Water Quality Management Plan requirements	4				
Step 1.2:	Exception to PDP definitions4					
Step 2: Co	nstruction Storm Water BMPs	5				
Step 3: Cit	y of Escondido PDP SWQMP Site Information Checklist (Form I-2a)	6				
Step 3.1:	Description of Existing Site Condition	6				
Step 3.2:	Description of Existing Site Drainage Patterns	7				
Step 3.3:	Description of Proposed Site Development	8				
Step 3.4:	Description of Proposed Site Drainage Patterns	9				
Step 3.5:	Potential Pollutant Source Areas	10				
Step 3.6:	Identification and Narrative of Receiving Water and Pollutants of Concern	11				
Step 3.7:	Hydromodification Management Requirements	12				
Step 3.7.	1: Critical Coarse Sediment Yield Areas*	13				
Step 3.7.2	2: Flow Control for Post-Project Runoff*	14				
Step 3.8:	Other Site Requirements and Constraints	15				
Step 4: So	urce Control BMP Checklist (Form I-2b)	16				
Step 5: Sit	e Design BMP Checklist (Form I-2c)	18				
Step 6: PD	P Structural BMPs (Form I-3)	20				
Step 6.1:	Description of structural BMP strategy	20				
Step 6.2:	Structural BMP Checklist	22				
Sten 6 3:	Offsite Alternative Compliance Participation Form	23				

Template Date: October 2016 PDP SWQMP

ATTACHMENTS

Attachment 1: Backup for PDP Pollutant Control BMPs

Attachment 1a: Storm Water Pollutant Control Worksheet Calculations (Worksheet B.2-1

DCV, Form I-4)

Attachment 1b: Form I-5, Categorization of Infiltration Feasibility Condition

Attachment 1c: Form I-6, Factor of Safety and Design Infiltration Rate Worksheet

Attachment 1d: Drainage Management Area (DMA) Exhibit Attachment 1e: Individual Structural BMP DMA Mapbook

Attachment 2: Backup for PDP Hydromodification Control Measures

Attachment 2a: Flow Control Facility Design

Attachment 2b: Hydromodification Management Exhibit

Attachment 2c: Management of Critical Coarse Sediment Yield Areas Attachment 2d: Geomorphic Assessment of Receiving Channels (optional)

Attachment 2e: Vector Control Plan (if applicable)

Attachment 3: Structural BMP Maintenance Plan

Attachment 3a: Structural BMP Maintenance Thresholds and Actions

Attachment 3b: Draft Maintenance Agreements / Notifications (when applicable)

Attachment 4: City of Escondido PDP Structural BMP Verification

Attachment 5: Copy of Plan Sheets Showing Permanent Storm Water BMPs

Attachment 6: Modular Wetland Maintenance & Specifications Attachment 7: Third Party Certification for Modular Wetland Attachment 8: Geotechnical Investigation – Palomar Heights

ACRONYMS

ACP Alternative Compliance Project
APN Assessor's Parcel Number
BMP Best Management Practice
DMA Drainage Management Area

EOW Engineer of Work

HMP Hydromodification Management Plan

HSG Hydrologic Soil Group

MS4 Municipal Separate Storm Sewer System

N/A Not Applicable

PDP Priority Development Project

PE Professional Engineer

SC Source Control SD Site Design

SDRWQCB San Diego Regional Water Quality Control Board

SIC Standard Industrial Classification SWDM Storm Water Design Manual

SWQMP Storm Water Quality Management Plan WMAA Watershed Management Area Analysis

WQIP Water Quality Improvement Plan

Template Date: October 2016 Preparation Date: 6/18/2019 PDP SWQMP

PDP SWQMP PREPARER'S CERTIFICATION PAGE

Project Name: PALOMAR HEIGHTS

Permit Application Number: [Insert Permit Application Number]

PREPARER'S CERTIFICATION

I hereby declare that I am the Engineer in Responsible Charge of design of storm water best management practices (BMPs) for this project, and that I have exercised responsible charge over the design of the BMPs as defined in Section 6703 of the Business and Professions Code, and that the design is consistent with the PDP requirements of the City of Escondido Storm Water Design Manual, which is a design manual for compliance with the City of Escondido Municipal Code (Chapter 22, Article 2) and regional MS4 Permit (California Regional Water Quality Control Board San Diego Region Order No. R9-2013-0001 as amended by R9-2015-0001 and R9-2015-0100) requirements for storm water management.

I have read and understand that the City of Escondido has adopted minimum requirements for managing urban runoff, including storm water, from land development activities, as described in the Storm Water Design Manual. I certify that this PDP SWQMP has been completed to the best of my ability and accurately reflects the project being proposed and the applicable BMPs proposed to minimize the potentially negative impacts of this project's land development activities on water quality. I understand and acknowledge that the plan check review of this PDP SWQMP by City staff is confined to a review and does not relieve me, as the Engineer in Responsible Charge of design of storm water BMPs for this project, of my responsibilities for project design.

Engineer of Work's Signature, PE Number &	Expiration Date	-
Raymond L. Martin	-	_
Print Name		
Hunsaker & Associates – San Diego, Inc. Company		-
12/2/2019 Date	Engineer's Seal:	PROFESSIONAL CITY OF THE PROPERTY OF THE PROPE
		PIE OF CALIFORNI

Template Date: October 2016

PDP SWQMP

Preparation Date: 6/18/2019

5

Template Date: October 2016 Preparation Date: 6/18/2019 PDP SWQMP 6

SUBMITTAL RECORD

Use this Table to keep a record of submittals of this PDP SWQMP. Each time the PDP SWQMP is re-submitted, provide the date and status of the project. In column 4 summarize the changes that have been made or indicate if response to plancheck comments is included. When applicable, insert response to plancheck comments behind this page.

Preliminary Design / Planning / CEQA

Submittal	Date	Summary of Changes	
Number			
1	6/18/2019	Initial Submittal	
2	9/13/2019	Revisions per plan check comments.	
3	12/2/2019	Revisions per plan check comments.	
4			

Final Design

Submittal Number	Date	Summary of Changes
1		Initial Submittal
2		
3		
4		

Plan Changes

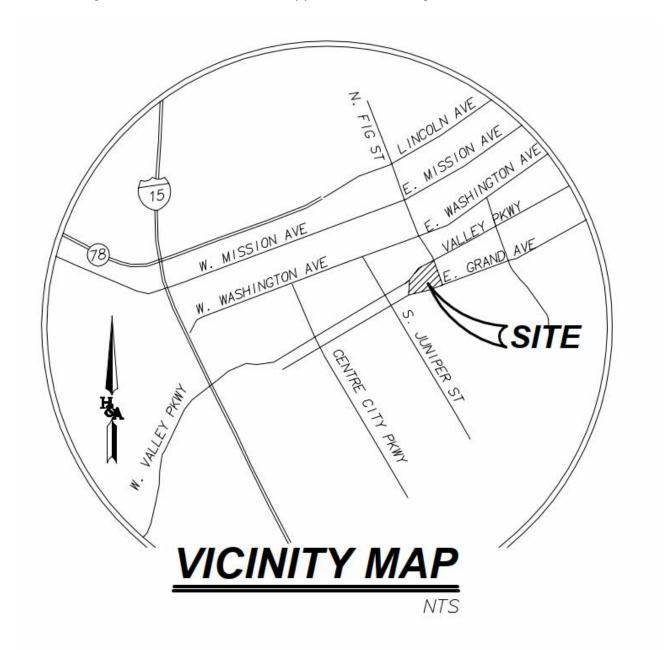
Submittal Number	Date	Summary of Changes
1		Initial Submittal
2		
3		
4		

Template Date: October 2016 Preparation Date: 6/18/2019 vii

PROJECT VICINITY MAP

Project Name: PALOMAR HEIGHTS

Record ID: [Insert Record ID or Permit Application Number]



Template Date: October 2016 Preparation Date: 6/18/2019
PDP SWQMP viii

Step 1: Project type determination (Standard or Priority Development Project) (Form I-2a)

Project Sumi	mary Information	
Project Name	PALOMAR HEIGHTS	
Project Address	555 E Valley Pkwy, Escondido, CA 92025	
	200 450 05 0 00 000 400 04 00 00 0	
Assessor's Parcel Number(s)	229-450-05 & 06, 230-163-01, 02, 03 & 05	
Permit Application Number		
Project Watershed (Hydrologic Unit)	Select One: ⊠ Carlsbad 904 □ San Dieguito 905	
Parcel Area		
(total area of Assessor's Parcel(s) associated with the project)	13.80 Acres <u>(601128 Square Feet)</u>	
Area to be disturbed by the project	40.00 4	
(Project Area)	13.00 Acres (<u>566318.3</u> Square Feet)	
Project Proposed Impervious Area (subset of Project Area)	8.16 Acres (<u>355268.7</u> Square Feet)	
Project Proposed Pervious Area (subset of Project Area)	4.84 Acres (211049.6 Square Feet)	
Note: Proposed Impervious Area + Proposed Pervi This may be less than the Parcel Area.	ous Area = Area to be Disturbed by the Project.	
Confirmation of Priority Dev	elopment Project Determination	
The project is (select one): \square New Development	*	
The total proposed newly created or replaced impe	rvious area is:ft²	

Solar energy farms that are not also one of the categories listed in Step 2b of Table 1-1. City staff must also determine that appropriate BMPs are provided to mitigate for downstream impacts due to significant changes to the existing hydrology

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP

1 of 42

¹ Redevelopment is defined as: The creation and/or replacement of impervious surface on an already developed site. Examples include the expansion of a building footprint, road widening, the addition to or replacement of a structure, and creation or addition of impervious surfaces. Replacement of impervious surfaces includes any activity that is not part of a routine maintenance activity where impervious material(s) are removed, exposing underlying soil during construction. Redevelopment does not include routine maintenance activities, such as trenching and resurfacing associated with utility work; pavement grinding; resurfacing existing roadways; new sidewalks construction; pedestrian ramps; or bike lanes on existing roads; and routine replacement of damaged pavement, such as pothole repair.

Is the	projec	t in ar	ny of the following categories, (a) through (f)?
Yes	No ⊠	(a)	New development projects that create 10,000 square feet or more of impervious surfaces (collectively over the entire project site). This includes commercial, industrial, residential, mixed-use, and public development projects on public or
Yes	No	(b)	private land. Redevelopment projects that create and/or replace 5,000 square feet or more of
\boxtimes		(2)	impervious surface (collectively over the entire project site on an existing site of 10,000 square feet or more of impervious surfaces). This includes commercial, industrial, residential, mixed-use, and public development projects on public or private land.
Yes ⊠	No	(c)	New and redevelopment projects that create and/or replace 5,000 square feet or more of impervious surface (collectively over the entire project site), and support one or more of the following uses: (i) Restaurants. This category is defined as a facility that sells prepared foods and drinks for consumption, including stationary lunch counters and refreshment stands selling prepared foods and drinks for immediate consumption (Standard Industrial Classification (SIC) code 5812). (ii) Hillside development projects. This category includes development on any natural slope that is twenty-five percent or greater. (iii) Parking lots. This category is defined as a land area or facility for the temporary parking or storage of motor vehicles used personally, for business, or for commerce. (iv) Streets, roads, highways, freeways, and driveways. This category is defined as any paved impervious surface used for the transportation of automobiles, trucks, motorcycles, and other vehicles.
Yes	No ⊠	(d)	New or redevelopment projects that create and/or replace 2,500 square feet or more of impervious surface (collectively over the entire project site), and discharging directly to an Environmentally Sensitive Area (ESA). "Discharging directly to" includes flow that is conveyed overland a distance of 200 feet or less from the project to the ESA, or conveyed in a pipe or open channel any distance as an isolated flow from the project to the ESA (i.e. not commingled with flows from adjacent lands). Note: ESAs are areas that include but are not limited to all Clean Water Act Section 303(d) impaired water bodies; areas designated as Areas of Special Biological Significance by the State Water Board and San Diego Water Board; State Water Quality Protected Areas; water bodies designated with the RARE beneficial use by the State Water Board and San Diego Water Board; and any other equivalent environmentally sensitive areas which have been identified by the Copermittees.
Yes	No ⊠	(e)	New development projects, or redevelopment projects that create and/or replace 5,000 square feet or more of impervious surface, that support one or more of the following uses: (i) Automotive repair shops. This category is defined as a facility that is categorized in any one of the following SIC codes: 5013, 5014, 5541, 7532-7534, or 7536-7539. (ii) Retail gasoline outlets (RGOs). This category includes RGOs that meet the following criteria: (a) 5,000 square feet or more or (b) a projected Average Daily Traffic (ADT) of 100 or more vehicles per day.

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 2 of 42

Yes	No ⊠	(e)	New development projects, or redevelopment projects that cre	
			5,000 square feet or more of impervious surface, that support one or more of the following uses:	
			(iii) Automotive repair shops. This category is defined as a facility that is	
			categorized in any one of the following SIC codes: 50	-
			7534, or 7536-7539.	, , ,
			(iv) Retail gasoline outlets (RGOs). This category include	s RGOs that meet the
			following criteria: (a) 5,000 square feet or more or (b)	
			Daily Traffic (ADT) of 100 or more vehicles per day.	, ,
Yes	No	(f)	New or redevelopment projects that result in the disturbance	
\boxtimes			of land and are expected to generate pollutants post construc	
			Note: See Storm Water Design Manual Section 1.4.2 for	r additional guidance.
Furthe	r guidand	e may	be found in Chapter 1 and Table 1-2 of the Storm Water Design Manual. or redevelopment PDPs only:	
11161	OllOWILL	y is io	redevelopment PDF3 only.	
The a	rea of	existi	ng (pre-project) impervious area at the project site is:	541015 ft ² (A)
	The total proposed newly created or replaced impervious area is 355268.7 ft ² (B)			
	ercent impervious surface created or replaced (B/A)*100: 65.66%			
	The percent impervious surface created or replaced is (select one based on the above calculation):			
			or equal to fifty percent (50%) - only newly created or replac	ed impervious areas
		cons	sidered a PDP and subject to stormwater requirements	
	OR		(C)	
	-		an fifty percent (50%) – the entire project site is considered	a PDP and subject to
	sto	rmwa	ater requirements	

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 3 of 42

Step 1.1: Storm Water Quality Management Plan requirements

Step	Answer	Progression
Is the project a Standard Project,	☐ Standard	Standard Project requirements apply, including
Priority Development Project (PDP), or	Project	Standard Project SWQMP.
exception to PDP definitions?	,	Complete Form I-1.
To answer this item, complete Step 1	⊠ PDP	Standard and PDP requirements apply,
Project Type Determination Checklist		including PDP SWQMP.
on Pages 1 and 2, and see PDP		SWQMP Required.
exemption information below.		
For further guidance, see Section 1.4	☐ PDP with	If participating in offsite alternative compliance,
of the Storm Water Design Manual in	ACP	complete Step 6.3 and an ACP SWQMP.
its entirety.		
	□ PDP	Go to Step 1.2 below.
	Exemption	

Step 1.2: Exemption to PDP definitions

La the manifest account from DDD definitions have done sitted of the fellowing.	If and
Is the project exempt from PDP definitions based on either of the following:	If so:
 Projects that are only new or retrofit paved sidewalks, bicycle lanes, or trails that meet the following criteria: Designed and constructed to direct storm water runoff to adjacent vegetated areas, or other non-erodible permeable areas; OR Designed and constructed to be hydraulically disconnected from paved streets or roads [i.e., runoff from the new improvement does not drain directly onto paved streets or roads]; OR Designed and constructed with permeable pavements or surfaces in accordance with County of San Diego Green Streets Infrastructure; 	Standard Project requirements apply, AND any additional requirements specific to the type of project. City concurrence with the exemption is required. Provide discussion and list any additional requirements below in this form.
☐ Projects that are only retrofitting or redeveloping existing paved alleys, streets or roads that are designed and constructed in accordance with the City of Escondido Guidance on Green Infrastructure.	PDP Exempt.
Discussion / justification, and additional requirements for exceptions to PDP	definitions, if applicable:

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 4 of 42

Step 2: Construction Storm Water BMPs

Construction storm water BMPs shall be shown on the Grading Plan and (if applicable) included in the Storm Water Pollution Prevention Plan (SWPPP).

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 5 of 42

Step 3: City of Escondido PDP SWQMP Site Information Checklist (Form I-2a)

Step 3.1: Description of Existing Site Condition

Current Status of the Site (select all that apply):
⊠ Existing development
□Previously graded but not built out
□Demolition completed without new construction
□Agricultural or other non-impervious use
□Vacant, undeveloped/natural
Description / Additional Information
Description / Additional Information:
Existing Land Cover Includes (select all that apply and provide each area on site):
☑ Vegetative Cover 1.38 Acres (60113 ft²)
□Non-Vegetated Pervious AreasAcres (Square Feet)
☑ Impervious Areas 12.42 Acres (541015 ft²)
·
Description / Additional Information:
Underwing Cail belongs to Undralagic Cail Crays (acleat all that apply).
Underlying Soil belongs to Hydrologic Soil Group (select all that apply): □NRCS Type A
□NRCS Type A
⊠ NRCS Type C
□NRCS Type D
Approximate Depth to Groundwater (GW) (or N/A for no infiltration BMPs):
□GW Depth < 5 feet
□5 feet < GW Depth < 10 feet
☑ 10 feet < GW Depth < 20 feet
□GW Depth > 20 feet
Existing Natural Hydrologic Features (select all that apply):
□Watercourses
□Seeps
□Springs
□Wetlands
⊠ None
□Other
Description / Additional Information:

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 6 of 42

Step 3.2: Description of Existing Site Drainage Patterns

How is storm water runoff conveyed from the site? At a minimum, this description should answer:

- (1) Whether existing drainage conveyance is natural or urban;
- (2) Is runoff from offsite conveyed through the site? if yes, quantify all offsite drainage areas, design flows, and locations where offsite flows enter the project site, and summarize how such flows are conveyed through the site;
- (3) Provide details regarding existing project site drainage conveyance network, including any Existing storm drains, concrete channels, swales, detention facilities, storm water treatment facilities, natural or constructed channels; and
- (4) Identify all discharge locations from the existing project site along with a summary of conveyance system size and capacity for each of the discharge locations. Provide summary of the pre-project drainage areas and design flows to each of the existing runoff discharge locations.

Describe existing site drainage patterns:

The existing drainage conveyance is urban, as the site is a hospital with associated structures and parking lots. No offsite bypass flow is conveyed through the site.

The site discharges flow onto Valley Parkway and Fig Street via area drains and curb outlets.

Flow discharged onto Valley Parkway is intercepted via one of the three inlets. Flow collected by the first and second inlet is routed via an 18" RCP to a 36" RCP where it is confluenced with the flow collected by the third inlet. This flow ultimately discharges into the Escondido Creek.

Flow discharged onto Fig Street from the property enters a curb inlet and is routed via a 12" RCP to a larger 33" RCP where it is confluenced with flow from Fig Street. This flow ultimately discharges into the Escondido Creek.

The existing infrastructure is sized appropriately sized to convey the drainage described above.

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 7 of 42

Step 3.3: **Description of Proposed Site Development**

Project Description / Proposed Land Use and/or Activities: The Palomar Heights project proposes the construction of 509 multifamily residential/mixed use units with associated roads, walkways, recreation, and landscape areas. The project proposes the construction of a new storm drain network, 2 tree wells, and 12 proprietary biofiltration units to provide water quality treatment for runoff generated in the proposed condition. List/describe proposed impervious features of the project (e.g., buildings, roadways, parking lots, courtyards, athletic courts, other impervious features): The proposed development will consist of 509 multifamily residential/mixed use units with associated streets, walkways, recreation areas, and storm drain systems. List/describe proposed pervious features of the project (e.g., landscape areas): Landscape areas Does the project include grading and changes to site topography? □Yes ⊠No Description / Additional Information: Existing drainage patterns will not be altered in the proposed condition. No proposed changes to site topography are proposed.

Insert acreage or square feet for the different land cover types in the table below:

Change in Land Cover Type Summary					
		Proposed	Percent		
	(acres or ft ²)	(acres or ft ²)	Change		
Vegetation	60,113	211,049.6	251.08%		
Pervious (non-vegetated)					
Impervious	541,015	355,268.7	52.28%		

Template Date: October 2016 Preparation Date: 6/18/2019] PDP SWQMP 8 of 42

Step 3.4: Description of Proposed Site Drainage Patterns

Does the project include changes to site drainage (e.g., installation of new storm water					
conveyance systems)? □Yes					
□No					
If yes, provide details regarding the proposed project site drainage conveyance network, including storm drains, concrete channels, swales, detention facilities, storm water treatment facilities, natural or constructed channels, and the method for conveying offsite flows through or around the proposed project site. Identify all discharge locations from the proposed project site along with a summary of the conveyance system size and capacity for each of the discharge locations. Provide a summary of pre- and post-project drainage areas and design flows to each of the runoff discharge locations. Reference the drainage study for detailed calculations.					
Describe proposed site drainage patterns:					
The proposed drainage will maintain the existing condition drainage patterns. No offsite bypass flow is conveyed through the site.					
Flow generated by each of the DMA's within the subdivision will reach 1 of the 13 proprietary biofiltration treatment BMP's before entering the onsite storm drain network.					
The proposed onsite drainage network within the subdivision will tie into the existing drainage network described in Step 3.2					
Refer to the Drainage Report for Palomar Heights prepared by Hunsaker & Associates for more information.					

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 9 of 42

Step 3.5: Potential Pollutant Source Areas

Identify whether any of the following features, activities, and/or pollutant source areas will be present (select all that apply).
 ⊠On-site storm drain inlets □Interior floor drains and elevator shaft sump pumps □Interior parking garages □Need for future indoor & structural pest control ☑Landscape/Outdoor Pesticide Use □Pools, spas, ponds, decorative fountains, and other water features □Food service □Refuse areas □Industrial processes □Outdoor storage of equipment or materials □Vehicle and Equipment Cleaning □Vehicle/Equipment Repair and Maintenance □Fuel Dispensing Areas □Loading Docks □Fire Sprinkler Test Water □Miscellaneous Drain or Wash Water
⊠Plazas, sidewalks, and parking lots □Other (provide description)
Description / Additional Information:

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 10 of 42

Step 3.6: Identification and Narrative of Receiving Water and Pollutants of Concern

Describe flow path of storm water from the project site discharge location(s), through urban storm conveyance systems as applicable, to receiving creeks, rivers, and lagoons as applicable, and ultimate discharge to the Pacific Ocean (or bay, lagoon, lake or reservoir, as applicable):

List any 303(d) impaired water bodies² within the path of storm water from the project site to the Pacific Ocean (or bay, lagoon, lake or reservoir, as applicable), identify the pollutant(s)/stressor(s) causing impairment, and identify any TMDLs and/or Highest Priority Pollutants from the WQIP for the impaired water bodies:

		TMDLs / WQIP Highest
303(d) Impaired Water Body	Pollutant(s)/Stressor(s)	Priority Pollutant
Escondido Creek	Benthic Community Effects,	Requires development
	Bifenthrin, DDT, Indicator Bacteria,	
	Malathion, Manganese, Nitrogen,	
	Phosphate, Selenium, Sulfates,	
	Total Dissolved Solids, Toxicity	

Identification of Project Site Pollutants*

Identify pollutants expected from the project site based on all proposed use(s) of the site (see Storm Water Design Manual Appendix B.6):

Pollutant	Not Applicable to the Project Site	Anticipated from the Project Site	Also a Receiving Water Pollutant of Concern
Sediment			
Nutrients			
Heavy Metals			
Organic Compounds			
Trash & Debris			
Oxygen Demanding Substances			
Oil & Grease			
Bacteria & Viruses			
Pesticides			

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP
11 of 42

^{*}Identification of project site pollutants below is only required if flow-thru treatment BMPs are implemented onsite in lieu of retention or biofiltration BMPs. Note the project must also participate in an alternative compliance program (unless prior lawful approval to meet earlier PDP requirements is demonstrated).

² The current list of Section 303(d) impaired water bodies can be found at http://www.waterboards.ca.gov/water issues/programs/water quality assessment/#impaired

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 12 of 42

Step 3.7: Hydromodification Management Requirements

Do hydromodification management requirements apply (see Section 1.6 of the Storm Water
Design Manual)?
□Yes, hydromodification management requirements for flow control and preservation of critical
coarse sediment yield areas are applicable. □No, the project will discharge runoff directly to existing underground storm drains discharging
directly to water storage reservoirs, lakes, enclosed embayments, or the Pacific Ocean.
□No, the project will discharge runoff directly to conveyance channels whose bed and bank are
concrete-lined all the way from the point of discharge to water storage reservoirs, lakes, enclosed embayments, or the Pacific Ocean.
☑ No, the project will discharge runoff directly to an area identified as appropriate for an
exemption by the WMAA ³ for the watershed in which the project resides.
Description / Additional Information (to be provided if a 'No' answer has been selected above):
A Hydromodification Management Applicability Evaluation for Escondido
Creek prepared by Geosyntec has determined that the Escondido Creek is is HMP exempt.
Therefore the Palomar Heights project is HMP exempt. Refer to the report included in Attachment 6

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 13 of 42

³The Watershed Management Area Analysis (WMAA) is an optional element for inclusion in the Water Quality Improvement Plans (WQIPs) described in the 2013 MS4 Permit [Provision B.3.b.(4)]. It is available online at the Project Clean Water website:

http://www.projectcleanwater.org/index.php?option=com_content&view=article&id=248

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP

14 of 42

Step 3.7.1: **Critical Coarse Sediment Yield Areas***

*This Section only required if hydromodification management requirements apply

Based on the maps provided within the WMAA, do potential critical coarse sediment yield areas exist within the project drainage boundaries?

Yes

No, no critical coarse sediment yield areas to be protected based on WMAA maps

If yes, have any of the optional analyses presented in Section 6.2 of the manual been performed?

- 6.2.1 Verification of GLUs (classification that provides an estimate of sediment yield based on geology, hillslope, and land cover) Onsite
- 6.2.2 Downstream Systems Sensitivity to Coarse Sediment
- 6.2.3 Optional Additional Analysis of Potential Critical Coarse Sediment Yield Areas Onsite No optional analyses performed, the project will avoid critical coarse sediment yield areas identified based on WMAA maps

If optional analyses were performed, what is the final result?

No critical coarse sediment yield areas to be protected based on verification of GLUs onsite. Critical coarse sediment yield areas exist but additional analysis has determined that protection is not required. Documentation attached in Attachment 8 of the SWQMP. Critical coarse sediment yield areas exist and require protection. The project will implement management measures described in Sections 6.2.4 and 6.2.5 as applicable, and the areas

are identified on the SWQMP Exhibit. Discussion / Additional Information:

N/A PROJECT IS HYDROMODIFICATION EXEMPT

Template Date: October 2016 Preparation Date: 6/18/2019] PDP SWQMP 15 of 42

Flow Control for Post-Project Runoff*

*This Section only required if hydromodification management requirements apply
List and describe point(s) of compliance (POCs) for flow control for hydromodification
management (see Section 6.3.1). For each POC, provide a POC identification name or number
correlating to the project's HMP Exhibit and a receiving channel identification name or number
correlating to the project's HMP Exhibit.
Has a geomorphic assessment been performed for the receiving channel(s)?
□No, the low flow threshold is 0.1Q2 (default low flow threshold)
□Yes, the result is the low flow threshold is 0.1Q2
☐Yes, the result is the low flow threshold is 0.3Q2
☐ Yes, the result is the low flow threshold is 0.5Q2
If a geomorphic assessment has been performed, provide title, date, and preparer:
h a geomorphic assessment has been performed, provide title, date, and preparer.
Discussion / Additional Information: (optional)

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 16 of 42

Step 3.8: **Other Site Requirements and Constraints**

When applicable, list other site requirements or constraints that will influence storm water management design, such as zoning requirements including setbacks and open space, or local codes governing minimum street width, sidewalk construction, allowable pavement types, and drainage requirements.

Optional Additional Information or Continuation of Previous Sections As Needed
Optional Additional Information or Continuation of Previous Sections As Needed This space provided for additional information or continuation of information from previous
sections as needed.
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Preparation Date: 6/18/2019] Template Date: October 2016 PDP SWQMP

17 of 42

Step 4:

Source Control BMP Checklist (Form I-2b) Source Control BMPs All development projects must implement source control BMPs 4.2.1 through 4.2.6 where applicable and feasible. See Chapter 4.2 and Appendix E of the City Storm Water Design Manual for information to implement source control BMPs shown in this checklist. The following checklists serve as guides only. Mark what elements are included in your project. See Storm Water Design Manual Chapter 4 and Appendix E for more information on determining appropriate BMPs for your project. Answer each category below pursuant to the following: "Yes" means the project will implement the source control BMP as described in Chapter 4.2 and/or Appendix E of the City Storm Water Design Manual. Discussion / justification is not required. "No" means the BMP is applicable to the project but it is not feasible to implement. Discussion / justification must be provided.

"N/A" means the BMP is not applicable at the project site because the project does not include the feature that is addressed by the BMP (e.g., the project has no outdoor

materials storage areas). Discussion / justification must be provided.				
Source Control Requirement	Applied?			
SC-1 Prevention of Illicit Discharges into the MS4	⊠Yes	□ No	□ N/A	
Direct irrigation water away from impervious surfacesDirect vehicle wash water away from impervious surfaces				
Other:				
Discussion / justification if SC-1 not implemented:				
SC-2 Storm Drain Stenciling or Signage	⊠Yes	□ No	□ N/A	
 Stencil or stamp storm drains with anti-dumping message Post signs prohibiting illegal dumping Other 				
Discussion / justification if SC-2 not implemented:				
SC-3 Protect Outdoor Materials Storage Areas from Rainfall, Run-On, Runoff, and Wind Dispersal	⊠Yes	□ No	□ N/A	
 Store materials inside a covered enclosure Direct runoff from downspouts and roofs away from storage Other 	e areas			
Discussion / justification if SC-3 not implemented:				

Template Date: October 2016 Preparation Date: 6/18/2019] PDP SWQMP 18 of 42

SC-4 Protect Materials Stored in Outdoor Work Areas from Rainfall, Run-On, Runoff, and Wind Dispersal	⊠Yes	□ No	□ N/A	
□ Locate work area away from storm drains or catch basins Work over impermeable surfaces where spills and pollutants can be captured and removed				
Discussion / justification if SC-4 not implemented:				
SC-5 Protect Trash Storage Areas from Rainfall, Run-On, Runoff, and Wind Dispersal	⊠Yes	□No	□N/A	
□ Locate trash containers in a roofed, walled enclosure □ Locate trash containers away from storm drains Discussion / justification if SC-5 not implemented:				
SC-6 Additional BMPs Based on Potential Sources of Runoff Pollutants (must answer for each source listed below):				
☐ A. On-site storm drain inlets	⊠Yes	□No	□N/A	
☐ B. Interior floor drains and elevator shaft sump pumps	□Yes	⊠No	□N/A	
☐ C. Interior parking garages	□Yes	⊠No	□N/A	
☐ D. Need for future indoor & structural pest control	□Yes	⊠No	□N/A	
☐ E. Landscape/outdoor pesticide use	⊠Yes	□No	□N/A	
☐ F. Pools, spas, ponds, fountains, and other water features	□Yes	⊠No	□N/A	
☐ G. Food service	□Yes	⊠No	□N/A	
☐ H. Refuse areas	□Yes	⊠No	□N/A	
☐ I. Industrial processes	□Yes	⊠No	□N/A	
☐ J. Outdoor storage of equipment or materials	□Yes	⊠No	□N/A	
☐ K. Vehicle and equipment cleaning	□Yes	⊠No	□N/A	
☐ L. Vehicle/equipment repair and maintenance	□Yes	⊠No	□N/A	
☐ M. Fuel dispensing areas	□Yes	⊠No	□N/A	
☐ N. Loading docks	□Yes	⊠No	□N/A	
☐ O. Fire sprinkler test water	□Yes	⊠No	□N/A	
☐ P. Miscellaneous drain or wash water	□Yes	⊠No	□N/A	
☐ Q. Plazas, sidewalks, and parking lots	⊠Yes	□No	⊠N/A	
Discussion / justification if SC-6 not implemented. Clearly identify which sources of runoff pollutants are discussed. Justification must be provided for <u>all</u> "No" answers shown above.				

Note: Show all source control measures described above that are included in design capture volume calculations in the plan sheets of Attachment 5.

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 19 of 42

Step 5: Site Design BMP Checklist (Form I-2c) Site Design BMPs All development projects must implement site design BMPs SD-A through SD-H where applicable and feasible. See Chapter 4.3 and Appendix E of the City Storm Water Design Manual for information to implement site design BMPs shown in this checklist. The following checklists serve as guides only. Mark what elements are included in your project. See Storm Water Design Manual Chapter 4 and Appendix E for more information on determining appropriate BMPs for your project. Answer each category below pursuant to the following: "Yes" means the project will implement the site design BMP as described in Chapter 4.3 and/or Appendix E of the City Storm Water Design Manual. Discussion / justification is not required. "No" means the BMP is applicable to the project but it is not feasible to implement. Discussion / justification must be provided. "N/A" means the BMP is not applicable at the project site because the project does not include the feature that is addressed by the BMP (e.g., the project site has no existing natural areas to conserve). Discussion / justification must be provided. Site Design Requirement Applied? SD-1 Maintain Natural Drainage Pathways and Hydrologic ⊠Yes □ No \square N/A **Features** Maintain existing drainage patterns Discussion / justification if SD-1 not implemented: **SD-2** Conserve Natural Areas, Soils, and Vegetation ⊠Yes □ No □ N/A Preserve trees (see Zoning Code Art. 55 Grading & Erosion Control; Art. 62 Landscape Regulations) Avoid sensitive areas such as wetlands and waterways П Discussion / justification if SD-2 not implemented: **SD-3** Minimize Impervious Area ⊠Yes □ N/A \square No Install parking and driving aisles to minimum width required to meet standards Discussion / justification if SD-3 not implemented:

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP
20 of 42

SD-4 Minimize Soil Compaction	⊠Yes	□ No	□ N/A
□ Avoid compaction in planned landscaped spaces			
□ Till and amend soil for improved infiltration capacity			
Discussion / justification if SD-4 not implemented:			
SD-5 Impervious Area Dispersion	□Yes	⊠No	□ N/A
 Drain rooftops, roads or sidewalks into adjacent landscape 	areas		
□ Drain impervious surfaces through pervious areas			
Discussion / justification if SD-5 not implemented:			
Impervious area dispersion is not applicable			
SD-6 Runoff Collection		□Yes	
Discussion / justification if SD-6 not implemented:	⊠Yes	□ No	□ N/A
SD-7 Landscaping with Native or Drought Tolerant Species	I	1	I
Discussion / justification if SD-7 not implemented:	⊠Yes	□ No	□ N/A
SD-8 Harvesting and Using Precipitation			
Discussion / justification if SD-8 not implemented:	□Yes	⊠No	□ N/A
Harvest and Use not feasible per harvest and use feasibility			,, .
screening (see Attachment 1).			

Note: Show all site design measures described above that are included in design capture volume calculations in the plan sheets of Attachment 5.

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 21 of 42

Step 6: PDP Structural BMPs (Form I-3)

All PDPs must implement structural BMPs for storm water pollutant control (see Chapter 5 of the Storm Water Design Manual). Selection of PDP structural BMPs for storm water pollutant control must be based on the selection process described in Chapter 5. PDPs subject to hydromodification management requirements must also implement structural BMPs for flow control for hydromodification management (see Chapter 6 of the Storm Water Design Manual). Both storm water pollutant control and flow control for hydromodification management can be achieved within the same structural BMP(s).

PDP structural BMPs must be verified by the City at the completion of construction. This may include requiring the project owner or project owner's representative and engineer of record to certify construction of the structural BMPs (see Section 8.2.3.2 of the Storm Water Design Manual). PDP structural BMPs must be maintained into perpetuity, and the City must confirm the maintenance (see Section 7 of the Storm Water Design Manual).

Use this section to provide narrative description of the general strategy for structural BMP implementation at the project site in the box below. Then complete the PDP structural BMP summary information sheet (Step 6.2) for each structural BMP within the project (copy the BMP summary information sheet [Step 6.2] as many times as needed to provide summary information for each individual structural BMP).

Step 6.1: Description of structural BMP strategy

Describe the general strategy for structural BMP implementation at the site. This information must describe how the steps for selecting and designing storm water pollutant control BMPs presented in Section 5.1 of the Storm Water Design Manual were followed, and the results (type of BMPs selected). For projects requiring hydromodification flow control BMPs, indicate whether pollutant control and flow control BMPs are integrated or separate. At the end of this discussion provide a summary of all the structural BMPs within the project including the type and number.

The selection, sizing, and design of storm water treatment BMP was performed based on guidance provided by the City of Escondido's storm water design manual.

The improvements along Valley Boulevard, Grand Avenue, and Fig Street will utilize USEPA Green Street techniques to provide water quality treatment.

Tree-wells will be used to provide water quality treatment for DMA 12.

Infiltration is infeasible and is justified by Geocon's geotechnical investigation (see Attachment 8). If infiltration of storm water occurs, downstream properties may be subjected to seeps, springs, slope instability, raised groundwater, movement of foundations and slabs, or other undesirable impacts as a result of water infiltration.

See flow chart from the City of Escondido's storm water design manual (see below).

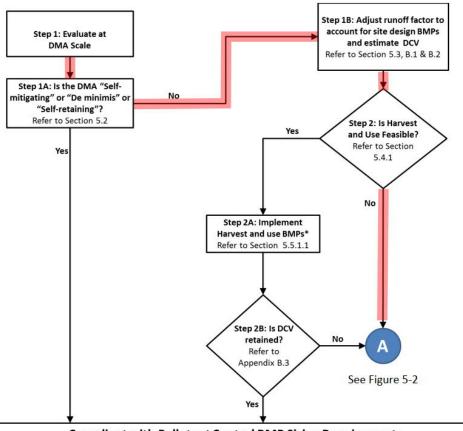
See also the County of San Diego Automated Stormwater Pollutant Control Worksheet (Version 2.0) that demonstrates that the project meets retention requirements.

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP
22 of 42

Description of structural **BMP** strategy continued (Page reserved for continuation of description of general strategy for structural **BMP** implementation at the site)

(Continued from previous page)

Chapter 5: Storm Water Pollutant Control Requirements for PDPs



Compliant with Pollutant Control BMP Sizing Requirements

Steps 6 & 7: Prepare O&M requirements and Storm Water Quality Management Plan – Refer to Chapters 7 and 8

FIGURE 5-1. Storm Water Pollutant Control BMP Selection Flow Chart

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 23 of 42

^{*} Step 2C: Project applicant has an option to also conduct feasibility analysis for infiltration and if infiltration is fully or partially feasible has an option to choose between infiltration and harvest and use BMPs. But if infiltration is not feasible and harvest and use is feasible, project applicant must implement harvest and use BMPs

Description of structural BMP strategy continued (Page reserved for continuation of description of general strategy for structural BMP Implementation at the site)

(Continued from previous page)

Infiltration Feasible? Refer to Section Step 3A&B: Full Step 3A&B: Partial Step 3A&B: No Infiltration Infiltration Infiltration Condition Condition Condition Refer to Section Refer to Section Refer to Section 5.5.1.2 5.5.2 Step 3C: Compute Step 3C: Compute Step 3C: Compute **Sizing Requirements** Sizing Requirements Sizing Requirements Refer to Appendix B.4 Refer to Appendix B.5 Refer to Appendix B.5 Step 4B: Implement BMP Step 4: Can th using maximum feasible RMP he footprint and document Yes designed for why the remaining BMP the remaining size could not fit on site* Refer to Appendix B.5 and E Step 4A: Design BMP for the required size. per design criteria and considerations listed in the fact sheets Step 5: Implement Flow-Step 5A: Participate in Refer to Appendix E thru Treatment Control AND Alternative Compliance BMPs Refer to Section 5.5.4 Program and Appendix B.6 Refer to Section 1.8 **Compliant with Pollutant Control BMP Sizing Requirements** Steps 6 & 7: Prepare O&M requirements and Storm Water Quality Management Plan – Refer to Chapters 7 and 8

Chapter 5: Storm Water Pollutant Control Requirements for PDPs

FIGURE 5-2. Storm Water Pollutant Control BMP Selection Flow Chart

Template Date: October 2016 Preparation Date: 6/18/2019] PDP SWQMP 24 of 42

^{*} Project approval at the discretion of [City Engineer]

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 25 of 42

Step 6.2: Structural BMP Checklist

(Copy this page as needed to provide information for each individual proposed structural BMP)	
Structural BMP ID No. MWS-1	
Construction Plan Sheet No. TBD	
Type of structural BMP:	
☐Retention by harvest and use (HU-1)	
□Retention by infiltration basin (INF-1)	
□Retention by bioretention (INF-2)	
□Retention by permeable pavement (INF-3)	
□Partial retention by biofiltration with partial retention (PR-1)	
□Biofiltration (BF-1)	
⊠Biofiltration with Nutrient Sensitive Media Design (BF-2)	
□ Proprietary Biofiltration (BF-3) meeting all requirements of Appendix F	
☐Flow-thru treatment control with prior lawful approval to meet earlier PDP requirements	
(provide BMP type/description in discussion section below)	
□ Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or	
biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)	
□ Flow-thru treatment control with alternative compliance (provide BMP type/description in	
discussion section below)	
□ Detention pond or vault for hydromodification management	
☐ Other (describe in discussion section below)	
Purpose:	
⊠Pollutant control only	
☐ Hydromodification control only	
□Combined pollutant control and hydromodification control	
□ Pre-treatment/forebay for another structural BMP	
□Other (describe in discussion section below)	
M/ha will contifu construction of this DMD2	TOD
Who will certify construction of this BMP? Provide name and contact information for the	TBD
party responsible to sign BMP verification	
forms (See Section 8.2.3.2 of the Storm Water	
Design Manual)	
Who will be the final owner of this BMP?	⊠HOA □ Property Owner □ City
	□Other (describe)
Who will maintain this BMP into perpetuity?	⊠HOA □ Property Owner □ City
	□Other (describe)
Discussion (as needed):	
(Continue on outposition	
(Continue on subsequent pages as necessary)	

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 27 of 42

	nformation for each individual proposed ral BMP)	
Structural BMP ID No. MWS-2		
Construction Plan Sheet No. TBD		
Type of structural BMP:		
☐Retention by harvest and use (HU-1)		
□Retention by infiltration basin (INF-1)		
□Retention by bioretention (INF-2)		
□Retention by permeable pavement (INF-3)		
☐ Partial retention by biofiltration with partial retention (PR-1)		
☐Biofiltration (BF-1)		
⊠Biofiltration with Nutrient Sensitive Media Design (BF-2)		
□ Proprietary Biofiltration (BF-3) meeting all requirements of Appendix F		
Flow-thru treatment control with prior lawful ap	•	
(provide BMP type/description in discussion s □ Flow-thru treatment control included as pre-tre	•	
·	·	
biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)		
□ Flow-thru treatment control with alternative compliance (provide BMP type/description in		
discussion section below)		
☐ Detention pond or vault for hydromodification	management	
☐ Other (describe in discussion section below)		
Durnaga		
Purpose: ⊠Pollutant control only		
☐ Hydromodification control only		
☐ Combined pollutant control and hydromodification control		
□ Pre-treatment/forebay for another structural BMP		
☐ Other (describe in discussion section below)		
,		
Who will certify construction of this BMP?	TBD	
Provide name and contact information for the		
party responsible to sign BMP verification		
forms (See Section 8.2.3.2 of the Storm Water Design Manual)		
Who will be the final owner of this BMP?	⊠HOA □Property Owner □City	
	□Other (describe)	
Who will maintain this BMP into perpetuity?		
	□Other (describe)	
Discussion (as needed):	,	
(Continue on subsequent pages as necessary)		

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 28 of 42

(Copy this page as needed to provide information for each individual proposed structural BMP)	
Structural BMP ID No. MWS-3	,
Construction Plan Sheet No. TBD	
Type of structural BMP:	
☐Retention by harvest and use (HU-1)	
☐Retention by infiltration basin (INF-1)	
□Retention by bioretention (INF-2)	
☐Retention by permeable pavement (INF-3)	
☐ Partial retention by biofiltration with partial rete	ention (PR-1)
☐Biofiltration (BF-1)	
⊠Biofiltration with Nutrient Sensitive Media Design (BF-2)	
□ Proprietary Biofiltration (BF-3) meeting all requ	• •
□Flow-thru treatment control with prior lawful ap	•
(provide BMP type/description in discussion s	
☐ Flow-thru treatment control included as pre-tre biofiltration BMP (provide BMP type/description	
biofiltration BMP it serves in discussion section	
□Flow-thru treatment control with alternative co	,
discussion section below)	
☐ Detention pond or vault for hydromodification	management
☐Other (describe in discussion section below)	-
Purpose:	
☑Pollutant control only	
☐ Hydromodification control only	tion control
□Combined pollutant control and hydromodification control	
□ Pre-treatment/forebay for another structural BMP	
☐Other (describe in discussion section below)	
Who will certify construction of this BMP?	TBD
Provide name and contact information for the	
party responsible to sign BMP verification	
forms (See Section 8.2.3.2 of the Storm Water	
Design Manual)	
Who will be the final owner of this BMP?	⊠HOA □ Property Owner □ City
Miles will assist the factle in this DMD into a secretarity of	Other (describe)
Who will maintain this BMP into perpetuity?	⊠HOA □ Property Owner □ City
Discussion (so posided):	☐ Other (describe)
Discussion (as needed):	
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Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 29 of 42

	nformation for each individual proposed	
structural BMP)		
Structural BMP ID No. MWS-4 Construction Plan Sheet No. TBD		
Type of structural BMP:		
□Retention by harvest and use (HU-1)		
☐Retention by infiltration basin (INF-1)	·	
□Retention by bioretention (INF-2)		
□Retention by permeable pavement (INF-3)		
□Partial retention by biofiltration with partial retention (PR-1)		
☐Biofiltration (BF-1)		
⊠Biofiltration with Nutrient Sensitive Media Design (BF-2)		
□Proprietary Biofiltration (BF-3) meeting all requirements of Appendix F		
☐ Flow-thru treatment control with prior lawful ap	·	
(provide BMP type/description in discussion s □ Flow-thru treatment control included as pre-tre	· · · · · · · · · · · · · · · · · · ·	
·	•	
biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)		
	☐ Flow-thru treatment control with alternative compliance (provide BMP type/description in	
discussion section below)		
☐ Detention pond or vault for hydromodification	management	
☐Other (describe in discussion section below)		
Purpose:		
☑Pollutant control only		
☐ Hydromodification control only		
□Combined pollutant control and hydromodification control		
□ Pre-treatment/forebay for another structural BMP		
□Other (describe in discussion section below)		
Who will certify construction of this BMP?	твр	
Provide name and contact information for the		
party responsible to sign BMP verification		
forms (See Section 8.2.3.2 of the Storm Water		
Design Manual) Who will be the final owner of this BMP?	EXILOA Due re entre Occurs en Doite	
who will be the final owner of this bivip?	⊠HOA □ Property Owner □ City	
Who will maintain this BMP into perpetuity?	□Other (describe) ⊠HOA □Property Owner □City	
Who will maintain this bivil into perpetuity:	⊠HOA □ Property Owner □ City□ Other (describe)	
Discussion (as needed):		
(Continue on subsequent pages as necessary)		

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 30 of 42

(Copy this page as needed to provide informati structural BMP)	(Copy this page as needed to provide information for each individual proposed structural BMP)	
Structural BMP ID No. MWS-5		
Construction Plan Sheet No. TBD		
Type of structural BMP:		
☐Retention by harvest and use (HU-1)		
☐Retention by infiltration basin (INF-1)		
□ Retention by bioretention (INF-2)		
☐Retention by permeable pavement (INF-3)		
□ Partial retention by biofiltration with partial retention (PR-1)		
☐ Biofiltration (BF-1)		
⊠Biofiltration with Nutrient Sensitive Media Design (BF-2)		
□ Proprietary Biofiltration (BF-3) meeting all requirements of Appendix F		
☐ Flow-thru treatment control with prior lawful approval to (provide BMP type/description in discussion section be	•	
☐ Flow-thru treatment control included as pre-treatment/	·	
biofiltration BMP (provide BMP type/description and in	•	
biofiltration BMP it serves in discussion section below)		
□Flow-thru treatment control with alternative compliance (provide BMP type/description in		
discussion section below)		
□ Detention pond or vault for hydromodification manager	ment	
☐Other (describe in discussion section below)		
Purpose:		
☑Pollutant control only		
☐ Hydromodification control only		
☐ Combined pollutant control and hydromodification control		
□ Pre-treatment/forebay for another structural BMP		
□Other (describe in discussion section below)		
,		
Who will certify construction of this BMP?		
Provide name and contact information for the		
party responsible to sign BMP verification forms (See Section 8.2.3.2 of the Storm Water		
Design Manual)		
Who will be the final owner of this BMP? ⊠HOA	□ Property Owner □ City	
□Othe	r (describe)	
Who will maintain this BMP into perpetuity? ⊠HOA	☐ Property Owner ☐ City	
□Othe	r (describe)	
Discussion (as needed):		
(Continue on or head worth news		
(Continue on subsequent pages as necessary)		

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 31 of 42

	nformation for each individual proposed ral BMP)	
Structural BMP ID No. MWS-6		
Construction Plan Sheet No. TBD		
Type of structural BMP:		
☐Retention by harvest and use (HU-1)		
☐Retention by infiltration basin (INF-1)		
□Retention by bioretention (INF-2)		
□Retention by permeable pavement (INF-3)		
□Partial retention by biofiltration with partial retention (PR-1)		
☐Biofiltration (BF-1)		
⊠Biofiltration with Nutrient Sensitive Media Design (BF-2)		
☐ Proprietary Biofiltration (BF-3) meeting all requ	• •	
☐Flow-thru treatment control with prior lawful ap	·	
(provide BMP type/description in discussion s	•	
□ Flow-thru treatment control included as pre-tre		
biofiltration BMP (provide BMP type/description and indicate which onsite retention or		
biofiltration BMP it serves in discussion section	·	
☐ Flow-thru treatment control with alternative compliance (provide BMP type/description in discussion section below)		
☐ Detention pond or vault for hydromodification	management	
☐Other (describe in discussion section below)		
Purpose:		
☐ Hydromodification control only		
☐ Combined pollutant control and hydromodifica		
□Pre-treatment/forebay for another structural BMP		
□Other (describe in discussion section below)		
Who will cortifu construction of this DMD2	ТВD	
Who will certify construction of this BMP? Provide name and contact information for the	ן טאון 	
party responsible to sign BMP verification		
forms (See Section 8.2.3.2 of the Storm Water		
Design Manual)		
Who will be the final owner of this BMP?	⊠HOA □ Property Owner □ City	
	☐Other (describe)	
Who will maintain this BMP into perpetuity?	⊠HOA □ Property Owner □ City	
	□Other (describe)	
Discussion (as needed):		
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(Continue on subsequent pages as necessary)		

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 32 of 42

(Copy this page as needed to provide information for each individual proposed structural BMP)	
Structural BMP ID No. MWS-7-A	
Construction Plan Sheet No. TBD	
Type of structural BMP:	
☐Retention by harvest and use (HU-1)	
□Retention by infiltration basin (INF-1)	
☐ Retention by bioretention (INF-2)	
☐Retention by permeable pavement (INF-3)	
☐ Partial retention by biofiltration with partial retention (PR-1)	
☐Biofiltration (BF-1)	
⊠Biofiltration with Nutrient Sensitive Media Design (BF-2)	
□ Proprietary Biofiltration (BF-3) meeting all requirements of Appendix F	
□ Flow-thru treatment control with prior lawful approval to meet earlier PDP requirements	
(provide BMP type/description in discussion s ☐ Flow-thru treatment control included as pre-tre	· · · · · · · · · · · · · · · · · · ·
·	•
biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)	
☐ Flow-thru treatment control with alternative compliance (provide BMP type/description in	
discussion section below)	
☐ Detention pond or vault for hydromodification	management
☐Other (describe in discussion section below)	
Durnaga	
Purpose: ⊠Pollutant control only	
☐ Hydromodification control only	
☐ Combined pollutant control and hydromodification control	
□ Pre-treatment/forebay for another structural BMP	
☐Other (describe in discussion section below)	
,	
Who will certify construction of this BMP?	TBD
Provide name and contact information for the	
party responsible to sign BMP verification	
forms (See Section 8.2.3.2 of the Storm Water Design Manual)	
Who will be the final owner of this BMP?	⊠HOA □ Property Owner □ City
Trine time se une timal entriel el une Elini I	□Other (describe)
Who will maintain this BMP into perpetuity?	
	□Other (describe)
Discussion (as needed):	(,
,	
(Continue on subsequent pages as necessary)	

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 33 of 42

	nformation for each individual proposed ral BMP)	
Structural BMP ID No. MWS-7-C	,	
Construction Plan Sheet No. TBD		
Type of structural BMP:		
☐Retention by harvest and use (HU-1)		
□Retention by infiltration basin (INF-1)		
□Retention by bioretention (INF-2)		
☐Retention by permeable pavement (INF-3)		
☐ Partial retention by biofiltration with partial retention (PR-1)		
☐Biofiltration (BF-1)		
⊠Biofiltration with Nutrient Sensitive Media Des	• , ,	
□ Proprietary Biofiltration (BF-3) meeting all requirements of Appendix F		
Flow-thru treatment control with prior lawful ap	·	
(provide BMP type/description in discussion s □ Flow-thru treatment control included as pre-tre	·	
biofiltration BMP (provide BMP type/description		
biofiltration BMP it serves in discussion section below)		
□ Flow-thru treatment control with alternative compliance (provide BMP type/description in		
discussion section below)		
□ Detention pond or vault for hydromodification	management	
☐Other (describe in discussion section below)		
Purpose:		
☐ Hydromodification control only		
☐Combined pollutant control and hydromodifica	ition control	
□ Pre-treatment/forebay for another structural BMP		
□Other (describe in discussion section below)		
, · · · · · · · · · · · · · · · · · · ·		
Who will certify construction of this BMP?	TBD	
Provide name and contact information for the		
party responsible to sign BMP verification forms (See Section 8.2.3.2 of the Storm Water		
Design Manual)		
Who will be the final owner of this BMP?	⊠HOA □ Property Owner □ City	
	□Other (describe)	
Who will maintain this BMP into perpetuity?	⊠HOA □ Property Owner □ City	
	□Other (describe)	
Discussion (as needed):	,	
(Continue on subsequent pages as necessary)		

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 34 of 42

(Copy this page as needed to provide information for each individual proposed structural BMP)		
Structural BMP ID No. MWS-7-D		
Construction Plan Sheet No. TBD		
Type of structural BMP:		
☐Retention by harvest and use (HU-1)		
□Retention by infiltration basin (INF-1)		
□ Retention by bioretention (INF-2)		
☐Retention by permeable pavement (INF-3)		
□ Partial retention by biofiltration with partial retention (PR-1)		
☐ Biofiltration (BF-1)		
⊠Biofiltration with Nutrient Sensitive Media Design (BF-2)		
□ Proprietary Biofiltration (BF-3) meeting all requirements of Appendix F		
☐ Flow-thru treatment control with prior lawful approval to m (provide BMP type/description in discussion section belo	•	
☐ Flow-thru treatment control included as pre-treatment/fore	•	
biofiltration BMP (provide BMP type/description and indic	•	
biofiltration BMP it serves in discussion section below)		
□Flow-thru treatment control with alternative compliance (provide BMP type/description in		
discussion section below)		
□ Detention pond or vault for hydromodification manageme	nt	
□Other (describe in discussion section below)		
Purpose:		
☐ Hydromodification control only		
☐ Combined pollutant control and hydromodification control		
□ Pre-treatment/forebay for another structural BMP		
□Other (describe in discussion section below)		
,		
Who will certify construction of this BMP?		
Provide name and contact information for the party responsible to sign BMP verification		
forms (See Section 8.2.3.2 of the Storm Water		
Design Manual)		
	□ Property Owner □ City	
□Other (c	describe)	
Who will maintain this BMP into perpetuity? ⊠HOA	□ Property Owner □ City	
□Other (c	describe)	
Discussion (as needed):		
(Continue on out a consent masses		
(Continue on subsequent pages as necessary)		

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 35 of 42

	(Copy this page as needed to provide information for each individual proposed structural BMP)	
Structural BMP ID No. MWS-8		
Construction Plan Sheet No. TBD		
Type of structural BMP:		
☐Retention by harvest and use (HU-1)		
☐Retention by infiltration basin (INF-1)		
□Retention by bioretention (INF-2)		
□Retention by permeable pavement (INF-3)		
□Partial retention by biofiltration with partial retention (PR-1)		
☐Biofiltration (BF-1)		
⊠Biofiltration with Nutrient Sensitive Media Des	ign (BF-2)	
☐ Proprietary Biofiltration (BF-3) meeting all requ	• •	
☐Flow-thru treatment control with prior lawful ap	·	
(provide BMP type/description in discussion s		
☐ Flow-thru treatment control included as pre-tre	•	
biofiltration BMP (provide BMP type/description and indicate which onsite retention or		
biofiltration BMP it serves in discussion section below)		
☐ Flow-thru treatment control with alternative compliance (provide BMP type/description in discussion section below)		
Detention pond or vault for hydromodification	management	
Other (describe in discussion section below)	management	
,		
Purpose:		
☑Pollutant control only		
☐ Hydromodification control only		
□Combined pollutant control and hydromodification control		
□Pre-treatment/forebay for another structural BMP		
□Other (describe in discussion section below)		
M/ho will contifue construction of this DMD2		
Who will certify construction of this BMP? Provide name and contact information for the	TBD	
party responsible to sign BMP verification		
forms (See Section 8.2.3.2 of the Storm Water		
Design Manual)		
Who will be the final owner of this BMP?	⊠HOA □ Property Owner □ City	
	☐Other (describe)	
Who will maintain this BMP into perpetuity?	⊠HOA □ Property Owner □ City	
	☐Other (describe)	
Discussion (as needed):		
(Continue on outbooks at the second		
(Continue on subsequent pages as necessary)		

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 35 of 42

(Copy this page as needed to provide information for each individual page structural BMP)	oroposed	
Structural BMP ID No. MWS-9		
Construction Plan Sheet No. TBD		
Type of structural BMP:		
□Retention by harvest and use (HU-1)		
□Retention by infiltration basin (INF-1)		
□Retention by bioretention (INF-2)		
☐Retention by permeable pavement (INF-3)		
□ Partial retention by biofiltration with partial retention (PR-1)		
☐ Biofiltration (BF-1)		
⊠Biofiltration with Nutrient Sensitive Media Design (BF-2)		
□ Proprietary Biofiltration (BF-3) meeting all requirements of Appendix F		
☐ Flow-thru treatment control with prior lawful approval to meet earlier PDP requile (provide BMP type/description in discussion section below)	rements	
□ Flow-thru treatment control included as pre-treatment/forebay for an onsite rete	ention or	
biofiltration BMP (provide BMP type/description and indicate which onsite retention or		
biofiltration BMP it serves in discussion section below)		
□Flow-thru treatment control with alternative compliance (provide BMP type/description in		
discussion section below)		
□ Detention pond or vault for hydromodification management		
□Other (describe in discussion section below)		
Purpose:		
☐ Hydromodification control only		
☐ Combined pollutant control and hydromodification control		
□ Pre-treatment/forebay for another structural BMP		
□Other (describe in discussion section below)		
,		
Who will certify construction of this BMP?		
Provide name and contact information for the		
party responsible to sign BMP verification forms (See Section 8.2.3.2 of the Storm Water		
Design Manual)		
	City	
□Other (describe)	·	
Who will maintain this BMP into perpetuity? ⊠HOA □Property Owner □	City	
□Other (describe)		
Discussion (as needed):		
(Continue on outperment name of		
(Continue on subsequent pages as necessary)		

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 36 of 42

	nformation for each individual proposed ral BMP)	
Structural BMP ID No. MWS-10		
Construction Plan Sheet No. TBD		
Type of structural BMP:		
☐Retention by harvest and use (HU-1)		
☐Retention by infiltration basin (INF-1)		
☐Retention by bioretention (INF-2)		
☐Retention by permeable pavement (INF-3)		
□ Partial retention by biofiltration with partial retention (PR-1)		
☐Biofiltration (BF-1)		
⊠Biofiltration with Nutrient Sensitive Media Design (BF-2)		
☐ Proprietary Biofiltration (BF-3) meeting all req	·	
☐ Flow-thru treatment control with prior lawful ap	·	
(provide BMP type/description in discussion s	· · · · · · · · · · · · · · · · · · ·	
□ Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or		
biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below)		
	·	
☐ Flow-thru treatment control with alternative compliance (provide BMP type/description in discussion section below)		
☐ Detention pond or vault for hydromodification	management	
□Other (describe in discussion section below)		
Purpose:		
☑Pollutant control only		
☐ Hydromodification control only		
Combined pollutant control and hydromodification control		
1	□ Pre-treatment/forebay for another structural BMP	
□Other (describe in discussion section below)		
Who will certify construction of this BMP?	тво	
Provide name and contact information for the	טטו	
party responsible to sign BMP verification		
forms (See Section 8.2.3.2 of the Storm Water		
Design Manual)		
Who will be the final owner of this BMP?	⊠HOA □ Property Owner □ City	
	☐Other (describe)	
Who will maintain this BMP into perpetuity?	⊠HOA □ Property Owner □ City	
	□Other (describe)	
Discussion (as needed):		
(Continue on subsequent pages as necessary)		
(Continue on Subsequent pages as necessary)		

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 37 of 42

	nformation for each individual proposed ral BMP)
Structural BMP ID No. MWS-11	
Construction Plan Sheet No. TBD	
Type of structural BMP:	
☐Retention by harvest and use (HU-1)	
☐Retention by infiltration basin (INF-1)	
□Retention by bioretention (INF-2)	
☐Retention by permeable pavement (INF-3)	
□ Partial retention by biofiltration with partial retention (PR-1)	
☐Biofiltration (BF-1)	
⊠Biofiltration with Nutrient Sensitive Media Design (BF-2)	
☐ Proprietary Biofiltration (BF-3) meeting all req	·
☐ Flow-thru treatment control with prior lawful ap	·
(provide BMP type/description in discussion s	· · · · · · · · · · · · · · · · · · ·
Flow-thru treatment control included as pre-tre	•
biofiltration BMP (provide BMP type/description and indicate which onsite retention or	
biofiltration BMP it serves in discussion section below)	
☐ Flow-thru treatment control with alternative compliance (provide BMP type/description in discussion section below)	
□ Detention pond or vault for hydromodification	management
☐Other (describe in discussion section below)	s.ragee.r
Purpose:	
☐ Hydromodification control only	
□Combined pollutant control and hydromodification control	
□ Pre-treatment/forebay for another structural BMP	
□Other (describe in discussion section below)	
Who will certify construction of this BMP?	тво
Provide name and contact information for the	טסון
party responsible to sign BMP verification	
forms (See Section 8.2.3.2 of the Storm Water	
Design Manual)	
Who will be the final owner of this BMP?	⊠HOA □ Property Owner □ City
	☐Other (describe)
Who will maintain this BMP into perpetuity?	⊠HOA □ Property Owner □ City
	☐Other (describe)
Discussion (as needed):	
(Continue on subsequent pages as passesser)	
(Continue on subsequent pages as necessary)	

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 38 of 42

	nformation for each individual proposed ral BMP)
Structural BMP ID No. MWS-12	
Construction Plan Sheet No. TBD	
Type of structural BMP:	
☐Retention by harvest and use (HU-1)	
☐Retention by infiltration basin (INF-1)	
☐Retention by bioretention (INF-2)	
☐Retention by permeable pavement (INF-3)	
\square Partial retention by biofiltration with partial rete	ention (PR-1)
☐Biofiltration (BF-1)	
⊠Biofiltration with Nutrient Sensitive Media Des	
☐ Proprietary Biofiltration (BF-3) meeting all req	·
□ Flow-thru treatment control with prior lawful ap	·
(provide BMP type/description in discussion s	•
Flow-thru treatment control included as pre-tre	•
biofiltration BMP (provide BMP type/description biofiltration BMP it serves in discussion sections.)	
☐ Flow-thru treatment control with alternative co	·
discussion section below)	impliance (previde Bitti type/decemption in
Detention pond or vault for hydromodification	management
☐Other (describe in discussion section below)	J
Purpose:	
⊠Pollutant control only	
☐ Hydromodification control only	
☐Combined pollutant control and hydromodifica	
□ Pre-treatment/forebay for another structural B	MP
☐Other (describe in discussion section below)	
Who will certify construction of this BMP?	тво
Provide name and contact information for the	טפון
party responsible to sign BMP verification	
forms (See Section 8.2.3.2 of the Storm Water	
Design Manual)	
Who will be the final owner of this BMP?	⊠HOA □ Property Owner □ City
	☐Other (describe)
Who will maintain this BMP into perpetuity?	⊠HOA □ Property Owner □ City
	□Other (describe)
Discussion (as needed):	
(Continue on subsequent pages as necessary)	
(Continue on Subsequent pages as necessary)	

Template Date: October 2016 Preparation Date: 6/18/2019]
PDP SWQMP 39 of 42

Step 6.3: Offsite Alternative Compliance Participation Form

THIS FORM IS NOT APPLICABLE AT THIS TIME	IE [:] An Alternative Compliance Program is
under consideration by the City of Escondido	
PDP INFORMATION	
Record ID:	
Assessor's Parcel Number(s) [APN(s)]	
What are your PDP Pollutant Control Debits? *See Attachment 1 of the PDP SWQMP	
What are your PDP HMP Debits? (if applicable) *See Attachment 2 of the PDP SWQMP	
ACP Information	
Record ID:	
Assessor's Parcel Number(s) [APN(s)]	
Project Owner/Address	
What are your ACP Pollutant Control Credits? *See Attachment 1 of the ACP SWQMP	
What are your ACP HMP Debits? (if applicable) *See Attachment 2 of the ACP SWQMP	
Is your ACP in the same watershed as your PDP? ☐Yes ☐No	Will your ACP project be completed prior to the completion of the PDP? ☐Yes ☐No
Does your ACP account for all Deficits generated by the PDP? Yes No (PDP and/or ACP must be redesigned to account for all deficits generated by the PDP.)	What is the difference between your PDP debits and ACP Credits? *(ACP Credits -Total PDP Debits = Total Earned Credits)

Template Date: October 2016 Preparation Date: 6/18/2019] PDP SWQMP

38 of 42

Template Date: October 2016

PDP SWQMP - Attachments

Preparation Date: 6/18/2019

ATTACHMENT 1

BACKUP FOR PDP POLLUTANT CONTROL BMPS

This is the cover sheet for Attachment 1.

Indicate which Items are Included behind this cover sheet:

Attachment	Contents	Checklist
Sequence	Contents	Oncomist
Attachment 1a	Storm Water Pollutant Control Worksheet Calculations -Worksheet B.2-1 (Required) -Worksheet B.3-1 (Form I-4; Required) -Worksheet B.4-1 (if applicable) -Worksheet B.5-1 (if applicable) -Worksheet B.5-2 (if applicable) -Worksheet B.5-3 (if applicable) -Worksheet B.6-1 (if applicable) -Summary Worksheet (optional)	⊠Included
Attachment 1b	Form I-5, Categorization of Infiltration Feasibility Condition (Required unless the project will use harvest and use BMPs) Refer to Appendices C and D of the Storm Water Design Manual to complete Form I-5.	
Attachment 1c	Form I-6, Factor of Safety and Design Infiltration Rate Worksheet (Required unless the project will use harvest and use BMPs) Refer to Appendices C and D of the Storm Water Design Manual to complete Form I-6.	project will use harvest and use BMPs
Attachment 1d	DMA Exhibit (Required) See DMA Exhibit Checklist on the back of this Attachment cover sheet.	⊠Included
Attachment 1e	Individual Structural BMP DMA Mapbook (Required) -Place each map on 8.5"x11" paperShow at a minimum the DMA, Structural BMP, and any existing hydrologic features within the DMA.	⊠Included

Template Date: October 2016 Preparation Date: 6/18/2019

PDP SWQMP - Attachments

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Template Date: October 2016 Preparation Date: 6/18/2019 PDP SWQMP - Attachments

PALOMAR HEIGHTS BIOFILTRATION BMP DMA CALCULATIONS

					Fraction of		Pervious	Summation		Fraction		Pervious	Summation		Fraction			Summation
	Imp. RF	Pervious RF	% Imp	DMA 1	Total	Imp Area	Area	RF x A	DMA 2	of Total	Imp Area	Area	RF x A	DMA 3	of Total	Imp Area	Pervious Area	RF x A
	imp. it.	T CI VIOUS	70 IIIIp	SQFT	Total	SQFT	SQFT	IN AA	SQFT	Oi Tota.	SOFT	SQFT	IN AA	SQFT	Or rota.	SOFT	SQFT	IN AA
BASIN	0.90	0.10	0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
INDUSTRIAL	0.90	0.10	87	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MULTIUSE/COMMERCIAL	0.90	0.10	90	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PARK	0.90	0.10	10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
RESIDENTIAL/PATIOS	0.90	0.10	90	23643.50	0.61	21279	2364.35	19388	23644	0.65	21279.15	2364.35	19387.67	16002.00	0.69	14401.80	1600.20	13121.64
ROAD/DRIVEWAY	0.90	0.10	85	11422.00	0.28	9709	1713.30	8909	10832	0.28	9207.20	1624.80	8448.96	5884.00	0.24	5001.40	882.60	4589.52
SCHOOL	0.90	0.10	80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
SLOPES	0.90	0.10	0	34397.50	0.11	0.00	34397.500	3440	19072	0.06	0.00	19071.50	1907.15	12644.00	0.07	0.00	12644.00	1264.40
		1																
		İ		69463.00	1.00	30987.85	38475.15	31736.58	53547.00	1.00	30486.35	23060.65	29743.78	34530.00	1.00	19403.20	15126.80	18975.56
							Weighted C =	0.46			We	eighted C =	0.56				Weighted C =	0.55
		1										· J					<u> </u>	
		1			Fraction of		Pervious	Summation		Fraction		Pervious	Summation		Fraction			Summation
	Imp. RF	Pervious RF	% Imp	DMA 4	Total	Imp Area	Area	RF x A	DMA 5	of Total	Imp Area	Area	RF x A	DMA 6	of Total	Imp Area	Pervious Area	RF x A
		1		SQFT		SQFT	SQFT		SQFT		SQFT	SQFT		SQFT		SQFT	SQFT	
BASIN	0.90	0.10	0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
INDUSTRIAL	0.90	0.10	87	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MULTIUSE/COMMERCIAL	0.90	0.10	90	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PARK	0.90	0.10	10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
RESIDENTIAL/PATIOS	0.90	0.10	90	0.00	0.00	0.00	0.00	0.00	26652	0.58	23986.80	2665.20	21854.64	36953.00	0.56	33257.70	3695.30	30301.46
ROAD/DRIVEWAY	0.90	0.10	85	14183.00	0.85	12055.55	2127.45	11062.74	19633	0.40	16688.05	2944.95	15313.74	27532.00	0.40	23402.20	4129.80	21474.96
SCHOOL	0.90	0.10	80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
SLOPES	0.90	0.10	0	19604.00	0.15	0.00	19604.00	1960.40	6818	0.02	0.00	6818.00	681.80	19187.30	0.04	0.00	19187.30	1918.73
		Ì																
		İ		33787.00	1.00	12055.55	21731.45	13023.14	53103.00	1.00	40674.85	12428.15	37850.18	83672.30	1.00	56659.90	27012.40	53695.15
		1			•		Weighted C =	0.39			We	eighted C =	0.71				Weighted C =	0.64
		•											•					
		1			Fraction of		Pervious	Summation		Fraction		Pervious	Summation		Fraction			Summation
	Imp. RF	Pervious RF	% Imp	DMA-7-A	Total	Imp Area	Area	RF x A	DMA-7-B	of Total	Imp Area	Area	RF x A	DMA-7-C	of Total	Imp Area	Pervious Area	RF x A
				SQFT		SQFT	SQFT		SQFT		SQFT	SQFT		SQFT		SQFT	SQFT	
BASIN	0.90	0.10	0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
INDUSTRIAL	0.90	0.10	87	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MULTIUSE/COMMERCIAL	0.90	0.10	90	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PARK	0.90	0.10	10	0.00	0.00	0.00	0.00	0.00	5839.00	0.88	583.90	5255.10	1051.02	0.00	0.00	0.00	0.00	0.00
RESIDENTIAL/PATIOS	0.90	0.10	90	9134.00	0.63	8220.60	913.40	7489.88	0.00	0.00	0.00	0.00	0.00	9134.00	0.63	8220.60	913.40	7489.88
ROAD/DRIVEWAY	0.90	0.10	85	5362.00	0.35	4557.70	804.30	4182.36	0.00	0.00	0.00	0.00	0.00	5362.00	0.35	4557.70	804.30	4182.36
SCHOOL	0.90	0.10	80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
SLOPES	0.90	0.10	0	2666.20	0.02	0.00	2666.20	266.62	1500.00	0.12	0.00	1500.00	150.00	1309.00	0.01	0.00	1309.00	130.90
				17162.20	1.00	12778.30	4383.90	11938.86	7339.00	1.00	583.90	6755.10	1201.02	15805.00	1.00	12778.30	3026.70	11803.14
							Weighted C =	0.70			We	eighted C =	0.16				Weighted C =	0.75

					Fraction of		Pervious	Summation
	Imp. RF	Pervious RF	% Imp	DMA-7-D	Total	Imp Area	Area	RF x A
				SQFT		SQFT	SQFT	
PARK	0.90	0.10	10	0.00	0.00	0.00	0.00	0.00
RESIDENTIAL/PATIOS	0.90	0.10	90	0.00	0.00	0.00	0.00	0.00
ROAD/DRIVEWAY	0.90	0.10	85	3163.00	1.00	2688.55	474.45	2467.14
SCHOOL	0.90	0.10	80	0.00	0.00	0.00	0.00	0.00
SLOPES	0.90	0.10	0	0.00	0.00	0.00	0.00	0.00
				3163.00	1.00	2688.55	474.45	2467.14
							Weighted C =	0.78

PALOMAR HEIGHTS BIOFILTRATION BMP DMA CALCULATIONS

					Fraction of		Pervious	Summation		Fraction		Pervious	Summation
	Imp. RF	Pervious RF	% Imp	DMA 8	Total	Imp Area	Area	RF x A	DMA 9	of Total	Imp Area	Area	RF x A
				SQFT		SQFT	SQFT		SQFT		SQFT	SQFT	
BASIN	0.90	0.10	0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
INDUSTRIAL	0.90	0.10	87	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MULTIUSE/COMMERCIAL	0.90	0.10	90	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PARK	0.90	0.10	10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
RESIDENTIAL/PATIOS	0.90	0.10	90	26652.00	0.54	23986.80	2665.20	21854.64	33951.00	0.52	30555.90	3395.10	27839.82
ROAD/DRIVEWAY	0.90	0.10	85	23533.00	0.45	20003.05	3529.95	18355.74	30591.00	0.45	26002.35	4588.65	23860.98
SCHOOL	0.90	0.10	80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
SLOPES	0.90	0.10	0	6362.00	0.02	0.00	6362.00	636.20	17866.00	0.03	0.00	17866.00	1786.60
				56547.00	1.00	43989.85	12557.15	40846.58	82408.00	1.00	56558.25	25849.75	53487.40
							Weighted C =	0.72			We	eighted C =	0.65

					Fraction of		Pervious	Summation		Fraction		Pervious	Summation		Fraction			Summation
	Imp. RF	Pervious RF	% Imp	DMA 10	Total	Imp Area	Area	RF x A	DMA 11	of Total	Imp Area	Area	RF x A	DMA 12	of Total	Imp Area	Pervious Area	RF x A
				SQFT		SQFT	SQFT		SQFT		SQFT	SQFT		SQFT		SQFT	SQFT	
BASIN	0.90	0.10	0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
INDUSTRIAL	0.90	0.10	87	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
MULTIUSE/COMMERCIAL	0.90	0.10	90	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
PARK	0.90	0.10	10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
RESIDENTIAL/PATIOS	0.90	0.10	90	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	27454.00	0.86	24708.60	2745.40	22512.28
ROAD/DRIVEWAY	0.90	0.10	85	7940.00	1.00	6749.00	1191.00	6193.20	5054.00	1.00	4295.90	758.10	3942.12	2830.00	0.08	2405.50	424.50	2207.40
SCHOOL	0.90	0.10	80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
SLOPES	0.90	0.10	0	0.00	0.00	0.00	0.00	0.00	0	0.00	0.00	0.00	0.00	14430.00	0.06	0.00	14430.00	1443.00
				7940.00	1.00	6749.00	1191.00	6193.20	5054.00	1.00	4295.90	758.10	3942.12	44714.00	1.00	27114.10	17599.90	26162.68
							Weighted C =	0.78			We	eighted C =	0.78	•		•	Weighted C =	0.59

	DMA 1: Design Capture Volume	Worksheet B-2.1					
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches			
2	Area tributary to BMP (s)	A=	1.595	acres			
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.46	unitless			
4	Street trees volume reduction	TCV=	0.00	cubic-feet			
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet			
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	1,481	cubic-feet			

	DMA 2: Design Capture Volume		Worksheet B-2.1	
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches
2	Area tributary to BMP (s)	A=	1.23	acres
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.56	unitless
4	Street trees volume reduction	TCV=	0.00	cubic-feet
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	1,388	cubic-feet

	DMA 3: Design Capture Volume		Worksheet B-2.1	
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches
2	Area tributary to BMP (s)	A=	0.79	acres
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.55	unitless
4	Street trees volume reduction	TCV=	0.00	cubic-feet
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	886	cubic-feet

	DMA 4: Design Capture Volume		Worksheet B-2.1	
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches
2	Area tributary to BMP (s)	A=	0.78	acres
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.39	unitless
4	Street trees volume reduction	TCV=	0.00	cubic-feet
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	608	cubic-feet

	DMA 5: Design Capture Volume		Worksheet B-2.1	
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches
2	Area tributary to BMP (s)	A=	1.22	acres
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.71	unitless
4	Street trees volume reduction	TCV=	0.00	cubic-feet
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	1,766	cubic-feet

	DMA 6: Design Capture Volume		Worksheet B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches	
2	Area tributary to BMP (s)	A= 1.92		acres	
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.64	unitless	
4	Street trees volume reduction	TCV=	0.00	cubic-feet	
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet	
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	2,506	cubic-feet	

	DMA-7-A: Design Capture Volume		Worksheet B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches	
2	Area tributary to BMP (s)	A= 0.39		acres	
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.70	unitless	
4	Street trees volume reduction	TCV=	0.00	cubic-feet	
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet	
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	557	cubic-feet	

	DMA-7-B: Design Capture Volume		Worksheet B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	tile 24-hr storm depth from Figure d= 0.56		inches	
2	Area tributary to BMP (s)	A= 0.17		acres	
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.16	unitless	
4	Street trees volume reduction	TCV=	0.00	cubic-feet	
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet	
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	56	cubic-feet	

	DMA-7-C: Design Capture Volume		Worksheet B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches	
2	Area tributary to BMP (s)	A= 0.36		acres	
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.75	unitless	
4	Street trees volume reduction	TCV=	0.00	cubic-feet	
5	5 Rain barrels volume reduction		0.00	cubic-feet	
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	551	cubic-feet	

DMA-7-D: Design Capture Volume		Worksheet B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches
2	Area tributary to BMP (s)	A= 0.07		acres
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.78	unitless
4	Street trees volume reduction	TCV= 0.00 c		cubic-feet
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	115	cubic-feet

DMA 8: Design Capture Volume		Worksheet B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches
2	Area tributary to BMP (s)	A= 1.30		acres
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.72	unitless
4	Street trees volume reduction	TCV=	0.00	cubic-feet
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	1,906	cubic-feet

DMA 9: Design Capture Volume		Worksheet B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches
2	Area tributary to BMP (s)	A= 1.89		acres
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.65	unitless
4	Street trees volume reduction	TCV= 0.00 cu		cubic-feet
5	Rain barrels volume reduction	RCV= 0.00 cu		cubic-feet
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	2,496	cubic-feet

	DMA 10: Design Capture Volume		Worksheet B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches	
2	Area tributary to BMP (s)	A=	0.18	acres	
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.78	unitless	
4	Street trees volume reduction	TCV=	0.00	cubic-feet	
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet	
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	289	cubic-feet	

DMA 11: Design Capture Volume		Worksheet B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches
2	Area tributary to BMP (s)	A= 0.12		acres
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.78	unitless
4	Street trees volume reduction	TCV=	0.00	cubic-feet
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	184	cubic-feet

	DMA 12: Design Capture Volume		Worksheet B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.56	inches	
2	Area tributary to BMP (s)	A=	1.03	acres	
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.59	unitless	
4	Street trees volume reduction	TCV=	0.00	cubic-feet	
5	5 Rain barrels volume reduction		0.00	cubic-feet	
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	1,221	cubic-feet	

SPECIFICATIONS

FLOW-BASED DESIGNS

The Modular Wetlands® System Linear can be used in stand-alone applications to meet treatment flow requirements. Since the Modular Wetlands® is the only biofiltration system that can accept inflow pipes several feet below the surface, it can be used not only in decentralized design applications but also as a large central end-of-the-line application for maximum feasibility.

MODEL#	DIMENSIONS	WETLANDMEDIA SURFACE AREA (sq. ft.)	TREATMENT FLOW RATE (cfs)
MWS-L-4-4	4' x 4'	23	0.052
MWS-L-4-6	4' x 6'	32	0.073
MW5-L-4-8	4' x 8'	50	0.115
MWS-L-4-13	4' x 13'	63	0.144
MWS-L-4-15	4' x 15'	76	0.175
MWS-L-4-17	4' x 17'	90	0.206
MWS-L-4-19	4' x 19'	103	0.237
MWS-L-4-21	4' x 21'	117	0.268
MWS-L-6-8	7' x 9'	64	0.147
MWS-L-8-8	8' x 8'	100	0.230
MWS-L-8-12	8' x 12'	151	0.346
MWS-L-8-16	8' x 16'	201	0.462
MWS-L-8-20	9' x 21'	252	0.577
MWS-L-8-24	9' x 25'	302	0.693
MWS-L-10-20	10' x 20'	302	0.693

	DMA 1: Flow-thru Design Flows	Worksheet B.6-1		
1	DCV	DCV	1,481	cubic-feet
2	DCV Retained	DCV Retained	0.00	cubic-feet
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet
4	1.5 DCV requiring flow-thru (Line 1 - Line 2 - 0.67*Line 3)	DCV flow-thru	1,481	cubic-feet
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless
6	Design rainfall intensity	i=	0.2	in/hr
7	Area tributary to BMP(s)	A=	1.59	acres
8	Area-weighted runoff factor (estimate using Appendix B.2)	C=	0.46	unitless
9	Calculate Flow Rate = AF x (C x i x A)	Q=	0.146	cfs
10	Treamtent Flow Rate = 1.5 X Q	Q=	0.219	cfs

- 1) Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of flow-thru BMPs. That is, if the flow-thru BMP is upstream of the project's retention and biofiltration BMPs then the flow-thru BMP shall be sized using an adjustment factor of 1.
- 2) Volume based (e.g., dry extended detention basin) flow-thru treatment control BMPs shall be sized to the volume in Line 4 and dlow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either volume in :ie 4 or flow
- 3) Propietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.

MODEL #	DIMENSIONS	WETLANDMEDIA SURFACE AREA (sq. ft.)	TREATMENT FLOW RATE (cfs)
MWS-L-4-19	4' x 19'	103	0.237

	DMA 2: Flow-thru Design Flows	Worksheet B.6-1		
1	DCV	DCV	1,388	cubic-feet
2	DCV Retained	DCV Retained	0.00	cubic-feet
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet
4	1.5 DCV requiring flow-thru (Line 1 - Line 2 - 0.67*Line 3)	DCV flow-thru	1,388	cubic-feet
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless
6	Design rainfall intensity	i=	0.2	in/hr
7	Area tributary to BMP(s)	A=	1.23	acres
8	Area-weighted runoff factor (estimate using Appendix B.2)	C=	0.56	unitless
9	Calculate Flow Rate = AF x (C x i x A)	Q=	0.14	cfs
10	Treamtent Flow Rate = 1.5 X Q	Q=	0.20	cfs

- 1) Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of flow-thru BMPs. That is, if the flow-thru BMP is upstream of the project's retention and biofiltration BMPs then the flow-thru BMP shall be sized using an adjustment factor of 1.
- 2) Volume based (e.g., dry extended detention basin) flow-thru treatment control BMPs shall be sized to the volume in Line 4 and dlow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either volume in :ie 4 or
- 3) Propietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.

MODEL #	DIMENSIONS	WETLANDMEDIA SURFACE AREA (sq. ft.)	TREATMENT FLOW RATE (cfs)
MWS-L-4-17	4' x 17'	90	0.206

DMA 3: Flow-thru Design Flows Worksheet B.6-1				
1	DCV	DCV	886	cubic-feet
2	DCV Retained	DCV Retained	0.00	cubic-feet
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet
4	1.5 DCV requiring flow-thru (Line 1 - Line 2 - 0.67*Line 3)	DCV flow-thru	886	cubic-feet
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless
6	Design rainfall intensity	i=	0.2	in/hr
7	Area tributary to BMP(s)	A=	0.79	acres
8	Area-weighted runoff factor (estimate using Appendix B.2)	C=	0.55	unitless
9	Calculate Flow Rate = AF x (C x i x A)	Q=	0.09	cfs
10	Treamtent Flow Rate = 1.5 X Q	Q=	0.13	cfs

- 1) Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of flow-thru BMPs. That is, if the flow-thru BMP is upstream of the project's retention and biofiltration BMPs then the flow-thru BMP shall be sized using an adjustment factor of 1.
- 2) Volume based (e.g., dry extended detention basin) flow-thru treatment control BMPs shall be sized to the volume in Line 4 and dlow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either volume
- 3) Propietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.

MODEL#	DIMENSIONS	WETLANDMEDIA SURFACE AREA (sq. ft.)	TREATMENT FLOW RATE (cfs)
MWS-L-4-13	4' x 13'	63	0.144

	DMA 4: Flow-thru Design Flows	Worl	ksheet B.6-1	
1	DCV	DCV	608	cubic-feet
2	DCV Retained	DCV Retained	0.00	cubic-feet
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet
4	1.5 DCV requiring flow-thru (Line 1 - Line 2 - 0.67*Line 3)	DCV flow-thru	608	cubic-feet
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless
6	Design rainfall intensity	i=	0.2	in/hr
7	Area tributary to BMP(s)	A=	0.78	acres
8	Area-weighted runoff factor (estimate using Appendix B.2)	C=	0.39	unitless
9	Calculate Flow Rate = AF x (C x i x A)	Q=	0.06	cfs
10	Treamtent Flow Rate = 1.5 X Q	Q=	0.09	cfs

- 1) Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of flow-thru BMPs. That is, if the flow-thru BMP is upstream of the project's retention and biofiltration BMPs then the flow-thru BMP shall be sized using an adjustment factor of 1.
- 2) Volume based (e.g., dry extended detention basin) flow-thru treatment control BMPs shall be sized to the volume in Line 4 and dlow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either volume
- 3) Propietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.

MODEL #	DIMENSIONS	WETLANDMEDIA SURFACE AREA (sq. ft.)	TREATMENT FLOW RATE (cfs)
MWS-L-4-8	4' x 8'	50	0.115

DMA 5: Flow-thru Design Flows Worksheet B.6-1				
1	DCV	DCV	1,766.34	cubic-feet
2	DCV Retained	DCV Retained	0.00	cubic-feet
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet
//	1.5 DCV requiring flow-thru (Line 1 - Line 2 - 0.67*Line 3)	DCV flow-thru	1,766	cubic-feet
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless
6	Design rainfall intensity	i=	0.2	in/hr
7	Area tributary to BMP(s)	A=	1.22	acres
8	Area-weighted runoff factor (estimate using Appendix B.2)	C=	0.71	unitless
	Calculate Flow Rate = AF x (C x i x A)	Q=	0.17	cfs
10	Treamtent Flow Rate = 1.5 X Q	Q=	0.26	cfs

- 1) Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of flow-thru BMPs. That is, if the flow-thru BMP is upstream of the project's retention and biofiltration BMPs then the flow-thru BMP shall be sized using an adjustment factor of 1.
- 2) Volume based (e.g., dry extended detention basin) flow-thru treatment control BMPs shall be sized to the volume in Line 4 and dlow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either volume in :ie 4 or
- 3) Propietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.

MODEL #	DIMENSIONS	WETLANDMEDIA SURFACE AREA (sq. ft.)	TREATMENT FLOW RATE (cfs)
MWS-L-4-21	4' x 21'	117	0.268

	DMA 6: Flow-thru Design Flows	Worl	ksheet B.6-1	
1	DCV	DCV	2,506	cubic-feet
2	DCV Retained	DCV Retained	0.00	cubic-feet
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet
4	1.5 DCV requiring flow-thru (Line 1 - Line 2 - 0.67*Line 3)	DCV flow-thru	2,506	cubic-feet
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless
6	Design rainfall intensity	i=	0.2	in/hr
7	Area tributary to BMP(s)	A=	1.92	acres
8	Area-weighted runoff factor (estimate using Appendix B.2)	C=	0.64	unitless
9	Calculate Flow Rate = AF x (C x i x A)	Q=	0.25	cfs
10	Treamtent Flow Rate = 1.5 X Q	Q=	0.37	cfs

- 1) Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of flow-thru BMPs. That is, if the flow-thru BMP is upstream of the project's retention and biofiltration BMPs then the flow-thru BMP shall be sized using an adjustment factor of 1.
- 2) Volume based (e.g., dry extended detention basin) flow-thru treatment control BMPs shall be sized to the volume in Line 4 and dlow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either
- 3) Propietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.

MODEL#	DIMENSIONS	WETLANDMEDIA SURFACE AREA (sq. ft.)	TREATMENT FLOW RATE (cfs)
MWS-L-8-16	8' x 16'	201	0.462

	DMA-7-A: Flow-thru Design Flows	Work	ksheet B.6-1	
1	DCV	DCV	557	cubic-feet
2	DCV Retained	DCV Retained	0.00	cubic-feet
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet
4	1.5 DCV requiring flow-thru (Line 1 - Line 2 - 0.67*Line 3)	DCV flow-thru	557	cubic-feet
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless
6	Design rainfall intensity	i=	0.2	in/hr
7	Area tributary to BMP(s)	A=	0.39	acres
8	Area-weighted runoff factor (estimate using Appendix B.2)	C=	0.70	unitless
9	Calculate Flow Rate = AF x (C x i x A)	Q=	0.05	cfs
10	Treamtent Flow Rate = 1.5 X Q	Q=	0.08	cfs

- 1) Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of flow-thru BMPs. That is, if the flow-thru BMP is upstream of the project's retention and biofiltration BMPs then the flow-thru BMP shall be sized using an adjustment factor of 1.
- 2) Volume based (e.g., dry extended detention basin) flow-thru treatment control BMPs shall be sized to the volume in Line 4 and dlow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either
- 3) Propietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.

MODEL #	DIMENSIONS	WETLANDMEDIA SURFACE AREA (sq. ft.)	TREATMENT FLOW RATE (cfs)
MW5-L-4-8	4' x 8'	50	0.115

	DMA-7-C: Flow-thru Design Flows	Work	ksheet B.6-1	
1	DCV	DCV	555	cubic-feet
2	DCV Retained	DCV Retained	0.00	cubic-feet
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet
4	1.5 DCV requiring flow-thru (Line 1 - Line 2 - 0.67*Line 3)	DCV flow-thru	555	cubic-feet
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless
6	Design rainfall intensity	i=	0.2	in/hr
7	Area tributary to BMP(s)	A=	0.38	acres
8	Area-weighted runoff factor (estimate using Appendix B.2)	C=	0.71	unitless
9	Calculate Flow Rate = AF x (C x i x A)	Q=	0.05	cfs
10	Treamtent Flow Rate = 1.5 X Q	Q=	0.08	cfs

- 1) Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of flow-thru BMPs. That is, if the flow-thru BMP is upstream of the project's retention and biofiltration BMPs then the flow-thru BMP shall be sized using an adjustment factor of 1.
- 2) Volume based (e.g., dry extended detention basin) flow-thru treatment control BMPs shall be sized to the volume in Line 4 and dlow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either
- 3) Propietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.

MODEL#	DIMENSIONS	WETLANDMEDIA SURFACE AREA (sq. ft.)	TREATMENT FLOW RATE (cfs)
MWS-L-4-8	4' x 8'	50	0.115

	DMA-7-D: Flow-thru Design Flows	Work	ksheet B.6-1	
1	DCV	DCV	115	cubic-feet
2	DCV Retained	DCV Retained	0.00	cubic-feet
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet
4	1.5 DCV requiring flow-thru (Line 1 - Line 2 - 0.67*Line 3)	DCV flow-thru	115	cubic-feet
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless
6	Design rainfall intensity	i=	0.2	in/hr
7	Area tributary to BMP(s)	A=	0.07	acres
8	Area-weighted runoff factor (estimate using Appendix B.2)	C=	0.78	unitless
9	Calculate Flow Rate = AF x (C x i x A)	Q=	0.01	cfs
10	Treamtent Flow Rate = 1.5 X Q	Q=	0.02	cfs

- 1) Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of flow-thru BMPs. That is, if the flow-thru BMP is upstream of the project's retention and biofiltration BMPs then the flow-thru BMP shall be sized using an adjustment factor of 1.
- 2) Volume based (e.g., dry extended detention basin) flow-thru treatment control BMPs shall be sized to the volume in Line 4 and dlow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either
- 3) Propietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.

MODEL #	DIMENSIONS	WETLANDMEDIA SURFACE AREA (sq. ft.)	TREATMENT FLOW RATE (cfs)
MWS-L-4-4	4' x 4'	23	0.052

	DMA 8: Flow-thru Design Flows	Work	ksheet B.6-1	
1	DCV	DCV	1,906	cubic-feet
2	DCV Retained	DCV Retained	0.00	cubic-feet
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet
4	1.5 DCV requiring flow-thru (Line 1 - Line 2 - 0.67*Line 3)	DCV flow-thru	1,906	cubic-feet
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless
6	Design rainfall intensity	i=	0.2	in/hr
7	Area tributary to BMP(s)	A=	1.30	acres
8	Area-weighted runoff factor (estimate using Appendix B.2)	C=	0.72	unitless
9	Calculate Flow Rate = AF x (C x i x A)	Q=	0.19	cfs
10	Treamtent Flow Rate = 1.5 X Q	Q=	0.28	cfs

- Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of flow-thru BMPs. That is, if the flow-thru BMP is upstream of the project's retention and biofiltration BMPs then the flow-thru BMP shall be sized using an adjustment factor of 1.
- 2) Volume based (e.g., dry extended detention basin) flow-thru treatment control BMPs shall be sized to the volume in Line 4 and dlow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either
- 3) Propietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.

MODEL #	DIMENSIONS	WETLANDMEDIA SURFACE AREA (sq. ft.)	TREATMENT FLOW RATE (cfs)
MWS-L-8-12	8' x 12'	151	0.346

	DMA 9: Flow-thru Design Flows	Worksheet B.6-1		
1	DCV	DCV	2,496	cubic-feet
2	DCV Retained	DCV Retained	0.00	cubic-feet
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet
4	1.5 DCV requiring flow-thru (Line 1 - Line 2 - 0.67*Line 3)	DCV flow-thru	2,496	cubic-feet
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless
6	Design rainfall intensity	i=	0.2	in/hr
7	Area tributary to BMP(s)	A=	1.89	acres
8	using Appendix B.2)	C=	0.65	unitless
9	Calculate Flow Rate = AF x (C x i x A)	Q=	0.25	cfs
10	Treamtent Flow Rate = 1.5 X Q	Q=	0.37	cfs

- Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of flow-thru BMPs. That is, if the flow-thru BMP is upstream of the project's retention and biofiltration BMPs then the flow-thru BMP shall be sized using an adjustment factor of 1.
- 2) Volume based (e.g., dry extended detention basin) flow-thru treatment control BMPs shall be sized to the volume in Line 4 and dlow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either
- 3) Propietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.

MODEL #	DIMENSIONS	WETLANDMEDIA SURFACE AREA (sq. ft.)	TREATMENT FLOW RATE (cfs)
MWS-L-8-16	8' x 16'	201	0.462

	DMA 10: Flow-thru Design Flows	Worksheet B.6-1		
1	DCV	DCV	289	cubic-feet
2	DCV Retained	DCV Retained	0.00	cubic-feet
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet
4	1.5 DCV requiring flow-thru (Line 1 - Line 2 - 0.67*Line 3)	DCV flow-thru	289	cubic-feet
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless
6	Design rainfall intensity	i=	0.2	in/hr
7	Area tributary to BMP(s)	A=	0.18	acres
8	using Appendix B.2)	C=	0.78	unitless
9	Calculate Flow Rate = AF x (C x i x A)	Q=	0.03	cfs
10	Treamtent Flow Rate = 1.5 X Q	Q=	0.04	cfs

- Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of flow-thru BMPs. That is, if the flow-thru BMP is upstream of the project's retention and biofiltration BMPs then the flow-thru BMP shall be sized using an adjustment factor of 1.
- 2) Volume based (e.g., dry extended detention basin) flow-thru treatment control BMPs shall be sized to the volume in Line 4 and dlow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either
- 3) Propietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.

MODEL #	DIMENSIONS	WETLANDMEDIA SURFACE AREA (sq. ft.)	TREATMENT FLOW RATE (cfs)
MWS-L-4-4	4' x 4'	23	0.052

PALOMAR HEIGHTS PROPRIETARY BIOFILTRATION FLOW-THRU SIZING CALCULATION

	DMA 11: Flow-thru Design Flows	Worksheet B.6-1					
1	DCV	DCV	184	cubic-feet			
2	DCV Retained	DCV Retained	0.00	cubic-feet			
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet			
4	1.5 DCV requiring flow-thru (Line 1 -		184	cubic-feet			
	Line 2 - 0.67*Line 3)	DCV flow-thru	104	CUDIC-TCCT			
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless			
6	Design rainfall intensity	i=	0.2	in/hr			
7	Area tributary to BMP(s)	A=	0.12	acres			
8	using Appendix B.2)	C=	0.78	unitless			
9	Calculate Flow Rate = AF x (C x i x A)	Q=	0.02	cfs			
10	Treamtent Flow Rate = 1.5 X Q	Q=	0.03	cfs			

- Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of flow-thru BMPs. That is, if the flow-thru BMP is upstream of the project's retention and biofiltration BMPs then the flow-thru BMP shall be sized using an adjustment factor of 1.
- 2) Volume based (e.g., dry extended detention basin) flow-thru treatment control BMPs shall be sized to the volume in Line 4 and dlow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either
- 3) Propietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.

MODEL#	DIMENSIONS	WETLANDMEDIA SURFACE AREA (sq. ft.)	TREATMENT FLOW RATE (cfs)
MWS-L-4-4	4' x 4'	23	0.052

Automated Worksheet B.1: Calculation of Design Capture Volume (V2.0)

Catagony	-#	Description Automated v	voi ksi leet	B. I. Calcul	ation of Desig	Jii Capture	volume (v	2.0) vi	vii	viii	ix	x	Units
Category	1	Drainage Basin ID or Name	DMA 1	DMA 2	DMA 3	DMA 4	DMA 5	DMA 6	DMA-7-A	DMA-7-C	DMA 8	DMA 9	unitless
	2	85th Percentile 24-hr Storm Depth	0.58	0.58	0.58	0.58	0.58	0.58	0.58	0.58	0.58	0.58	inches
	3	Impervious Surfaces Not Directed to Dispersion Area (C=0.90)	35.066	34.476	21.886	14.183	46,285	56.660	29.886	50.185	64.542	7.940	sq-ft
Standard	4	Semi-Pervious Surfaces Not Serving as Dispersion Area (C=0.30)	33,000	34,470	21,000	14,103	40,203	30,000	27,000	30,103	04,342	7,740	sq-ft
Drainage	5	Engineered Pervious Surfaces Not Serving as Dispersion Area (C=0.30)											sq-ft
Basin Inputs	6	Natural Type A Soil Not Serving as Dispersion Area (C=0.10)											sq-ft
Basiii iiipats	7	Natural Type B Soil Not Serving as Dispersion Area (C=0.10)											sq-ft sq-ft
	8	Natural Type C Soil Not Serving as Dispersion Area (C=0.14) Natural Type C Soil Not Serving as Dispersion Area (C=0.23)	34,368	19.072	12.644	19.604	6.818	19.187	11.667	6,362	17,866		sq-ft
	9	Natural Type C Soil Not Serving as Dispersion Area (C=0.23) Natural Type D Soil Not Serving as Dispersion Area (C=0.30)	34,300	17,072	12,044	17,004	0,010	17,107	11,007	0,302	17,000		sq-ft
	10	Does Tributary Incorporate Dispersion, Tree Wells, and/or Rain Barrels?	No	No	No	No	No	No	No	No	No	No	yes/no
	11	Impervious Surfaces Directed to Dispersion Area per SD-B (Ci=0.90)	INU	INU	INU	INU	INU	INU	INU	INU	INU	INU	,
	12	Semi-Pervious Surfaces Serving as Dispersion Area per SD-B (Ci=0.90)											sq-ft sq-ft
	13	Engineered Pervious Surfaces Serving as Dispersion Area per SD-B (Ci=0.30) Engineered Pervious Surfaces Serving as Dispersion Area per SD-B (Ci=0.10)											
Dispersion	14	Natural Type A Soil Serving as Dispersion Area per SD-B (Ci=0.10)											sq-ft
Area, Tree Wel	15	Natural Type B Soil Serving as Dispersion Area per SD-B (Ci=0.10) Natural Type B Soil Serving as Dispersion Area per SD-B (Ci=0.14)											sq-ft sq-ft
& Rain Barrel													
Inputs	16 17	Natural Type C Soil Serving as Dispersion Area per SD-B (Ci=0.23) Natural Type D Soil Serving as Dispersion Area per SD-B (Ci=0.30)											sq-ft
(Optional)		<i>3</i> 1											sq-ft
	18 19	Number of Tree Wells Proposed per SD-A Average Mature Tree Canopy Diameter											#
	20	Number of Rain Barrels Proposed per SD-E											1t
	20	Average Rain Barrel Size											gal gal
	22	Average Rain Barrer Size Total Tributary Area	69,433	53,548	34,530	33,787	53,103	75,847	41,553	56,547	82,408	7,940	gai sq-ft
Initial Runoff		Initial Runoff Factor for Standard Drainage Areas	09,433	0.66	0.65	0.51	0.81	0.73	0.71	0.82	02,400	0.90	unitless
Factor	23	Initial Runoff Factor for Dispersed & Dispersion Areas	0.00	0.00	0.00	0.00	0.00	0.73	0.71	0.82	0.75	0.90	unitless
Calculation	25	Initial Runoff Factor Tol Dispersed & Dispersion Areas	0.57	0.66	0.65	0.00	0.00	0.00	0.00	0.82	0.00	0.90	unitless
Calculation	26	Initial Design Capture Volume	1.913	1,708	1.085	833	2.079	2.676	1.426	2,241	2,987	345	cubic-feet
	27	Total Impervious Area Dispersed to Pervious Surface	0	0	0	0	0	0	0	0	0	0	sq-ft
	28	Total Pervious Dispersion Area	0	0	0	0	0	0	0	0	0	0	sq-ft
Dispersion	29	Ratio of Dispersed Impervious Area to Pervious Dispersion Area	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	ratio
Area	30	Adjustment Factor for Dispersed & Dispersion Areas	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	ratio
Adjustments	31	Runoff Factor After Dispersion Techniques	0.57	0.66	0.65	0.51	0.81	0.73	0.71	0.82	0.75	0.90	unitless
	32	Design Capture Volume After Dispersion Techniques	1,913	1.708	1.085	833	2,079	2.676	1,426	2,241	2,987	345	cubic-feet
Tree & Barrel	33	Total Tree Well Volume Reduction	0	0	0	0	0	0	0	0	2,987	345	cubic-reet
Adjustments	34	Total Tree Well Volume Reduction	0	0	0	0	0	0	0	0	0	0	cubic-reet cubic-feet
Aujustificitis	35	Final Adjusted Runoff Factor	0.57	0.66	0.65	0.51	0.81	0.73	0.71	0.82	0.75	0.90	unitless
	36	Final Adjusted Runon Factor	39.577	35.342	22,445	17.231	43.013	55,368	29.503	46.369	61,806	7,146	sq-ft
Results	36	Initial Design Capture Volume Retained by Site Design Elements	39,577	35,342	0	0	43,013	55,368	29,503	46,369	0 1,806	7,146	sq-rt cubic-feet
	38	Final Design Capture Volume Retained by Site Design Elements Final Design Capture Volume Tributary to BMP	1.913	1,708	1.085	833	2.079	2,676	1.426	2.241	2,987	345	cubic-reet cubic-feet
No Warning M		ů i	1,913	1,708	1,085	033	2,019	2,070	1,420	2,241	2,987	345	cubic-feet
INO WAITIING IV	essage	<u>a</u>											

Automated Worksheet B.2: Retention Requirements (V2.0)

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Category	#	Description	_	ii	iii	iv	V	vi	vii	viii	ix	X	Units
	1	Drainage Basin ID or Name	DMA 1	DMA 2	DMA 3	DMA 4	DMA 5	DMA 6	DMA-7-A	DMA-7-C	DMA 8	DMA 9	unitless
	2	85th Percentile Rainfall Depth	0.58	0.58	0.58	0.58	0.58	0.58	0.58	0.58	0.58	0.58	inches
	3	Predominant NRCS Soil Type Within BMP Location	С	С	С	С	С	С	С	С	С	С	unitless
Basic Analysis	4	Is proposed BMP location Restricted or Unrestricted for Infiltration Activities?	Restricted	Restricted	Restricted	Restricted	Restricted	Restricted	Restricted	Restricted	Restricted	Restricted	unitless
	5	Nature of Restriction	n/a	unitless									
	6	Do Minimum Retention Requirements Apply to this Project?	Yes	yes/no									
	7	Are Habitable Structures Greater than 9 Stories Proposed?	No	yes/no									
Advanced	8	Has Geotechnical Engineer Performed an Infiltration Analysis?	No	yes/no									
Analysis	9	Design Infiltration Rate Recommended by Geotechnical Engineer											in/hr
	10	Design Infiltration Rate Used To Determine Retention Requirements	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	in/hr
Result	11	Percent of Average Annual Runoff that Must be Retained within DMA	4.5%	4.5%	4.5%	4.5%	4.5%	4.5%	4.5%	4.5%	4.5%	4.5%	percentage
Result	12	Fraction of DCV Requiring Retention	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	ratio
	13	Required Retention Volume	38	34	22	17	42	54	29	45	60	7	cubic-feet
No Warning Me	essages						_					•	

No Warning Messages

Automated Worksheet B.1: Calculation of Design Capture Volume (V2.0)

Category	#	Description	i	ii	iii	iv	Units
	1	Drainage Basin ID or Name	DMA-7-B	DMA 10	DMA 11	DMA 12	unitless
	2	85th Percentile 24-hr Storm Depth	0.58	0.58	0.58	0.58	inches
	3	Impervious Surfaces Not Directed to Dispersion Area (C=0.90)	6,193	7,940	5,054	35,444	sq-ft
Standard	4	Semi-Pervious Surfaces Not Serving as Dispersion Area (C=0.30)	· · · · · · · · · · · · · · · · · · ·				sq-ft
Drainage Basin	5	Engineered Pervious Surfaces Not Serving as Dispersion Area (C=0.10)	1,500			9,270	sq-ft
Inputs	6	Natural Type A Soil Not Serving as Dispersion Area (C=0.10)	· · · · · · · · · · · · · · · · · · ·				sq-ft
	7	Natural Type B Soil Not Serving as Dispersion Area (C=0.14)					sq-ft
	8	Natural Type C Soil Not Serving as Dispersion Area (C=0.23)			17,866		sq-ft
	9	Natural Type D Soil Not Serving as Dispersion Area (C=0.30)					sq-ft
	10	Does Tributary Incorporate Dispersion, Tree Wells, and/or Rain Barrels?	Yes	No	No	Yes	yes/no
	11	Impervious Surfaces Directed to Dispersion Area per SD-B (Ci=0.90)					sq-ft
	12	Semi-Pervious Surfaces Serving as Dispersion Area per SD-B (Ci=0.30)					sq-ft
	13	Engineered Pervious Surfaces Serving as Dispersion Area per SD-B (Ci=0.10)					sq-ft
Dispersion	14	Natural Type A Soil Serving as Dispersion Area per SD-B (Ci=0.10)					sq-ft
Area, Tree Well	15	Natural Type B Soil Serving as Dispersion Area per SD-B (Ci=0.14)					sq-ft
& Rain Barrel Inputs	16	Natural Type C Soil Serving as Dispersion Area per SD-B (Ci=0.23)					sq-ft
(Optional)	17	Natural Type D Soil Serving as Dispersion Area per SD-B (Ci=0.30)					sq-ft
(Optional)	18	Number of Tree Wells Proposed per SD-A	7			16	#
	19	Average Mature Tree Canopy Diameter	10			15	ft
	20	Number of Rain Barrels Proposed per SD-E					#
	21	Average Rain Barrel Size					gal
	22	Total Tributary Area	7,693	7,940	22,920	44,714	sq-ft
Initial Runoff	23	Initial Runoff Factor for Standard Drainage Areas	0.74	0.57	0.38	0.73	unitless
Factor	24	Initial Runoff Factor for Dispersed & Dispersion Areas	0.00	0.00	0.00	0.00	unitless
Calculation	25	Initial Weighted Runoff Factor	0.74	0.57	0.38	0.73	unitless
	26	Initial Design Capture Volume	275	219	421	1,578	cubic-feet
	27	Total Impervious Area Dispersed to Pervious Surface	0	0	0	0	sq-ft
Dispersion	28	Total Pervious Dispersion Area	0	0	0	0	sq-ft
Dispersion Area	29	Ratio of Dispersed Impervious Area to Pervious Dispersion Area	n/a	n/a	n/a	n/a	ratio
Adjustments	30	Adjustment Factor for Dispersed & Dispersion Areas	1.00	1.00	1.00	1.00	ratio
. Joj diotrinorito	31	Runoff Factor After Dispersion Techniques	0.74	0.57	0.38	0.73	unitless
	32	Design Capture Volume After Dispersion Techniques	275	219	421	1,578	cubic-feet
Tree & Barrel	33	Total Tree Well Volume Reduction	280	0	0	1,600	cubic-feet
Adjustments	34	Total Rain Barrel Volume Reduction	0	0	0	0	cubic-feet
	35	Final Adjusted Runoff Factor	0.00	0.57	0.38	0.00	unitless
Results	36	Final Effective Tributary Area	0	4,526	8,710	0	sq-ft
- Results	37	Initial Design Capture Volume Retained by Site Design Elements	280	0	0	1,600	cubic-feet
	38	Final Design Capture Volume Tributary to BMP	0	219	421	0	cubic-feet
No Warning Me	ssages						

No Warning Messages

Automated Worksheet B.2: Retention Requirements (V2.0)

Category	#	Description	i	ii	iii	iv	Units
	1	Drainage Basin ID or Name	DMA-7-B	DMA 11	DMA 11	DMA 12	unitless
	2	85th Percentile Rainfall Depth	0.58	0.58	0.58	0.58	inches
	3	Predominant NRCS Soil Type Within BMP Location	С	С	С	С	unitless
Basic Analysis	4	Is proposed BMP location Restricted or Unrestricted for Infiltration Activities?	Restricted	Restricted	Restricted	Restricted	unitless
	5	Nature of Restriction	n/a	n/a	n/a	n/a	unitless
	6	Do Minimum Retention Requirements Apply to this Project?	Yes	Yes	Yes	Yes	yes/no
	7	Are Habitable Structures Greater than 9 Stories Proposed?	No	No	No	No	yes/no
Advanced	8	Has Geotechnical Engineer Performed an Infiltration Analysis?	No	No	No	No	yes/no
Analysis	9	Design Infiltration Rate Recommended by Geotechnical Engineer					in/hr
	10	Design Infiltration Rate Used To Determine Retention Requirements	0.000	0.000	0.000	0.000	in/hr
Result	11	Percent of Average Annual Runoff that Must be Retained within DMA	4.5%	4.5%	4.5%	4.5%	percentage
Result	12	Fraction of DCV Requiring Retention	0.02	0.02	0.02	0.02	ratio
	13	Required Retention Volume	0	4	8	0	cubic-feet

No Warning Messages

Automated Worksheet B.1: Calculation of Design Capture Volume (V2.0)

Drainage Basin ID or Name 85th Percentile 24-hr Storm Depth Impervious Surfaces Not Directed to Dispersion Area (C=0.90) Semi-Pervious Surfaces Not Serving as Dispersion Area (C=0.30) Engineered Pervious Surfaces Not Serving as Dispersion Area (C=0.10) Natural Type A Soil Not Serving as Dispersion Area (C=0.10) Natural Type B Soil Not Serving as Dispersion Area (C=0.14) Natural Type C Soil Not Serving as Dispersion Area (C=0.23)	DMA-7-D 0.58 3,163	unitless inches sq-ft sq-ft
Impervious Surfaces Not Directed to Dispersion Area (C=0.90) Semi-Pervious Surfaces Not Serving as Dispersion Area (C=0.30) Engineered Pervious Surfaces Not Serving as Dispersion Area (C=0.10) Natural Type A Soil Not Serving as Dispersion Area (C=0.10) Natural Type B Soil Not Serving as Dispersion Area (C=0.14) Natural Type C Soil Not Serving as Dispersion Area (C=0.23)		sq-ft sq-ft
Semi-Pervious Surfaces Not Serving as Dispersion Area (C=0.30) Engineered Pervious Surfaces Not Serving as Dispersion Area (C=0.10) Natural Type A Soil Not Serving as Dispersion Area (C=0.10) Natural Type B Soil Not Serving as Dispersion Area (C=0.14) Natural Type C Soil Not Serving as Dispersion Area (C=0.23)	3,163	sq-ft
Engineered Pervious Surfaces Not Serving as Dispersion Area (C=0.10) Natural Type A Soil Not Serving as Dispersion Area (C=0.10) Natural Type B Soil Not Serving as Dispersion Area (C=0.14) Natural Type C Soil Not Serving as Dispersion Area (C=0.23)		
Natural Type A Soil <u>Not Serving as Dispersion Area</u> (C=0.10) Natural Type B Soil <u>Not Serving as Dispersion Area</u> (C=0.14) Natural Type C Soil <u>Not Serving as Dispersion Area</u> (C=0.23)		
Natural Type B Soil <u>Not Serving as Dispersion Area</u> (C=0.14) Natural Type C Soil <u>Not Serving as Dispersion Area</u> (C=0.23)		sq-ft
Natural Type C Soil Not Serving as Dispersion Area (C=0.23)		sq-ft
		sq-ft
		sq-ft
Natural Type D Soil Not Serving as Dispersion Area (C=0.30)		sq-ft
Does Tributary Incorporate Dispersion, Tree Wells, and/or Rain Barrels?	No	yes/no
Impervious Surfaces Directed to Dispersion Area per SD-B (Ci=0.90)		sq-ft
Semi-Pervious Surfaces Serving as Dispersion Area per SD-B (Ci=0.30)		sq-ft
Engineered Pervious Surfaces Serving as Dispersion Area per SD-B (Ci=0.10)		sq-ft
Natural Type A Soil Serving as Dispersion Area per SD-B (Ci=0.10)		sq-ft
Natural Type B Soil Serving as Dispersion Area per SD-B (Ci=0.14)		sq-ft
Natural Type C Soil Serving as Dispersion Area per SD-B (Ci=0.23)		sq-ft
Natural Type D Soil Serving as Dispersion Area per SD-B (Ci=0.30)		sq-ft
Number of Tree Wells Proposed per SD-A		#
Average Mature Tree Canopy Diameter		ft
Number of Rain Barrels Proposed per SD-E		#
Average Rain Barrel Size		gal
Total Tributary Area	3,163	sq-ft
Initial Runoff Factor for Standard Drainage Areas	0.90	unitless
Initial Runoff Factor for Dispersed & Dispersion Areas	0.00	unitless
Initial Weighted Runoff Factor	0.90	unitless
Initial Design Capture Volume	138	cubic-fee
Total Impervious Area Dispersed to Pervious Surface	0	sq-ft
Total Pervious Dispersion Area	0	sq-ft
Ratio of Dispersed Impervious Area to Pervious Dispersion Area	n/a	ratio
Adjustment Factor for Dispersed & Dispersion Areas	1.00	ratio
Runoff Factor After Dispersion Techniques	0.90	unitless
Design Capture Volume After Dispersion Techniques	138	cubic-fee
Total Tree Well Volume Reduction		cubic-fee
	0	cubic-fee
		unitless
•		sq-ft
		cubic-fee
		cubic-fee
_	Total Tree Well Volume Reduction Total Rain Barrel Volume Reduction Final Adjusted Runoff Factor Final Effective Tributary Area Initial Design Capture Volume Retained by Site Design Elements Final Design Capture Volume Tributary to BMP	Total Rain Barrel Volume Reduction 0 Final Adjusted Runoff Factor 0.90 Final Effective Tributary Area 2,847 Initial Design Capture Volume Retained by Site Design Elements 0

Automated Worksheet B.2: Retention Requirements (V2.0)

Category	#	Description	i	Units
	1	Drainage Basin ID or Name	DMA-7-D	unitless
	2	85th Percentile Rainfall Depth	0.58	inches
	3	Predominant NRCS Soil Type Within BMP Location	С	unitless
Basic Analysis	4	Is proposed BMP location Restricted or Unrestricted for Infiltration Activities?	Restricted	unitless
	5	Nature of Restriction	n/a	unitless
	6	Do Minimum Retention Requirements Apply to this Project?	Yes	yes/no
	7	Are Habitable Structures Greater than 9 Stories Proposed?	No	yes/no
Advanced	8	Has Geotechnical Engineer Performed an Infiltration Analysis?	No	yes/no
Analysis	9	Design Infiltration Rate Recommended by Geotechnical Engineer		in/hr
	10	Design Infiltration Rate Used To Determine Retention Requirements	0.000	in/hr
Result	11	Percent of Average Annual Runoff that Must be Retained within DMA	4.5%	percentage
Result	12	Fraction of DCV Requiring Retention	0.02	ratio
	13	Required Retention Volume	3	cubic-feet
No Warning Me	ssage	<u>S</u>		

Automated Worksheet B.2: Retention Requirements (V2.0)

Drainage Basin ID or Name 85th Percentile Rainfall Depth Predominant NRCS Soil Type Within BMP Location Is proposed BMP location Restricted or Unrestricted for Infiltration Activities?	С	DMA 2 0.58 C	DMA 3 0.58	DMA 4 0.58	DMA 5 0.58	DMA 6	DMA-7-A	DMA-7-C	DMA 8	DMA 9	unitless
Predominant NRCS Soil Type Within BMP Location	С	C	0.58	0.58	0.58	0.50					
<u>.</u>		C			0.50	0.58	0.58	0.58	0.58	0.58	inches
Is proposed BMP location Restricted or Unrestricted for Infiltration Activities?	Б	C	С	С	С	С	С	С	С	С	unitless
	Restricted	Restricted	Restricted	Restricted	Restricted	Restricted	Restricted	Restricted	Restricted	Restricted	unitless
Nature of Restriction	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	n/a	unitless
Do Minimum Retention Requirements Apply to this Project?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	yes/no
Are Habitable Structures Greater than 9 Stories Proposed?	No	No	No	No	No	No	No	No	No	No	yes/no
Has Geotechnical Engineer Performed an Infiltration Analysis?	No	No	No	No	No	No	No	No	No	No	yes/no
Design Infiltration Rate Recommended by Geotechnical Engineer											in/hr
Design Infiltration Rate Used To Determine Retention Requirements	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	in/hr
Percent of Average Annual Runoff that Must be Retained within DMA	4.5%	4.5%	4.5%	4.5%	4.5%	4.5%	4.5%	4.5%	4.5%	4.5%	percentage
Fraction of DCV Requiring Retention	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	ratio
Required Retention Volume	38	34	22	17	12	54	20	45	40	7	cubic-feet
	Has Geotechnical Engineer Performed an Infiltration Analysis? Design Infiltration Rate Recommended by Geotechnical Engineer Design Infiltration Rate Used To Determine Retention Requirements Percent of Average Annual Runoff that Must be Retained within DMA Fraction of DCV Requiring Retention	Has Geotechnical Engineer Performed an Infiltration Analysis? Design Infiltration Rate Recommended by Geotechnical Engineer Design Infiltration Rate Used To Determine Retention Requirements 0.000 Percent of Average Annual Runoff that Must be Retained within DMA 4.5% Fraction of DCV Requiring Retention 0.02	Has Geotechnical Engineer Performed an Infiltration Analysis? No No Design Infiltration Rate Recommended by Geotechnical Engineer Design Infiltration Rate Used To Determine Retention Requirements 0.000 0.000 Percent of Average Annual Runoff that Must be Retained within DMA 4.5% 4.5% Fraction of DCV Requiring Retention 0.02 0.02	Has Geotechnical Engineer Performed an Infiltration Analysis? No No No No Design Infiltration Rate Recommended by Geotechnical Engineer Design Infiltration Rate Used To Determine Retention Requirements 0.000 0.000 0.000 Percent of Average Annual Runoff that Must be Retained within DMA 4.5% 4.5% 4.5% Fraction of DCV Requiring Retention 0.02 0.02 0.02	Has Geotechnical Engineer Performed an Infiltration Analysis? No No No No No No No No No No No No No N	Has Geotechnical Engineer Performed an Infiltration Analysis? No No No No No No No No No No No No No N	Has Geotechnical Engineer Performed an Infiltration Analysis? No No No No No No No No No No No No No N	Has Geotechnical Engineer Performed an Infiltration Analysis? No No No No No No No No No No No No No N	Has Geotechnical Engineer Performed an Infiltration Analysis? No No No No No No No No No No No No No N	Has Geotechnical Engineer Performed an Infiltration Analysis? No No No No No No No No No No No No No N	Has Geotechnical Engineer Performed an Infiltration Analysis? No No No No No No No No No No No No No

Automated Worksheet B.2: Retention Requirements (V2.0)

Category	#	Description	i	ii	iii	iν	Units
	1	Drainage Basin ID or Name	DMA-7-B	DMA 11	DMA 11	DMA 12	unitless
	2	85th Percentile Rainfall Depth	0.58	0.58	0.58	0.58	inches
	3	Predominant NRCS Soil Type Within BMP Location	С	С	С	С	unitless
Basic Analysis	4	Is proposed BMP location Restricted or Unrestricted for Infiltration Activities?	Restricted	Restricted	Restricted	Restricted	unitless
	5	Nature of Restriction	n/a	n/a	n/a	n/a	unitless
	6	Do Minimum Retention Requirements Apply to this Project?	Yes	Yes	Yes	Yes	yes/no
	7	Are Habitable Structures Greater than 9 Stories Proposed?	No	No	No	No	yes/no
Advanced	8	Has Geotechnical Engineer Performed an Infiltration Analysis?	No	No	No	No	yes/no
Analysis	9	Design Infiltration Rate Recommended by Geotechnical Engineer					in/hr
	10	Design Infiltration Rate Used To Determine Retention Requirements	0.000	0.000	0.000	0.000	in/hr
Result	11	Percent of Average Annual Runoff that Must be Retained within DMA	4.5%	4.5%	4.5%	4.5%	percentage
Result	12	Fraction of DCV Requiring Retention	0.02	0.02	0.02	0.02	ratio
	13	Required Retention Volume	0	4	8	0	cubic-feet
No Warning Me	ssage	8		•			•

Automated Worksheet B.2: Retention Requirements (V2.0)

Category	#	Description		Units
	1	Drainage Basin ID or Name	DMA-7-D	unitless
	2	85th Percentile Rainfall Depth	0.58	inches
	3	Predominant NRCS Soil Type Within BMP Location	С	unitless
Basic Analysis	4	Is proposed BMP location Restricted or Unrestricted for Infiltration Activities?	Restricted	unitless
	5	Nature of Restriction	n/a	unitless
	6	Do Minimum Retention Requirements Apply to this Project?	Yes	yes/no
	7	Are Habitable Structures Greater than 9 Stories Proposed?	No	yes/no
Advanced	8	Has Geotechnical Engineer Performed an Infiltration Analysis?	No	yes/no
Analysis	9	Design Infiltration Rate Recommended by Geotechnical Engineer		in/hr
	10	Design Infiltration Rate Used To Determine Retention Requirements	0.000	in/hr
Daniela	11	Percent of Average Annual Runoff that Must be Retained within DMA	4.5%	percenta
Result	12	Fraction of DCV Requiring Retention	0.02	ratio
	13	Required Retention Volume	3	cubic-fee

Total Required Retention Volume for DMA's 1-11 = **363 CUFT**

Modular Wetland Volume Calculation for MWS-L-4-17-V: (4' * 9.83' * 3.33' = 131.06 CUFT)

Void Space for Storage for MWS-L-4-17-V: (131.06 CUFT) * (0.45) = 58.97 CUFT

Total Number of MWS units with volume greater than or equal to 131.06 CUFT: 7 Units

Storage provided by the 7 units: (7 units) * (58.97 CUFT/units) = 412.8 CUFT

412.8 CUFT > 360 CUFT

Therefore, retention requirements are met.

Harvest and Use Feasibility Checklist

Worksheet B.3-1

1. Is there a demand for harvested water	(check all that apply) a	at the project site that is	reliably present during
the wet season?			

- □ Toilet and urinal flushing
- □ Landscape irrigation
- □ Other

2. If there is a demand; estimate the anticipated average wet season demand over a period of 36 hours. Guidance for planning level demand calculations for toilet/urinal flushing and landscape irrigation is provided in Section B.3.2.

[Provide a summary of calculations here]

Mod. ETWU =
$$2.8 \left[\frac{(.70)(97,228)}{.90} \right] (0.015) = 3176$$
 Cubic Feet

3. Calculate the DCV using worksheet B-2.1.

$$DCV = 12,753$$
 Cubic Feet

$$.25 (12,753) = 3,188$$
 Cubic Feet

3a. Is the 36 hour demand greater than or equal to the DCV?

/ ⊠ No **□**



3b. Is the 36 hour demand greater than 0.25DCV but less than the full DCV?

/ ⊠ No 🖈



3c. Is the 36 hour demand less than 0.25DCV?



Harvest and use appears to be feasible. Conduct more detailed evaluation and sizing calculations to confirm that DCV can be used at an adequate rate to meet drawdown criteria.

Harvest and use may be feasible. Conduct more detailed evaluation and sizing calculations to determine feasibility. Harvest and use may only be able to be used for a portion of the site, or (optionally) the storage may need to be upsized to meet long term capture targets while draining in longer than 36 hours.

Harvest and use İS considered to be infeasible.

The Harvest and Use calculations were performed for the residential DMA's only.

Categorization of Infiltration Feasibility Form I-5 Condition Part 1 - Full Infiltration Feasibility Screening Criteria Would infiltration of the full design volume be feasible from a physical perspective without any undesirable consequences that cannot be reasonably mitigated? Criteria **Screening Question** Yes No Is the estimated reliable infiltration rate below proposed facility locations greater than 0.5 inches per hour? The 1 response to this Screening Question shall be based on a Χ comprehensive evaluation of the factors presented in Appendix C.2 and Appendix D. Provide basis: Based on the NRCS Web Soil Survey, The project is located in predominately hydrologic type C soils with a corresponding infiltration rate of 0.10 in/hr Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability. Can infiltration greater than 0.5 inches per hour be allowed without increasing risk of geotechnical hazards (slope stability, groundwater mounding, utilities, or other factors) 2 Χ that cannot be mitigated to an acceptable level? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.2. Provide basis:

Based on the preliminary Geotechnical Study performed by Geocon Inc., infiltration of storm water may have adverse impacts to downstream properties such as slope instability, raised groundwater levels, and movement of foundations and slabs.

Infiltration is therefore not advised. Refer to the Geotechnical study for more info Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability.

Template Date: October 2016 Preparation Date: 6/18/2019 PDP SWQMP - Attachments

	Form I-5		
Criteria	Screening Question	Yes	No
3	Can infiltration greater than 0.5 inches per hour be allowed without increasing risk of groundwater contamination (shallow water table, storm water pollutants or other factors) that cannot be mitigated to an acceptable level? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.		х
tion of slope slabs. Infiltra _{ummariz}	on the preliminary Geotechnical Study performed by storm water may have adverse impacts to downstre instability, raised groundwater levels, and movement tion is therefore not advised. Refer to the Geotechnic te findings of studies; provide reference to studies, calculations, maps, day of study/data source applicability.	am properti t of foundati cal study for	es such as ons and more info
4	Can infiltration greater than 0.5 inches per hour be allowed without causing potential water balance issues such as change of seasonality of ephemeral streams or increased discharge of contaminated groundwater to surface waters? The response to		x
Provide b	this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3. asis:		
Infiltra	tion is infeasbile		
	re findings of studies; provide reference to studies, calculations, maps, dans of study/data source applicability.	ata sources, etc.	Provide narrative
Part 1	If all answers to rows 1 - 4 are "Yes" a full infiltration design is potent. The feasibility screening category is Full Infiltration	tially feasible.	
Result*	If any answer from row 1-4 is "No", infiltration may be possible to so would not generally be feasible or desirable to achieve a "full infiltration		

Template Date: October 2016 Preparation Date: 6/18/2019 PDP SWQMP - Attachments

Form I-5

Part 2 – Partial Infiltration vs. No Infiltration Feasibility Screening Criteria

Would infiltration of water in any appreciable amount be physically feasible without any negative consequences that cannot be reasonably mitigated?

Criteria	Screening Question	Yes	No
5	Do soil and geologic conditions allow for infiltration in any appreciable rate or volume? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.2 and Appendix D.		х

Provide basis:

Based on the NRCS Web Soil Survey, The project is located in predominately hydrologic type C soils with a corresponding infiltration rate of 0.10 in/hr

Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.

6	Can Infiltration in any appreciable quantity be allowed without increasing risk of geotechnical hazards (slope stability, groundwater mounding, utilities, or other factors) that cannot be mitigated to an acceptable level? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.2.		х
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Provide basis:

Based on the preliminary Geotechnical Study performed by Geocon Inc., infiltration of storm water may have adverse impacts to downstream properties such as slope instability, raised groundwater levels, and movement of foundations and slabs.

Infiltration is therefore not advised. Refer to the Geotechnical study for more info

Summarize findings of studies; provide reference to studies, calculations, maps, data sources, etc. Provide narrative discussion of study/data source applicability and why it was not feasible to mitigate low infiltration rates.

Template Date: October 2016 Preparation Date: 6/18/2019

PDP SWQMP - Attachments

	Form I-5		
Criteria	Screening Question	Yes	No
7	Can Infiltration in any appreciable quantity be allowed without posing significant risk for groundwater related concerns (shallow water table, storm water pollutants or other factors)? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.		х
Provide b	asis:		
slabs. Infiltrat	nstability, raised groundwater levels, and movement ion is therefore not advised. Refer to the Geotechnic e findings of studies; provide reference to studies, calculations, maps, day of study/data source applicability and why it was not feasible to mitigate	al study for m	ore info
8	Can infiltration be allowed without violating downstream water rights? The response to this Screening Question shall be based on a comprehensive evaluation of the factors presented in Appendix C.3.		х
Provide b			
per the	y on water rights was not performed. However, infiltrated Geotechnical Engineer recomendation.		
	e findings of studies; provide reference to studies, calculations, maps, day of study/data source applicability and why it was not feasible to mitigate	e low infiltration ra	
Part 2	If all answers from row 5-8 are yes then partial infiltration design is por The feasibility screening category is Partial Infiltration.		no infiltra- tion
Result*	If any answer from row 5-8 is no, then infiltration of any volume is cor infeasible within the drainage area. The feasibility screening category is Infiltration.		

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Use this checklist to ensure the required information has been included on the DMA Fxhibit.

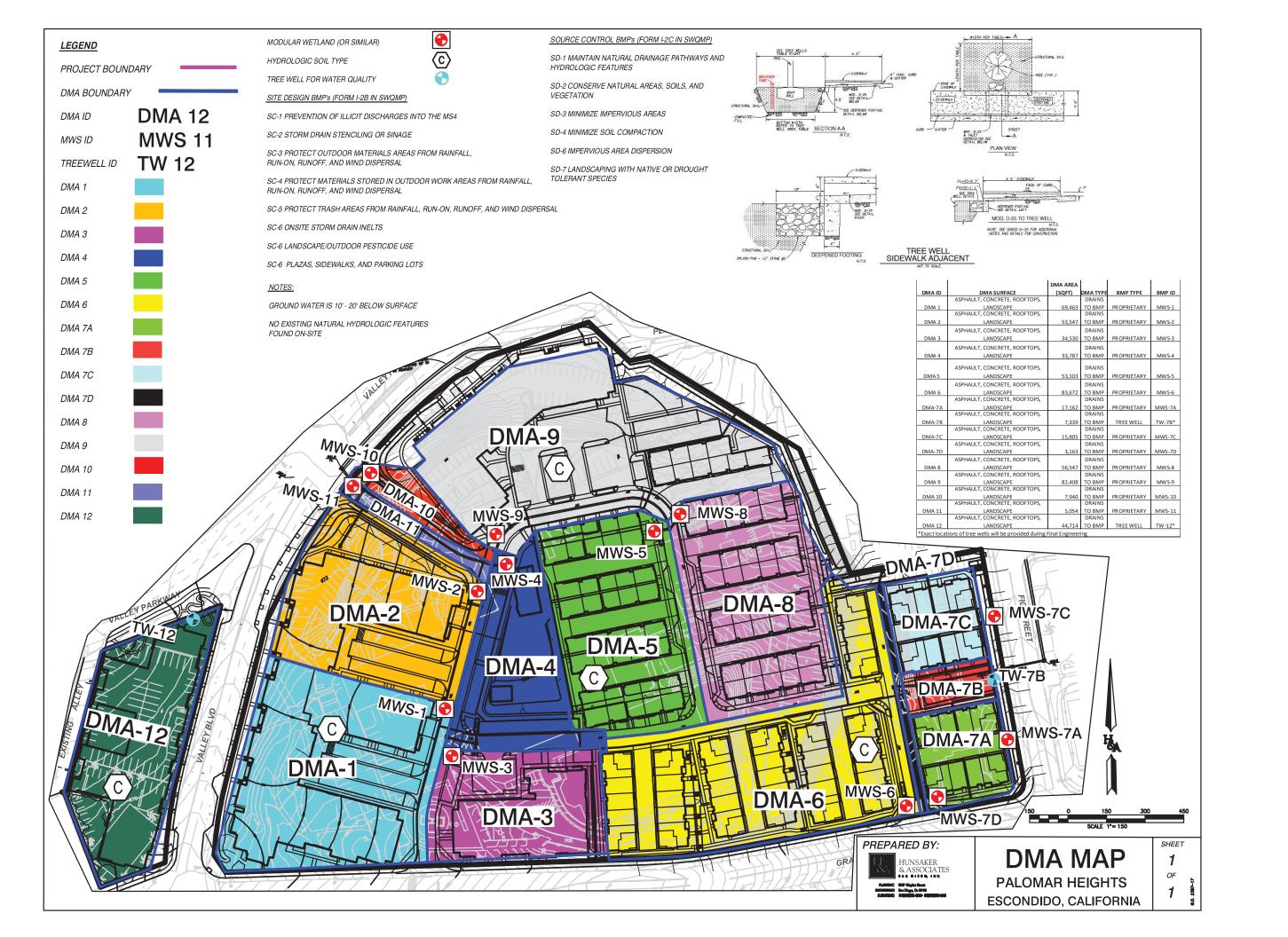
The DMA Exhibit must identify:

- ☑Approximate depth to groundwater
- ⊠Existing natural hydrologic features (watercourses, seeps, springs, wetlands)
- ☐ Critical coarse sediment yield areas to be protected
- ⊠Existing topography and impervious areas
- ⊠Existing and proposed site drainage network and connections to drainage offsite

- ⊠Proposed design features and surface treatments used to minimize imperviousness
- ☑Drainage management area (DMA) boundaries, DMA ID numbers, and DMA areas (square footage or acreage), and DMA type (i.e., drains to BMP, self-retaining, or self-mitigating)
- ⊠Structural BMPs (identify location, structural BMP ID#, type of BMP, and size/detail)

Template Date: October 2016 Preparation Date: 6/18/2019

PDP SWQMP - Attachments

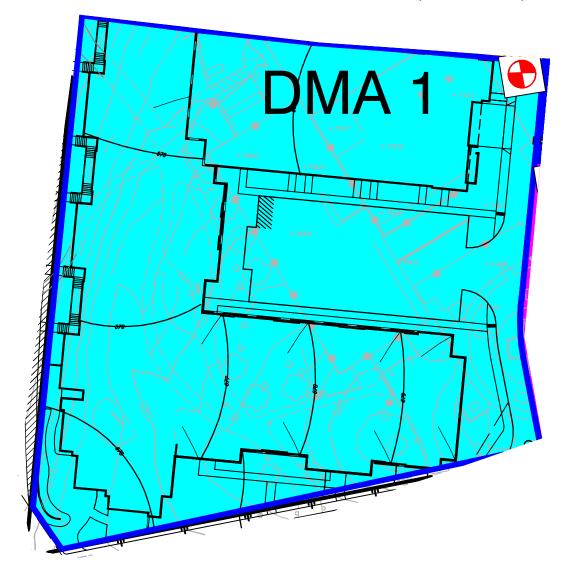


MODULAR WETLAND (OR SIMILAR)



LEGEND: SEE DMA MAP SCALE: 1" = 70'





PREPARED BY:



4G 9707 Weples Street 4G San Diego, Co 92121 4G PHOSSASSO 4300 - P.O

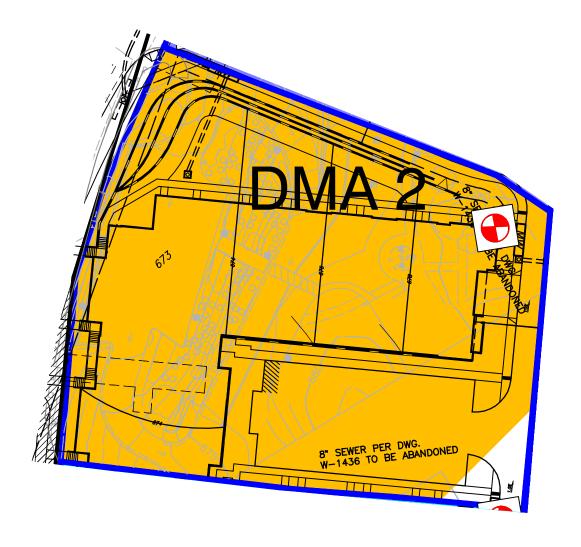
DMA 1 MAPBOOK FOR PALOMAR HEIGHTS

MODULAR WETLAND (OR SIMILAR)



LEGEND: SEE DMA MAP SCALE: 1" = 70'

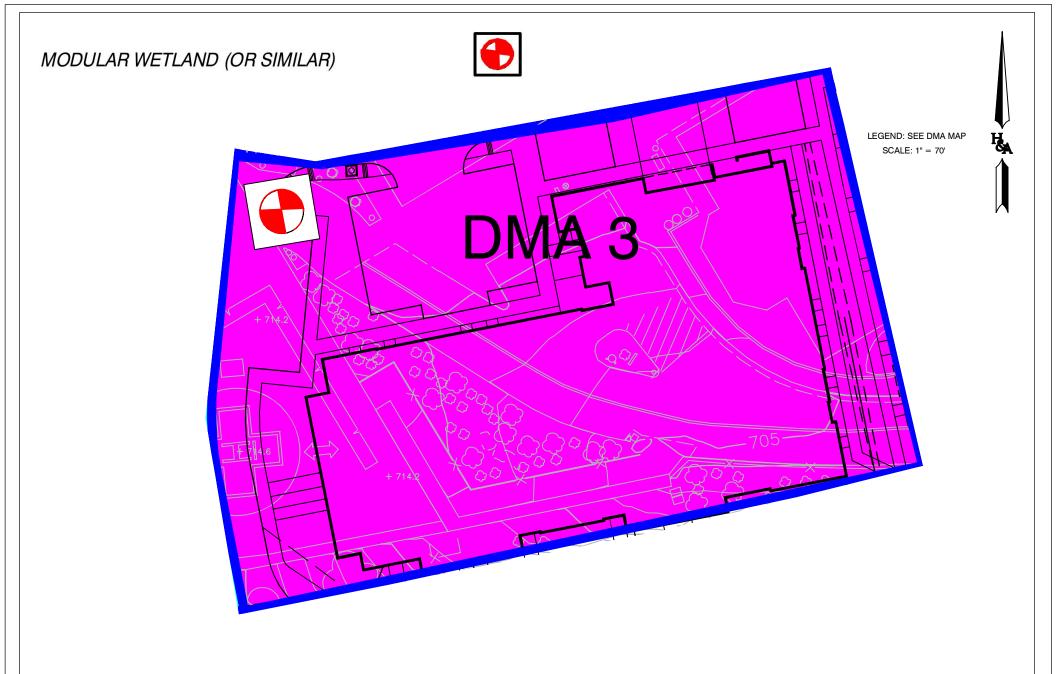




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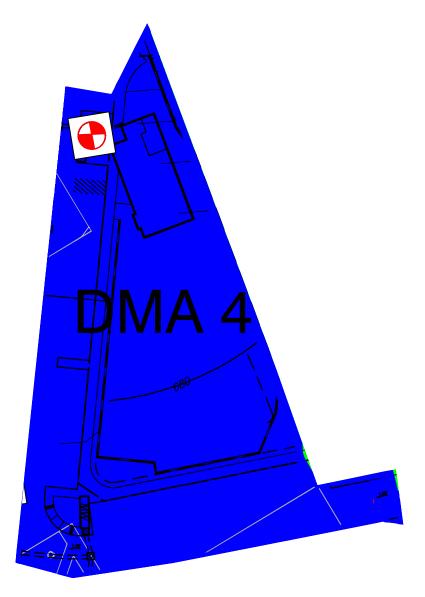
DMA 2 MAPBOOK FOR PALOMAR HEIGHTS



PREPARED BY:



DMA 3 MAPBOOK FOR PALOMAR HEIGHTS



LEGEND: SEE DMA MAP SCALE: 1" = 70'



MODULAR WETLAND (OR SIMILAR)

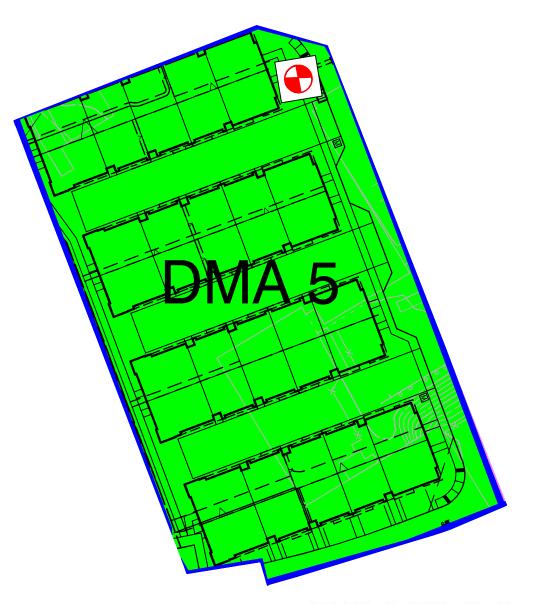


PREPARED BY:



NG 9707 Whyles Street NG San Diago, Co 92121

DMA 4 MAPBOOK FOR PALOMAR HEIGHTS



LEGEND: SEE DMA MAP SCALE: 1" = 70'



MODULAR WETLAND (OR SIMILAR)



PREPARED BY:

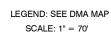


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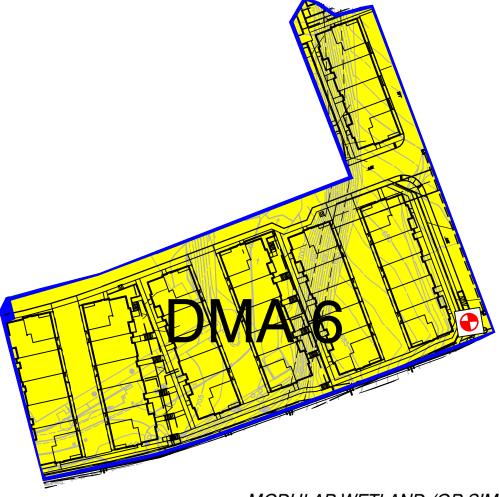
DMA 5 MAPBOOK FOR PALOMAR HEIGHTS

ESCONDIDO CALIFORNIA

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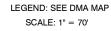
MODULAR WETLAND (OR SIMILAR)



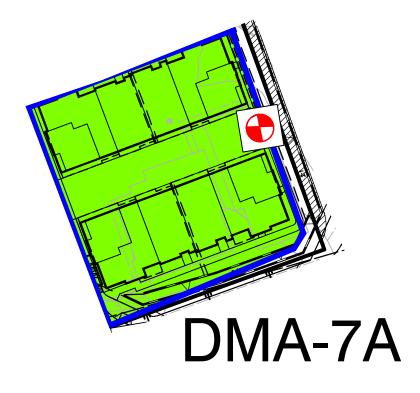
PREPARED BY:



DMA 6 MAPBOOK FOR PALOMAR HEIGHTS





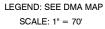


MODULAR WETLAND (OR SIMILAR)

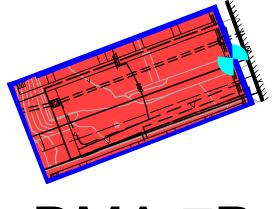




PLOONING 1907 Wighe Street BNGMESHING Son Diago, Co. 12121 SURVEYING PHOSSISSE-1600-1708/SISSE-160 DMA-7A MAPBOOK FOR PALOMAR HEIGHTS







DMA-7B

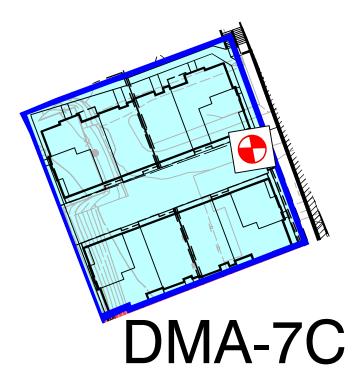
TREE WELL FOR WATER QUALITY



PREPARED BY:



DMA-7B MAPBOOK FOR PALOMAR HEIGHTS



LEGEND: SEE DMA MAP SCALE: 1" = 70'



MODULAR WETLAND (OR SIMILAR)

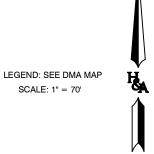


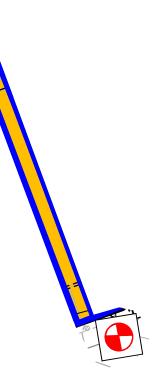
PREPARED BY:

HUNSAKER
& ASSOCIATES
BAN DIEC Q. INC

PLANNIC STOY WINDS STREET
BUCHERING SIN Dings, Ca 9229

DMA-7C MAPBOOK FOR PALOMAR HEIGHTS





DMA-7D

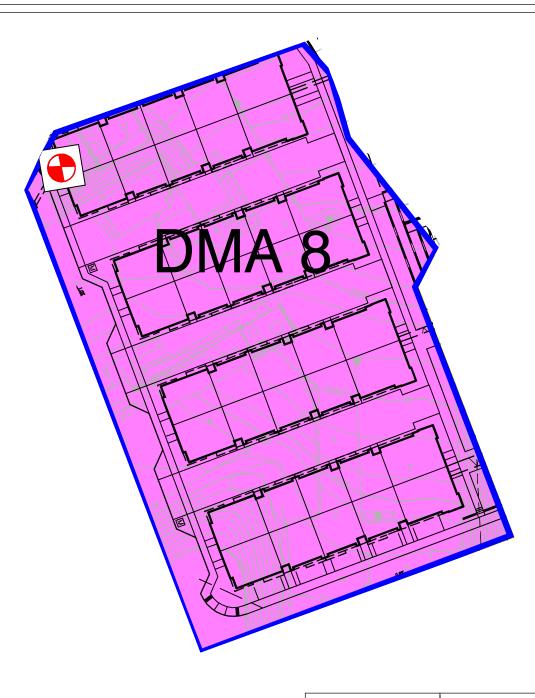
MODULAR WETLAND (OR SIMILAR)



PREPARED BY:



DMA-7D MAPBOOK FOR **PALOMAR HEIGHTS**



LEGEND: SEE DMA MAP SCALE: 1" = 70'



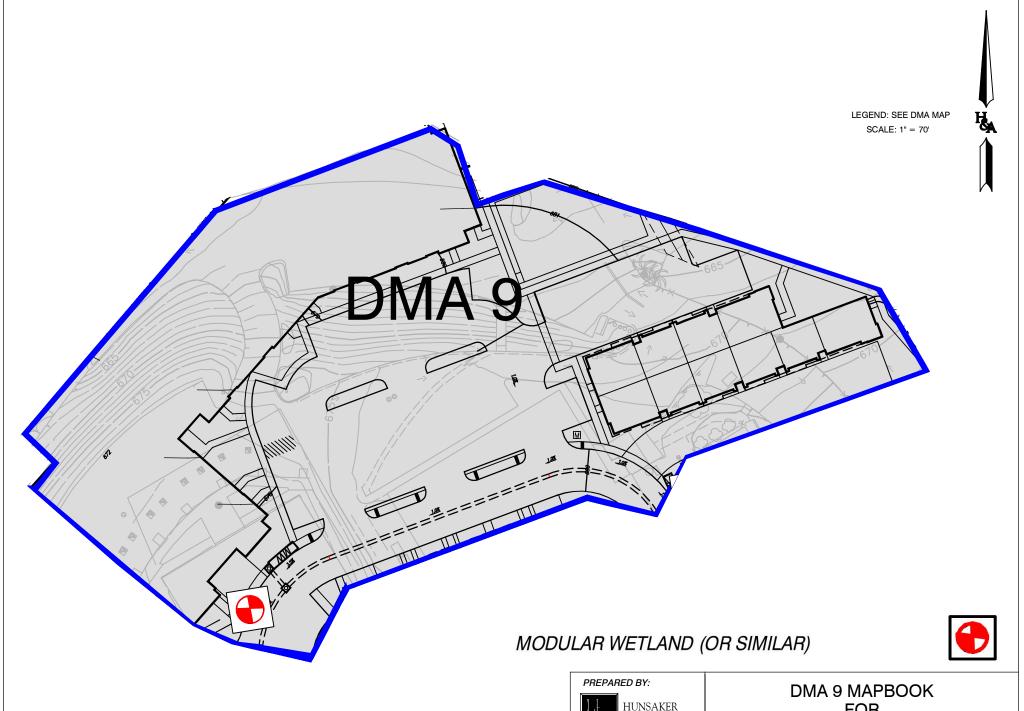
MODULAR WETLAND (OR SIMILAR)



PREPARED BY:

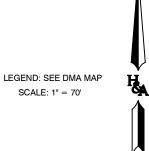


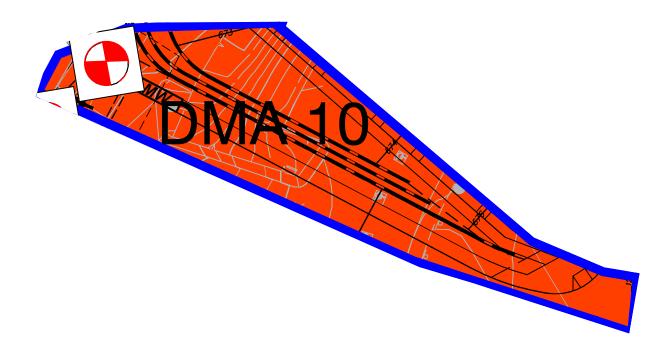
DMA 8 MAPBOOK FOR PALOMAR HEIGHTS





FOR PALOMAR HEIGHTS





MODULAR WETLAND (OR SIMILAR)



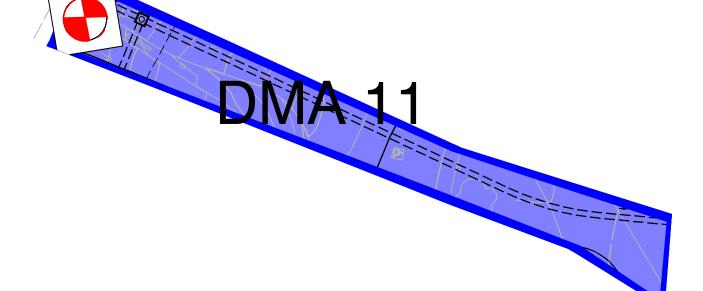
PREPARED BY:



DMA 10 MAPBOOK FOR PALOMAR HEIGHTS

LEGEND: SEE DMA MAP SCALE: 1" = 70'





MODULAR WETLAND (OR SIMILAR)

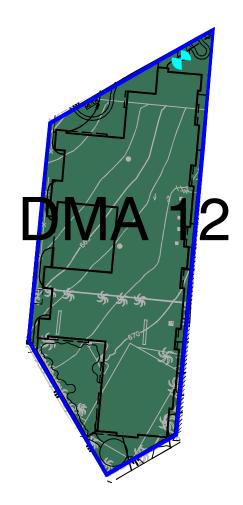


PREPARED BY:



DMA 11 MAPBOOK FOR PALOMAR HEIGHTS





TREE WELL FOR WATER QUALITY



PREPARED BY:



DMA 12 MAPBOOK FOR PALOMAR HEIGHTS

ESCONDIDO CALIFORNIA

SITE SPECIFIC DATA*				
PROJECT NAME				
PROJECT LOCATION				
STRUCTURE ID				
PERFORMANCE DATA				
TREATMENT VOLUME (CF)				
TREATMENT HGL (FT)		3.4		
BYPASS FLOW RATE (CFS)		DEPENDANT ON PIPE SIZE		

PROJECT PARAMETERS						
PIPE D	DATA	1.E.		MATERIAL		DIAMETER
INLET PIP	<u> </u>					
OUTLET P	IPE 1					
RIM ELEV	4 <i>TION</i>					
SURFACE	LOADING	REQUIR	PEME	ŊΤ	P.	ARKWAY
FRAME &	PRETRE	ATMENT	BIC	DFILTRA	1 <i>TION</i>	DISCHARGE
COVER	3	0	OF	PEN M	EDIA	24
WETLANDN	IEDIA VO	LUME (C	CY)			
MEDIA DE	LIVERED					
ORIFICE S	IZE (DIA,)				
MAX PICK WEIGHT (LBS)						
NOTES:						
*PER ENGINEER OF RECORD						

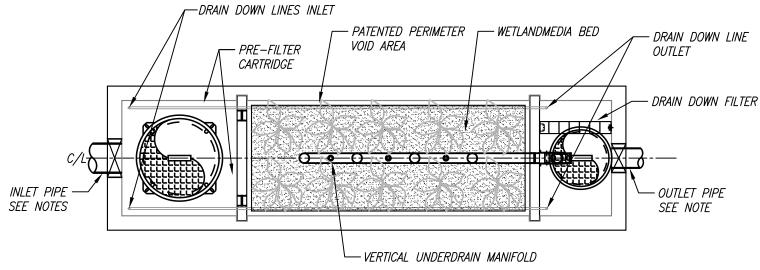
INSTALLATION NOTES

- CONTRACTOR TO PROVIDE ALL LABOR, EQUIPMENT, MATERIALS AND INCIDENTALS REQUIRED TO OFFLOAD AND INSTALL THE SYSTEM AND APPURTENANCES IN ACCORDANCE WITH THIS DRAWING AND THE MANUFACTURERS SPECIFICATIONS, UNLESS OTHERWISE STATED IN MANUFACTURERS CONTRACT.
- 2. MANUFACTURER RECOMMENDS A MINIMUM 6"LEVEL ROCK BASE UNLESS SPECIFIED BY THE PROJECT ENGINEER. CONTRACTOR IS RESPONSIBLE TO VERIFY PROJECT ENGINEERS RECOMMENDED BASE SPECIFICATIONS.
- 3. ALL PIPES MUST BE FLUSH WITH INSIDE SURFACE OF CONCRETE. (PIPES CANNOT INTRUDE BEYOND FLUSH).
- INVERT OF OUTFLOW PIPE MUST BE FLUSH WITH DISCHARGE CHAMBER FLOOR.
- ALL GAPS AROUND PIPES SHALL BE SEALED WATER TIGHT WITH A NON-SHRINK GROUT PER MANUFACTURERS STANDARD CONNECTION DETAIL AND SHALL MEET OR EXCEED REGIONAL PIPE CONNECTION STANDARDS.
- 6. CONTRACTOR RESPONSIBLE FOR INSTALLATION OF ALL RISERS, MANHOLES, AND HATCHES. CONTRACTOR TO GROUT ALL MANHOLES AND HATCHES TO MATCH FINISHED SURFACE UNLESS SPECIFIED OTHERWISE.

GENERAL NOTES

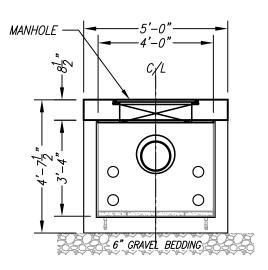
- MANUFACTURER TO PROVIDE ALL MATERIALS UNLESS OTHERWISE NOTED.
- ALL DIMENSIONS, ELEVATIONS, SPECIFICATIONS AND CAPACITIES ARE SUBJECT TO CHANGE. FOR PROJECT SPECIFIC DRAWINGS DETAILING EXACT DIMENSIONS, WEIGHTS AND ACCESSORIES PLEASE CONTACT MANUFACTURER.

PRELIMINARY MODULAR WETLAND BIOFILTRATION DETAIL. MORE INFORMATION WILL BE PROVIDED DURING A LATER SUBMITTAL



PLAN VIEW





· MANHOLE FLOW CONTROL -RISER

RIGHT END VIEW

ES	PLANT — TABLISHMENT MEDIA	AA.	SA.					
0.00 HGL						***]
	Marks providence providence (minorane)	5 }						
	PRETREATMENT CHAMBER	- 4"	ВІ	-9'-10" OFILTRATIO CHAMBER	N/	<i>4"</i> —►	DISCHARGE CHAMBER	9
6"	_		17 18	"-0"—— "-0"——			-	6"

~,	~	//	TI.	\sim A	. , .	// /-	V
	<i>F</i> 14	<i>,</i> 21	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	, ,,,,		<i>(IE</i> I	7/
LL	LV	_	,,,		4 V		~

MWS UNIT DESIGN DATA				
TREATMENT CAPACITY (CFS)	0.206			
OPERATING HEAD (FT)	3.4			
PRETREATMENT SURFACE AREA (SF	70.56			
WETLAND LOADING RATE (GPM/MIN	1.03			

MWS-L-4-17-V STORMWATER BIOFILTRATION SYSTEM STANDARD DETAIL

THE PRODUCT DESCRIBED MAY BE PROTECTED BY ONE OR MORE OF THE FOLLOWING US PATENTS: 7.425.262: 7.470.362: 7.674.378. 8,303,816; RELATED FOREIGN PATENTS OR OTHER PATENTS PENDING PROPRIETARY AND CONFIDENTIAL:

THE INFORMATION CONTAINED IN THIS DRAWING IS THE SOLE PROPERTY OF MODULAR WETLANDS SYSTEMS, ANY REPRODUCTION IN PART OR AS A WHOLE WITHOUT THE WRITTEN PERMISSION OF MODULAR WETLANDS SYSTEMS IS PROHIBITED.





DVERT TM MODULAR WATER QUALITY DIVERSION WEIR SYSTEM

PROPOSED CATCH BASIN

EQUIPED WITH MODULAR WETLAND

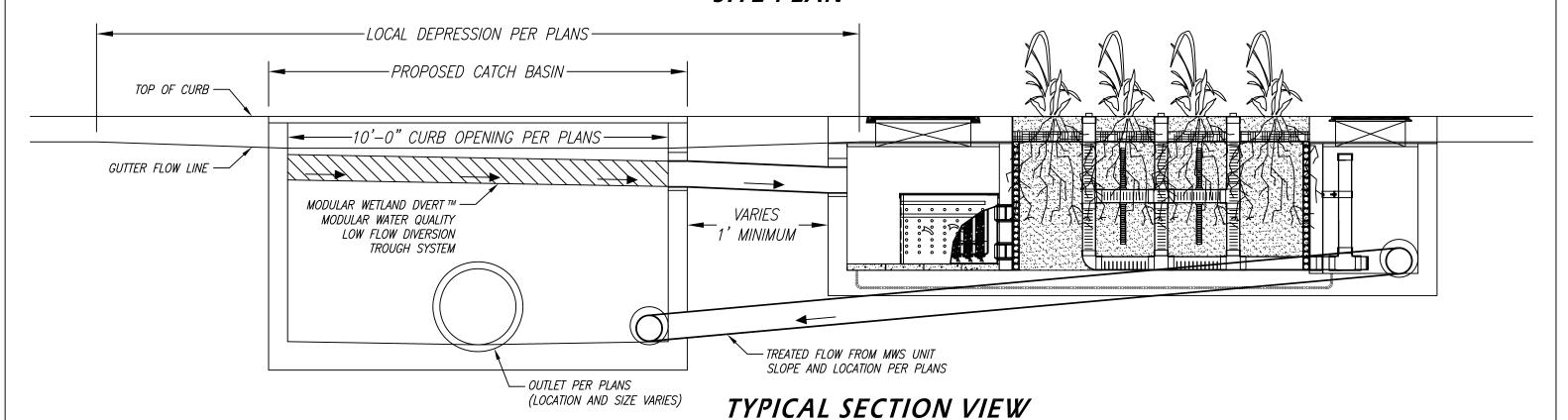
DVERT™ SYSTEM

MODULAR WETLAND VAULT TYPE

HYBRID BIOFILTRATION UNIT

PRELIMINARY MODULAR WETLAND BIOFILTRATION DETAIL. MORE INFORMATION WILL BE PROVIDED DURING A LATER SUBMITTAL

SITE PLAN



INSTALLATION NOTES:

- 1. TROUGH TO BE CONNECTED TO CONCRETE BELOW CURB OPENING USING 1/2" × 1-1/2" 316 STAINLESS STEEL SPIKE MUSHROOM HEAD DRIVE ANCHORS SPACED 12" ON CETNTER
- 2. USE DAP CONCRETE WATERTIGHT FILLER & SEALANT TO SEAL SEAM BETWEEN FIBERGLASS WEIR & CONCRETE WALL OF CATCH BASIN.

MODULAR WETLAND SYSTEMS INC. P.O. BOX 869		NAME	DATE	TITLE:		$\bigcap \bigcup \Box \bigcap$	T	
OCEANSIDE, CA 92049	DRAWN					DVLI	/	
www.ModularWetlands.com	EDITED					SYSTF	_///	
PROPRIETARY AND CONFIDENTIAL						$\mathcal{I}\mathcal{I}\mathcal{I}\mathcal{I}\mathcal{L}$	/ V /	
THE INFORMATION CONTAINED IN THIS DRAWING IS THE SOLE PROPERTY OF MODULAR WETLAND SYSTEMS INC. ANY REPRODUCTION IN PART OR AS A WHOLE WITHOUT THE WRITTEN PERMISSION OF	COMMENTS:			SIZE	DWG.	NO.		REV
MODULAR WETLAND SYSTEMS INC. IS PROHIBITED.				SCALE	NTS	UNITS = INCHES	SHEET	T 1 OF 1

ATTACHMENT 2

BACKUP FOR PDP HYDROMODIFICATION CONTROL MEASURES

This is the cover sheet for Attachment 2.

☐ Mark this box if this attachment is empty because the project is exempt from PDP hydromodification management requirements.

Indicate which Items are Included behind this cover sheet:

Attachment Sequence	Contents	Checklist
Attachment 2a	Flow Control Facility Design, including Structural BMP Drawdown Calculations and Overflow Design Summary (Required) See Chapter 6 and Appendix G of the Storm Water Design Manual	□Included □Submitted as separate stand- alone document
Attachment 2b	Hydromodification Management Exhibit (Required)	□Included See Hydromodification Management Exhibit Checklist on the back of this Attachment cover sheet.
Attachment 2c	Management of Critical Coarse Sediment Yield Areas See Section 6.2 and Appendix H of the Storm Water Design Manual.	□Exhibit depicting onsite and/or upstream sources of critical coarse sediment as mapped in the WMAA AND, □Demonstration that the project effectively avoids and bypasses sources of mapped critical coarse sediment OR, □Demonstration that project does not generate a net impact on the receiving water.
Attachment 2d	Geomorphic Assessment of Receiving Channels (Optional) See Section 6.3.4 of the Storm Water Design Manual.	□Not performed□Included□Submitted as separate standalone document
Attachment 2e	Vector Control Plan (Required when structural BMPs will not drain in 96 hours)	□Included □Not required because BMPs will drain in less than 96 hours

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Use this checklist to ensure the required information has been included on the Hydromodification Management Exhibit:

The Hydromodification Management Exhibit must identify:
□Underlying hydrologic soil group
□Approximate depth to groundwater
□Existing natural hydrologic features (watercourses, seeps, springs, wetlands)
□Critical coarse sediment yield areas to be protected
□Existing topography
□Existing and proposed site drainage network and connections to drainage offsite
□ Proposed grading
□Proposed impervious features
□ Proposed design features and surface treatments used to minimize imperviousness
□Point(s) of Compliance (POC) for Hydromodification Management
□Existing and proposed drainage boundary and drainage area to each POC (when necessary
create separate exhibits for pre-development and post-project conditions)
☐ Structural BMPs for hydromodification management (identify location, type of BMP, and
size/detail)

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ATTACHMENT 3

Structural BMP Maintenance Information

This is the cover sheet for Attachment 3.

Indicate which Items are Included behind this cover sheet:

Attachment Sequence	Contents	Checklist
Attachment 3a	Structural BMP Maintenance Plan (Required)	□Included
		See Structural BMP Maintenance Information Checklist on the back of this Attachment cover sheet.
Attachment 3b	Draft Storm Water Control Facilities Maintenance Agreement (SWCFMA) (when applicable)	□Included □Not Applicable

ATTACHMENT WILL BE PROVIDED DURING FINAL ENGINEERING.

This page was left intentionally blank.

Use this checklist to ensure the required information has been included in the Structural BMP Maintenance Information Attachment:

Attachment 3a must identify:

Specific maintenance indicators and actions for proposed structural BMP(s). This must
be based on Section 7.7 of the Storm Water Design Manual and enhanced to reflect
actual proposed components of the structural BMP(s)
☐ How to access the structural BMP(s) to inspect and perform maintenance
□ Features that are provided to facilitate inspection (e.g., observation ports, cleanouts, silt
posts, or other features that allow the inspector to view necessary components of the structural BMP and compare to maintenance thresholds)
☐ Manufacturer and part number for proprietary parts of structural BMP(s) when applicable
☐ Maintenance thresholds specific to the structural BMP(s), with a location-specific frame
of reference (e.g., level of accumulated materials that triggers removal of the materials, to be identified based on viewing marks on silt posts or measured with a survey rod with respect to a fixed benchmark within the BMP)
☐Recommended equipment to perform maintenance
□When applicable, necessary special training or certification requirements for inspection
and maintenance personnel such as confined space entry or hazardous waste
management

Attachment 3b: For all Structural BMPs, Attachment 3b must include a draft maintenance agreement in the City's standard format (PDP applicant to contact City staff to obtain the current maintenance agreement forms or download from City's website).

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ATTACHMENT 4

City of Escondido PDP Structural BMP Verification for Permitted Land Development Projects

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City of Escondido Storm Water Structural BMP Verification Form Page 1 of 4			
Project Sur	nmary Information		
Project Name: PALOMAR HEIGHTS			
Record ID (e.g., grading/improvement plan number)	TBD		
Project Address	555 E VALLEY PKWY, ESCONDIDO, CA 92025		
Assessor's Parcel Number(s) (APN(s))	229-450-05 & 06, 230-163-01, 02, 03 & 05		
Project Watershed	CARLSBAD 904		
(Complete Hydrologic Unit, Area, and Subarea Name with Numeric Identifier)			
Maintenance Notification / Agreement No.			
Responsible Party	y for Construction Phase		
Developer's Name	INTEGRAL PARTNERS LLC		
Address	2235 ENCINITAS BOULEVARD SUITE 216 ENCINITAS, CA 92024		
Email Address			
Phone Number			
Engineer of Work	RAYMOND L. MARTN		
Engineer's Phone Number	(858) 558-4500		
Responsible Party	for Ongoing Maintenance		
Owner's Name(s)*	TBD		
Address			
Email Address			
Phone Number			
*Note: If a corporation or LLC, provide inform	nation for principal partner or Agent for Service of		

*Note: If a corporation or LLC, provide information for principal partner or Agent for Service of Process. If an HOA, provide information for the Board or property manager at time of project closeout.

City of Escondido Storm Water Structural BMP Verification Form Page 2 of 4 Stormwater Structural Pollutant Control & Hydromodification Control BMPs* (List all from SWQMP)				
Description/Type of Structural BMP	Plan Sheet #	Structural BMP ID#	Maintenance Agreement Recorded Doc #	Revisions
Proprietary biofiltration unit_	TBD	MWS -1		
	1 [MWS -2		
		MWS - 3		
		MWS-4		
		MWS -5		
		MWS - 6		
		MWS - 7		
		MWS - 8		
		MWS - 9		
		MWS -10		
V	<u> </u>	MWS - 11		
*All Priority Development Pro	jects (PDP:	s) require a Structu	ral BMP	

Note: If this is a partial verification of Structural BMPs, provide a list and map denoting Structural BMPs that have already been submitted, those for this submission, and those anticipated in future submissions.

City of Escondido Storm Structural BMP Verification Form Page 3 of 4

Checklist for Engineer of Work (EOW) to submit to Field Engineering:

 Copy of the final accepted SWQMP and any accepted Copy of the most current plan showing the Storm Was plans/cross-section sheets of the Structural BMPs are built Structural BMP. Photograph of each Structural BMP. Photograph(s) of each Structural BMP during the comproper construction. Copy of the approved Structural BMP maintenance as 	ater Structural BMP Table, and the location of each verified as-
By signing below, I certify that the Structural BMP(s) for this BMPs are in substantial conformance with the approved understand the City reserves the right to inspect the above approved plans and Storm Water Ordinance. Should it be constructed to plan or code, corrective actions may be nece	plans and applicable regulations. In BMPs to verify compliance with the determined that the BMPs were not
Please sign your name and seal.	
Professional Engineer's Printed Name:	[SEAL]
Professional Engineer's Signed Name:	
Date:	

City of Escondido Storm Water Structural BMP Verification Form Page 4 of 4

CITY - OFFICIAL USE ONLY:

Permit #:	
ty Inspector:	
ate Project has/expects to close:	
ate verification received from Engineer of Work (EOW):	
y signing below, City Inspector concurs that every noted Structural BMP has been insan.	stalled per
ty Inspector's Signature:Date:	
OR Environmental Programs:	
ate Received from Field Engineering:	
nvironmental Programs Submittal Reviewer:	
nvironmental Programs Reviewer concurs that the information provided for the follow ructural BMPs is acceptable to enter into the Structural BMP Maintenance verificatio ventory:	
List acceptable Structural BMPs:	
nvironmental Programs Reviewer's Signature:	

Preparation Date: 6/18/2019

Template Date: October 2016 **PDP** SWQM**P** - Attachments

ATTACHMENT 5

Copy of Plan Sheets Showing Permanent Storm Water BMPs, Source Control, and Site Design

This is the cover sheet for Attachment 5.

Use this checklist to ensure the required information has been included on the plans:

The plans must identify:

- ⊠Structural BMP(s) with ID numbers matching Step 6 Summary of PDP Structural BMPs
- ⊠The grading and drainage design shown on the plans must be consistent with the delineation of DMAs shown on the DMA exhibit
- ☑Details and specifications for construction of structural BMP(s)
- ⊠Signage indicating the location and boundary of structural BMP(s) as required by City staff
- ⊠How to access the structural BMP(s) to inspect and perform maintenance
- ☑Features that are provided to facilitate inspection (e.g., observation ports, cleanouts, silt posts, or other features that allow the inspector to view necessary components of the structural BMP and compare to maintenance thresholds)
- ⊠Manufacturer and part number for proprietary parts of structural BMP(s) when applicable
- Maintenance thresholds specific to the structural BMP(s), with a location-specific frame of reference (e.g., level of accumulated materials that triggers removal of the materials, to be identified based on viewing marks on silt posts or measured with a survey rod with respect to a fixed benchmark within the BMP)
- ⊠Recommended equipment to perform maintenance
- ⊠When applicable, necessary special training or certification requirements for inspection and maintenance personnel such as confined space entry or hazardous waste management
- ⊠Include landscaping plan sheets showing vegetation requirements for vegetated structural BMP(s)
- ⊠All BMPs must be fully dimensioned on the plans
- ⊠When proprietary BMPs are used, site-specific cross section with outflow, inflow, and model number must be provided. Photocopies of general brochures are not acceptable.
- □ Include all source control and site design measures described in Steps 4 and 5 of the SWQMP. Can be included as a separate exhibit as necessary.

*Note: Plan sheets included in this attachment can be full size or half size.

VICINITY MAP

LOT 3 10.06 AC

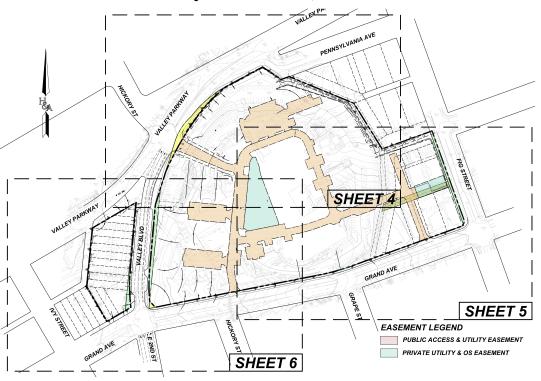
~~¤ CLEANOUT HEADWALL
RIP-RAP
CATCH BASIN/INLET

602.0

-600-

TENTATIVE MAP PALOMAR HEIGHTS

City of Escondido, California



APPDEVIATIONS

LEGEND EXISTING PROJECT BOUNDARY PROPOSED PROJECT BOUNDARY

LOT NUMBER & LOT AREA PROPOSED LOT LINE PROPOSED EASEMENT LINE PROPOSED SEWER MAIN WITH MANHOLE EXISTING SEWER MAIN WITH MANHOLE

PROPOSED FIRE HYDRANT

EXISTING FIRE HYDRANT PROPOSED PVT. ST. LIGHT

EXISTING CONTOURS

PROPOSED CONTOURS PROPOSED SLOPE (2: 1 MAX.)

CUT/FILL LINE

PROPOSED WATER MAIN
(8" PVC UNLESS OTHERWISE NOTED)

PROPOSED CENTERLINE PVT. ST. ELEVATION

I IMITS OF PROPOSED GRADING/DAYLIGHT -------

1DDI	REVIATIONS		
FL	FLOW LINE		
TW	TOP OF WALL	R/W	RIGHT OF WAY
BW	BOTTOM OF WALL	PL	PROPERTY LINE
TF	TOP OF FOOTING	GB	GRADE BREAK
FS	FINISH SURFACE	PI	POINT OF INTERSECTION (V.C.
S	SEWER	P	PAD ELEVATION
W	WATER	SF	GROSS SQ. FT.
RW	RECLAIMED WATER	NSF	NET SQ. FT.
SD	STORM DRAIN	FP	FLOOD PLAIN
NTS	NOT TO SCALE	VC	VERTICAL CURVE
ELEV	ELEVATION	MH	MANHOLE
ΙE	INVERT ELEVATION	ROP	REINFORCED CONCRETE PIPE

PUBLIC UTILITIES AND DISTRICTS

WATER
SEWER
POLICECITY OF ESCONDIDO
FIRE
GAS AND ELECTRICITYSAN DIEGO GAS AND ELECTRIC
HIGH SCHOOL DISTRICTESCONDIDO UNION HIGH SCHOOL DISTRICT
ELEMENTARY SCHOOL DISTRICTESCONDIDO UNION ELEMENTARY SCHOOL DISTRICT

CONDOMINIUM NOTE

THIS IS A MAP OF A CONDOMINIUM PROJECT AS DEFINED IN SECTION 4125 OF THE CIVIL CODE OF THE STATE OF CALIFORNIA AND IS FILED PRESUMT TO THE SUBDIVISION MAP ACT. THE TOTAL MAMER'OF OF RESIDENTIAL CONDOMINIUM DIRELLING UNITS IS 510 (90 UNITS LOT 1, 148 UNITS LOT 2, 70 UNITS LOT 3, 40 UNITS LOT 6, 400 30 UNITS LOT 5, 42 UNITS LOT 6, A00 30 UNITS LOT 7,

PROJECT SUMMARY

PROJECT SUMMARY	SF	AC
TOTAL GROSS SITE AREA	601,247	13.80
DEDICATION	5,395	0.12
VACATION	4,813	0.11
ACQUISITION	7,767	0.18
TOTAL NET PROJECT AREA	600,665	13.97
PROPOSED PRIVATE DRIVE AISLES/PARKING	154,464	3.55
PROPOSED BUILDING COVERAGE	241,367	5.54
PROPOSED LANDSCAPING COVERAGE	204,834	4.88
TOTAL PROPOSED NET SITE AREA	600,665	13.97

LOTTING SUMMARY

OT SUMMARY	SF	AC
LOT 1	46,150	1.06
LOT 2	129,549	2.97
LOT 3	87,213	2.00
LOT 4	39,130	0.90
LOT 5	137,980	3.17
LOT 6	73,176	1.68
LOT 7	60,517	1.39
LOT A	34,771	0.80
Lot Area Total	608,486	13.97

PROJECT MAP

TOPOGRAPHY

TOPO SOURCE: R.J. LUNG CO., INC. - FLOWN FEBRUARY 9, 2018 BENDHWARK: CITY OF ESCONDIDO NO. 357-A, CHISELED BOX CUT ON TOP OF CURB 20 FEET WEST OF GRAND AVE. ENTRANCE TO HOSPITAL PARKING STRUCTURE. EL 862.86' (NOVO '29)

OPEN SPACE SUMMARY

REQUIRED: 300 S.F. PER UNIT X 510 UNITS TOTAL REQUIRED - 153,000 S.F

SOILS ENGINEER

SAN DIEGO, CA 92121

LEGAL DESCRIPTION

SEE SHEET NO. 8 FOR LEGAL DESCRIPTION

EASEMENT NOTES

SEE SHEET NO. 8 FOR EASEMENT INFORMATION

SHEET INDEX

SHEET 1 TITLE SHEET EXISTING STREET SECTIONS
PROPOSED STREET SECTIONS & DETAILS SHEET 4 PROJECT DESIGN SHEET 5 PROJECT DESIGN

PROJECT DESIGN EXISTING TOPO AND CONDITIONS MAD RECORD BOUNDARY & ENCUMBRANCES

GENERAL NOTES

TOTAL PROJECT GROSS SITE AREA:
1071AL PROJECT PROVOSED NET SITE AREA:
1071AL PROJECT PROVOSED NET SITE AREA:
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1071AL PROPOSED SITE PROJECT WACHTON AREA:
1071AL PROPOSED SITE PROJECT WACHTON AREA:
1071AL PROPOSED SITE PROJECT P

PROJECT GROSS/NET DENSITY: 36.5
THOMAS BROTHERS COORDINATES: 1129J2 & 1130A2
INDIVIDUAL TRASH PICKUP IS PROPOSED WITH THIS SUBDIVISION.

GENERAL DESIGN NOTES

1 ALL PRIVATE STREET DESIGNS PRIVATE STREET LIGHTS AND FIRE HYDRANTS SHALL CONFORM TO CITY OF ESCONDIDO DESIGN STANDARDS AND/OR AS APPROVED BY THE CITY

CONTINUED SHALL BE FOR CITY POINTERS AND PUBLIC UTILITIES AND DISTRICTS.

BRYTISHER FINENDLES 28 AND 10 FOOT
FINISHED GROODS ARE APPROXIMETE OUTY AND SUBJECT TO CHANGE IN FINAL DESIGN
CONSISTENT WITH THE CITY'S SUBSTANTIAL CONFORMACE FOLLOY,
APPROXIMETE FAM GROINGO COUNTITIES FAM DISTRICT 103,000 C.Y.

FAM QUANTITIES SHAW DO NOT INCLUSE THE FILL TO 3,000 C.Y.
FAM QUANTITIES SHAW DO NOT INCLUSE THE FIRST TO CONFORMATE FOR THE FIRST THE FOOD THE FIRST TH THE FINAL MAP MAY CONSIST OF ONE OR MORE MULTIPLE LOTS AS SHOWN ON THIS

TENTATIVE MAP. A SOILS REPORT WAS PRODUCED IN COORDINATIONW ITH THIS PROJECT FROM GEOCON INCORPORA: WHICH IS DATED SPETEMBER 9, 2019.

OWNER'S CERTIFICATE

APPLICANT/SUBDIVIDER

SUITE 216 ENCINITAS, CA 92024

APPLICANT DATE

ENGINEER



RAYMOND L. MARTIN R.C.L MY REGISTRATION EXPIRES ON 6/30/20

PREPAR	ED BY:	NO.	REVISIONS	DATE	BY
		- 1	PRE-APP	12/20/18	H&A
\vdash	HUNSAKER	2	1ST SUBMITTAL	06/25/19	H&A
1,00	& ASSOCIATES	3	2ND SUBMITTAL	09/11/19	H&A
- CYF1	SAN DIEGO, INC	4			
		5			
PLANNING ENGINEERING	9707 Waples Street San Dieso, Ca 92121	6			
SURVEYING	PH(858)558-4500 - FX(858)558-1414	7			

TENTATIVE MAP PALOMAR HEIGHTS

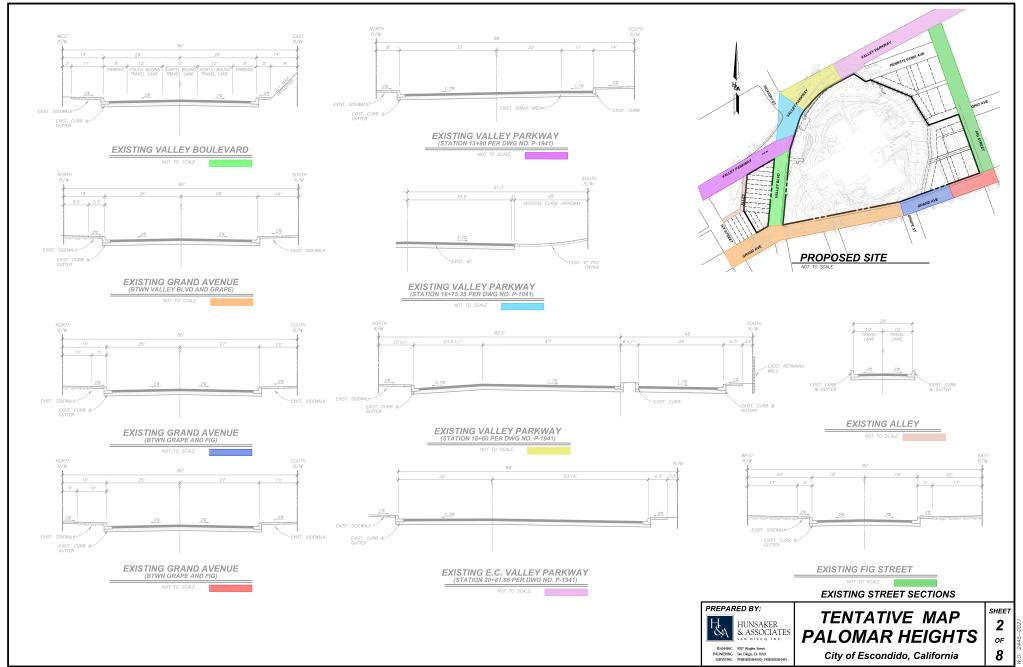
City of Escondido, California

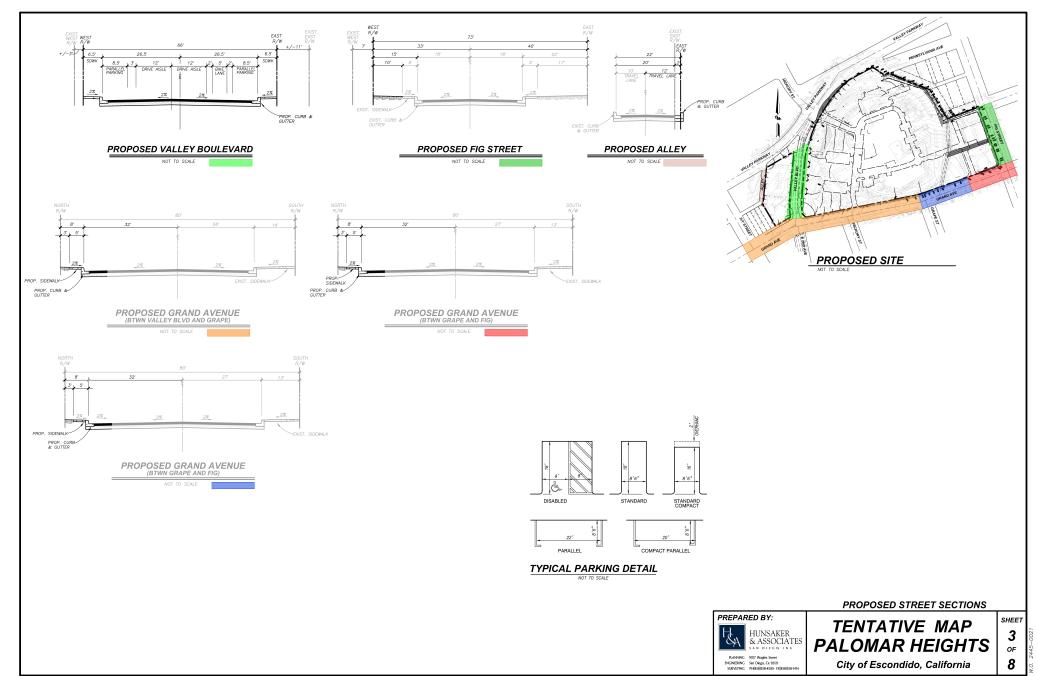
OF 8

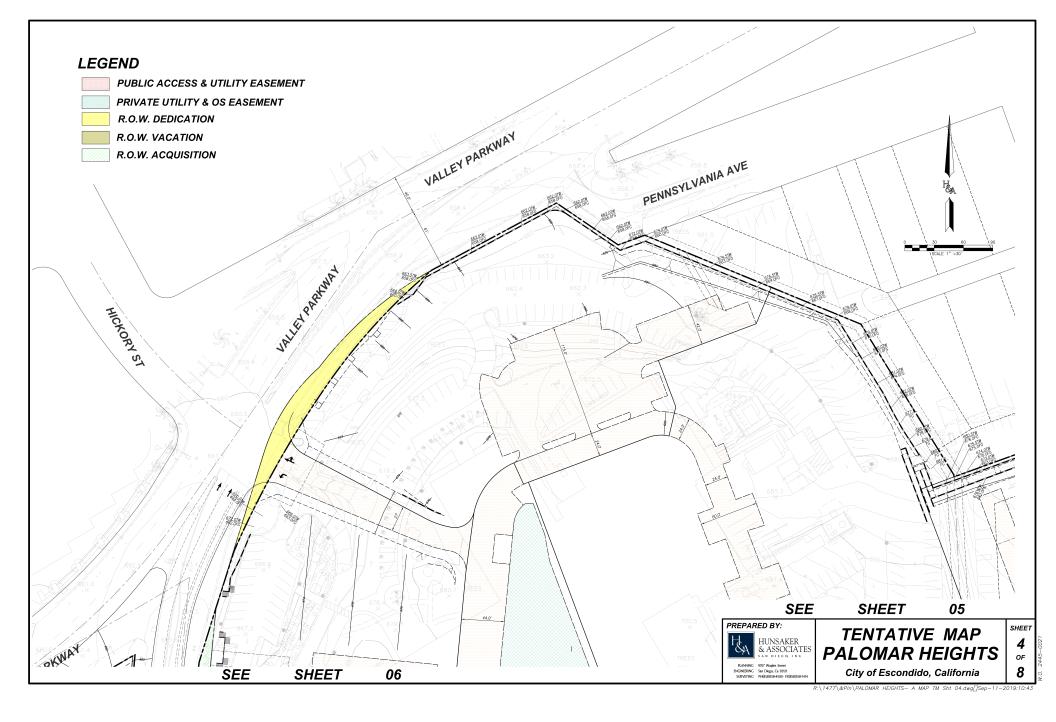
SHEET

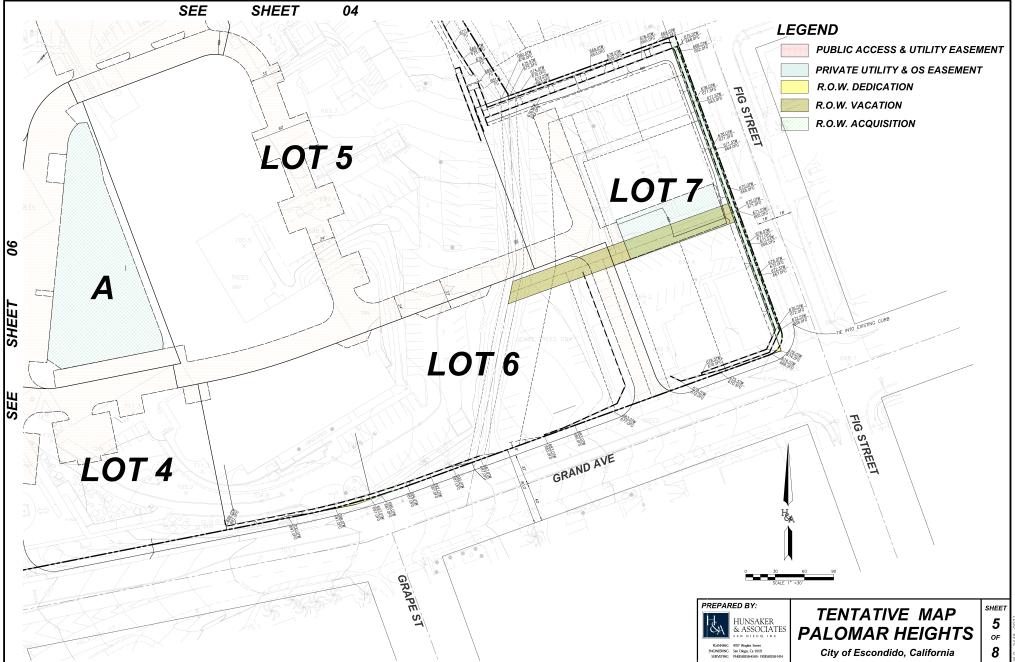
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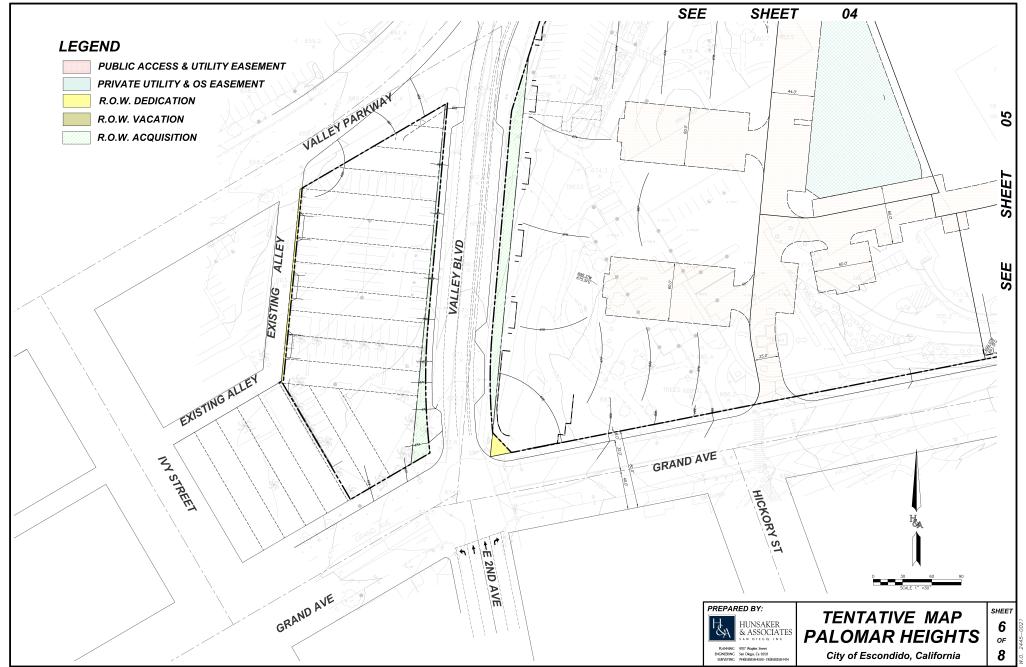
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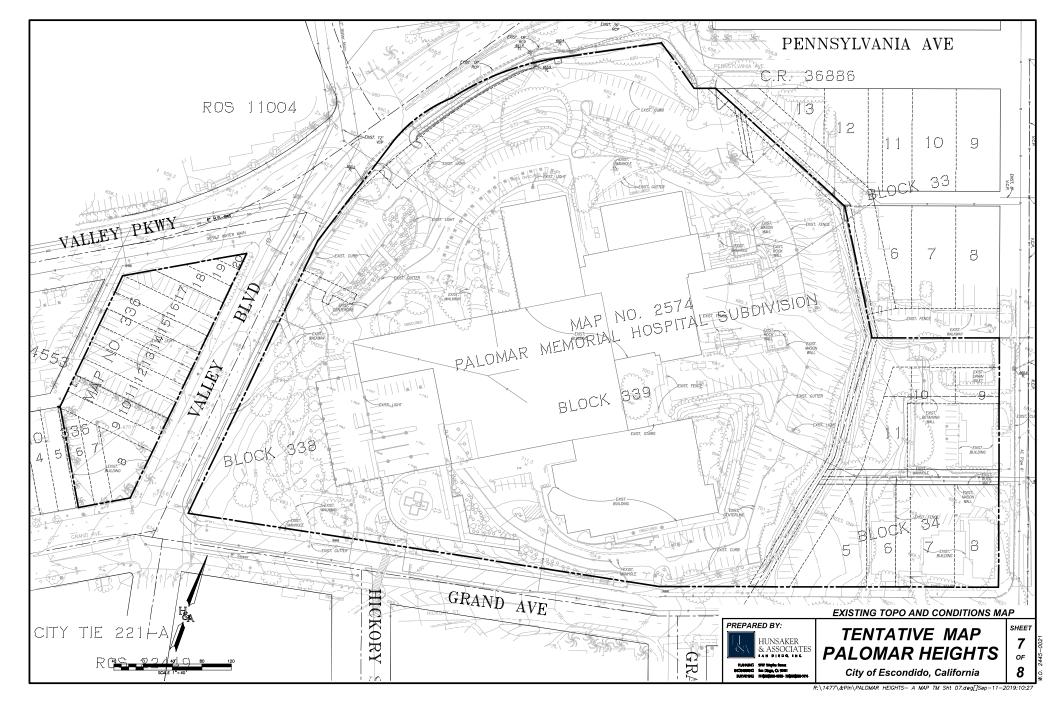












LEGAL DESCRIPTION:

THE LAND REFERRED TO HEREIN BELOW IS SITUATED IN THE CITY OF ESCONDIDO, IN THE COUNTY OF SAN DIEGO, STATE OF CALIFORNIA, AND IS DESCRIBED AS FOLLOWS:

LOCATION OF PAGAMEN MEMORIAL POSTILL SURVIVIOUS, WITHER CITY OF ESCONDIOL, COUNTY OF SAN DROOD O

PARCEL B: (APN: 229-450-05)

SCOT SIZE OF PARAMEN MARKON, PROPERLY, SUPPOSED, IN THE STY OF SCHOOLD COURTY OF AN PERLO, STATE OF APPRIANCE ACCORNIC TO METHER OF ALL STATE DID IN THE COTTO, OF THE COURTY PROTECTION OF SAN DEED COURTY, JUNE 1, 1949 (OCCENTE WITH THAT PORTION OF HOLDING STREET AUGUSTIN TO SAN DE SAN ON THE SIZE OF WORDED AND COSED TO PHILDE USE, OF CORPREDID OFF OF SESSULTION NO. 47—209 RECORDED JANUSEY 4, 1969, IT IN THE THAT STREET AND ALL STREET STREET AND ALL

PARCEL D: (APN: 230-163-01)

ALL THAT PORTION OF LOTS 8, 10, AND 11 OF BLOCK 34 AND OF THE EXSTERTY ONE HALF OF GRAPE STREET ADDRIVES AND LOTS 11 ON THE REST AS MAJETIS AND CLOSED TO PENILL USE AND OF A PORTION OF OH-OH ARRIVES AS MAJETIS AND CLOSED THE PENILL USE AN POLICY EXPONENCE AND ADDRIVES AND ADDRIVES AND ADDRIVES AND ADDRIVES AND ADDRIVES AND ADDRIVES AND ADDRIVES AND ADDRIVES AND ADDRIVES AND ADDRIVES AND THE ADDRIVES AND A

PARCEL E: APN 230-163-03

LOTS 7 AND 8 AND THE EASTERLY 20 TEET OF LOT 8, OF BLOCK 34 OF PALCHAR MEMORIAL HOSPITAL SUBDIVISION, IN THE CITY OF ESCONDOD, COUNTY OF SAM DEGO, STATE OF GULFORMA, ACCORDING TO MAP THEFEOF NO. 2574, TELD IN THE OTHER OF THE COUNTY OF MULTI, 1984. TODETHE WITH THE SOUTHERS WHITE WHITE THE SOUTHERS WHITE THE SOUTHERS WHITE THE SOUTHERS WHITE THE SOUTHERS WHITE THE SOUTHERS WHITE THE SOUTHERS WHITE THE SOUTHERS WHITE THE SOUTHERS WHITE THE SOUTHERS WHITE THE SOUTHERS WHITE THE SOUTHERS WHITE THE SOUTHERS WHITE THE SOUTHERS WHITE THE SOUTHERS WHIT THE SOUTHERS WHITE THE SOUTHERS WHITE THE SOUTHERS

ALL THAT PORTION OF LOTS 9, 10 AND 11 OF BLOCK 34 OF PALOMAR MEMORIAL HOSPITAL SUBDIVISION, IN THE CITY OF ESCONDING COUNTY OF SAN DECO, STATE OF CALIFORNIA, ACCORDING TO MAY THEREOF 2574, FILED IN THE OFFICE OF THE COUNTY MECONDER OF SAID SAN DECO COUNTY, JULY 1, 1949, DESCRIBED AS FOLLOWS.

REGINNOL AT THE SOUTHEASTERY CORPET OF SUD LOT 9, THENCE ADMY THE EASTERY BURNARY THEREOF.

SOUTH BY 32-0 WEST TEXOS OF TEXT THENCE PARKELL WITH THE EASTERY BURNARY OF SAY LOT 9.

SOUTH BY 32-0 WEST TEXOS OF TEXT THENCE PARKELL WITH THE EASTERY BURNARY OF SAY LOT 9. SOUTH

FOR THE TEXT THE SOUTHER SOUTH OF THE SOUTH OF THE SOUTHER THE SOUTH

LOTS 18, 19 AND 20 N BOOK 64 OF ESCONDOD, CITY OF ESCONDOD, COUNTY OF SAN DIEGO, STATE OF CHARGONNA, ACCORDING TO LAMP THEREOF NO. 338, FILED IN THE OFFICE OF THE COUNTY RECORDER OF SAN DIEGO COUNTY, JULY 10, 1866

PARCEL H: APN 229-442-03 & 18

LOTS 6, 7, 8, 9, 10, 14 AND 15 EXCEPT THE WEST 3 FEET OF SAID LOT 6, IN BLOCK 64 OF ESCONDIDO, COUNTY OF SAID NEGO, STATE OF CALIFORNIA, ACCORDING TO MAP THEREOF NO. 336, FILED IN THE OFFICE OF THE COUNTY RECORDER OF SAN DIEGO COUNTY, JULY 10, 1886.

PARCEL I: APN 229-442-04

LOTS 11, 12 AND 13 IN BLOCK 64 OF THE CITY OF ESCONDIDG, COUNTY OF SAN DIEGO, STATE OF CAUFORNIA, ACCORDING TO MAP THEREOF NO. 336, FILED IN THE OFFICE OF THE COUNTY RECORDER OF SAN DIEGO COUNTY, UNLY 10, 1886.

LOTS 16 & 17 IN BLOCK 64 IN THE CITY OF ESCONDIDO, IN THE COUNTY OF SAN DIEGO, STATE OF CALIFORNIA, ACCORDING TO MAP THEREOF NO. 336, ON FILE IN THE OFFICE OF THE COUNTY RECORDER OF SAN DIEGO COUNTY.

LOT 6 N BLOCK 34 OF PALOMAR MEMORIAL HOSPITAL SUBDIVISION, IN THE CITY OF EXCONDIDO, COUNTY OF SAN DEGO, STATE OF CALFORNA, ACCORDING TO MAP NO. 2574, RIED IN THE OFFICE OF THE COUNTY RECORDER OF SAN DIEGO COUNTY, AUY 1, 1949, EXCEPTING THEREFORM THE EXISTEN OF DETET THEREO. TO CONTINUE WITH THE SOUTHERLY HAUF OF THE ALLEY CONTIQUOUS WITH SAID PARCEL WHICH UPON WACKTON WOULD REVERT TO SAID LAND BY OPERATION OF LAW,

ENCUMBRANCES:

THE FOLLOWING MATTERS AFFECT PARCELS A. B. C AND D:

- AS RESERVED IN A DOCUMENT. AS MESENTEL IN A DOCUMENT; OPERATION AND MAINTENANCE OF PUBLIC UTILITIES PECORDING DATE: MAY 29, 1961 RECORDING NO: 92029 OF OFFICIAL RECORDS AFFECTS: A FORTION OF PARCELS A, C AND D
- A ESBERTIST FOR THE PRINCESS, SHOWN BELOW AND RIGHTS INCODENTAL THERETO AND PROPERTY OF THE PROPERTY OF THE PROPERTY OF THE PROPERTY OF THE PROPERTY OF THE TRANSMISSION AND DISTRIBUTION OF ELECTRICITY RECORDING FOR INVOICEMENT AND DISTRIBUTION OF ELECTRICITY RECORDING FOR INVOICEMENT AND OFFICE AND PROPERTY AND PROPERTY AND THE PROPERTY OF THE PRO
- A EXEMPTION FOR THE PURPOSES(S) SHAWN BELOW AND RIGHTS INCIDENTAL THERETO, AS GRANTED IN A DOCUMENT:

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- LESINGTICS FOR THE PURPOSESS SHOWN BELOW AND ROUTS INCIDENTAL THERETO, SHIRED TO, CUT OF ESCONDAGO PRINTERNANCES THERETO RECOGNICE DATES STORM AND PRINTERNANCES THERETO RECOGNICE DATES STORM AND PRINTERNANCES THERETO RECOGNICE DATES STORM OF PAREED A THEM, RECORDS AFFECTS A PORTION OF PAREED.
- A LEASONING FOR THE NUMBOOKES SHOWN BELOW AND RIGHTS INCORPITAL THERETO, AND CHARLEST OF THE TROUBLOOD FUNDED. FRANCE SHOWS THE STORY OF THE PROPERTY OF THE

THE FOLLOWING MATTERS AFFECT PARCEL H:

AN EXSEMENT FOR THE PURPOSE SHOWN BELOW AND RIGHTS INCIDENTAL THERETO AS SET FORTH IN A DOCUMENT.
GRAHED TO, SAM DEGO JOS AND ELECTRIC COMPANY
PURPOSE, PLANT OF UTILITES, INCRESS, EDEESS
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THE FOLLOWING MATTERS AFFECT PARCEL I:

THE FOLLOWING EASEMENTS RESERVED BY THE ESCONDIDO LAND & TOWN COMPANY (SUCCEEDED BY THE CITY OF ESCONDIDO) FOR THE PIPELINES AS FOLLOWS: IN BOOK 67, PAGE 35 OF DEEDS, AFFECTS LOT 13; IN BOOK 67, PAGE 37 OF DEEDS, AFFECTS LOTS 11 & 12;

THE FOLLOWING MATTERS AFFECT PARCEL J:

THE FOLLOWING EASEMENTS RESERVED BY THE ESCOUNDED LAND & TOWN COMPANY (SUCCEEDED BY THE CITY OF ESCOUNDED) FOR THE PEPELNES AS FOLLOWS:

IN BOOK 118, PAGE 58 OF DEEDS, AFFECTS LOT 17,

IN BOOK 118, PAGE 59, OF DEEDS, AFFECTS LOT 17,

THE FOLLOWING MATTERS AFFECT PARCEL HE AN EXEMENT FOR THE PURPOSE SHOWN BELOW MAD RIGHTS INCIDENTAL THERETO AS SET FORTH IN A DOCUMENT.
GRANIED TO. SAN DIEGO GAS AND ELECTRIC COMPANY PURPOSE: PUBLIC UTLITIES, INCRESS, EGRESS
RECORRELE. APRIL 28, 1932 SE INSTRUMENT NO. 1932-0248179, OF OFFICIAL

RECORDS
AFFECTS: THE NORTHERLY 13.00 FEET OF THE WESTERLY 8.00 FEET OF SAID LOT 10

YALLEY PARKWAY PENNSYLVANIA AVE 455-4828*# 76.20 À B=195_ ~ R=367 OHIO AVE N70054075 174.58 PCL A APN 229-450-08 PALOMAR MEMORIAL HOSPITAL SUBDIVISION MAP NO 2574 339 ANTIEN LABORATA WOODS SEE 462.14 APN 230-158-04.

GRAND AVE 338 PCL B N78'50'55'E 650,50' HICKORY GRAND AVE

BOUNDARY AND ENCUMBRANCES



TENTATIVE MAP PALOMAR HEIGHTS

City of Escondido, California

8

R:\1477\&PIn\PALOMAR HEIGHTS- A MAP TM Sht 08.dwg[]Sep-11-2019:10:26

SHEET 8 OF

VICINITY MAP

LOT 3 10.06 AC

•—Ø CLEANOUT HEADWALL
RIP-RAP
CATCH BASIN/INLET

602.0

-600-

MW

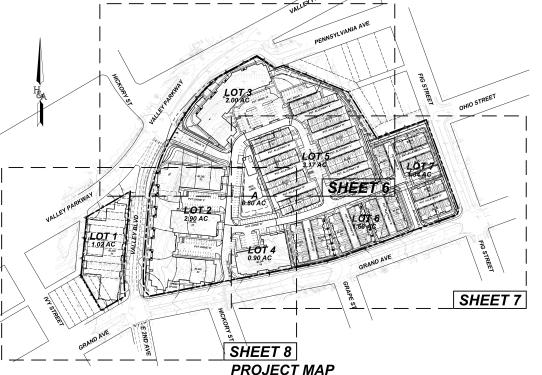
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... CITY OF ESCONDIDO

....CITY OF ESCONDIDO
....CITY OF ESCONDIDO
...SAN DIEGO GAS AND ELECTRIC
....ESCONDIDO UNION HIGH SCHOOL DISTRICT
....ESCONDIDO UNION ELEMENTARY SCHOOL DISTRICT

PRELIMINARY GRADING PLAN PALOMAR HEIGHTS

City of Escondido, California



OPEN SPACE SUMMARY

34,844 S.F. 194,170 S.F.

REQUIRED: 300 S.F. PER UNIT X 510 UNITS TOTAL REQUIRED - 153,000 S.F

GENERAL NOTES

ENERAL NOTES

TOTAL PROJECT GROSS SITE AREA:

TOTAL PROJECT GROSS SITE AREA:

TOTAL PROJECT GROSS SITE AREA:

13.80 AGRES

13.97 AGRES

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PROJECT GROSS/NET DENSITY: 36.5

GENERAL DESIGN NOTES

- 1. ALL PRIVATE STREET DESIGNS, PRIVATE STREET LIGHTS, AND FIRE HYDRANTS SHALL CONFORM TO CITY OF ESCONDIDO DESIGN STANDARDS AND/OR AS APPROVED BY THE CITY
- ENGINEER. EASEMENTS SHALL BE PER CITY ENGINEER AND PUBLIC UTILITIES AND DISTRICTS.

- EASEMINS SHALL BE PER CITY DRINKER AND PUBLIC UTILITIES AND DISTRICTS.
 CONTORN INTERMALS: 24 MO 10 FOOTY, AND SUBJECT TO CHANGE IN FINAL DESIGN
 CONCISIENT IN THIS PLOTTY'S SESSIMATIAL CONFORMACE POLICY.
 APPROXIMATE RAN GRADING COUNTITIES I RAN CUT: 03.00 C.Y.,
 APPROXIMATE RAN GRADING COUNTITIES I RAN CUT: 03.00 C.Y.,
 APPROXIMATE RAN GRADING COUNTITIES I RAN CUT: 03.00 C.Y.,
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 APPROXIMATE RAN GRADING COUNTITIES AND COUNTING COUNTING
 CONCIDENTION DESIGNAL ENGINE AFTER THINKS SHINAGE AND BULKING INTO
 CONSIDERATION DESIGN ON INTERIOR OF PROVIDED IN SOILS BROKEN.
 SOILS REPORT PREPARED BY
 LIVE AND COUNTING COUN

OWNER'S CERTIFICATE

APPLICANT/SUBDIVIDER

THE PALOMAR HEIGHTS PROJECT 2235 ENCINITAS BOULEVARD ENCINITAS CA 92024

APPL I CANT DATE

ABBREVIATIONS

FIRE.....GAS AND ELECTRICITY...

HIGH SCHOOL DISTRICT..... ELEMENTARY SCHOOL DISTRICT....

LEGAL DESCRIPTION

EASEMENT NOTES

SEE SHEET NO. 12 FOR LEGAL DESCRIPTION

SEE SHEET NO. 12 FOR EASEMENT INFORMATION.

LEGEND

EXISTING PROJECT BOUNDARY PROPOSED PROJECT BOUNDARY

EXISTING SEWER MAIN WITH MANHOLE PROPOSED WATER MAIN (8" PVC UNLESS OTHERWISE NOTED)

PROPOSED LOT LINE PROPOSED EASEMENT LINE

PROPOSED FIRE HYDRANT

EXISTING FIRE HYDRANT

EXISTING CONTOURS

PROPOSED CONTOURS PROPOSED SLOPE (2: 1 MAX.)

CUT/FILL LINE

WATER METER

PROPOSED STORM DRAIN SYSTEM

PROPOSED CENTERLINE PVT. ST. ELEVATION

I IMITS OF PROPOSED GRADING/DAYLIGHT ------

BW	BUTTOM OF WALL	PL.	PRUPERIT LINE
TF	TOP OF FOOTING	GB	GRADE BREAK
FS	FINISH SURFACE	PI	POINT OF INTERSECTION (V.C.)
S	SEWER	P	PAD ELEVATION
W	WATER	SF	GROSS SQ. FT.
RW	RECLAIMED WATER	NSF	NET SQ. FT.
SD	STORM DRAIN	FP	FLOOD PLAIN
NTS	NOT TO SCALE	VC	VERTICAL CURVE
ELEV	ELEVATION	MH	MANHOLE
IE	INVERT ELEVATION	RCP	REINFORCED CONCRETE PIPE

PUBLIC UTILITIES AND DISTRICTS

PROJECT SUMMARY

LOTTING SUMMARY

PROJECT SUMMARY	SF	AC
TOTAL GROSS SITE AREA	601,247	13.80
DEDICATION	5,395	0.12
VACATION	4,813	0.11
ACQUISITION	7,767	0.18
TOTAL NET PROJECT AREA	600,665	13.97
PROPOSED PRIVATE DRIVE AISLES/PARKING	154,464	3.55
PROPOSED BUILDING COVERAGE	241,367	5.54
PROPOSED LANDSCAPING COVERAGE	204,834	4.88
TOTAL PROPOSED NET SITE AREA	600,665	13.97

SF	AC
46,150	1.06
129,549	2.97
87,213	2.00
39,130	0.90
137,980	3.17
73,176	1.68
60,517	1.39
34,771	0.80
608,486	13.97
	46,150 129,549 87,213 39,130 137,980 73,176 60,517 34,771

PARKING REQUIRED

TYPE/BEDROOM	RATIO	UNITS	REQ.
SENIOR			
1 BEDROOM	0.75	70	52.5
2 BEDROOM	1.5	20	30
SENIOR UN	T SUBTO	TAL	82.5
MULTI-DWELLIN	SUNITS		
1 BEDROOM	1.5	129	193.5
2 BEDROOM	1.75	173	302.75
3 BEDROOM	2	118	236
MULTI DWELL	ING SUBT	OTAL	732.25
Guest	0.25	420	105
TOTAL PA	RKING RE	ο.	919.75

PARKING PROVIDED

PARKING PROVIDED			
TYPE	SPACES		
GARAGE	659		
STANDARD	194		
PARALLEL	26		
COMPACT PARALLEL	1		
ACCESSIBLE	11		
TOTAL	891		

SOILS ENGINEER

GEOCON INCORPORATED 6960 FLANDERS DRIVE SAN DIEGO, CA 92121

TOPOGRAPHY

TOPO SOURCE: R.J. LUNG CO., INC. - FLOWN FEBRUARY 9, 2018 BENCHMARK: CITY OF ESCANDIDO NO. 357-A, CHISELED BOX CUT ON TOP OF CURB 20 FEET WEST OF GRAND AVE. ENTRANCE TO HOSPITAL PARKING STRUCTURE. EL 682.86' (NGVD '29)

SHEET INDEX

SHEET 1	TITLE SHEET
SHEET 2	EXISTING STREET SECTIONS
SHEET 3	PROPOSED STREET SECTIONS & DETAILS
SHEET 4	DETAILS
SHEET 5	SITE CROSS SECTIONS
SHEET 6	PROJECT DESIGN
SHEET 7	PROJECT DESIGN
SHEET 8	PROJECT DESIGN
SHEET 9	PROPOSED & EXISTING UTILITIES
SHEET 10	VALLEY PARKWAY INTERSECTION DETAIL
SHEET 11	EXISTING TOPO AND CONDITIONS MAP
SHEET 12	RECORD BOUNDARY & ENCUMBRANCES

ENGINEER

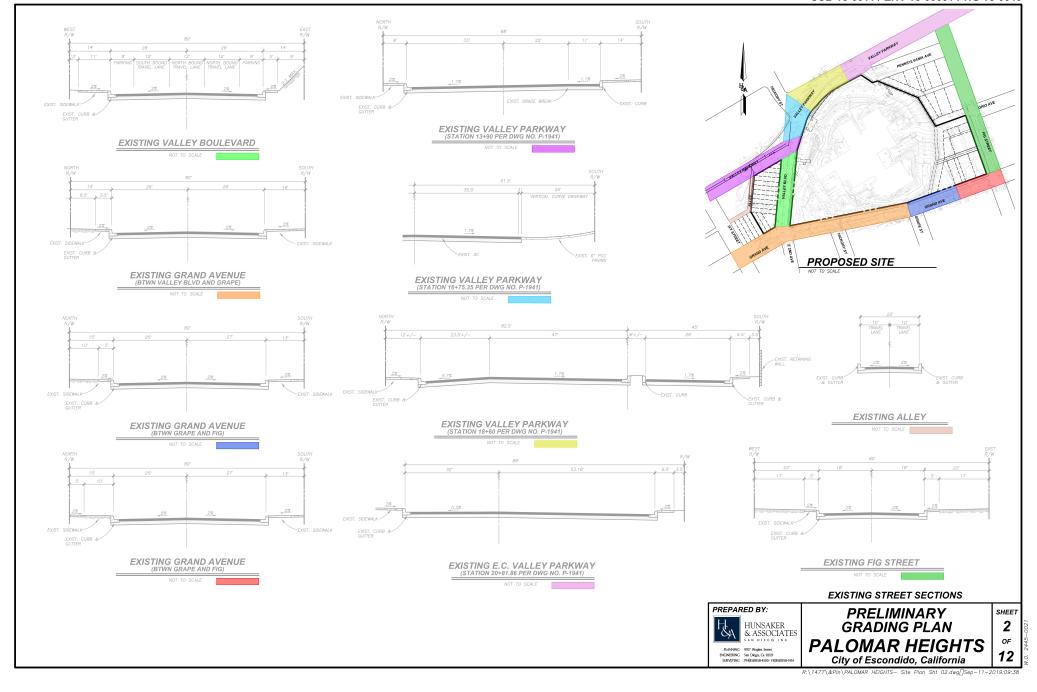


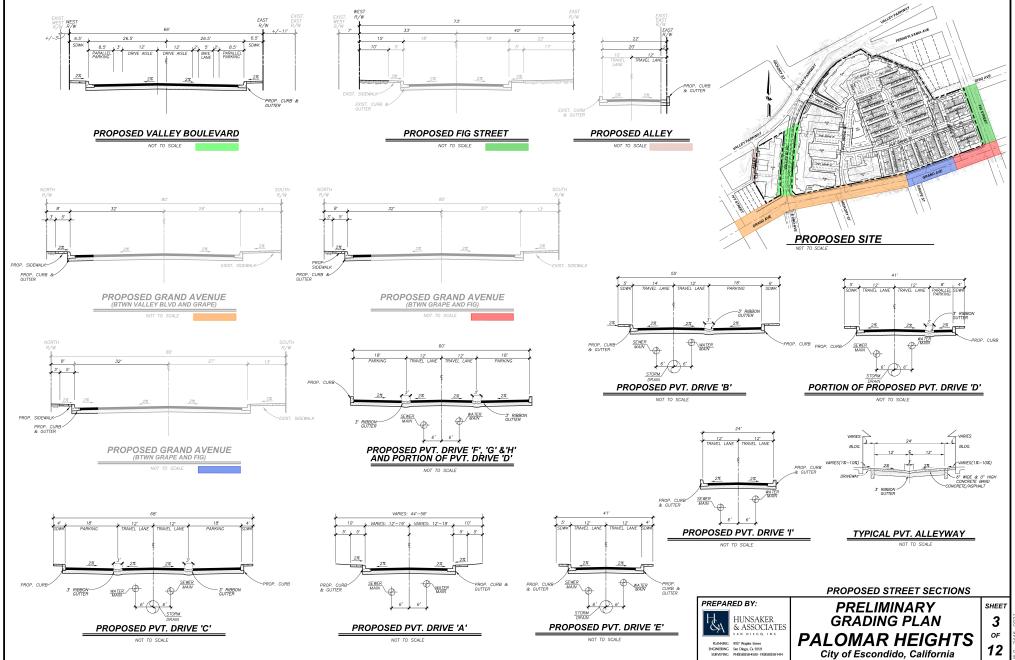
RAYMOND L. MARTIN R.C.E MY REGISTRATION EXPIRES ON 6/30/20

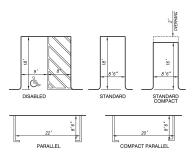
PREPAR	ED BY:	NO.	REVISIONS	DATE	ΒY
		1	1ST SUBMITTAL	06/25/19	H&A
PLANING STORE BIOMERYC Son Deg. C. 97(27) 90(7) Wiples Stores 180(NOMERYC Son Deg. C. 97(27) 90(97) PRISSISSISSISSISSISSISSISSISSISSISSISSISS	HINSAKER	2	2ND SUBMITTAL	09/11/19	H&A
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PRELIMINARY GRADING PLAN PALOMAR HEIGHTS City of Escondido, California

SHEET 1 OF 12





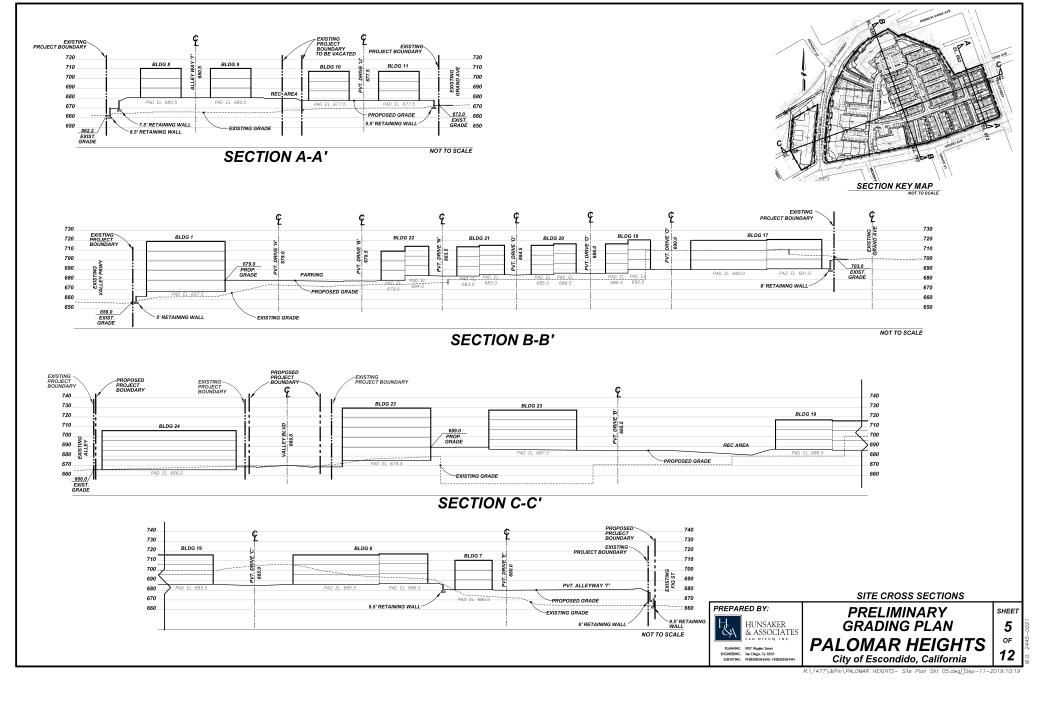


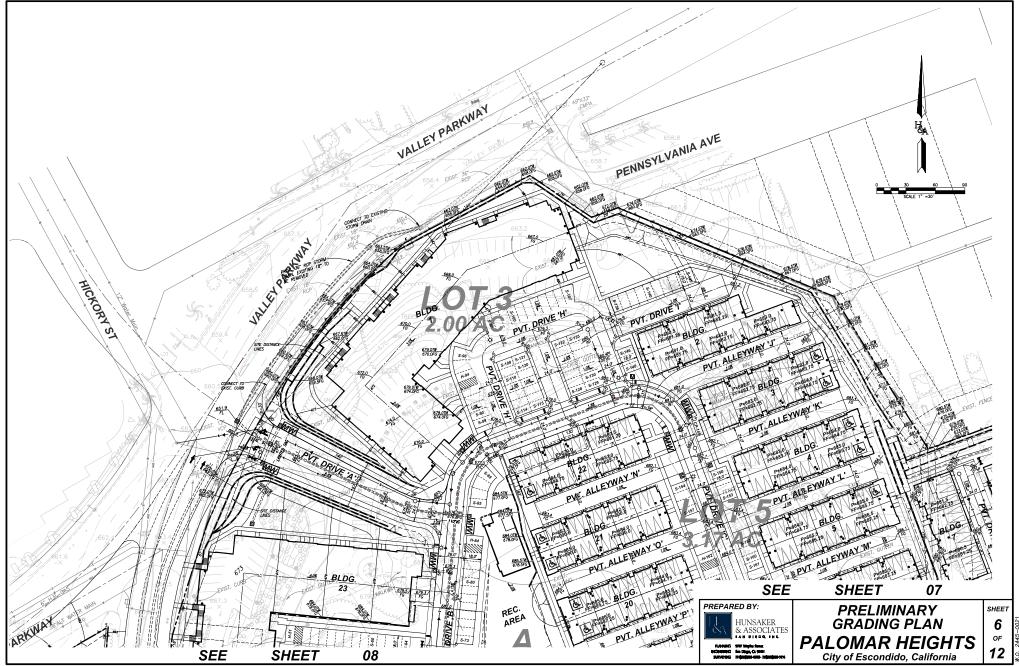
TYPICAL PARKING DETAIL

DETAILS

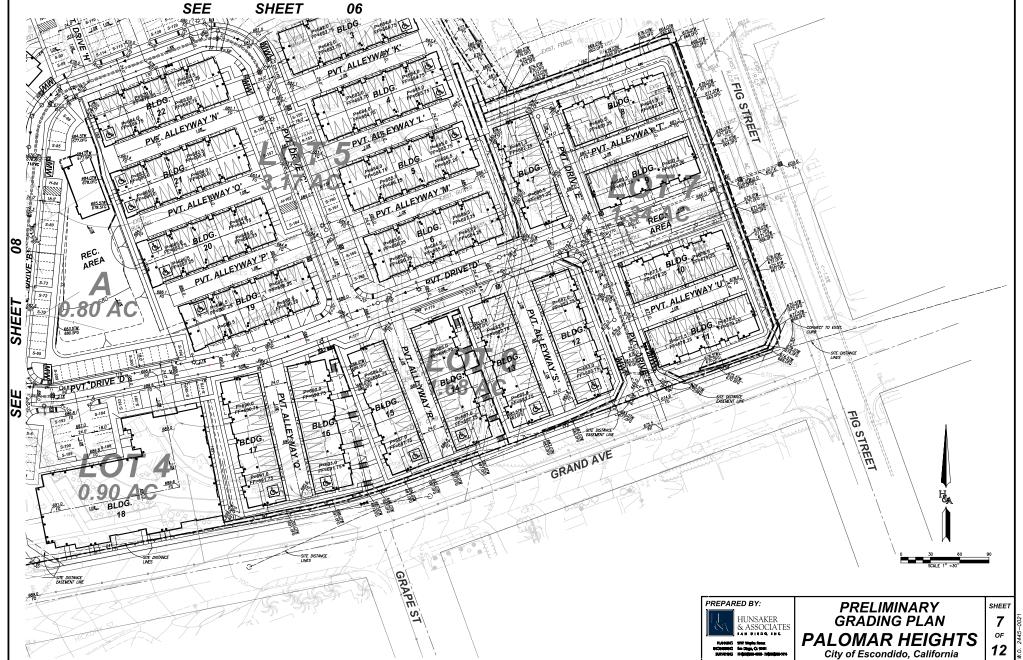


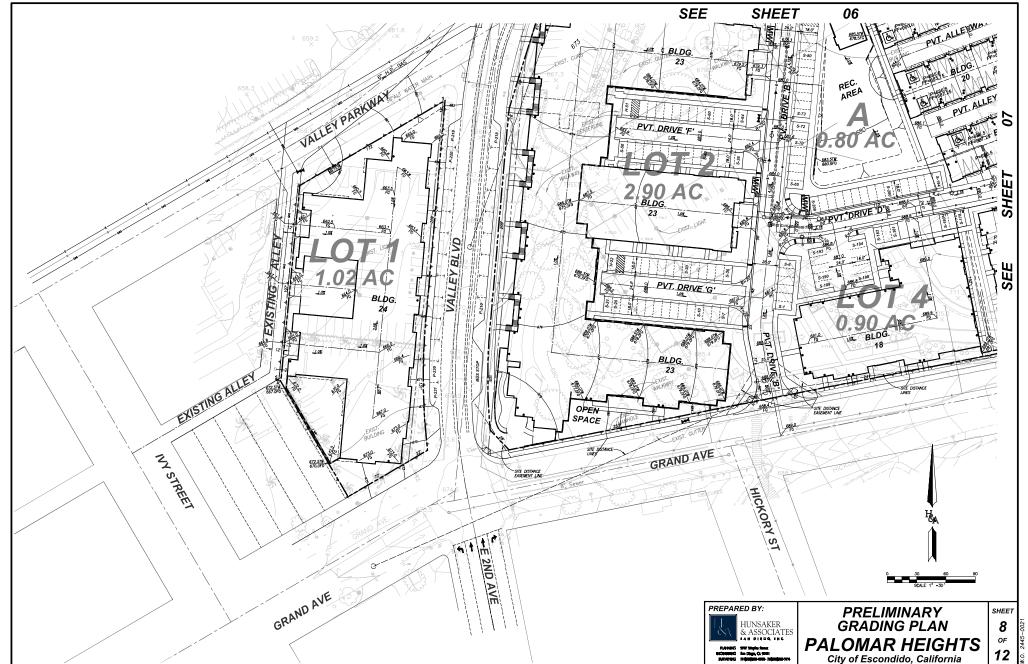
PRELIMINARY
GRADING PLAN
PALOMAR HEIGHTS
City of Escondido, California

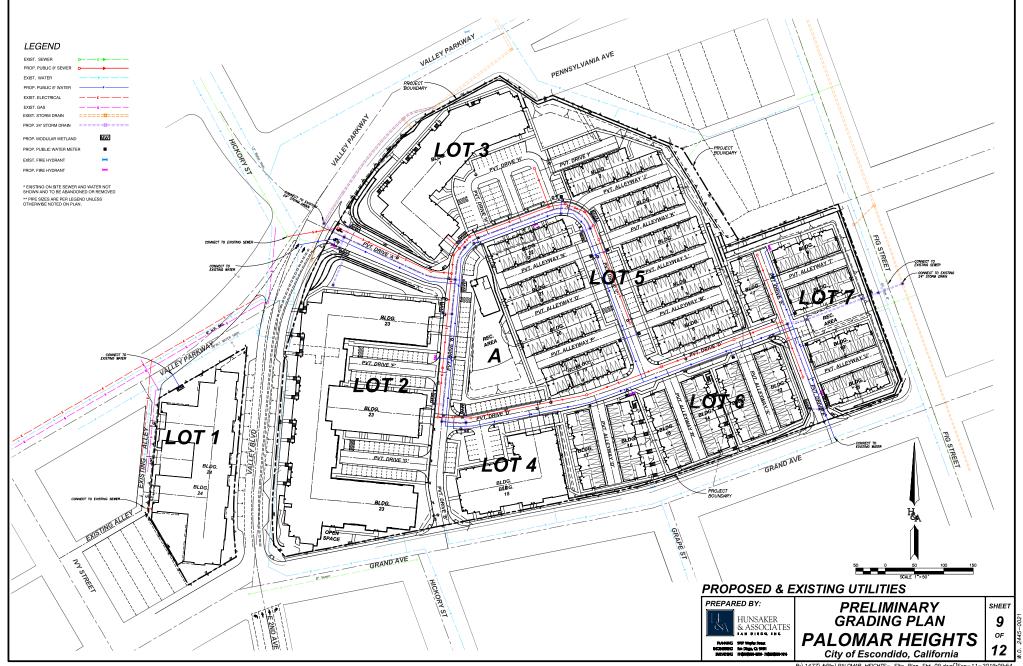


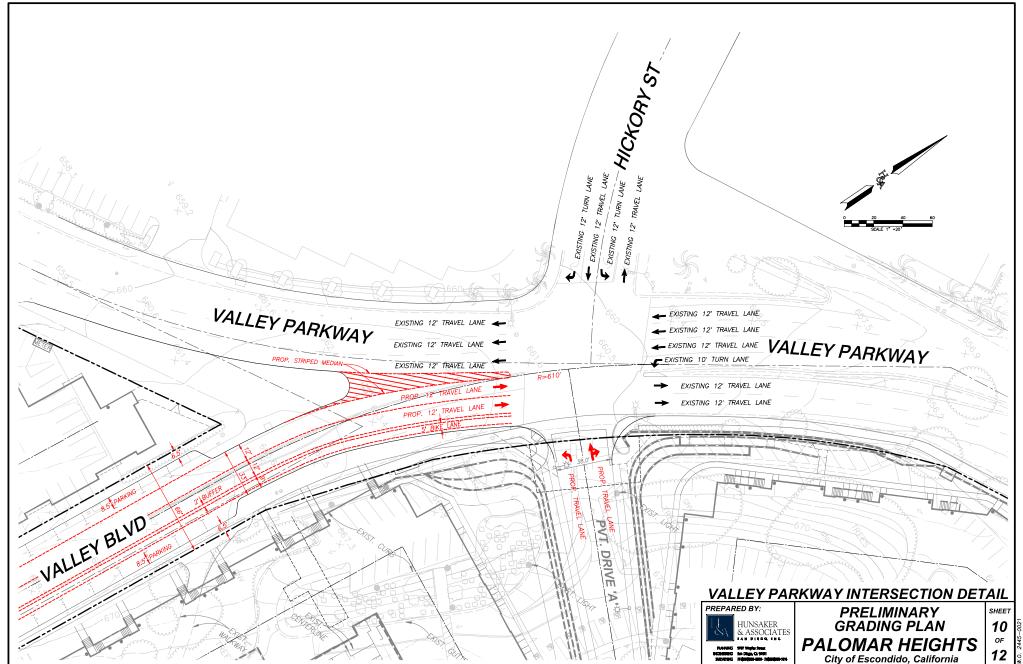


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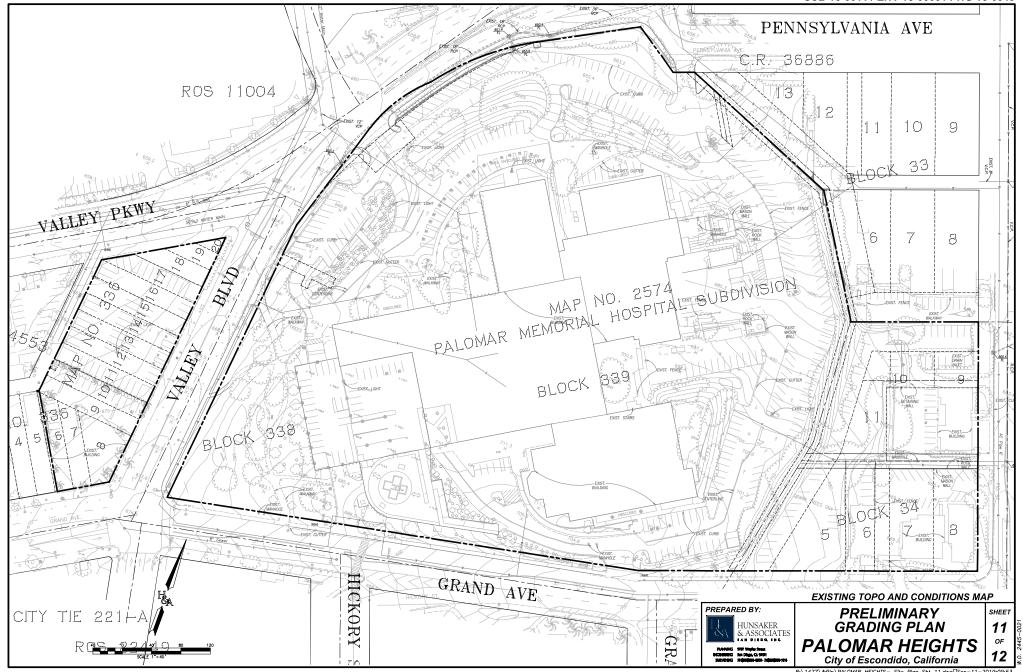








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TITLE REFERENCE:

THIS SURVEY IS BASED ON THE COMMITMENT FOR TITLE HISURANCE ISSUED BY FNTG-CHICAGO TITLE COMPANY AS ORDER NO. 00100370-998-SD1-RT4, EFFECTIVE DATE NOVEMBER 28, 2018

LEGAL DESCRIPTION:

THE LAND REFERRED TO HEREIN BELOW IS SITUATED IN THE CITY OF ESCONDIDO, WITHE COUNTY OF SAN DIEGO, STATE OF CALIFORNIA, AND IS DESCRIBED AS FOLLOWS:

PARCEL B: (APN: 229-450-05)

SOCIETA SE OF PALAMA BARROMA, HASSITY, SARROMON, IN THE SETT OF ESTIMATOR, CALIFF YE WAN ERRO, STATE OF AUTHORIA OCCORNO TO MAIN PROBLEM. SENT AN EXPLOSE OF THE COUNTY RECORDER OF SAN ERRO COUNTY, AME, 1, 1444 (OCCITIENT HITH THAT PORTION OF HICKORY SIREET ALMACENT TO SAUD BLOCK SAN OF THE SETT WANTED WAN CORNEL TO PHASE CESS, CESTERIDE OFFOR THE SOLUTION NO. 85—8200 HICKORDER MAINST AT, 1864 YE FEEL HE, 1865 - 251400 MAIN FEEL FEEL MAINST 27, 1864 AS THE NO. DOCK COUNTY, ANY, TO, 1864.

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PARCEL D: (APN: 230-163-01)

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PARCEL E: APN 230-163-03

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LOTS 18, 19 AND 20 IN BOOK 64 OF ESCONDIDO, CITY OF ESCONDIDO, COUNTY OF SAN DIEGO, STATE OF CAUFORMA, ACCORDING TO MAP THEREOF NO. 336, FILED IN THE OFFICE OF THE COUNTY RECORDER OF SAN DIEGO COUNTY, JULY 10, 1886

PARCEL H: APN 229-442-03 & 18

LOTS 8, 7, 8, 9, 10, 14 AND 15 EXCEPT THE WEST 3 FEET OF SAID LOT 8, W BLOCK 64 OF ESCONDIDO, W THE CITY OF ESCONDIDO, COUNTY OF SAV DEEGO, STATE OF CALFORMIA, ACCORDING TO MAP THEREOF NO. 336, FILED IN THE OFFICE OF THE COUNTY PECORDER OF SAN DECO COUNTY, JULY 10, 1889.

PARCEL I: APN 229-442-04

LOTS 11, 12 AND 13 IN BLOCK 64 OF THE CITY OF ESCONDIDO, COUNTY OF SAV DIEGO, STATE OF CALIFORNIA, ACCORDING TO MAP THEREOF NO. 336, FILED IN THE OFFICE OF THE COUNTY RECORDER OF SAN DIEGO COUNTY, JULY 10, 1869.

PARCEL J: APN 229-442-02

LOTS 16 & 17 IN BLOCK 64 IN THE CITY OF ESCONDIDO, IN THE COUNTY OF SAN DIEGO, STATE OF CALIFORNIA, ACCORDING TO MAP THEREOF NO. 336, ON FILE IN THE OFFICE OF THE COUNTY RECORDER OF SAN DIEGO COUNTY.

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AFFECTS: THE NORTHERLY 13.00 FEET OF THE WESTERLY 8.00 FEET OF SAID LOT 10
AFFECTS: LOTS 7 - 10 & 14, 15, PORTION 6

THE FOLLOWING MATTERS AFFECT PARCEL I:

THE FOLLOWING EASEMENTS RESERVED BY THE ESCONDIDO LAND & TOWN COMPANY (SUCCEEDED BY THE CITY OF RESCONDIDO) FOR THE PIPELINES AS FOLLOWS: IN BOOK 67, PAGE 35 OF DEEDS, AFFECTS LOT 51; N BOOK 67, PAGE 37 OF DEEDS, AFFECTS LOTS 11 & 12;

THE FOLLOWING MATTERS AFFECT PARCEL J:

THE FOLLOWING EASEMEN'S RESERVED BY THE ESCOULDED LAND & TOWN COMPANY (SUCCEEDED BY THE CITY OF ESCONDIDO) FOR THE PHYLINES AS FOLLOWS: AN BOOK 118, PAGE 380 OF DEEDS, ATTECTS LOT 17.

N BOOK 118, PAGE 320, OF DEEDS, ATTECTS LOT 17.

THE FOLLOWING MATTERS AFFECT PARCEL H:

AN EASEMENT FOR THE PURPOSE SHOWN BELOW AND RIGHTS INCIDENTAL THERETO AS SET FORTH IN A DOCUMENT. AS SET FORTH IN A DOCUMENT.
GRANTED TO:. SAN DIEGO GAS AND ELECTRIC COMPANY
PURPOSE: PUBLIC UTILITIES, INGRESS, EGRESS
RECORDED:. APRIL 28, 1992 AS INSTRUMENT NO. 1992—0248179, OF OFFICIAL RECORDS
AFFECTS. THE NORTHERLY 13.00 FEET OF THE WESTERLY 8.00 FEET OF SAID LOT 10

VALLEY PARKWAY PENNSYLVANIA AVE 9 B=195__ R=367 OHIO AVE Moderate Hage PCL A APN 229-450 £ 1 PALOMAR MEMORIAL HOSPITAL SUBDIVISION PORTION OF ALLEY TO BE VACATED AND ABANDONED MAP NO 2574 VALLEY PARKWAY WOOKS WE 482.18 APN 239-183-04 BLVD 338 PCL B - GRAND AVE

BOUNDARY AND ENCUMBRANCES



PRELIMINARY **GRADING PLAN PALOMAR HEIGHTS** City of Escondido, California

SHEET 12 OF 12

PRIORITY DEVELOPMENT PROJECT (PDP) SWQMP

ATTACHMENT 6

Copy of Hydromodification Management Applicability Evaluation:

Escondido Creek

Date: January 8th, 2018

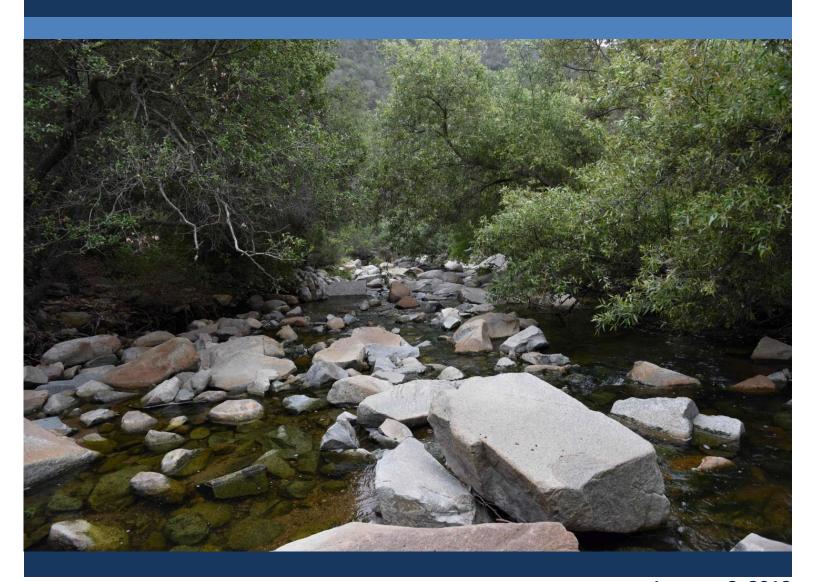
Prepared by: Geosyntec, Chang Consultants,

Moffat & Nichol

Template Date: October 2016 Preparation Date: 6/18/2019

PDP SWQMP - Attachments

Hydromodification Management Applicability Evaluation: Escondido Creek



January 8, 2018

Prepared for:



Prepared by:







TABLE OF CONTENTS

EXECUTIV	VE SUMMARY	III
1. INTRO	ODUCTION	1
	BACKGROUND AND PURPOSE	
	SCOPE AND OVERVIEW OF TECHNICAL ANALYSES	
	REPORT ORGANIZATION	•
1.4. TE	FERMS OF REFERENCE	6
2. WATE	ERSHED CHARACTERIZATION	7
2.1. CA	Carlsbad WMA	7
2.2. Es	ESCONDIDO CREEK HYDROLOGIC AREA	• • • • • • • • • • • • • • • • • • • •
2.2.1.	Dominant Hydrologic Processes	
2.2.2.	Stream Characterization	
2.2.3.	Land Uses	
2.2.4.	Potential Critical Coarse Sediment Yield Areas	_
2.2.5.	Physical Structures	17
3. HYDR	ROMODIFICATION MANAGEMENT APPLICABILITY FOR ESCO	NDIDO
CRFFK		19
	EVALUATION FOR FLOW CONTROL	
3.1.1.	Specific Stream Power	
3.1.2.		
	EVALUATION FOR COARSE SEDIMENT SUPPLY	
3.3. Ev	EVALUATION OF SAN ELIJO LAGOON	24
4. CONC	CLUSIONS	26
4.1. Es	Escondido Creek	26
4.1.1.	Limitations	27
4.2. FA	FACTORS OF SAFETY	27
5. REFER	RENCES	29
	ATTACHMENTS	
ATTACHME	ENT A HYDROMODIFICATION SCREENING FOR THE ESCONDIDO CR	EEK
ATTACHME	ENT B SEDIMENT TRANSPORT ANALYSIS FOR THE ESCONDIDO CRE	EK
ATTACHME	ENT C EROSION POTENTIAL	
ATTACHME		

Executive Summary

The San Diego Regional Water Quality Control Board (SDRWQCB) reissued a municipal storm water National Pollutant Discharge Elimination System Permit in 2013 (Municipal Separate Storm Sewer Systems [MS4] Permit) that covers the San Diego region, including the Carlsbad watershed management area (WMA). The Regional MS4 Permit requires each WMA within the San Diego Region to develop a Water Quality Improvement Plan (WQIP), which provides Copermittees an option to perform a regional Watershed Management Area Analysis (WMAA) to develop watershed-specific requirements for structural best management practice (BMP) implementation for Priority Development Projects (PDPs). Within this provision is the opportunity to identify areas within the WMA where it is appropriate to allow for exemptions from hydromodification management requirements in addition to those already allowed for PDPs by the Regional MS4 Permit. A regional WMAA was prepared in 2015 on behalf of the San Diego Copermittees that included technical evaluations of the five major river reaches included in the Final Hydromodification Management Plan (Brown and Caldwell, 2011). These evaluations were conducted to determine if it is still appropriate to carry forward hydromodification management exemptions in the respective WQIPs (San Diego County Copermittees, 2015). Escondido Creek (Creek) was not analyzed in the regional WMAA, so a separate effort was undertaken and is documented in this report (the Study).

Using a geographic information system (GIS), watershed data compiled and developed in the regional WMAA, and additional evaluation metrics and analyses, the main stem of Escondido Creek was evaluated to determine if hydromodification management controls are applicable for PDPs discharging directly to the creek. The Study extents of Escondido Creek were evaluated from the non-hardened, natural Creek main stem beginning from the end of the concrete channel at the western Escondido City limit (i.e., Harmony Grove Road; upper study limit) to the confluence with the San Elijo Lagoon (lower study limit). The evaluations, using regional WMAA data, consisted of Southern California Coastal Water Research Project (SCCWRP) field assessments, sediment transport analyses, flow control and coarse sediment supply analyses, and an evaluation of impacts to the San Elijo Lagoon. Theses analyses comprise the Study described in this report and in the attachments.

Based on the findings of the Study, requiring hydromodification controls for PDPs discharging directly to the main stem of Escondido Creek does not confer a protective benefit to the Creek because it does not appear susceptible to hydromodification, i.e., the Creek is not anticipated to experience accelerated, unnatural erosion from direct stormwater discharges from PDPs. The exemption also does not interfere with the planned restoration of the San Elijo Lagoon. Therefore, it is recommended that PDPs directly discharging stormwater runoff to the study reach be exempt from hydromodification management requirements. The study reach recommended for exemption is from the end of the concrete channel approximately coincident with the Harmony Grove Road crossing to the junction with San Elijo Lagoon and includes the concrete-lined portion of Escondido Creek running through the City of Escondido as it forms a contiguous non-susceptible conveyance to the watershed outlet.

PRIORITY DEVELOPMENT PROJECT (PDP) SWQMP

ATTACHMENT 6

Maintenance Guidelines for Modular Wetland System - Linear

Prepared by: Bioclean

Template Date: October 2016 Preparation Date: 6/18/2019 PDP SWQMP - Attachments

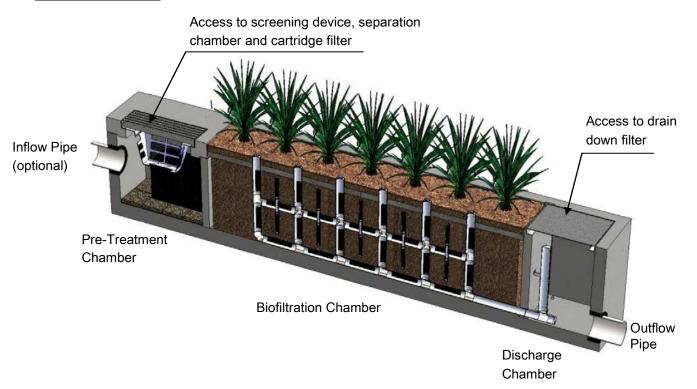


Maintenance Guidelines for Modular Wetland System - Linear

Maintenance Summary

- Remove Trash from Screening Device average maintenance interval is 6 to 12 months.
 - (5 minute average service time).
- Remove Sediment from Separation Chamber average maintenance interval is 12 to 24 months.
 - (10 minute average service time).
- Replace Cartridge Filter Media average maintenance interval 12 to 24 months.
 - (10-15 minute per cartridge average service time).
- o Replace Drain Down Filter Media average maintenance interval is 12 to 24 months.
 - (5 minute average service time).
- Trim Vegetation average maintenance interval is 6 to 12 months.
 - (Service time varies).

System Diagram





Maintenance Procedures

Screening Device

- Remove grate or manhole cover to gain access to the screening device in the Pre-Treatment Chamber. Vault type units do not have screening device. Maintenance can be performed without entry.
- 2. Remove all pollutants collected by the screening device. Removal can be done manually or with the use of a vacuum truck. The hose of the vacuum truck will not damage the screening device.
- Screening device can easily be removed from the Pre-Treatment Chamber to gain access to separation chamber and media filters below. Replace grate or manhole cover when completed.

Separation Chamber

- 1. Perform maintenance procedures of screening device listed above before maintaining the separation chamber.
- 2. With a pressure washer spray down pollutants accumulated on walls and cartridge filters.
- 3. Vacuum out Separation Chamber and remove all accumulated pollutants. Replace screening device, grate or manhole cover when completed.

Cartridge Filters

- 1. Perform maintenance procedures on screening device and separation chamber before maintaining cartridge filters.
- 2. Enter separation chamber.
- 3. Unscrew the two bolts holding the lid on each cartridge filter and remove lid.
- 4. Remove each of 4 to 8 media cages holding the media in place.
- 5. Spray down the cartridge filter to remove any accumulated pollutants.
- 6. Vacuum out old media and accumulated pollutants.
- 7. Reinstall media cages and fill with new media from manufacturer or outside supplier. Manufacturer will provide specification of media and sources to purchase.
- 8. Replace the lid and tighten down bolts. Replace screening device, grate or manhole cover when completed.

Drain Down Filter

- 1. Remove hatch or manhole cover over discharge chamber and enter chamber.
- 2. Unlock and lift drain down filter housing and remove old media block. Replace with new media block. Lower drain down filter housing and lock into place.
- 3. Exit chamber and replace hatch or manhole cover.



Maintenance Notes

- Following maintenance and/or inspection, it is recommended the maintenance operator prepare a maintenance/inspection record. The record should include any maintenance activities performed, amount and description of debris collected, and condition of the system and its various filter mechanisms.
- 2. The owner should keep maintenance/inspection record(s) for a minimum of five years from the date of maintenance. These records should be made available to the governing municipality for inspection upon request at any time.
- 3. Transport all debris, trash, organics and sediments to approved facility for disposal in accordance with local and state requirements.
- 4. Entry into chambers may require confined space training based on state and local regulations.
- 5. No fertilizer shall be used in the Biofiltration Chamber.
- 6. Irrigation should be provided as recommended by manufacturer and/or landscape architect. Amount of irrigation required is dependent on plant species. Some plants may require irrigation.



Maintenance Procedure Illustration

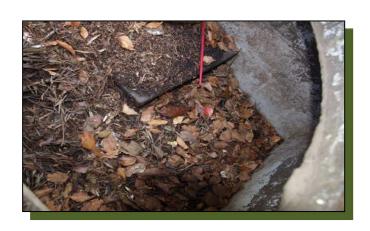
Screening Device

The screening device is located directly under the manhole or grate over the Pre-Treatment Chamber. It's mounted directly underneath for easy access and cleaning. Device can be cleaned by hand or with a vacuum truck.



Separation Chamber

The separation chamber is located directly beneath the screening device. It can be quickly cleaned using a vacuum truck or by hand. A pressure washer is useful to assist in the cleaning process.









Cartridge Filters

The cartridge filters are located in the Pre-Treatment chamber connected to the wall adjacent to the biofiltration chamber. The cartridges have removable tops to access the individual media filters. Once the cartridge is open media can be easily removed and replaced by hand or a vacuum truck.







Drain Down Filter

The drain down filter is located in the Discharge Chamber. The drain filter unlocks from the wall mount and hinges up. Remove filter block and replace with new block.





Trim Vegetation

Vegetation should be maintained in the same manner as surrounding vegetation and trimmed as needed. No fertilizer shall be used on the plants. Irrigation per the recommendation of the manufacturer and or landscape architect. Different types of vegetation requires different amounts of irrigation.











Inspection Form



Bio Clean

P. 855-566-3938

F. 760-433-3176

E. Info@BioCleanEnvironmental.com

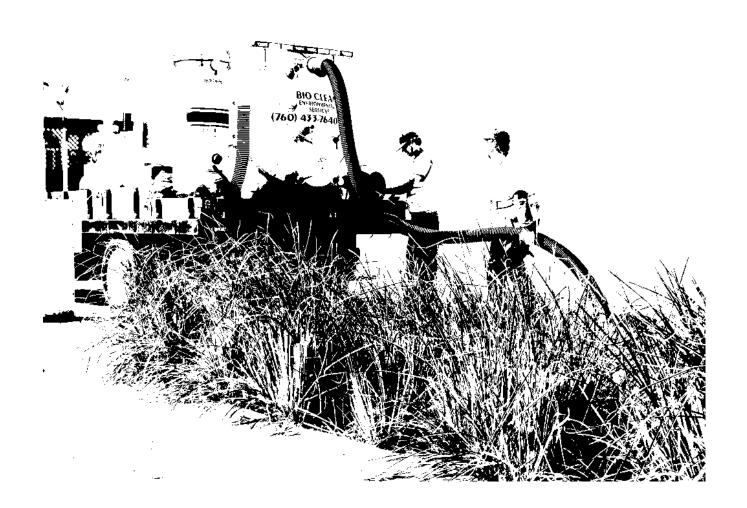


Inspection Report Modular Wetlands System

Project Name										For Office	Use Only	1
Project Address						4-14-2		(7in Onda)		(Daviewed D	Α)	
Owner / Management Company						(city))	(Zip Code)		(Reviewed By	')	
Contact Phone ()								(Date) Office person	nnel to com the left.	plete section to		
Inspector Name				Dat	te	/	/		_ Tim	e		AM / PM
Type of Inspection Routine Follow Up Complaint Storm Storm Event in Last 72-hours? No Yes											es	
Weather Condition Additional Notes												
			In	spection	Check	list						
Modular Wetland System T	ype (Curb,	Grate or U		•			ze (22	2', 14' or	etc.):			
Structural Integrity:								Yes	No	C	ommen	ts
Damage to pre-treatment access pressure?	cover (manh	ole cover/gra	ate) or cannot I	be opened us	sing norma	l lifting						
Damage to discharge chamber a pressure?	ccess cover (manhole cov	/er/grate) or ca	annot be oper	ned using r	normal lif	ting					
Does the MWS unit show signs of	of structural of	leterioration ((cracks in the v	wall, damage	to frame)?	,						
Is the inlet/outlet pipe or drain do	wn pipe dam	aged or othe	rwise not funct	tioning proper	ly?							
Working Condition:												
Is there evidence of illicit dischargunit?	ge or excessi	ve oil, grease	e, or other auto	omobile fluids	entering a	and clogg	ging the					
Is there standing water in inappro	priate areas	after a dry pe	eriod?									
Is the filter insert (if applicable) at	t capacity and	d/or is there a	an accumulatio	on of debris/tra	ash on the	shelf sy	stem?					
Does the depth of sediment/trash specify which one in the commer							If yes,					Depth:
Does the cartridge filter media ne	ed replacem	ent in pre-tre	atment chamb	er and/or disc	charge cha	mber?				Chamber:		
Any signs of improper functioning	g in the discha	arge chambe	r? Note issue:	s in comment	s section.							
Other Inspection Items:												
Is there an accumulation of sedin	nent/trash/de	bris in the we	etland media (i	f applicable)?	,							
Is it evident that the plants are all	ive and healtl	ny (if applicat	ole)? Please no	ote Plant Info	rmation be	low.						
Is there a septic or foul odor com	ing from insid	le the system	1?									
Waste:	Yes	No		Reco	mmende	ed Mair	ntenai	nce		Plant	Inform	ation
Sediment / Silt / Clay			N	No Cleaning N	leeded					Damage to F	Plants	
Trash / Bags / Bottles			s	Schedule Mair	ntenance a	s Planne	ed			Plant Replac	ement	
Green Waste / Leaves / Foliage			N	Needs Immed	iate Mainte	enance				Plant Trimmi	ng	
Additional Notes:												



Maintenance Report



Bio Clean

P. 855-566-3938

F. 760-433-3176

E. Info@BioCleanEnvironmental.com



Cleaning and Maintenance Report Modular Wetlands System

Project Name							For 0	For Office Use Only	
Project Address (city) (Zip Code)								ewed By)	
Owner / Management Company									
Contact				Phone ()	_	(Date) Office	e personnel to complete section to the left.	
Inspector Name				Date	/		Time	AM / PM	
Type of Inspection Routine Follow Up Complaint				☐ Storm		Storm Event in	Last 72-hours?	□ No □ Yes	
Weather	Condition			Additiona	al Notes				
						<u> </u>	1		
Site Map #	GPS Coordinates of Insert	Manufacturer / Description / Sizing	Trash Accumulation	Foliage Accumulation	Sediment Accumulation	Total Debris Accumulation	Condition of Media 25/50/75/100 (will be changed @ 75%)	a Operational Per Manufactures' Specifications (If not, why?)	
	Lat:	MWS Catch Basins							
		MWS Sedimentation Basin							
		Media Filter Condition							
		- Plant Condition							
		Drain Down Media Condition							
		Discharge Chamber Condition							
		Drain Down Pipe Condition							
		Inlet and Outlet Pipe Condition							
Commen	ts:								

PRIORITY DEVELOPMENT PROJECT (PDP) SWQMP

ATTACHMENT 7

Third Party Certification for Modular Wetland Linear

Prepared by:

Washington State Department of Ecology

Template Date: October 2016 Preparation Date: 6/18/2019

PDP SWQMP - Attachments



July 2017

GENERAL USE LEVEL DESIGNATION FOR BASIC, ENHANCED, AND PHOSPHORUS TREATMENT

For the

MWS-Linear Modular Wetland

Ecology's Decision:

Based on Modular Wetland Systems, Inc. application submissions, including the Technical Evaluation Report, dated April 1, 2014, Ecology hereby issues the following use level designation:

- 1. General use level designation (GULD) for the MWS-Linear Modular Wetland Stormwater Treatment System for Basic treatment
 - Sized at a hydraulic loading rate of 1 gallon per minute (gpm) per square foot (sq ft) of wetland cell surface area. For moderate pollutant loading rates (low to medium density residential basins), size the Prefilters at 3.0 gpm/sq ft of cartridge surface area. For high loading rates (commercial and industrial basins), size the Prefilters at 2.1 gpm/sq ft of cartridge surface area.
- 2. General use level designation (GULD) for the MWS-Linear Modular Wetland Stormwater Treatment System for Phosphorus treatment
 - Sized at a hydraulic loading rate of 1 gallon per minute (gpm) per square foot (sq ft) of wetland cell surface area. For moderate pollutant loading rates (low to medium density residential basins), size the Prefilters at 3.0 gpm/sq ft of cartridge surface area. For high loading rates (commercial and industrial basins), size the Prefilters at 2.1 gpm/sq ft of cartridge surface area.
- 3. General use level designation (GULD) for the MWS-Linear Modular Wetland Stormwater Treatment System for Enhanced treatment
 - Sized at a hydraulic loading rate of 1 gallon per minute (gpm) per square foot (sq ft) of wetland cell surface area. For moderate pollutant loading rates (low to medium density residential basins), size the Prefilters at 3.0 gpm/sq ft of cartridge surface area. For high loading rates (commercial and industrial basins), size the Prefilters at 2.1 gpm/sq ft of cartridge surface area.

- 4. Ecology approves the MWS Linear Modular Wetland Stormwater Treatment System units for Basic, Phosphorus, and Enhanced treatment at the hydraulic loading rate listed above. Designers shall calculate the water quality design flow rates using the following procedures:
 - Western Washington: For treatment installed upstream of detention or retention, the water quality design flow rate is the peak 15-minute flow rate as calculated using the latest version of the Western Washington Hydrology Model or other Ecology-approved continuous runoff model.
 - Eastern Washington: For treatment installed upstream of detention or retention, the water quality design flow rate is the peak 15-minute flow rate as calculated using one of the three methods described in Chapter 2.2.5 of the Stormwater Management Manual for Eastern Washington (SWMMEW) or local manual.
 - Entire State: For treatment installed downstream of detention, the water quality design flow rate is the full 2-year release rate of the detention facility.
- 5. These use level designations have no expiration date but may be revoked or amended by Ecology, and are subject to the conditions specified below.

Ecology's Conditions of Use:

Applicants shall comply with the following conditions:

- 1. Design, assemble, install, operate, and maintain the MWS Linear Modular Wetland Stormwater Treatment System units, in accordance with Modular Wetland Systems, Inc. applicable manuals and documents and the Ecology Decision.
- Each site plan must undergo Modular Wetland Systems, Inc. review and approval before
 site installation. This ensures that site grading and slope are appropriate for use of a MWS

 Linear Modular Wetland Stormwater Treatment System unit.
- 3. MWS Linear Modular Wetland Stormwater Treatment System media shall conform to the specifications submitted to, and approved by, Ecology.
- 4. The applicant tested the MWS Linear Modular Wetland Stormwater Treatment System with an external bypass weir. This weir limited the depth of water flowing through the media, and therefore the active treatment area, to below the root zone of the plants. This GULD applies to MWS Linear Modular Wetland Stormwater Treatment Systems whether plants are included in the final product or not.
- 5. Maintenance: The required maintenance interval for stormwater treatment devices is often dependent upon the degree of pollutant loading from a particular drainage basin. Therefore, Ecology does not endorse or recommend a "one size fits all" maintenance cycle for a particular model/size of manufactured filter treatment device.
 - Typically, Modular Wetland Systems, Inc. designs MWS Linear Modular Wetland systems for a target prefilter media life of 6 to 12 months.
 - Indications of the need for maintenance include effluent flow decreasing to below the design flow rate or decrease in treatment below required levels.
 - Owners/operators must inspect MWS Linear Modular Wetland systems for a minimum of twelve months from the start of post-construction operation to determine site-specific

maintenance schedules and requirements. You must conduct inspections monthly during the wet season, and every other month during the dry season. (According to the SWMMWW, the wet season in western Washington is October 1 to April 30. According to SWMMEW, the wet season in eastern Washington is October 1 to June 30). After the first year of operation, owners/operators must conduct inspections based on the findings during the first year of inspections.

- Conduct inspections by qualified personnel, follow manufacturer's guidelines, and use methods capable of determining either a decrease in treated effluent flowrate and/or a decrease in pollutant removal ability.
- When inspections are performed, the following findings typically serve as maintenance triggers:
 - Standing water remains in the vault between rain events, or
 - Bypass occurs during storms smaller than the design storm.
 - If excessive floatables (trash and debris) are present (but no standing water or excessive sedimentation), perform a minor maintenance consisting of gross solids removal, not prefilter media replacement.
 - Additional data collection will be used to create a correlation between pretreatment chamber sediment depth and pre-filter clogging (see *Issues to be Addressed by the Company* section below)
- 6. Discharges from the MWS Linear Modular Wetland Stormwater Treatment System units shall not cause or contribute to water quality standards violations in receiving waters.

Applicant: Modular Wetland Systems, Inc.

Applicant's Address: PO. Box 869

Oceanside, CA 92054

Application Documents:

- Original Application for Conditional Use Level Designation, Modular Wetland System, Linear Stormwater Filtration System Modular Wetland Systems, Inc., January 2011
- *Quality Assurance Project Plan*: Modular Wetland system Linear Treatment System performance Monitoring Project, draft, January 2011.
- Revised Application for Conditional Use Level Designation, Modular Wetland System, Linear Stormwater Filtration System Modular Wetland Systems, Inc., May 2011
- Memorandum: Modular Wetland System-Linear GULD Application Supplementary Data, April 2014
- Technical Evaluation Report: Modular Wetland System Stormwater Treatment System Performance Monitoring, April 2014.

Applicant's Use Level Request:

General use level designation as a Basic, Enhanced, and Phosphorus treatment device in accordance with Ecology's Guidance for Evaluating Emerging Stormwater Treatment Technologies Technology Assessment Protocol – Ecology (TAPE) January 2011 Revision.

Applicant's Performance Claims:

- The MWS Linear Modular wetland is capable of removing a minimum of 80-percent of TSS from stormwater with influent concentrations between 100 and 200 mg/l.
- The MWS Linear Modular wetland is capable of removing a minimum of 50-percent of Total Phosphorus from stormwater with influent concentrations between 0.1 and 0.5 mg/l.
- The MWS Linear Modular wetland is capable of removing a minimum of 30-percent of dissolved Copper from stormwater with influent concentrations between 0.005 and 0.020 mg/l.
- The MWS Linear Modular wetland is capable of removing a minimum of 60-percent of dissolved Zinc from stormwater with influent concentrations between 0.02 and 0.30 mg/l.

Ecology Recommendations:

 Modular Wetland Systems, Inc. has shown Ecology, through laboratory and fieldtesting, that the MWS - Linear Modular Wetland Stormwater Treatment System filter system is capable of attaining Ecology's Basic, Total phosphorus, and Enhanced treatment goals.

Findings of Fact:

Laboratory Testing

The MWS-Linear Modular wetland has the:

- Capability to remove 99 percent of total suspended solids (using Sil-Co-Sil 106) in a quarter-scale model with influent concentrations of 270 mg/L.
- Capability to remove 91 percent of total suspended solids (using Sil-Co-Sil 106) in laboratory conditions with influent concentrations of 84.6 mg/L at a flow rate of 3.0 gpm per square foot of media.
- Capability to remove 93 percent of dissolved Copper in a quarter-scale model with influent concentrations of 0.757 mg/L.
- Capability to remove 79 percent of dissolved Copper in laboratory conditions with influent concentrations of 0.567 mg/L at a flow rate of 3.0 gpm per square foot of media.
- Capability to remove 80.5-percent of dissolved Zinc in a quarter-scale model with influent concentrations of 0.95 mg/L at a flow rate of 3.0 gpm per square foot of media.
- Capability to remove 78-percent of dissolved Zinc in laboratory conditions with influent concentrations of 0.75 mg/L at a flow rate of 3.0 gpm per square foot of media.

Field Testing

- Modular Wetland Systems, Inc. conducted monitoring of an MWS-Linear (Model # MWS-L-4-13) from April 2012 through May 2013, at a transportation maintenance facility in Portland, Oregon. The manufacturer collected flow-weighted composite samples of the system's influent and effluent during 28 separate storm events. The system treated approximately 75 percent of the runoff from 53.5 inches of rainfall during the monitoring period. The applicant sized the system at 1 gpm/sq ft. (wetland media) and 3gpm/sq ft. (prefilter).
- Influent TSS concentrations for qualifying sampled storm events ranged from 20 to 339 mg/L. Average TSS removal for influent concentrations greater than 100 mg/L (n=7) averaged 85 percent. For influent concentrations in the range of 20-100 mg/L (n=18), the upper 95 percent confidence interval about the mean effluent concentration was 12.8 mg/L.
- Total phosphorus removal for 17 events with influent TP concentrations in the range of 0.1 to 0.5 mg/L averaged 65 percent. A bootstrap estimate of the lower 95 percent confidence limit (LCL95) of the mean total phosphorus reduction was 58 percent.
- The lower 95 percent confidence limit of the mean percent removal was 60.5 percent for dissolved zinc for influent concentrations in the range of 0.02 to 0.3 mg/L (n=11). The lower 95 percent confidence limit of the mean percent removal was 32.5 percent for dissolved copper for influent concentrations in the range of 0.005 to 0.02 mg/L (n=14) at flow rates up to 28 gpm (design flow rate 41 gpm). Laboratory test data augmented the data set, showing dissolved copper removal at the design flow rate of 41 gpm (93 percent reduction in influent dissolved copper of 0.757 mg/L).

Issues to be addressed by the Company:

- 1. Modular Wetland Systems, Inc. should collect maintenance and inspection data for the first year on all installations in the Northwest in order to assess standard maintenance requirements for various land uses in the region. Modular Wetland Systems, Inc. should use these data to establish required maintenance cycles.
- 2. Modular Wetland Systems, Inc. should collect pre-treatment chamber sediment depth data for the first year of operation for all installations in the Northwest. Modular Wetland Systems, Inc. will use these data to create a correlation between sediment depth and pre-filter clogging.

Technology Description:

Download at http://www.modularwetlands.com/

Contact Information:

Applicant: Zach Kent

BioClean A Forterra Company.

398 Vi9a El Centro Oceanside, CA 92058 <u>zach.kent@forterrabp.com</u> Applicant website: http://www.modularwetlands.com/

Ecology web link: http://www.ecy.wa.gov/programs/wg/stormwater/newtech/index.html

Ecology: Douglas C. Howie, P.E.

Department of Ecology Water Quality Program

(360) 407-6444

douglas.howie@ecy.wa.gov

Revision History

Date	Revision				
June 2011	Original use-level-designation document				
September 2012	Revised dates for TER and expiration				
January 2013	Modified Design Storm Description, added Revision Table, added maintenance discussion, modified format in accordance with Ecology standard				
December 2013	Updated name of Applicant				
April 2014	Approved GULD designation for Basic, Phosphorus, and Enhanced treatment				
December 2015	Updated GULD to document the acceptance of MWS-Linear Modular Wetland installations with or without the inclusion of plants				
July 2017	Revised Manufacturer Contact Information (name, address, and email)				

PRIORITY DEVELOPMENT PROJECT (PDP) SWQMP

ATTACHMENT 8

Geotechnical Investigation - Palomar Heights Escondido, California

Date: April 17, 2018

Preapared by: Geocon Inc.

Template Date: October 2016 Preparation Date: 6/18/2019

PDP SWQMP - Attachments

GEOTECHNICAL INVESTIGATION

PALOMAR HEIGHTS ESCONDIDO, CALIFORNIA



GEOTECHNICAL ENVIRONMENTAL MATERIALS PREPARED FOR

INTEGRAL PARTNERS FUNDING, LLC ENCINITAS, CALIFORNIA

APRIL 17, 2018 PROJECT NO. G2109-11-02

TABLE OF CONTENTS

1.	PURI	POSE AND SCOPE	1
2.	SITE	AND PROJECT DESCRIPTION	1
۷٠	SIIL	AND I ROJLET BLICKII HOIV	1
3.	GEO	LOGIC SETTING	2
	~ ~ ~ ~		_
4.		AND GEOLOGIC CONDITIONS	
	4.1	Undocumented Fill (Qudf)	
	4.2	Younger Alluvium (Qya)	
	4.3	Older Alluvium (Qoa)	
	4.4	Granitic Rock (Kgd)	3
5.	GRO	UNDWATER	4
_	CEO	LOGIC HAZARDS	4
6.			
	6.1	Faulting	
	6.2	Seismicity	
	6.3	Ground Rupture	
	6.4	Seiches and Tsunamis	
	6.5	Liquefaction Potential	
	6.6	Landslides	
	6.7	Hydroconsolidation	/
7.	CON	CLUSIONS AND RECOMMENDATIONS	8
	7.1	General	
	7.2	Soil and Excavation Characteristics	9
	7.3	Seismic Design Criteria – California Building Code	
	7.4	Temporary Excavations	
	7.5	Grading	
	7.6	Earthwork Grading Factors	13
	7.7	Settlement	14
	7.8	Conventional Shallow Foundation Recommendations – New Structures	14
	7.9	Conventional Shallow Foundation Recommendations – Existing Structures	16
	7.10	Concrete Slabs-On-Grade	17
	7.11	Concrete Flatwork	18
	7.12	Retaining Walls	19
	7.13	Lateral Loading	21
	7.14	Preliminary Pavement Recommendations	
	7.15	Interlocking Decorative Paver Pavement (Permeable and Non-Permeable)	24
	7.16	Site Drainage and Moisture Protection	26
	7.17	Storm Water Management	
	7.18	Grading and Foundation Plan Review	29

TABLE OF CONTENTS (Concluded)

MAPS AND ILLUSTRATIONS

Figure 1, Vicinity Map

Figure 2, Geologic Map (Map Pocket)

Figure 3, Geologic Cross-Section A-A' (Map Pocket)

Figure 4, Wall/Column Footing Dimension Detail

Figure 5, Retaining Wall Loading Diagram

Figure 6, Typical Retaining Wall Drain Detail

Figure 7, Hydrologic Soil Map

APPENDIX A

FIELD INVESTIGATION

Figures A-1 – A-10, Logs of Small-Diameter Borings

APPENDIX B

LABORATORY TESTING (GEOCON, 2003)

Table B-I, Summary of Laboratory Maximum Dry Density and Optimum Moisture Content Test Results

Table B-II, Summary of Laboratory Shear Strength Test Results

Table B-III, Summary of Laboratory Expansion Index Test Results

Table B-IV, Summary of Laboratory Water-Soluble Sulfate Test Results

Table B-V, Summary of Laboratory Resistance Value (R-Value) Test Results

Figures B-1 – B-5, Consolidation Curves

APPENDIX C

GEOTECHNICAL INFORMATION FROM PREVIOUS 1999 DAMES & MOORE REPORT

APPENDIX D

RECOMMENDED GRADING SPECIFICATIONS

LIST OF REFERENCES

- 7.16.2 In the case of basement walls or building walls retaining landscaping areas, a water-proofing system should be used on the wall and joints, and a Miradrain drainage panel (or similar) should be placed over the waterproofing. A perforated drainpipe of schedule 40 or better should be installed at the base of the wall below the floor slab and drained to an appropriate discharge area. Accordion-type pipe is not acceptable. The project architect or civil engineer should provide detailed specifications on the plans for all waterproofing and drainage.
- 7.16.3 Underground utilities should be leak free. Utility and irrigation lines should be checked periodically for leaks, and detected leaks should be repaired promptly. Detrimental soil movement could occur if water is allowed to infiltrate the soil for prolonged periods of time.
- 7.16.4 Landscaping planters adjacent to paved areas are not recommended due to the potential for surface or irrigation water to infiltrate the pavement's subgrade and base course. We recommend that area drains to collect excess irrigation water and transmit it to drainage structures or impervious above-grade planter boxes be used. In addition, where landscaping is planned adjacent to the pavement, we recommend construction of a cutoff wall along the edge of the pavement that extends at least 6 inches below the bottom of the base material.

7.17 Storm Water Management

- 7.17.1 Storm water management devices may be incorporated into the future development of the property in accordance with the 2016 Escondido Storm Water Design Manual. If not properly constructed, there is a potential for distress to improvements and properties located hydrologically down gradient or adjacent to these devices. Factors such as the amount of water to be detained, its residence time, and soil permeability have an important effect on seepage transmission and the potential adverse impacts that may occur if the storm water management features are not properly designed and constructed. We have not performed a hydrogeological study at the site. If infiltration of storm water runoff occurs, downstream properties may be subjected to seeps, springs, slope instability, raised groundwater, movement of foundations and slabs, or other undesirable impacts as a result of water infiltration.
- 7.17.2 The United States Department of Agriculture (USDA), Natural Resources Conservation Services, possesses general information regarding the existing soil conditions for areas within the United States. The USDA website also provides the Hydrologic Soil Group. Table 7.17.1 presents the descriptions of the hydrologic soil groups. If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the

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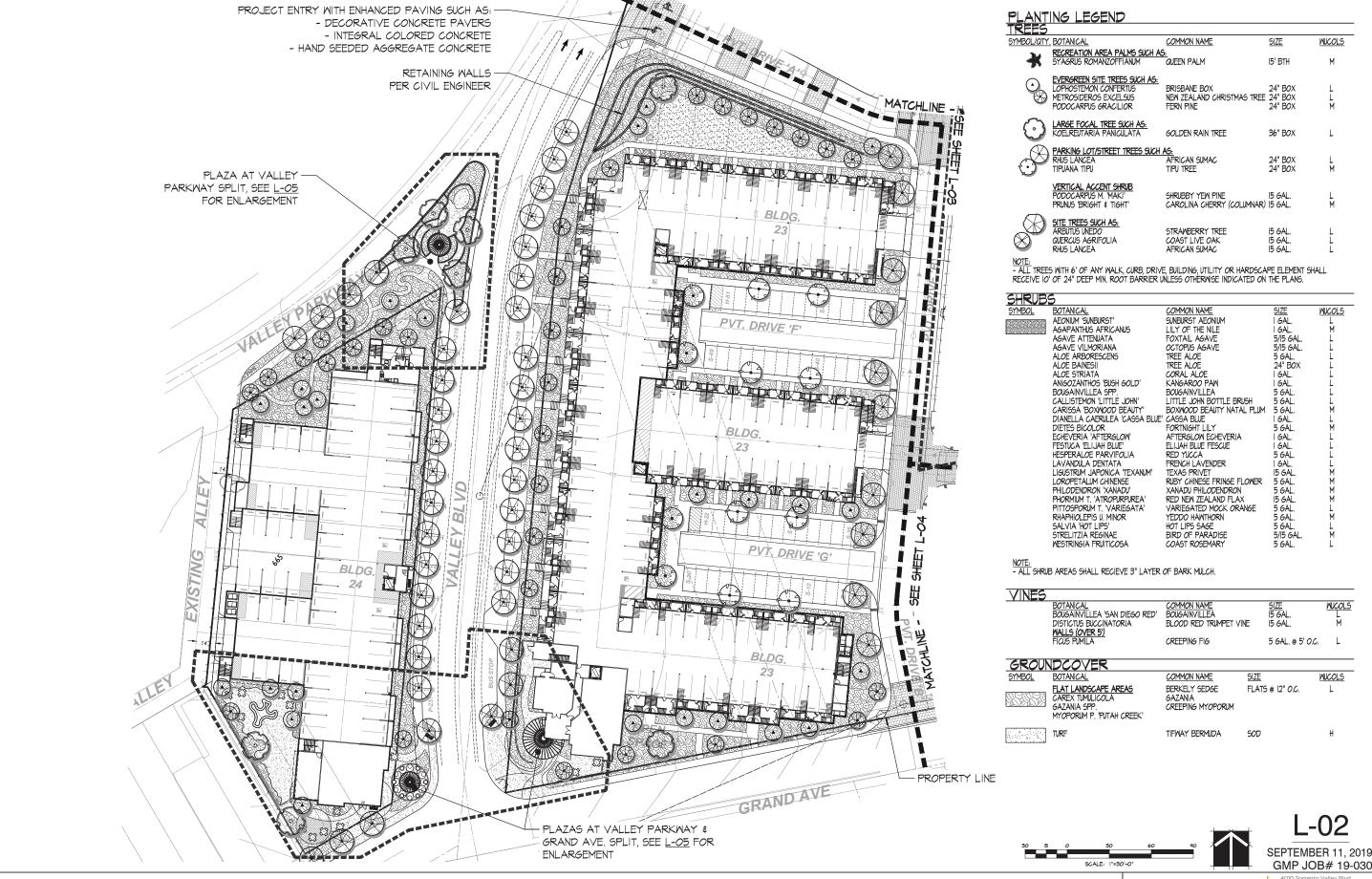
ATTACHMENT 8

Landscape/Site Plan

Prepared by:

GMP Landscape Archtecture & Planning

Template Date: October 2016 Preparation Date: 6/18/2019 PDP SWQMP - Attachments



PALOMAR HEIGHTS: ESCONDIDO, CA

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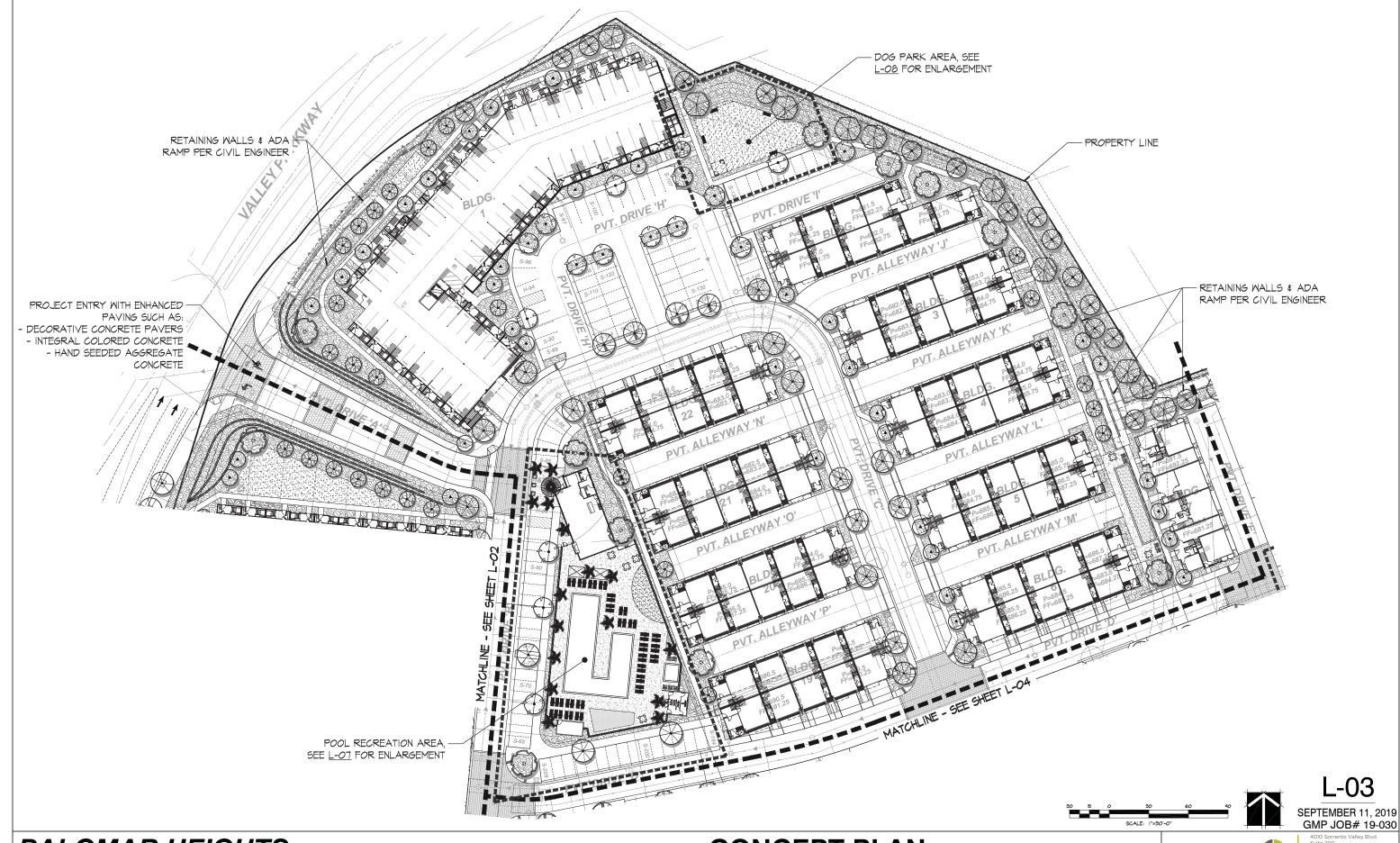
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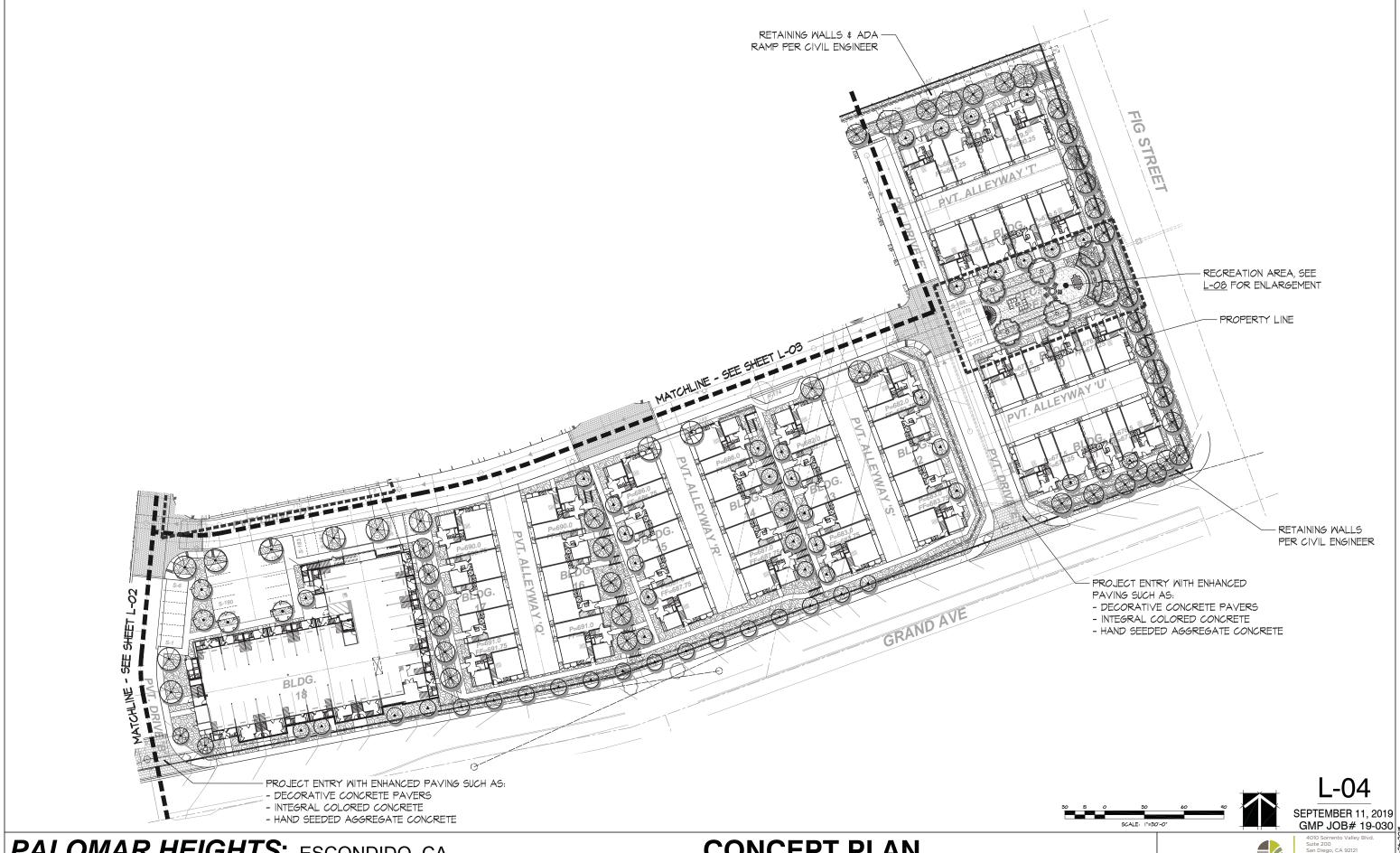
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